

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FAI 90/94	*	COOK	419	201

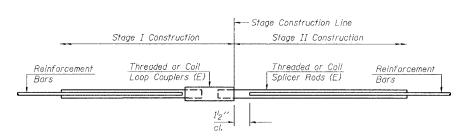
• 2005-008F CONTRACT #62580

PIER NO. 7

FINGER PLATE PLAN AND ELEVATION DETAILS					
REVISIONS No. DATE INITIALS 1	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS	DRAWN BY DATE RLK 5/03 CHECKED BY DATE SDS 5/03			
2 3 4 5	FAI RTE 90/94 DAN RYAN EXPRESSWAY SN 016-1111 (NORTHBOUND) SEC 2005-008F COOK COUNTY	PROJECT No. 4222 - 3			
7 8 9	HOMER L. CHASTAIN & ASSOCIATES CONSULTING ENGINEERS BOSO W. BRYN MAWE AVENUE SUITE 303 CENGOGO, LILLANOIS 60631 832-744-0466 PAX SE2-74-04065	SHEET NO.			

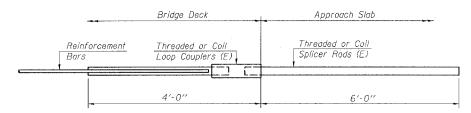
Note: For optional splice detail see flame cutting diagram on Sheet No. 4

STATE OF ILLINOIS



SPLICER DETAIL

Bar Size	No. Assemblies Required	L.ocation
#5	24	At Stage Construction Line in N.B. Deck
, , , , , , , , , , , , , , , , , , , ,		



INTEGRAL ABUTMENT BAR SPLICER ASSEMBLY DETAIL FOR #5 BAR

Min.	Capacity	=	23.0	kip	S		tensi	on	
Min.	Pull-out	St	rength	=	9.	.2	kips	~	tension

DEPARTMENT OF TRANSPORTATION

	The diameter of this part is the same as the diameter of the bar spliced.			-The diameter of this part is equal or larger than the diameter of bar spliced.		
ROLLED THREAD DOWEL BAR						

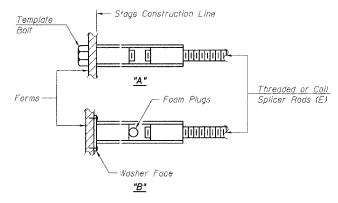
** ONE PIECE



WELDED SECTIONS

BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.

ROUTE NO. SECTION FAI 419 COOK 202

* 2005-008F CONTRACT #62580

of 8 Sheets

Sheet No 8

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity 1.25 x fy x A_t

(Tension in kips) = 1.25 x fy x A_t

Minimum *Pull-out Strength = 1.25 x fs_{allow} x A_t

Where fy = Yield strength of lapped reinforcement bars in ksi. $fs_{allow} = Allowable tensile stress in lapped reinforcement bars in ksi (Service Load)$

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

	BAR SPLIC	ER ASSEMBLI	ES		
		Strength Requirements			
	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension		
#5	2'-0"	23.0	9.2		
#6	2'-7"	33.1	13.3		
#7	3'-5"	45.1	18.0		
#8	4'-6''	58.9	23.6		

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."

PIER NO. 7

BAR SPLICER ASSEMBLY DETAILS						
REVISIONS No. DATE INITIALS	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS	DRAWN BY DATE RLK 5/03 CHECKED BY DATE SDS 5/03				
3 4	FAI RTE 90/94 DAN RYAN EXPRESSWAY SN 016-1111 (NORTHBOUND)	BOOK NUMBER				
5	SEC 2005-008F COOK COUNTY HOMER L. SEGG W REYN MAWR AVENUE	4222-3				
8 9	CHASTAIN & ASSOCIATES CONSULTING ENGINEERS CHCAGO, BLENGIS 60831 SECTION SULTING ENGINEERS SULTE 204 CHCAGO, BLENGIS 60831 SECTION SULTING ENGINEERS	SHEET No.				

* 2005-008F Contract #62580

GENERAL NOTES

Fasteners shall be high strength bolts. Bolts $^34''$ ϕ , open holes $^{13}_{16}$ 17 ϕ . unless otherwise noted.

Finger Plate Expansion Joints shall be assembled in the proper position with the ends in place and shall be left assembled for shop inspection.

The finger plates shall be flame cut as provided in Article 505.04(k) of the

Standard Specifications.

Field welding of construction accessories will not be permitted to beams or

Reinforcement bars shall conform to the requirements of AASHTO M 31 or M 322 Grade 60.

Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price for the work.

Prior to pouring the new concrete deck, all loose rust, loose mill scale, and other loose potentially detrimental foreign material shall be removed from the surfaces of the beams or girders in contact with concrete. The cost of this work will be included in the pay item covering removal of the existing concrete. All heavy rust and other tightly adhered potentially detrimental foreign matter shall also be removed from the surfaces of the beams or girders in contact with concrete. Tightly adhered paint may remain unless otherwise noted. This removal shall be accomplished by methods that will not damage the steel. The cost of this work will be paid for according to Article 109.04.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. All Construction joints shall be bonded.

Existing design and fabrication plans are available upon request. All structural steel shall be AASHTO M270 Grade 50 unless noted

otherwise. Calculated weight = 48,120 lbs.

Tapered shims shall be added under the stools as required by the Engineer to make a smooth finger joint.

Joint openings shall be adjusted according to Article 503.10(c) of the standard specifications when the deck is poured at an ambient temperature other than 50°F. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

Traffic to be maintained during construction, see Roadway Plans for Stage Construction Details.

(SN 016-1113) (SN 016-1114) SN 016-1114)
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KEY PLAN DAN RYAN EXPRESSWAY (I-90/I-94) VIADUCT COOK COUNTY, ILLINOIS



DESIGN STRESSES

f'c = 3500 psi

fy = 60,000 psi (Reinf)

fy = 50,000 psi (M270 Grade 50)

BILL OF MATERIAL					
ITEM	UNIT	TOTAL			
Concrete Removal	Cu Yd	22.1			
Concrete Superstructure	Cu Yd	25.1			
Erecting Structural Steel	L.S.	0.18			
Reinforcement Bars, Epoxy Coated	Pound	3490			
Mechanical Splice	Each	174			
Bar Splicers	Each	48			
Erecting Fabric Reinforced Elestomeric Trough	Foot	94			

ILLINOIS DEPARTMENT OF TRANSPORTATION KEY PLAN

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
OVER SOUTH BRANCH - CHICAGO RIVER
COOK COUNTY
S.N. 016-1113 - PIER 1

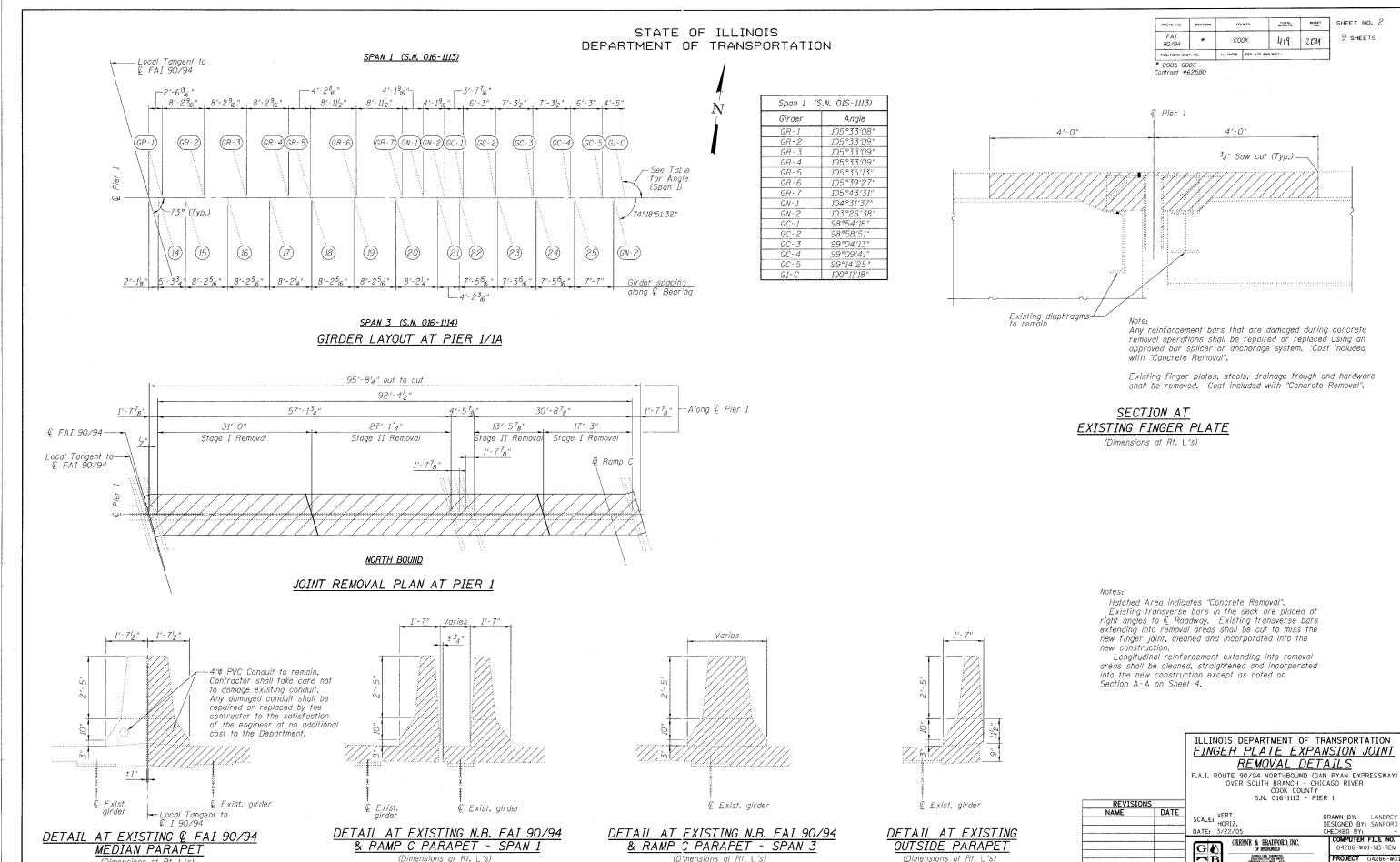
REVISIONS DATE

SCALE: VERT. HORIZ. DATE: 3/22/0

GREENE & BRADFORD, INC.

CHECKED BY: COMPUTER FILE NO. 04286-W01-NB-KEY PROJECT 04286-W01 8/3/05-MML

DRAWN BY; LANDREY DESIGNED BY: SANFORD



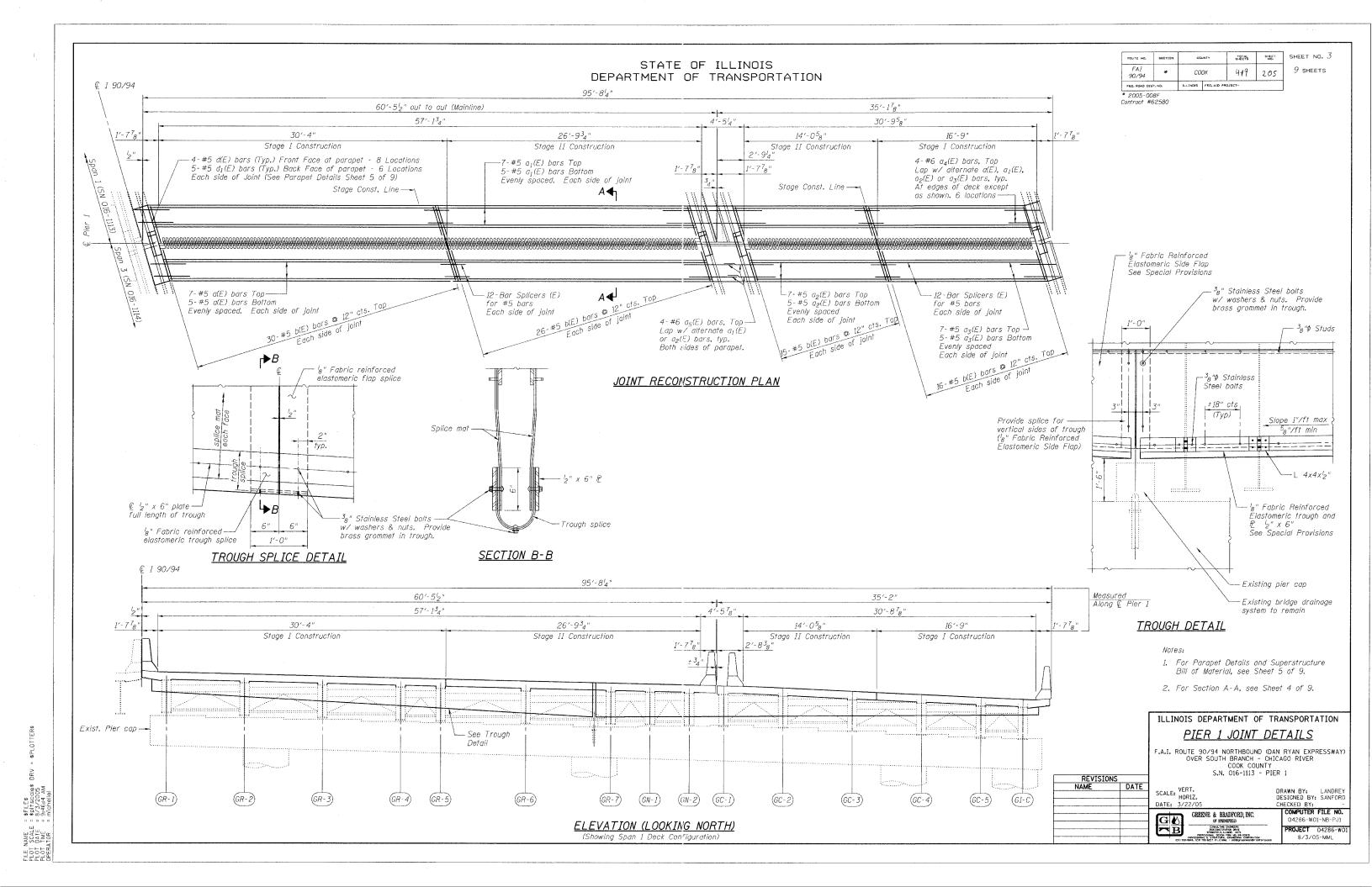
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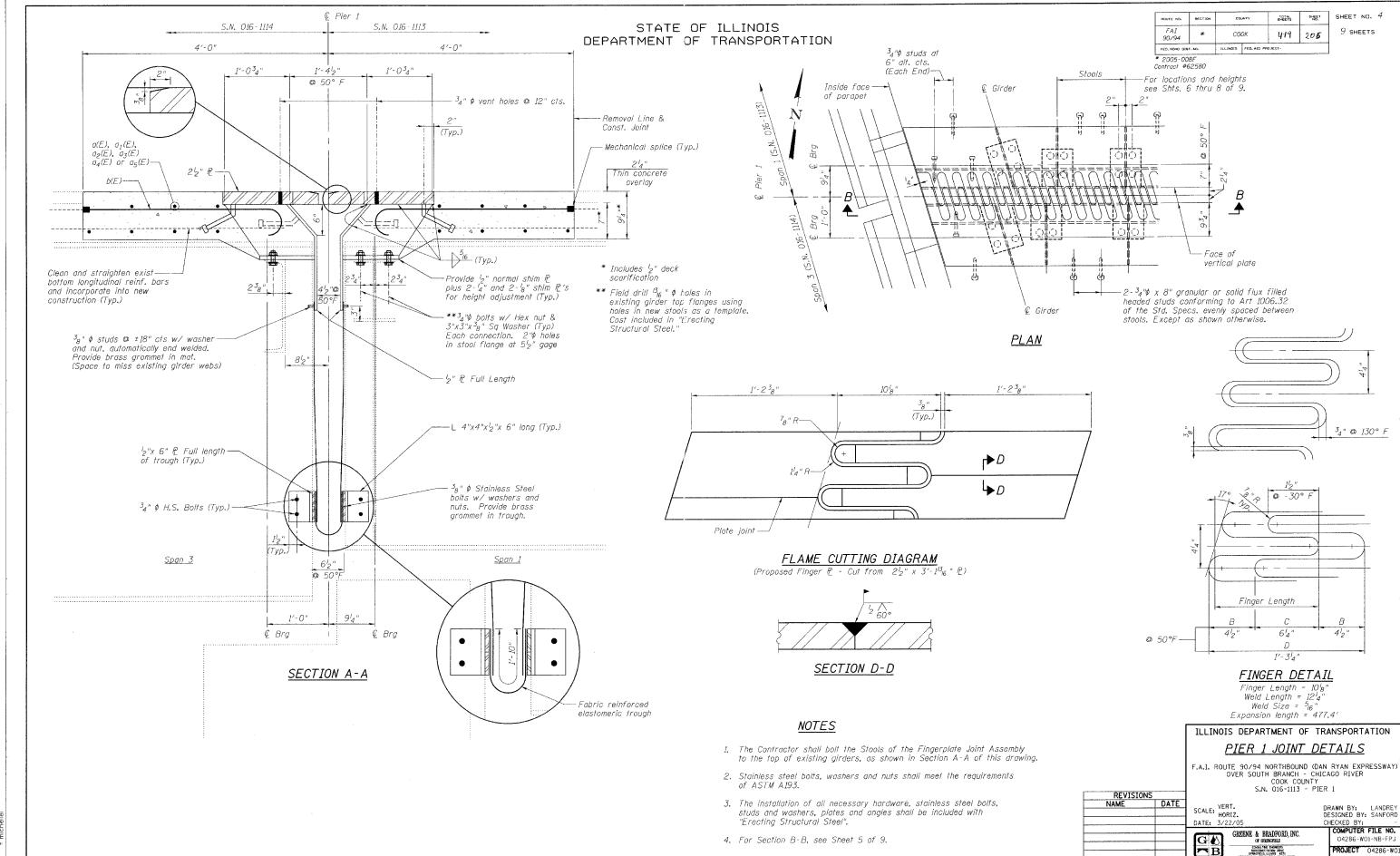
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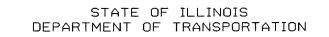
(Dimensions at Rt. L's)





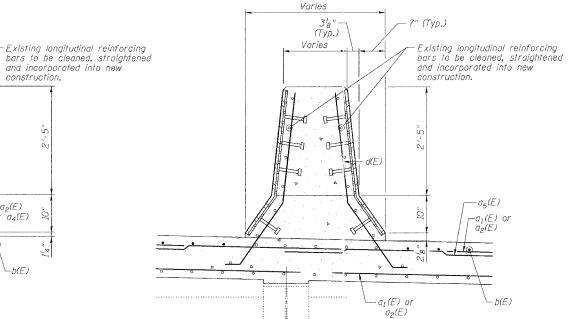
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PROJECT 04286-W01 8/3/05-MML





* 2005-008F Contract #62580



2'-2" _ ³4" Plate 3₄" ¢ Countersunk bolts at ± 12" cts. ³₄"\$ x 6" granular 45" @ 50° or solid flux filled headed studs conforming to Article 1006.32 of the Standard Specifications. 2'-65

SECTION C-C

DETAIL AT N.B. FAI 90/94 & RAMP C PARAPET - SPAN 1 (Dimensions at Rt. L's)

♀ Girder

Varies

d(E)

 $a_1(E)$ or $a_4(E)$ -

b(E)

and incorporated into new

construction.

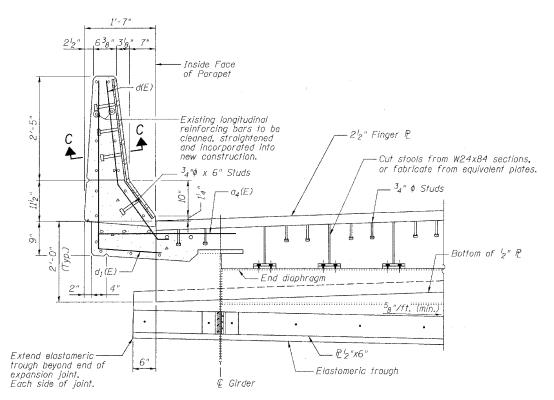
or $a_4(E)$

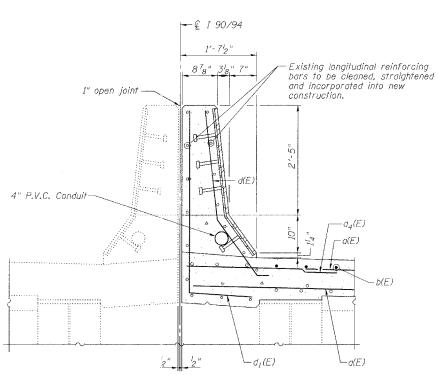
 $-d_1(E)$

 $-a_2(E)$

DETAIL AT N.B. FAI 90/94 & RAMP E PARAPET - SPAN 3 (Dimensions at Rt. L's)

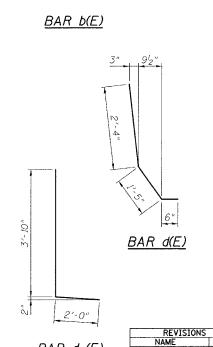
⊈ Girder





DETAIL AT MEDIAN PARAPET

(Looking North)



BAR d₁(E)

3'-3"

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
σ(E)	24	#5	30'-9"	
11(E)	24	#5	27'-3"	· ———
12(E)	24	#5	14'-6"	
13(E)	24	#5	17'-3"	
14(E)	24	#6	4'-6"	
75(E)	8	#6	5′-6"	
b(E)	174	#5	3'-10"	
t(E)	32	#5	4'-3"	
11(E)	30	#5	5′- <i>1</i> 0"	
Reinforcement Bars. Epoxy Coated			Pound	3490
Concrete Superstructure			Cu. Yd.	25.1
Concrete Removal			Cu. Yd.	22.1
Bar Splicers			Each	48
<i>lechani</i>	cal Splice		Each	174
Painfara	ing hare	dociana	tod (E) ch	all bo

Reinforcing bars designated (E) shall be epoxy coated.

ILLINOIS DEPARTMENT OF TRANSPORTATION PARAPET DETAILS

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
OVER SOUTH BRANCH - CHICAGO RIVER
COOK COUNTY
S.N. 016-1113 - PIER 1

DATE GN

B

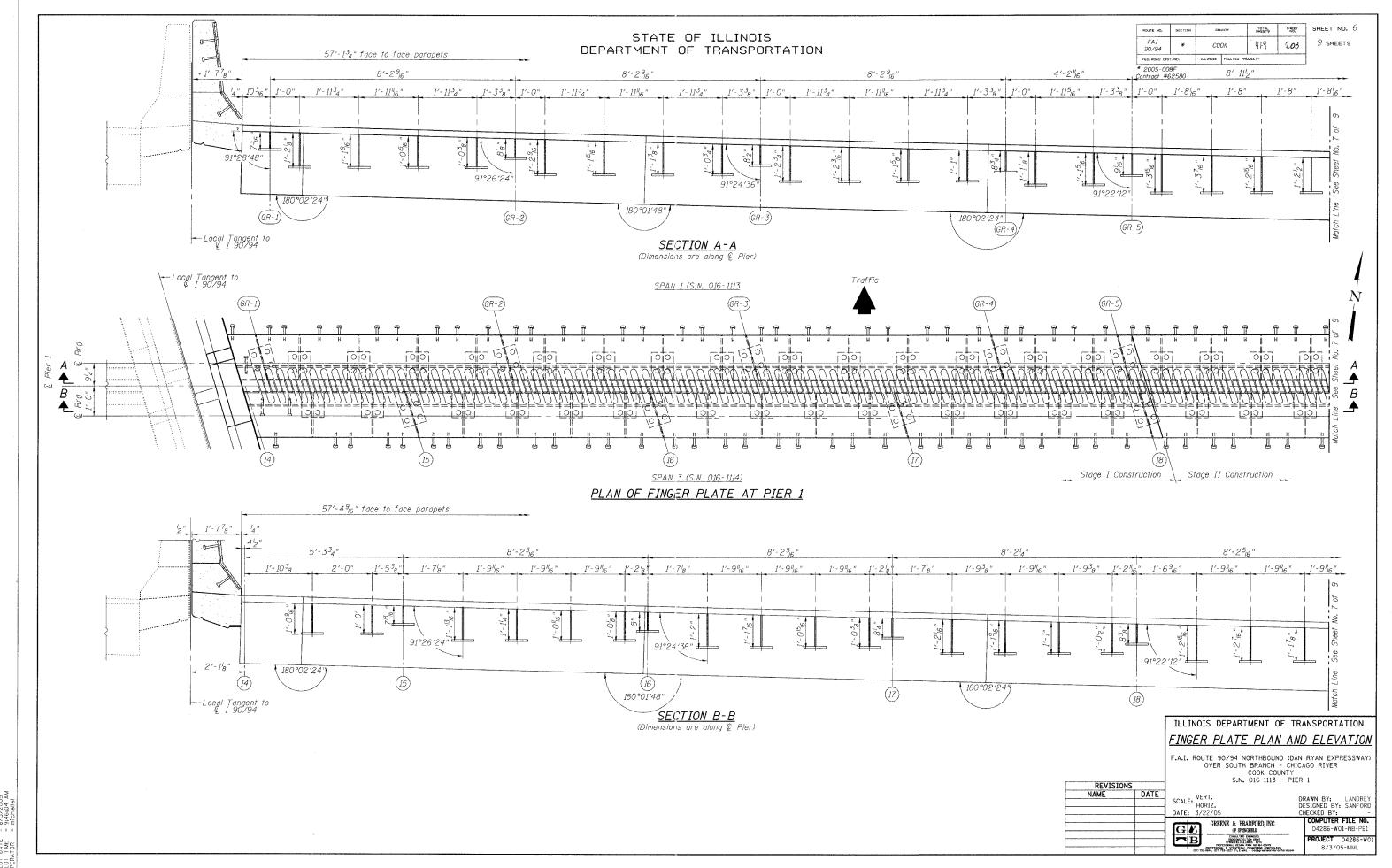
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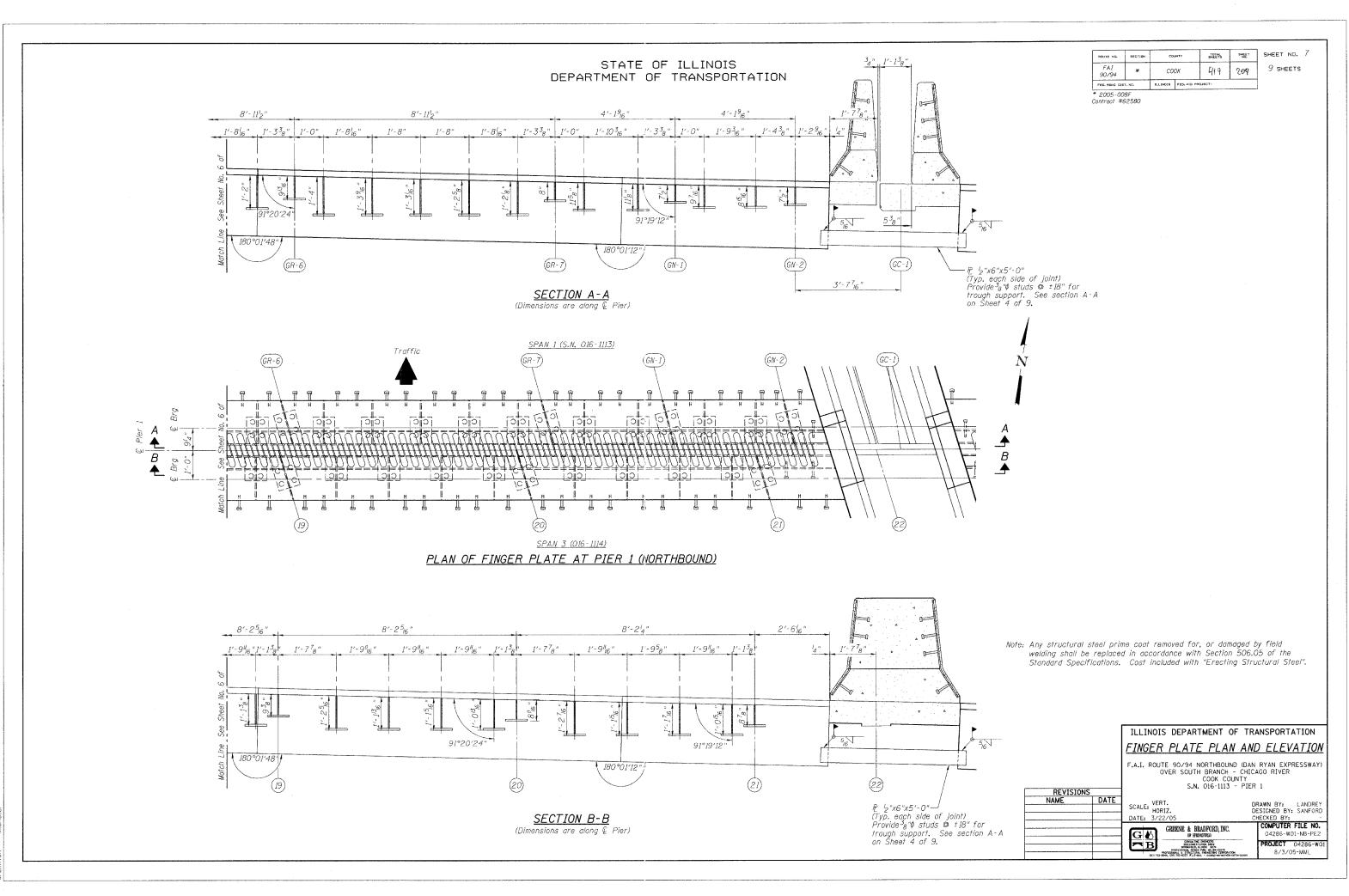
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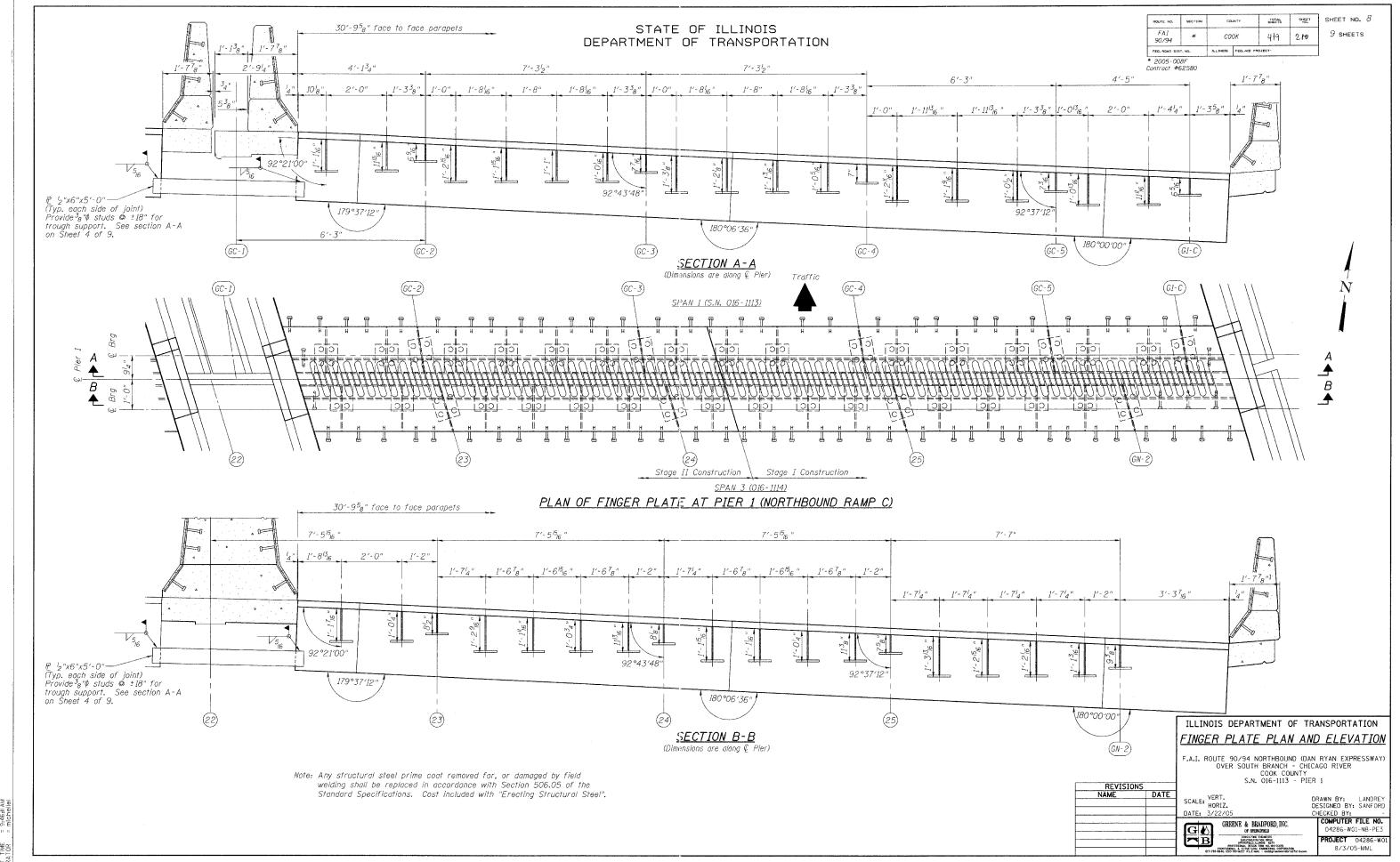
SECTION B-B

(For location of Section B-B, See Sheet 4 of 9)

SCALE DATE TIME







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OT DATE = 8/3/200
OT TIME = 9:46:II AM
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SHEET NO. 9

9 SHEETS

* 2005-008F Contract #62580

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval. Approval

shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity (Tension in kips) = $1.25 \times fy \times A_t$

Minimum *Pull-out Strength = 1.25 x fs_{allow} x A_t (Tension in kips)

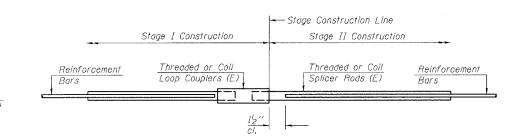
Where fy = Yield strength of lapped reinforcement bars in ksi.

fs_{allow}= Allowable tensile stress in lapped reinforcement bars in ksi (Service Load) A_t = Tensile stress area of lapped reinforcement bars.

* = 28 day concrete

BAR SPLICER ASSEMBLIES						
		Strength Requirements				
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension			
#4	1'-8''	14.7	5.9			
#5	2'-0''	23.0	9.2			
#6	2'-7"	33.1	<i>1</i> 3.3			
#7	3′-5′′	45.1	18.0			
#8	4'-6''	58.9	23.6			
#9	5′-9′′	75.0	30.0			
#10	7′-3′′	95.0	38.0			
#11	9′-0′′	117.4	46.8			

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



STANDARD

REVISIONS NAME

Bar Size	No. Assemblies Required	Location
#5	48	Deck

ILLINOIS DEPARTMENT OF TRANSPORTATION BAR SPLICER ASSEMBLY DETAILS

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY) OVER SOUTH BRANCH - CHICAGO RIVER COOK COUNTY S.N. 016-1113 - PIER 1

SCALE: VERT. HORIZ. DATE: 3/22/0 G N B

GREENE & BRADFORD, INC.

DRAWN BY: LANDREY DESIGNED BY: SANFORD CHECKED BY: -COMPUTER FILE NO. 04286-W01-NB-BSD PROJECT 04286-WO 8/3/05-MML

Foam Plugs

Threaded or Coil

Splicer Rods (E)



ROLLED THREAD DOWEL BAR

** ONE PIECE - Wire Connector **WELDED SECTIONS**

BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



<u>"A"</u>

- Stage Construction Line

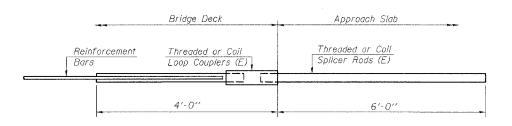
Template

Forms -

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

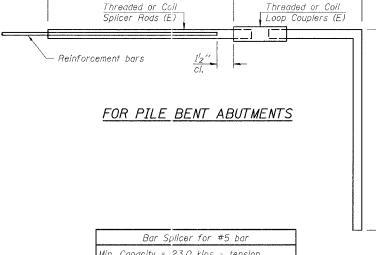
(E): Indicates epoxy coating.

6'-0"



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tensio No. Required = N/A



Approach slab

Abutment hatch block

Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tension No. Required = N/A

JAME SCALE DATE TIME

WENTWORTH AVE. PIER NO. 26 LOCATION (SN 016-1116) (SN 016-1117) HIELDS AVE. - STEWART AVE. 016-0137) WALLACE ST. (SN 016-1113) SN 016-1114

KEY PLAN DAN RYAN EXPRESSWAY (1-90/1-94) VIADUCT COOK COUNTY, ILLINOIS



DESIGN STRESSES

f'c = 3500 psi fy = 60,000 psi (Reinf) fy = 50,000 psi (M270 Grade 50)

BILL OF MATERIAL			
ITEM	UNIT	TOTAL	
Concrete Removal	Cu Yd	15.3	
Concrete Superstructure	Cu Yd	18.3	
Erecting Structural Steel	L.S.	0.12	
Reinforcement Bars, Epoxy Coated	Pound	2500	
Mechanical Splice	Each	130	
Bar Splicers	Each	24	
Erecting Fabric Reinforced Elastomeric Trough	Foot	68	

GENERAL NOTES

Fasteners shall be high strength bolts. Bolts 3_4 " ϕ , open holes ${}^{13}_{16}$ " ϕ , unless otherwise noted.

Finger Plate Expansion Joints shall be assembled in the proper position with the ends in place and shall be left assembled for shop inspection.

The finger plates shall be flame cut as provided in Article 505.04(k) of the Standard Specifications.

Field welding of construction accessories will not be permitted to beams or

Reinforcement bars shall conform to the requirements of AASHTO M 31 or M 322 Grade 60.

Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price for the work.

Prior to pouring the new concrete deck, all loose rust, loose mill scale, and other loose potentially detrimental foreign material shall be removed from the surfaces of the beams or girders in contact with concrete. The cost of this work will be included in the pay item covering removal of the existing concrete. All heavy rust and other tightly adhered potentially detrimental foreign matter shall also be removed from the surfaces of the beams or girders in contact with concrete. Tightly adhered paint may remain unless otherwise noted. This removal shall be accomplished by methods that will not damage the steel. The cost of this work will be paid for according to Article 109.04.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. All Construction joints shall be bonded.

Existing design and fabrication plans are available upon request. All structural steel shall be AASHTO M270 Grade 50 unless noted otherwise. Calculated weight = 32,260 lbs.

Tapered shims shall be added under the stools as required by the Engineer to make a smooth finger joint.

Joint openings shall be adjusted according to Article 503.10(c) of the standard specifications when the deck is poured at an ambient temperature other than 50°F.

Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

Traffic to be maintained during construction, see Roadway Plans for Stage Construction Details.

> ILLINOIS DEPARTMENT OF TRANSPORTATION KEY PLAN

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
OVER 28th ST. TO S. STEWART ST.
COOK COUNTY S.N. 016-1117 - PIER 26

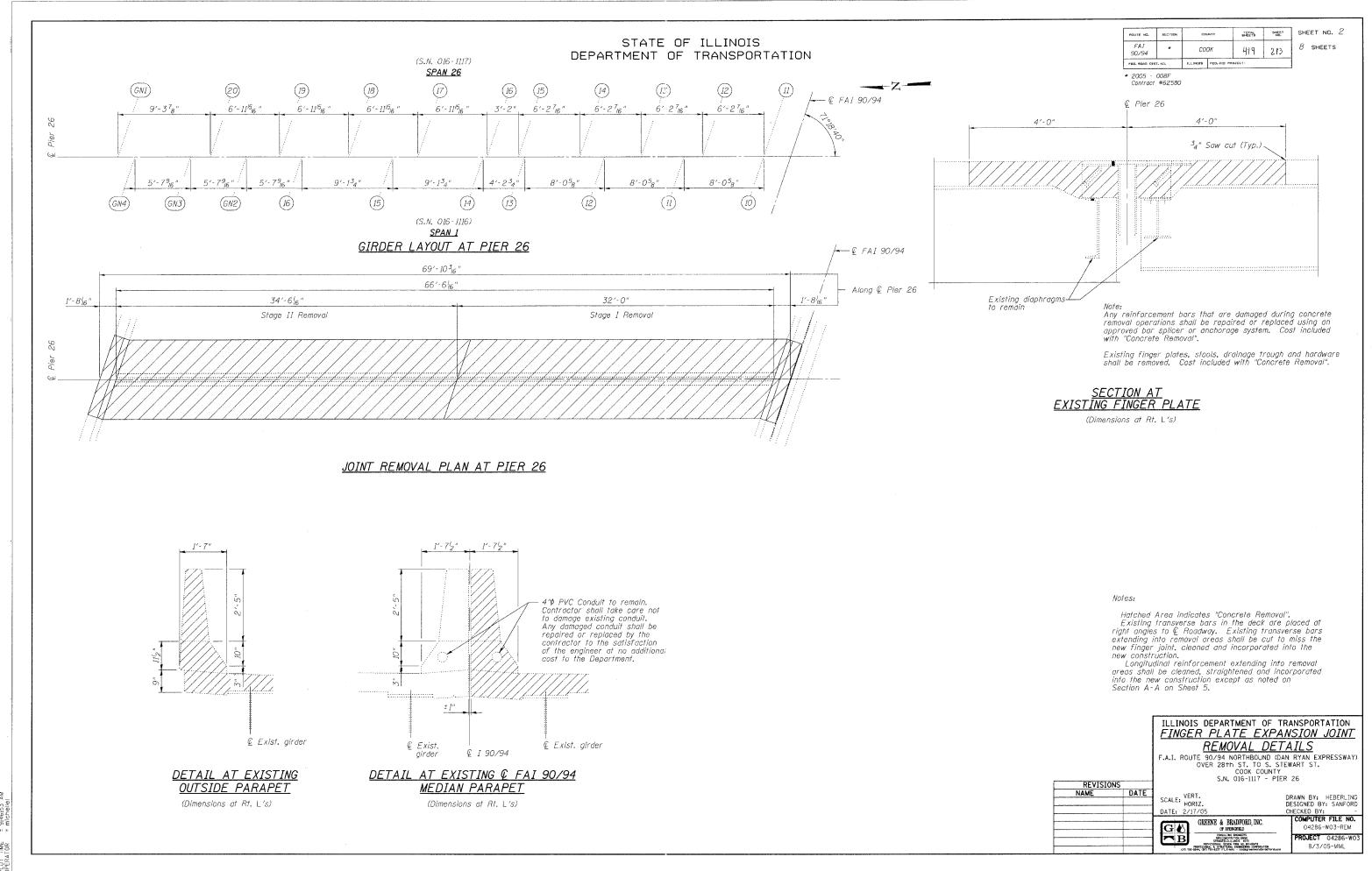
REVISIONS NAME \neg B

SCALE: VERT. DATE: 2/17/05 GW

GREENE & BRADFORD, INC.

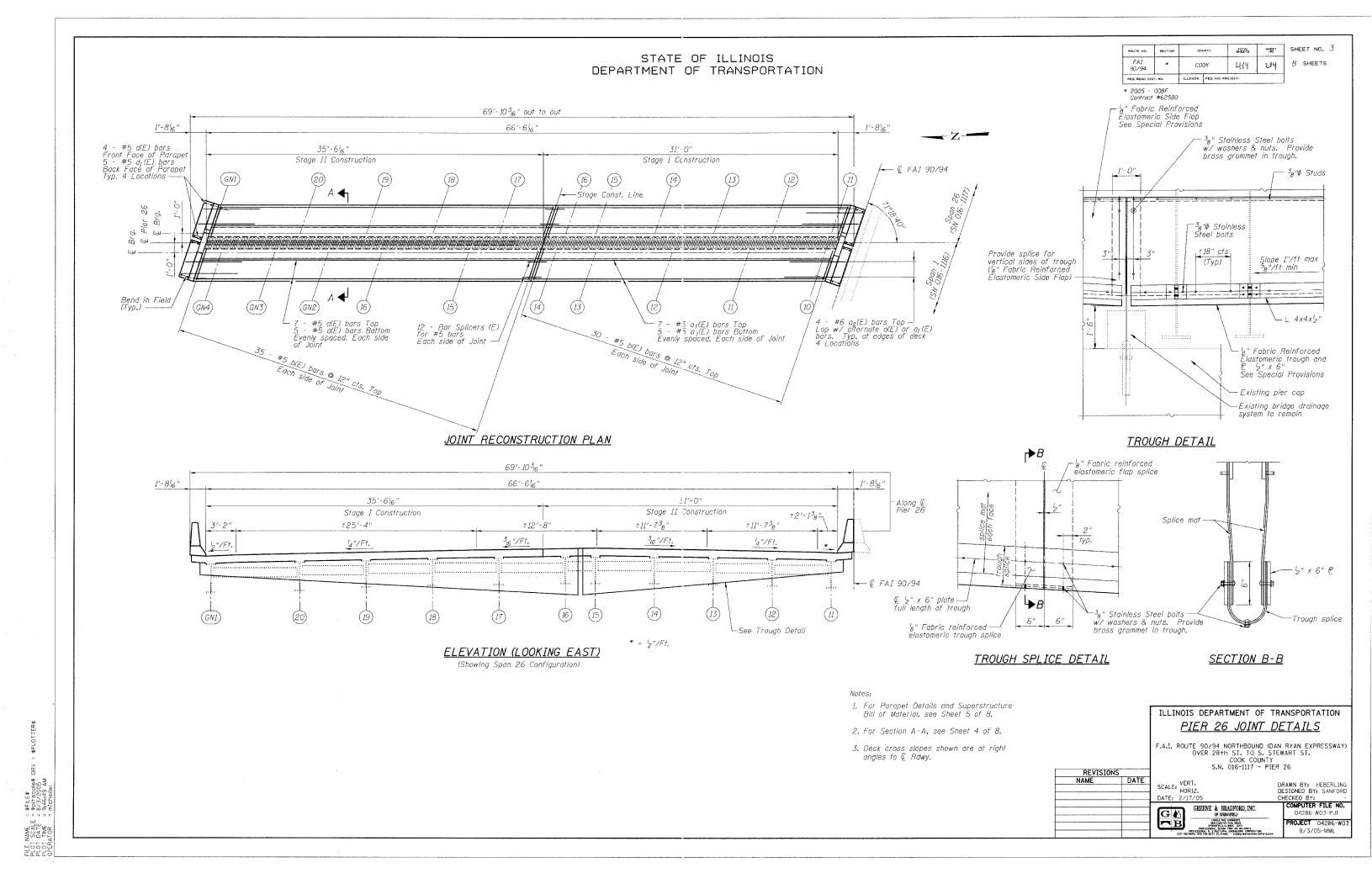
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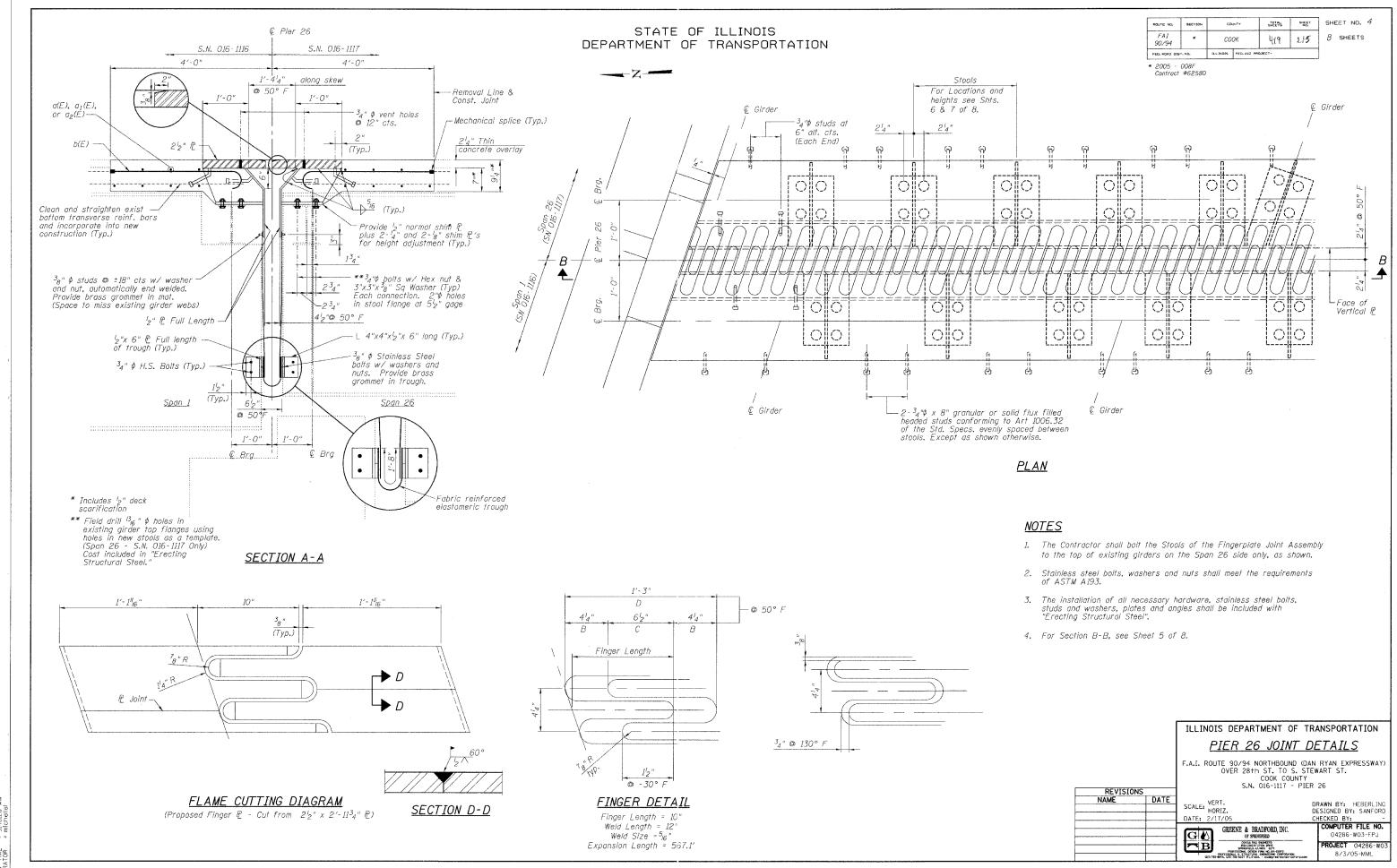
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SCALE = \$pi*zscale\$ DRV = \$PLOTTER\$
DATE = 87/3/2005
IME = 94/3/203
ATOR = michelel



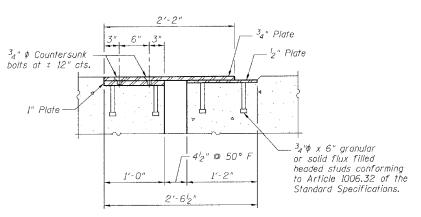


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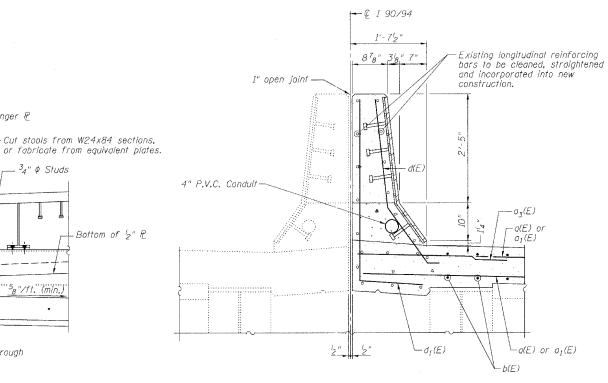
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	co.	JNTY	TOTAL SHEETS	SHEET NO.	SH	EET NO. 5
FAI 90/94	*	co	OK .	419	216	8	SHEETS
FED. ROAD DIS	f, Nö.	ILLINOIS	FED. A(D) PR	OJECT-			

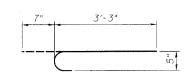
* 2005 - 008F Contract #62580



SECTION C-C



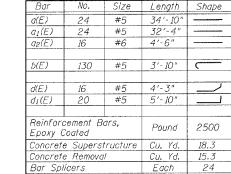
DETAIL AT MEDIAN PARAPET



BAR b(E)

 $-a_3(E)$ $\vdash a(E)$ or

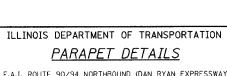
-a(E) or $a_1(E)$



BILL OF MATERIAL

Reinforcing bars designated (E) shall be epoxy coated.

Bars indicated thus 1x2- #5 etc. Indicates 1 line of bars with 2 lengths per line.



Each

130

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
OVER 28th ST. TO S. STEWART ST.
COOK COUNTY S.N. 016-1117 - PIER 26

SCALE: VERT. HORIZ. DATE: 2/17/05 DRAWN BY: HEBERLING DESIGNED BY: SANFORD COMPUTER FILE NO.

04286-W03-PD PROJECT 04286-W0

8/3/05-MML

GREENE & BRADFORD, INC. B

SECTION B-B (For location of Section B-B, See Sheet 4 of 8)

@ Girder

End diaphragm

~__P2"x6"

— Elastomeric trough

638"318" 7"

Extend elastomeric trough beyond end of

expansion joint. Each side of joint.

d(E)

Inside Face

reinforcing bars to be cleaned, straightened

and incorporated into new construction.

-2" Finger ₽

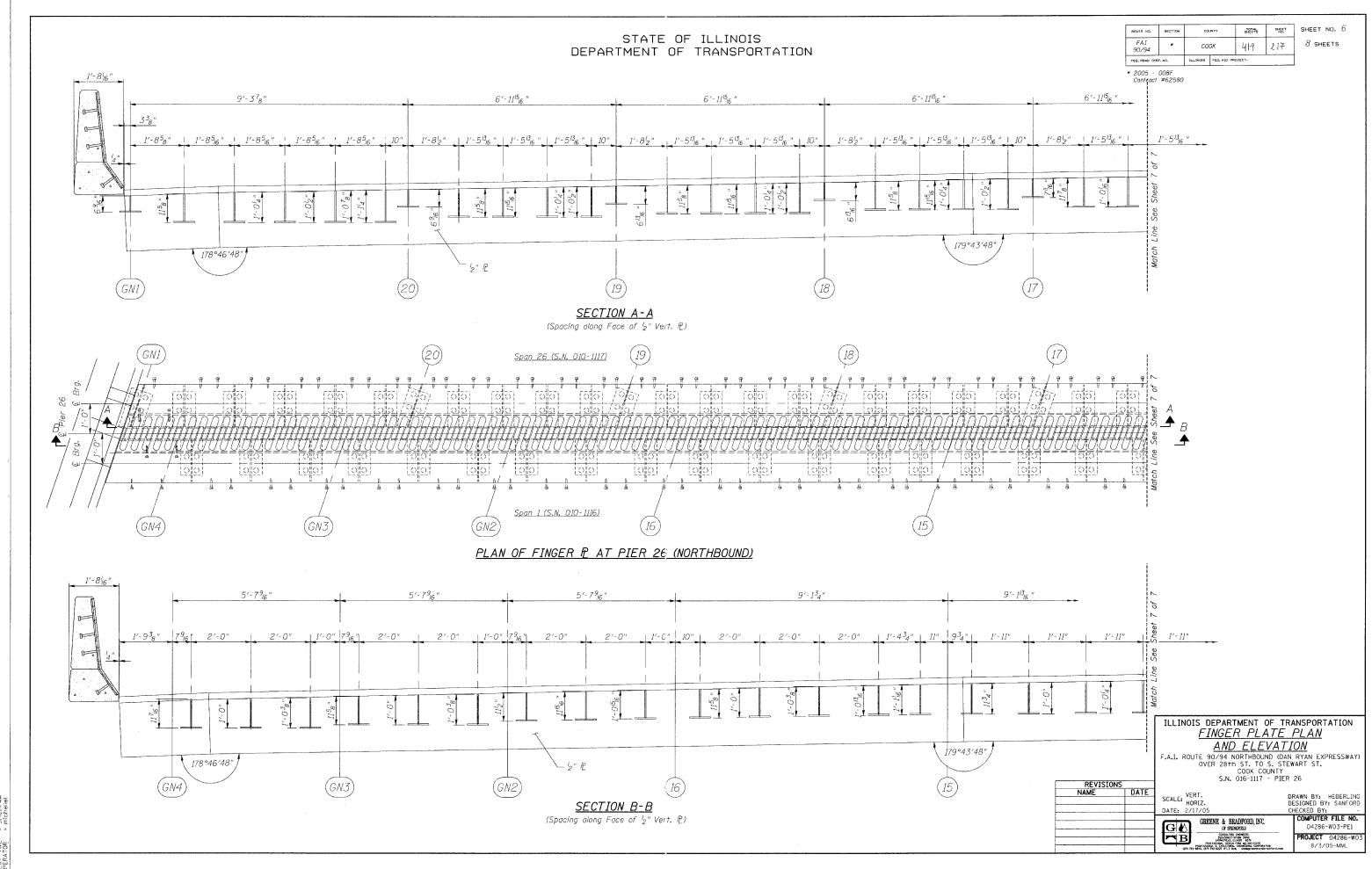
³₄" ¢ Studs

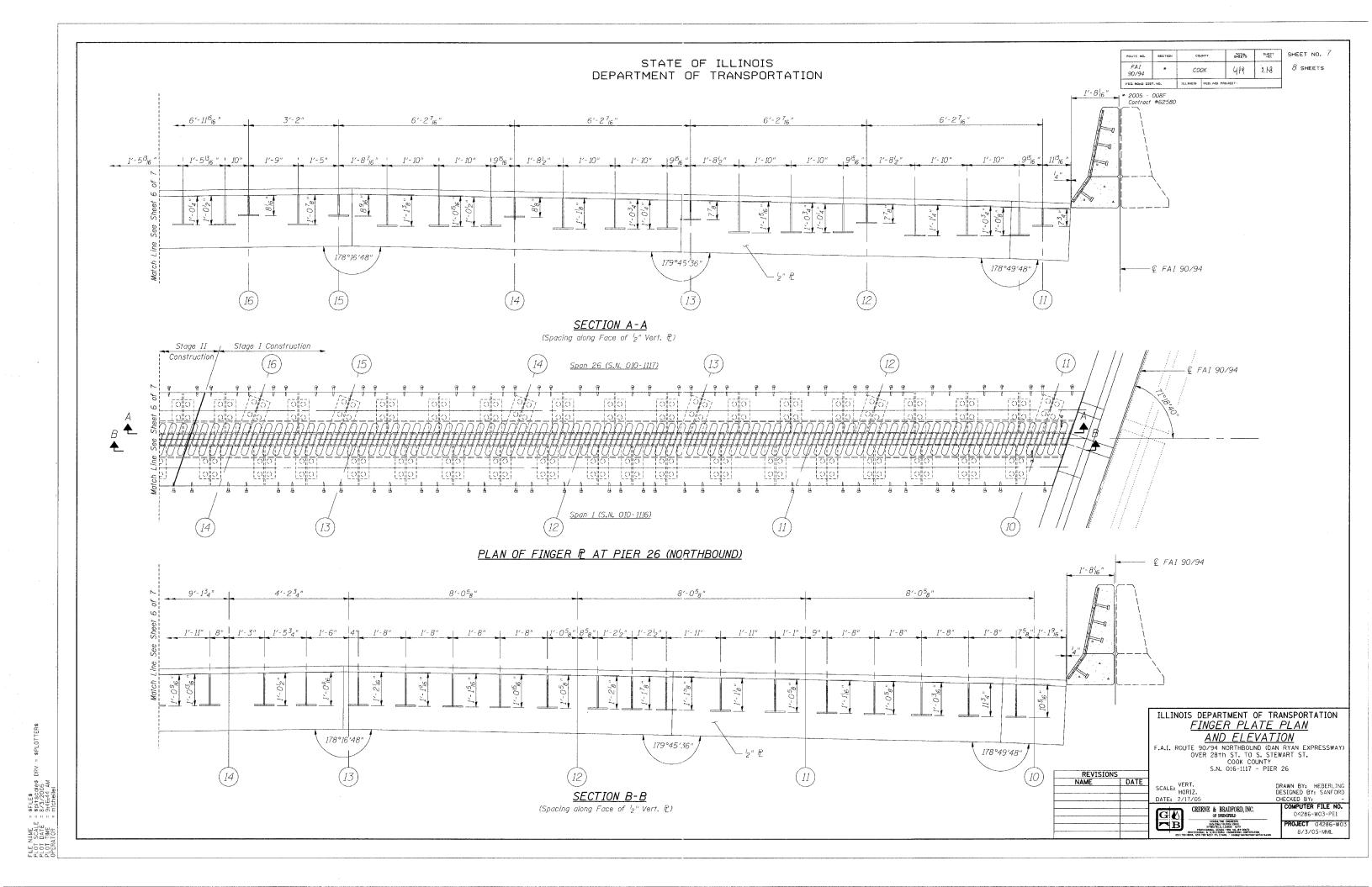
5₈"/ft. (min.)

BAR d(E) 2'-0"

 $BAR d_1(E)$

REVISIONS NAME





SHEET NO. 8 8 SHEETS

* 2005 - 008F Contract #62580

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum "Pull-out Strength = 1.25 x fs allow x A_t

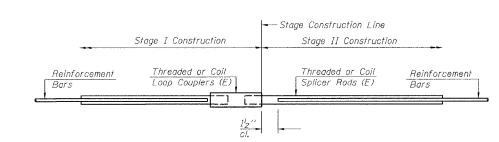
Minimum "Pull-out Strength = 1.25 x fs allow x A_t (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

fs_{allow} = Allowable tensile stress in lapped reinforcement bars in ksi (Service Load) A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

	BAR SPLIC	ER ASSEMBLI	ES	
		Strength Requirements		
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length		Min, Pull-Out Strength kips - tension	
#4	1'-8''	14.7	5.9	
#5	2'-0"	23.0	9.2	
#6	2'-7''	33.1	13.3	
#7	3′-5″	45.1	<i>18.0</i>	
#8	4'-6''	58,9	23.6	
#9	5′-9′′	75.0	30.0	
#10	7′-3′′	95.0	38.0	
#]]	9′-0′′	117.4	46.8	

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



STANDARD

Bar Size	No. Assemblies Required	Location
#5	24	Deck

ILLINOIS DEPARTMENT OF TRANSPORTATION BAR SPLICER ASSEMBLY DETAILS

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY) OVER 28th ST. TO S. STEWART ST. COOK COUNTY S.N. 016-1117 - PIER 26

DATE SCALE: VERT. HORIZ. DATE: 2/17/05

GW

DRAWN BY: HEBERLING DESIGNED BY: SANFORD CHECKED BY: -GREENE & BRADFORD, INC.

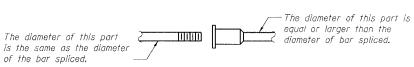
COMPUTER FILE NO. 04286-W03-BSD PROJECT 04286-WC 8/3/05-MML

DEPARTMENT OF TRANSPORTATION

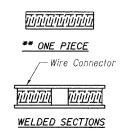
Foam Plugs

Threaded or Coil

Splicer Rods (E)



ROLLED THREAD DOWEL BAR



BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



<u>"A "</u>

- Stage Construction Line

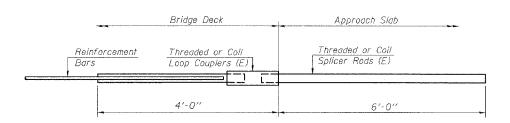
Template

Forms-

"A": Set bar splicer assembly by means of a template bolt. "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

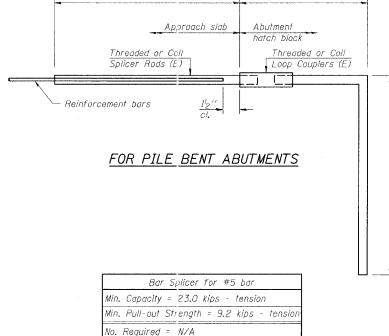
(E): Indicates epoxy coating.

6'-0"



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

	Bar	Splicer	for #	5 bar	
Min.	Capacity	= 23.0	kips -	tension	
Min.	Pull-out	Strength	= 9.2	kips -	tension



10-22-04

BSD-1

* 2005 - 008F Contract #62580

WENTWORTH AVE. PIER NO. C10 LOCATION (SN 016-1113) (SN 016-1113)

KEY PLAN DAN RYAN EXPRESSWAY (I-90/I-94) VIADUCT COOK COUNTY, ILLINOIS

SN 016-1114



DESIGN STRESSES

f'c = 3500 psi fy = 60,000 psi (Reinf)

fy = 50,000 psi (M270 Grade 50)

BILL OF MATERIAL			
ITEM	UNIT	TOTAL	
Concrete Removal	Cu Yd	9.4	
Concrete Superstructure	Cu Yd	11,2	
Erecting Structural Steel	L.S.	0.05	
Reinforcement Bars, Epoxy Coated	Pound	1600	
Mechanical Splice	Each	80	
Bar Splicers	Each	24	
Erecting Fabric Reinforced Elastomeric Trough	Foot	39	

GENERAL NOTES

Fasteners shall be high strength bolts. Bolts ${}^{3}_{4}$ " ϕ , open holes ${}^{13}_{16}$ " ϕ , unless otherwise noted.

Finger Plate Expansion Joints shall be assembled in the proper position with the ends in place and shall be left assembled for shop inspection.

The finger plates shall be flame cut as provided in Article 505.04(k) of the Standard Specifications.

Field welding of construction accessories will not be permitted to beams or

Reinforcement bars shall conform to the requirements of AASHTO M 31 or M 322 Grade 60.

Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price for the work.

Prior to pouring the new concrete deck, all loose rust, loose mill scale, and other loose potentially detrimental foreign material shall be removed from the surfaces of the beams or girders in contact with concrete. The cost of this work will be included in the pay item covering removal of the existing concrete. All heavy rust and other tightly adhered potentially detrimental foreign matter shall also be removed from the surfaces of the beams or girders in contact with concrete. Tightly adhered paint may remain unless otherwise noted. This removal shall be accomplished by methods that will not damage the steel. The cost of this work will be paid for according to Article 109.04.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. All Construction joints shall be bonded.

Existing design and fabrication plans are available upon request. All structural steel shall be AASHTO M270 Grade 50 unless noted otherwise. Calculated weight = 14,440 lbs.

Tapered shims shall be added under the stools as required by the Engineer to make a smooth finger joint.

Joint openings shall be adjusted according to Article 503.10(c) of the standard specifications when the deck is poured at an ambient temperature other than 50°F. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

Traffic to be maintained during construction, see Roadway Plans for Stage Construction Details.

ILLINOIS DEPARTMENT OF TRANSPORTATION

KEY PLAN

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY) OVER SOUTH BRANCH - CHICAGO RIVER COOK COUNTY S.N. 016-1113 - PIER C10

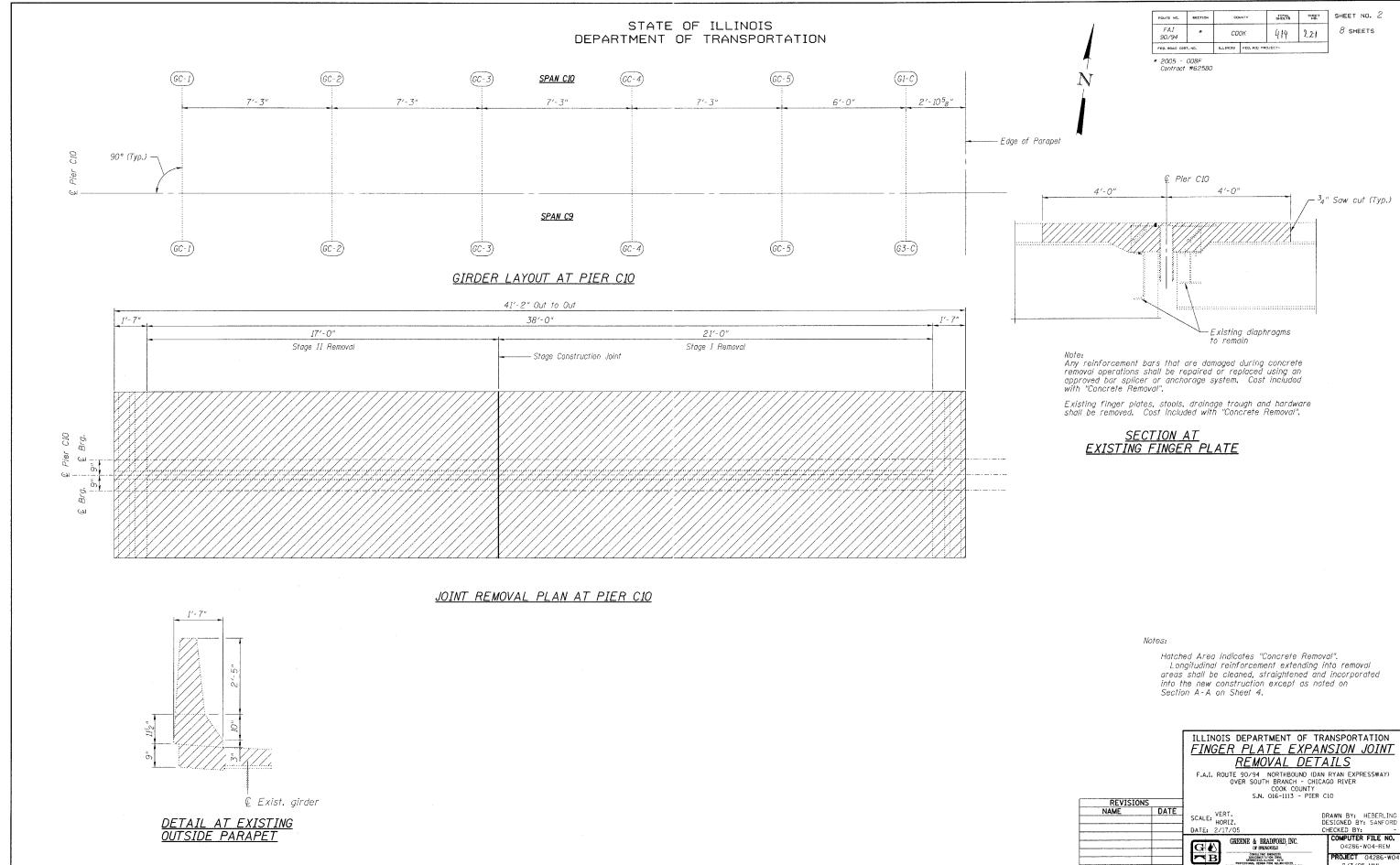
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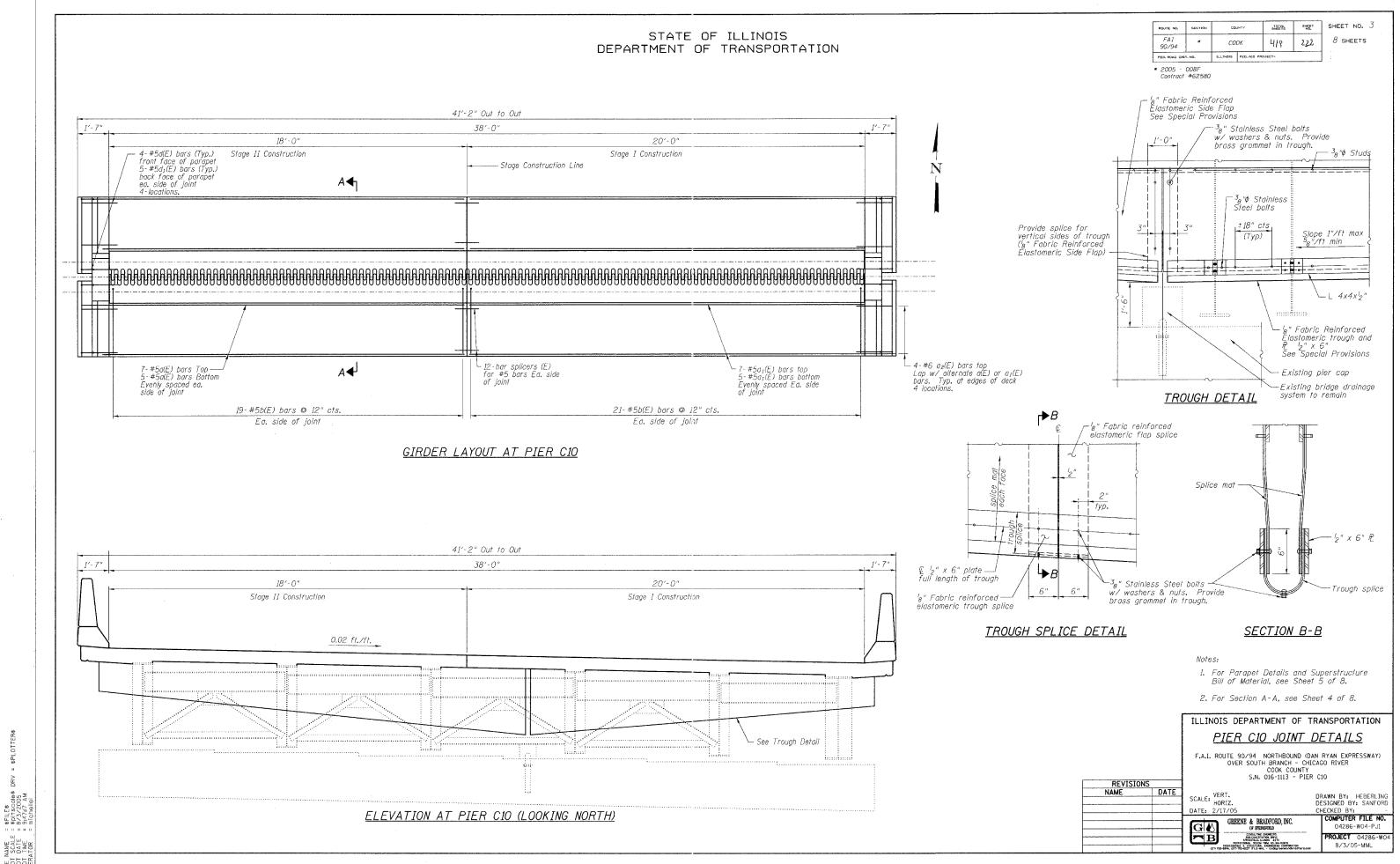
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8/3/05-MML

GREENE & BRADFORD, INC. rB



04286-W04-REM PROJECT 04286-WO 8/3/05-MML



STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

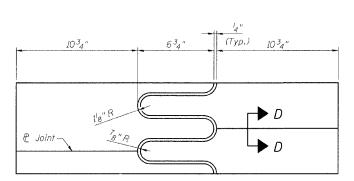
SHEET NO. 4 SHEET NO. ROUTE NO. TOTAL FAI 90/94 8 sheets COOK 419 223

* 2005 - 008F Contract #62580

Except as shown otherwise

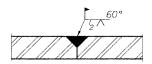
³4"\$ studs at 6" alt. cts. (Each End) — Stools For locations and heights see Sht, 6 of 7 Inside Face of Parapet ନ 073 (3) ---В (3) ---L έŊ 也 -2 -34"\$ x 8" granular or solid flux filled headed studs conforming to Art. 1006,32 of the Std. Specs evenly spaced betweem stools.

PLAN



FLAME CUTTING DIAGRAM (Proposed Finger R ~ Cut from 2" x 2'-4'2" R)

<u>NOTES</u>



SECTION D-D

"Erecting Structural Steel".

of ASTM A193. 2. The installation of all necessary hardware, stainless steel bolts, studs and washers, plates and angles shall be included with

1. Stainless steel bolts, washers and nuts shall meet the requirements

3. For Section B-B, see Sheet 5 of 9.

3₄" @ 130° F Finger Length В C

FINGER DETAIL

Finger Length = $6\frac{3}{4}$ " Weld Length = $9\frac{3}{4}$ " Weld Size = 516" Expansion length = 333.5'

ILLINOIS DEPARTMENT OF TRANSPORTATION PIER C10 JOINT DETAILS

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY) OVER SOUTH BRANCH - CHICAGO RIVER
COOK COUNTY
S.N. 016-1113 - PIER C10

DATE **∽**B

SCALE: VERT. HORIZ. DATE: 2/17/05 GU

GREENE & BRADFORD, INC.

COMPUTER FILE NO. 04286-W04-FPJ PROJECT 04286-WO 8/3/05-MML

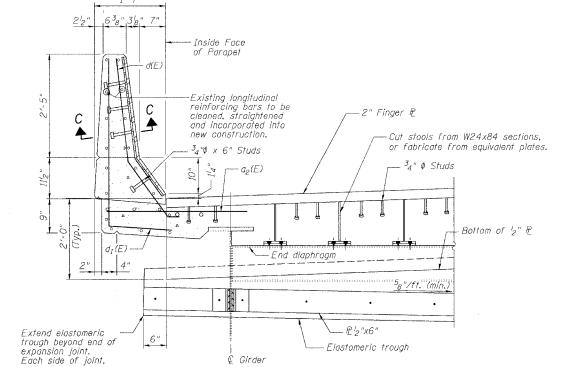
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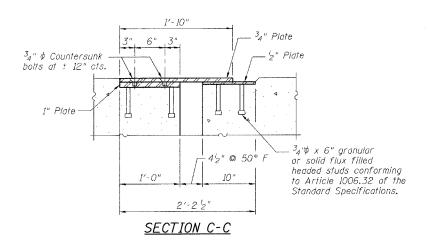
* 2005 - 008F Contract #62580

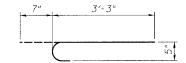


SECTION B-B

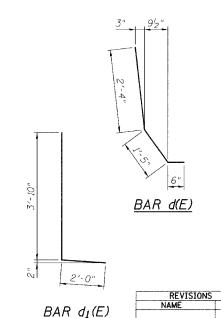
⊈ Girder

(For location of Section B-B, See Sheet 4 of 8)





BAR b(E)



Each 24 Bar Splicers Each 80 Mechanical Splice Reinforcing bars designated (E) shall be

Concrete Superstructure Cu. Yd.

BILL OF MATERIAL

24

16 80

a₁(E) 24

d1(E) 20

Reinforcement Bars, Epoxy Coated

Concrete Removal

epoxy coated.

Size Length Shape

#5 4'-3"

Pound

Cu. Yd. 9.4

1600

#5 18'-6" ——— #5 20'-6" ———

#6 4'-6"

#5 3'-10"

ILLINOIS DEPARTMENT OF TRANSPORTATION PARAPET DETAILS

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
OVER SOUTH BRANCH - CHICAGO RIVER
COOK COUNTY
S.N. 016-1113 - PIER CIO

DATE

SCALE: VERT. DATE: 2/17/05

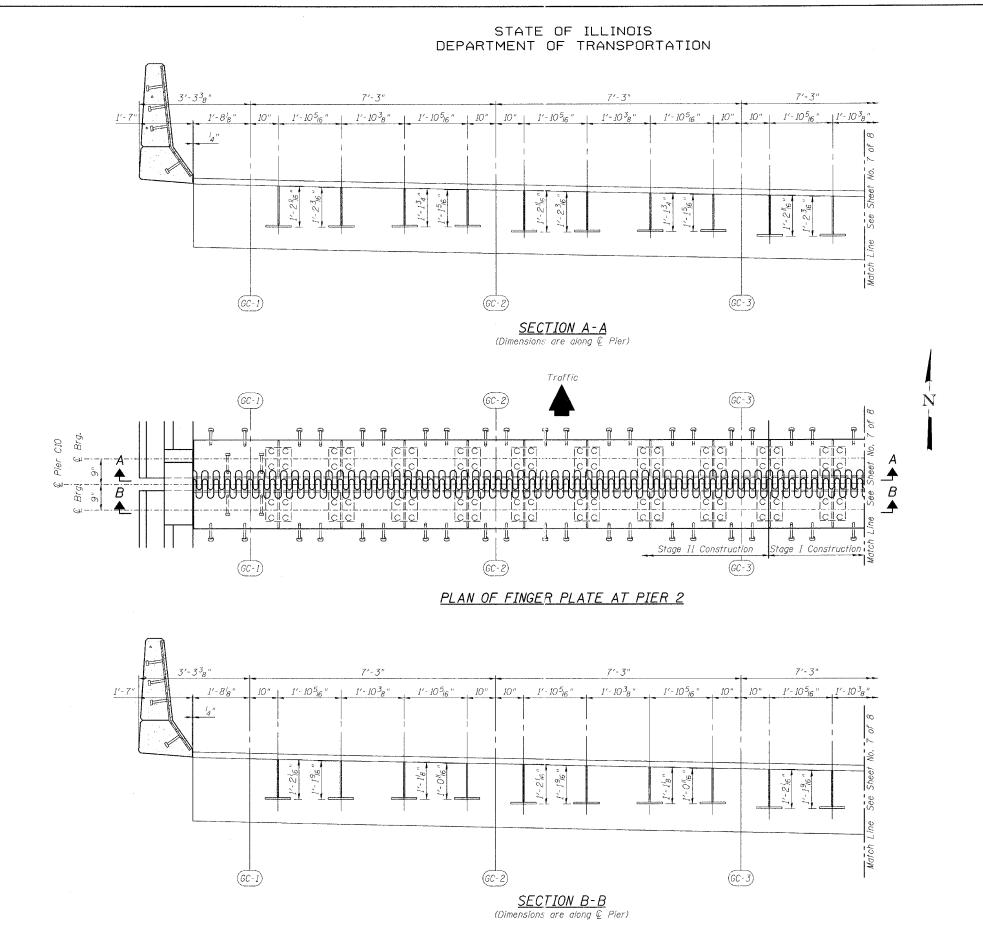
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NAME SCALE DATE TIME ATOR



SHEET NO. 6 TOTAL SHEETS SHEET NO. ROUTE NO. FAI 90/94 $8~{\rm sheets}$ COOK 419 225

* 2005 - 008F Contract #62580

ILLINOIS DEPARTMENT OF TRANSPORTATION FINGER PLATE PLAN AND ELEVATION

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
OVER SOUTH BRANCH - CHICAGO RIVER
COOK COUNTY
S.N. 016-1113 - PIER C10

REVISIONS NAME G N TB

SCALE: VERT. HORIZ. DATE: 2/17/05

GREENE & BRADFORD, INC.

OF SPRINGING.

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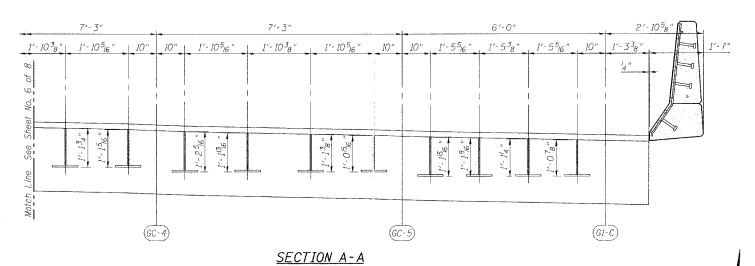
COMMAND FINE PROMISS.

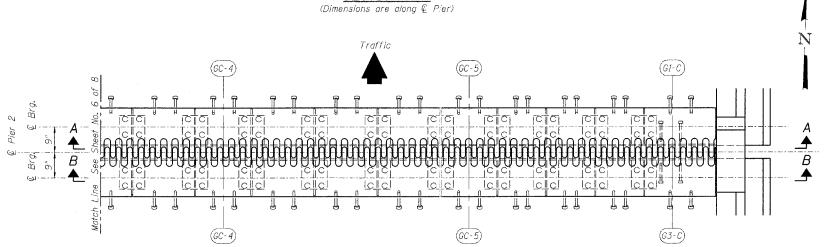
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DRAWN BY: HEBERLING DESIGNED BY: SANFORD CHECKED BY: -COMPUTER FILE NO. 04286-W04-PE1 PROJECT 04286-WO-8/3/05-MML

SHEET NO. 78 sheets

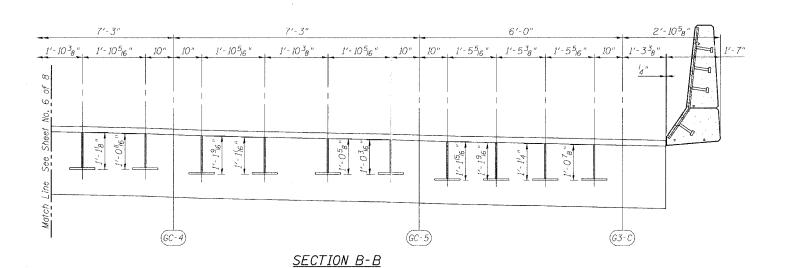
* 2005 - 008F Contract #62580





PLAN OF FINGER PLATE AT PIER 2

(Dimensions are along € Pier)



ILLINOIS DEPARTMENT OF TRANSPORTATION FINGER PLATE PLAN AND ELEVATION

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY) OVER SOUTH BRANCH - CHICAGO RIVER COOK COUNTY S.N. 016-1113 - PIER C10

REVISIONS NAME DATE G &

SCALE: VERT.

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GREENE & BRADFORD, INC.

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PROJECT 04286-WC 8/3/05-MML

sheet no. 88 sheets

* 2005 - 008F Contract #62580

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and fied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- Minimum Capacity (Tension in kips) = 1.25 x fy x A_1
- (lension in kips).

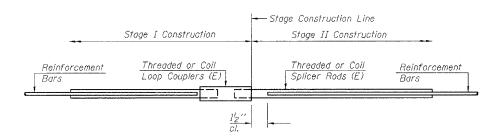
 Minimum *Pull-out Strength = 1.25 x fs_{allow} x A_t (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

fs_{allow}= Allowable tensile stress in lapped reinforcement bars in ksi (Service Load) A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

	BAR SPLIC	CER ASSEMBLI	ES	
		Strength Requirements		
Bar Size to be Spliced			Min. Pull-Out Strength kips - tension	
#4	1′-8′′	14.7	5.9	
#5	2'-0''	23.0	9.2	
#6	2'-7''	33.1	13.3	
#7	3′-5″	45.1	18.0	
#8	4'-6''	58.9	23.6	
#9	5′-9′′	75.0	30.0	
#10	7′-3′′	95.0	38.0	
#11	9'-0''	117.4	46.8	

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



STANDARD

Bar Size	No. Assemblies Required	Location
#5	24	Deck

ILLINOIS DEPARTMENT OF TRANSPORTATION BAR SPLICER ASSEMBLY DETAILS

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY) OVER SOUTH BRANCH - CHICAGO RIVER COOK COUNTY S.N. 016-1113 - PIER CIO

REVISIONS NAME DATE SCALE: VERT. HORIZ. DATE: 2/17/05 GREENE & BRADFORD, INC. GU

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PROJECT 04286-W0

8/3/05-MML

-The diameter of this part is equal or larger than the The diameter of this part diameter of bar spliced. is the same as the diameter of the bar spliced.

ROLLED THREAD DOWEL BAR

** ONE PIECE - Wire Connector

BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

Forms -Foam Plugs Splicer Rods (E) Washer Face <u>"B"</u>

Threaded or Coil

— Stage Construction Line

Template

INSTALLATION AND SETTING METHODS

<u>"A"</u>

"A": Set bar splicer assembly by means of a template bolt, "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

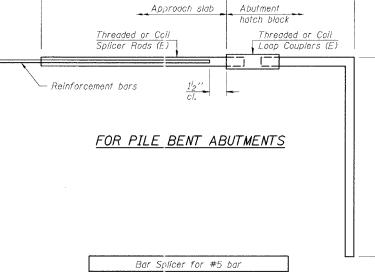
(E): Indicates epoxy coating.

6'-0"

Bridge Deck Approach Slab Threaded or Coil Reinforcement Threaded or Coil Splicer Rods (E) Loop Couplers (E) 6'-0"

FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tension No. Required = N/A



Min. Capacity = 23.0 kips - tension Min. Pull-out Strength ≈ 9.2 kips - tension No. Required = N/A

YAME SCALE DATE TIME

BSD-1

10-22-04

WENTWORTH AVE.

- WALLACE ST.

3. STEWART AVE.

SHEET NO. ${\it 1}$

8 sheets

* 2005-008F Contract #62580

GENERAL NOTES

Fasteners shall be high strength bolts. Bolts ${}^{3}_{4}$ " ϕ , open holes ${}^{13}_{16}$ " ϕ ,

Finger Plate Expansion Joints shall be assembled in the proper position with the ends in place and shall be left assembled for shop inspection.

Standard Specifications,

Reinforcement bars shall conform to the requirements of AASHTO M 31

Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional

Prior to pouring the new concrete deck, all loose rust, loose mill scale, and All heavy rust and other tightly adhered potentially detrimental foreign matter shall also be removed from the surfaces of the beams or girders in contact

Existing design and fabrication plans are available upon request.

All structural steel shall be AASHTO M270 Grade 50 unless noted

make a smooth finger joint.

specifications when the deck is poured at an ambient temperature other than 50°F.

unless otherwise noted.

The finger plates shall be flame cut as provided in Article 505.04(k) of the

Field welding of construction accessories will not be permitted to beams or

or M 322 Grade 60.

compensation for a change in the scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price for the work.

other loose potentially detrimental foreign material shall be removed from the surfaces of the beams or girders in contact with concrete. The cost of this work will be included in the pay item covering removal of the existing concrete. with concrete. Tightly adhered paint may remain unless otherwise noted. This removal shall be accomplished by methods that will not damage the steel. The cost of this work will be paid for according to Article 109.04.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. All Construction joints shall be bonded.

otherwise. Calculated weight = 25,537 lbs. Tapered shims shall be added under the stools as required by the Engineer to

Joint openings shall be adjusted according to Article 503.10(c) of the standard Existing structural steel that will be in contact with new structural steel shall be

cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

Traffic to be maintained during construction, see Roadway Plans for Stage Construction Details.



COOK COUNTY, ILLINOIS



DESIGN STRESSES

f'c = 3500 psi

PIER NO. 2

LOCATION

(SN 016-1111)

fy = 60,000 psi (Reinf)

fy = 50,000 psi (M270 Grade 50)

(SN 016-1113)

BILL OF MATERIAL			
ITEM	UNIT	TOTAL	
Concrete Removal	Cu Yd	14.7	
Concrete Superstructure	Cu Yd	17.6	
Erecting Structural Steel	L.S.	0.09	
Reinforcement Bars, Epoxy Coated	Pound	2490	
Mechanical Splice	Each	134	
Bar Splicers	Each	24	
Erecting Fabric Reinforced Elastomeric Trough	Foot	66	

ILLINOIS DEPARTMENT OF TRANSPORTATION KEY PLAN

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
16TH STREET TO 18TH STREET
COOK COUNTY
S.N. 016-1111 - PIER 2

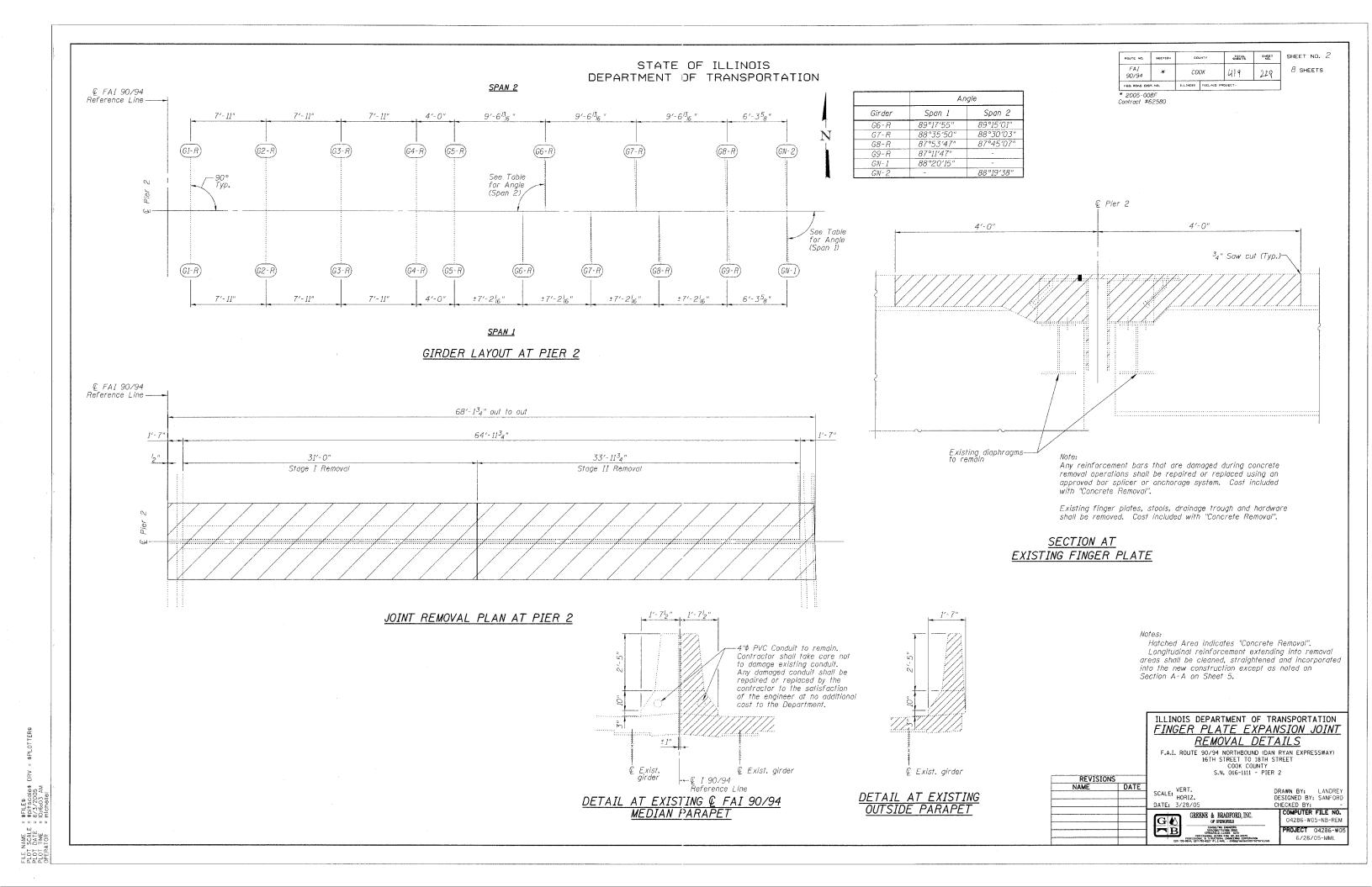
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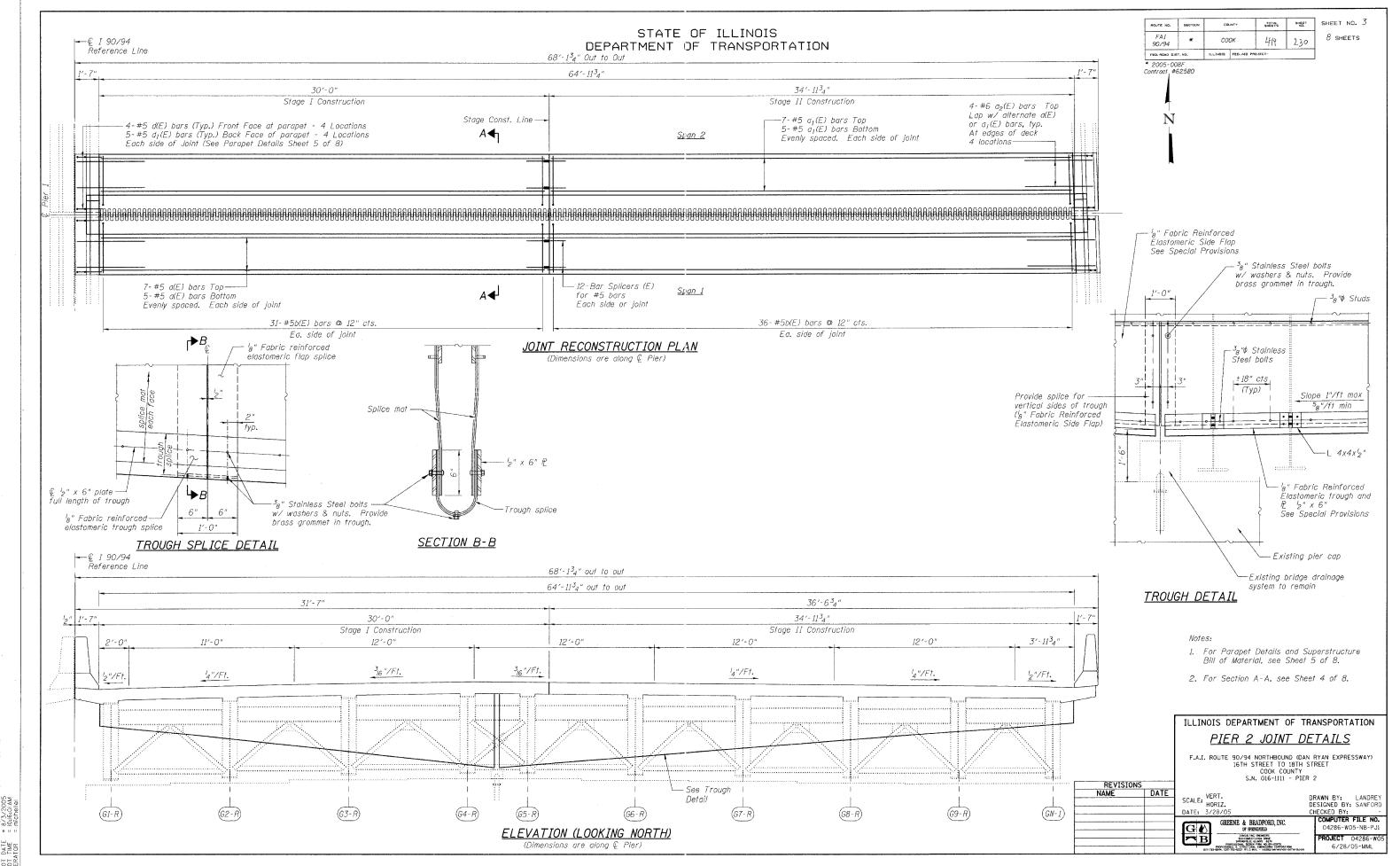
REVISIONS

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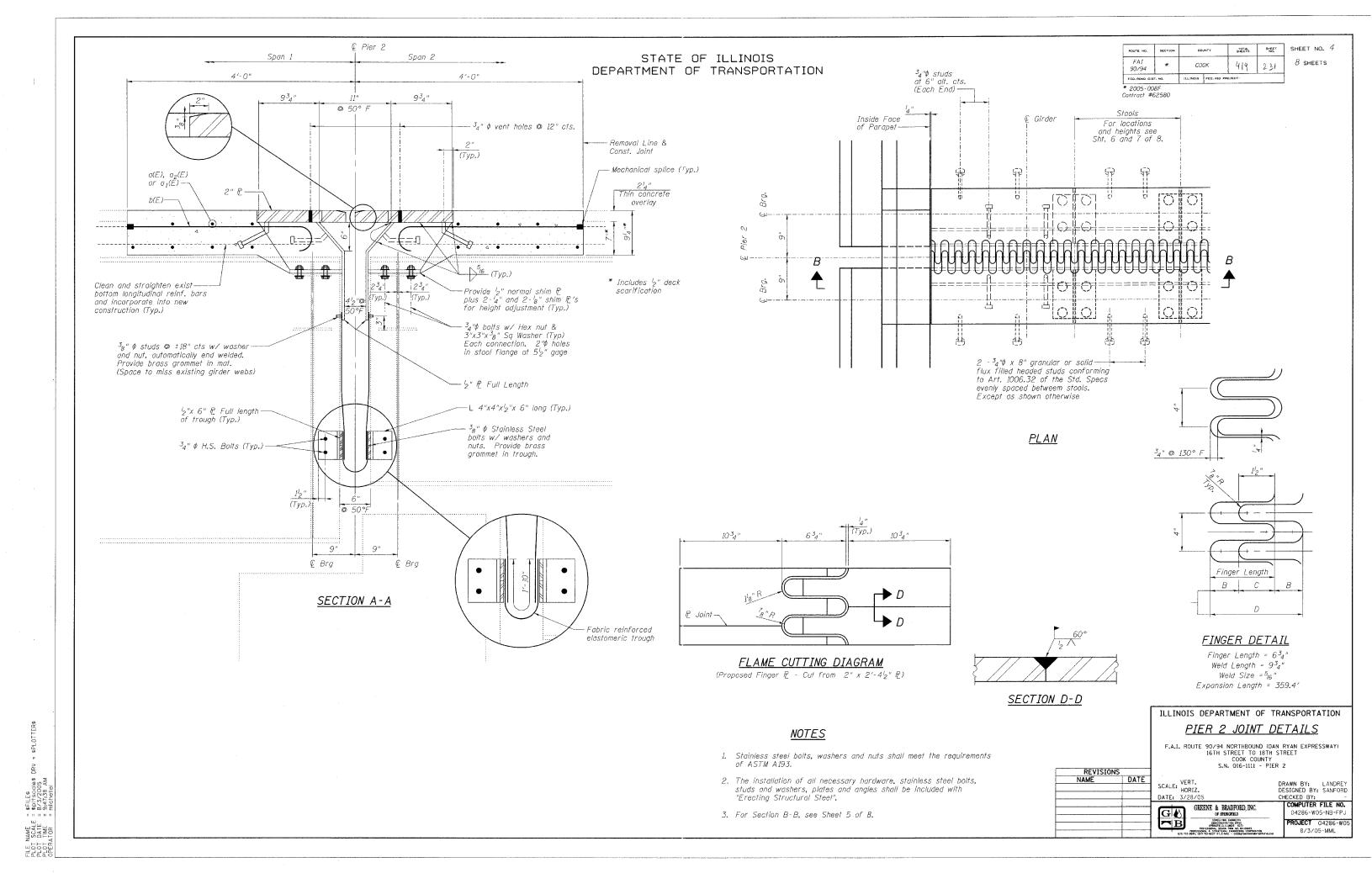
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FILE NAME = \$FILE\$
PLOT SCALE = \$pitsoc
PLOT DATE = 8/3/20
PLOT TIME = 10:16:01



³₄" ¢ Countersunk bolts at ± 12" cts.

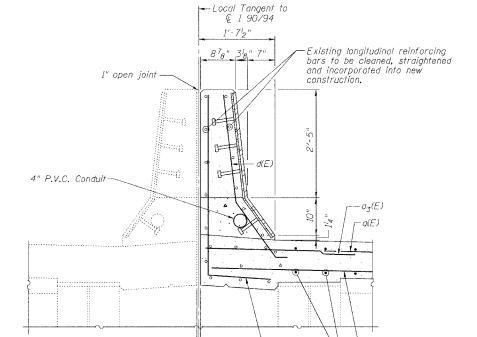
1" Plate

SHEET NO. 5 FAI 90/94 8 sheets COOK 419 232 FED. ROAD DIST. NO.

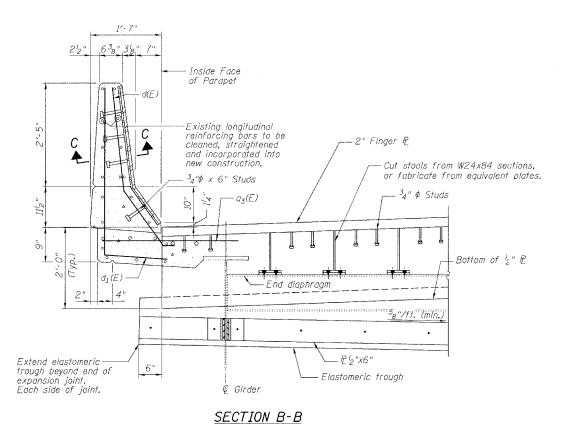
* 2005-008F Contract #62580

- ³4"\$ x 6" granular or solid flux filled headed studs conforming

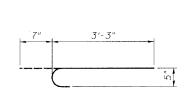
to Article 1006.32 of the Standard Specifications.



DETAIL AT MEDIAN PARAPET (Looking North)



(For location of Section B-B, See Sheet 4 of 8)



4½" @ 50°

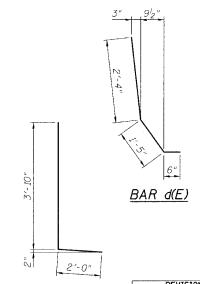
1'-0"

2'-6'2'

SECTION C-C

- ³4" Plate

BAR b(E)



BAR $d_1(E)$

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	24	#5	30'-6"	
a ₁ (E)	24	#5	35′-6"	
a ₂ (E)	16	#6	4'-6"	
b(E)	134	#5	3'-10"	
d(E)	16	#5	4'-3"	
d1(E)	20	#5	5′-10"	
Reinford Epoxy (cement B Coated	ars,	Pound	2490
Concret	e Supersi	ructure	Cu. Yd.	17.6
Concrete Removal		Cu. Yd.	14.7	
Bar Splicers		Each	24	
Mechani	cal Splice)	Each	134
Delefore			had (C) ab	-11 h -

Reinforcing bars designated (E) shall be epoxy coafed.

ILLINOIS DEPARTMENT OF TRANSPORTATION PARAPET DETAILS

F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
16TH STREET TO 18TH STREET
COOK COUNTY
S.N. 016-1111 - PIER 2

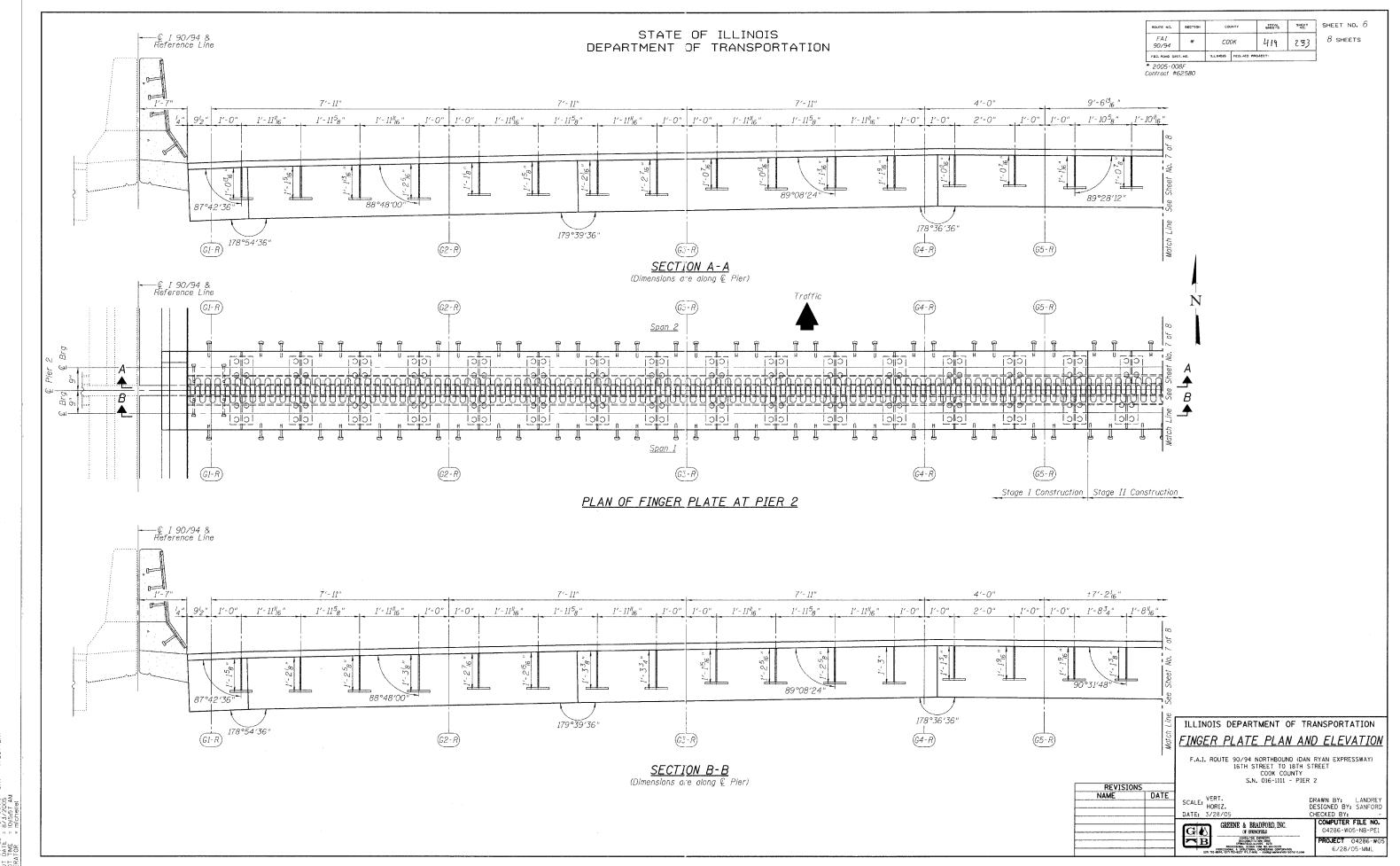
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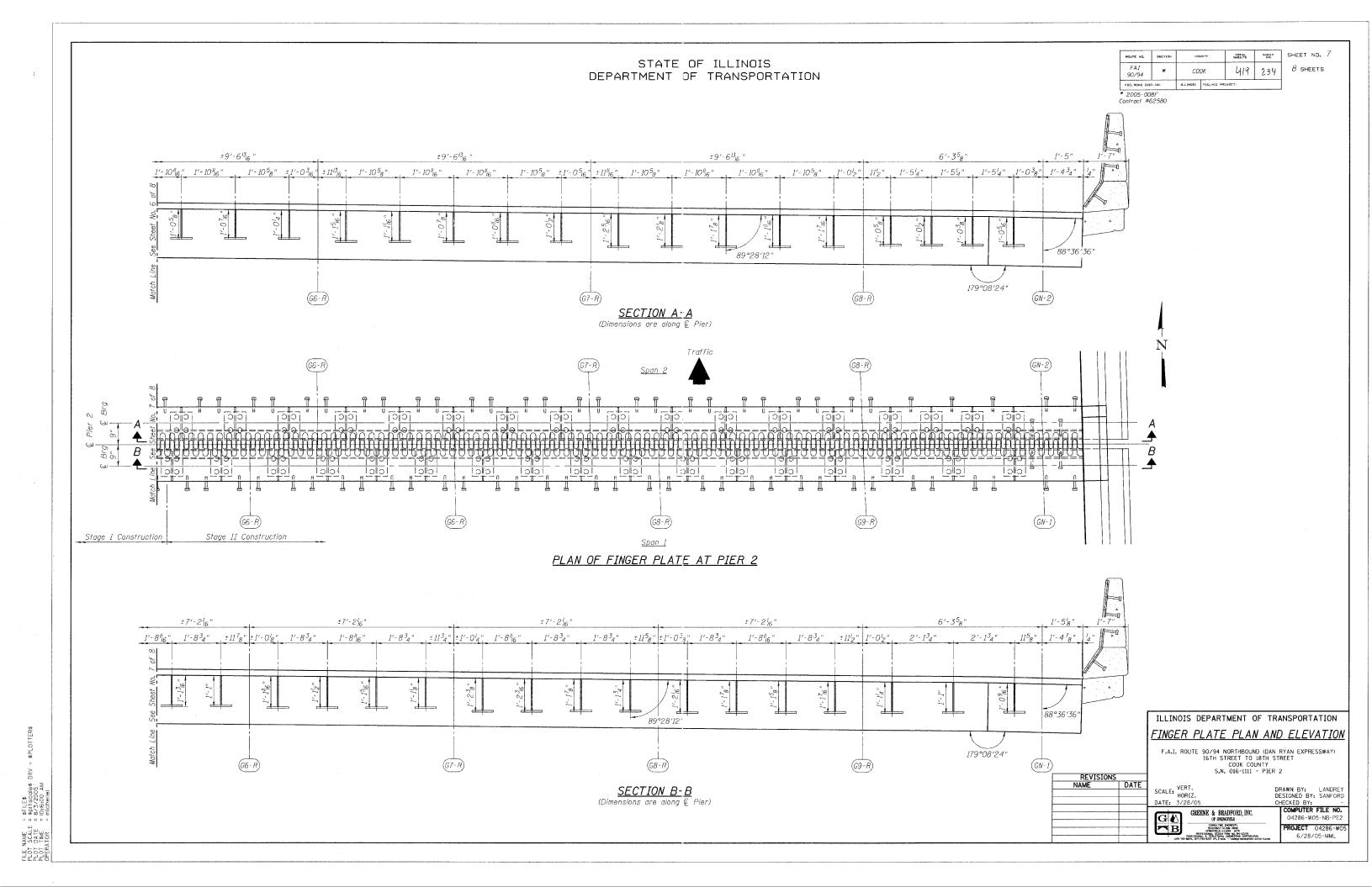
GREENE & BRADFORD, INC.

DRAWN BY: LANDREY DESIGNED BY: SANFORD CHECKED BY: -

04286-W05-NB-PD PROJECT 04286-W05 6/28/05-MML



VAME SCALE DATE TIME



SHEET NO. 88 sheets

Contract #62580

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars. Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length.

All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

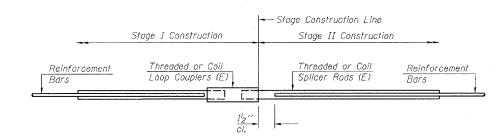
Minimum Capacity = 1.25 x fy x A_t

(lension iii kipo) Minimum *Pull-out Strength = 1.25 x fs_{allow} x A_t

Where fy = Yield strength of lapped reinforcement bars in ksi. fs_{allow}= Allowable tensile stress in lapped reinforcement bars in ksi (Service Load) A_t = Tensile stress area of lapped reinforcement bars.
* = 28 day concrete

BAR SPLICER ASSEMBLIES				
		Strength Requirements		
	Splicer Rod or Dowel Bar Length		Min. Pull-Out Strength kips - tension	
#4	1'-8''	14.7	5.9	
#5	2'-0"	23.0	9.2	
#6	2'-7"	33.1	13.3	
#7	3′-5″	45.1	18.0	
#8	4'-6"	58.9	23.6	
#9	5′-9″	75.0	30.0	
#10	7′-3′′	95.0	38.0	
#11	9'-0''	117.4	46.8	

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



STANDARD

Bar Size	No. Assemblies Required	Location
#5	24	Deck

ILLINOIS DEPARTMENT OF TRANSPORTATION BAR SPLICER ASSEMBLY DETAILS

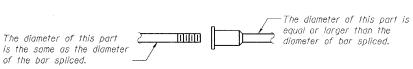
F.A.I. ROUTE 90/94 NORTHBOUND (DAN RYAN EXPRESSWAY)
16TH STREET TO 18TH STREET
COOK COUNTY
S.N. 016-1111 - PIER 2

DATE SCALE: VERT. DATE: 3/28/0 $rac{B}$

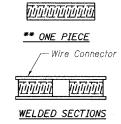
GREENE & BRADFORD, INC.

DRAWN BY: LANDREY
DESIGNED BY: SANFORD
CHECKED BY: COMPUTER FILE NO. 04286-W05-NB-BSD PROJECT 04286-W05 6/28/05-MML

STATE OF ILLINOIS

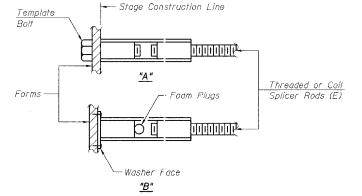


ROLLED THREAD DOWEL BAR



BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

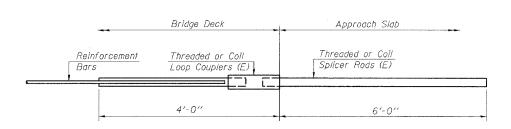


INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template boit. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

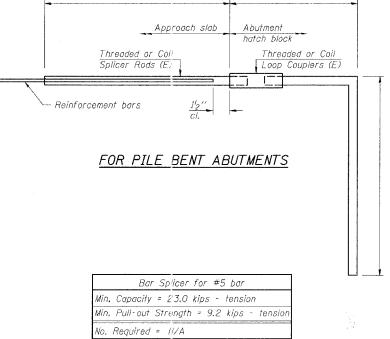
(E): Indicates epoxy coating.

6'-0"

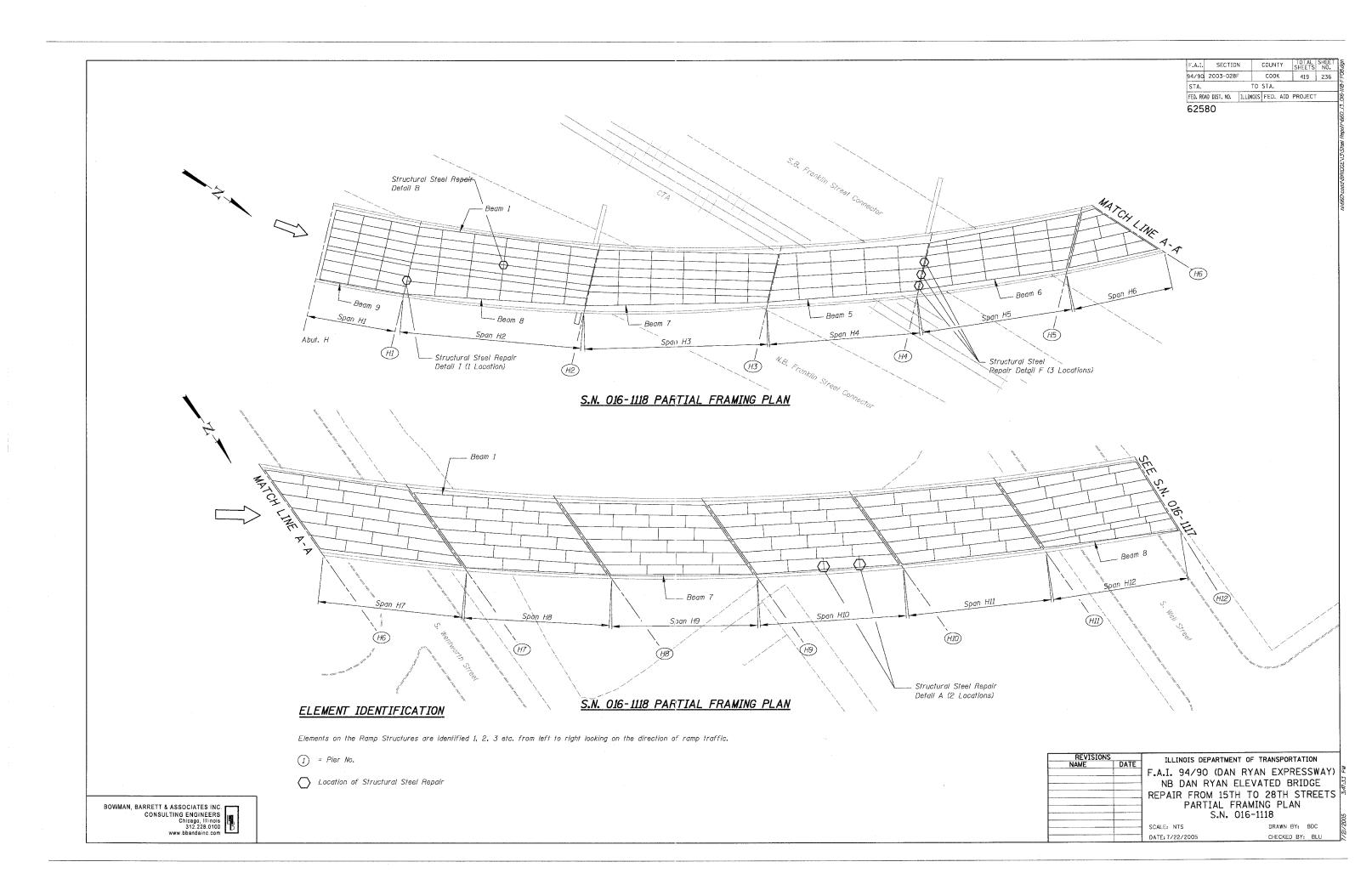


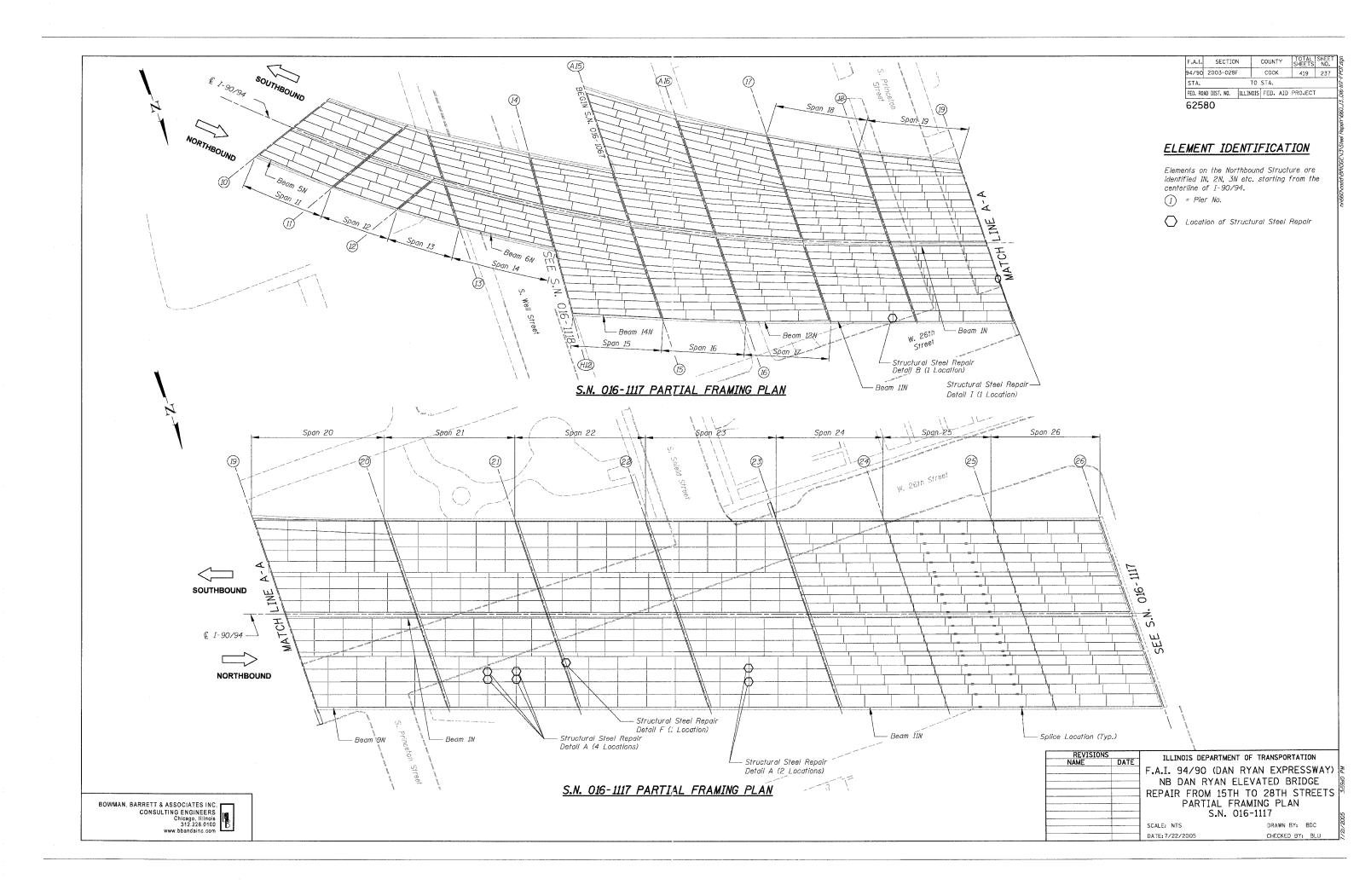
FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

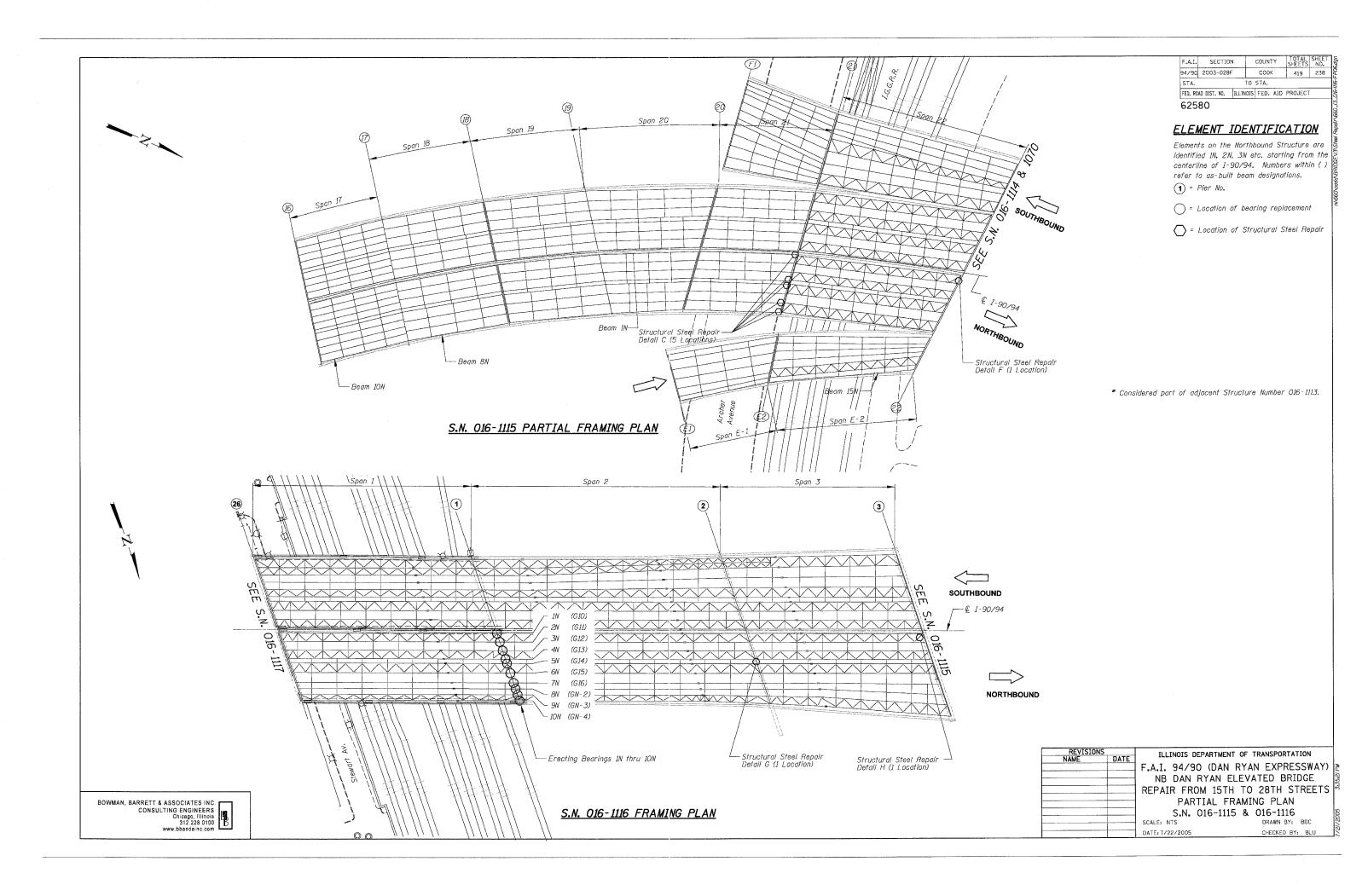
Bar Splicer for #5 bar							
Min.	Сарас	ity =	23.0	kips	-	tension	
Min.	Pull-o	ut St	rength	= 9	.2	kips -	tension

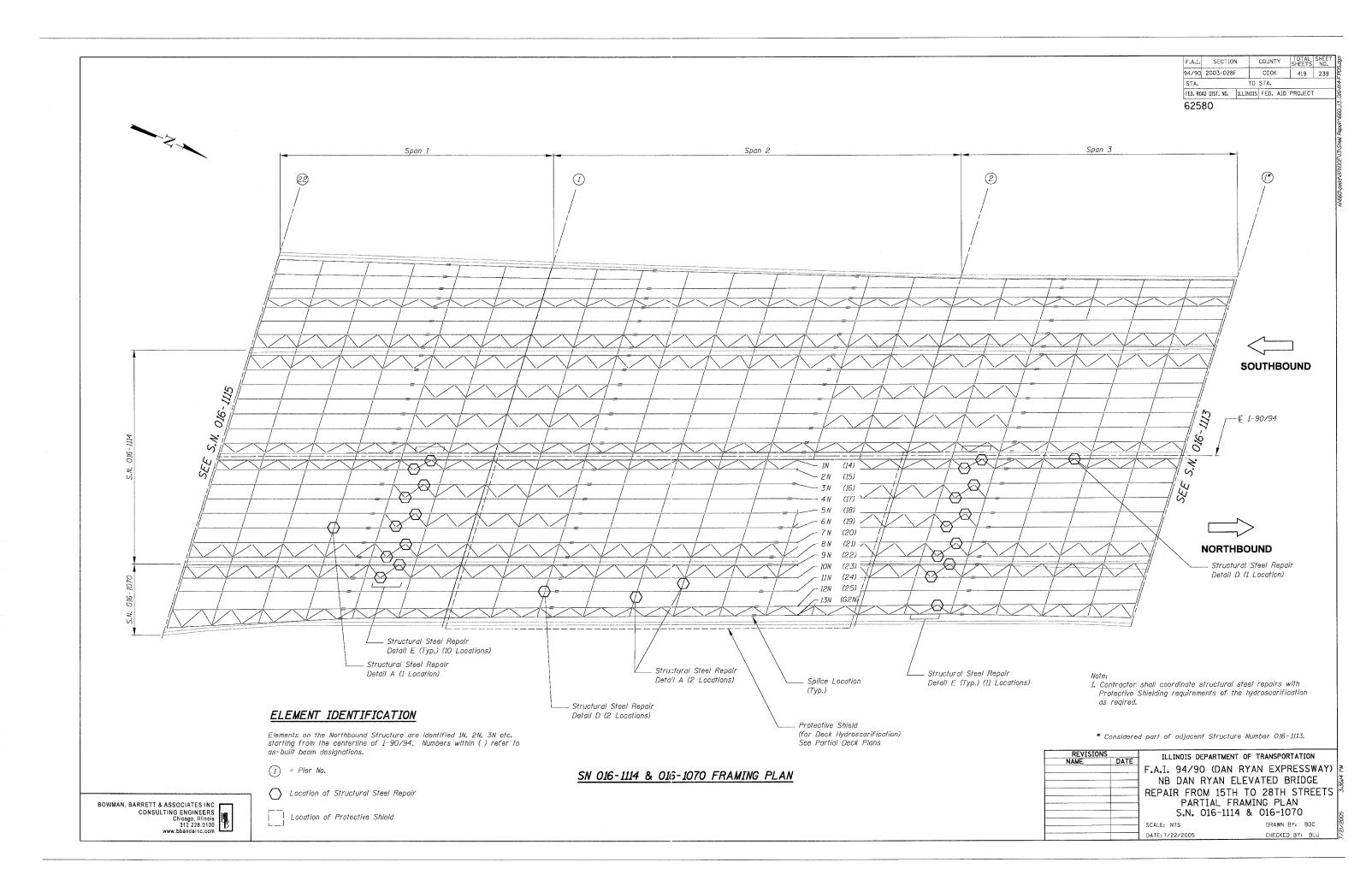


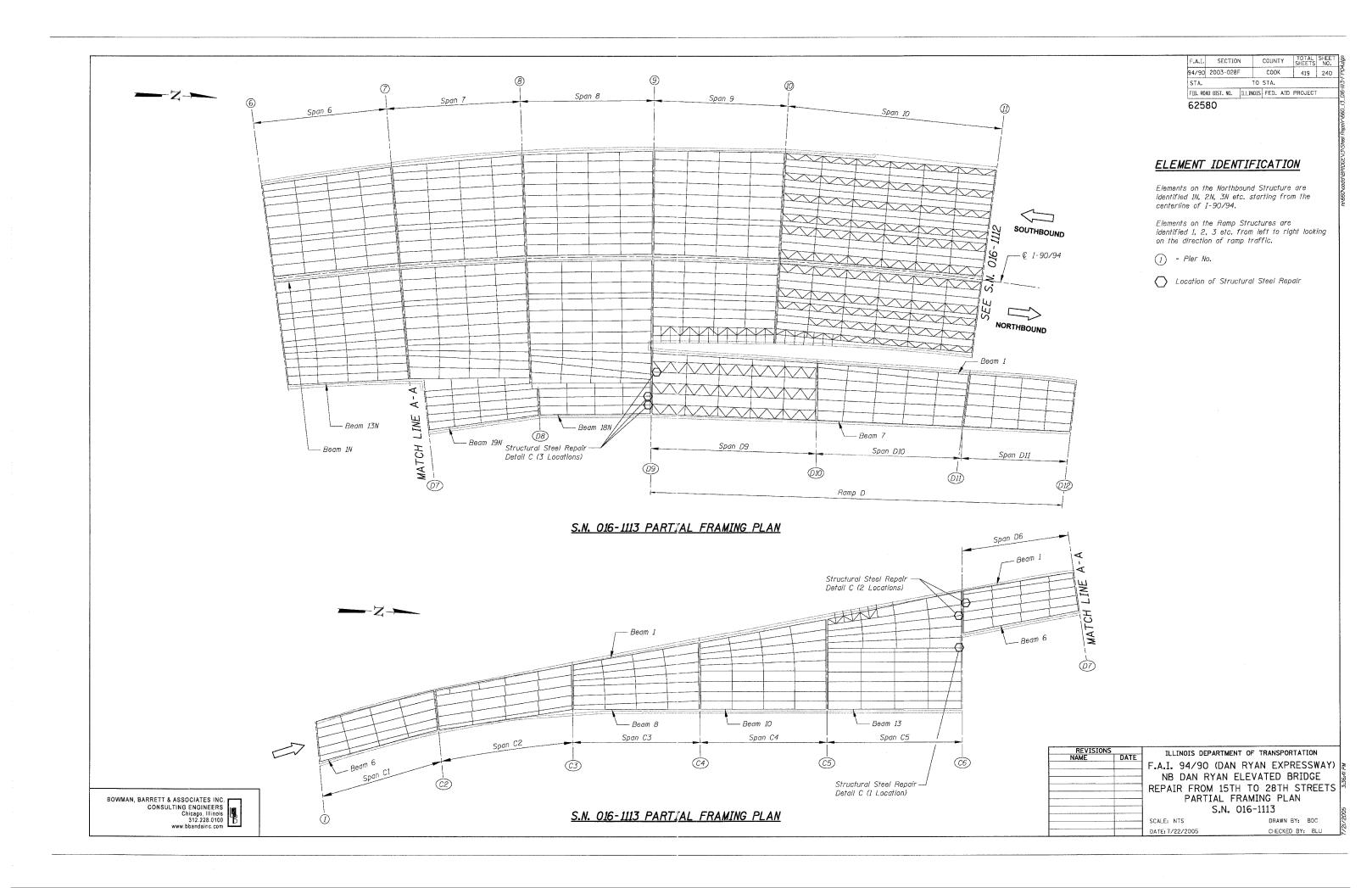
BSD-1

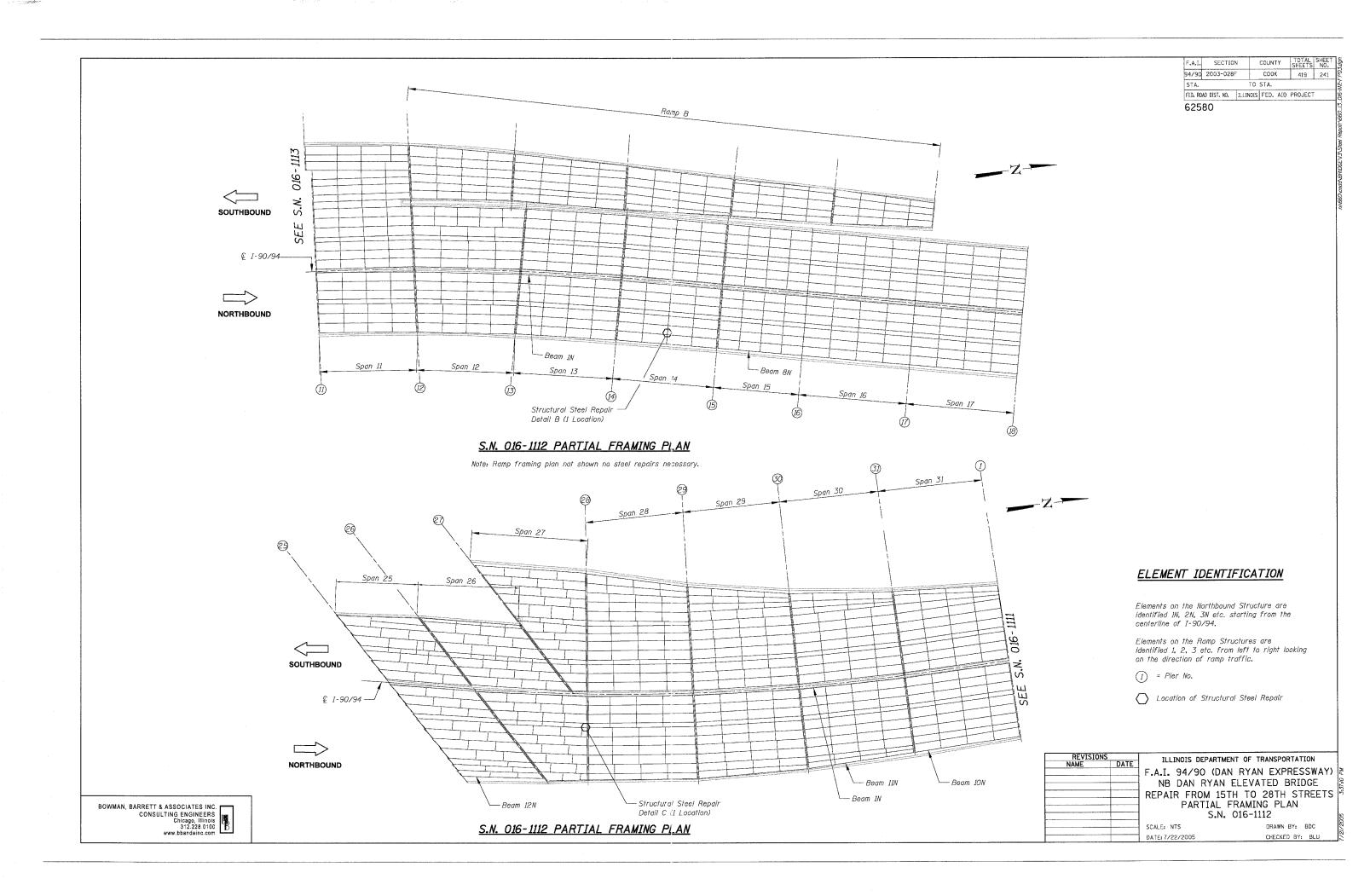


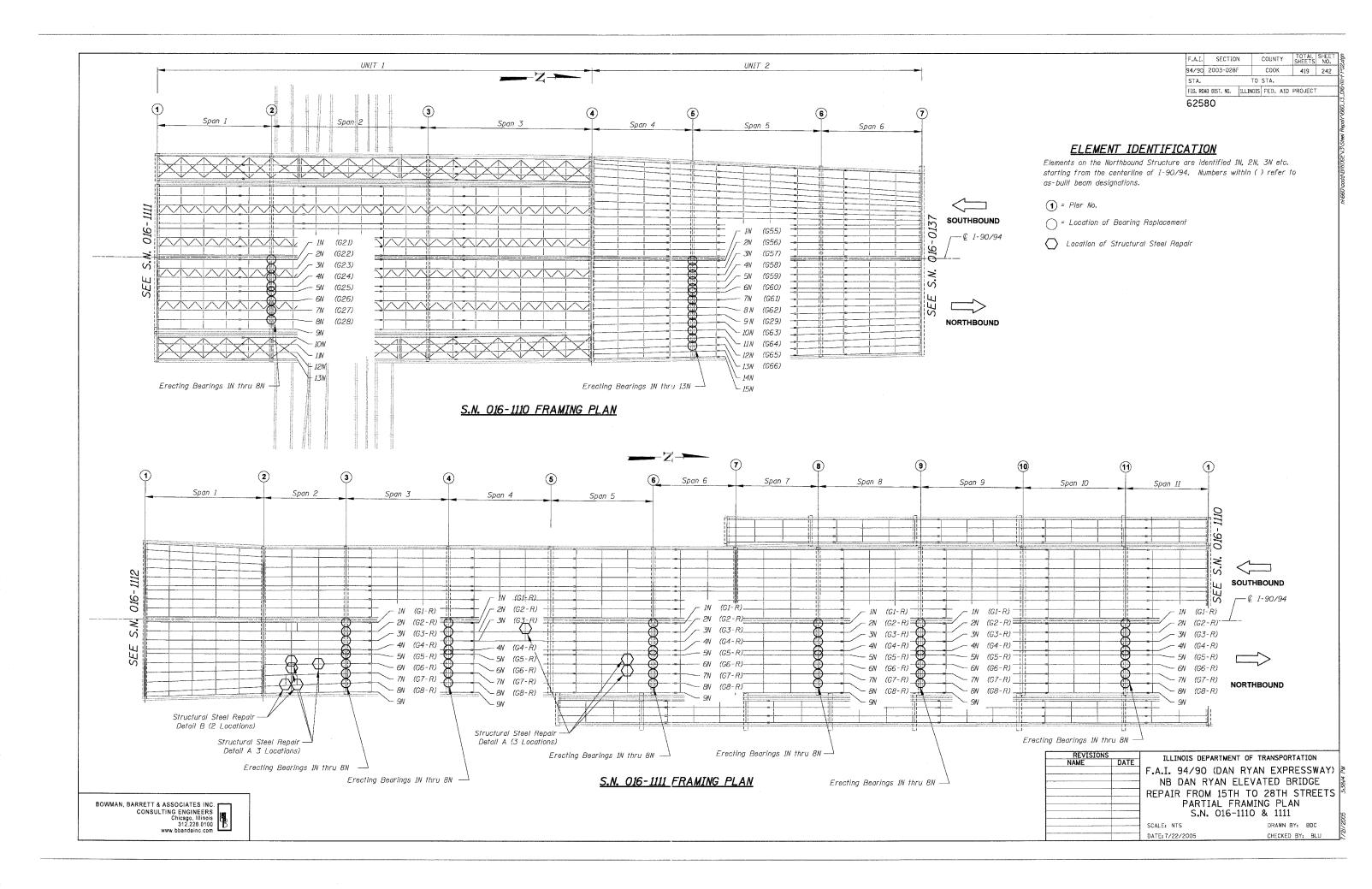


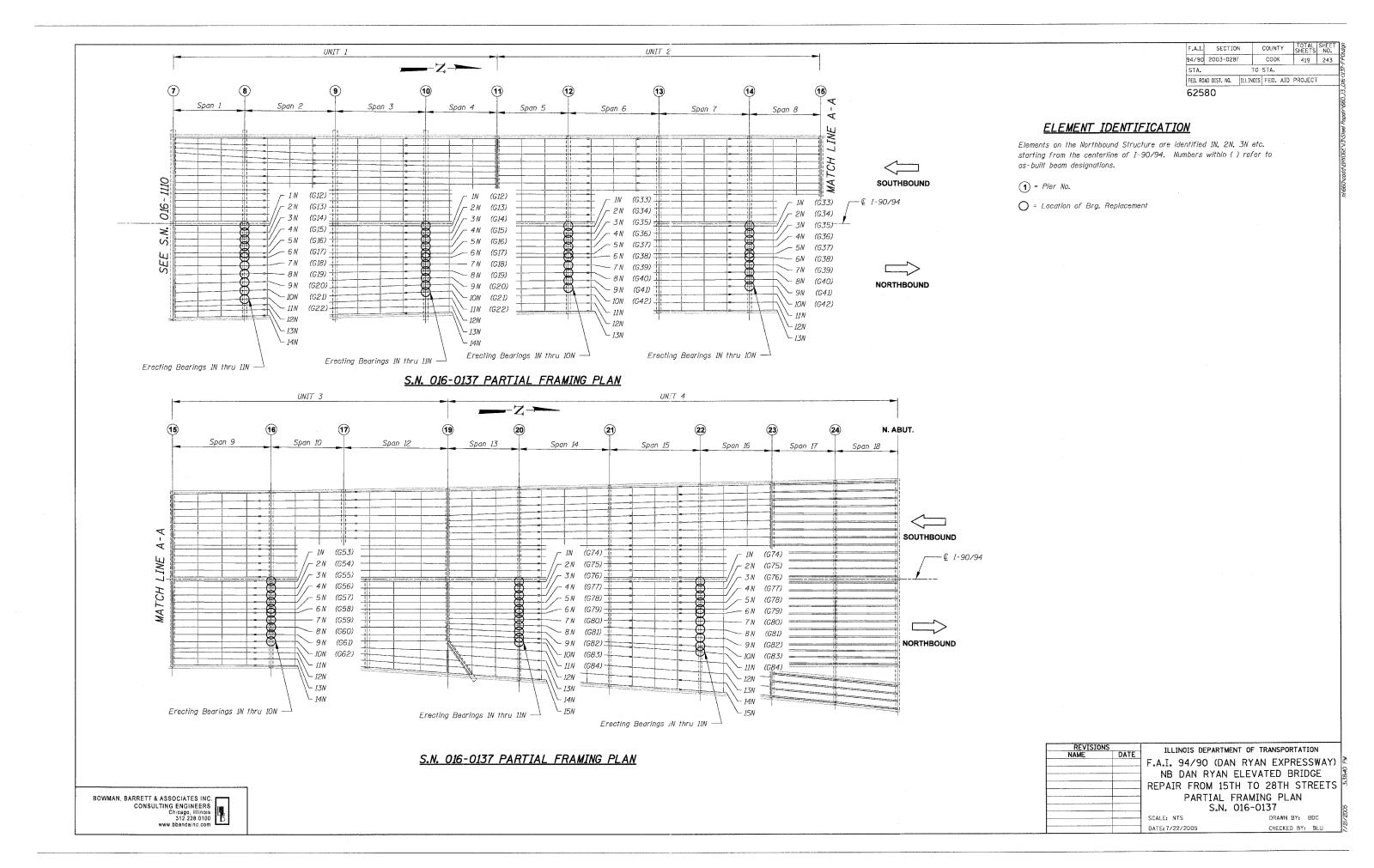


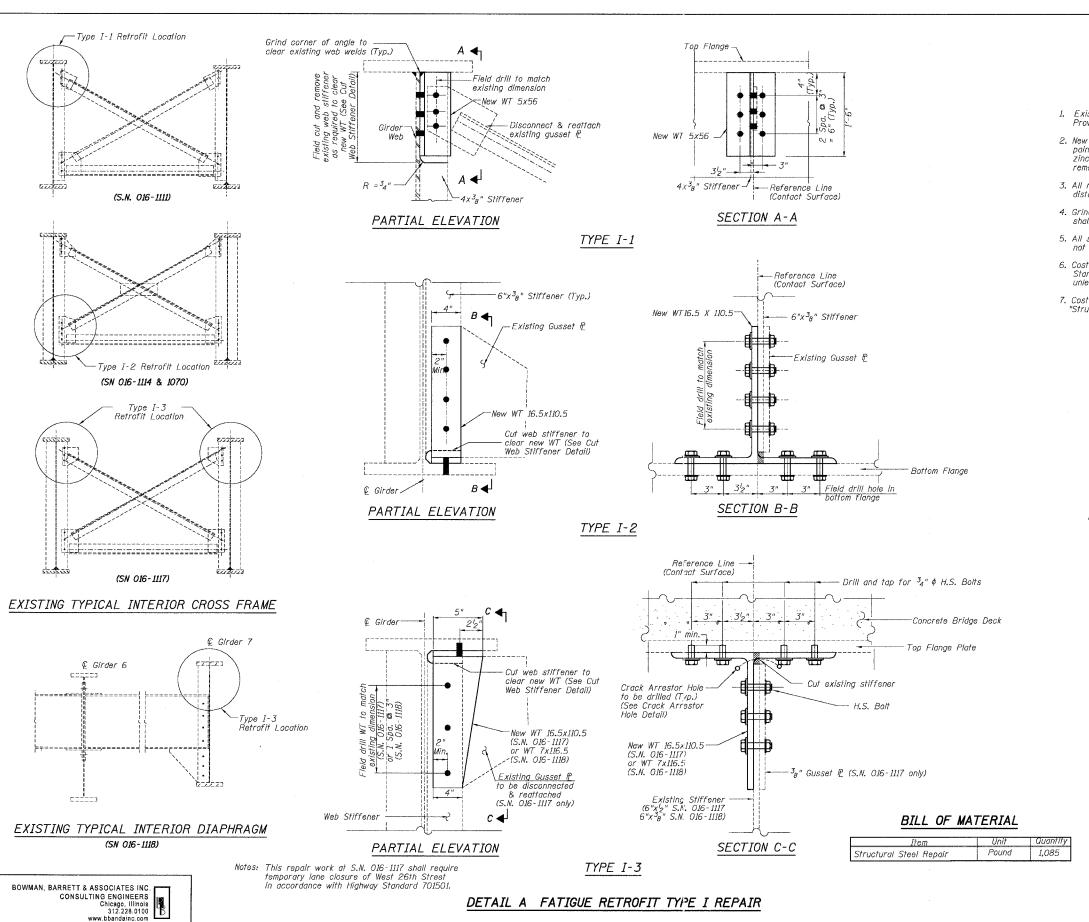








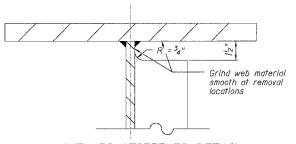




62580

STRUCTURAL STEEL REPAIR GENERAL NOTES

- Existing structural steel shall be painted in accordance with Guide Bridge Special Provisions for Cleaning and Painting Contact Surface Areas of Existing Steel Structures.
- 2. New structural steel shall conform to AASHTO M-270 Gr. 36, and be shop painted with inorganic zinc rich primer per AASHTO M300 Type I. Spot paint with zinc silicate primer applied on heads of field welds and all areas where paint has been removed or damaged shall be required. Cost included in Structural Steel Repair.
- 3. All new fasteners shall be $^3\!\!_{Z}$ n ϕ H.S. bolts unless otherwise noted. Minimum edge distance for bolts shall be $^1\!\!_{Z}$ n
- Grinding shall be done parallel to the beam direction. Cost of grinding and field drill shall be included with Structural Steel Repair.
- All structural steel repairs shall be coordinated with deck MOT to assure live load is not directly transmitted through members being repaired.
- Costs of furnishing, erecting and removal of required Traffic Control and Protection Standards for steel repairs shall be included in the cost of "Structural Steel Repair" unless otherwise noted.
- 7. Cost of removal and disposal of existing steel plates shall be included in the cost of "Structural Steel Repair".

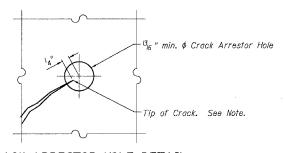


CUT WEB STIFFENER DETAIL

- Note: 1. Field cut web stiffener using saw, drill and / or grinding.

 Flame cutting will not be permitted within 2" of girder web or flange.

 Grind surfaces after cutting to a surface roughness of (Ra) 250u in. or less.
 - 2. Contractor shall fabricate retrofit hardware providing holes subpunched for attachment to girder flange. Holes shall be reamed full size for attachment to the flange and drilled for attachment in the field.



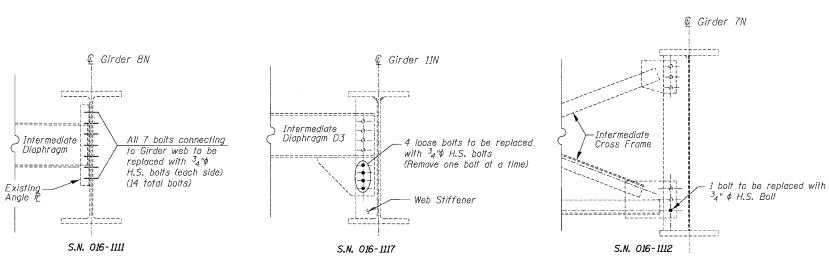
CRACK ARRESTOR HOLE DETAIL

Note: Locate crack tip using liquid dye penetrant or magnetic particle testing.

Drill ¹⁵₆, " min. \$\phi\$ Crack Arrestor hole at the crack tip. After crack
arrestor hole has been drilled, dye penetrant or magnetic particle testing
shall be used to verify that the drilled hole has captured the crack tip.

Cost shall be included in the cost of "Structural Steel Repair".

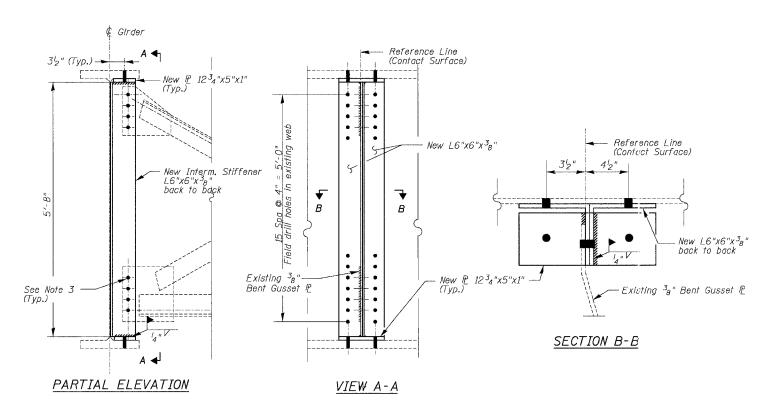
				1
REVISIONS NAME	DATE	ILLINOIS DEPARTMENT OF	TRANSPORTATION	
NAME	DATE	F.A.I. 94/90 (DAN RYA	N EXPRESSWAY)	₩.d
		NO DAN BYAN ELEM	TED DDIDGE	ζ.
		REPAIR FROM 15TH TO	28TH STREETS	M.
		STRUCTURAL STE	EL REPAIR	
		DETAIL A	4	8
		SCALE: NTS	DRAWN BY: BDC	1/2
		DATE: 7/22/2005	CHECKED BY: BLU	15



Notes: This work at S.N. 016-1117 shall require temporary sidewalk closure in accordance with Highway Standard 701801.

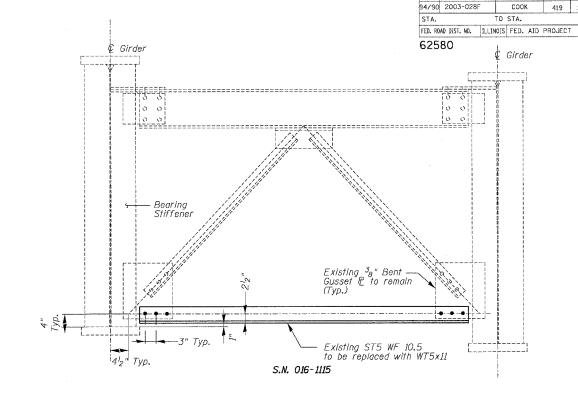
DETAIL B BOLT REPLACEMENT

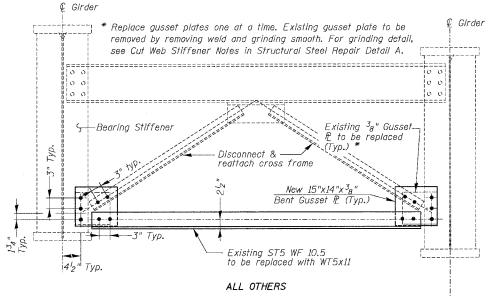
(Looking North)



DETAIL D STIFFENER REPLACEMENT

- Notes: 1. Remove existing stiffener prior to install new bolted stiffener. See Structural Steel Repair Detail A for Cut Web Stiffener Detail.
 - 2. New stiffener to be vertical & 90 degrees to web.
 - 3. For gusset plate connections, bolt holes shall be field drilled to match existing locations or as directed by Engineer.
 For location where gusset plate is not present (S.N. 016-1114, Girder 1N of Span 3), no connetion bolt and hole is required.





Notes: This replacement work shall require temporary lane closure of South Canal Street in accordance with Highway Standard 701501.

DETAIL F BRACING MEMBER REPLACEMENT

REVISIONS		THE THOIS DEPARTMENT	T OF TRANSPORTATION	1
NAME	DATE			_
		F.A.I. 94/90 (DAN	RYAN EXPRESSWAY)	ď
		NB DAN RYAN F	LEVATED BRIDGE	Š
		REPAIR FROM 15TH	TO 28TH STREETS	A.
		STRUCTURAL :	STEEL REPAIR	
		DETAILS	B, D & F	52
		SCALE: NTS	DRAWN BY: BDC	1/2
		DATE: 7/22/2005	CHECKED BY: BLU	12

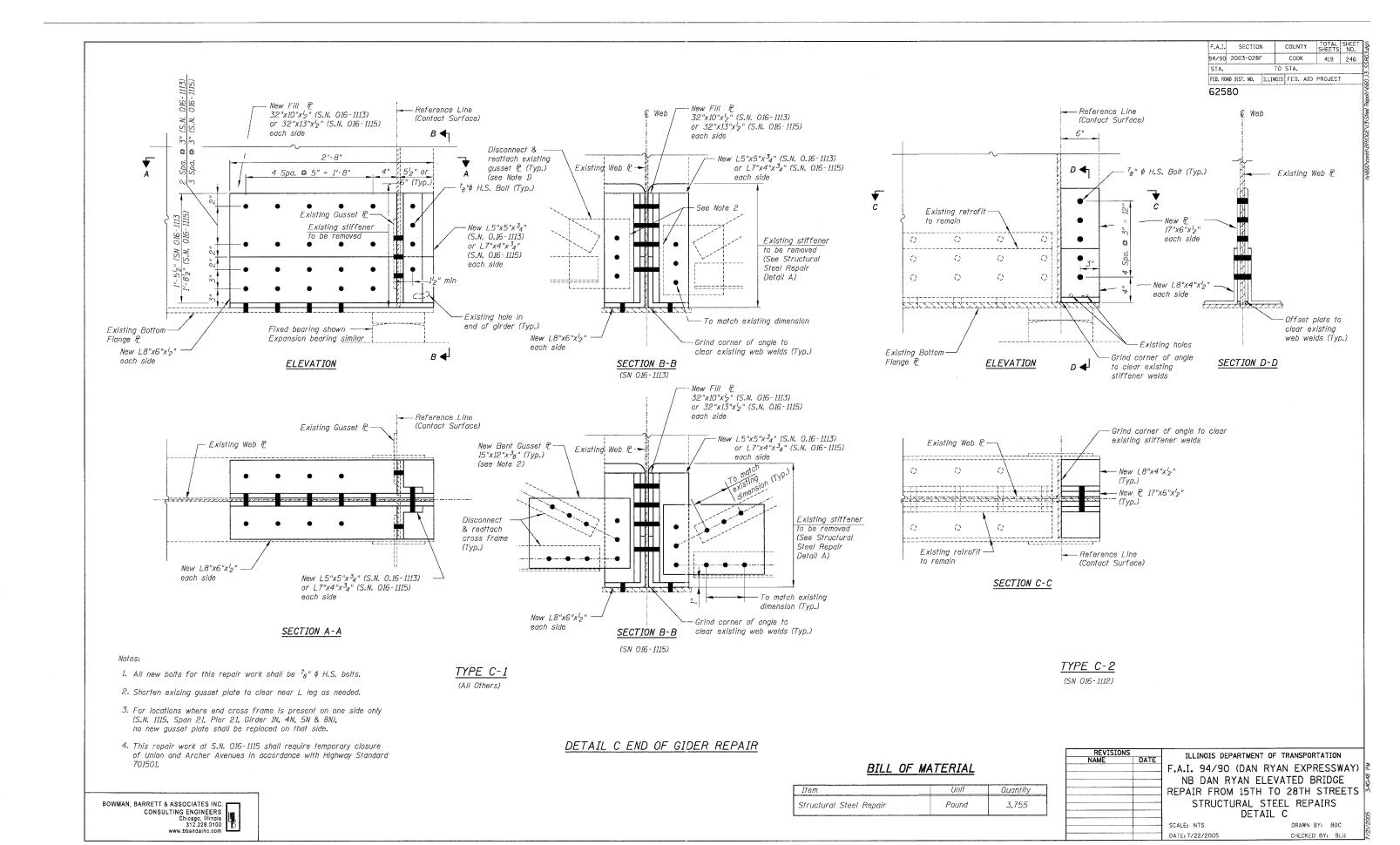
F.A.I. SECTION

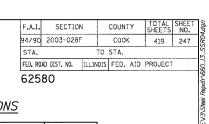
COUNTY TOTAL SHEETS

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel Repair	Pound	1,380

BOWMAN, BARRETT & ASSOCIATES INC.
CONSULTING ENGINEERS
Chicago, Illinois
312.228.0100
www.bbandainc.com





HOAN-TYPE CONNECTION DETAIL DIMENSIONS

Structure Number	Span	Girder No	0**	b	Sh	Sv	h	d
016 - 1114	1	14	3"	6 ⁷ /6 "	12	7	6 ⁷ 8"	⁵ 8
		<i>1</i> 5	-3 ¹ 2"	6 ^l 2"	12	7	9³ ₁₆ "	⁵ 8
		16	-3 ³ 4"	6"	11 ¹³ 16	12	6 ³ 8"	1/16
		17	314"	6"	11 ¹³ 16	12	10 ¹ 8 "	1/16
		18	- 3 ⁹ 16 "	6"	11 ¹³ 16	12	7 ¹ 2"	"16
		19	314"	6"	11 ¹³ 16	12	11 ⁹ 16 "	5 ₈
		20	- 3 ¹³ 16 "	6"	11 ¹³ 16	12	6 ³ 8"	⁵ 8
		21	314"	6"	11 ¹³ 16	12	10 ³ 16 "	⁵ 8
016-1070	1	22	-2 ³ 16"	6"	12	10 ⁷ 8	6 ³ 8"	5 ₈
		23	3/16"	5 ¹⁵ 16 "	12	10 ⁷ 8	10 ¹⁵ 16 "	- 58
016 - 1114	3	14	316"	5 ⁹ 16"	12	7	6½"	9 ₁₆
		<i>1</i> 5	-37 ₁₆ "	5 ⁹ 16"	12	7	6 ³ 8"	9 ₁₆
		<i>1</i> 6	- 31/16"	6"	11 ¹³ 16	12	6 ³ 8"	1/2
		17	3 ⁵ 16"	6"	11 ¹³ 16	12	6½"	1/2
		18	-3 ³ 4"	6"	11 ¹³ 16	12	7/16"	916
		19	3 ⁵ 16 "	6"	11 ¹³ 16	12	6 ¹⁵ 16 "	9 ₁₆
		20	- 3 ¹¹ 16"	6"	11 ¹³ 16	12	7 ¹ 2"	1/2
		21	314"	6"	11 ¹³ 16	12	7 ³ 8"	1/2
016-1070	3	22	-4 ¹ 8"	6"	12	10 ⁷ 8	6 ³ 8"	1/2
		23	34"	5 ¹⁵ 16 "	12	10 ⁷ 8	6 6"	1/2
		25	3 ⁵ 16"	6"	12	6 ¹³ 16	6 ⁷ 16 "	1/2

^{** &}quot;+" for north offset from WP, and "-" for south offset from WP.

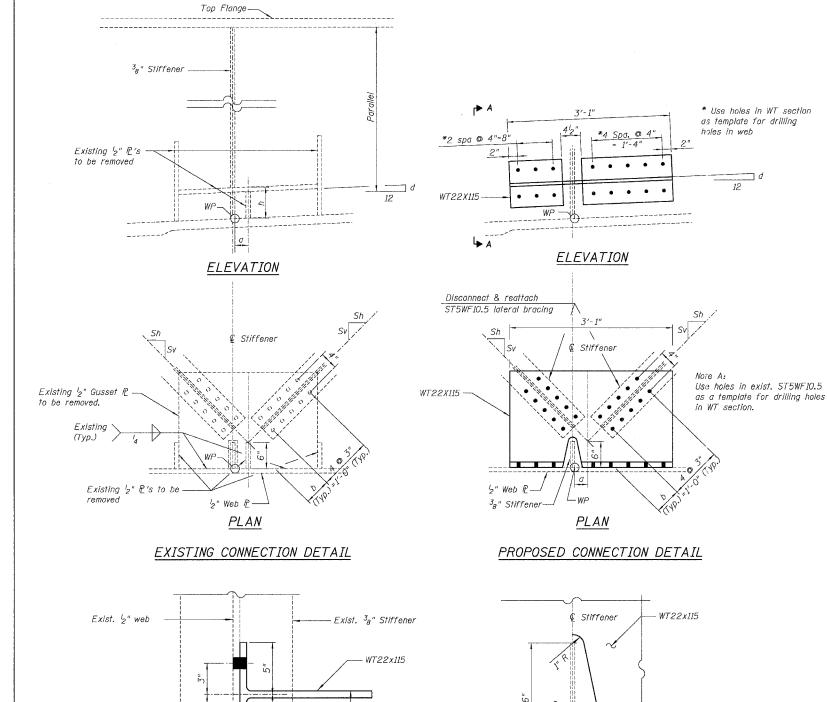
PROCEDURE FOR REPAIR DETAIL

- 1. Remove existing horizontal gusset plate as shown. The minimum distance from cut to face of web shall be the larger of ${}^{l}_{4}$ " or web to gusset ${}^{p}_{4}$ weld size, with removal of remaining material by grinding as described below. The cut shall be made vertically through the gusset without angling the cut towards the web. Equipment and method of cutting shall be approved by the Enginner. Any method of removal to be used shall ensure that no damage is done to the existing web, vertical stiffener or welds connecting these elements. Cutting shall be done in a manner such that the paint on the opposite face of the web is not damaged. If damage occurs, the damaged area shall be repainted at the contractor's expense and procedures shall be modified to prevent damage at subsequent removal locations.
- 2. Remove material between cut and web by grinding and grind smooth at web suface. Web plate surfaces shall have a roughness average (Ra) of 250µ in. or less. Grinding equipment shall be approved by the Engineer. The grinding operation should not gouge the girder web P.
- 3. The web surface at the modification shall be inspected using dye penetrantt or magnetic particle methods. Any cracks found shall be identified and reported to the Bureau of Bridges and Structures for further disposition.

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel Repair	Pound	8,240

REVISIONS NAME	DATE	ILLINOIS DEPARTM	ENT OF TRANSPORTATION
HOME		NB DAN RYAN	N RYAN EXPRESSWAY) & ELEVATED BRIDGE & STREETS
		STRUCTURAL	L STEEL REPAIR
		SCALE: NTS DATE: 7/22/2005	DRAWN BY: BDC CHECKED BY: BLU



BOWMAN, BARRETT & ASSOCIATES INC.
CONSULTING ENGINEERS
Chicago, Illinois
312 228.0100
www.bbandainc.com

Grind corner of WT to clear existing web welds

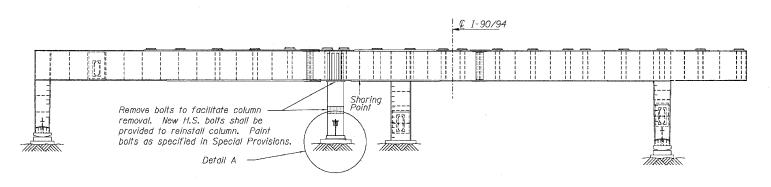
Exist. 18"x1^l2" flange plate

VIEW A-A

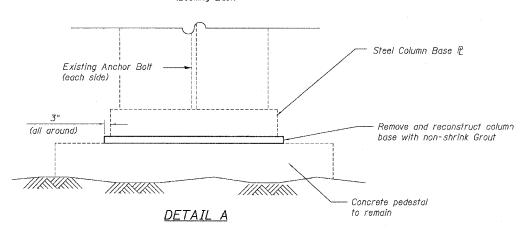
as needed

DETAIL E HOAN-TYPE CONNECTION REPAIR

TYPICAL CONNECTION BRACKET DIMENSIONS



PIER 2 - WEST ELEVATION (Looking East)

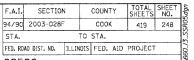


SUGGESTED PROCEDURE FOR REPAIR DETAIL

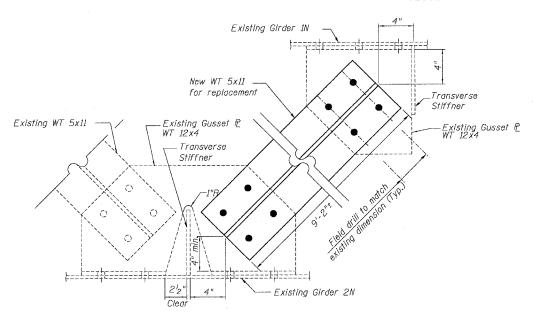
- Shore steel cap beam to provide temporary support for live load and impact only. A minimum of two support points shall be located at intermediate stiffener locations under cap beam webs as shown in the plan. See Special Provisions for Temporary Shoring and Cribbing and Table for Min. Jacking Capacity. Shoring and cribbing cap should provide gap at column base to facilitate removal.
- 2. Unbolt column at cap connection and mid-height splice and remove. Unbolt anchor bolts and lift bottom portion of column to facilitate existing grout pad removal.
- Remove existing grout and roughen existing pedestal over area of new grout base. Care shall be used not to damage anchor bolts.
- 4. Once removal and preparation have been completed to the satisfaction of the Engineer, reinstall column parts. Provide new H.S. boits for all removed or loosened bolts. Re-use of existing bolts will not be allowed. Leave anchor bolts unfastened.
- 5. Form and pour new non-shrink grout base. Ensure full contact with column base and allow grout to properly cure.
- 6. Remove shoring and cribbing when directed by the Engineer. Re-install anchor nuts and tighten as directed by the Engineer. Paint new bolts and nuts as necessary.

NOTES

- 1. This repair work shall be completed during Stage I of the mainline deck rehabilitation and prior to any deck work in Spans 2 & 3 of S.N. 016-1116. Adjacent railroad yard truck ramp shall be maintained. Contractor shall coordinate with Railroad and Rard for partial ramp closure.
- Contractor can provide alternative scheme to remove existing grout without complete removal of column. The alternative scheme shall be approved by Engineer prior to contruction.
- Costs of furnishing, erecting and removal of structural members, traffic control, and materials required in this work shall be included in the cost of "Column Repair". Temporary Shoring and Cribbing shall be paid separately.



62580



DETAIL H BRACING MEMBER REPLACEMENT

(S.N. 016-1116, Span 3)

Notes: This work shall require temporary sidewalk and lane closure of South Canal Street in accordance with Highway Standards 701801 & 701501.

SUPPORTING REACTIONS & MINIMUM JACKING CAPACITY

S/N	Pier	(LL+I) (k)	Min. Jack. Capacity (k) (Each)
016 - 1116	2	640	480

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel Repair	Pound	115
Column Repair	Lump Sum	1
Temporary Shoring and Cribbing	Each	1

REVISIONS
NAME
DATE
F.A.I. 94/90 (DAN RYAN EXPRESSWAY)
NB DAN RYAN ELEVATED BRIDGE
REPAIR FROM 15TH TO 28TH STREETS
STRUCTURAL STEEL REPAIR
DETAIL G & H
SCALE: NTS
DETAIL G & H
DETAIL G &

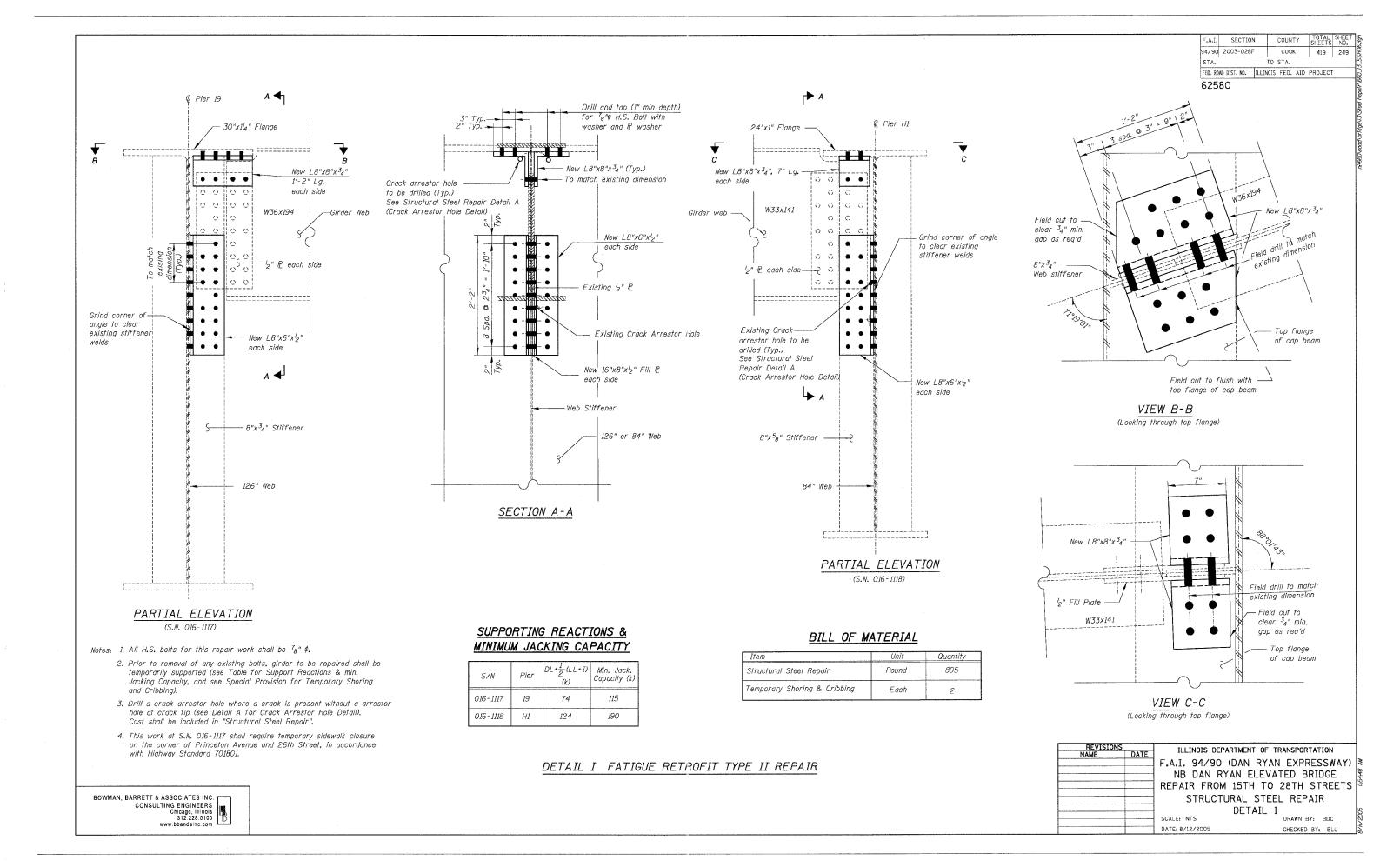
CHECKED BY: BLL

DATE: 8/12/2005

DETAIL G COLUMN BASE REPAIR

SN 016-1116, Pier 2

BOWMAN, BARRETT & ASSOCIATES INC.
CONSULTING ENGINEERS
Chicago, Illinois
312 228.0100
www.bbancainc.com



ELASTOMERIC BEARING SCHEDULE (TYPE I AND II)

Bearing Location	Pier	Туре	Bearing Designation	No. Req. 1	d Th			Eld	astomeric	Bearing				·	Top Plate	!		В	ottom Pla	te			Anchor B	olts				Side Reta	iner	
				1		We	Le	Te	Τp	Nρ	Ts	Ns	Wt	Lt	Ttn	Tts	Rt	Wb	Lb	Tb	Da	La	Rb	H	Wρ	У	Z	ţ	Нр	V
	8	I	12-b	11	5 ¹³ 16	12	18	2 ¹³ 16	916	4	316	3	13	20	3	3	9	-	-	-	1/2	18	12 ⁷ 8	-	3	234	5/2	58	134	5 ³ 4
	10	I	11- a	11	5	11	16	238	1/2	4	8	3	12	18	2716	21316	8	-	-	-	11/4	15	11/2	-	234	238	434	2	1/2	54
	12	I	12-b	10	576	12	18	21316	916	4	316	3	13	20	2/16	213/16	9	-	-	-	$l_{2}^{\prime }$	18	12 78	-	3	234	512	58	134	5 ³ 4
016-0137	14	I	12-b	10	5716	12	18	21316	916	4	316	3	. 13	20	2716	213/16	9	-	-	-	1/2	18	12 78	-	3	234	5½	5 ₈	134	5 ³ 4
	16	I	12-b	10	576	12	18	213/16	916	4	3/6	3	13	20	276	213/16	9	-	-	-	11/2	18	12 78	-	3	234	5/2	5 ₈	134	5 ³ 4
	20	I	11- a	11	5	11	16	238	3	4	1/8	3	12	18	276	21316	8	-	-	-	14	<i>1</i> 5	11/2	-	234	23 ₈	434	2	I_2^l	514
	22	I	12-b	11	5716	12	18	21316	9 ₁₆	4	316	3	13	20	2716	213/16	9	-	-	-	1/2	18	12 78	-	3	234	5 ¹ 2	⁵ 8	134	5 ³ 4
					— ,			.,	-		2		•								.,		7		7	0.7	=1		, , ,	73
016-1110	2	I	15-c	8	7'2	15	24	4'2	34	5	³ 16	4	16	26	3	3	12	-	-	-	12	18	15 78	-	3	234	5/2	- 8	134	+ / 8 -
010 1110	5	I	14 - c	13	75/6	14	22	4316	"16	5	3 ₁₆	4	15	24	3	3	11	-	-	-	1/2	18	14 ⁷ 8	-	3	234	5/2	⁵ 8	134	7'4
	3	II	13-b	8	85	13	20	31516	5,	4	3,6	3	14	22	3	3	10	14	3314	1/2	1'2	18	1378	2	3	234	5/2	58	134	67 ₈
	4	7	13-b	8	6%	13	20	3/10	5,	4	3,,,	3	14	22	3	3	10	-			1/2	18	13 7 _B	-	3	23/	5/2	5 _R	134	6
010 1111	6	7	13-b	8	66	13	20	3/6	5,	4	3,c	3	14	22	3	3	10	_	-	-	1/2	18	137 _B	-	3	234	5/2	5 _R	134	6
016 - 1111	8	11	13-b	8	8/2	13	20	315,6	5,	4	316	.3	14	22	.3	.3	10	14	3314	1/2	1/2	18	1378	2	3	234	5/2	-5 ₈	134	67 ₈
	9	1	13-b	8	6 g	13	20	31/6	5,	4	3,0	3	14	22	3	3	10	-			1/2	18	1378	-	3	234	5/2	5 _R	134	6
	11	7	13-b	8	6/8	13	20	3/6	5,	4	3,0	3	14	22	3	7	10			-	1/2	18	13 %	-	3	234	5/2	5,	13,	6
	12		100	+	108			J 716	8		16					 					1	10	100	 						<u> </u>

F.A.I.	SECTION	1	COUN	ſΥ	SHEETS	NO.
94/90	2003-028F		COOL	(419	250
STA.		TO	STA.			
FED. ROA	AD DIST. NO.	ILLINOIS	FED.	AID	PROJECT	-

62580

SUPPORTING REACTIONS AND MINIMUM JACKING CAPACITY

S/N	Pier	DL+ 2(LL+I)	Min. Jack. Capacity (k)			
	8	130	195			
	10	113	170			
	12	126	190			
016-0137	14	126	190			
	16	113	170			
	20	130	195			
	22	136	205			
010 1110	2	231	350			
016 - 1110	5	173	260			
	3 .	161	245			
	4	161	245			
016 - 1111	6	161	245			
	8	161	245			
	9	161	245			
	11	161	245			
	1	480	720			
016 - 1116	1(GN2 - 4)	365	550			



included with Elastomeric Bearing

TYPE I ELASTOMERIC EXP. BRG.

Assembly, Type II.

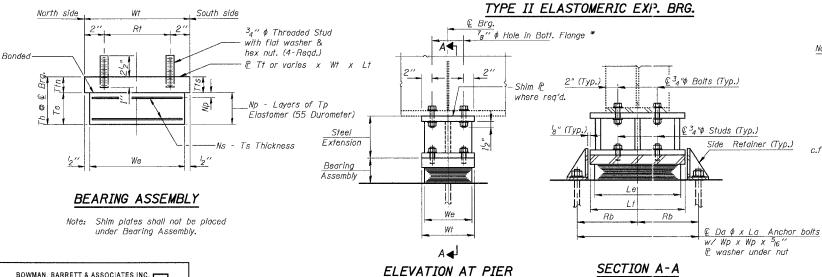
Steel Np- Layers of Tp Élastomer (55 Durometer) -Bearing Assembly - T Steel Plates Tb x Wb x Lb Wb/2 Wb/2 -Shim 🕆 where rea'd. lg" elastomeric neoprene leveling pad according to Article 1052.02 of BOTTOM BEARING ASSEMBLY *Field drill holes. Cost the Standard Specifications Cost

included in Erecting Bearings

34"\$ Bolts (Typ.) £34"¢ Studs (Typ.) Side Retainer (Typ.) Le/2 Le/2

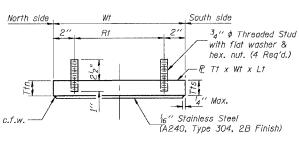
 $\[\[\] \]$ Da $\[\phi \]$ X La Anchor bolts with Wp x Wp x $\[\] \[\] \[\]$ washer under nut. Hp ♥ Holes in bottom ₽.

SECTION B-B



Wb

ELEVATION AT PIER



1. For side retainer detail see sheet no 251

3. For Steel Extension details and dimensions

see sheet no 251

2. All values in Elastomeric Bearing Table are inches.

TOP BEARING ASSEMBLY FOR TYPE II BEARINGS

PROCEDURE TO JACK AND REMOVE **EXISTING BEARINGS**

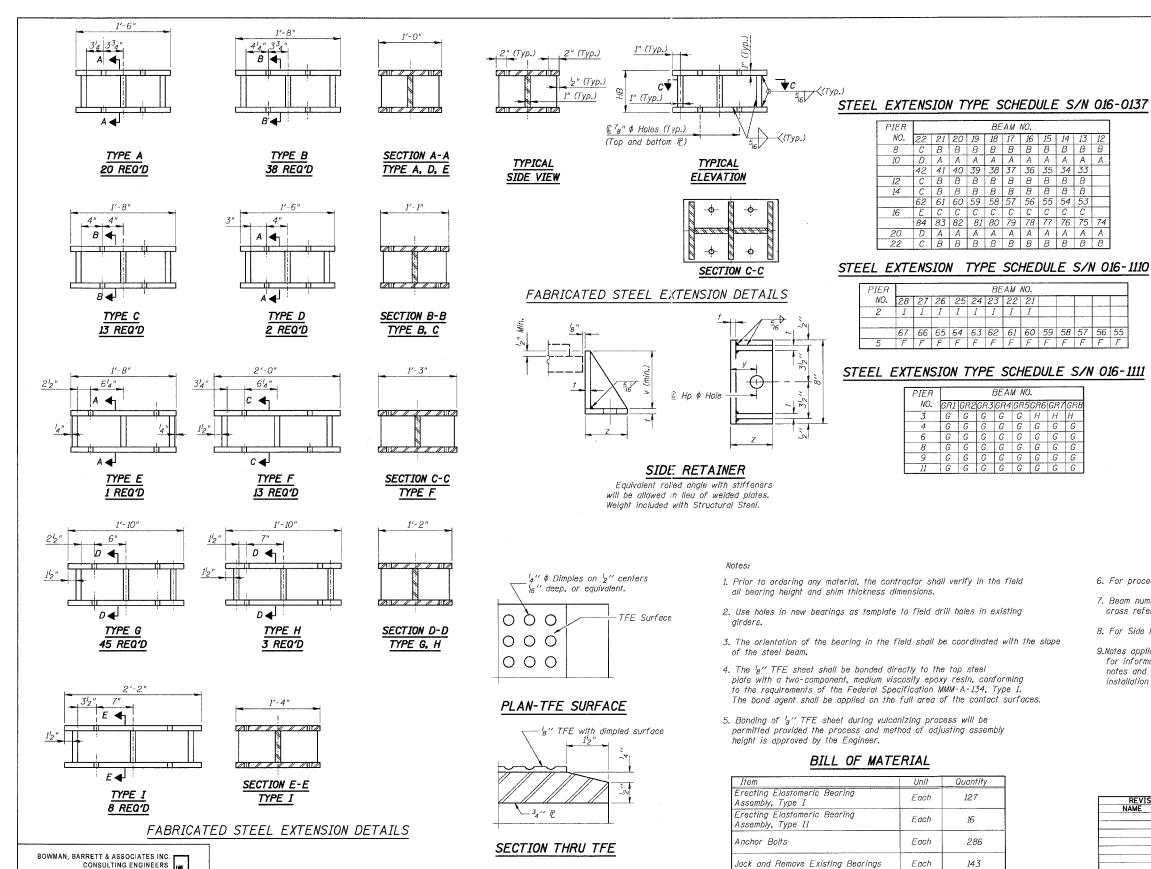
- 1. See Special Provision "Jack and Remove Existing Bearings"
- 2. Jacking and removing existing bearings, including top and bottom plates and lead plates, shall be done after deck scarification is completed and before the new overlay is poured.
- 3. See table for support reactions and min. jacking capacities.
- 4. For jacking capacity see table above.
- 5. Burn existing anchor bolts flush with the top of existing concrete surface. Grind existing anchor bolts smooth and seal with epoxy. Cost is included with "Jack and Remove Existing Bearings".
- 6. Top plates welded to the bottom flange of beams shall be removed using air arc-methods. Grind smooth all weld material remaining on the bottom flange.
- 7. The new bearings shall be in place and jacks shall be lowered before the overlay is poured.
- 8. Notes applicable to bearing manufacturer are provided for information only. Erection contractor shall only refer to notes and details applicable for existing bearing removal and installation of provided bearings and extensions.
- 9. If the analysis submitted by the contractor for the jacking / temporary support system to be used shows temporary stiffereners are required to prevent web crippling or buckling the stiffeners shall be steel and bolted to the web. If the stiffeners are not required, hardwood timbers shall be installed tightly between the top and bottom flange to prevent flange rotation.

ILLINOIS DEPARTMENT OF TRANSPORTATION F.A.I. 94/90 (DAN RYAN EXPRESSWAY) 🗦 NB DAN RYAN ELEVATED BRIDGE REPAIR FROM 15TH TO 28TH STREETS ELASTOMERIC BEARING DETAILS SCALE: NTS DRAWN BY: LM DATE: 7/22/2005 CHECKED BY: BLU

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F.A.I. SECTION COUNTY 94/90 2003-028F COOK 419 251 STA. TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

62580

HB SCHEDULE S/N 016-0137

PIER		BEAM NO.												
NO.	22	21	20	19	18	17	16	15	14	13	12			
8	91/16	91/6	91/16	91/16	91/16	978	9 ¹³ 16	91/16	91/16	91/16	91/16			
10	10/2	10 2	10 %	10½	1012	101/2	10 ¹ 2	11/2	10 ¹¹ 16	10%	10%			
	42	41	40	39	38	37	36	35	34	33				
12	10/16	1016	1016	10/6	1016	10 ⁷ 16	1014	1016	1016	1016				
14	1016	1016	1016	1016	1016	10 ⁷ 16	1014	1016	1016	1016				
	62	61	60	59	58	57	56	55	54	53				
16	10/16	1016	1016	1016	1016	1016	10 ¦6	1016	1016	1016				
	84	83	82	81	80	79	78	77	76	75	74			
20		101/2			101/2		10½			10%				
22	10/6	1016	10/6	10/6	10 ³ 16	105/6	1016	1016	1016	1016	1016			

HB SCHEDULE S/N 016-1110

PIER		BEAM NO.											
NO.	28	27	26	25	24	23	22	21					
2	1178	11 ⁷ 8	11 ⁷ 8	11 ⁷ 8	11 ⁷ 8	1178	11^{7}_{θ}	11 ⁷ 8					
	67	66	65	64	63	62	61	60	59	58	57	56	55
5	916	91/6	916	916	916	9/6	91/6	91/6	916	9/16	91/6	916	9/6

HB SCHEDULE S/N 016-1111

PIER			į	BEAI	M NO		-	
NO.	GR1	GR2	GR3	GR4	GR5	GR6	GR7	GR8
3	97 ₁₆	9716	9716	9716	9716	9916	9916	9916
4	11 ¹³ 16	11 ¹⁵ 16	11 ¹⁵ 16	11 ¹⁵ 16				
6	11 ¹³ 16	11 ¹⁵ 16	11 ¹⁵ 16	11 ¹⁵ 16				
8	9716	9716	9716	9716	9716	9916	9916	9916
9	11 ¹³ 16	11 ¹⁵ 16	11 ¹⁵ 16	11 ¹⁵ 16				
11	11 ¹³ 16	11 ¹⁵ 16	11 ¹⁵ 16	11 ¹⁵ 16				

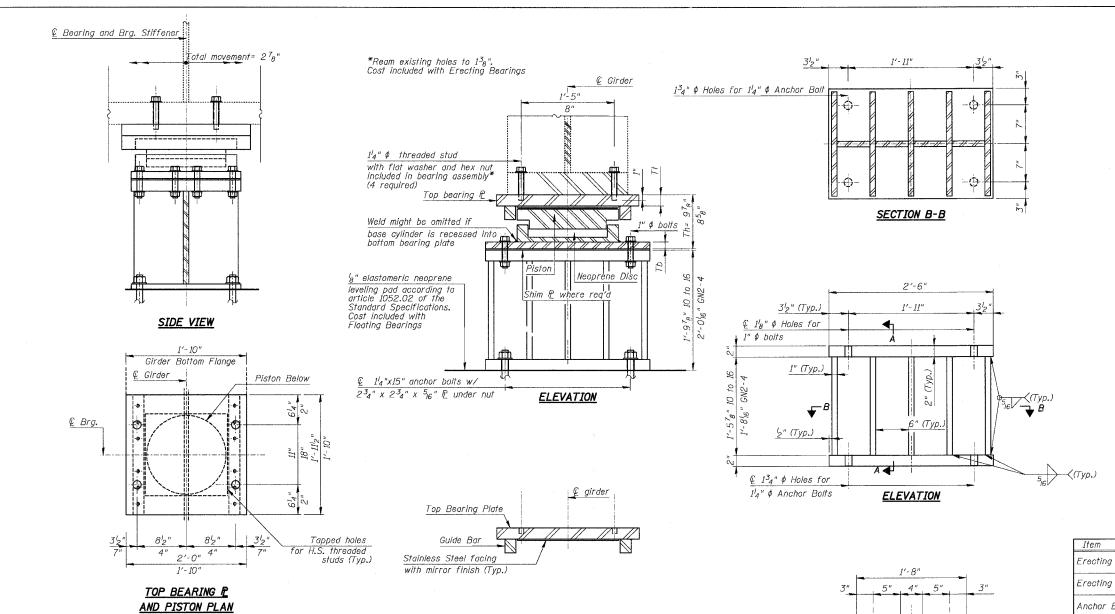
- 3. The orientation of the bearing in the field shall be coordinated with the slope
- plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I.

Item	Unit	Quantity
Erecting Elastomeric Bearing Assembly, Type I	Each	127
Erecting Elastomeric Bearing Assembly, Type II	Each	16
Anchor Bolts	Each	286
Jack and Remove Existing Bearings	Each	143

- 6. For procedure to jack and remove existing bearing see sheet 250.
- 7. Beam numbers reference as-built drawings, see Framing plans for cross reference.
- 8. For Side Retainers dimensions see sheet 250.
- 9.Notes applicable to bearing manufacturer are provided for information only. Erection contractor shall only refer to notes and details applicable for existing bearing removal and installation of provided bearings and extensions.

REVISIONS NAME	DATE	ILLINOIS DEPARTMENT	OF TRANSPORTATION RYAN EXPRESSWAY)	2
			_EVATED BRIDGE TO 28TH STREETS XTENSION	
		SCALE: NTS DATE: 7/22/2005	DRAWN BY: LM CHECKED BY: BLU	7/20/2005

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F.A.I. SECTION COUNTY 94/90 2003-028F COOK 419 252 TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

62580

Notes:

- 1. The $_{B}^{\prime}$ " TFE sheet shall be bonded directly to the piston with a two component medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MM-A-134, Type 1. The bond agent shall be applied to the full area of the contact surfaces.
- Total Bearing Heights (Th) are based on values taken from a specific manufacturer's design tables. Actual bearing heights may differ from contract plans. Contractor to verify bearing heights and adjust steel extension height if required.
- 3. The Vertical Design Load in table is the actual controlling vertical service load.
- 4. Floating Bearings dimensions and details are based on the DS Brown Versifiex HLMR Uni Directional PMG series. Contractor shall make necessary modifications based on the actual bearings provided.
- 5. As alternates to the bolted connection shown, the guide bars may be connected to the top bearing plate by groove welds or the guide bar and top bearing may be fabricated as a single piece.
- 6. Bearing manufacturer shall coordinate with steel extension manufacturer in order to provide required height from top of pier to bottom of beam
- 7. Notes applicable to bearing manufacturer are provided for information only. Erection contractor shall only refer to notes and details applicable for existing bearing removal and installation of provided bearings and extensions.
- 8. For Jacking procedure see sheet 250.

SN 016-1116 - BILL OF MATERIAL

Item	Unit	Quantity
Erecting Floating Bearing, Guided Exp., 550 Kips	Each	7
Erecting Floating Bearing, Guided Exp., 400 Kips	Each	3
Anchor Bolts	Each	40
Jack and Remove Existing Bearings	Each	10

SN 016-1116 - FLOATING BEARING SCHEDULE

2" (Typ.)

SECTION A-A

FABRICATED STEEL

EXTENSION DETAIL

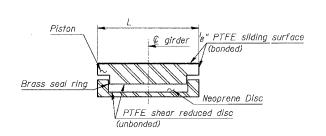
TOTAL OF 10 REQ'D.

Beam No.	Vertical Design Load, k	Lateral Design Load, k	Total Req'd Movement (in.)	T†	Tb	Th	L	D
10-16	550	110	278	234	2	978	181/2	1734
GN2-4	400	80	2 ⁷ 8	2	2	8 ⁵ 8	<i>1</i> 5	<i>1</i> 5

Dimensions are in inches. Ream numbers reference as-built drawings, see Framing Plans for cross reference.

				1
REVISIONS NAME	DATE	ILLINOIS DEPARTMENT	OF TRANSPORTATION	1
INAME	DATE	F.A.I. 94/90 (DAN	RYAN EXPRESSWAY)	AM
			LEVATED BRIDGE	4.49
		REPAIR FROM 15TH	TO 28TH STREETS	11.5
		FLOATING	BEARING	
		DET	AILS	33
		SCALE: NTS	DRAWN BY: LM	120
		DATE: 9/12/2005	CHECKED BY. BILL	15

TOP BEARING PLATE ASSEMBLY



PISTON ASSEMBLY

BOTTOM BEARING P AND BASE CYLINDER PLAN

246"

© Girder

-⊕-

Base Cylinder

(1)

-⊕-

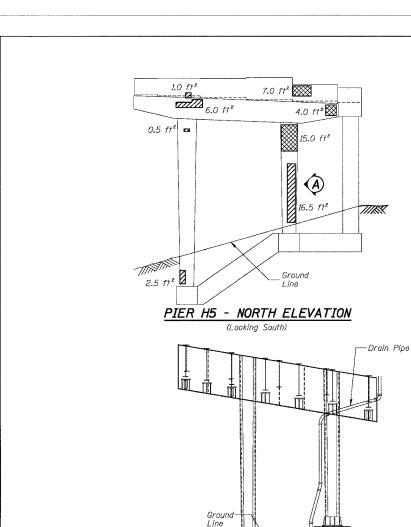
-(-)-

10 REQ'D (SEE SCHEDULE THIS SHEET)

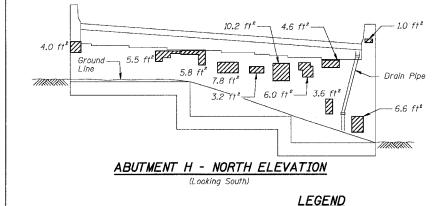
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Brg.

FLOATING BEARING DETAIL



PIER H3 - NORTH ELEVATION (Looking South)



7//8/1/8/

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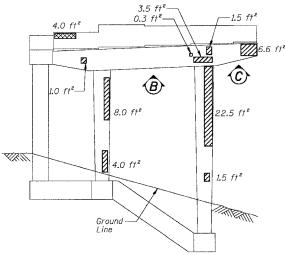
High Performance Enhanced Shotcrete

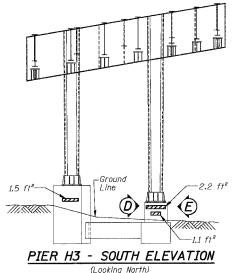
Formed Concrete Repair (Depth Greater than 5")



Cracks to be Epoxy Sealed

A View Indicator





PIER H5 - SOUTH ELEVATION (Looking North)

> 6.0 ft² PIER H5 - VIEW C Looking up at Bottom of Cap

5.0 ft²

PIER H5 - VIEW A

Looking East at Side of Column

3.2 ft² 1.0 ft²

PIER H3 - VIEW D

28.4 ft²

PIER H3 - VIEW E Looking West at Side of Pedestal

SECTION

FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

27.8 ft2

PIER H5 - VIEW B

94/90 2003-028F

62580

COOK 419 253

Looking East at Side of Pedestal

REVISIONS	DATE	ILLINOIS DEPARTMENT OF TRANSPORTATION
NAME.	DATE	F.A.I. 94/90 (DAN RYAN EXPRESSWAY
		NB DAN RYAN ELEVATED BRIDGE
		REPAIR FROM 15TH TO 28TH STREET
	-	CONCRETE PIER REPAIRS
		SN 016-1118, PIERS H-3,H-5, S. ABUTMEN
	1	

QUANTITY

247.1

61.0

SQ. FT.

SQ. FT

B TM:

DATE: 7/22/2005

SCALE: NTS DRAWN BY: BDC CHECKED BY: BLU

Notes:

- 1. If top of pier cap concrete repairs extend under the bearings the Engineer shall determine if bearing removal is required to accomplish the repair. If directed to remove bearings, the Contractor shall jack and remove bearings according to the procedures outlined on sheet and the Special Provisions. The bearing removal and resetting will be paid at the contract unit price for "Jack and Remove Existing Bearings". New anchor bolts, if required, shall be provided as directed by the Engineer and will be paid at the contract unit price for "Anchor Bolts".
- 2. For concrete repairs greater than 5 inches in depth at cantilevered pier caps, temporary support shall be provided by the Contractor. Details for the temporary support system shall be submitted for review and approval. Concrete repairs at this locations shall be done one side at a time and coordinated with the MOT to assure no live load is directly above. The cost of shoring shall be included in the cost for "Temporary Shoring & Cribbing:".
- 3. Defect Locations and Areas based on inspections during Summer 2003. Total repair quantities have been increased by 15% to account for defect growth. Actual locations and areas of repair shall be determined in field by the Engineer.
- 4. Extra care shall be taken by the Contractor during the Concrete Repairs not to damage any conduits, junction boxes or drainage down spouts attached to the columns or pier caps. The Contractor shall remove and re-install any conduits, junction boxes or drainage down spouts that interfere with the concrete repairs at no additional cost to the department.

ITEM

High Performance

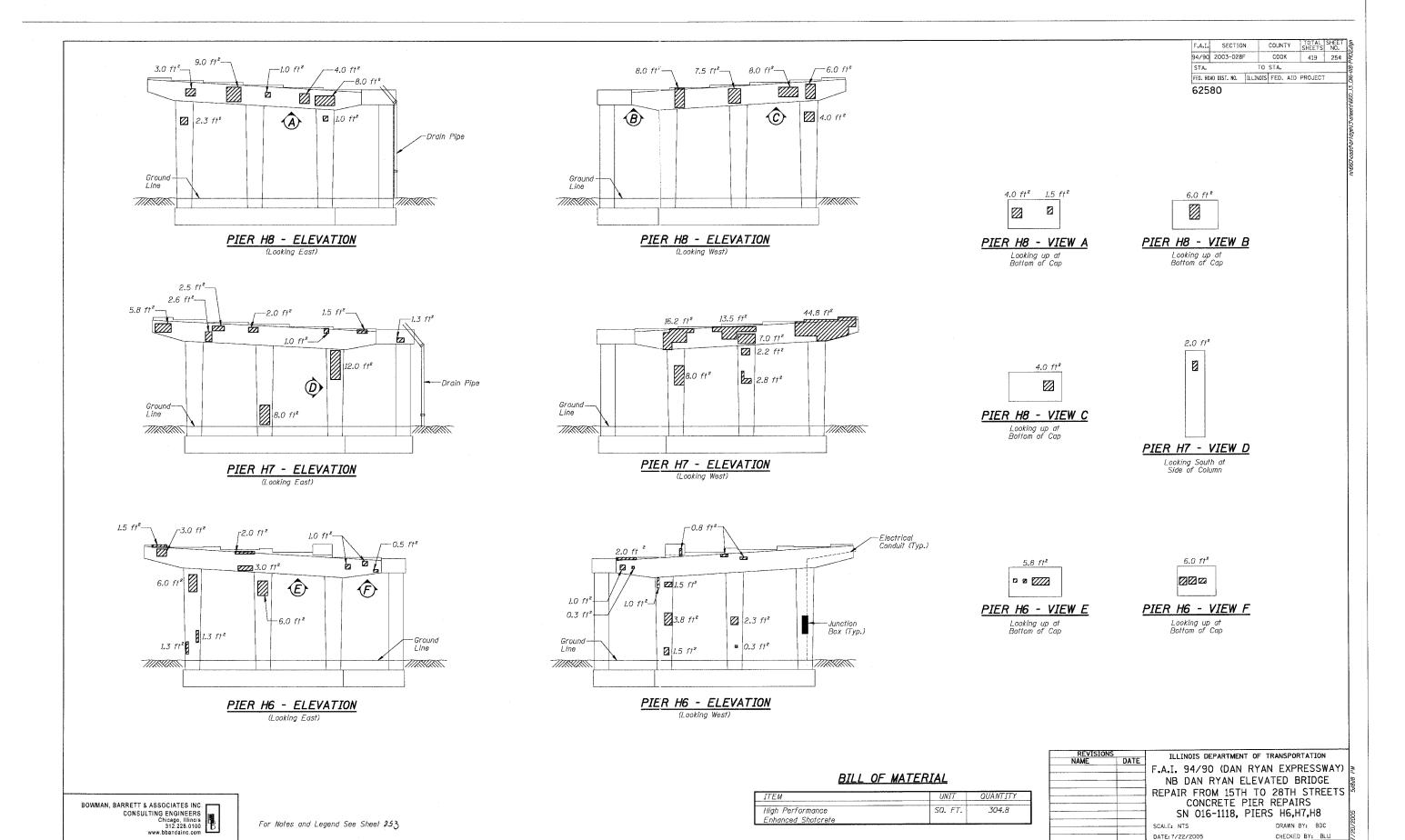
Enhanced Shotcrete

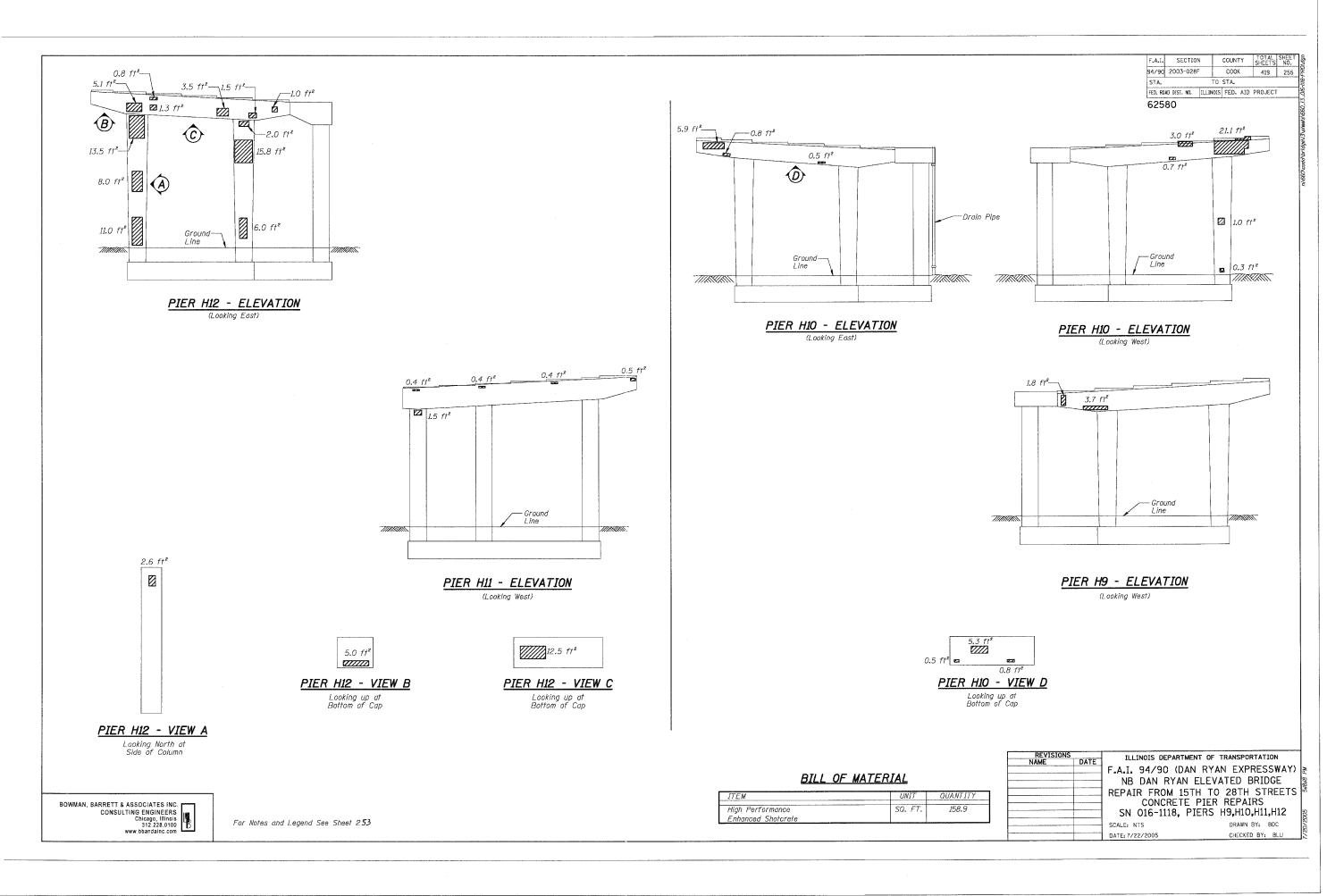
Formed Concrete Repair

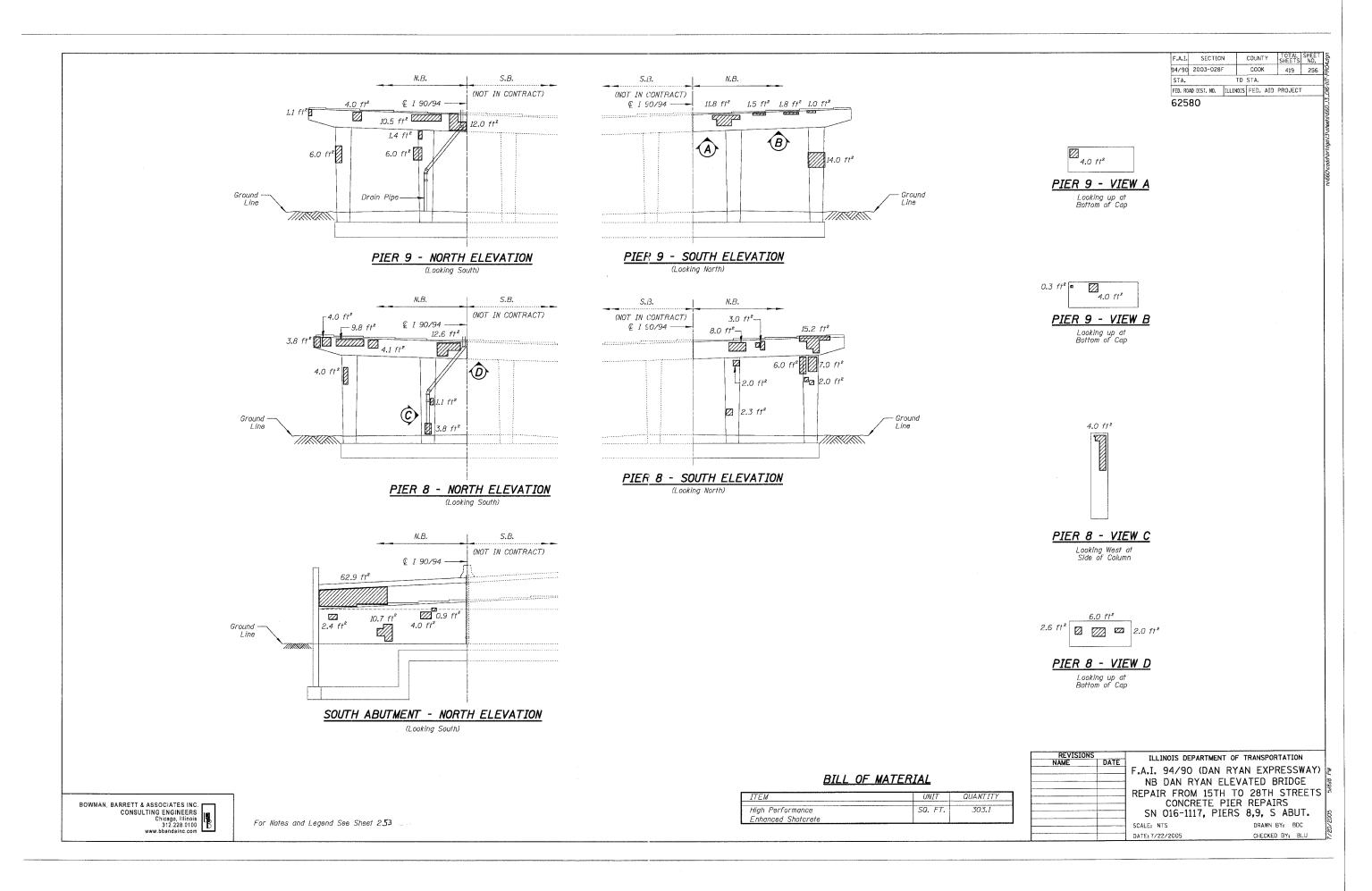
(Depth Greater than 5")

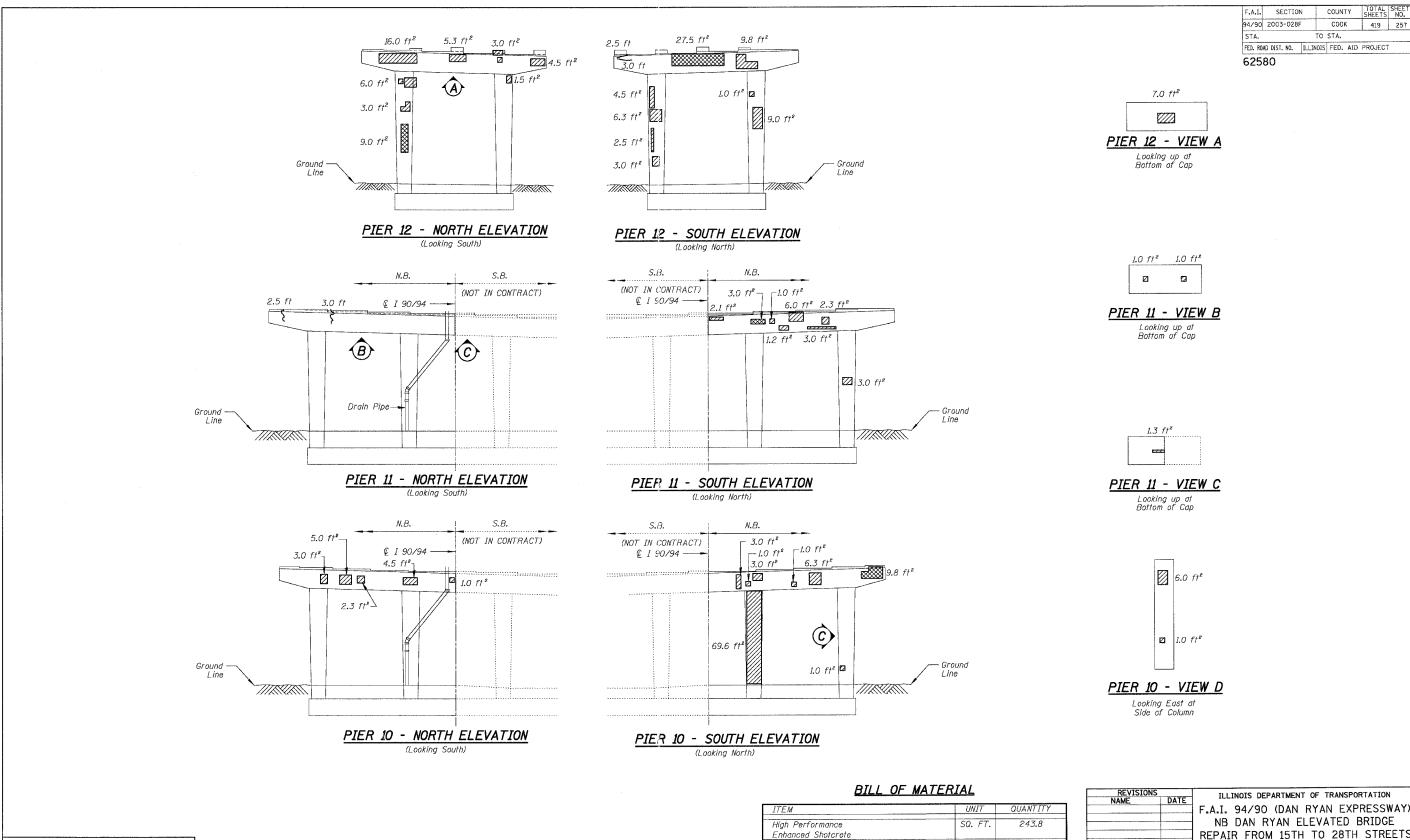
BILL OF MATERIAL

- 5. Repair one face of column at a time.
- 6. Views shown for piers with Found Defects only.
- 7. For Drainage Repairs, See Drainage Repair Detail Sheets.









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For Notes and Legend See Sheet 253

BILL OF MATERIAL					
ITEM	UNIT	QUANTITY			
High Performance Enhanced Shotcrete	SQ. FT.	243.8			
Formed Concrete Repair	SQ. FT.	56.7			

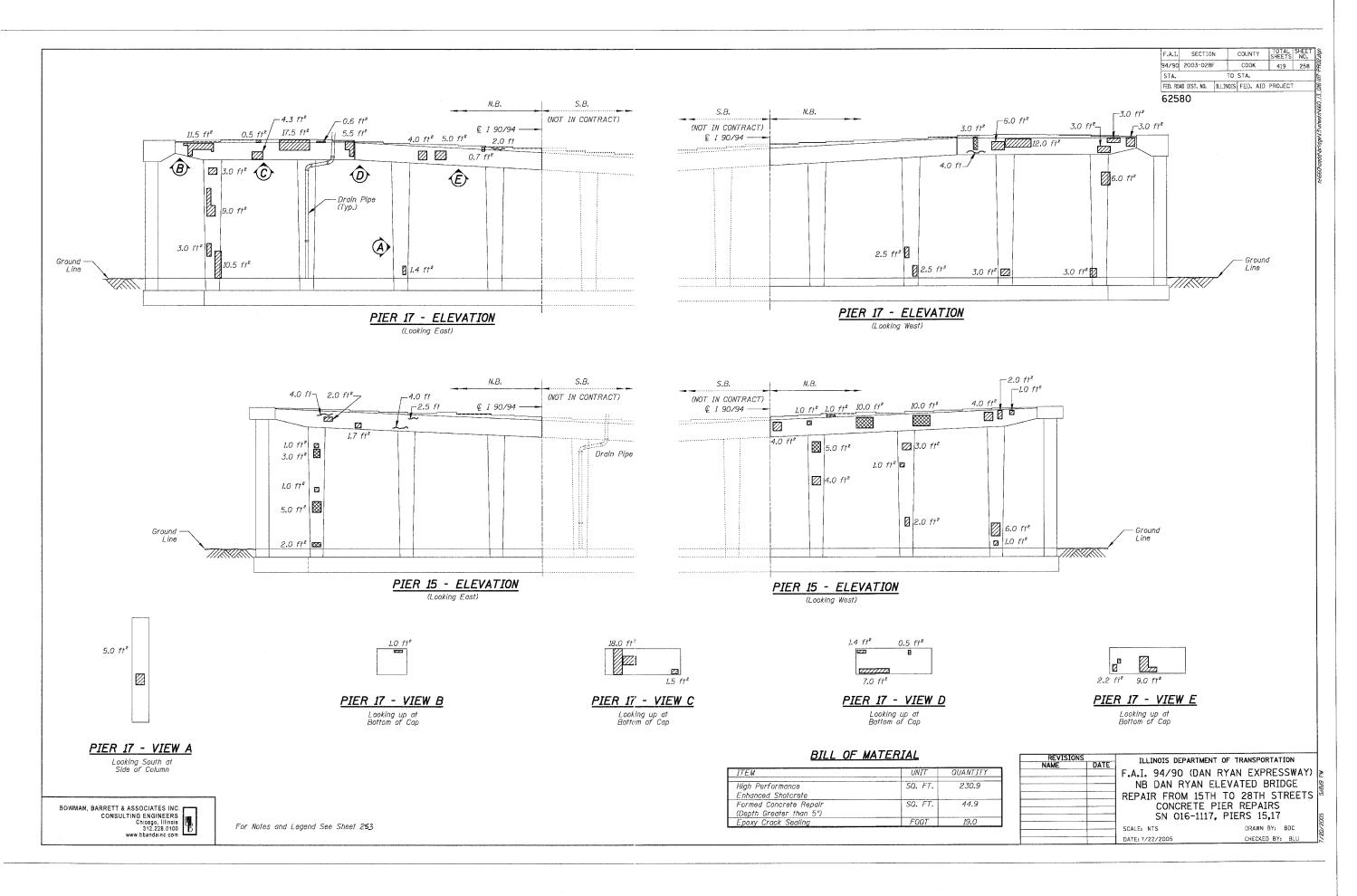
FOOT

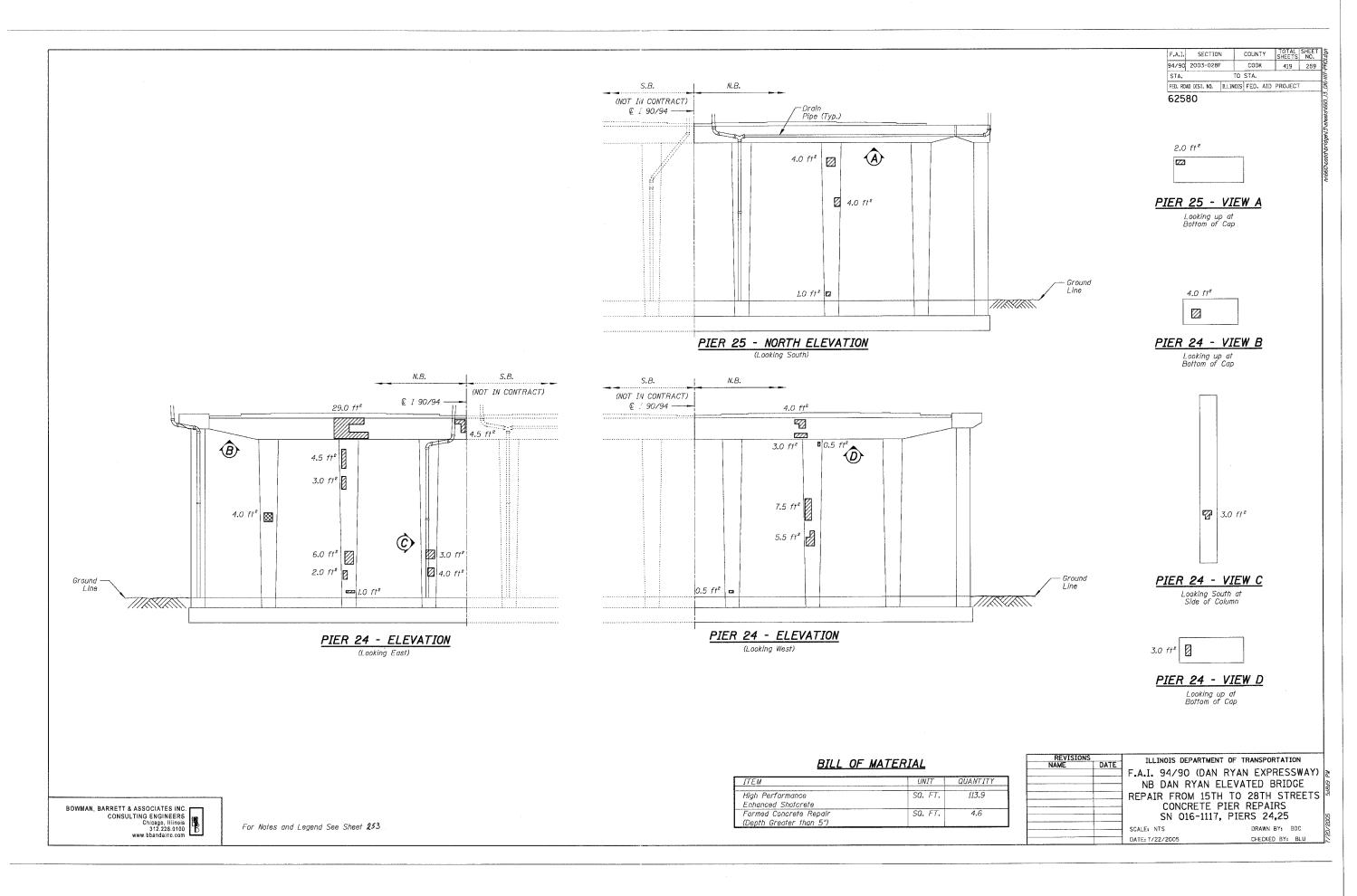
(Depth Greater than 5")
Epoxy Crack Sealing

	I NAME
TITY	
3.8	
.7	
.7	

F.A.I. 94/90 (DAN RYAN EXPRESSWAY) | NB DAN RYAN ELEVATED BRIDGE REPAIR FROM 15TH TO 28TH STREETS CONCRETE PIER REPAIRS SN 016-1117, PIERS 10,11,12

SCALE: NTS DATE: 7/22/2005 DRAWN BY: BDC CHECKED BY: BLU





62580

2.1 ft²

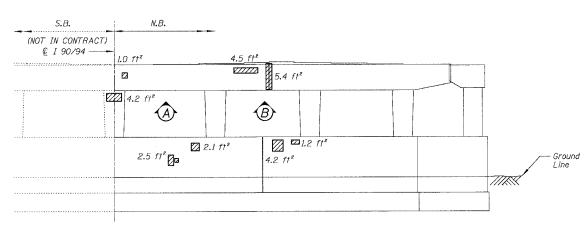
PIER 1 - VIEW A

Looking up at
Bottom of Cap

66.2 ft²

PIER 1 - VIEW B

Looking up at Bottom of Cap



PIER 1 - WEST ELEVATION

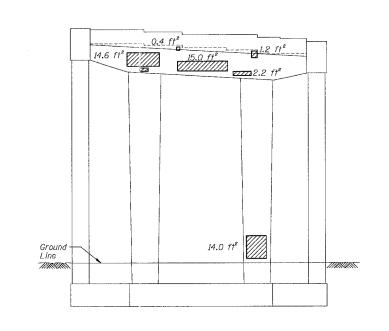
(Looking East)

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
High Performance	SQ. FT.	107.5
Enhanced Shotcrete	1	

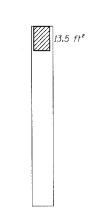
REVISIONS	5 1	THE THOSE OF BADTME	ENT OF TRANSPORTATION
NAME	DATE	F.A.I. 94/90 (DAI NB DAN RYAN REPAIR FROM 15 CONCRETE	N RYAN EXPRESSWAY) ELEVATED BRIDGE TH TO 28TH STREETS PIER REPAIRS -1116, PIER 1
		SCALE: NTS DATE: 7/22/2005	DRAWN BY: BDC CHECKED BY: BLU

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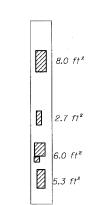
PIER E-2 - SOUTH ELEVATION

(Looking North)



PIER E-1 - VIEW A

Looking East at Side of Column



F.A.I. SECTION COUNTY TOTAL SHEETS NO.

TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

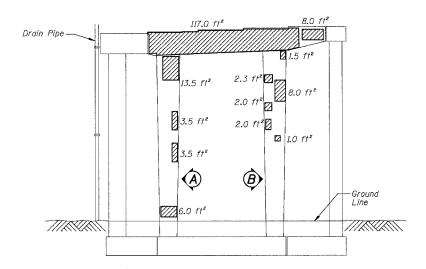
94/90 2003-028F

62580

COOK 419 261

PIER E-1 - VIEW B

Looking West at Side of Column



PIER E-1 - NORTH ELEVATION (Looking South)

13.2 ft2 4.0 ft2 3.8 ft2 2.3 ft 2 6.0 ft 2.0 ft² Ø 4.5 ft² 9.0 ft² 8.9 ft² 1.0 ft² – Ground Line 3.0 ft² 1.0 ft² 2 /// ///

PIER E-1 - SOUTH ELEVATION

(Looking North)



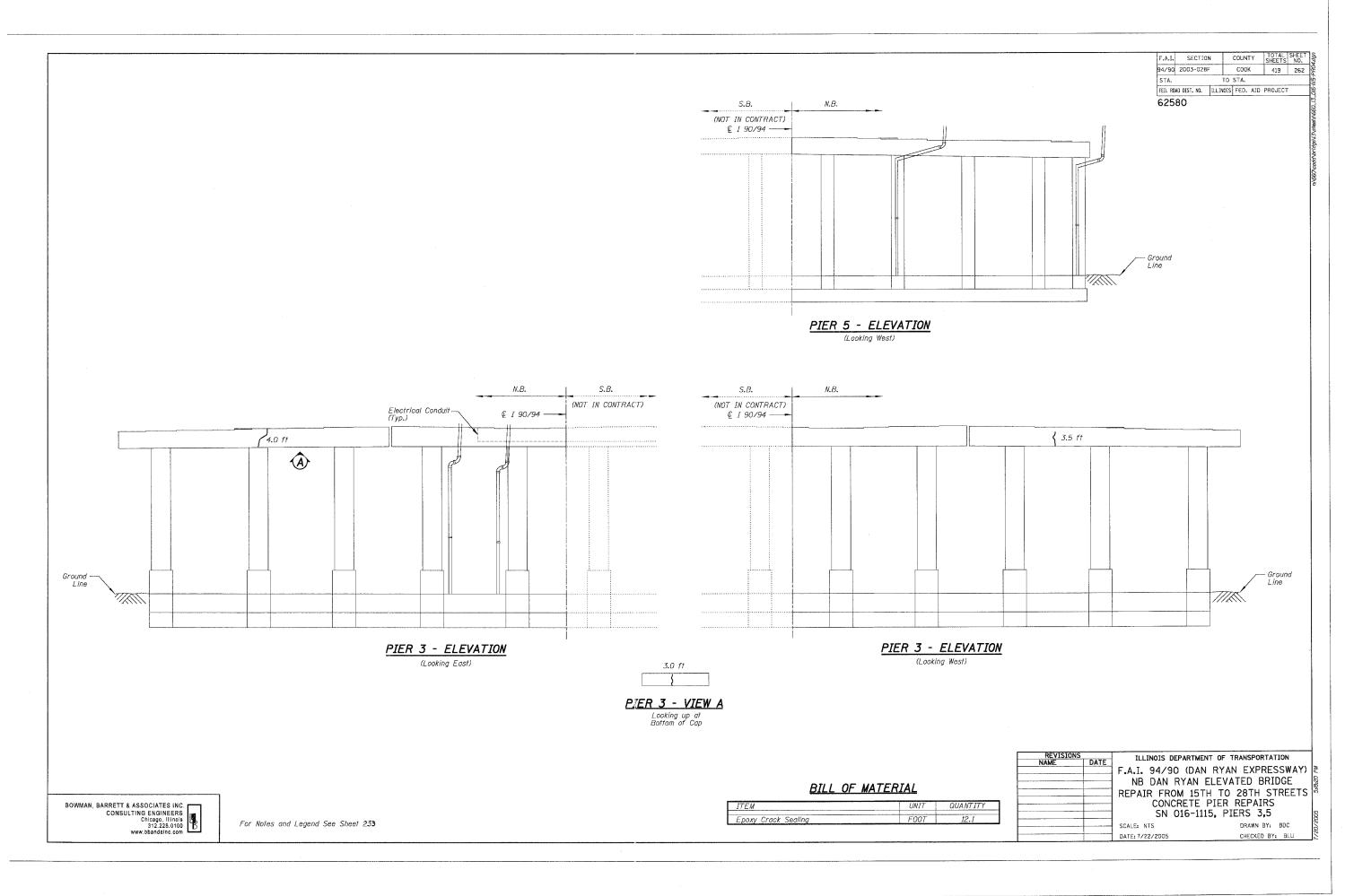
Looking up at Bottom of Cap

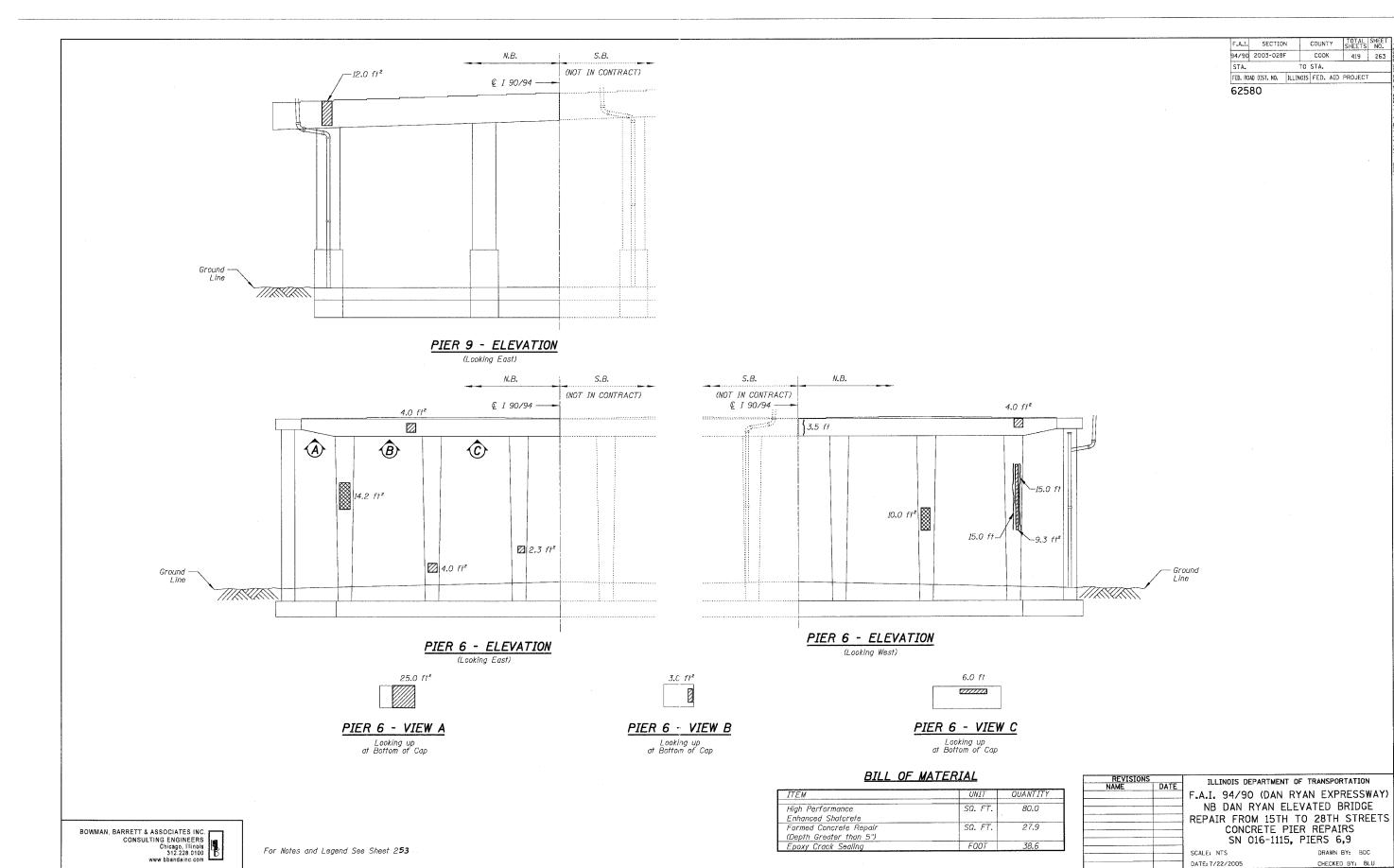
BILL OF MATERIAL

ITEM	UNIT	QUANTITY
High Performance Enhanced Shotcrete	SQ. FT.	436.0
Formed Concrete Repair (Depth Greater than 5")	SQ. FT.	10.2

ILLINOIS DEPARTMENT OF TRANSPORTATION F.A.I. 94/90 (DAN RYAN EXPRESSWAY) NB DAN RYAN ELEVATED BRIDGE REPAIR FROM 15TH TO 28TH STREETS CONCRETE PIER REPAIRS SN 016-1115, PIERS E-1,E-2 SCALE: NTS DRAWN BY: BDC DATE: 7/22/2005 CHECKED BY: BLU

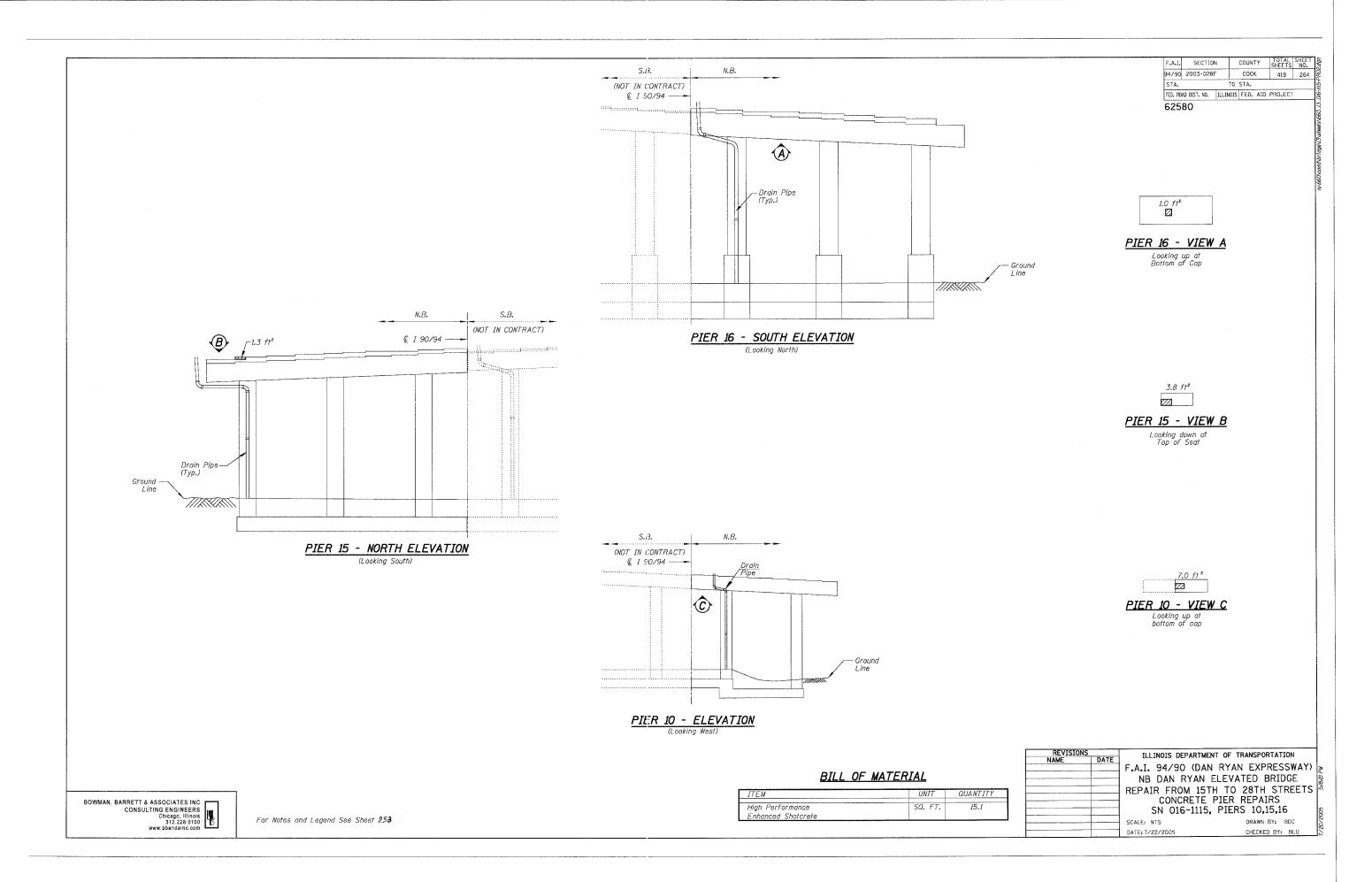
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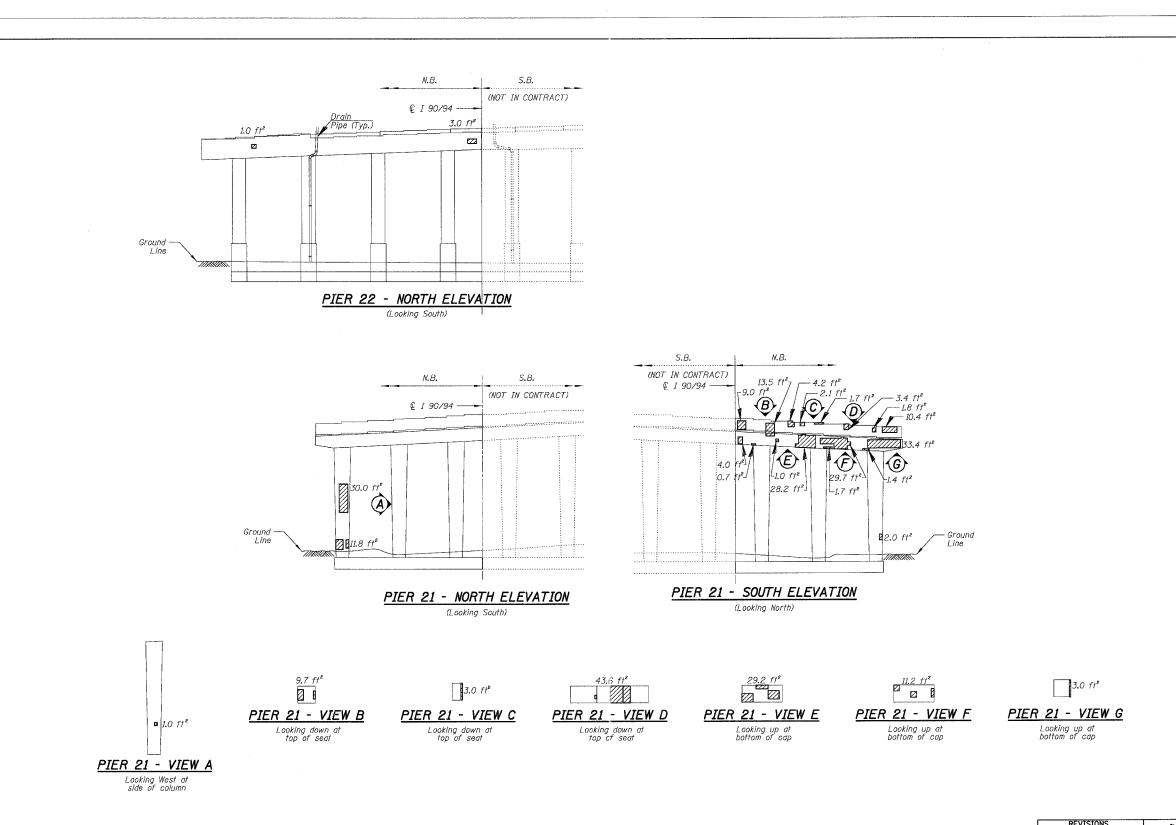




DRAWN BY: BDC

CHECKED BY: BLU





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 ITEM
 UNIT
 QUANTITY

 High Performance
 S0. FT.
 339.0

 Enhanced Shotcrete
 S0. FT.
 339.0

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ILLINOIS DEPARTMENT OF TRANSPORTATION
F.A.I. 94/90 (DAN RYAN EXPRESSWAY)
NB DAN RYAN ELEVATED BRIDGE
REPAIR FROM 15TH TO 28TH STREETS
CONCRETE PIER REPAIRS
SN 016-1115, PIERS 21,22

F.A.I. SECTION COUNTY TOTAL SHEETS NO.

STA. TO STA.

FED. ROAD DIST. NO. | ILLINOIS | FED. AID | PROJECT

COOK 419 265

94/90 2003-028F

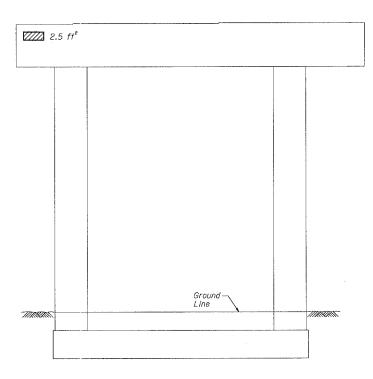
62580

 SCALE:
 NTS
 DRAWN BY:
 BDC

 DATE:7/22/2005
 CHECKED BY:
 BLU

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PIER 11 - SOUTH ELEVATION

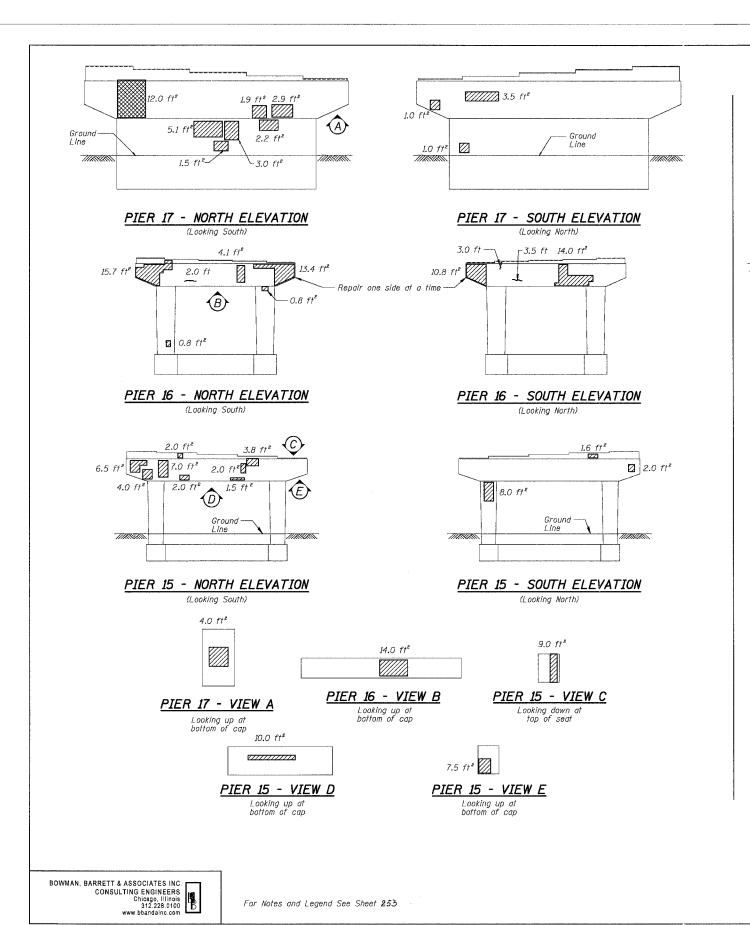
(Looking North)

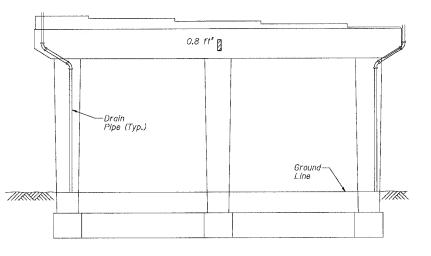
BILL OF MATERIAL

TTEM UNIT QUANTITY
High Performance S0. FT. 2.9
Enhanced Shotcrete

REVISION		TILINOIS DEPARTA	MENT OF TRANSPORTATION	
NAME	DATE	F.A.I. 94/90 (DA NB DAN RYAN REPAIR FROM 15	AN RYAN EXPRESSWAY) I ELEVATED BRIDGE TH TO 28TH STREETS E PIER REPAIRS	8
		SN 016-	44.40 DIED 44	20/2005
		SCALE: NTS DATE: 7/22/2005	DRAWN BY: BDC CHECKED BY: BLU	1/20/

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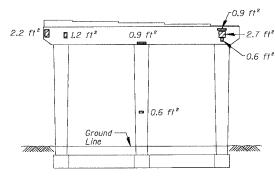






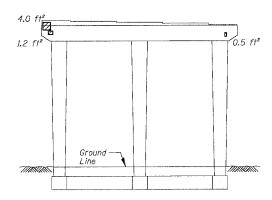
PIER 14 - NORTH ELEVATION

(Looking South)



PIER 13 - NORTH ELEVATION

(Looking South

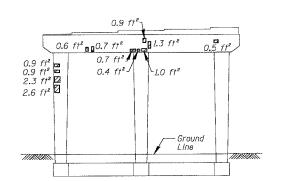


PIER 12 - NORTH ELEVATION

(Looking South)

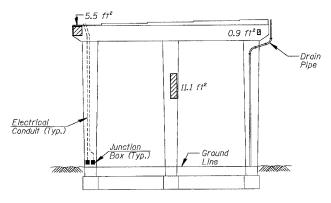
<u>BILL OF MATERIAL</u>

ITEM	UNIT	QUANTITY
High Performance Enhanced Shotcrete	SQ. FT.	244.4
Formed Concrete Repair (Depth Greater than 5")	SQ. FT.	13.8
Epoxy Crack Sealing	FOOT	9.8



PIER 13 - SOUTH ELEVATION

(Looking North)

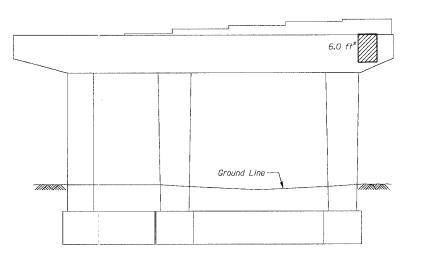


PIER 12 - SOUTH ELEVATION

(Looking North)

REVISION		ILLINOIS DEPARTMEN	NT OF TRANSPORTATION
NAME	DATE	NB DAN RYAN E REPAIR FROM 15T CONCRETE	RYAN EXPRESSWAY) ELEVATED BRIDGE H TO 28TH STREETS PIER REPAIRS ERS 12,13,14,15,16,17
		SCALE: NTS DATE: 7/22/2005	DRAWN BY: BDC CHECKED BY: BLU

F.A.I.	SECTION		COUN.	ГҮ	TOTAL	SHEET NO.
94/90	2003-028F		COO	<	419	268
STA.		ΤO	STA.			
FED. RO	AD DIST. NO.	ILLINOIS	FED.	AID	PROJECT	



PIER 40 - SOUTH ELEVATION
(Looking North)

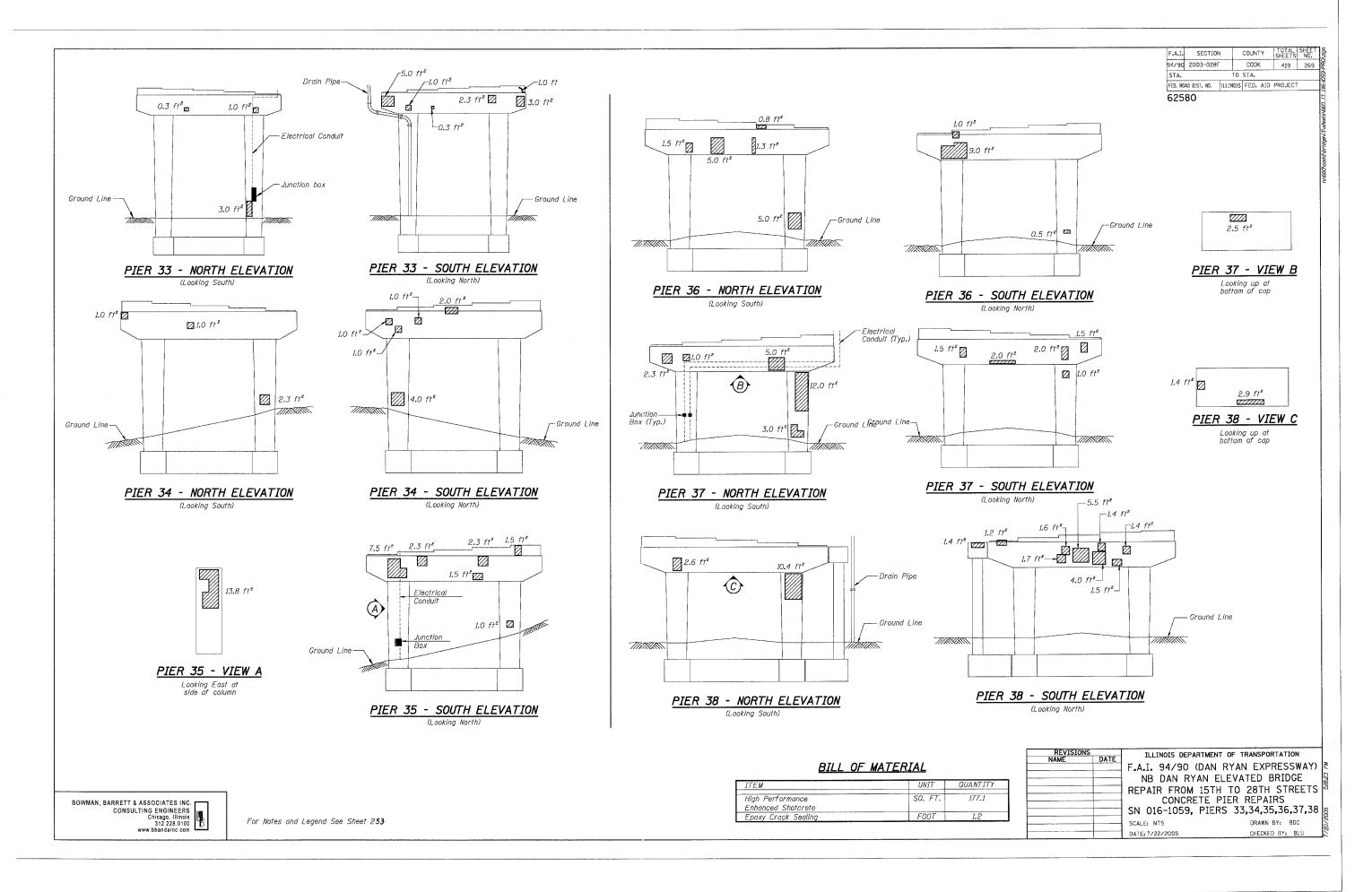
BILL OF MATERIAL

ITEM UNIT QUANTITY

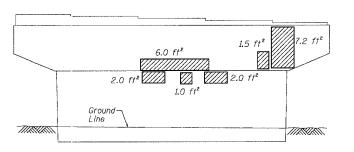
High Performance SQ. FT. 6.9
Enhanced Shotcrete

REVISION	NS DATE	ILLINOIS DEPARTM	MENT OF TRANSPORTATION
NAME DATE		NB DAN RYAN REPAIR FROM 15 CONCRETE	N RYAN EXPRESSWAY) ELEVATED BRIDGE TH TO 28TH STREETS PIER REPAIRS 1059, PIER 40
		SCALE: NTS	DRAWN BY: BDC
		DATE: 7/22/2005	CHECKED BY: BLU

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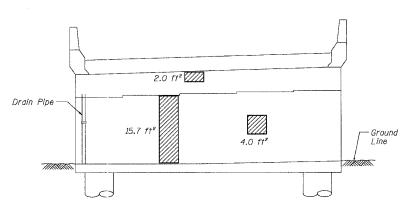


F.A.I.	SECTION	١	COUN	ΓΥ	SHEETS	NO
94/90	2003-028F		COO	(419	2
STA.		TO	STA.			
FED. RO	AD DIST. NO.	ILLINOIS	FED.	AID	PROJECT	



PIER 29 - SOUTH ELEVATION

(Looking North)



ABUTMENT 30 - NORTH ELEVATION (Looking South)

BILL OF MATERIAL

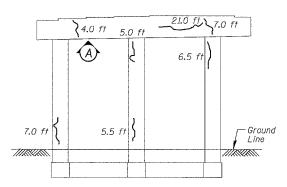
ITEM UNIT QUANTITY

High Performance SO. FT. 48.0

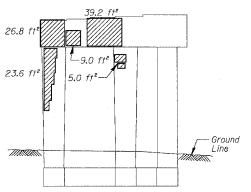
Enhanced Shotcrete

REVISION	VS.	TILINOIS DEPARTMENT	OF TRANSPORTATION
NAME DATE			
		F.A.I. 94/90 (DAN	RYAN EXPRESSWAY
		NB DAN RYAN E	LEVATED BRIDGE
		REPAIR FROM 15TH	TO 28TH STREETS
			PIER REPAIRS
		SN 016-1047, PIER	
		SCALE: NTS	DRAWN BY: BDC
		DATE: 7/22/2005	CHECKED BY: BLU

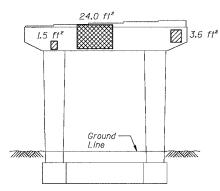
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PIER 22 - NORTH ELEVATION (Looking South)



PIER 23 - NORTH ELEVATION (Looking South)

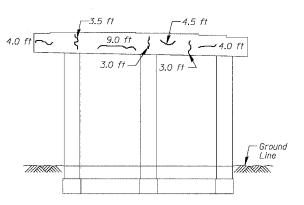


PIER 25 - NORTH ELEVATION (Looking South)

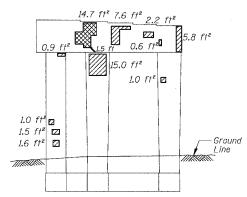


PIER 22 - VIEW A

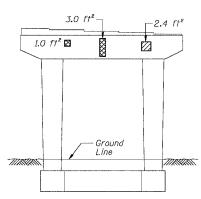
Looking up at bottom of cap



PIER 22 - SOUTH ELEVATION (Looking North)



PIER 23 - SOUTH ELEVATION (Looking North)



PIER 25 - SOUTH ELEVATION (Looking North)



BILL OF MATERIAL

PIER 28 - NORTH ELEVATION

(Looking South)

Drain Pipe-

7//8/18

4.0 ft2

1.6 ft²

PIER 27 - NORTH ELEVATION

5.0 ft2

Ground Line

(Looking South)

/-- Ground

///XV/X

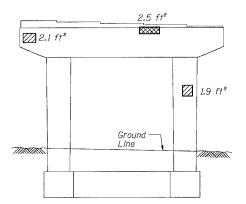
16.0 ft2

1112000

ITEM	UNIT	QUANTITY
High Performance Enhanced Shotcrete	SQ. FT.	222.6
Formed Concrete Repair (Depth areater than 5")	SQ. FT.	68.2
Epoxy Crack Sealing	FOOT	113.9

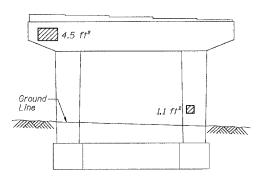
F.A.I.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
94/90	2003-028F	COOK	419	271
STA.		TO STA.		
FED. ROA	D DIST. NO. ILL	NOIS FED. AID	PROJECT	

62580



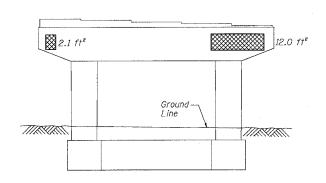
PIER 26 - SOUTH ELEVATION

(Looking North)



PIER 27 - SOUTH ELEVATION

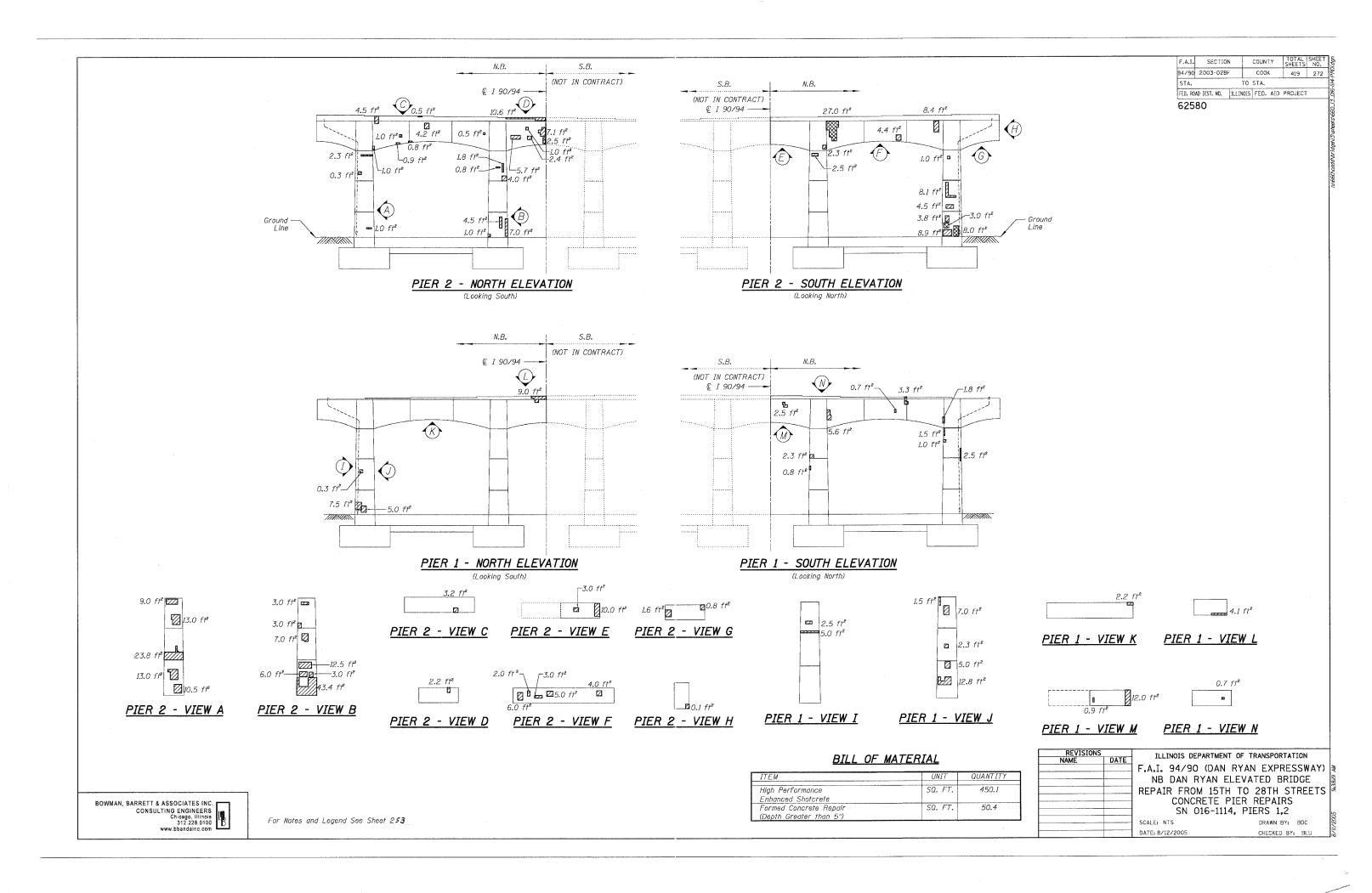
(Looking North)



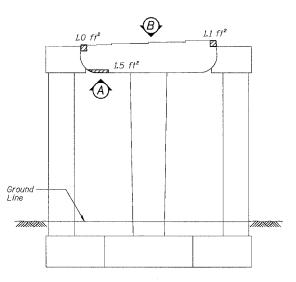
PIER 28 - SOUTH ELEVATION (Looking North)

REVISIONS NAME	DATE	ILLINOIS DEPARTMENT O	F TRANSPORTATION
NAME		F.A.I. 94/90 (DAN R	YAN EXPRESSWAY)
		NB DAN RYAN ELE	VATED BRIDGE
		REPAIR FROM 15TH 7	TO 28TH STREETS
		CONCRETE PIE	R REPAIRS
		SN 016-1047, PIERS	22,23,25,26,27,28
		SCALE: NTS	DRAWN BY: BDC
		DATE: 8/12/2005	CHECKED BY: BLU

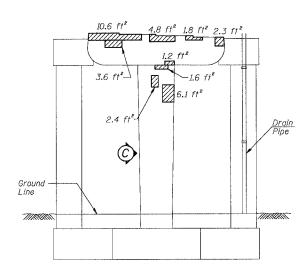
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62580



PIER D-7 - NORTH ELEVATION
(Looking South)



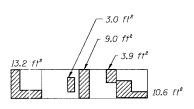
PIER D-7 - SOUTH ELEVATION

(Looking North)



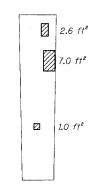
PIER D-7 - VIEW A

Looking up at bottom of cap



<u> PIER D-7 - VIEW B</u>

Looking down at top of seat



PIER D-7 - VIEW C

Looking East at side of column

BILL OF MATERIAL

ITEM UNIT QUANTITY

High Performance SO. FT. 110.7

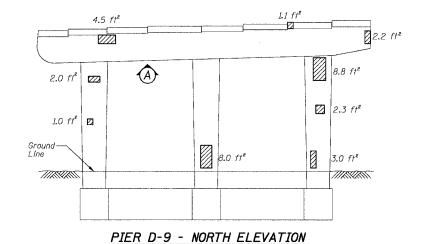
Enhanced Shotcrete

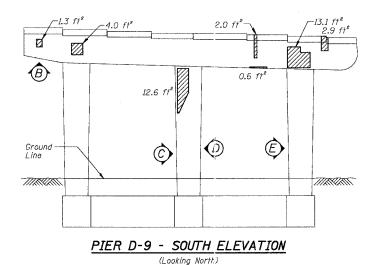
REVISIONS NAME DATE		ILLINOIS DEPARTM	MENT OF TRANSPORTATION
ITANIL	DATE	F.A.I. 94/90 (DA	N RYAN EXPRESSWAY)
		NB DAN RYAN	ELEVATED BRIDGE
			TH TO 28TH STREETS
			PIER REPAIRS
		SN 016-1	1113, PIER D-7
		SCALE: NTS	DRAWN BY: BDC
		DATE: 7/22/2005	CHECKED BY: BLU

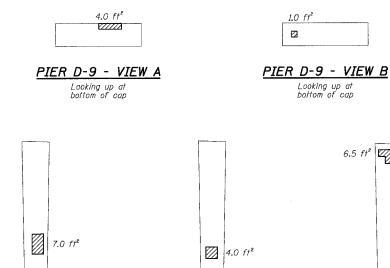
BOWMAN, BARRETT & ASSOCIATES INC.
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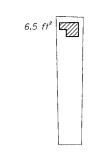
For Notes and Legend See Sheet 2:53











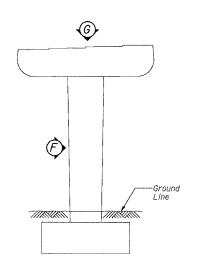


PIER D-9 - VIEW C Looking East at side of column

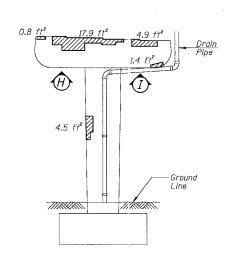
PIER D-9 - VIEW D Looking West at side of column

PIER D-9 - VIEW E

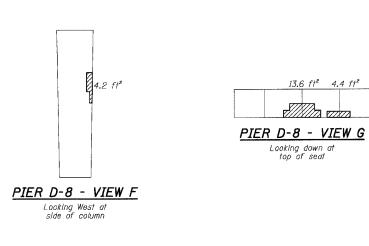
Looking East at side of column



PIER D-8 - NORTH ELEVATION (Looking South)



PIER D-8 - SOUTH ELEVATION (Looking North)





PIER D-8 - VIEW H Looking up at bottom of cap



PIER D-8 - VIEW I Looking up at bottom of cap

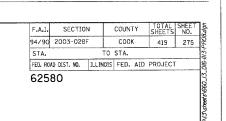
BILL OF MATERIAL

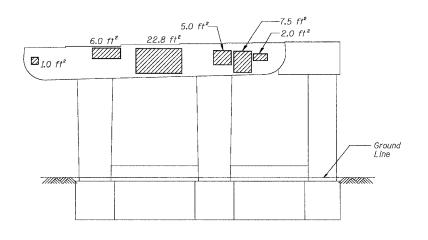
ITEM SQ. FT 178.9 High Performance Enhanced Shotcrete

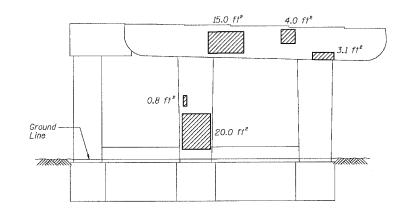
REVISIONS			
NAME	DATE		IT OF TRANSPORTATION
- I VI VIII	DAIL		RYAN EXPRESSWAY)
		NB DAN RYAN E	ELEVATED BRIDGE
		REPAIR FROM 15TH	H TO 28TH STREETS
		CONCRETE	PIER REPAIRS
			PIERS D-8,D-9
	-	SCALE: NTS	DRAWN BY: BDC
		DATE: 7/22/2005	CHECKED BY: BLU

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For Notes and Legend See Sheet 253.





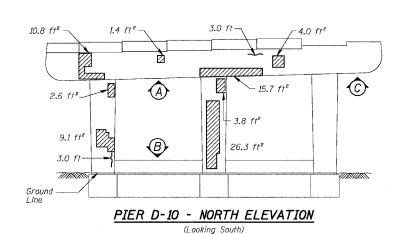


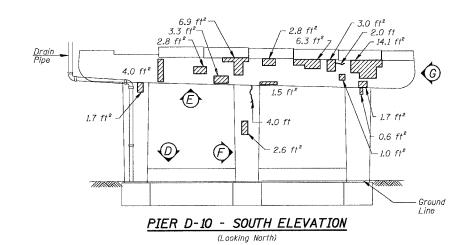
PIER D-11 - NORTH ELEVATION

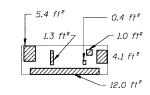
(Looking South)

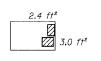
PIER D-11 - SOUTH ELEVATION

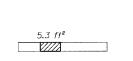
(Looking North)

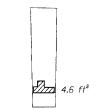














PIER D-10 - VIEW A

Looking up at bottom of cap

PIER D-10 - VIEW B

Looking down at top of grade beam

PIER D-10 - VIEW C Looking up at bottom of cap

PIER D-10 - VIEW D Looking down at top of grade beam

PIER D-10 - VIEW E Looking up at bottom of cap

PIER D-10 - VIEW F Looking East at side of column

PIER D-10 - VIEW G Looking West at end of cap

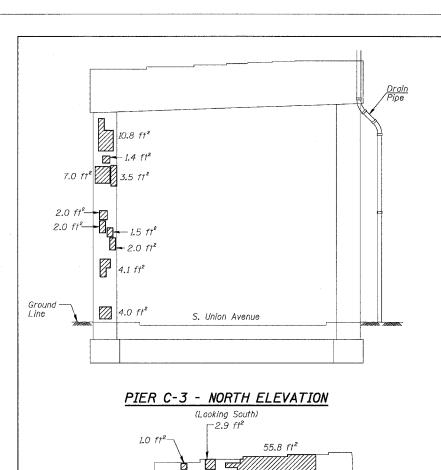
BILL OF MATERIAL

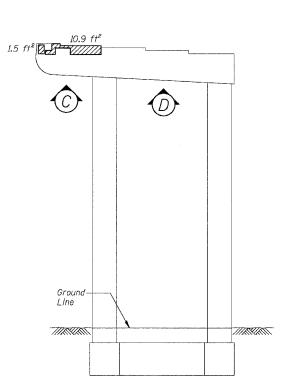
ITEM	UNIT	QUANTITY
High Performance Enhanced Shotcrete	SQ. FT.	301.6
Epoxy Crack Sealing	FOOT	13.8

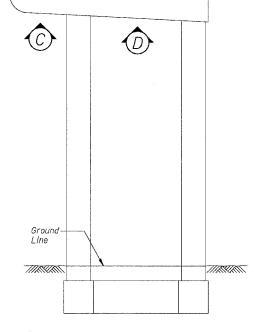
			.,
REVISION	S DATE	ILLINOIS DEPARTMENT O	F TRANSPORTATION
NAME	DATE	F.A.I. 94/90 (DAN R)	AN EXPRESSWAY
		NB DAN RYAN ELE	
		REPAIR FROM 15TH T	
		CONCRETE PIE	
		SN 016-1113, PIE	RS D-10,D-11
		SCALE: NTS	DRAWN BY: BDC
		DATE: 7/22/2005	CHECKED BY: BLU

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For Notes and Legend See Sheet 2:53







PIER C-2 - SOUTH ELEVATION (Looking North)

8.0 ft² 8.8 ft²

PIER C-2 - VIEW A

Looking East at side of column

Looking East at side of column

PIER C-2 - VIEW B

3.2 ft²

PIER C-2 - VIEW C

Looking up at bottom of cap

/-3.4 ft2 15.0 ft²

PIER C-2 - VIEW D Looking up at bottom of cap

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
High Performance Enhanced Shotcrete	Sa. FT.	186.5
Formed Concrete Repair (Depth Greater than 5")	SQ. FT.	4.4

	IS	REVISIO
	DATE	NAME
F./		
חר		
RE		
SCA		. ,

ILLINOIS DEPARTMENT OF TRANSPORTATION .A.I. 94/90 (DAN RYAN EXPRESSWAY) |₹ NB DAN RYAN ELEVATED BRIDGE REPAIR FROM 15TH TO 28TH STREETS CONCRETE PIER REPAIRS SN 016-1113, PIERS C-2,C-3

CALE: NTS DRAWN BY: BDC DATE: 8/12/2005 CHECKED BY: BLU

F.A.I. SECTION

94/90 2003-028F STA.

62580

COUNTY TOTAL SHEE NO. COOK 419 276

TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

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CONSULTING ENGINEERS
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3.8 ft²

3.0 ft²

Ground-Line

 \bigcirc

PIER C-2 - NORTH ELEVATION

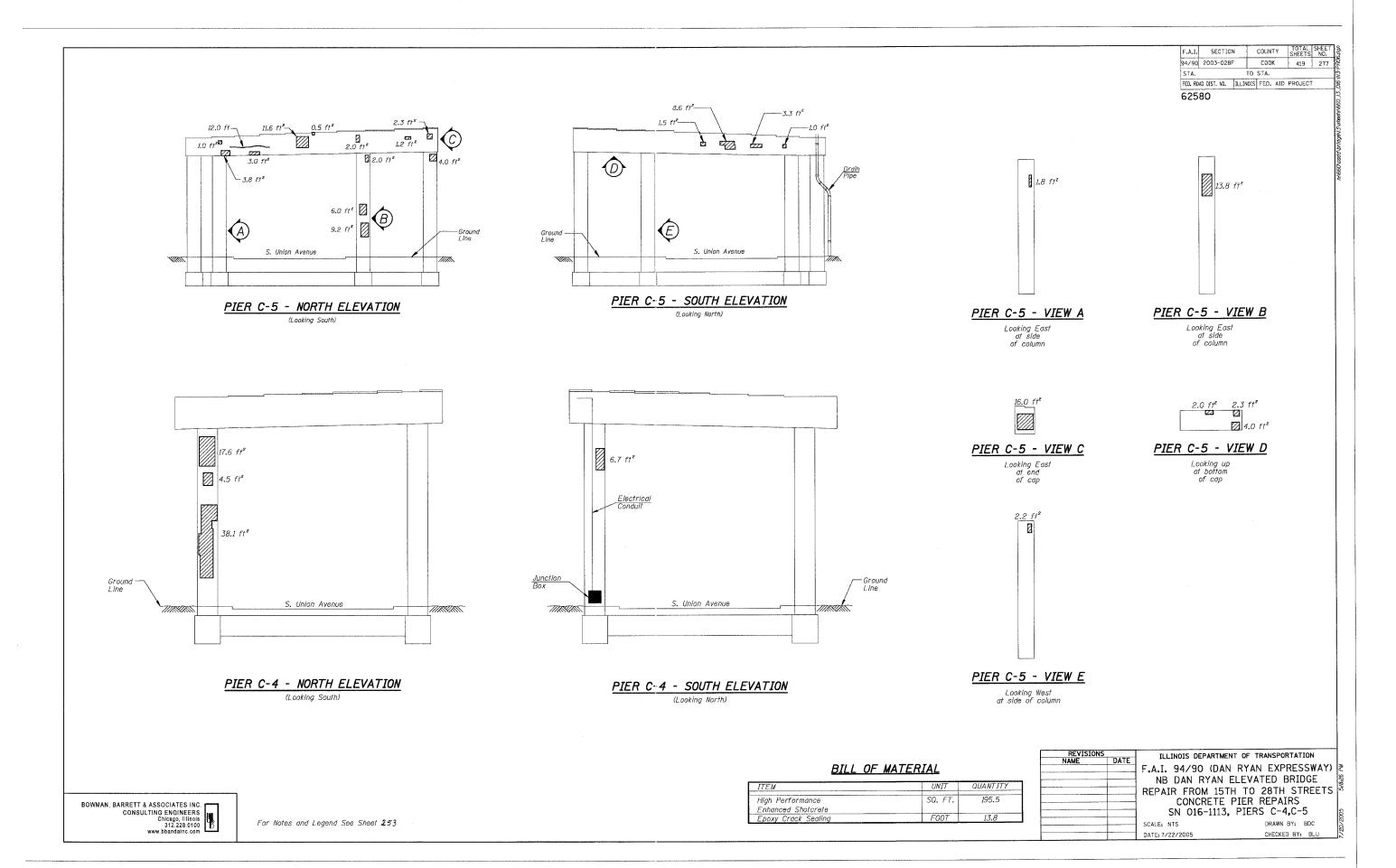
(Looking South)

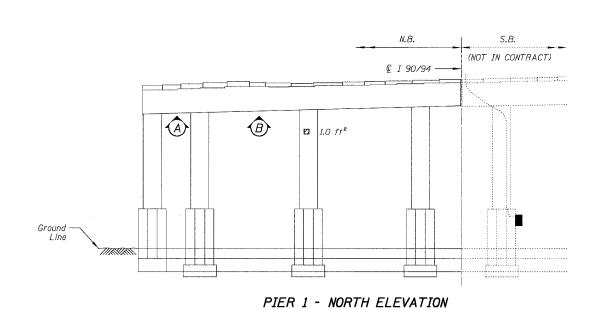
For Notes and Legend See Sheet 2'53

2.0 ft²

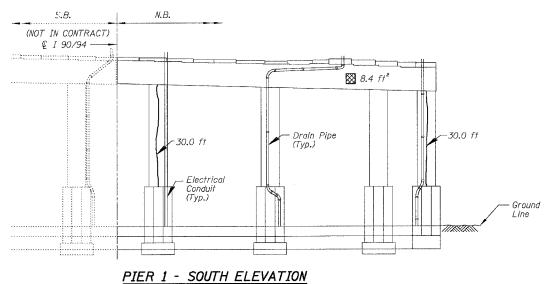
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(Looking South)





14.5 ft²

F.A.I. SECTION

94/90 2003-028F

STA.

62580

4.5 ft 2

PIER 1 - VIEW A Looking up at bottom of cap

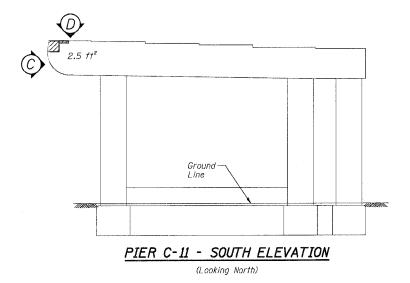
COUNTY TOTAL SHEETS

TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

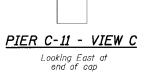
COOK 419 278

PIER 1 - VIEW B

Looking up at bottom of cap



(Looking North)



3.0 ft²



PIER C-11 - VIEW D

Looking down at top of seat

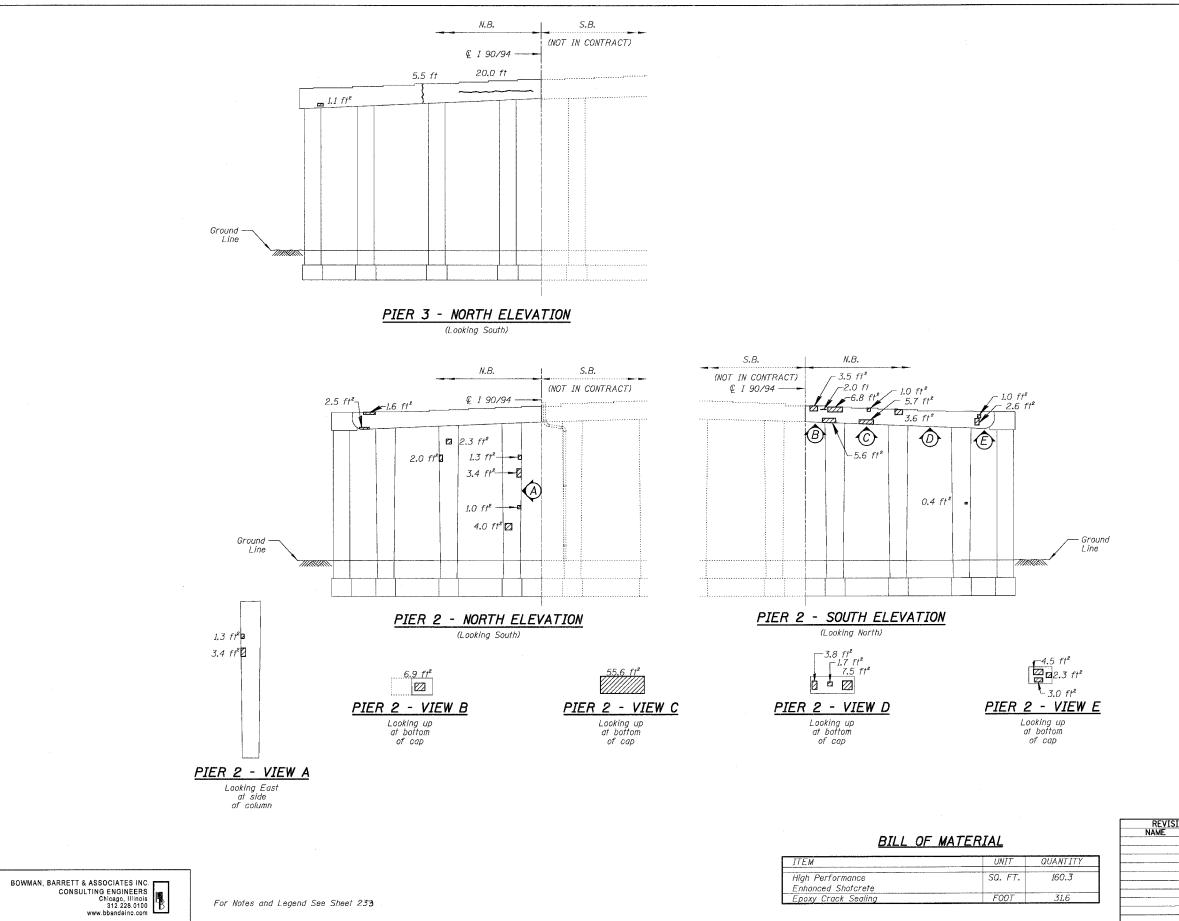
BILL OF MATERIAL

ITEM	UNIT	QUANTITY
High Performance	SQ. FT.	34.2
Enhanced Shotcrete		
Formed Concrete Repair	SQ. FT.	13.1
(Depth Greater than 5")		
Engry Crack Sealing	FOOT	69.0

REVISIONS AME	DATE	ILLINOIS DEPARTMENT F.A.I. 94/90 (DAN F NB DAN RYAN EL REPAIR FROM 15TH CONCRETE PI SN 016-1113,	RYAN EXPRESSWAY) EVATED BRIDGE TO 28TH STREETS IER REPAIRS
	-	SCALE: NTS	DRAWN BY: BDC
		DATE: 8/12/2005	CHECKED BY: BLU

BOWMAN, BARRETT & ASSOCIATES INC.
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Chicago, Illinois
312 228 0100
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For Notes and Legend See Sheet 253



For Notes and Legend See Sheet 253

ITEM	UNIT	QUANTITY
High Performance Enhanced Shotcrete	SQ. FT.	160.3
Epoxy Crack Sealing	FOOT	31.6

REVISIONS		ILLINOIS DEPART	TMENT OF TRANSPORTATION
NAME	DATE		
		F.A.I. 94/90 (D	DAN RYAN EXPRESSWAY)
		NB DAN RYA	AN ELEVATED BRIDGE
		REPAIR FROM 1	15TH TO 28TH STREETS
		CONCRET	TE PIER REPAIRS
		SN 016-	-1113, PIERS 2,3
		SCALE: NTS	DRAWN BY: BDC
		DATE: 8/12/2005	CHECKED BY: BLU

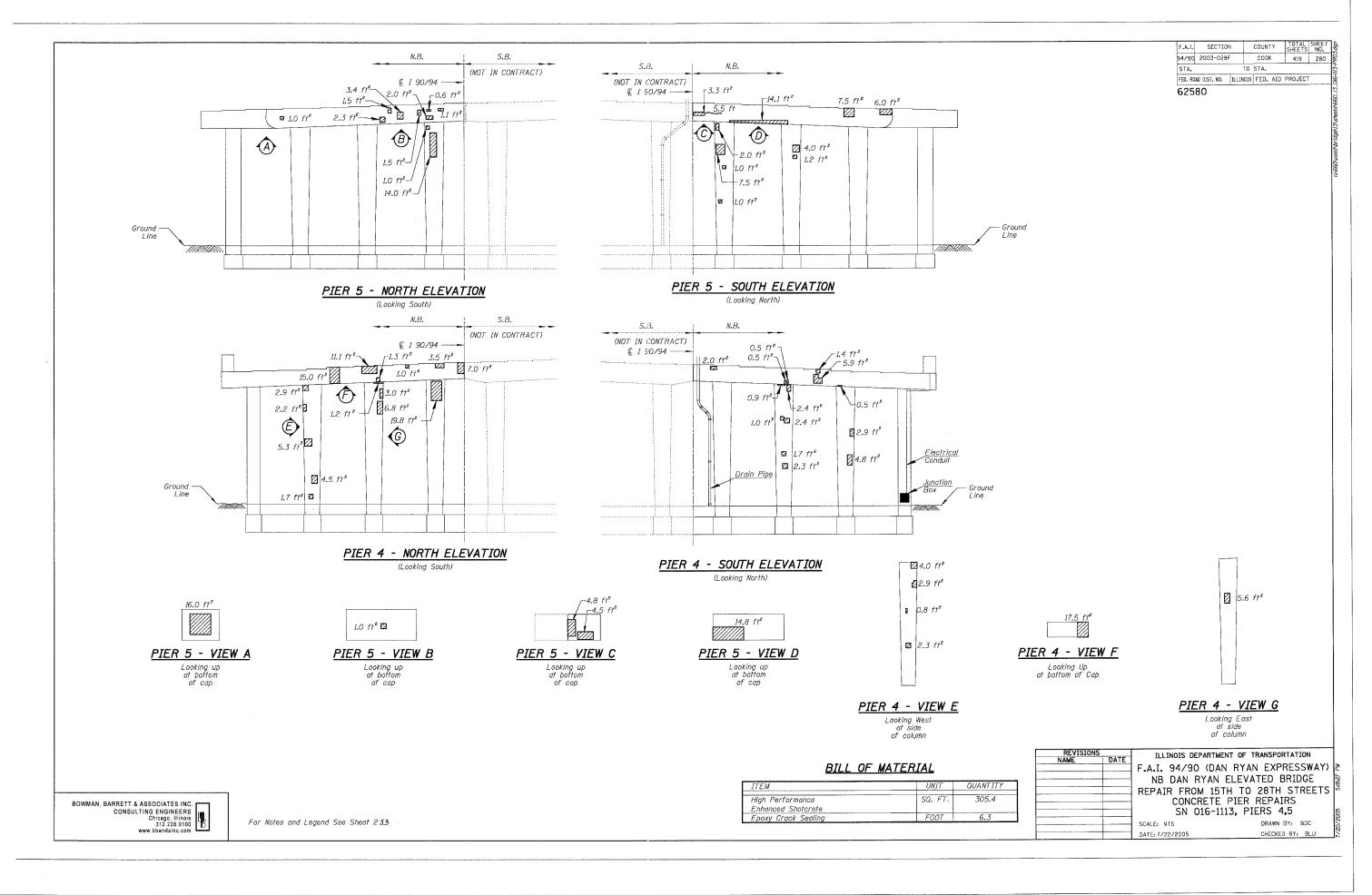
F.A.I. SECTION COUNTY TOTAL SHEE SHEETS NO. 94/90 2003-028F COOK 419 279

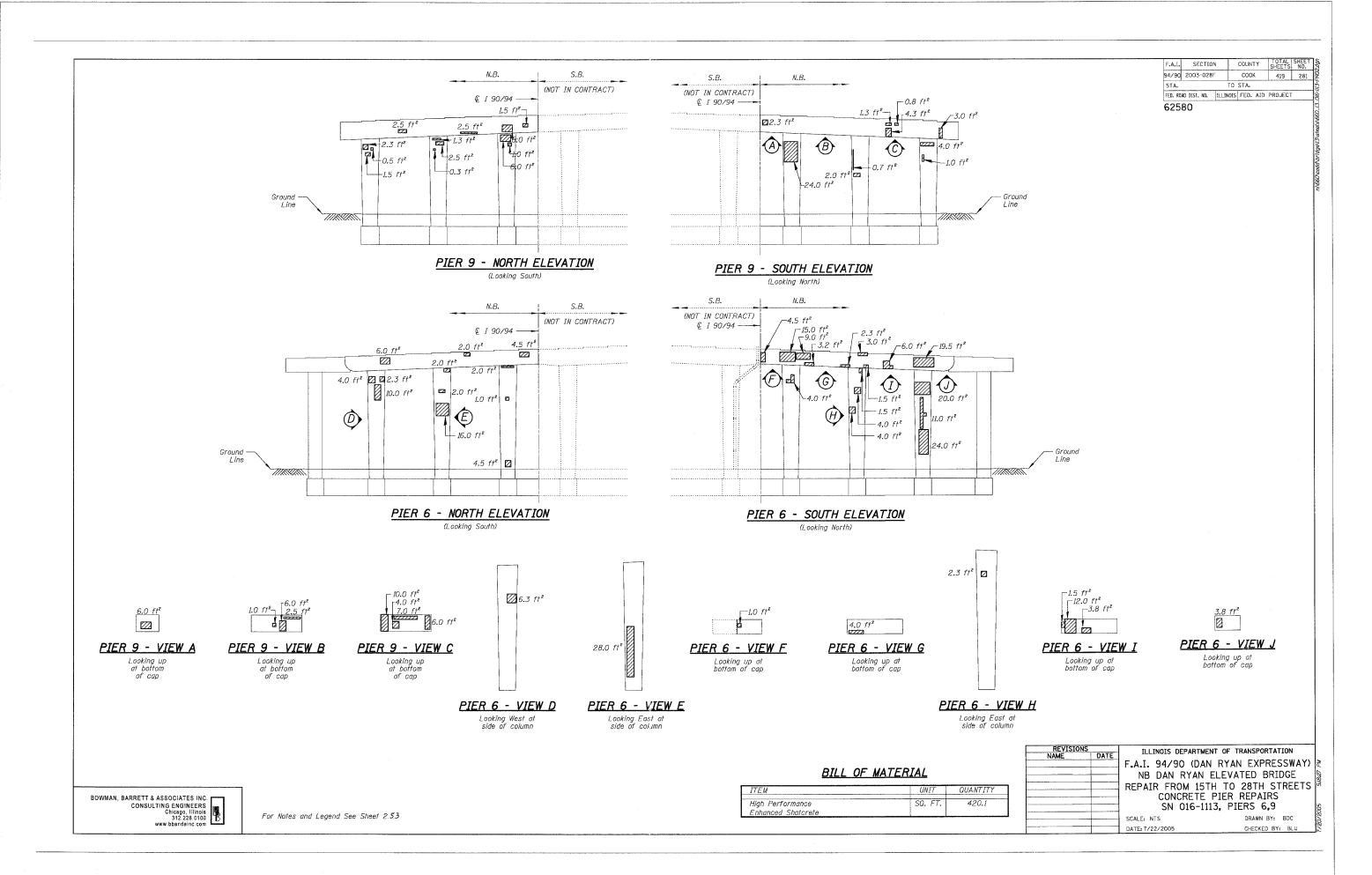
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

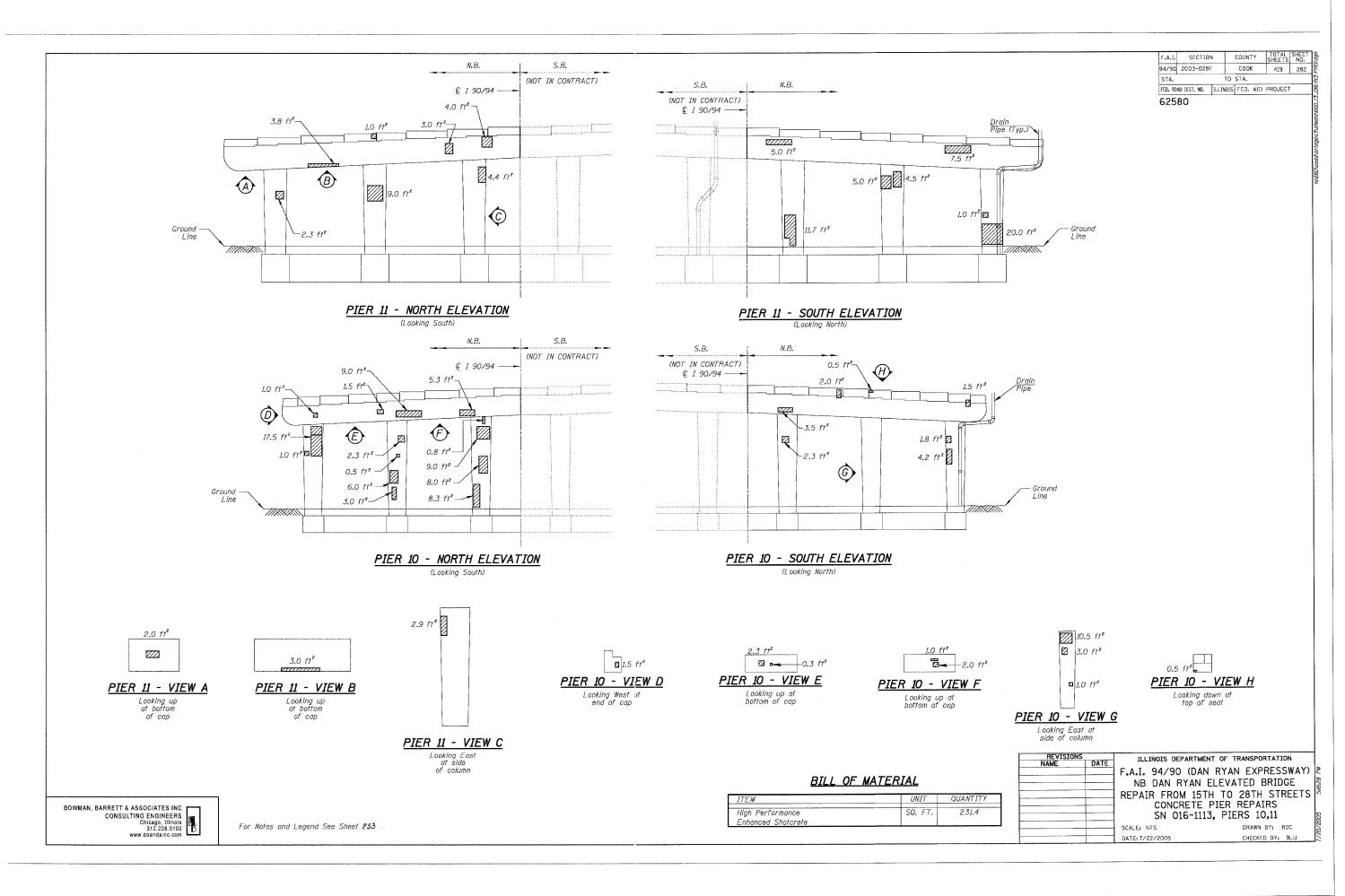
TO STA.

STA.

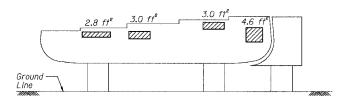
62580



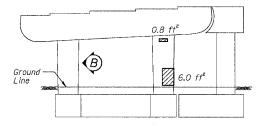




62580



PIER D-13 - NORTH ELEVATION
(Looking South)



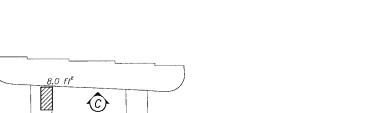
PIER D-12 - NORTH ELEVATION

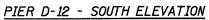
(Looking South)
Repairs are required on side of column also.



PIER D-13 - SOUTH ELEVATION

(Looking North)





(Looking North) Repairs are required on side of column also.



PIER D-13 - VIEW A

Looking West at end of cap



PIER D-12 - VIEW B

Looking East at side of column



PIER D-12 - VIEW C

Looking up at bottom of cap

BILL OF MATERIAL

ITEM UNIT QUANTITY
High Performance SQ. FT. 53.5
Enhanced Shotcrete

REVISIONS
NAME DATE
F

ILLINOIS DEPARTMENT OF TRANSPORTATION

F.A.I. 94/90 (DAN RYAN EXPRESSWAY)

NB DAN RYAN ELEVATED BRIDGE

REPAIR FROM 15TH TO 28TH STREETS

CONCRETE PIER REPAIRS

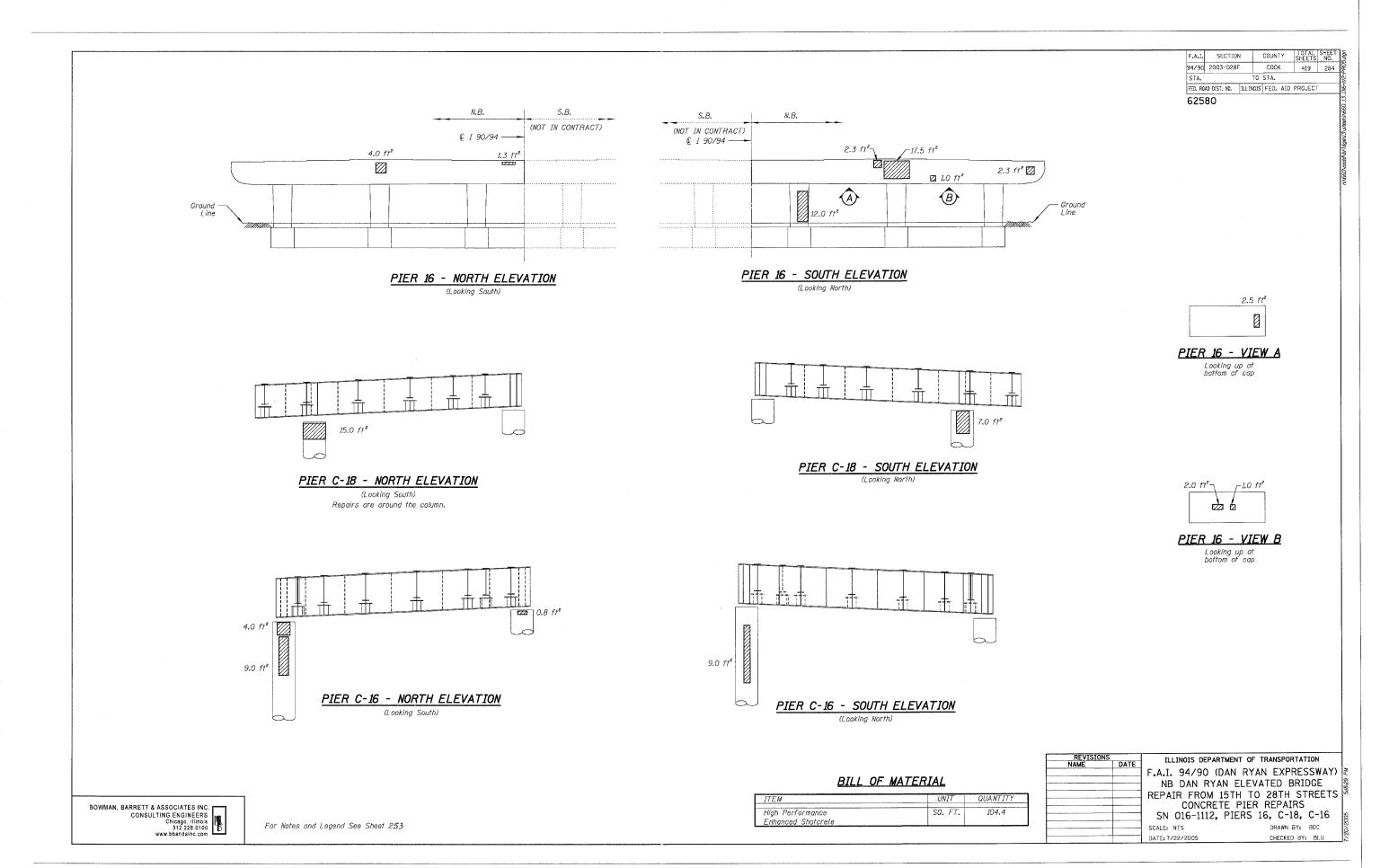
SN 016-1112, PIERS D-12, D-13

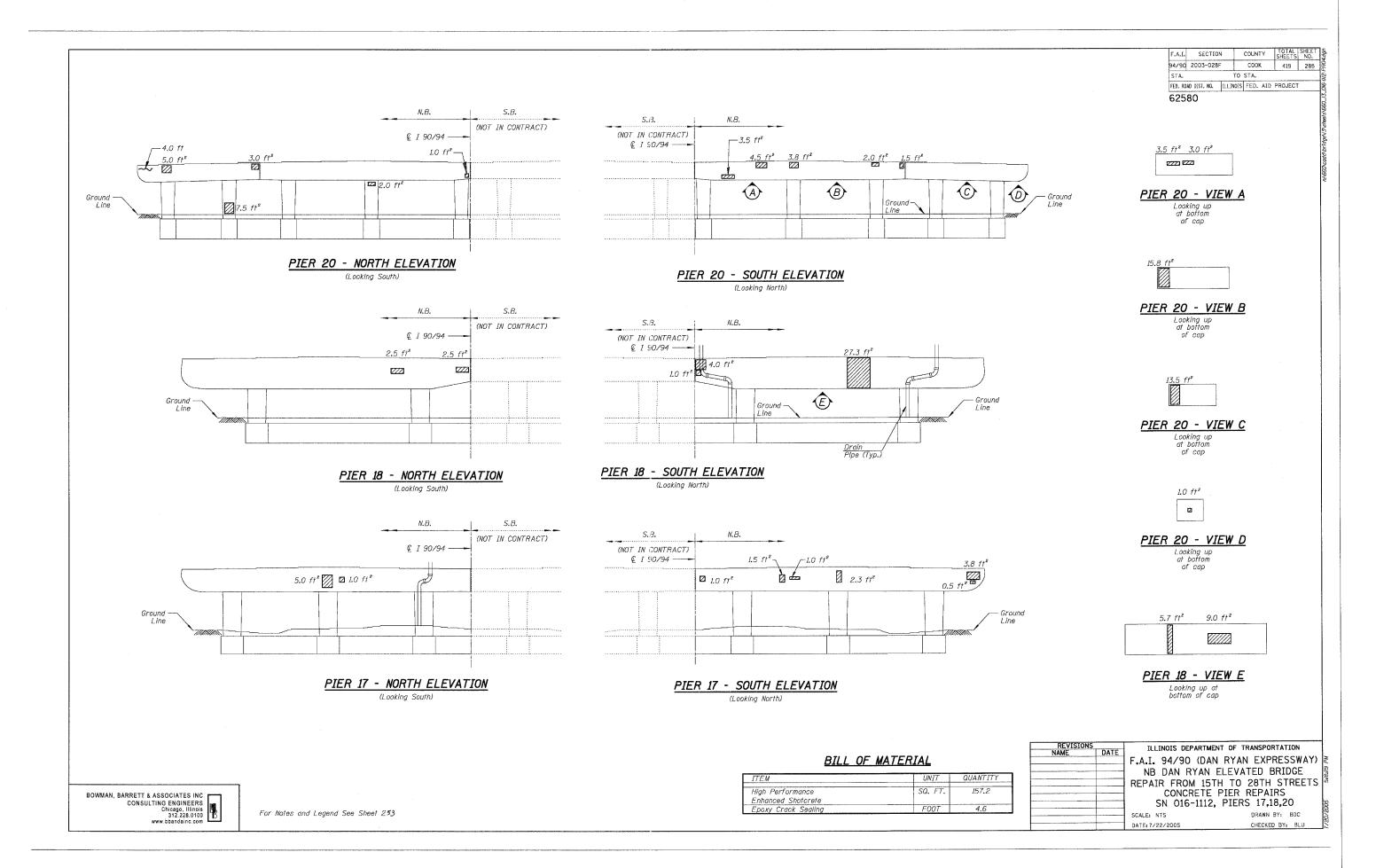
SCALE: NTS DRAWN BY: BDC

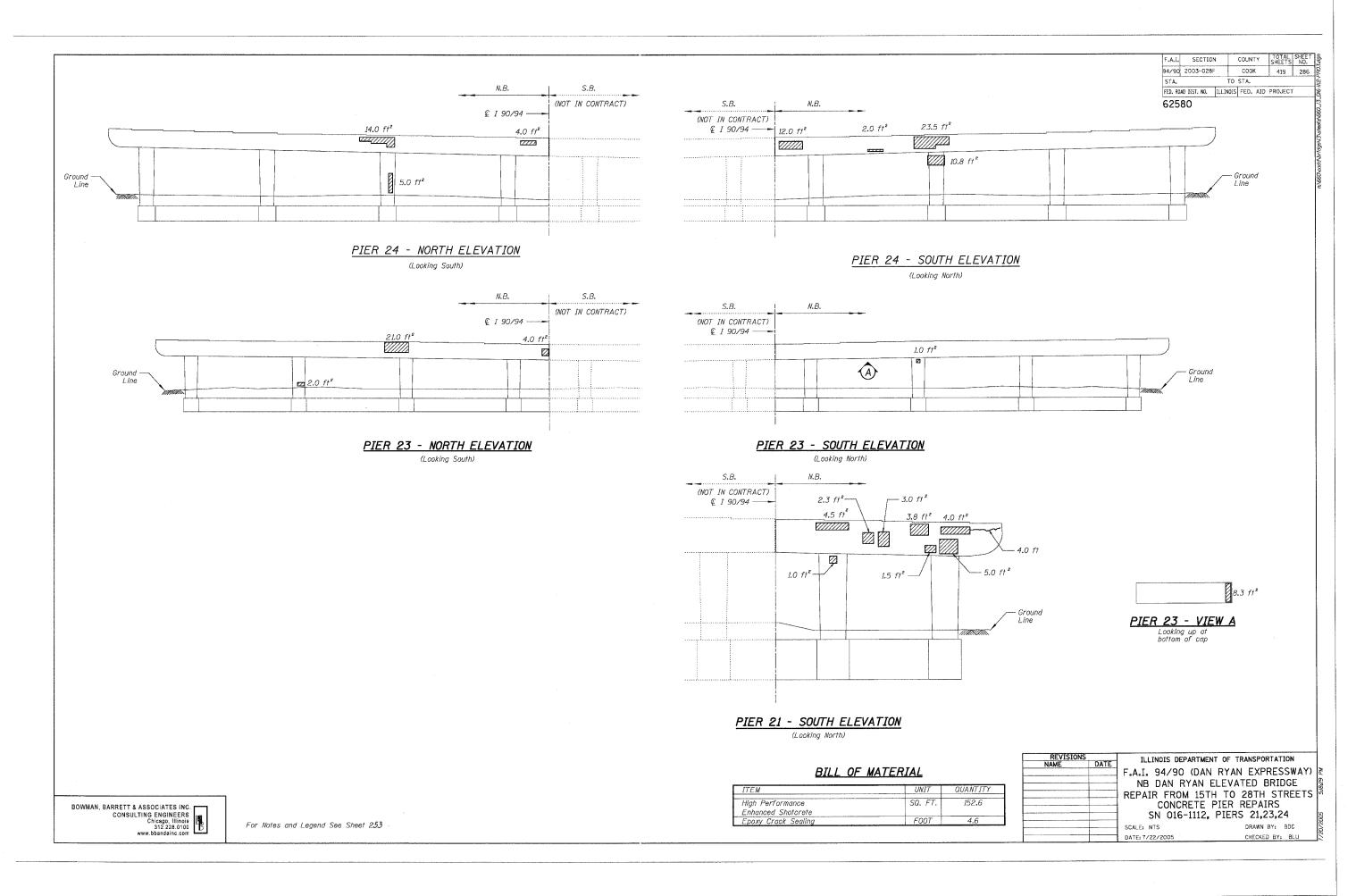
DATE: 7/22/2005 CHECKED BY: BLU

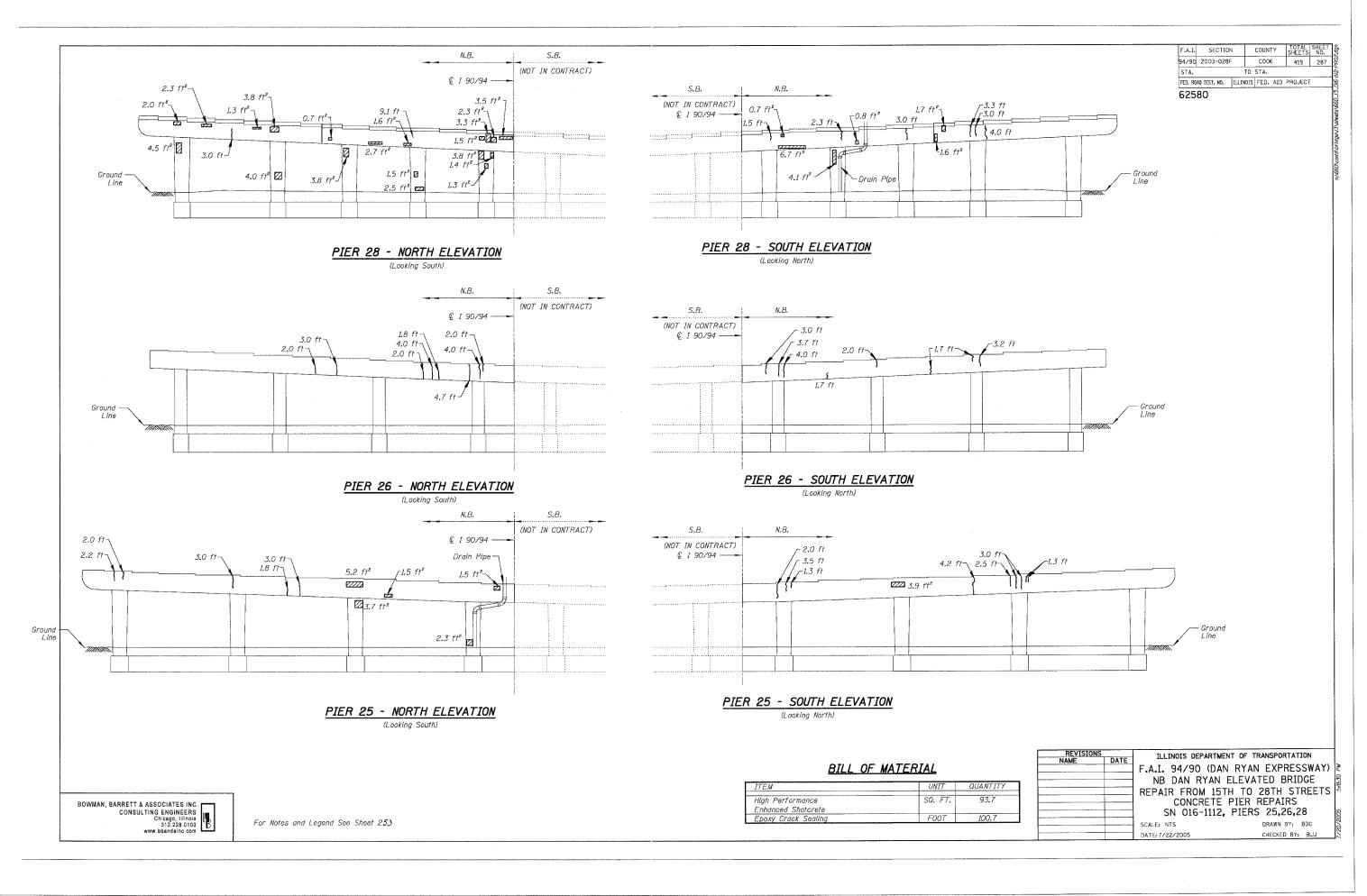
BOWMAN, BARRETT & ASSOCIATES INC.
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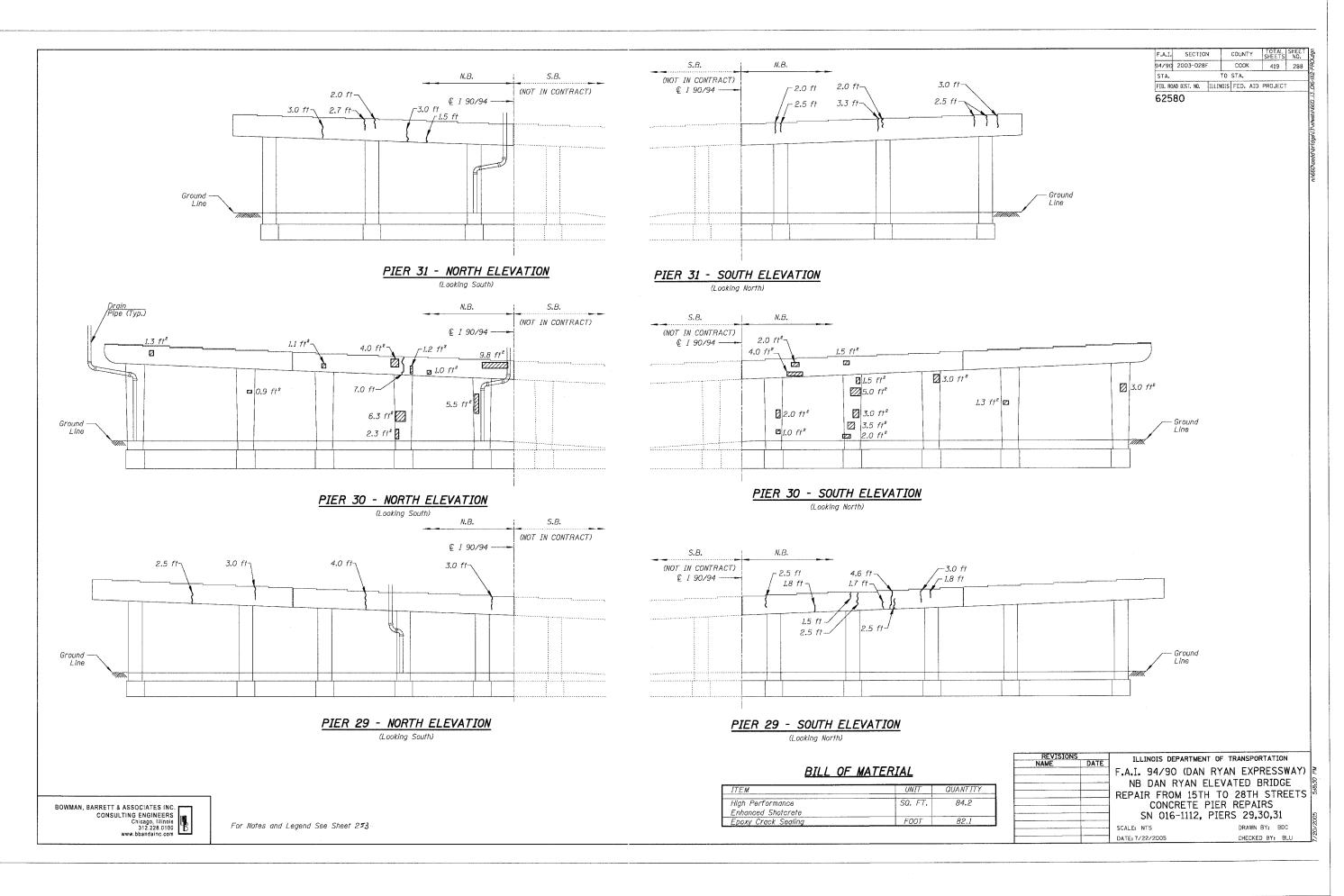
For Notes and Legend See Sheet 2.53



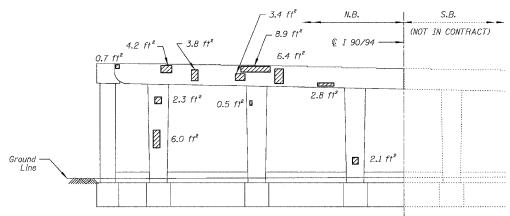






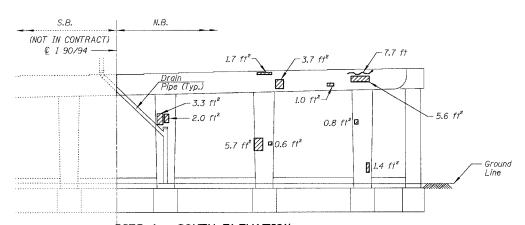








(Looking South)



PIER 1 - SOUTH ELEVATION

(Looking North)

BILL OF MATERIAL

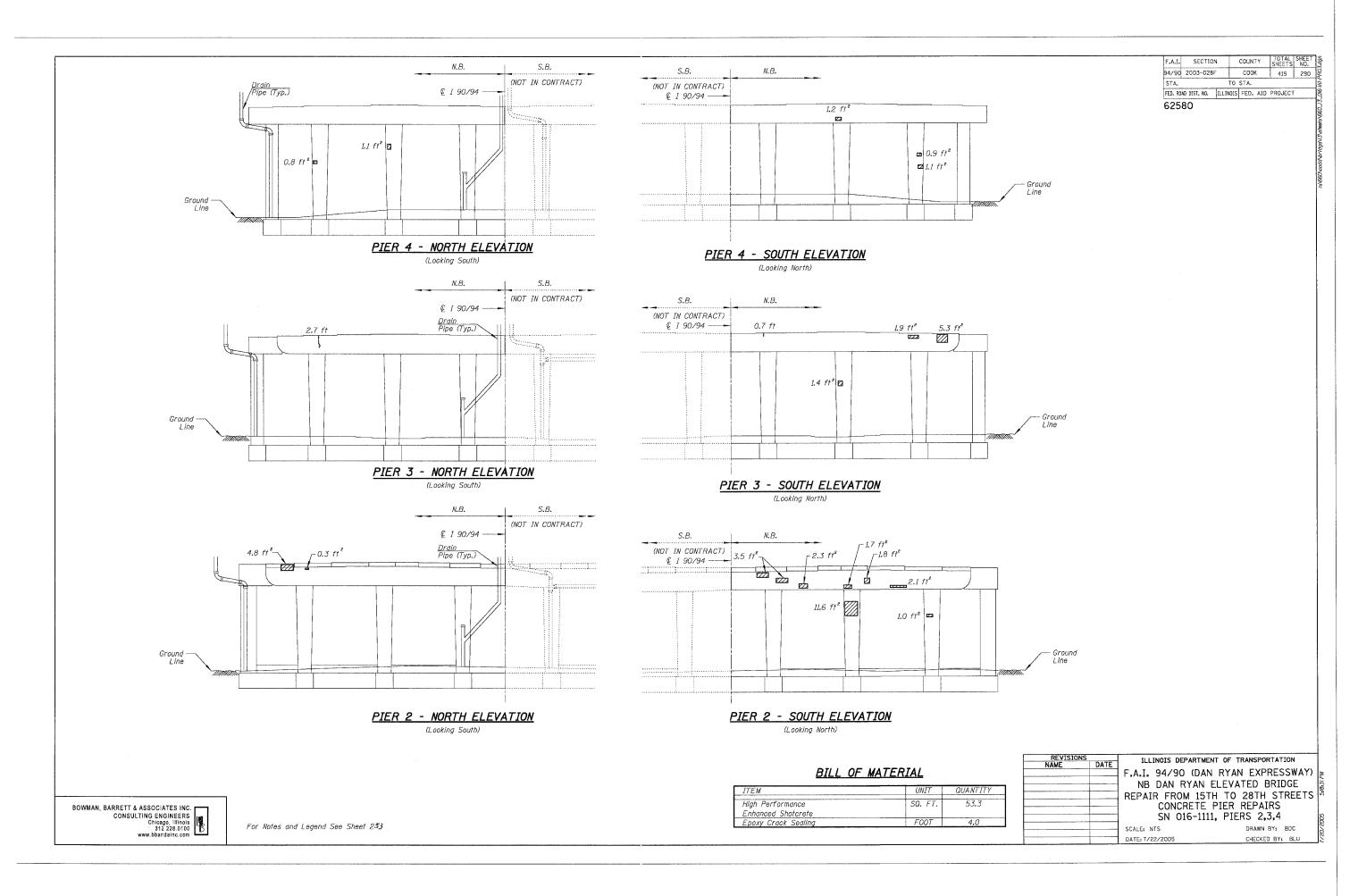
UNIT QUANTITY High Performance Enhanced Shotcrete Epoxy Crack Sealing

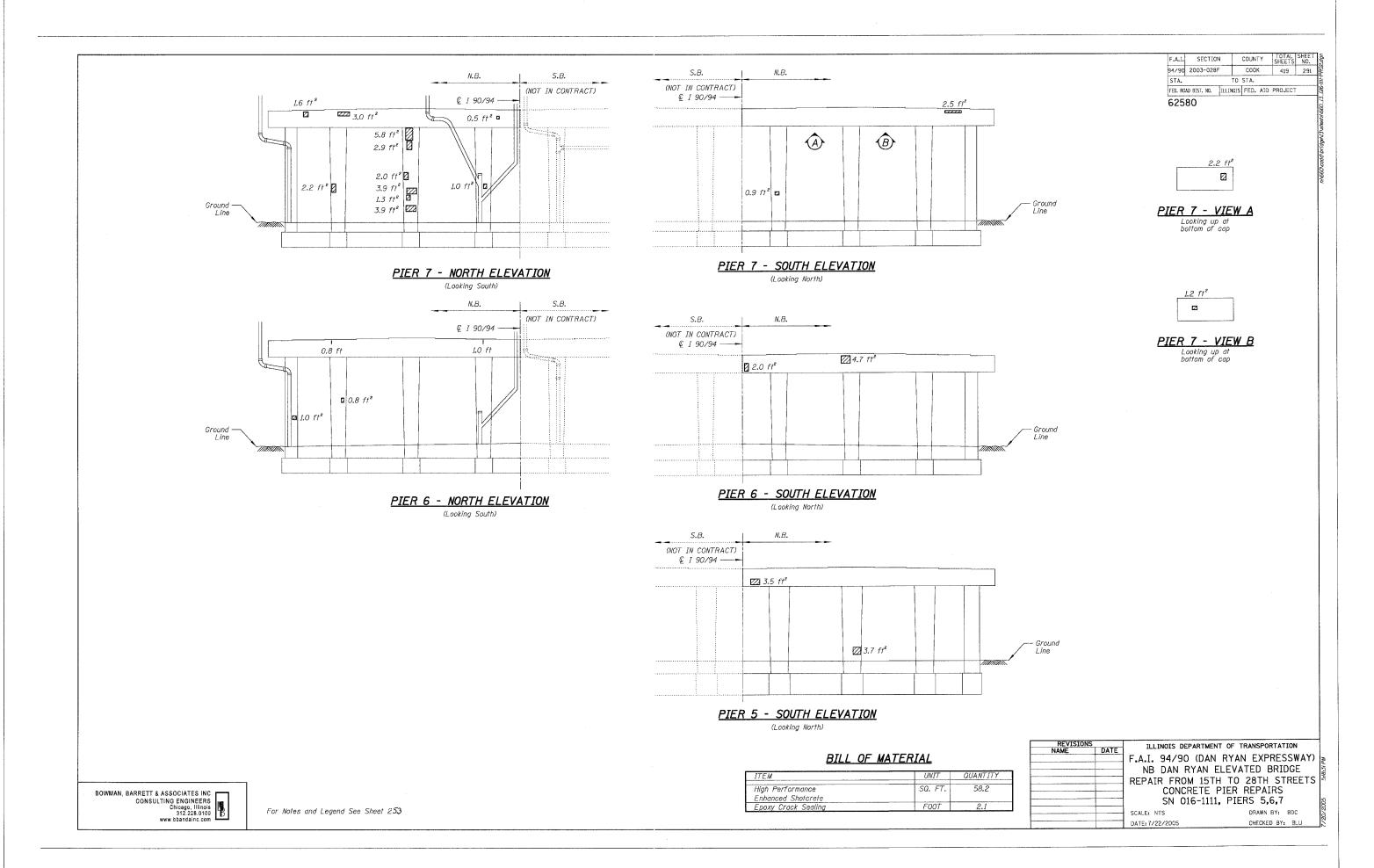
REVISIONS DATE		ILLINOIS DEPARTM	ENT OF TRANSPORTATION
IVAIVIC	DAIL	F.A.I. 94/90 (DAN RYAN EXPRESSWAY)	
		NB DAN RYAN	ELEVATED BRIDGE
		REPAIR FROM 15TH TO 28TH STREETS	
		CONCRETE PIER REPAIRS	
		SN 016-1111, PIER 1	
		SCALE: NTS	DRAWN BY: BDC

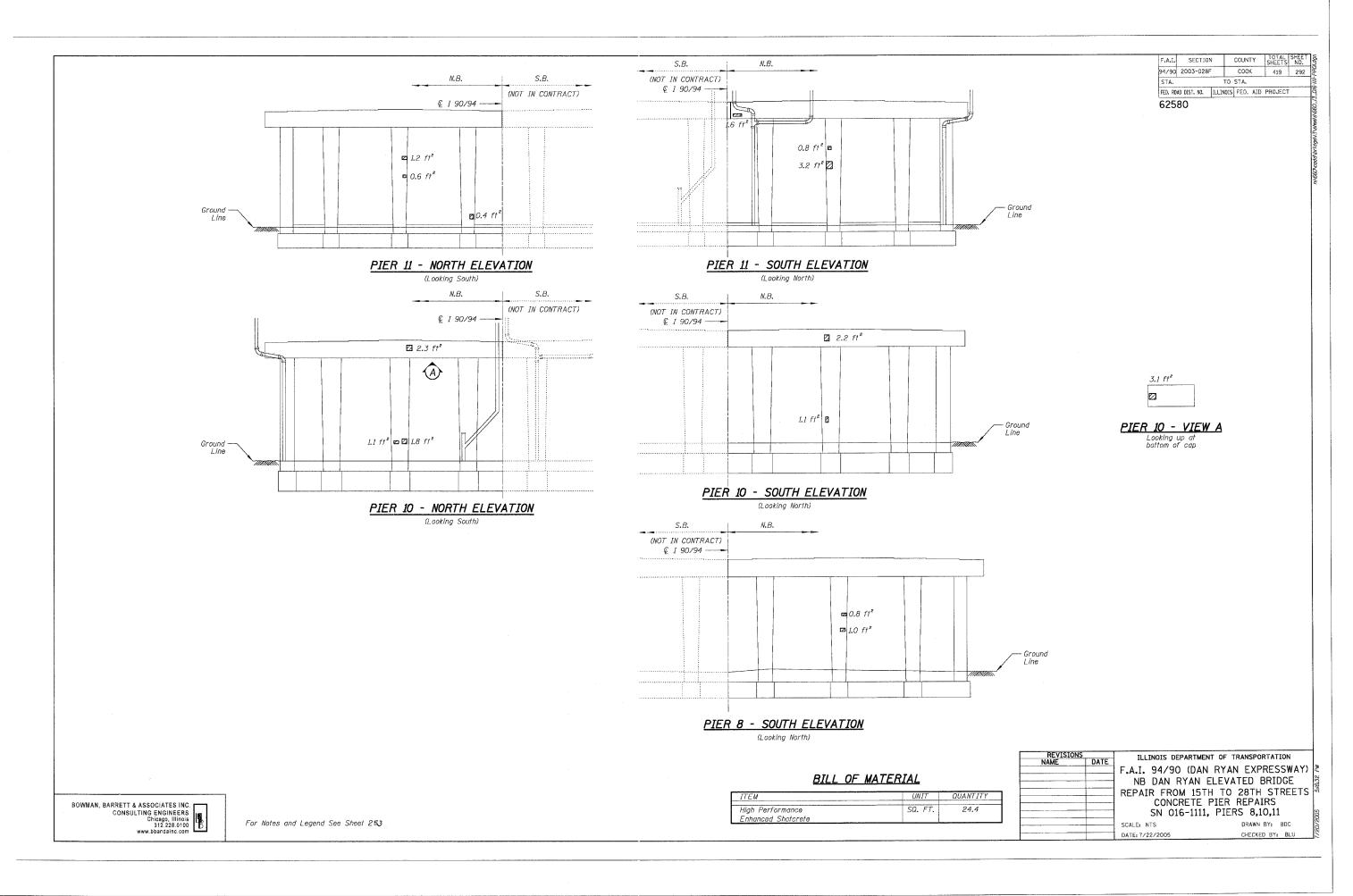
DATE: 7/22/2005

N BY: BDC

For Notes and Legend See Sheet 253







S.B.

(NOT IN CONTRACT)

© 1 90/94

6.3 ft²

6.3 ft²

7.0 ft²

PIER 2 - SOUTH ELEVATION

(Looking North)

S.B.

(NOT IN CONTRACT)

© 1 90/94

F.A.I. SECTION COUNTY TOTAL SHEET: NO. 94/90 2003-028F COOK 419 293 STA. TO STA.

FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

62580

(Looking North)

BILL OF MATERIAL

 ITEM
 UNIT
 QUANTITY

 High Performance
 SO. FT.
 47.3

 Enhanced Shotcrete
 Epoxy Crack Sealing
 FOOT
 4.6

REVISIONS
NAME
DATE
F.A.I. 94
NB DA
REPAIR I
CO
S
SCALE: NTS

ILLINOIS DEPARTMENT OF TRANSPORTATION

F.A.I. 94/90 (DAN RYAN EXPRESSWAY)

NB DAN RYAN ELEVATED BRIDGE

REPAIR FROM 15TH TO 28TH STREETS

CONCRETE PIER REPAIRS

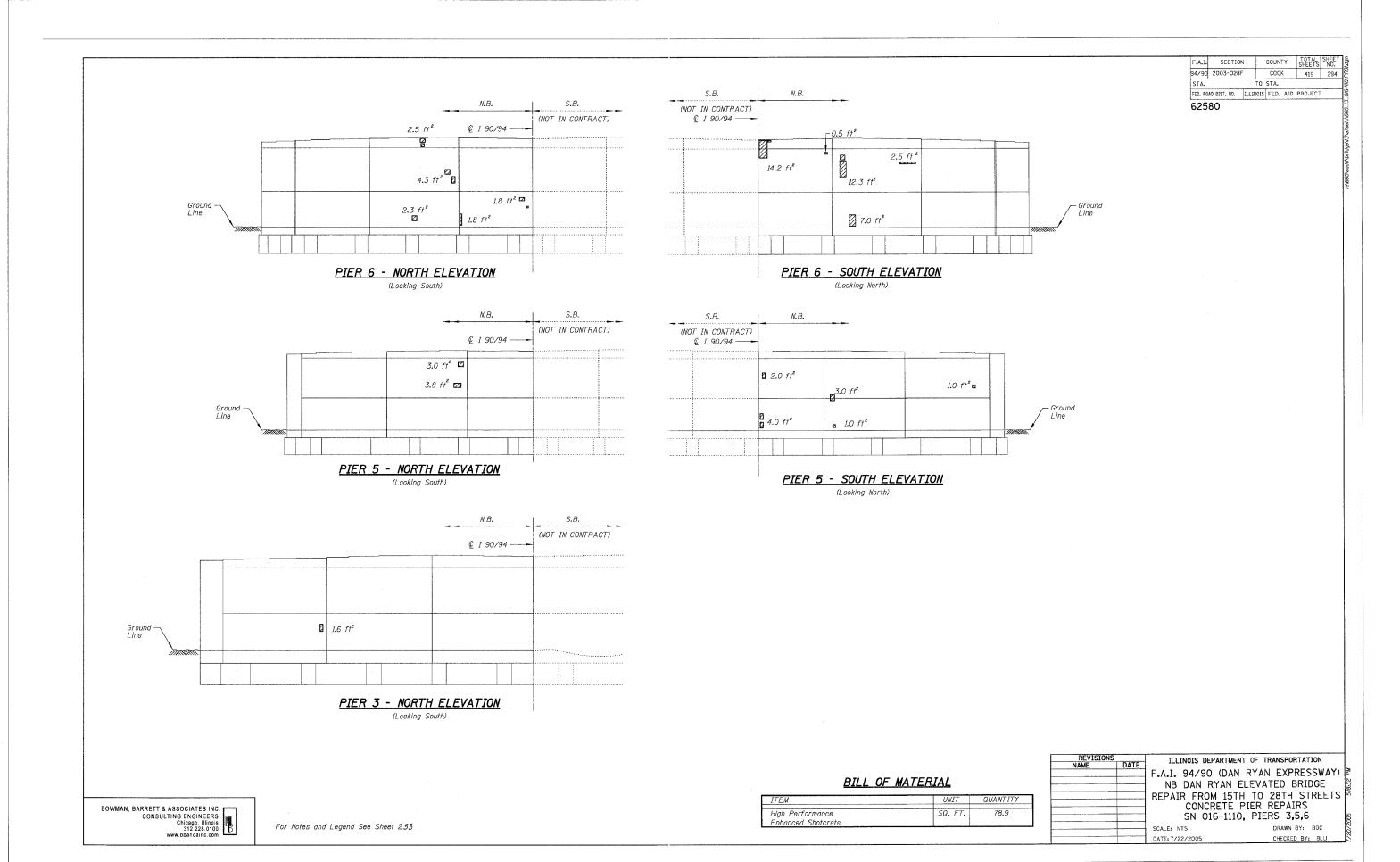
SN 016-1110, PIERS 1,2

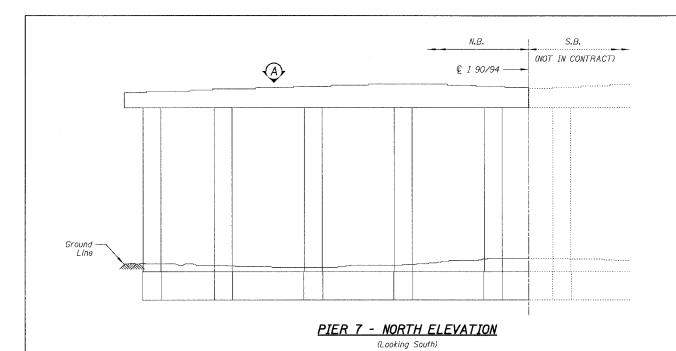
SCALE: NTS DRAWN BY: BDC

DATE: 7/22/2005 CHECKED BY: BLU

BOWMAN, BARRETT & ASSOCIATES INC.
CONSULTING ENGINEERS
Chicago, Illinois
312 228 0.100
www.bbandainc.com

For Notes and Legend See Sheet 253





For Notes and Legend See Sheet 253

62580

2.5 ff²

PIER 7 - VIEW A

Looking down at top of seat

BILL OF MATERIAL

ITEM UNIT QUANTITY

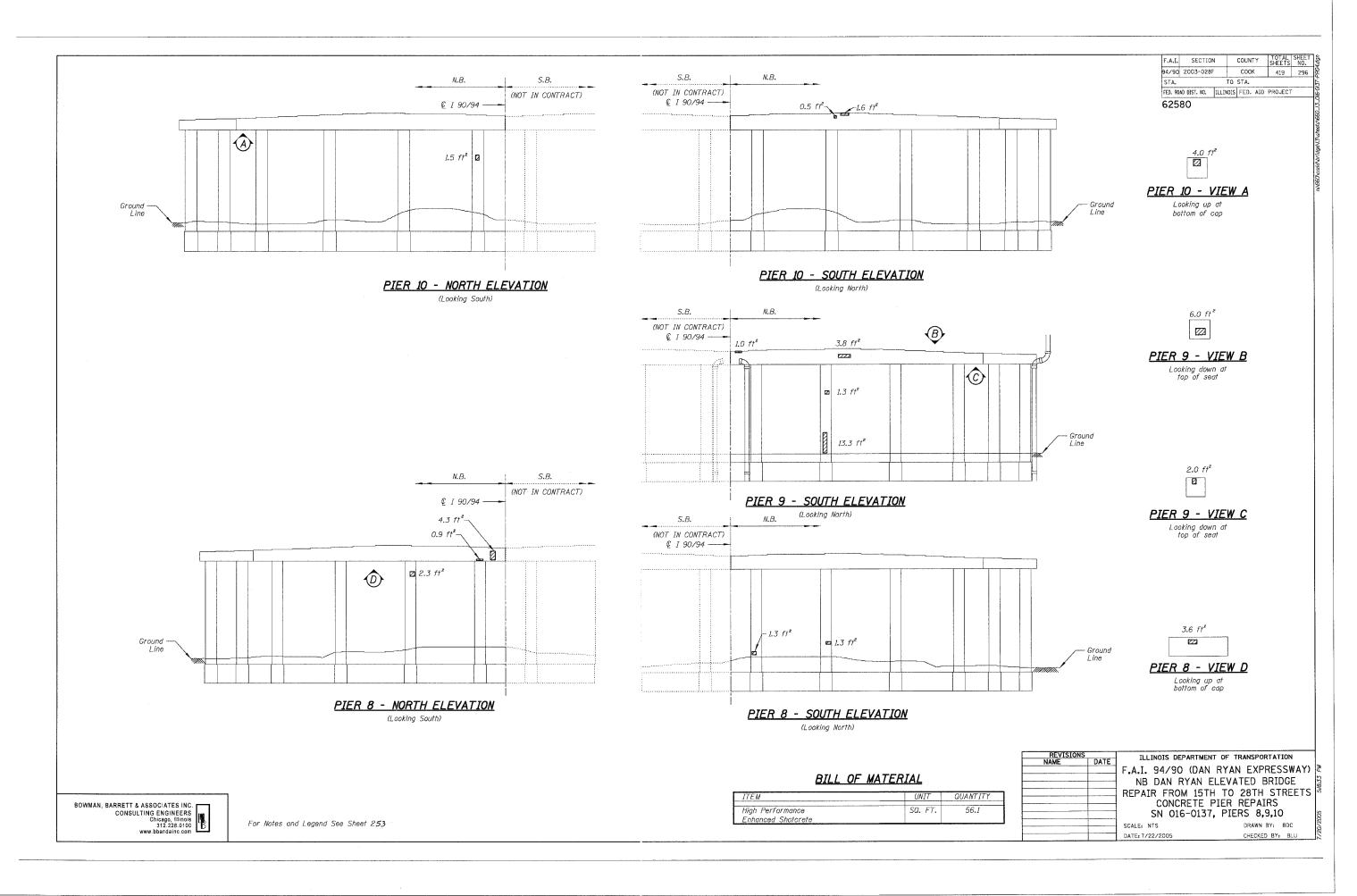
High Performance SQ. FT. 2.9

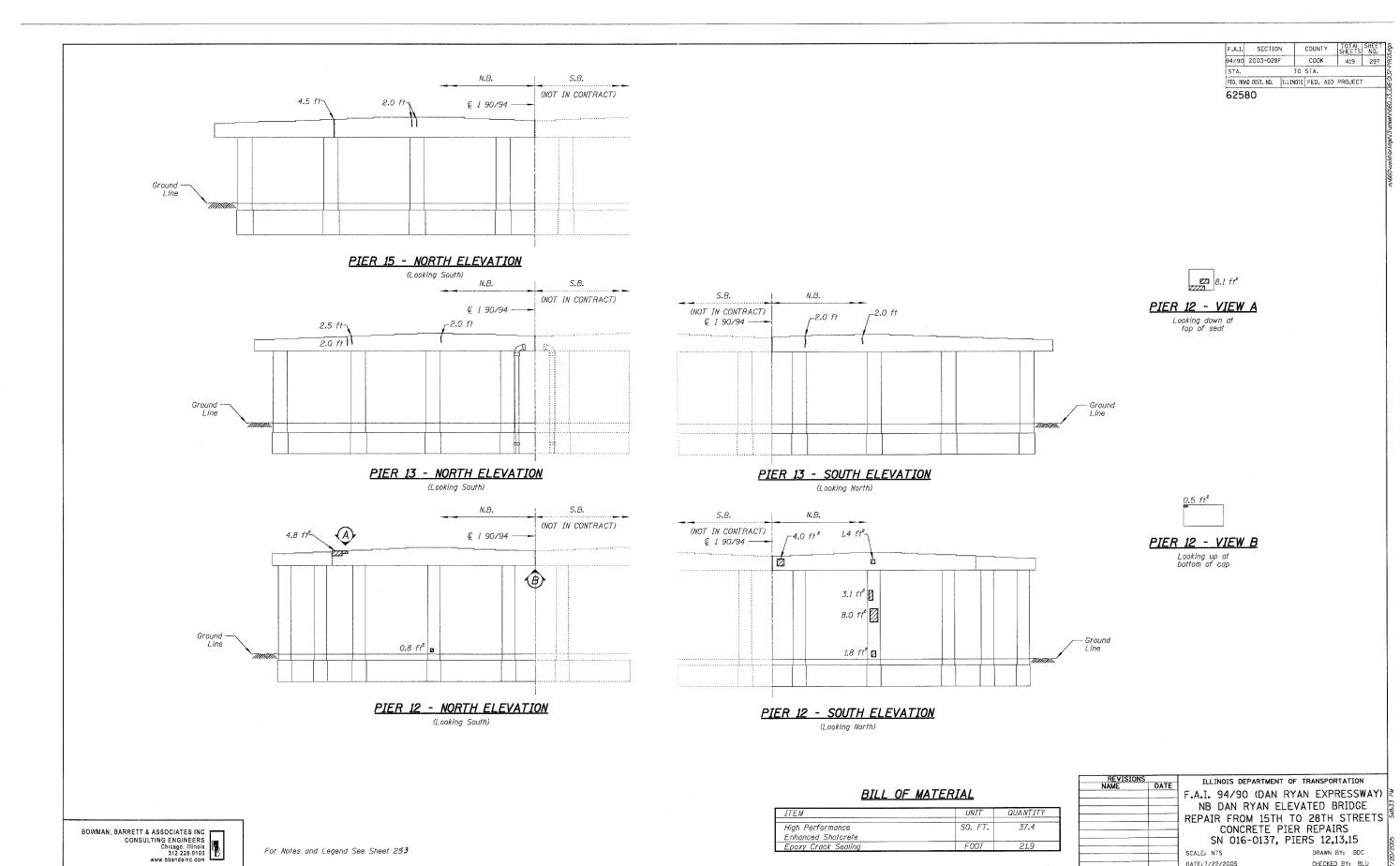
Enhanced Shotcrete

REVISIONS NAME	DATE	ILLINOIS DEPARTMENT OF TRANSPORTATION			
		F.A.I. 94/90 (DAN RYAN EXPRESS) NB DAN RYAN ELEVATED BRIDG			
		REPAIR FROM 15TH TO 28TH STR CONCRETE PIER REPAIRS			
		SN 016-0	137, PIER 7		
		SCALE: NTS	DRAWN BY: BDC		
		DATE: 7/22/2005	CHECKED BY: BLU		

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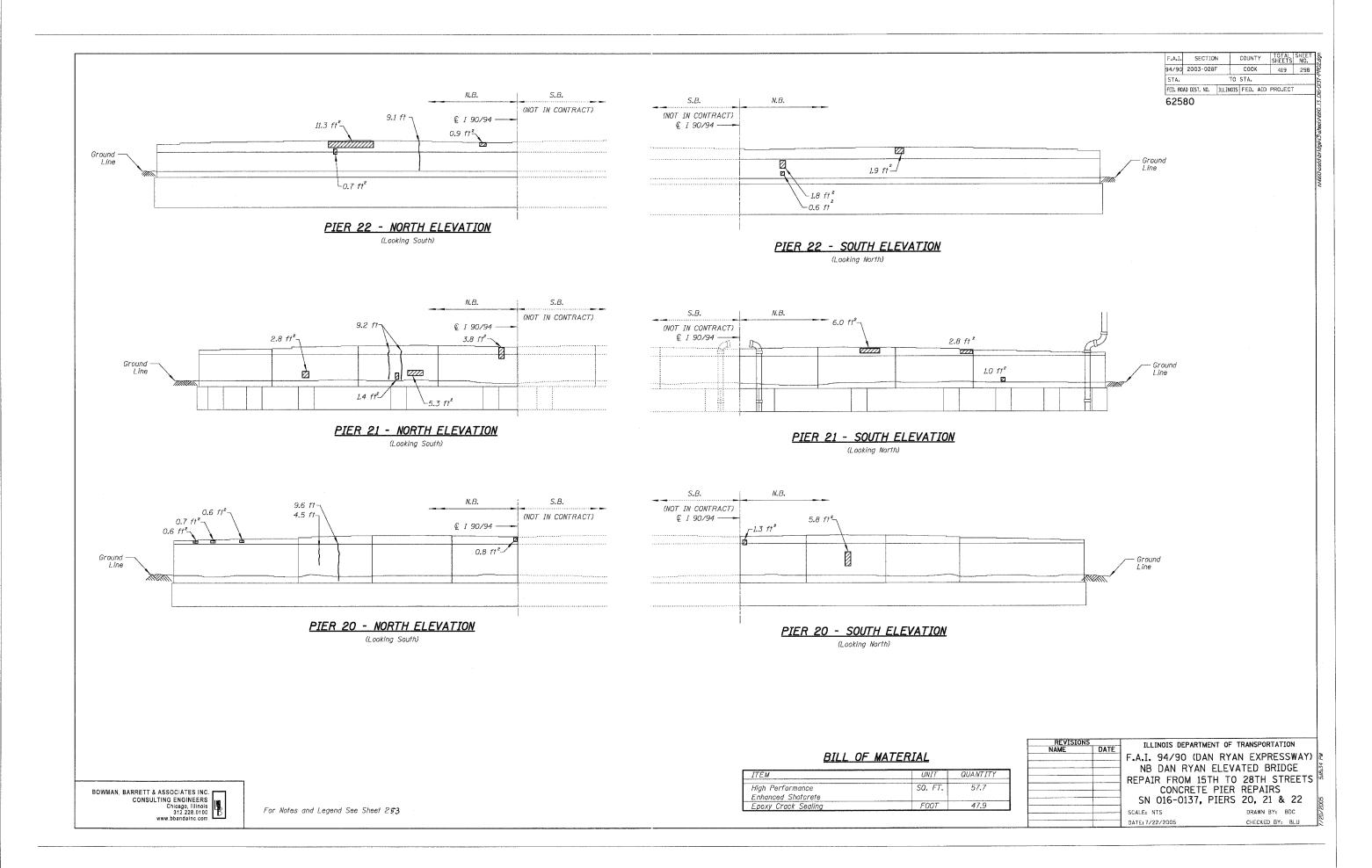
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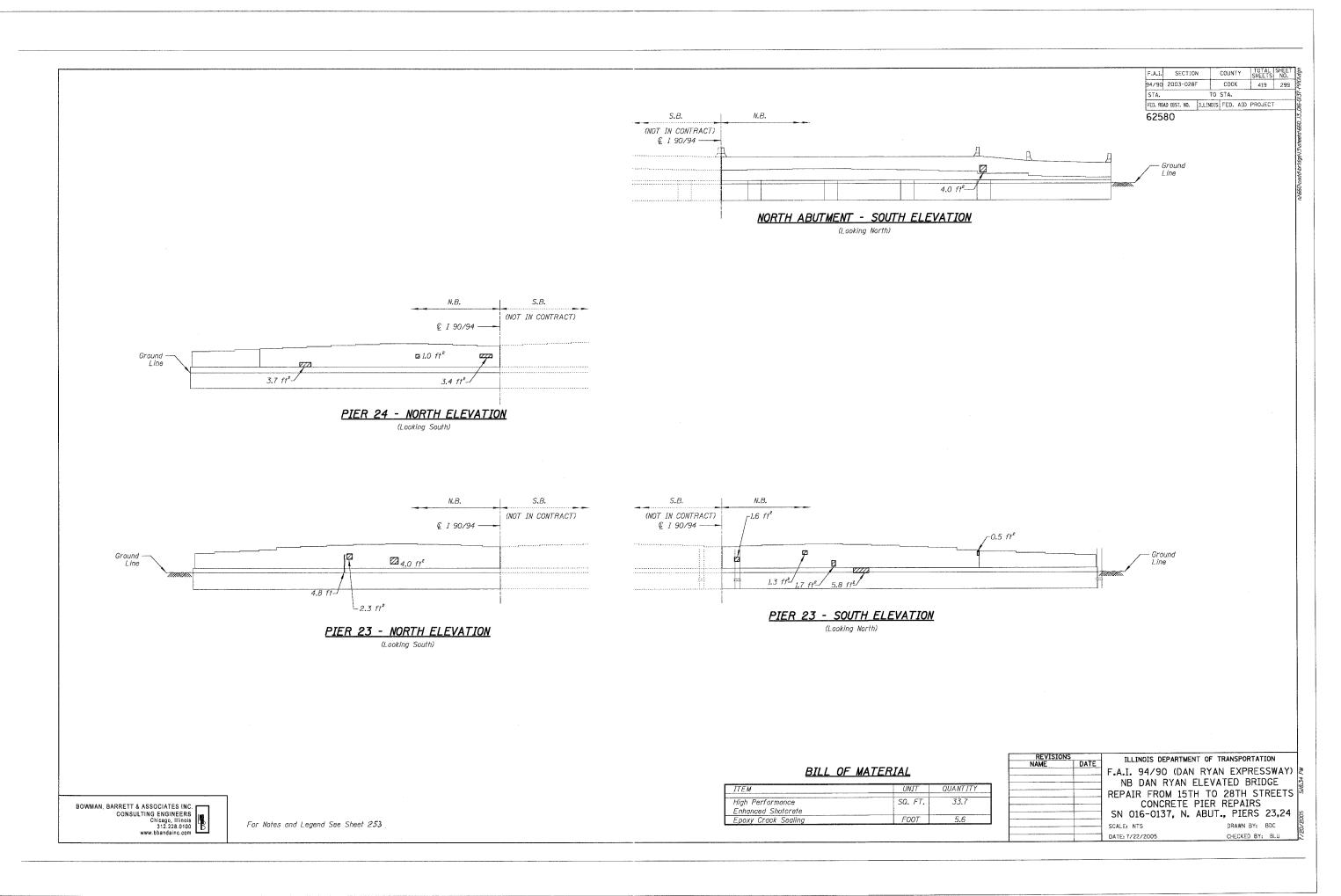


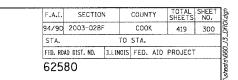


DATE: 7/22/2005

CHECKED BY: BLU



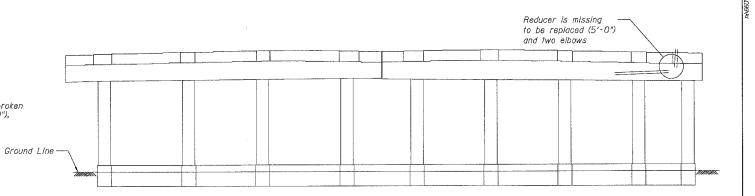




- Drain Pipe (Typ.) Ground Line-Remove and replace existing -- Drain pipe section broken to be replaced (7'-0"), below ground work cleanout. See Section A-A sheet no 306 may be required. See Section A-A sheet no 306

SN 016-0137 - PIER 9 - SOUTH ELEVATION

(Looking North)



SN 016-1110 - PIER 4 - SOUTH ELEVATION

(Looking North)

Drain pipe section missing to be replaced (10'-0") — Drain Pipe (Typ.) See Section A-A sheet no 306 Ground Line-Drain pipe section missing

SN 016-0137 - PIER 19 - SOUTH ELEVATION to be replaced (10'-0") See Section A-A sheet no 306 (Looking North)

-Drain pipe section missing to be replaced (5'-0") See Section A-A sheet no 306 Ground Line --SN 016-1110 - PIER 1 - NORTH ELEVATION Drain Pipe (Typ.) (Looking South)

DRAINAGE REPAIR NOTES

- 1. The locations shown as requiring drainage repairs identify the general deficiency and provide an estimated length of new pipe required. It shall be the Contractors responsibility to verify the locations, dimensions and details prior to ordering the material. Such variations shall not be cause for additional comprensation for a change in the scope of work, however the contractor will be paid for the Quantity actually furnished at the unit price bid for the work.
- 2. The Contractor shall provide all piping, brackets, hangers, fittings and accessories required to return the existing drainage system to a fully functional leak free system. See Special Provisions.
- 3. When existing drain pipes are wrapped with duct tape, the tape shall be removed if pipe is broken or miss connected. The pipe shall be replaced or otherwise new tape shall be provided. The type of the tape shall be approved by the Engineer. The cost shall be included in "Bridge Drainage System Repairs".
- 4. The Contractor shall wrap any joints that are leaking as directed by the Engineer. The cost shall be included in "Bridge Drainage System Repairs".
- 5. Contractor shall reconnect drainage troughs for finger plate expansion joints to drainage system as directed by the Engineer. It is estimated that 5'-0" of the existing drain pipe will need to be replaced to connect to the new troughs at each specified Joint.
- 6. For locations see framing plan
- 7. SN 016-0137, Span 1 Drain pipe section between beams 9S & 10S broken to be replaced (10'-0")

BILL OF MATERIAL

ITEM DESCRIPTION	UNITT	QUANTITY		
ITEM DESCRIPTION	UNIT	SN 016-0137	SN 016-1110	
Bridge Drainage System Repairs	FOOT	37	<i>1</i> 5	

REVISIONS		ILLINOIS DEPARTMENT OF TRANSPORTATION		
NAME	DATE	ILLINOIS DE ANTIMENT OF TRANSFORTATION		
		F.A.I. 94/90 (DAN RY	AN EXPRESSWAY)	
		NB DAN RYAN ELEV	ATED BRIDGE	
		REPAIR FROM 15TH TO	28TH STREETS	
		DRAINAGE R	EPAIRS	
		SN 016-0137 8	016-1110	
		SCALE: NTS	DRAWN BY: BDC	
		DATE: 8/12/2005	CHECKED BY: BLU	

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