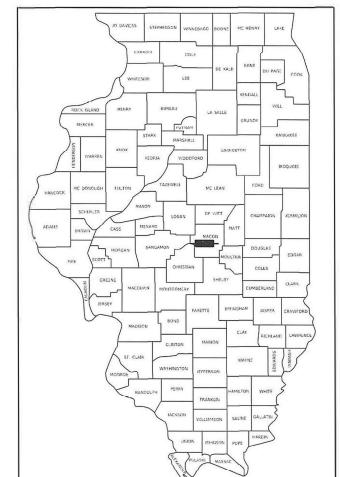
# STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

(128BR-1)B-1 ILLINOIS CONTRACT NO. 74368

D-97-023-09



STATE OF ILLINOIS

SUBMITTED OCTUBER 9

LOCATION OF SECTION INDICATED THUS: -

PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

**REV - MS** 

FOR INDEX OF SHEETS, SEE SHEET NO. 2

AADT (2021) = 4550

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# **PROPOSED** HIGHWAY PLANS

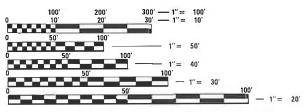
**FAP ROUTE 714 (IL 48) SECTION** (128BR-1)B-1 PROJECT STP-VNWG(689) STRUCTURE REPLACEMENT **MACON COUNTY** 

C-97-046-09

MALONIOLNEY

BLUE MOUND

PROJECT LOCATION **EXISTING SN 058-0029** PROPOSED SN 058-0137 STATION 382 + 15.80



ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATORS 1-800-892-0123

PROJECT ENGINEER: BRIAN LEWIS PROJECT MANAGER: BRIAN LEWIS

PHONE NO.: (217) 342-8360 **CONTRACT NO. 74368** 

GROSS LENGTH = 84.67 FT. = 0.016 MILE NET LENGTH = 84.67 FT. = 0.016 MILE

INDEX OF SHEETS

### GENERAL NOTES

THIS PROJECT IS LOCATED ON FAP 714 (IL 48) IN MACON COUNTY, OVER MOSQUITO CREEK, 1.5 MILES NORTHEAST OF BLUE MOUND.

THE WORK ON THIS PROJECT CONSISTS OF REMOVING AND REPLACING THE EXISTING STRUCTURE, NEW APPROACH PAVEMENTS, EARTH EXCAVATION AND EMBANKMENT, HOT-MIX ASPHALT MILLING, RESURFACING, HOT-MIX ASPHALT SHOULDERS, AGGREGATE SHOULDERS, GUARDRAIL, PAVEMENT MARKING, TRAFFIC CONTROL, AND ANY OTHER WORK NECESSARY TO COMPLETE THIS SECTION. THIS WORK WILL BE COMPLETED UTILIZING STAGE CONSTRUCTION.

ALL WORK ON, UNDER, OVER OR ADJACENT TO NORFOLK SOUTHERN RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH THE NORFOLK SOUTHERN PUBLIC PROJECT MANUAL.

PRIOR TO COMMENCING WORK THE CONTRACTOR SHALL REQUEST AND FOLLOW THE LATEST VERSION OF NORFOLK SOUTHERN'S "SPECIAL PROVISIONS FOR PROTECTION OF RAILROAD INTERESTS."

IF THE PROJECT REQUIRES THAT NS TRACK EMBANKMENT SLOPES MUST BE CLEARED TO ALLOW FOR PROJECT IMPROVEMENTS, THERE SHALL BE NO "CLEARING AND GRUBBING." ANY SLOPES REQUIRED TO BE ALTERED SHALL BE IMPROVED WITH RIPRAP TO MAINTAIN EXISTING SLOPE STABILITY, IN SUCH INSTANCES, PRIOR TO PLACEMENT OF FILTER FABRIC AND PROPOSED RIPRAP, THE EXISTING NS TRACK EMBANKMENT SLOPES SHALL BE CLEARED OF ALL DEBRIS INCLUDING TREE STUMPS, DEAD TREE BRANCHES, LARGE VEGETATION, AND OTHER OBJECTS THAT HAVE THE POTENTIAL TO PUNCTURE OR OTHERWISE DAMAGE THE PROPOSED FILTER FABRIC WHEN THE PROPOSED RIPRAP IS PLACED. HOWEVER, IF SUCH ITEMS, DEAD OR ALIVE, ARE ANCHORED IN THE SLOPE, THEY SHALL BE CUT OFF FLUSH WITH THE GROUND AND THEIR ROOTS LEFT IN PLACE TO ASSIST IN RETAINING STABILITY OF THE RAILROAD SLOPE, LIGHT VEGETATION MAY BE PRESSED DOWN ON THE NS EMBANKMENT SLOPE, TAKING CARE NOT TO STRIP, RAKE OR RIP OUT THE VEGETATION.

EXCAVATION WILL NOT BE ALLOWED WITHIN 5 FEET OF THE EXISTING WINGWALLS OF THE NORFOLK SOUTHERN BRIDGE. ANY DITCH EXCAVATION WITHIN 25 FEET OF ANY PORTION OF THE NORFOLK SOUTHERN BRIDGE SHALL BE LINED WITH RIPRAP GRADATION A4 AND SHALL HAVE FINISHED SLOPES NOSTEEPER THAN 1V:2H. THERE SHALL BE NO EXCAVATION ON, OR AT THE TOE OF THE NORFOLK SOUTHERN STRUCTURE SLOPES WITHOUT REVIEW AND COMPLIANCE WITH NORFOLK SOUTHERN RAILWAY SHORING REQUIREMENTS.

TOP OF RAIL ELEVATIONS SHALL BE CHECKED BY THE CONTRACTOR PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCY SHALL BE BROUGHT TO THE ATTENTION OF THE NORFOLK SOUTHERN ENGINEER OF PUBLIC IMPROVEMENTS.

THE DEPARTMENT WILL PROVIDE NORFOLK SOUTHERN RAILWAY WITH AS-BUILT DRAWINGS OF THE BRIDGE SHOWING THE ACTUAL DIMENSIONS AS CONSTRUCTED. DEPTH, SIZE, AND LOCATION OF FOUNDATION COMPONENTS SHALL BE SHOWN ON THE DRAWINGS.

TREES THREE (3) INCHES OR GREATER IN DIAMETER AT BREAST HEIGHT SHALL NOT BE CLEARED FROM APRIL 1ST THROUGH SEPTEMBER 30TH OF ANY GIVEN YEAR.

ONE NEW PERMANENT SURVEY MARKER SHALL BE CAST IN PLACE IN A WINGWALL OF THE NEW STRUCTURE.

PIPE UNDERDRAINS FOR STRUCTURES SHALL EXTEND TO THE BOTTOM OF THE EMBANKMENT SLOPE AND TERMINATE WITH A CONCRETE HEADWALL.

THE FOLLOWING MIXTURE REQUIREMENTS ARE APPLICABLE TO THIS PROJECT:

APPLICATION	AC/PG	DESIGN AIR VOIDS	MIXTURE COMPOSITION	AGGREGATE	PARAMETER
POLYMERIZED HMA SURFACE COURSE, MIX "D", N90	SBS PG 70-22	4.0% @ N=90	IL-9.5	MIXTURE D	QC/QA
POLYMERIZED HMA BINDER COURSE, IL-19.0, N90 (VAR DEPTH)	SBS PG 70-22	4.0% @ N=90	IL-19.0	N/A	QC/QA
HMA SHOULDERS (TOP LIFT)	PG 64-22	4.0% @ N=70	ĭL-9.5	MIXTURE C	QC/QA
HMA SHOULDERS (BOTTOM LIFT)	PG 64-22	4.0% @ N=70	IL-19.0	N/A	QC/QA

SHEET_NO.	ITEM
1	COVER SHEET
2	GENERAL NOTES, INDEX OF SHEETS & LIST OF STANDARDS
3-5	SUMMARY OF QUANTITIES
6	TYPICAL SECTION
7	SCHEDULES OF QUANTITIES
8-9	PLAN AND PROFILE SHEETS
10-11	STAGING DETAILS
12	EROSION CONTROL DETAILS
13	ROW SHEET
14	MILLING & PAVING DETAILS
15	GUARDRAIL DETAILS
16-41	STRUCTURAL SHEETS
42-45	DISTRICT DETAILS
46-54	CROSS SECTIONS

THE FOLLOWING STANDARDS ARE A PART OF THESE PLANS AND ARE INCLUDED FOLLOWING THE LAST NUMBERED SHEET OF THE PLANS.

	STD. NO.	DESCRIPTION
0	00001-09	STANDARD SYMBOLS, ABBREVIATIONS, AND PATTERNS
0	01001-02	AREAS OF REINFORCEMENT BARS
	001006	DECIMAL OF AN INCH AND OF A FOOT
2	80001-07	TEMPORARY EROSION CONTROL SYSTEMS
4	20401-13	PAVEMENT CONNECTOR (PCC) FOR BRIDGE APPROACH SLAB
4	82001-02	HMA SHOULDER ADJACENT TO FLEXIBLE PAVEMENT
5	15001-04	NAME PLATE FOR BRIDGES
6	30001-13	STEEL PLATE BEAM GUARDRAIL
6	30201-07	PCC/HMA STABILIZATION AT STEEL PLATE BEAM GUARDRAIL
6	30301-09	SHOULDER WIDENING FOR TYPE 1 GUARDRAIL TERMINALS
6	31031-18	TRAFFIC BARRIER TERMINAL, TYPE 6
7	01001-02	OFF-ROAD OPERATIONS, 2L, 2W, MORE THAN 15' AWAY
7	01006-05	OFF-ROAD OPERATIONS, 2L, 2W, 15' TO 24" FROM PAVEMENT EDGE
7	01011-04	OFF-ROAD MOVING OPERATIONS, 2L, 2W, DAY ONLY
7	01201-05	LANE CLOSURE, 2L, 2W, DAY ONLY, FOR SPEEDS >= 45 MPH
7	01301-04	LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
7	01311-03	LANE CLOSURE, 2L, 2W, MOVING OPERATIONS - DAY ONLY
7	01321-19	LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
7	01326-04	LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING, FOR SPEEDS $>=45$ MPH
7	01901-11	TRAFFIC CONTROL DEVICES
7	04001-08	TEMPORARY CONCRETE BARRIER
7	25001-01	OBJECT AND TERMINAL MARKERS
7	80001-05	TYPICAL PAVEMENT MARKINGS
7	81001-04	TYPICAL APPLICATIONS RAISED REFLECTIVE PAVMENT MARKERS
7	01316-14	LANE CLOSURE, 2L, 2w, BRIDGE REPAIR

THE PROPOSED PROJECT IS NOT ANTICIPATED TO CHANGE THE QUANTITY AND/OR CHARACTER OF FLOW ONTO THE NORFOLK SOUTHERN RAILWAY RIGHT-OF WAY.

USER NAME = jessica.hille	DESIGNED	REVISED =
	DRAWN	REVISED -
PLOT SCALE = 100.0000 * / in.	CHECKED	REVISED =
PLOT DATE = 12/31/2024	DATE	REVISED +

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

GENERA			INDEX STAND	OF SHEETS ARDS	&	
SHEET 1	OF	1	SHEETS	STA.		TO STA.

SCALE:

2	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHE
3 F	R-1)B-1		MACON	54	2
			CONTRACT	NO. 74	1368
	ILLINOIS	FED. A	ID PROJECT		

SECTION

(128BR-1)B-1

REV - MS

	SUMMARY OF QUANTITIE	· C			TRUCTION TYPE CODE	<b>-</b>	SUMMARY OF QUANTITIES				CTION TYPE CODE
CODE NO	ITEM	UNIT	TOTAL QUANTITIES	0010		CODE NO	ITEM	UNIT	TOTAL QUANTITIES	0010	
	112.19	J. C.	GUANTITES			- CODE NO	A1 L 49	JAN 1	GOARTITES		
0100500	TREE REMOVAL, ACRES	ACRE	0.5	0.5		35400300	PORTLAND CEMENT CONCRETE BASE COURSE WIDENING	SQ YD	251	251	
							8"				
0200100	EARTH EXCAVATION	CU YD	6	6							
						40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	1119	1119	
0200500	EARTH EXCAVATION (WIDENING)	CU YD	56	56							
						40600370	LONGITUDINAL JOINT SEALANT	F00T	497	497	
0300100	CHANNEL EXCAVATION	CU YD	1357	1357							
0400900	FURNISHED EXCAVATION	CU YD	1407	1407		40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SO YD	367	367	
0400800	FURNISHED EXCAVATION	0 10	1407	1407		40600990	TEMPORARY RAMP	SQ YD	67	67	
0600110	GRANULAR EMBANKMENT, SPECIAL	TON	262	262		10000330			0.	<u> </u>	
	•					40603240	POLYMERIZED HOT-MIX ASPHALT BINDER COURSE,	TON	486	486	
21101505	TOPSOIL EXCAVATION AND PLACEMENT	CU YD	584	584			IL-19.0, N90				
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	252	252		40604164	POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE,	TON	111	111	
							IL-9.5, MIX "D", N90				
28000305	TEMPORARY DITCH CHECKS	FOOT	20	20							
						42000060	WELDED WIRE REINFORCEMENT	SQ YD	80	80	
28000400	PERIMETER EROSION BARRIER	FOOT	770	770							
						42000080	PAVEMENT CONNECTOR (PCC) FOR BRIDGE APPROACH	SQ YD	137	137	
28100107	STONE RIPRAP, CLASS A4	SO YD	89	89			SLAB				
28100109	STONE RIPRAP, CLASS A5	SO YD	901	901		44000100	PAVEMENT REMOVAL	SQ YD	1 35	135	
28200200	FILTER FABRIC	SO YD	901	901		48101200	AGGREGATE SHOULDERS, TYPE B	TON	196	196	
31101900	SUBBASE GRANULAR MATERIAL, TYPE C	TON	32	32		48203100	HOT-MIX ASPHALT SHOULDERS	TON	563	563	

USER NAME = jessica.hille	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 100.0000 ' / in.	CHECKED -	REVISED -
PLOT DATE = 12/31/2024	DATE -	REVISED -

SCALE:

	CHMMADY OF CHANTITIES							F.A.P. SECTION			COUNTY	TOTAL SHEETS	SHEET NO.		
								714	(128BF	(128BR-1)B-1			54	3	
													CONTRACT	NO. 74	1368
	SHEET 1	OF	3	SHEETS	STA.		TO STA.				ILLINOIS	FED. A	ID PROJECT		

80% FED 20% STATE 80% FED 20% STATE

	CUMMARY OF CHANTITIES			CONS	TRUCTION TYPE CODE		CUMMARY OF CHANTITIES			CONS	STRUCTION TYPE CODE
	SUMMARY OF QUANTITIES	1=	TOTAL	0010		]	SUMMARY OF QUANTITIES		TOTAL	0010	
CODE NO	ITEM	UNIT	OUANTITIES			CODE NO	ITEM	UNIT	QUANTITIES		
50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1	1		51500100	NAME PLATES	EACH	1	1	
50200100	STRUCTURE EXCAVATION	CU YD	216	216		52100520	ANCHOR BOLTS, 1"	EACH	24	24	
50300225	CONCRETE STRUCTURES	CU YD	77.3	77.3		52200010	TEMPORARY SHEET PILING	SQ FT	301	301	
50300255	CONCRETE SUPERSTRUCTURE	CU YD	159.3	159.3		52200020	TEMPORARY SOIL RETENTION SYSTEM	SO FT	281	281	
50300260	BRIDGE DECK GROOVING	SO YD	602	602		58600101	GRANULAR BACKFILL FOR STRUCTURES	CU YD	124	124	
50300300	PROTECTIVE COAT	SO YD	742	742		59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	69	69	
50301350	CONCRETE SUPERSTRUCTURE (APPROACH SLAB)	CU YD	118.1	118.1		60100060	CONCRETE HEADWALLS FOR PIPE DRAINS	EACH	4	4	
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1	1		60146304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	150	150	
50500505	STUD SHEAR CONNECTORS	EACH	1968	1968		<b>*</b> 63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT	FOOT	425	425	
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	88490	88490			POSTS				
						63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4	4	
0800515	BAR SPLICERS	EACH	602	602		× 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL)	EACH	4	4	
51201610	FURNISHING STEEL PILES HP12X63	FOOT	604	604			TANGENT				
1202305	DRIVING PILES	FOOT	604	604		63200310	GUARDRAIL REMOVAL	FOOT	800	800	
1203610	TEST PILE STEEL HP12X63	EACH	1	1		66600105	FURNISHING AND ERECTING RIGHT OF WAY MARKERS	EACH	3	3	
							ALTY ITEM				REV - 1

MODEL: Default

USER NAME = jessica.hille

PLOT SCALE = 100.0000 ' / in.

PLOT DATE = 12/31/2024

DESIGNED -

CHECKED -

DRAWN

DATE

REVISED

REVISED

REVISED -

REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SCALE:

						F.A.P. RTE.	SECTION	COUNTY	COUNTY TOTAL SHEE SHEETS NO.		
	SUMM	AKY	OF QU	ANTITIES		714	(128BR-1)B-1	MACON	54	4	
								CONTRACT	Γ NO. <b>7</b> 4	4368	
SHEET 2	OF	3	SHEETS	STA.	TO STA.		ILLINOIS FED. A	ID PROJECT			

80% FED 20% STATE

80% FED 20% STATE

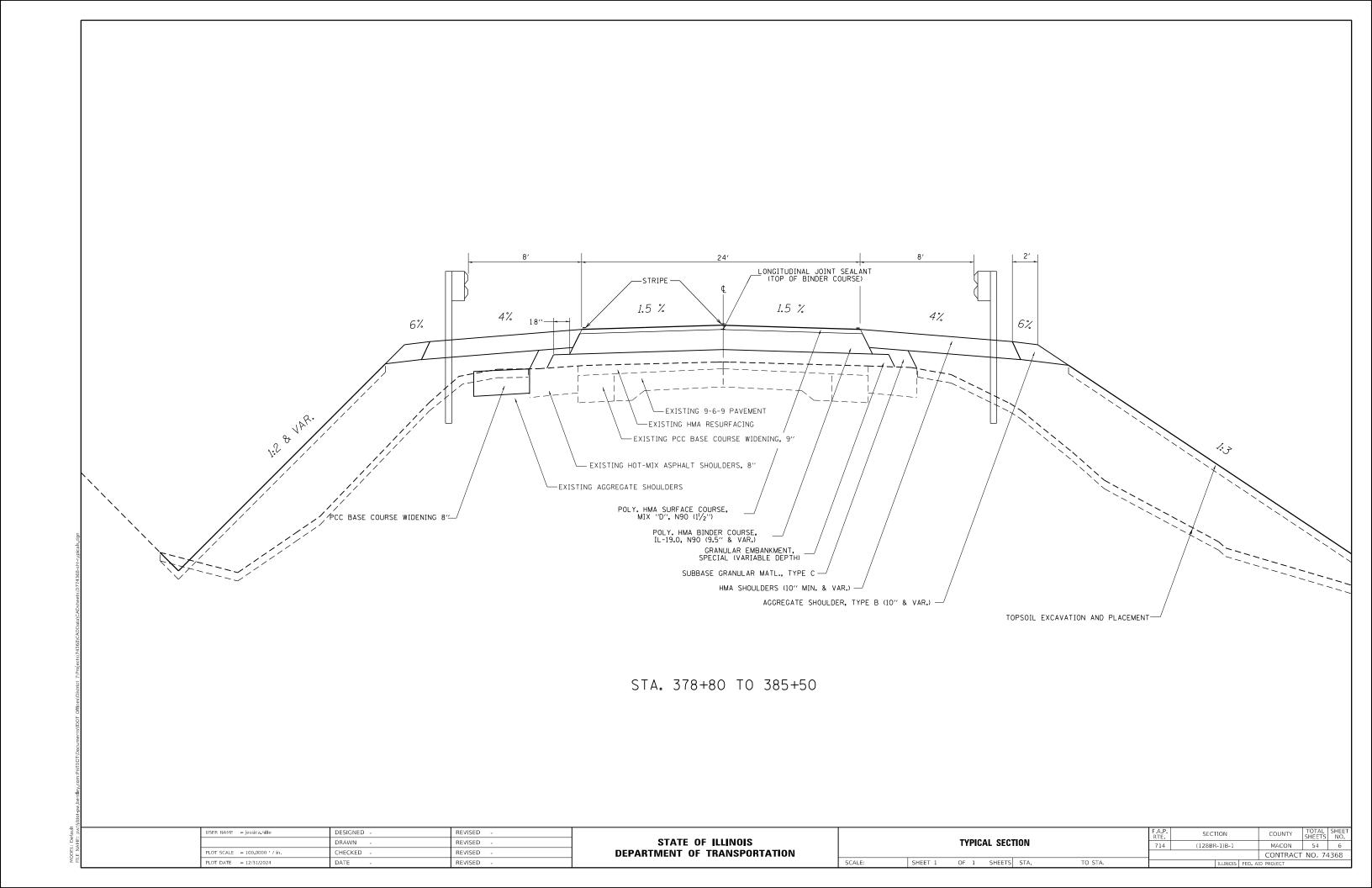
	SUMMARY OF QUANTITIES		}		STRUCTION TYPE CODE	$\dashv$ I	SUMMARY OF QUANTITIES				CTION TYPE CODE
CODE NO	ITEM	UNIT	TOTAL QUANTITIES	0010		CODE NO	ITEM	UNIT	TOTAL QUANTITIES	0010	
						70300221	TEMPORARY PAVEMENT MARKING - LINE 4"- PAINT	FOOT	1770	1770	
6700205	PERMANENT SURVEY MARKERS, TYPE I	EACH	1	1							
						70400100	TEMPORARY CONCRETE BARRIER	FOOT	750	750	
57000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	10	10							
						70400125	PINNING TEMPORARY CONCRETE BARRIER	EACH	48	48	
7100100	MOBILIZATION	L SUM	1	1							
						70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	675	675	
70100100	TRAFFIC CONTROL AND PROTECTION, STANDARD	EACH	1	1							
	701316					70600250	IMPACT ATTENUATORS, TEMPORARY (NON-	EACH	2	2	
							REDIRECTIVE), TEST LEVEL 3				
70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD	EACH	1	1							
	701321					70600350	IMPACT ATTENUATORS, RELOCATE (NON-	EACH	2	2	
							REDIRECTIVE), TEST LEVEL 3				
70100450	·	L SUM	1	1							
	701201					72501000	TERMINAL MARKER - DIRECT APPLIED	EACH	4	4	
70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD	L SUM	1	1		78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	1770	1770	
	701326					× 78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	11	11	
						78200005	GUARDRAIL REFLECTORS, TYPE A	EACH	10	10	
70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1	1							
						78300200	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	EACH	11	11	
70107005	PAVEMENT MARKING BLACKOUT TAPE, 5"	FOOT	529	529							
						X2501000	SEEDING, CLASS 2 (SPECIAL)	ACRE	1	1	
70107025	CHANGEABLE MESSAGE SIGN	CAL DA	28	28		x5080530	BAR TERMINATORS	EACH	588	588	
						Z0001900	ASBESTOS BEARING PAD REMOVAL	EACH	26	26	
70300100	SHORT TERM PAVEMENT MARKING	FOOT	286	286							
						Z0004552	APPROACH SLAB REMOVAL	SO YD	116	116	
70300150	SHORT TERM PAVEMENT MARKING REMOVAL	SQ FT	94	94							
						Z0048665	RAILROAD PROTECTIVE LIABILITY INSURANCE	L SUM	1	1	

USER NAME = jessica.hille	DESIGNED -	REVISED -
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STATI	E OI	FILLINOIS
DEPARTMENT	0F	TRANSPORTATION

SCALE:

		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.			
Su	IMMAKY	OF QUA	ANIIIIES		714	(128BR-1)B-1	MACON	54	5
							CONTRACT	NO. 74	1368
SHEET 3	OF 3	SHEETS	STA.	TO STA.		ILLINOIS FED. A	D PROJECT		



				HOT-MIX ASPHALT SURFACE REMOVAL - JOINT	BITUMINOUS MATERIA (TACK COAT)	POLYMERIZED HOT-MI ASPHALT BINDER COU IL-19.0, N90	POLYMERIZED HOT-MI ASPHALT SURFACE COURSE, IL-9.5, MIX N90	GRANULAR EMBANKM SPECIAL	LONGITUDINAL JOINT SEALANT
STATION	TO	STATION	LENGTH	SQ YD	POUND	TON	TON	TON	FOOT
378+80.00	-	379+00.00	20.0	66.7	45.0		4.5		20.0
379+00.00	-	379+25.00	25.0	83.3	56.3		5.6		25.0
379+25.00	-	379+35.00	10.0	33.3	22.5	e.	2.2		10.0
379+35.00		379+75.00	40.0		90.0	9.0	9.0		40.0
379+75.00		380+00.00	25.0		56.3	12.7	5.6		25.0
380+00.00	-0	380+25.00	25.0		56.3	19.7	5.6		25.0
380+25.00		380+50.00	25.0		<u>5</u> 6.3	45.6	5.6		25.0
380+50.00	-	380+75.00	25.0		56.3	50.0	5.6	5.5	25.0
380+75.00	-	381+00.00	25.0		56.3	50.0	5.6	18.4	25.0
381+00.00	-	381+25.00	25.0		56.3	50.0	5.6	27.8	25.0
381+25.00	-	381+29.47	4.5		10.1	9.0	1.0	6.3	4.5
381+29.47	-	381+44.47	15.0	BRIDGE APPROACH CONNECTOR				15.8	
381+44.47		381+74.47	30.0	BRIDGE APPROACH PAVEMENT				62.5	
381+74.47	-	382+57.13	82.7		BRIDGI	E DECK			
382+57.13	-	382+87.13	30.0	BF	RIDGE APPRO	ACH PAVEMEI	VT	61.8	
382+87.13	-	383+02.13	15.0	BR	IDGE APPROA	CH CONNECT	OR	14.6	
383+02.13	20	383+25.00	22.9		51.5	46.0	5.1	26.0	22.9
383+25.00		383+50.00	25.0		56.3	50.0	5.6	17.0	25.0
383+50.00	-	383+75.00	25.0		56.3	50.0	5.6	6.0	25.0
383+75.00	-	384+00.00	25.0		56.3	50.0	5.6		25.0
384+00.00	-	384+25.00	25.0		56.3	17.0	5.6		25.0
384+25.00		384+50.00	25.0		56.3	15.0	5.6		25.0
384+50.00	-	384+95.00	45.0		101.3	12.0	10.1		45.0
384+95.00	-	385+00.00	5.0	16.7	11.3		1.1		5.0
385+00.00	-	385+25.00	25.0	83.3	56.3		5.6		25.0
385+25.00	-	385+50.00	25.0	83.3	56.3		5.6		25.0
		TOTAL	S =	367	1119	486	111	262	497

Г	APPROACH SLAB REMOVAL							
ŀ	STATION	то	STATION	WIDTH	SQ YD			
Γ	381+74.60	-	381+94.60	26	58			

PAVEMENT REMOVAL

TO

STATION

381+74.60

382 + 90.13

TOTAL

WIDTH

SQ YD

67.5

135

STATION

381+41.47

382+67.90

STON	SS A5			
LOCAT		SQ YD		
SHOWN IN STRUC		901		
DOWNSTREAM BAI	DOWNSTREAM BANK PROTECTION			
	TOTAL		1057	

	TREE REMOVAL							
SIDE	STATION	TO	STATION	ACRE				
LT.	378+94	-	381+95	0.1				
RT.	380+29		382+59	0.1				
RT.	382+31		386+00	0.2				
			TOTAL	0.4				

FURNISHING AND ERECTING ROW MARKERS

OFFSET

40.00

47.50

60.00

TOTAL

STATION

378+50.00

379 + 00.00

380+74.42

SIDE

RT.

RT.

RT.

				SHORT TERM PAVEMENT MARKING	SHORT TERM PAVEMENT MARKING REMOVAL	TEMPORARY PAVEMENT MARKING, LINE - 4 INC	PAINT PAVEMENT MARKING, LINE - 4 INC	RAISED REFLECTIVE PAVEMENT MARKER	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL
STATION	TO	STATION	LENGTH	FEET	SQ FT	FEET	FEET	EACH	EACH
378+25.90	-	386+11.70	785.8	286	48	1769	1769	11	11
		TOTALS	5 =	286	28	1769	1769	11	11

PAVEMENT MARKING

	EARTHWORK SUMMARY									
		EARTH EXCAVATION								
		EMBANKMENT								
		ADJUSTED		EARTHWORK	TOPSOIL					
		FOR		BALANCE	EXCAVATION					
	EARTH	SHRINKAGE		WASTE(+) OR	AND					
LOCATION	EXCAVATION	(25%)	EMBANKMENT	SHORTAGE (-)	PLACEMENT					
	CU YD	CU YD	CU YD	CU YD	CU YD					
378+50 TO 386+00	6	5	2429	-2425	584					
CHANNEL EXCAVATION	1357	1018	0	1018	0					
TOTALS =	1363	1022	2429	-1407	584					

20200100 EARTH EXCAVATION =	
20300100 CHANNEL EXCAVATION =	1357
20400800 FURNISHED EXCAVATION =	
21101505 TOPSOIL EXCAVATION AND PLACEMENT=	584

SUGGESTED SEQUENCE OF OPERATIONS:

STAGE 1:

REMOVE TREES (SUBJECT TO DATE RESTRICTIONS)

CONSTRUCT WIDENING FOR STAGE 1

DEPLOY TRAFFIC CONTROL STANDARD 701321

CONSTRUCT STAGE 1 OF NEW STRUCTURE

CONSTRUCT EMBANKMENT ON EAST SIDE

CONSTRUCT BINDER AND SHOULDER FOR SOUTHBOUND

INSTALL GUARDRAIL ON EAST SIDE OF ROAD

STAGE 2:

RELOCATE BARRIER FOR STAGE 2 AND MOVE TRAFFIC TO SB LANE

CONSTRUCT STAGE 2 OF NEW STRUCTURE

CONSTRUCT EMBANKMENT ON WEST SIDE

CONSTRUCT BINDER AND SHOULDER FOR NORTHBOUND

INSTALL GUARDRAIL ON WEST SIDE OF ROAD

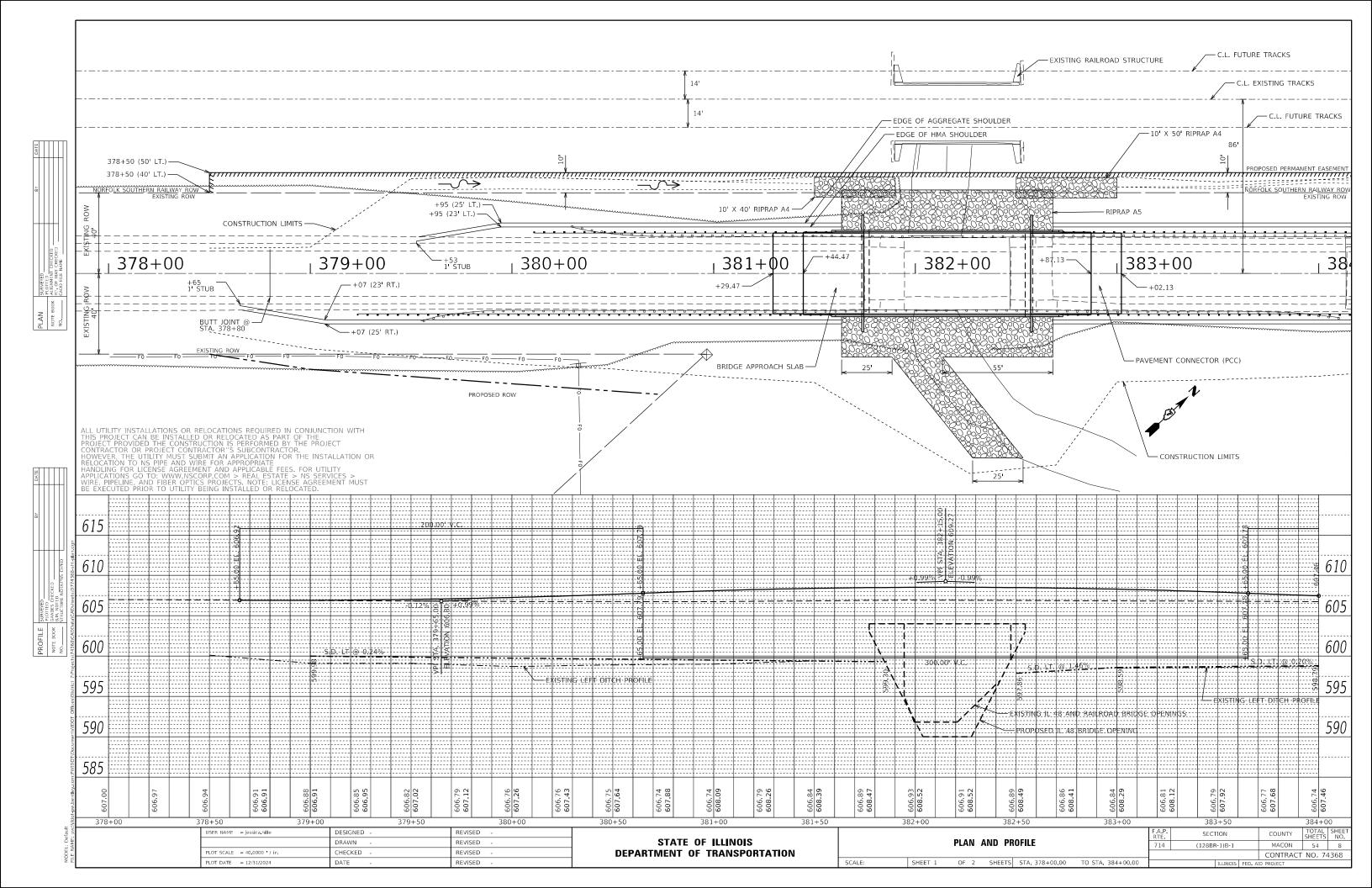
REMOVE BARRIER AND RESTORE TWO-WAY TRAFFIC

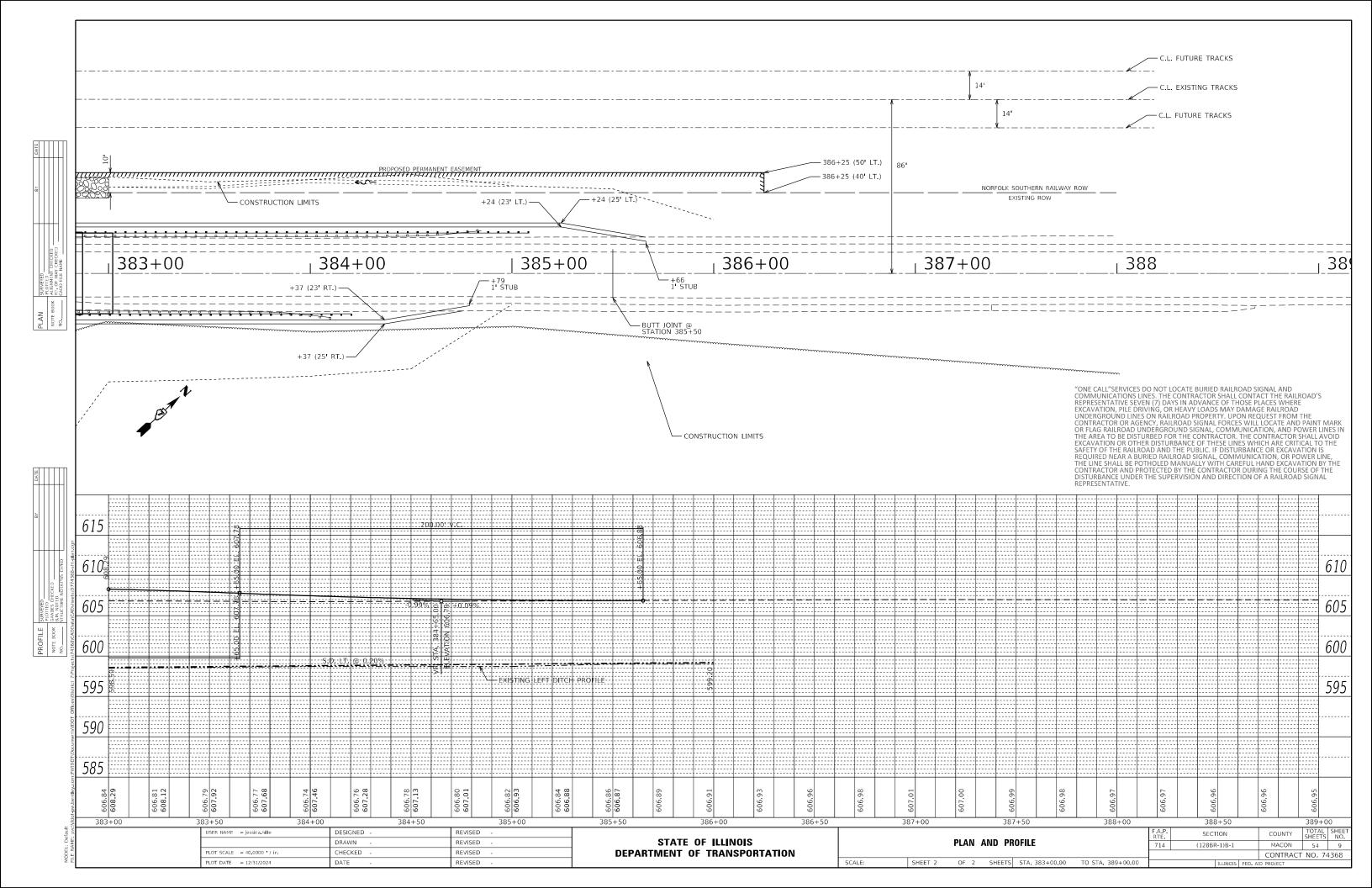
CONSTRUCT BUTT JOINTS AND SURFACE COURSE FOR BOTH LANES

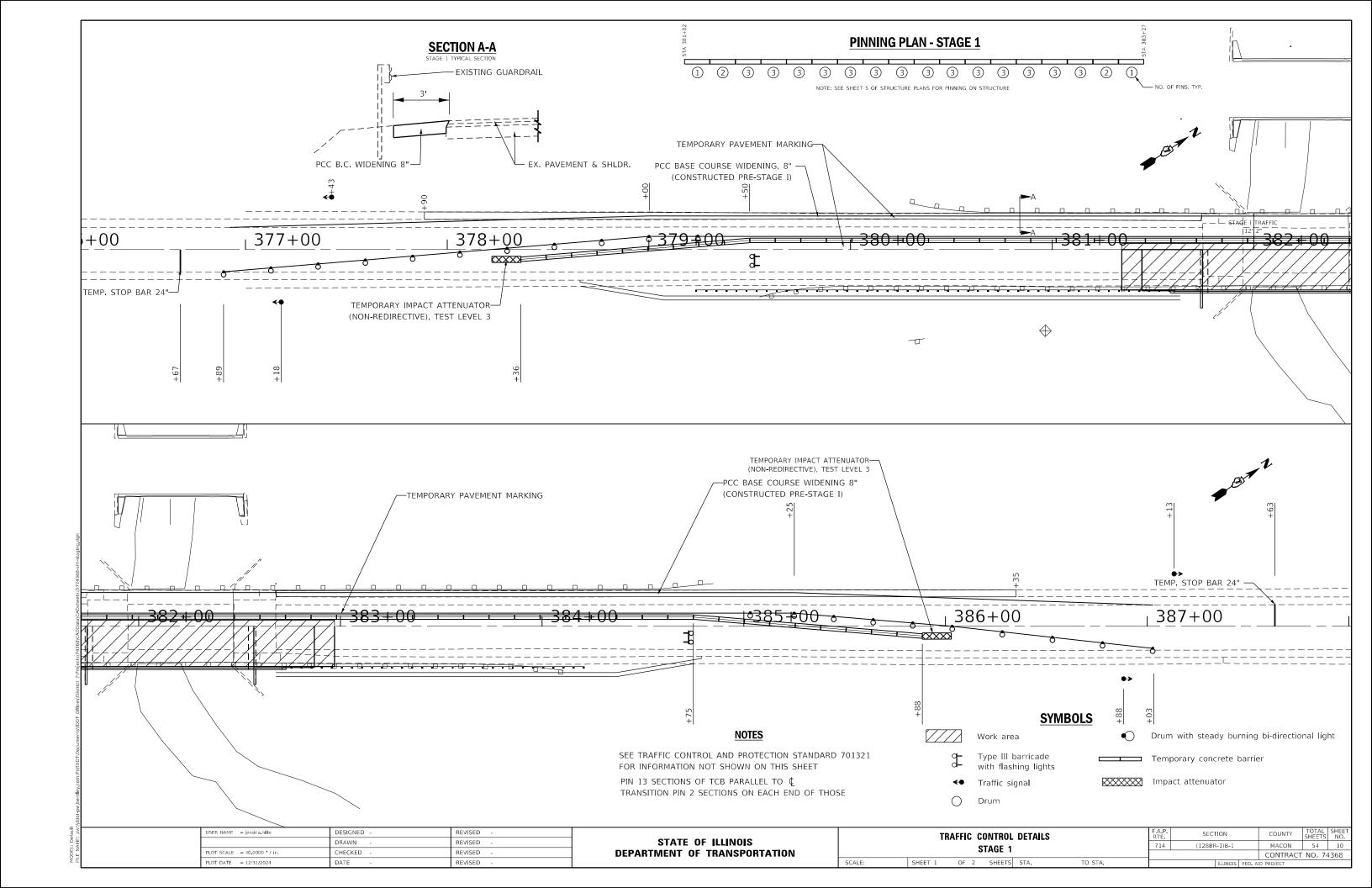
CONSTRUCT SEEDING, REMAINING RIPRAP AND PAVEMENT MARKING

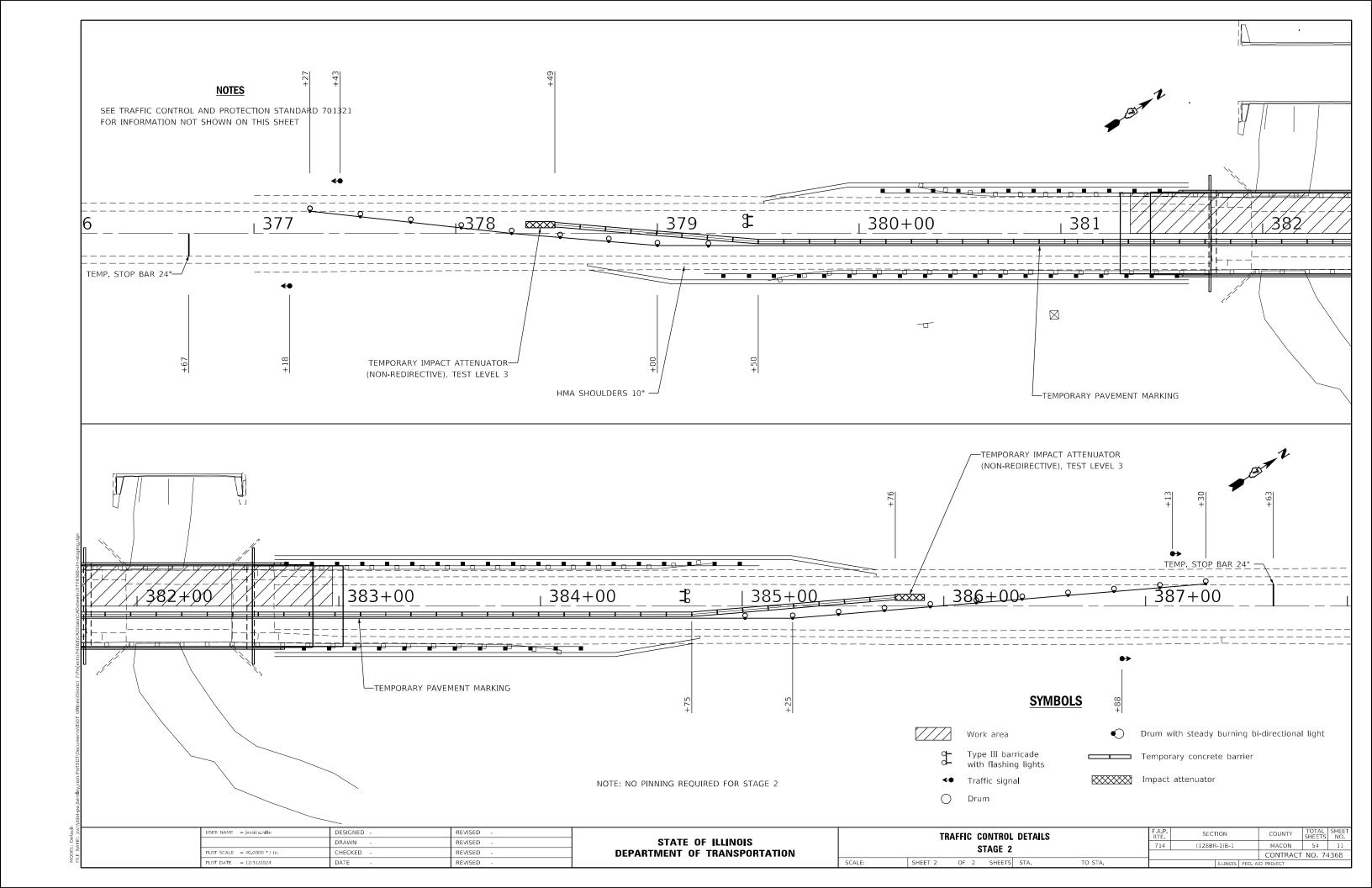
JSER NAME = jessica hille DESIGNED -REVISED SECTION STATE OF ILLINOIS SCHEDULES OF QUANTITIES DRAWN REVISED (128BR-1)B-1 MACON 54 7 CHECKED REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 74368 PLOT DATE = 12/31/2024 SHEET 1 OF 1 SHEETS STA. TO STA. REVISED DATE

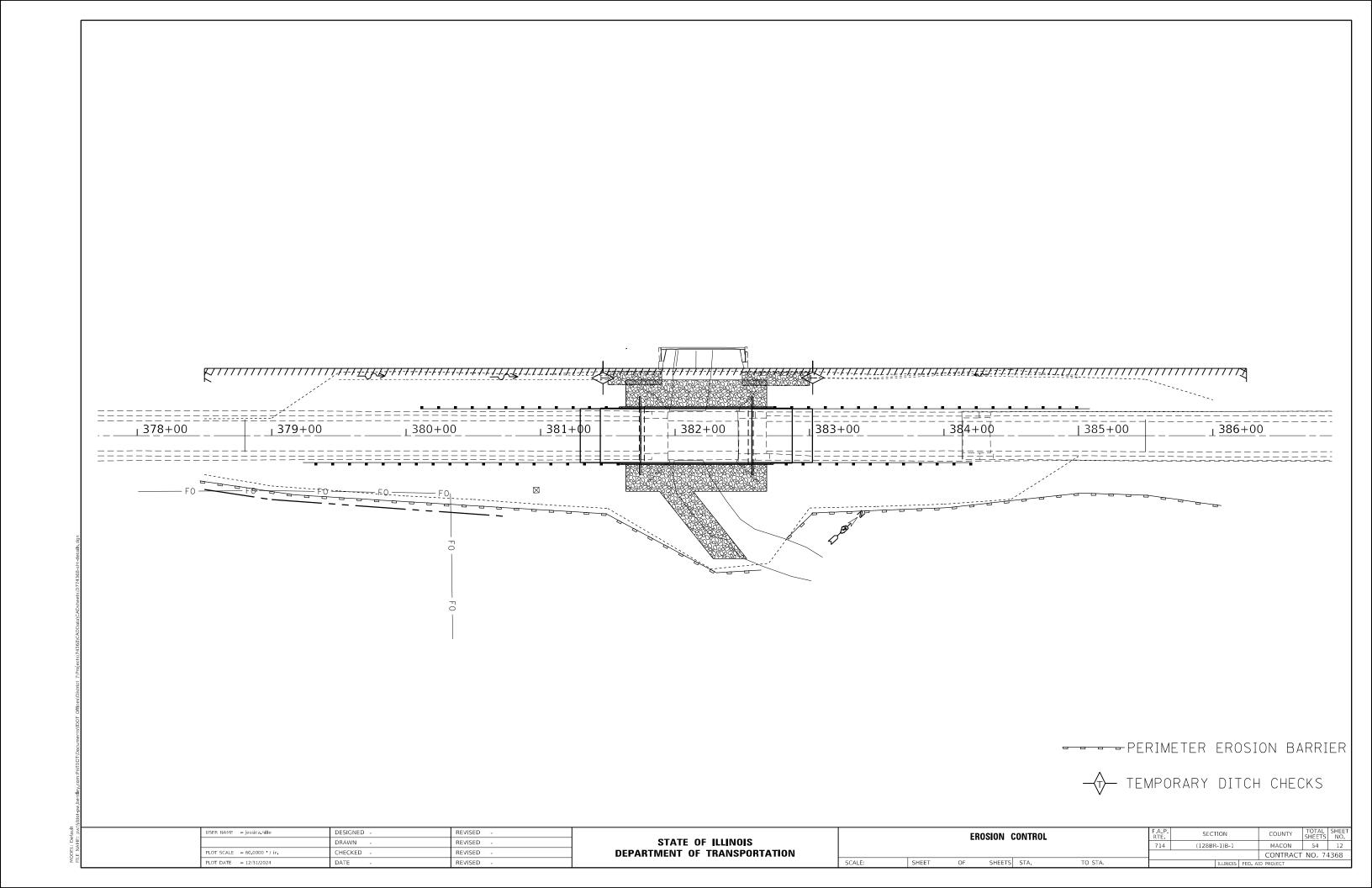
EACH

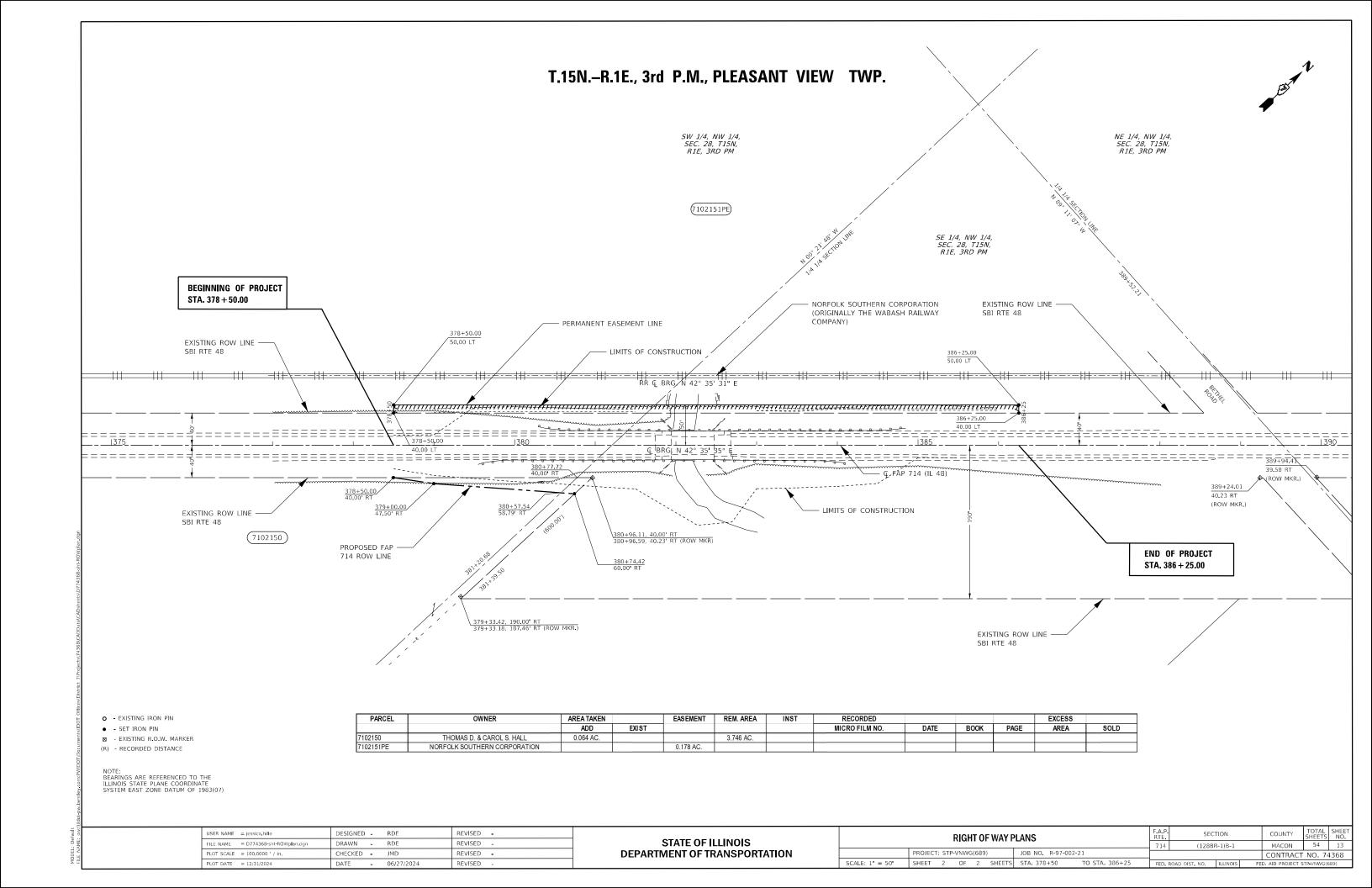












381+35 608.32 606.81 1.51 PAVEMENT CONNECTOR JSER NAME = jessica hille DESIGNED -REVISED SECTION COUNTY **PAVING DETAIL** STATE OF ILLINOIS DRAWN REVISED 714 (128BR-1)B-1 MACON 54 14 **DEPARTMENT OF TRANSPORTATION** PLOT SCALE = 100.0000 / in. CHECKED REVISED CONTRACT NO. 74368 PLOT DATE = 12/31/2024 DATE REVISED SCALE: SHEET OF SHEETS STA. TO STA.

BUTT JOINT & PROFILE CHANGE DETAIL

GRANULAR EMBANKMENT, SPECIAL PROPOSED PROFILE -

HMA BINDER COURSE, IL 19.0, N90

PROPOSED PROFILE

HMA BINDER COURSE, IL 19.0, N90

STATION	ELEVA	TIONS	INCREASE	NOTES
	PROPOSED	EXISTING	(FEET)	
378+80	606.91	606.90	0.01	BUTTJOINT
378+85	606.91	606.89	0.01	
378+90	606.91	606.89	0.02	
378+95	606.91	606.88	0.03	
379+00	606.91	606.88	0.03	
379+05	606.91	606.87	0.04	
379+10	606.92	606.86	0.06	
379+15	606.93	606.86	0.07	
379+20	606.94	606.85	0.08	
379+25	606.95	606.85	0.10	
379+30	606.96	606.84	0.12	
379+35	606.97	606.84	0.13	END HMA SURF REM BT JT
379+40	606.98	606.83	0.15	
379+45	607.00	606.82	0.17	
379+50	607.02	606.82	0.20	BEGIN BINDER
379+55	607.03	606.81	0.22	ZZZY DINYDEN
379+60	607.05	606.81	0.25	
379+65	607.07	606.80	0.23	
379+70	607.10	606.80	0.27	
379+75		606.79	0.30	
379+75 379+80	607.12 607.15	606.79	0.33	
379+85	607.17	606.78	0.39	
379+90	607.20	606.77	0.43	
379+95	607.23	606.77	0.46	
380+00	607.26	606.76	0.50	
380+05	607.29	606.76	0.53	
380+10	607.33	606.76	0.57	
380+15	607.36	606.76	0.60	
380+20	607.40	606.76	0.64	
380+25	607.43	606.76	0.68	
380+30	607.47	606.75	0.72	
380+35	607.51	606.75	0.76	
380+40	607.56	606.75	0.80	
380+45	607.60	606.75	0.85	
380+50	607.64	606.75	0.89	
380+55	607.69	606.75	0.94	
380+60	607.74	606.75	0.99	BEGIN G. E. S.
380+65	607.79	606.75	1.04	
380+70	607.83	G0G.74	1.09	
380+75	607.88	606.74	1.14	
380+80	607.93	606.74	1.18	
380+85	607.97	606.74	1.23	
380+90	608.01	606.74	1.27	
380+95	608.05	606.74	1.31	
381+00	608.09	606.74	1.35	
381+05	608.13	606.75	1.38	
381+10	608.16	606.76	1.41	
381+15	608.20	606.77	1.43	
381+20	608.23	606.78	1.45	
381+25	608.26	606.79	1.47	
381+30	608.29	606.80	1.49	
204 - 25	500.25	505.00	2.73	DANGE ACTUAL CONTRACTOR

L<sub>1.5"</sub>

- HMA SURFACE COURSE, MIX "D", N90

HMA SURFACE REMOVAL -BUTT JOINT

EXISTING PROFILE -

1.5"

STATION	ELEVA.	TIONS	INCREASE	NOTES
	PROPOSED	EXISTING	(FEET)	
382+95	608.31	606.84	1.47	PAVEMENT CONNECTOR
383+00	608.29	606.84	1.45	
383+05	608.26	606.83	1.42	
383+10	608.23	606.83	1.40	
383+15	608.19	606.82	1.37	
383+20	608.16	606.82	1.34	
383+25	608.12	606.81	1.31	
383+30	608.09	606.81	1.28	
383+35	608.05	606.80	1.24	
383+40	608.01	606.80	1.21	
383+45	607.97	606.79	1.17	
383+50	607.92	606.79	1.13	
383+55	607.88	606.79	1.09	
383+60	607.83	606.78	1.05	
383+65	607.78	606.78	1.00	END G. E. S.
383+70	607.73	606.77	0.96	2,7,5 0, 2, 5,
383+75	607.73	606.77	0.90	
383+80	607.64	606.76	0.87	
383+85	607.59	606.76	0.83	
383+90	607.55	606.75	0.80	
383+95			0.80	
	607.51	606.75	200 0000	
384+00	607.46	606.74	0.72	
384+05	607.43	606.75	0.68	
384+10	607.39	606.75	0.64	
384+15	607.35	606.75	0.60	
384+20	607.32	606.76	0.56	
384+25	607.28	606.76	0.52	
384+30	607.25	606.77	0.48	
384+35	607.22	606.77	0.45	
384+40	607.19	606.77	0.41	
384+45	607.16	606.78	0.38	
384+50	607.13	606.78	0.35	
384+55	607.11	606.78	0.32	
384+60	607.08	606.79	0.29	
384+65	607.06	606.79	0.27	
384+70	607.04	606.80	0.24	
384+75	607.01	606.80	0.22	END BINDER
384+80	607.00	606.80	0.19	
384+85	606.98	606.81	0.17	
384+90	606.96	606.81	0.15	
384+95	606.95	606.81	0.13	BEGIN HMA SURF REM BT J
385+00	606.93	606.82	0.11	
385+05	606.92	606.82	0.10	
385+10	606.91	606.83	0.08	
385+15	606.90	606.83	0.07	
385+20	606.89	606.84	0.05	
385+25	606.88	606.84	0.04	
385+30	606.88	606.85	0.03	
385+35	606.87	606.85	0.02	
385+40	606.87	606.85	0.02	
385+45	606.87	606.86	0.01	
385+50	606.87	606.86	0.01	BUTT JOINT

HMA SURFACE REMOVAL -BUTT JOINT

1.5"

385+50

55′±

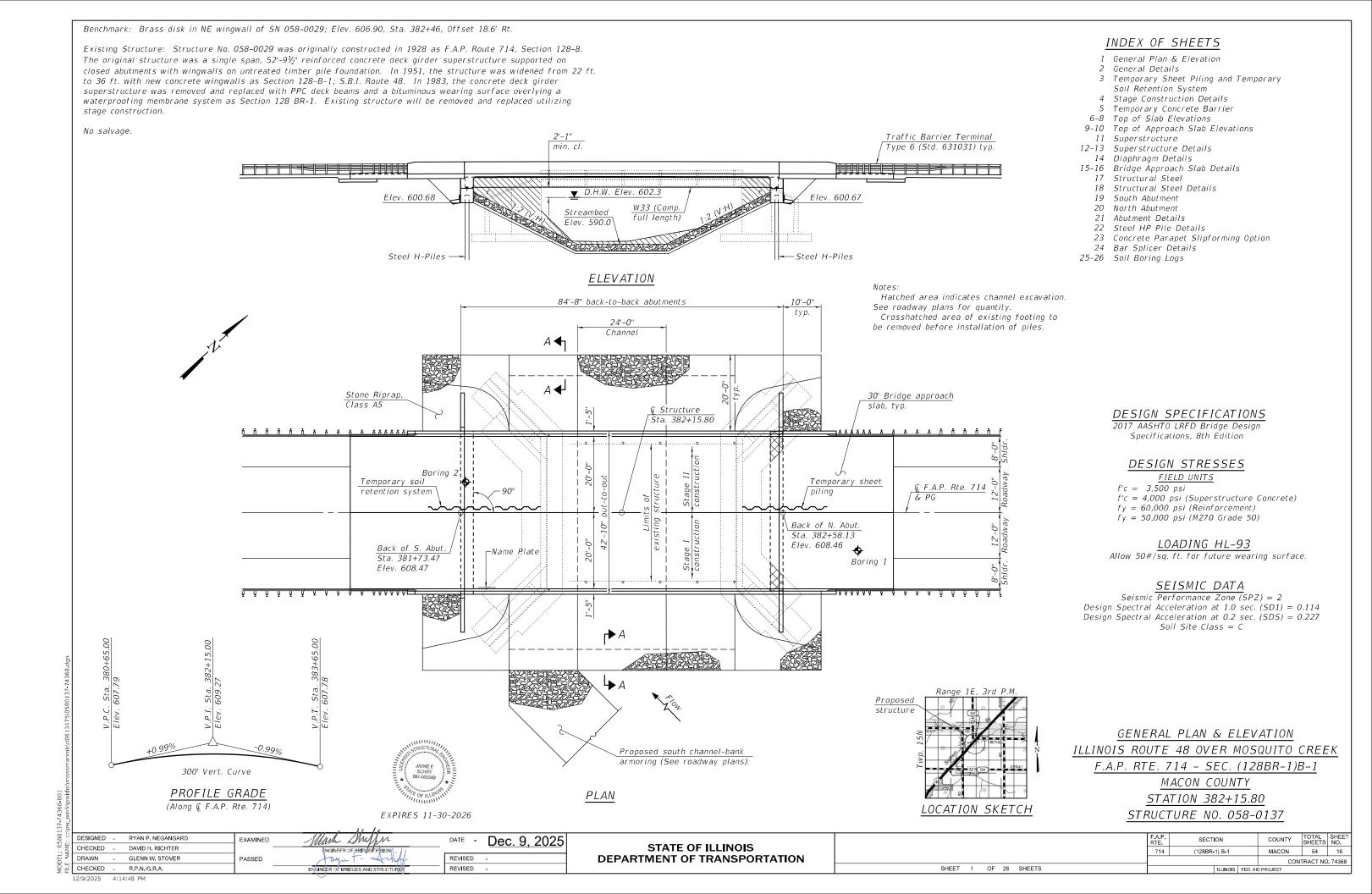
L<sub>1.5"</sub>

EXISTING PROFILE

GUARDRAIL REFLECTOR TYPE A TERMINAL MARKER -DIRECT APPLIED TRAFFIC BARRIER TERMINAL, TYPE 6 STATION TO STATION FEET EACH EACH EACH EACH LT.
LT.
LT.
LT.
LT.
LT.
LT.
LT. 380+10 380+60 380+60 381+23 62.5 381 + 22 382+72 383+09 383+09 384+57 384+59 385+09 150.0 RT. RT. RT. RT. 379+23 379+73 379+73 381+23 150.0 381+23 382+72 381+60 383+09 383+09 383+72 383+72 62.5 384+22 TOTALS = 425 4 COUNTY TOTAL SHEET NO.

MACON 54 15 USER NAME = jessica.hille DESIGNED -REVISED SECTION **GUARDRAIL DETAILS** STATE OF ILLINOIS DRAWN REVISED 714 (128BR-1)B-1 **DEPARTMENT OF TRANSPORTATION** PLOT SCALE = 60.0000 ' / in. CHECKED REVISED CONTRACT NO. 74368 PLOT DATE = 12/31/2024 DATE REVISED SCALE: SHEET OF SHEETS STA. TO STA.

		LON +25	1.	<del></del>		FON +85	
<u>1378+00</u>		1380+00	 382+00	1383+00			<u>1 386+00</u> _
	-92+ NOT		i'i	C Band	90+ NO7		
						GUARDRAIL	ı,



### DESIGN SCOUR ELEVATION TABLE

	Event / Limit Design Scour Elevations (ft.)									
Event / Limit	Design S	ions (ft.)								
State	S. Abut.	N. Abut.	Item 113							
Q100	600.68	600.67								
Q200	600.68	600.67	8							
Design	600.68	600.67	0							
Check	600.68	600.67								

### SECTION A-A

### GENERAL NOTES

Fasteners shall be ASTM F 3125 Grade A325 Type 1, mechanically galvanized bolts. Bolts  $\frac{3}{4}$  in. Ø, holes  $^{15}\!\!f_{16}$  in. Ø, unless otherwise noted.

Calculated weight of Structural Steel = 121,440 lbs. (M270 grade 50).

Calculated weight of Structural Steel = 8,910 lbs. (M270 grade 36).

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be gray, Munsell No. 5B 7/1.

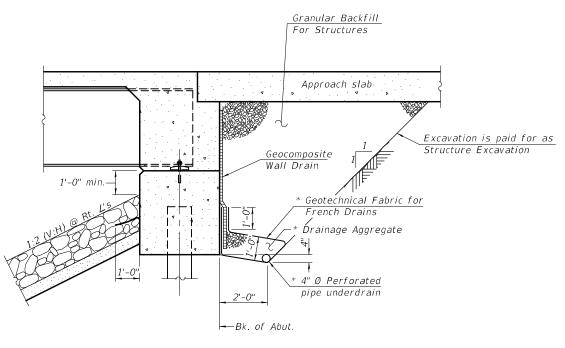
Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.

The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.

The contractor is advised that the existing PPC deck beams are in deteriorated condition with reduced load carrying capacity. It is the contractor's responsibility to account for the condition of the beams when developing construction procedures for removal and replacement of the superstructure.

STATION 382 + 15.80
BUILT 20 BY
STATE OF ILLINOIS
FAP RTE. 714 SEC. (128BR-1)B-1
LOADING HL-93
STRUCTURE NO. 058-0137

NAME PLATE
See Std. 515001



### SECTION THRU INTEGRAL ABUTMENT

\* Included in the cost of Pipe Underdrains for Structures.

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

December 9, 2025

REVISED

REVISED

### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Stone Riprap, Class A5	Sq. Yd.		901	901
Filter Fabric	Sq. Yd.		901	901
Removal of Existing Structures	Each			1
Structure Excavation	Cu. Yd.		216	216
Concrete Structures	Cu. Yd.		77.3	77.3
Concrete Superstructure	Cu. Yd.	159.3		159.3
Bridge Deck Grooving	Sq. Yd.	602		602
Protective Coat	Sq. Yd.	742		742
Concrete Superstructure (Approach Slab)	Cu. Yd.	118.1		118.1
Furnishing & Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	1968		1968
Reinforcement Bars, Epoxy Coated	Pound	79,850	8,640	88,490
Bar Splicers	Each	502	100	602
Furnishing Steel Piles HP12X63	Foot		604	604
Driving Piles	Foot		604	604
Test Pile Steel HP12X63	Each		1	1
Name Plates	Each	1		1
Anchor Bolts, 1"	Each		24	24
Temporary Sheet Piling	Sq. Ft.		301	301
Temporary Soil Retention System	Sq. Ft.		281	281
Granular Backfill for Structures	Cu. Yd.		124	124
Geocomposite Wall Drain	Sq. Yd.		69	69
Asbestos Bearing Pad Removal	Each		26	26
Pipe Underdrain for Structures 4"	Foot		150	150
Bar Terminators	Each	172	416	588

### WATERWAY INFORMATION

Existing Overtopping Elev. = 606.74 at Sta. 381+00 Drainage Area = 12.4 sq. mi. Proposed Overtopping Elev. = 606.87 at Sta. 385+50									
Drainage Area = 12.	4 Sq. II	11. PI	oposea	overtopp	nng Ere	v = b c	16.87 at	. Sta. 3	85+50
Flood	Freq.	Q	0 peni	ng Ft²	Nat.	Head	- Ft.	Headwa	ater El.
7 7000	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
	10	1720	397	495	600.9	0.3	0.2	601.2	601.1
Design	50	2820	468	594	602.3	0.5	0.3	602.8	602.6
Base	100	3310	497	636	602.9	0.5	0.3	603.4	603.2
Scour Design Check	200	3830	527	682	603.5	0.6	0.3	604.1	603.8
Overtop Existing	N/A	-	-	-	-	-	-	-	-
Overtop Proposed	N/A	-	-	-	-	-	-	-	-
Max. Calc.	500	4540	566	743	604.3	1.2	0.8	605.5	605.1

10 Year velocity through existing structure = 4.3 ft./sec.

10 Year velocity through proposed structure = 3.5 ft./sec.

DESIGNED - RYAN P. NEGANGARD

CHECKED - DAVID H. RICHTER

DRAWN - GLENN W. STOVER

CHECKED - R.P.N./ G.R.A.

ENGNEER OF BRIDGE VESEN

DAVID + STOVER

PASSED

ENGNEER OF BRIDGE VESEN

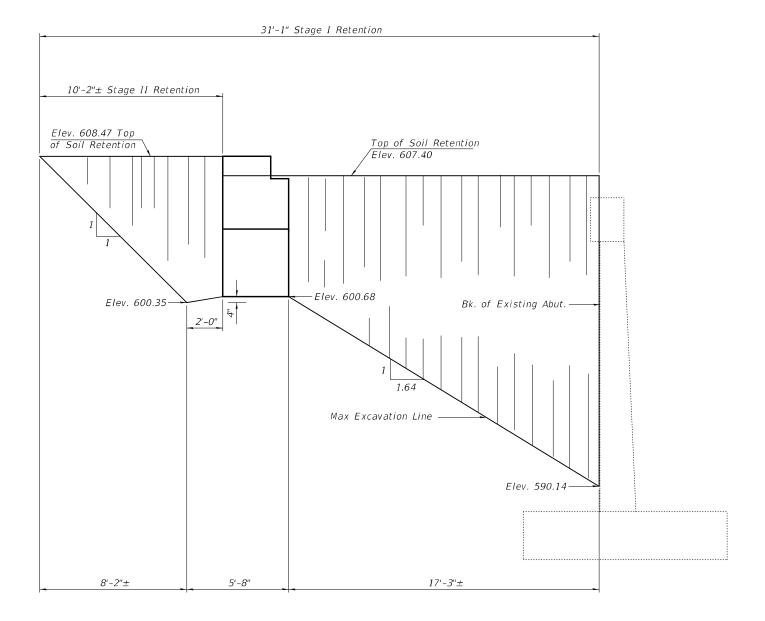
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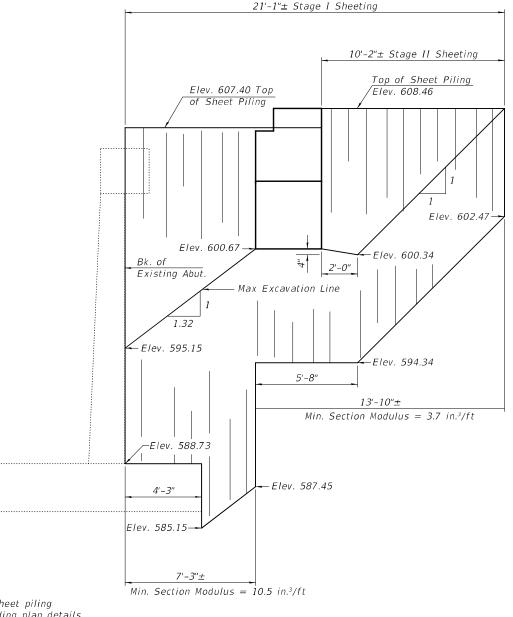
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 GENERAL DETAILS
 FAP. RTE.
 SECTION
 COUNTY SHEETS NO.
 SHEET NO.

 STRUCTURE 058-0137
 714
 (1288R-1)B-1
 MACON
 54
 17

 SHEET 2
 0F 26
 SHEETS
 CONTRACT NO. 74368





### TEMPORARY SOIL RETENTION SYSTEM

(South Abutment Looking West)

If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer. (North Abutment)

The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling. (North Abutment)

A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer. (South Abutment)

#### TEMPORARY SHEET PILING

(North Abutment Looking West)

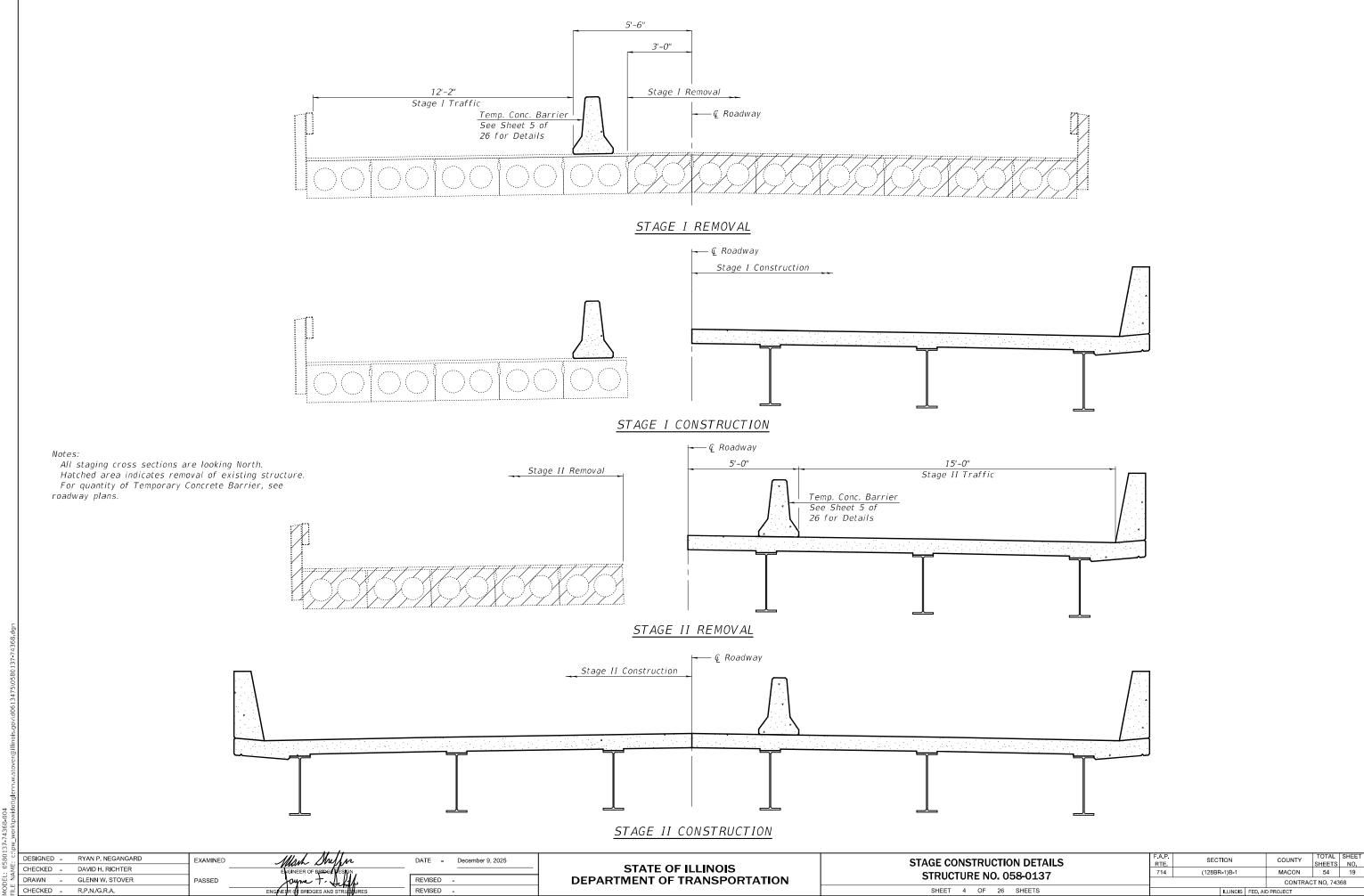
DESIGNED - RYAN P. NEGANGARD EXAMINED CHECKED - DAVID H. RICHTER GLENN W. STOVER PASSED 우른 CHECKED - R.P.N./G.R.A.

December 9, 2025 REVISED -REVISED

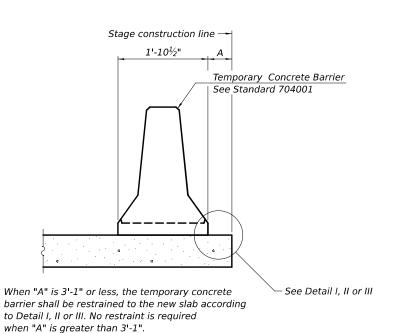
**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  TEMPORARY SHEET PILING & T.S.R. SYSTEM **STRUCTURE NO. 058-0137** SHEET 3 OF 26 SHEETS

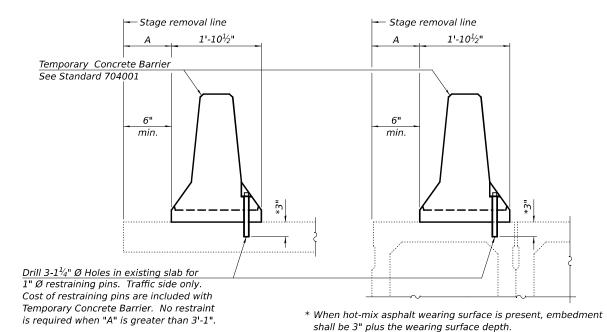
SECTION COUNTY MACON 54 18 (128BR-1) B-1 CONTRACT NO. 74368

12/10/2025 8:24:16 AM



12/10/2025 8:24:17 AM





US Std.  $1\frac{1}{16}$ " I.D.  $\times 2\frac{1}{2}$ " O.D. x approx. 8 gauge thick washer 1" Ø pin RESTRAINING PIN

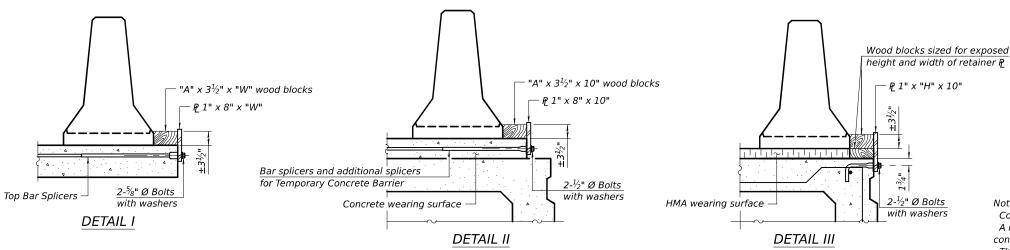
 $\frac{7}{16}$ " Ø hole

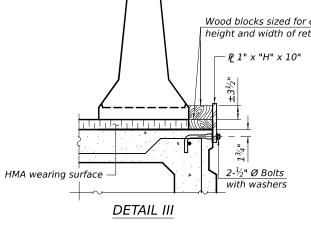
1x8 UNC

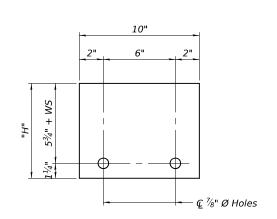
NEW SLAB OR NEW DECK BEAM

EXISTING DECK BEAM

#### SECTIONS THRU SLAB OR DECK BEAM







# STEEL RETAINER P 1" x "H" x 10" (Detail III)

"W" Detail I Detail II 2" Top bars Spa. 2" Detail I Detail II € 7/8" Ø Holes

STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)

#### RAILING CRITERIA

NCHRP 350 Test Level	3
Railing Weight (plf)	440

R-27 5-15-2023

DESIGNED - RYAN P. NEGANGARD EXAMINED December 9, 2025 CHECKED -DAVID H. RICHTER GLENN W. STOVER PASSED REVISED -CHECKED - R.P.N./G.R.A. REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION** 

### TEMPORARY CONCRETE BARRIER **STRUCTURE NO. 058-0137** SHEET 5 OF 26 SHEETS

F.A.P. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
714 (128BR-1) B-1				MACON	54	20
			CONTRA	CT NO.	74368	
		ILLINOIS	ID PROJECT			

Notes:

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate  ${\mathcal C}$  of each temporary concrete barrier.

BAR SPLICER FOR #4 BAR - DETAIL III

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

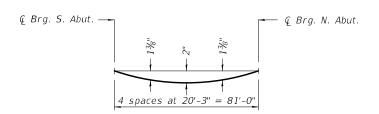
When the 'A' dimension is less than  $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

Detail I - Installation for a new bridge deck or bridge slab.

Detail II - Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

12/10/2025 8:59:30 AM

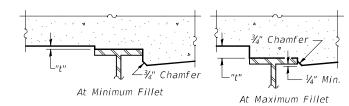


# DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

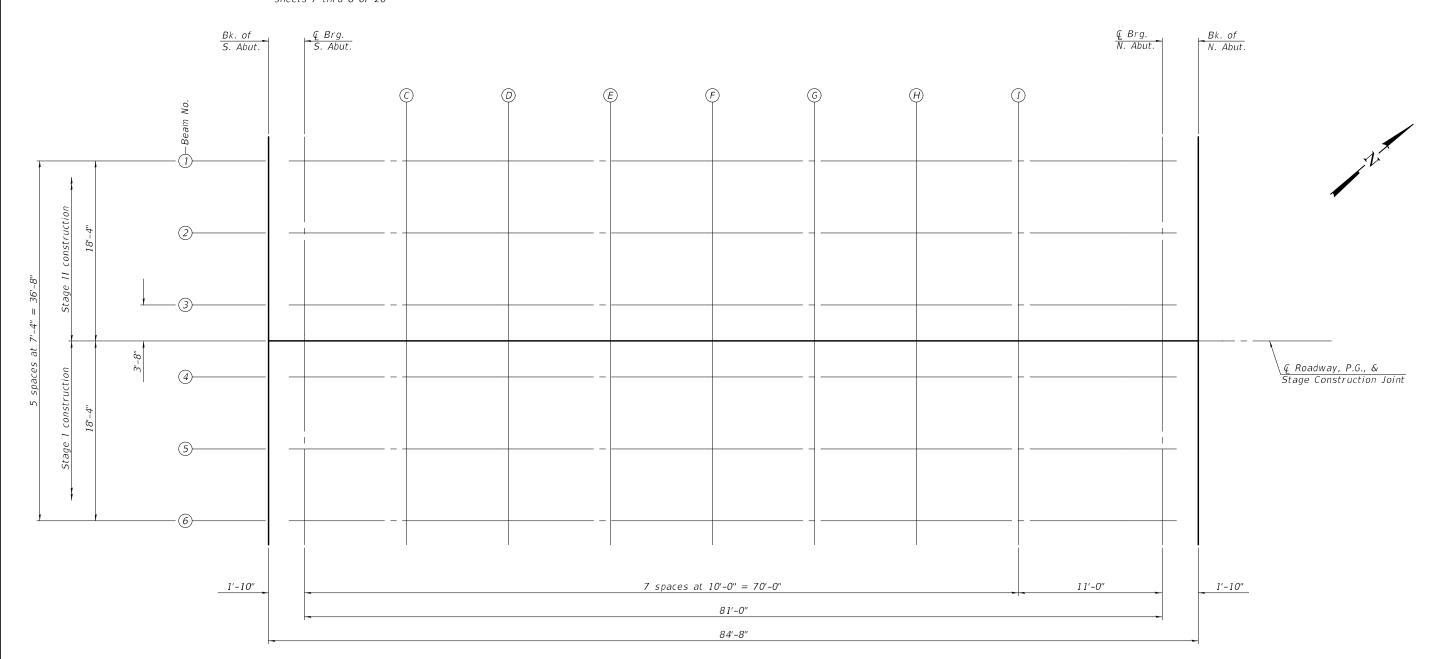
Note:

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on sheets 7 thru 8 of 26



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheets 7 thru 8 of 26, minus slab thickness, equals the fillet heights "t" above top flange of beams.

### FILLET HEIGHTS



PLAN

137 \pw				1. 1.								
088	DESIGNED -	RYAN P. NEGANGARD	EXAMINED	March Mulher	DATE - December 9, 2025		TOP OF SLAB ELEVATIONS	F.A.P. SI	ECTION	COUNTY	TOTAL	SHEET
9 H	CHECKED -	DAVID H. RICHTER	_	ENGINEER OF BRIDGE DESIGN		STATE OF ILLINOIS		714 (128	BR-1) B-1	MACON	54	21
P F	DRAWN -	GLENN W. STOVER	PASSED	Jayne + Alff	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 058-0137	(,	,		ONTRACT N	O 74368
<u>8</u> H	CHECKED -	R.P.N./G.R.A.		ENGINEER OF BRIDGES AND STRUCTURE	RES REVISED -		SHEET 6 OF 26 SHEETS		ILLINOIS FEE	). AID PROJECT		

### BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of S. Abut.	381+73.47	- 18 . 33	608.17	608.17
Q Brg. S. Abut.	381+75.30	- 18 . 33	608.17	608.17
C D E F G H I	381+85.30 381+95.30 382+05.30 382+15.30 382+25.30 382+35.30 382+45.30	- 18 . 33 - 18 . 33	608.19 608.21 608.22 608.22 608.21 608.19	608.25 608.32 608.36 608.38 608.36 608.33 608.25
Q Brg. N. Abut.	382+56 . 30	- 18 . 33	608.16	608.16
Bk. of N. Abut.	382+58.13	- 18 . 33	608.16	608.16

# <u>BEAM 2</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of S. Abut.	381+73.47	- 11.00	608.31	608.31
Q Brg. S. Abut.	381+75.30	- 11.00	608.31	608.31
C D E F G H I	381+85.30 381+95.30 382+05.30 382+15.30 382+25.30 382+35.30 382+45.30	- 11 . 00 - 11 . 00	608.33 608.35 608.36 608.36 608.35 608.33	608.39 608.47 608.50 608.53 608.50 608.47 608.40
Q Brg. N. Abut.	382+56 . 30	- 11.00	608.30	608.30
Bk. of N. Abut.	382+58.13	- 11.00	608.30	608.30

# BEAM 3

Location	Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of S. Abut.	381+73.47	-3.67	608.42	608.42
Q Brg. S. Abut.	381+75.30	-3.67	608.42	608.42
С	381+85.30	-3.67	608.44	608.50
D E	381+95.30	-3.67	608.46	608.58
F	382+05.30	-3.67	608.47	608.61
· ·	382+15.30	-3.67	608.47	608.64
G	382+25.30	-3.67	608.47	608.61
H	382+35.30	-3.67	608.46	608.58
I	382+45.30	-3.67	608.44	608.51
Q Brg. N. Abut.	382+56.30	-3.67	608.41	608.41
Bk. of N. Abut.	382+58.13	-3.67	608.41	608.41

# Q ROADWAY, P.G., & STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of S. Abut.	381+73.47	0.00	608.47	608.47
Q Brg. S. Abut.	381+75.30	0.00	608.48	608.48
C D E F G H I	381+85.30 381+95.30 382+05.30 382+15.30 382+25.30 382+35.30 382+45.30	0.00 0.00 0.00 0.00 0.00 0.00	608.50 608.52 608.52 608.53 608.52 608.51	608.56 608.63 608.66 608.69 608.67 608.63 608.56
Q Brg. N. Abut.	382+56.30	0.00	608.47	608.47
Bk. of N. Abut.	382+58.13	0.00	608.46	608.46

MODEL: 0580137-74368-007

DESIGNED - RYAN P. NEGANGARD EXAMINED CHECKED - DAVID H. RICHTER

DRAWN - GLENN W. STOVER PASSED CHECKED - R.P.N./G.R.A.

DATE - December 9, 2025

REVISED - REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 058-0137

SHEET 7 OF 26 SHEETS

A.P. SECTION COUNTY TOTAL SHEETS NO.
714 (128BR-1) B-1 MACON 54 22
CONTRACT NO. 74368

# BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of S. Abut.	381+73.47	3.67	608.42	608.42
Q Brg. S. Abut.	381+75.30	3.67	608.42	608.42
C D E F G H I	381+85.30 381+95.30 382+05.30 382+15.30 382+25.30 382+35.30 382+45.30	3.67 3.67 3.67 3.67 3.67 3.67	608.44 608.46 608.47 608.47 608.47 608.46 608.44	608.50 608.58 608.61 608.64 608.61 608.58 608.51
Q Brg. N. Abut.	382+56.30	3.67	608.41	608.41
Bk. of N. Abut.	382+58.13	3.67	608.41	608.41

# BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of S. Abut.	381+73.47	11.00	608.31	608.31
Q Brg. S. Abut.	381+75.30	11.00	608.31	608.31
C D E F G H I	381+85.30 381+95.30 382+05.30 382+15.30 382+25.30 382+35.30 382+45.30	11.00 11.00 11.00 11.00 11.00 11.00	608.33 608.35 608.36 608.36 608.35 608.33	608.39 608.47 608.50 608.53 608.50 608.47 608.40
Q Brg. N. Abut.	382+56.30	11.00	608.30	608.30
Bk. of N. Abut.	382+58.13	11.00	608.30	608.30

# <u>BEAM 6</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of S. Abut.	381+73.47	18.33	608.17	608.17
Q Brg. S. Abut.	381+75.30	18.33	608.17	608.17
C D E F G H I	381+85.30 381+95.30 382+05.30 382+15.30 382+25.30 382+35.30 382+45.30	18.33 18.33 18.33 18.33 18.33 18.33 18.33	608.19 608.21 608.22 608.22 608.22 608.21 608.19	608.25 608.32 608.36 608.38 608.36 608.33 608.25
Q Brg. N. Abut.	382+56.30	18.33	608.16	608.16
Bk. of N. Abut.	382+58.13	18.33	608.16	608.16

DESIGNED - RYAN P. NEGANGARD EXAMINED CHECKED – DAVID H. RICHTER DRAWN - GLENN W. STOVER
CHECKED - R.P.N./G.R.A. PASSED

REVISED -

REVISED -

DATE - December 9, 2025 STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS **STRUCTURE NO. 058-0137** SHEET 8 OF 26 SHEETS

COUNTY TOTAL SHEET NO.

MACON 54 23 SECTION (128BR-1) B-1 CONTRACT NO. 74368

### WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
S. End of S. Appr.	381+44.47	-20.00	608.03
A B	381+54.47 381+64.47	-20.00 -20.00	608.07 608.10
N. End of S. Appr.	381+74.47	-20.00	608.13

### WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
S. End of S. Appr.	381+44.47	-12.00	608.19
A B	381+54.47 381+64.47	-12.00 -12.00	608.23 608.26
N. End of S. Appr.	381+74.47	-12.00	608.29

### G ROADWAY, P.G., & STAGE CONST. JOINT

Location	Station	Offset	Theoretica Grade Elevations
S. End of S. Appr.	381+44.47	0.00	608.37
A B	381+54.47 381+64.47	0.00 0.00	608.41 608.44
N. End of S. Appr.	381+74.47	0.00	608.47

# -West Edge of Shoulder Stage II Construction -West Edge of Roadway © Roadway, P.G, & Stage Construction Joint. Stage I Construction Back of S. Abut. N. End of S. Approach Slab -East Edge of Roadway S. End of S. Approach Slab └─East Edge Of Shoulder 3 spaces at 10'-0" = 30'-0"

### EAST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
S. End of S. Appr.	381+44.47	12.00	608.19
А В	381+54.47 381+64.47	12.00 12.00	608.23 608.26
N. End of S. Appr.	381+74.47	12.00	608.29

### EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
S. End of S. Appr.	381+44.47	20.00	608.03
А В	381+54.47 381+64.47	20.00 20.00	608.07 608.10
N. End of S. Appr.	381+74.47	20.00	608.13

<u>PLAN</u>

DESIGNED - RYAN P. NEGANGARD EXAMINED CHECKED - DAVID H. RICHTER DRAWN - GLENN W. STOVER PASSED S 는 CHECKED - R.P.N./G.R.A.

DATE - December 9, 2025 REVISED REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TOP OF SOUTH APPROACH SLAB ELEVATIONS **STRUCTURE NO. 058-0137** SHEET 9 OF 26 SHEETS

SECTION (128BR-1) B-1 MACON CONTRACT NO. 74368

### WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
S. End of N. Appr.	382+57.13	-20.00	608.13
J K	382+67.13 382+77.13	-20.00 -20.00	608.10 608.06
N. End of N. Appr.	382+87.13	-20.00	608.01

### WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretica Grade Elevations
S. End of N. Appr.	382+57.13	-12.00	608.29
J K	382+67.13 382+77.13	-12.00 -12.00	608.26 608.22
N. End of N. Appr.	382+87.13	-12.00	608.17

### G ROADWAY, P.G., & STAGE CONST. JOINT

Location	Station	Offset	Theoretical Grade
			Elevations
S. End of N. Appr.	382+57.13	0.00	608.47
J K	382+67.13 382+77.13	0.00 0.00	608.44 608.40
N. End of N. Appr.	382+87.13	0.00	608.35

# ∽West Edge of Shoulder Stage II Construction —West Edge of Roadway © Roadway, P.G., & Stage Const. Joint Stage I Construction N. End of N. Approach Slab Back of N. Abut. ►East Edge of Roadway S. End of N. Approach Slab └─East Edge Of Shoulder 3 spaces at 10'-0" = 30'-0"

### EAST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
S. End of N. Appr.	382+57.13	12.00	608.29
J K	382+67.13 382+77.13	12.00 12.00	608.26 608.22
N. End of N. Appr.	382+87.13	12.00	608.17

### EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretica Grade Elevations
S. End of N. Appr.	382+57.13	20.00	608.13
J K	382+67.13 382+77.13	20.00 20.00	608.10 608.06
N. End of N. Appr.	382+87.13	20.00	608.01

<u>PLAN</u>

DESIGNED - RYAN P. NEGANGARD CHECKED - DAVID H. RICHTER DRAWN - GLENN W. STOVER Q H CHECKED - R.P.N./G.R.A.

EXAMINED

PASSED

DATE - December 9, 2025 REVISED -REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF NORTH APPROACH SLAB ELEVATIONS **STRUCTURE NO. 058-0137** SHEET 10 OF 26 SHEETS

COUNTY TOTAL SHEET NO.

MACON 54 25 SECTION (128BR-1) B-1 CONTRACT NO. 74368

Notes: MIN. BAR LAP See sheets 12 thru 13 of 26 for Superstructure details #5 bar = 3'-6" and Bill of Material. Bars indicated thus 42 x 3-#5 etc. indicates 42 lines of bars with 3 lengths per line. 125-#5 d1(E) bars at 8" cts. 1'-0" typ. Stage II Construction 166-#5 a(E) bars at  $5\frac{1}{2}$ " cts. Top 4-#5 a(E) bars a top, each end 102-#5 a1(E) bars at 9" cts. Bottom bars at 12" cts. each : 22 stage 11) 4-Bar splicers (E) at 12" cts. – Bk. of N. Abut - Stage Const. Joint || for #5 a(E) bars, top each end. — ⊊ Roadway & P.G. Out to Out Bk. of S. Abut. #5 v100(E) headed (22 stage I 42'-10" 166-Bar Splicers (E) at  $5\frac{1}{2}$ " cts. for #5 a(E) bars Top 102-Bar Splicers (E) at 9" cts. for #5 a1(E) bars Bottom 166-#5 a(E) bars at  $5\frac{1}{2}$ " cts. Top 102-#5 a1(E) bars at 9" cts. Bottom 4-#5 a(E) bars top, each end 166-#6 a2(E) bars at 5½" cts. Top 3 x 3-#5 b(E) bars (Lap with each a(E) bars) Top of Slab 82'-8" End to End Deck <u>PLAN</u> DESIGNED - RYAN P. NEGANGARD TOTAL SHEET NO. Mark Sheffer EXAMINED December 9, 2025 SUPERSTRUCTURE SECTION COUNTY STATE OF ILLINOIS CHECKED - DAVID H. RICHTER 714 MACON 54 26 (128BR-1) B-1

**DEPARTMENT OF TRANSPORTATION** 

**STRUCTURE NO. 058-0137** 

SHEET 11 OF 26 SHEETS

CONTRACT NO. 74368

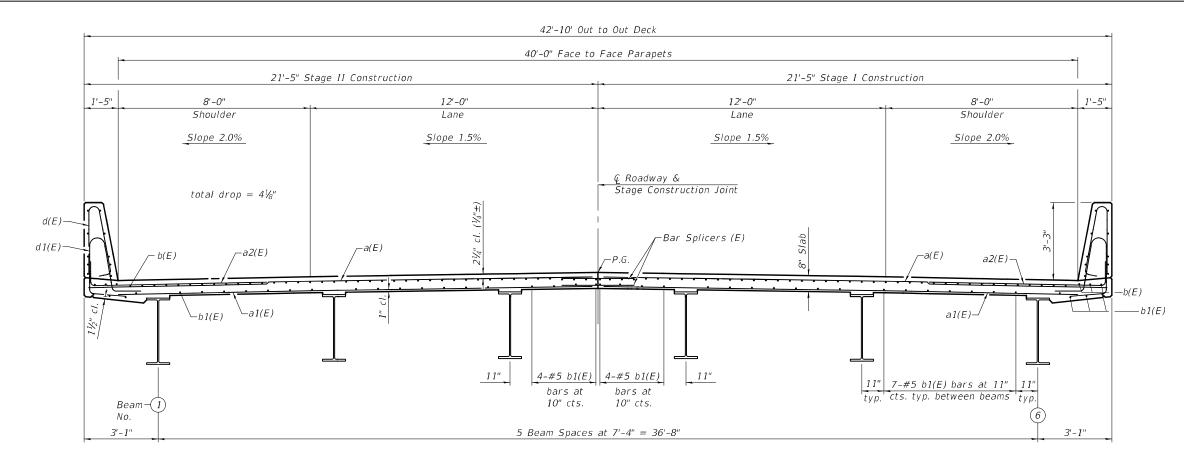
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GLENN W. STOVER

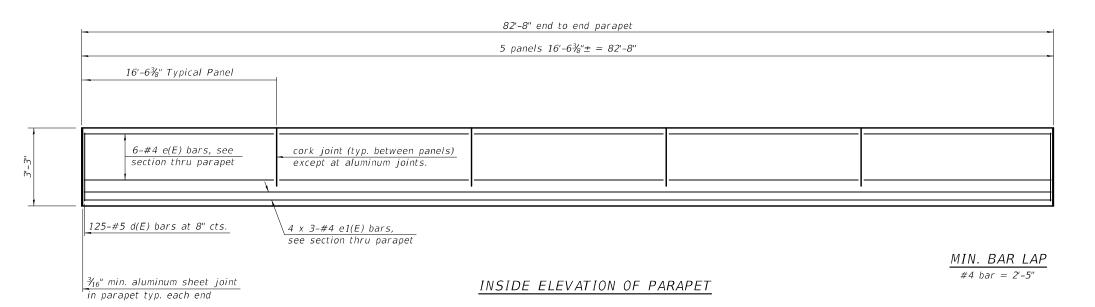
PASSED

REVISED -

REVISED

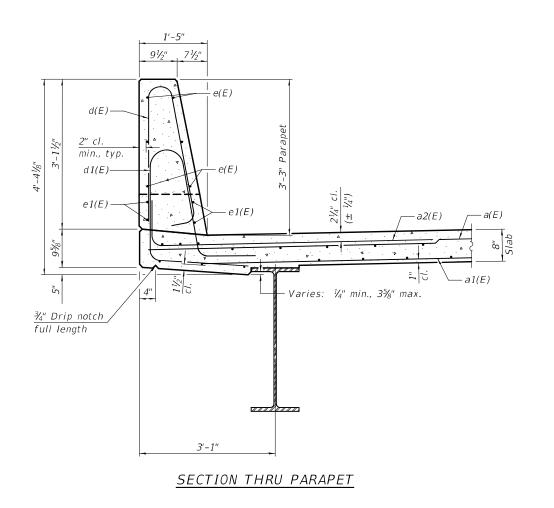


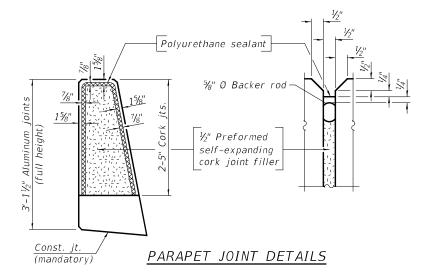
# CROSS SECTION (Looking North)



Notes:
See sheet 13 of 26 for Bar details
and Bill of Material.
Bars indicated thus 4 x 3-#4 etc. indicates 4
lines of bars with 3 lengths per line.

DESIGNED - RYAN P. NEGANGARD	EXAMINED	Mark Shelfon	DATE - December 9, 2025	OTATE OF ULINION	SUPERSTRUCTURE DETAILS	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEET SHEETS NO.
CHECKED - DAVID H. RICHTER  DRAWN - GLENN W. STOVER	PASSED	EVIGINEER OF BRIDGE VESIGN	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 058-0137	714	(128BR-1) B-1	MACON	54 27 INTRACT NO. 74368
CHECKED - R.P.N./G.R.A.	_	ENGINEER OF BRIDGES AND STRUGUU	JRES REVISED -		SHEET 12 OF 26 SHEETS		ILLINOIS FED. A	ID PROJECT	





### SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	348	#5	21'-1"	
a1(E)	204	#5	20'-3"	
a2(E)	332	#6	8'-4"	
b(E)	144	#5	29'-10"	
b1(E)	168	#5	23'-3"	
d(E)	250	#5	6'-5"	Δ
d1(E)	250	#5	8'-3"	_
e(E)	60	#4	16'-3"	
e1(E)	24	#4	29'-1"	
m10(E)	16	#6	21'-1"	
m11(E)	24	#6	7'-0"	
m12(E)	12	#6	2'-9"	
s10(E)	84	#5	7'-1"	П
s11(E)	76	#5	10'-0"	Ü
v100(E)	88	#5	3'-1"	
Reinfor		Bars,	Pound	32,120
Ероху (			. oana	52,120
Concret		Cu. Yds.	151.5	
Superst	ructure	<u> </u>	ca. 105.	151.5

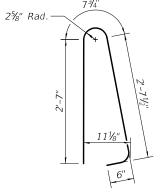
Notes:

The ₹16" min. aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.

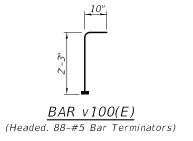
The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall

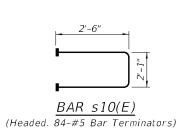
be gray.

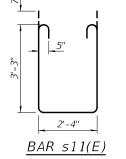
Bar terminators, paid for seperately. See Total Bill of Material.



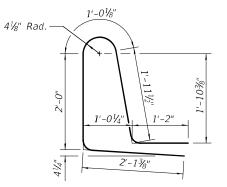
 $BAR \ d(E)$ 











*BAR d1(E)* 

7.					
	DESIGNED	-	RYAN P. NEGANGARD	EXAMINED	
AME	CHECKED	-	DAVID H. RICHTER	_	
E N/	DRAWN	-	GLENN W. STOVER	PASSED	
ш	OUEOVED		D D N /O D A	_	

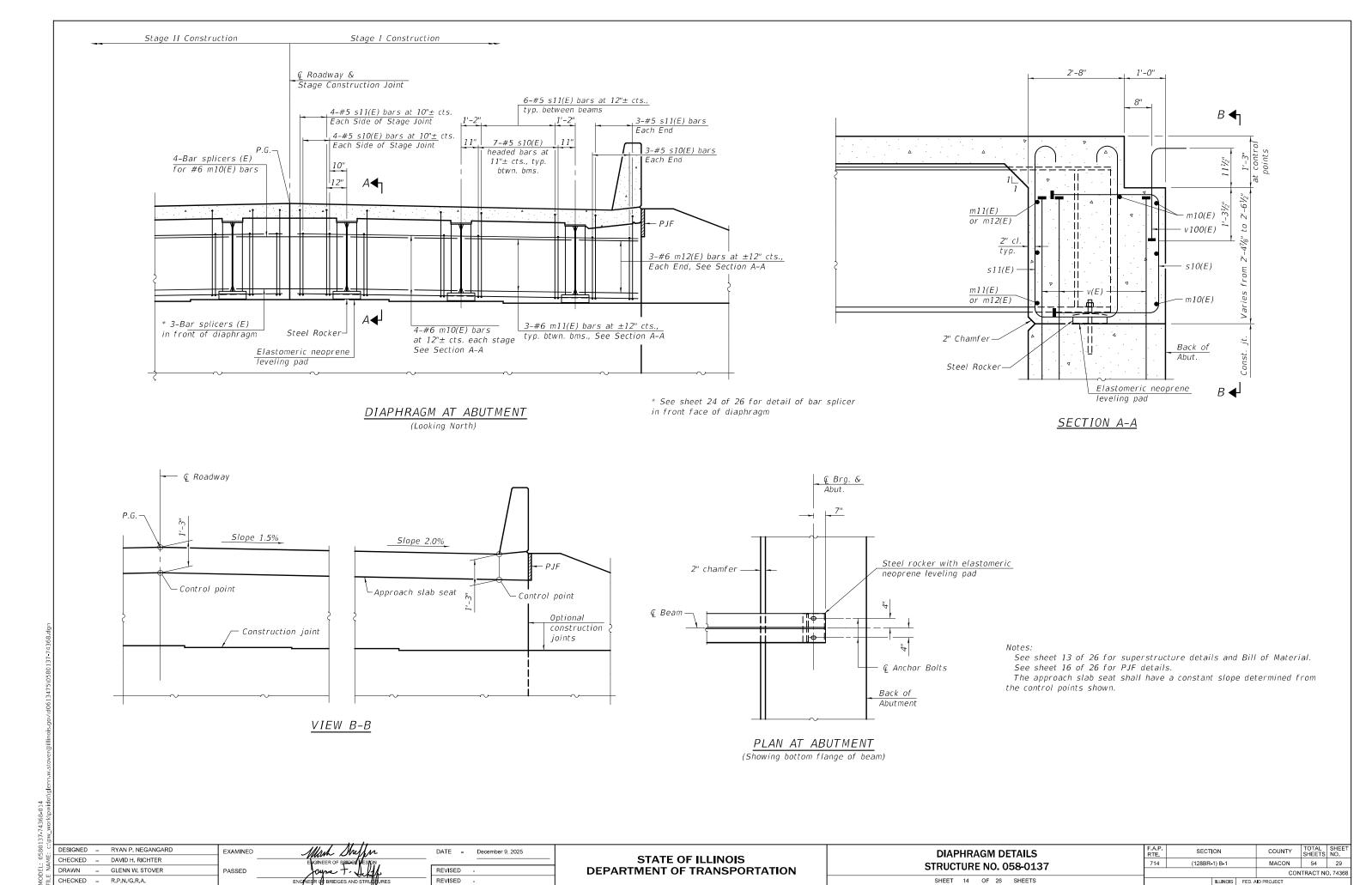
Mark Sheffer	DATE - December 9, 2025
ENGINEER OF BRIDGE PESIGN	
Jayne + All	REVISED -
ENGINEER (1) BRIDGES AND STRUCTURES	REVISED -
•	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

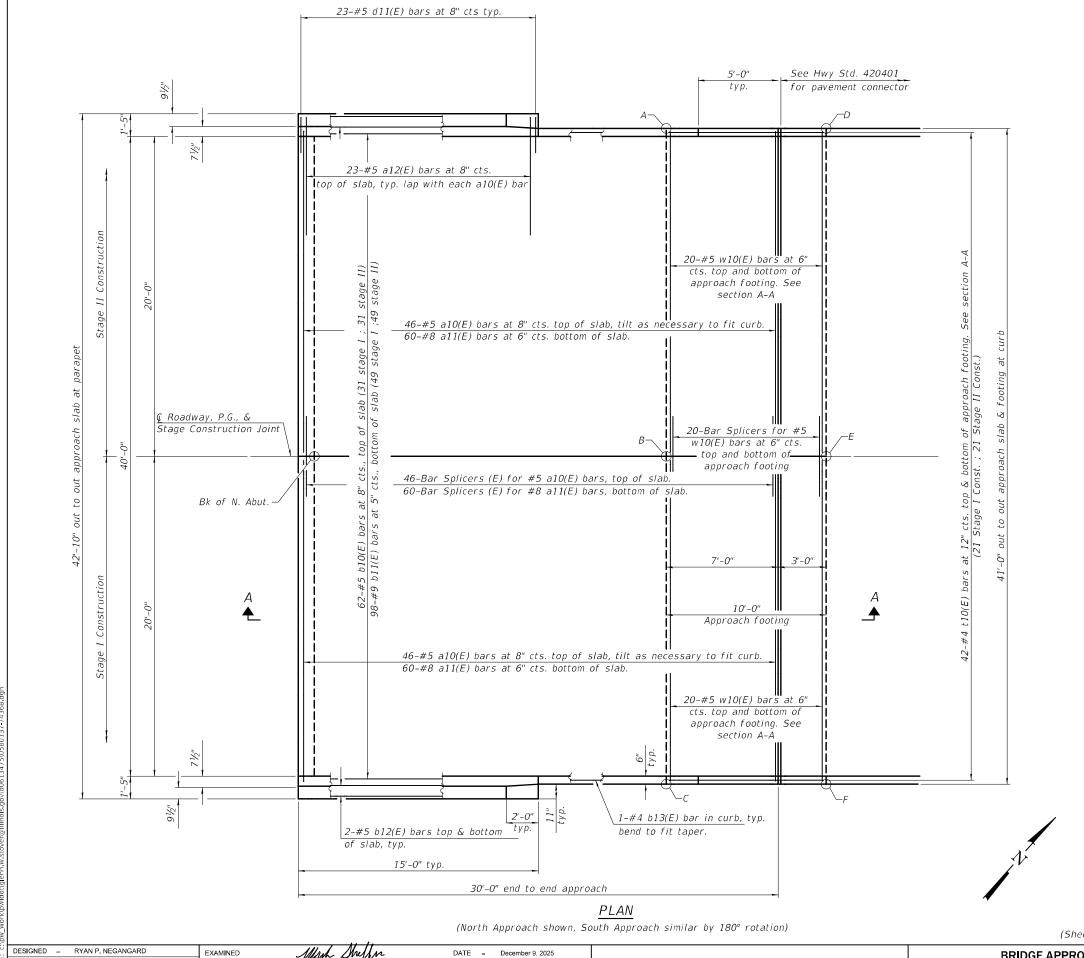
				DETAILS 058-0137	
SHEET	13	OF	26	SHEETS	

F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
714	(128BR-1) B-1	MACON	54	28	
			CON	TRACT N	D. 7436
	ILLINOIS	FED. A	D PROJECT		

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12/10/2025 8:24:20 AM



REVISED

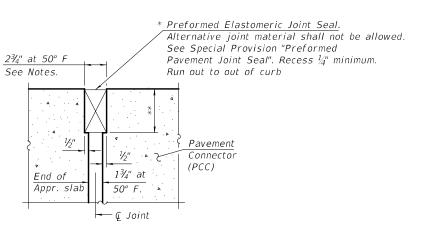
REVISED

STATE OF ILLINOIS

**DEPARTMENT OF TRANSPORTATION** 

# TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

	South Approach		North A	pproach
Point	Тор	Bottom	Тор	Bottom
Α	606.80	605.97	606.79	605.96
В	607.15	606.32	607.14	606.31
С	606.80	605.97	606.79	605.96
D	606.75	605.92	606.74	605.91
Ε	607.10	606.27	607.09	606.26
F	606.75	605.92	606.74	605.91



- \* Cost included with Concrete Superstructure (Approach Slab).
- \*\* Per manufacturer recommendations

DETAIL A

(Sheet 1 of 2)

BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 058-0137

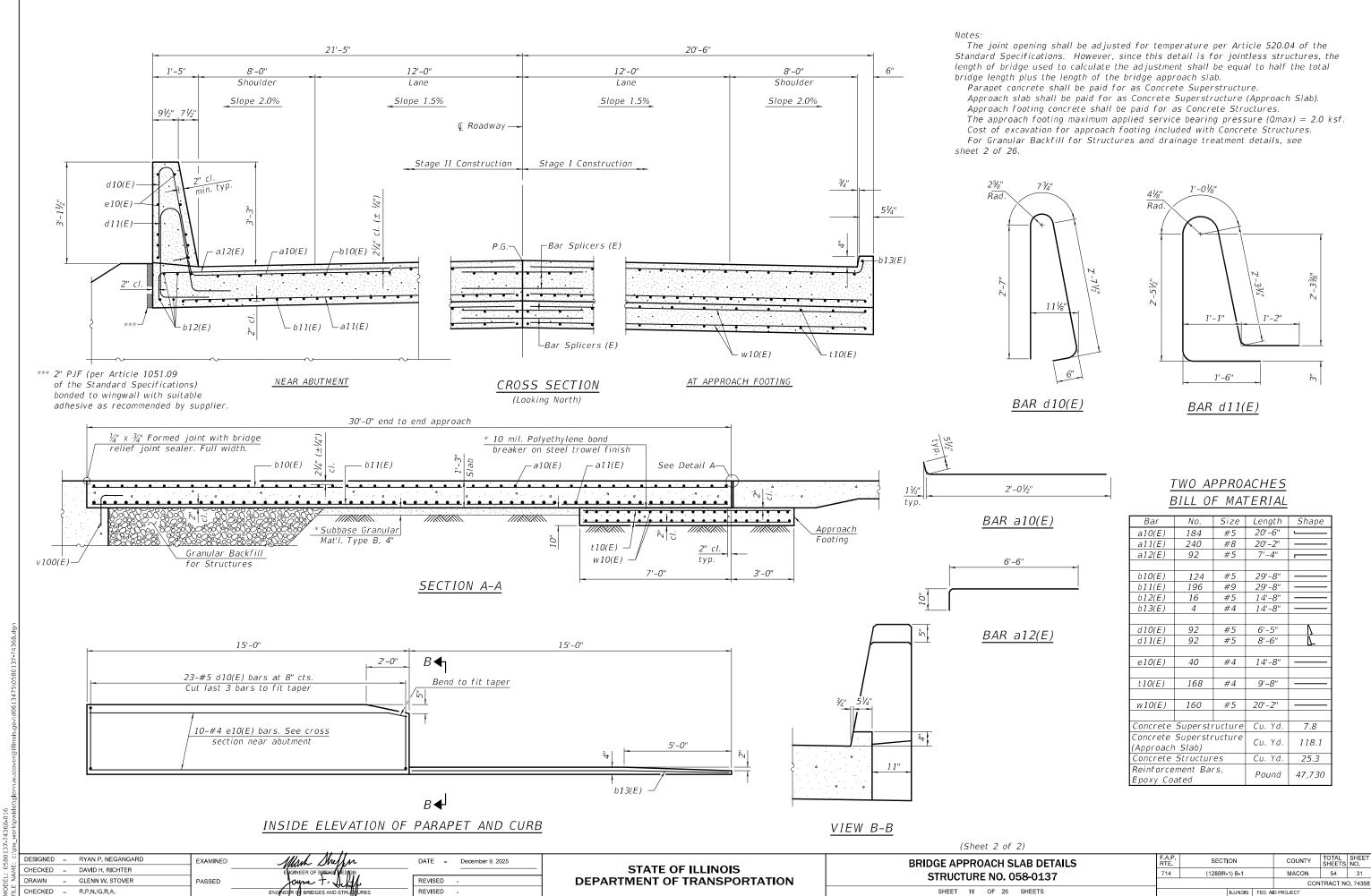
SHEET 15 OF 26 SHEETS

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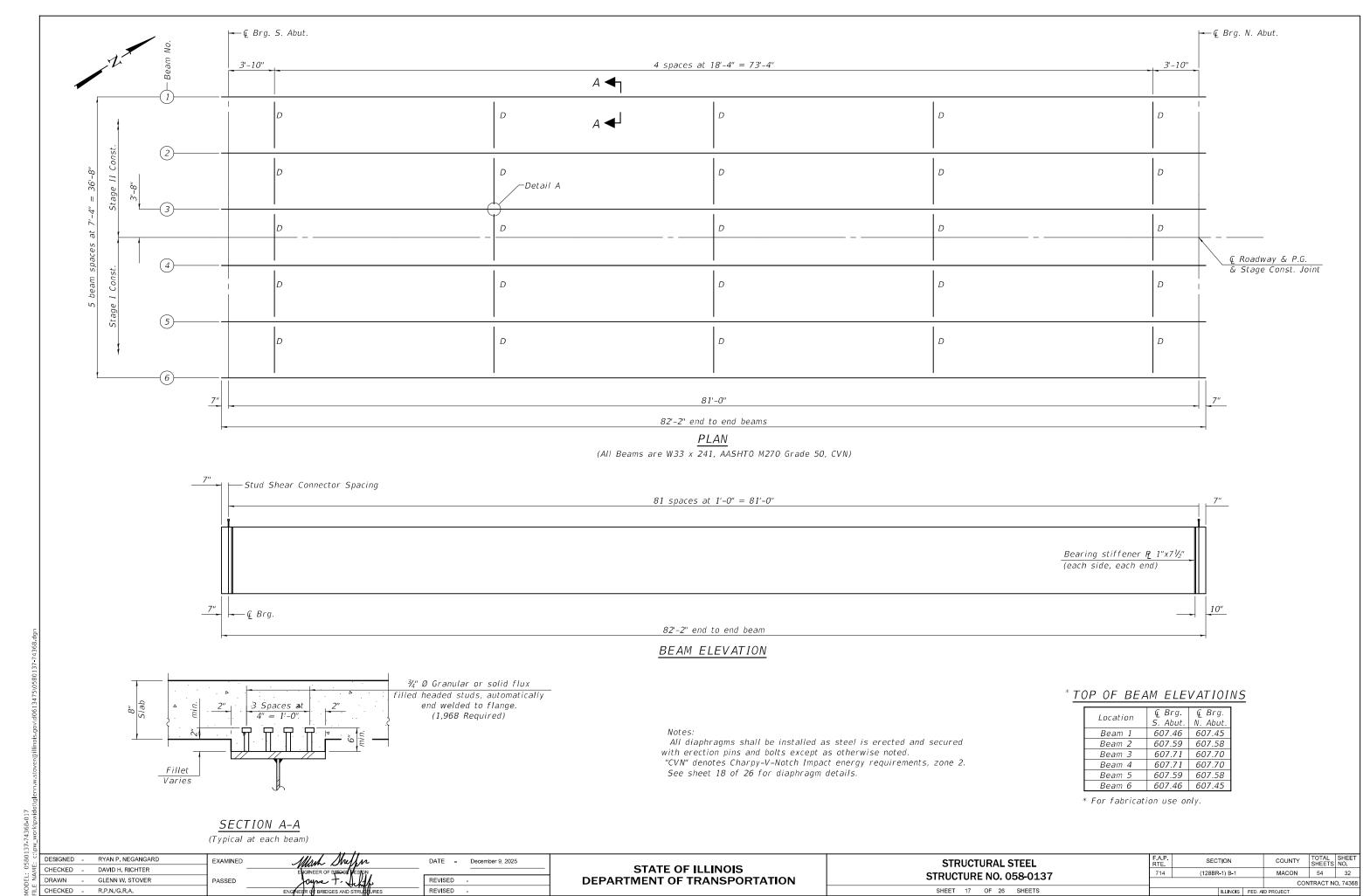
CHECKED - DAVID H. RICHTER

GLENN W. STOVER

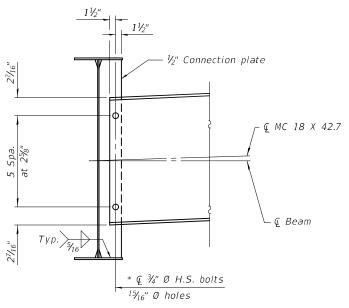
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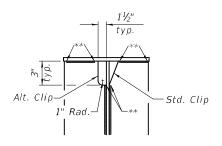


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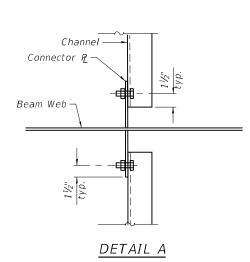
### DIAPHRAGM D

\* Install only one 7/8" Ø H.S. bolt in center most hole above © of beam at each end of the stage line diaphragm. The bolts shall be finger-tightened prior to deck pour to permit rotation. Install ¾" Ø H.S. bolts and fully tighten immediately after stage II deck pour is complete.



### WELD LIMITS & CLIP DETAILS

\*\* Stop welds  $\frac{1}{4}$ "  $(\pm \frac{1}{8}$ ") from edges as shown, typ.



BEAM REACTION TABLE						
		Abut.				
		Interior	Exterior			
LLDF		0.767	0.655			
0CF		-	-			
R DC1	(k)	42.3	39.9			
R DC2	(k)	7.1	7.1			
R DW	(k)	13.5	13.5			
R4	(k)	68.7	58.7			
RIM	(k)	16.2	13.8			
RTOTAL	(k)	147.8	133.0			

#### Notes:

Two hardened washers required for each set of oversized holes.

Alternate channels of equal depth and larger weight are permitted to facilitate material acquisition. Alternate channels, if utilized, shall be provided at no additional cost to the Department.

Anchor bolts at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

All bearing plates and bearing stiffeners shall be AASHTO M270, Grade 50.

#### BILL OF MATERIAL

1	Item	Unit	Total
	Anchor Bolts, 1"	Each	24

fit. tvp.

See Clip Detail

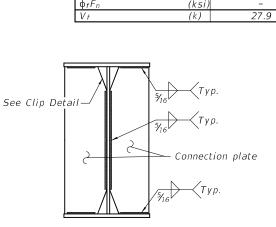
Bearing stiffener

Mill stiffener

to bear, typ.

BEARING STIFFENER

DETAIL



INTERIOR BEAM MOMENT TABLE

(in4)

(in4)

(in4)

(in³)

(in³)

(in³)

(in3)

(k/')

('k)

(k/')

('k)

(k/')

('k)

('k)

('k)

('k)

(ksi)

(ksi,

(ksi)

(ksi,

(ksi)

(ksi.

Ic(3n)

 $I_c(cr)$ 

Sc(n)

Sc(3n)

 $S_c(cr)$ 

DC2

M DC2

DW

Mow

LLDF

 $\phi_f M_n$ 

M4 + IM

fs DC1

fs DC2

fs DW

 $f_s$  ( $\{\pm +IM\}$ )

 $0.95RhF_{vf}$ 

M<sub>u</sub> (Strength I)

fs (Service II)

fs (Total)(Strength I) (ksi)

0.5 Span 1

14200

33824

24721

830

1144

1037

1.044

856.2

0.175

1435

0.333

273.1

0.585

1223.6

3800 6

5520

12.38

1.66

3.16

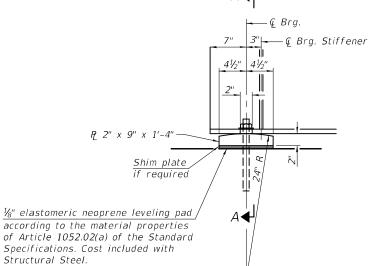
12.83

33.89

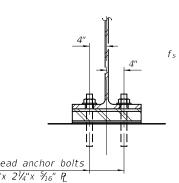
47.5

44.75

CONNECTION PLATE DETAIL



ELEVATION AT ABUTMENT



Ç 1" Ø x 12" all thread anchor bolts (Grade 55) with 21/4"x 21/4"x 5/16" P

washers under nuts.

 $1\frac{3}{6}$ "x 2" slotted holes in flange.

 $1\frac{1}{2}$ " Ø holes in bearing plate.

SECTION A-A

FIXED BEARING

(12 Required)

 $I_s$ ,  $S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s(Total-Strength\ I,\ and$ Service II) due to non-composite dead loads (in.4 and in.3).  $I_{c}(n)$ ,  $S_{c}(n)$ : Composite moment of inertia and section modulus of the steel

and deck based upon the modular ratio, "n", used for computing  $f_s(Total-Strength\ I,\ and\ Service\ II)$  in uncracked sections due to short-term composite live loads (in.4 and in.3).

 $I_c(3n)$ ,  $S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing fs(Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

 $I_c(cr)$ ,  $S_c(cr)$ : Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing fs (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.⁴ and in.³).

DC1: Un-factored non-composite dead load (kips/ft.).

M DC1: Un-factored moment due to non-composite dead load (kip-ft.). Un-factored long-term composite (superimposed excluding future

wearing surface) dead load (kips/ft.). M DC2: Un-factored moment due to long-term composite (superimposed

excluding future wearing surface) dead load (kip-ft.). Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

Live Load Distribution Factor for moment and shear computed according to Article 4.6.2.2 and further IDOT provisions.

M ¼ + 1M: Un-factored live load moment plus dynamic load allowance (impact) (kin-ft.).

Mu (Strength I): Factored design moment (kip-ft.).

 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{\pm} + IM$ 

 $\phi_f M_n$ : Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).

fs DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated

below (ksi). MDC1 / Snc

fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated

below (ksi).  $M_{DC2}$  /  $S_c(3n)$  or  $M_{DC2}$  /  $S_c(cr)$  as applicable.

f<sub>s</sub> DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

 $M_{DW}$  /  $S_c(3n)$  or  $M_{DW}$  /  $S_c(cr)$  as applicable.

 $f_s$  ( $\frac{1}{2} + IM$ ): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as

calculated below (ksi).

 $M_{4+IM}/S_c(n)$  or  $M_{DW}/S_c(cr)$  as applicable. fs (Service II): Sum of stresses as computed below (ksi).

 $f_{s}DC1 + f_{s}DC2 + f_{s}DW + 1.3 f_{s}(4 + IM)$ 

0.95RhFyf: Composite stress capacity for Service II loading according

to Article 6.10.4.2 (ksi).

 $f_{\text{S}}$  (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).

 $1.25 (f_S DC1 + f_S DC2) + 1.5 f_S DW + 1.75 f_S (4 + IM)$ 

 $\phi_f F_n$ : Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

Maximum factored shear range in span computed according to Article 6.10.10.

Obtuse Correction Factor applied to non-continuous exterior beam

ends and computed according to Article 4.6.2.2.3c-1 or as further simplified by IDOT provisions.

R<sub>DC1</sub>: Un-factored reaction due to non-composite dead load (kip).

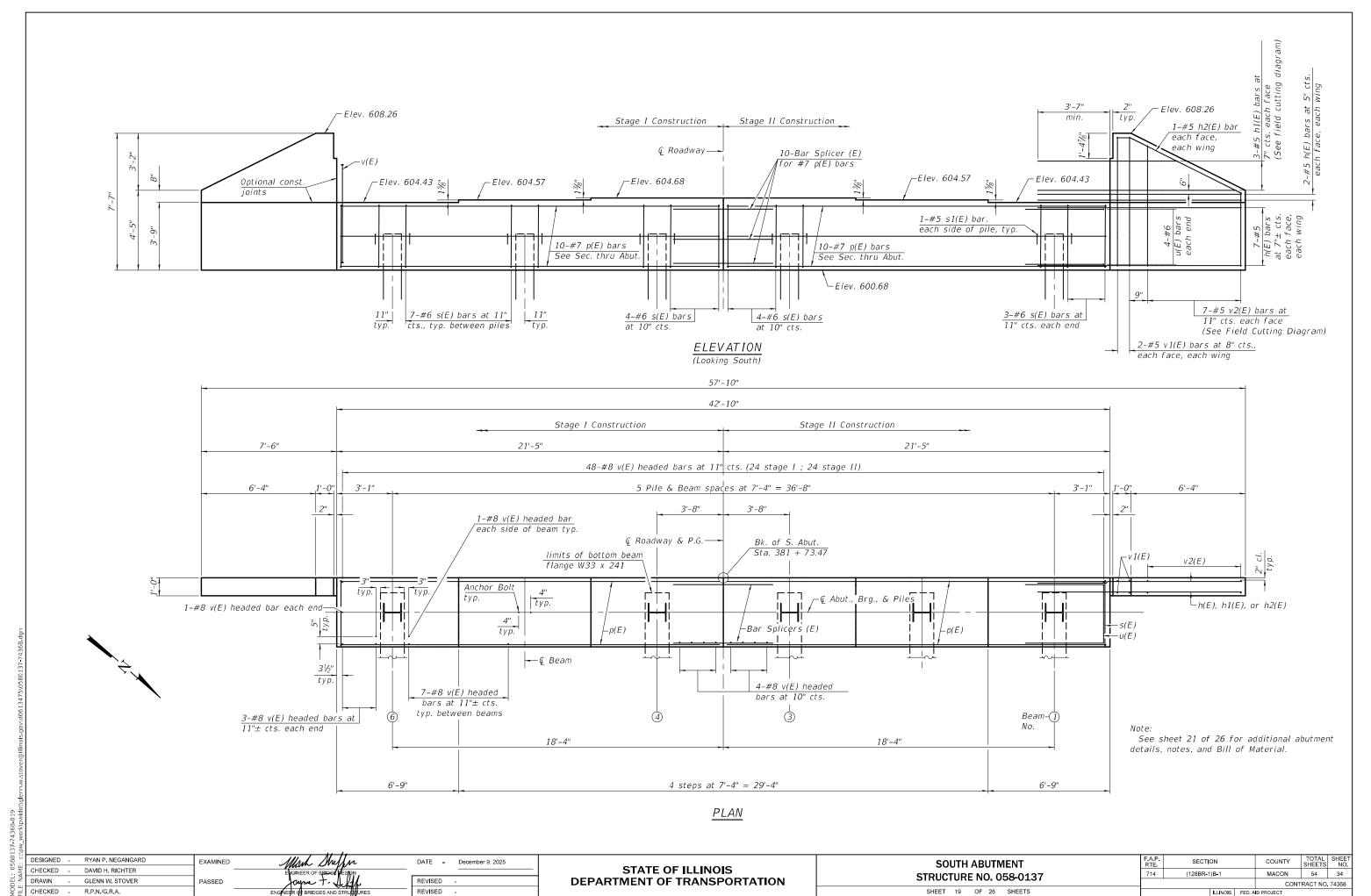
R<sub>DC2</sub>: Un-factored reaction due to long-term composite (superimposed excluding future wearing surface) dead load (kip).

Un-factored reaction due to long-term composite (superimposed future wearing surface only) dead load (kip).

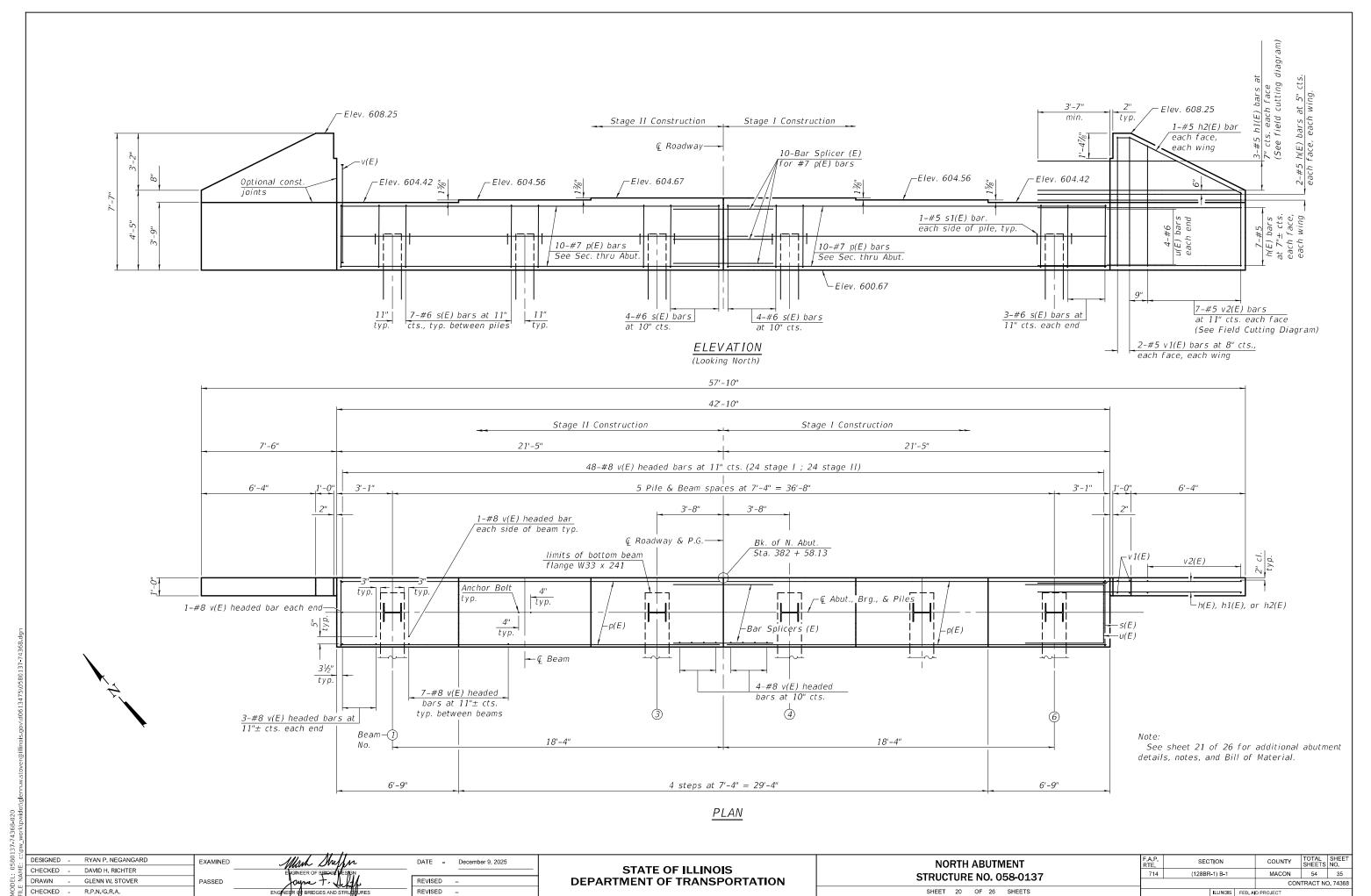
Un-factored live load reaction (kip).

Un-factored dynamic load allowance (impact) (kip).

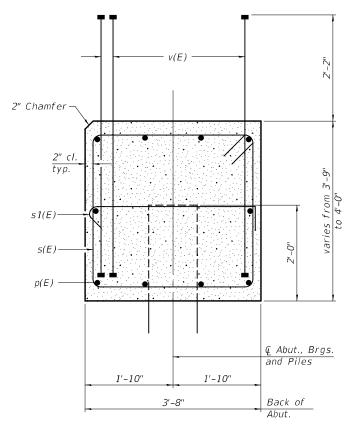
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580 :: c:	DESIGNED - RYAN P. NEGANGARD EXAMINED	Mark Sheffer	DATE - December 9, 2025		STRUCTURAL STEEL DETAILS	F.A.P. SF	ECTION	COUNTY	TOTAL SI	EET ).
A	CHECKED - DAVID H. RICHTER -	ENGINEER OF BRIDGI VESIGN		STATE OF ILLINOIS	STRUCTURE NO. 058-0137	714 (12	3BR-1)B-1	MACON	54	33
	DRAWN - GLENN W. STOVER PASSED	Jayne +. Jehth	REVISED -	DEPARTMENT OF TRANSPORTATION	31R0C10RE NO. 038-0131	1		C	ONTRACT NO. 7	4368
₽∃I	CHECKED - R.P.N./G.R.A.	ENGINEER OF BRIDGES AND STRUCTURES	REVISED -		SHEET 18 OF 26 SHEETS		ILLINOIS	FED. AID PROJECT		



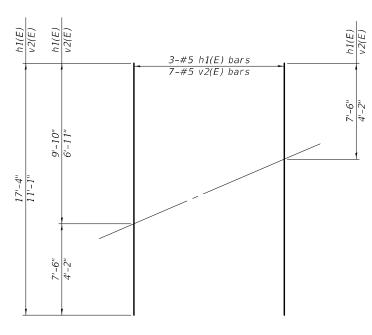
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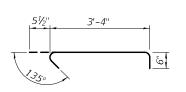
# SECTION THRU *ABUTMENT*



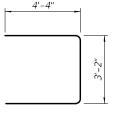
### FIELD CUTTING DIAGRAM

Order h1(E) and v2(E) full length. Cut as shown and use remainder of bars in opposite wing.

Pour steps monolithically with cap. Bar terminators, paid for seperately. See Total Bill of Material. For details of piles see sheet 22 of 26.



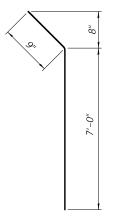
BAR s1(E)



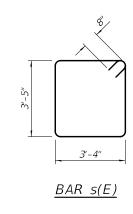
BAR u(E)



(Headed. 416-#8 Bar terminators)



 $BAR \ h2(E)$ 



# SOUTH ABUTMENT BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	36	#5	11'-0"	
h1(E)	6	#5	17'-4"	
h2(E)	4	#5	7'-9"	
p(E)	20	#7	21'-1"	
s(E)	42	#6	14'-10"	<u> </u>
s1(E)	12	#5	4'-4"	Ţ
u(E)	8	#6	11'-10"	
v(E)	104	#8	5'-7"	<b>.</b>
v1(E)	8	#5	7'-3"	
v2(E)	14	#5	11'-1"	
Structu	ire Exc	avation	Cu. Yd.	119
Concre	te Stru	ctures	Cu. Yd.	26.0
Reinfo	rcement	: Bars,	Pound	4,320
	Coated	r oana	4,520	
	hing St HP12x6.	Foot	324	
Driving		Foot	324	
ווועווע	FILES		1 001	J24

# SOUTH ABUTMENT PILE DATA

Type: HP12x63 Nominal Required Bearing: 497 kips Factored Resistance Available: 273 kips Est. Length: 54'-0" No. Production Piles: 6 No. Test Piles: 0

### NORTH ABUTMENT BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	36	#5	11'-0"	
h1(E)	6	#5	17'-4"	
h2(E)	4	#5	7'-9"	$\Big $
p(E)	20	#7	21'-1"	
c/E)	17	4.6	1 /1 1 011	га
s(E)	42	#6	14'-10"	
s1(E)	12	#5	4'-4"	
u(E)	8	#6	11'-10"	
4(2)		,, 0		_
v(E)	104	#8	5'-7"	1
v1(E)	8	#5	7'-3"	
v2(E)	14	#5	11'-1"	
Structi	ire Exc	avation	Cu. Yd.	97
Concre	te Stru	ctures	Cu. Yd.	26.0
	rcement	Bars,	Pound	4,320
	Coated		1 oana	4,320
	hing St		Foot	280
	HP12x6.	3		
	g Piles		Foot	280
Test P HP12x	ile Stee 63	2/	Each	1

# NORTH ABUTMENT PILE DATA

Type: HP12x63

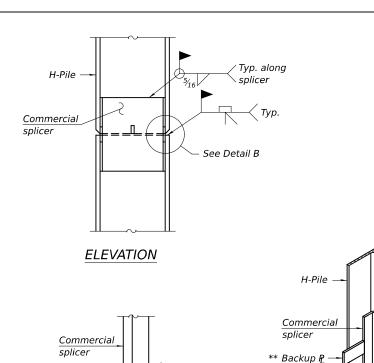
Nominal Required Bearing: 497 kips Factored Resistance Available: 273 kips Est. Length: 56'-0" No. Production Piles: 5 No. Test Piles: 1

의				
NAME: c:\p	DESIGNED	-	RYAN P. NEGANGARD	
	CHECKED	-	DAVID H. RICHTER	
_	DRAWN	-	GLENN W. STOVER	
Щ	CHECKED		R P N /G R A	

F.A.P. RTE	SECTION	NC		COUNTY	TOTAL SHEETS	SHEET NO.		
714	(128BR-1	)B-1		MACON	54	36		
			CONTRACT NO. 74368					

#### STEEL PILE TABLE

Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 18x181	18	18	1	36"
x157	173/4"	17 <sup>7</sup> /8"	7/8"	36"
x135	17½"	17 <sup>3</sup> /4"	3/4"	36"
HP 16x183	16½"	16½"	11/8"	36"
x162	161/4"	16½"	1"	36"
x141	16	16	7/8"	36"
x121	15 <sup>3</sup> / <sub>4</sub> "	15 <sup>7</sup> /8"	3/4"	36"
HP 14x117	141/4"	14 <sup>7</sup> / <sub>8</sub> "	<sup>13</sup> / <sub>16</sub> "	30"
x102	14"	14 <sup>3</sup> / <sub>4</sub> "	<sup>11</sup> ⁄ <sub>16</sub> "	30"
x89	137/8"	143/4"	5/8"	30"
x73	135/8"	145/8"	1/2"	30"
HP 12x84	12 <sup>1</sup> / <sub>4</sub> "	12 <sup>1</sup> / <sub>4</sub> "	<sup>11</sup> ⁄ <sub>16</sub> "	24"
x74	12½"	12½"	5/8"	24"
x63	12"	12½"	1/2"	24"
x53	11¾"	12"	7/ <sub>16</sub> "	24"
HP 10x57	10"	10 <sup>1</sup> / <sub>4</sub> "	%16"	24"
x42	93/4"	10½"	7/ <sub>16</sub> "	24"
HP 8x36	8"	8½"	7⁄ <sub>16</sub> "	18"

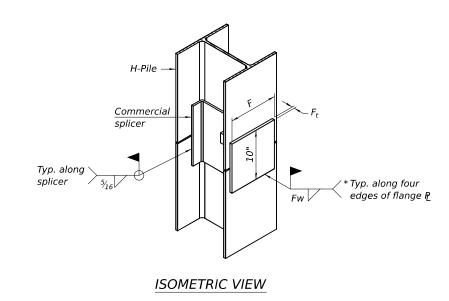


Backup P

DETAIL B

# WELDED COMMERCIAL SPLICE

ISOMETRIC VIEW



# - H-pile See Detail A Pile shoe **ELEVATION** H-pile

#### SHOE ATTACHMENT

DETAIL A

Note:

Typ. shop or

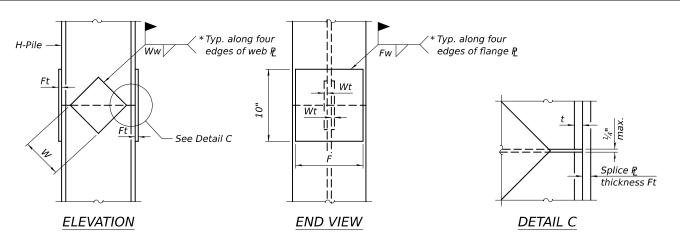
field weld

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The steel H-piles shall be according to AASHTO M270 Grade 50.

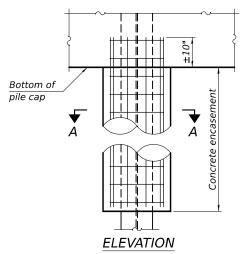
### WELDED COMMERCIAL SPLICE ALTERNATE

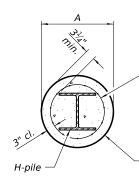
- \* Interrupt welds  $\frac{1}{4}$ " from end of web and/or each flange.
- \*\* Remove portions of backup P's that extend outside the flanges.
- \*\*\* Weld size per pile shoe manufacturer (5/16" min.).



Designation	F	Ft	Fw	w	Wt	Ww
HP 18x181	15½"	1½"	1"	9½"	7/8"	3/4"
x157	15 <sup>1</sup> ⁄ <sub>4</sub> "	11/4"	1"	9½"	7/8"	3/4"
x135	15½"	11/4"	1"	9½"	7/8"	3/4"
HP 16x183	13 <sup>3</sup> / <sub>4</sub> "	1½"	1"	81/4"	7/8"	3/4"
x162	13½"	1½"	1"	81/4"	3/4"	5/8"
×141	13½"	11/4"	7/8"	81/4"	3/4"	5/8"
x121	13½"	11/4"	7/8"	81/4"	3/4"	5/8"
HP 14x117	12½"	11/4"	7/8"	73/4"	5/8"	1/2"
x102	12½"	1"	3/4"	73/4"	5/8"	1/2"
x89	12½"	7/8"	11/16"	73/4"	5/8"	1/2"
x73	12½"	3/4"	9/16"	73/4"	5/8"	1/2"
HP 12x84	10"	1"	11/16"	6½"	5/8"	1/2"
x74	10"	7/8"	11/16"	6½"	5/8"	1/2"
x63	10"	3/4"	1/2"	6½"	1/2"	3/8"
x53	10"	3/4"	1/2"	6½"	1/2"	3/8"
HP 10x57	8"	7/8"	9/16"	51/4"	1/2"	3/8"
x42	8"	3/4"	%16"	51/4"	1/2"	3/8"
HP 8x36	63/4"	5/8"	7/16"	4"	1/2"	3/8"

#### WELDED PLATE FIELD SPLICE





Welded wire fabric 6 x 6- W4.0 x W4.0 weighing 58#/100 sq. ft. Bend as required to fit into wall. Reinforcement for encasements at abutments is included with Concrete Encasement according to Article 503.13 of the Standard Specifications.

Forms for encasement may be omitted when soil conditions permit.

SECTION A-A

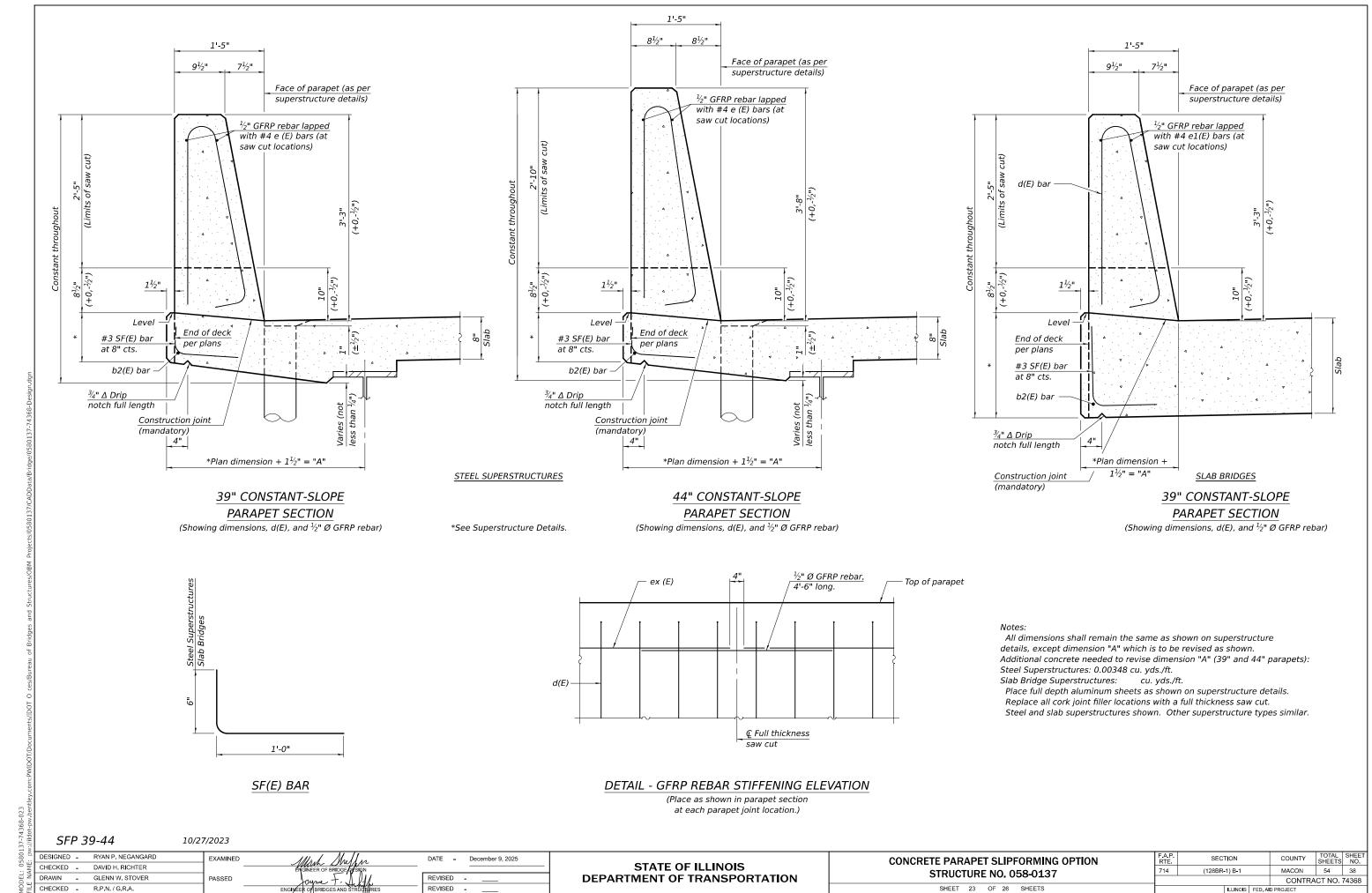
(when specified)

INDIVIDUAL PILE CONCRETE ENCASEMENT

F-HP 4-4-2025 DESIGNED - RYAN P. NEGANGARD EXAMINED CHECKED -DAVID H. RICHTER GLENN W. STOVER REVISED -CHECKED - R.P.N. / G.R.A. REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

SECTION **HP PILE DETAILS** MACON 54 37 (128BR-1)B-1 **STRUCTURE NO. 058-0137** CONTRACT NO. 74368 SHEET 22 OF 26 SHEETS



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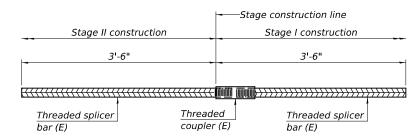
#### STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

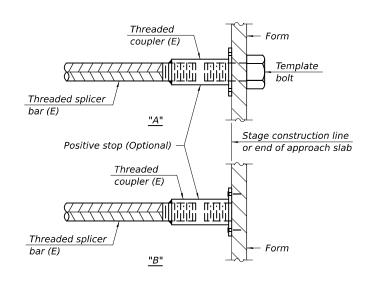
Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

	Bar	No. assemblies	Minimum
Location	size	required	lap length
Slab	#5	276	3'-6"
Approach Slab Top	#5	92	3'-4"
Approach Slab Bottom	#8	120	4'-9"
Approach Slab Footing	#5	80	3'-2"
Abutment Diaphragm, Back Face	#6	8	4'-0"
Abutment Diaphragm, Front Face	#6	6	See Diaphragm Bar Splicer Detail
Abutment Cap	#7	20	5'-0"



#### DIAPHRAGM BAR SPLICER DETAIL

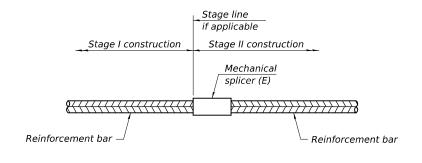


#### **INSTALLATION AND SETTING METHODS**

"A": Set bar splicer assembly by means of a template bolt.

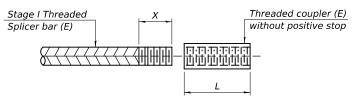
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.



#### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



#### THREADING OF ASSEMBLIES

The threaded length "X" shall be no more than L/2. The bar should be tightened until 0-1 thread(s) is/are exposed.

#### Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements

for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

DESIGNED - RYAN P. NEGANGARD

CHECKED - DAVID H. RICHTER

DRAWN - GLENN W. STOVER

CHECKED - R.P.N. / G.R.A.

ENGINEER OF BRIDGE AND STROUGH PRES

REVISED - REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
STRUCTURE NO. 058-0137

SHEET 24 OF 26 SHEETS

ROUTE FAP 714	DESCRI	PTION			Abutments	ьс	GGE	) BYE	. Sand	ds
SECTION (12BR-1)B-1	ι	OCAT	ION _	Mosqu	uito Creek, SEC. 14, TWP. 15N, R	NG. 1E, 31	rd PM	,		
COUNTY Macon DRILLI	NG ME	THOD	Holl		ide N 39.723321, Longitude W em auger & split spoon HAMME			Auto	140#	
STRUCT. NO. 058-0129 (E) Station 058-0137 (P) Station 058-0137 (P) Station 058-0137 (P) Station 058-0137 (P) Station 058-0138 (E) 382+15.8 (F) Station 076-108 (F) 382+77.8 (F) 382+77.8 (F) 584-77 (F	P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev.   591.4     Stream Bed Elev.   591.0     Groundwater Elev.:   First Encounter   581.     Upon Completion   589.     After 24 Hrs.   592.	2 ft 8 ft.▼ 3 ft.▽	D E P T H	B L O W S	U C S Qu (tsf)	
9" Asphalt Pavement. 606.	.	0 - 7	(,	(,-,	Medium, moist, grey, SILTY CLA			1	0.6	ł
9" Concrete Pavement. 605.  Moist, dark grey, CLAY with Silt.					LOAM. (continued)	584.77	#	0	В	ŀ
	_				Medium, moist, grey, SANDY LOAM with Silt.		#	1 1	0.5 B	r
	_				Soft, wet.		#			İ
Stiff, with organics.	_5	3 3 4	1.3 B	31		580.87	<b>₹</b> 25	1 4 8	0 <b>.</b> 3	-
		3	1.2	29	Grey, GRAVELLY SAND.  Very stiff, moist, grey, SANDY CLAY LOAM.	579.77 579.27 578.77		10 10	2.9	
597.	27 -10	2	В		SANDY LOAM with gravel SANDY CLAY LOAM TILL		-30	20	В	ŀ
Medium, moist, dark grey, SILTY CLAY.	-10	2 3	1.0 B	28	Hard, moist, grey.		Ŧ	31 50 5-3/4	6.9 S	ŀ
593.	67	1 2 3	2,1	28	Very dense, augers locked in pl	ace.				
Very stiff, moist, dark grey, CLAY.  592.  Medium, moist, dark grey, SILTY		1	В				-35	42		
CLAY.		2 2	0.7 B	28			_5	50 1/4 50	6.9 S	
589. Medium, moist, grey, SILTY CLAY LOAM.	77 ¯ ∇ _	1 2 2	0.9 B	25				I-1/2		
	-20	0					-40	50		

Division of Highways   Date   8/30/2	(P)	Illinois Depa of Transport	rtment ation	S	OIL BORING I	.OG	Page <u>2</u> c	
SECTION		Division of Highways					Date _8/30	ე/
COUNTY   Macon   DRILLING METHOD   Hollow stem auger & split spoon   HAMMER TYPE   Auto 140#	ROUTE	FAP 714 D	escription _		Abutments	LOGG	ED BYE. Sand	SC
COUNTY   Macon   DRILLING METHOD   Hollow stem auger & split spoon   HAMMER TYPE   Auto 140#	SECTION _	(12BR-1)B-1	LOCATION	Mosqu Latitu	ito Creek, SEC. 14, TWP. 15N, RN	NG. 1E, 3rd F	M,	_
STRUCT. NO. 058-0137 (P) Station 382+20.8 (E) 382+15.8 (P) BORING NO.1 North Abutment (North) Station 382+77.8 Offset 10.0 ft East Ground Surface Elev. 606.77 ft (ft) (/6") (tsf) (%)    5-5/8" NT 5   50     Extent of exploration.   5-5/8" NS 15.3 82+46, 18.6" RT. End of Boring   5-50     5-50   5-50   5-50     5-50   5-50   5-50     5-50   5-50   5-50     5-50   5-50   5-50     5-50   5-50     5-50   5-50     5-50   5-50     5-50   5-50     5-50   5-50     5-50   5-50     5-50   5-50     5-50   5-50     5-50	COUNTY _	Macon DRILLIN	IG METHOD _				Auto 140#	
565.77 50  Extent of exploration. 750  Benchmark: BM 100 brass disk in NE Headwall of Str. No. 058-0029, Sta. 382+46, 18.6' RT. End of Boring 45  -50  -50  -50	Station3  BORING NO.  Station  Offset	058-0137 (P) 082+20.8 (E) 382+15.8 (P) 1 North Abutment (North) 382+77.8 10.0 ft East	E L P O T W H S C	C O S I S SQu T	Stream Bed Elev.   591.03	2 ft 3 ft. <u>▼</u> 3 ft. <u>⊽</u>		
Extent of exploration. 4-3/16" 50  Benchmark: BM 100 brass disk in NE Headwall of Str. No. 058-0029, Sta. 382+46, 18.6' RT. End of Boring 4-45		565	→ ′ I	NT 5				
Benchmark: BM 100 brass disk in NE Headwall of Str. No. 058-0029, Sta. 382+46, 18.6' RT. End of Boring  45	Extent of exp		4-3/16"					
	NE Headwall Sta. 382+46	of Str. No. 058-0029, , 18 6' RT	45 					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)

MUDEL: USBU137-743bB-U25 FILE NAME: c:\pw\_work\pwidot\glenn.w.stover@

DESIGNED - RYAN P. NEGANGARD
CHECKED - DAVID H. RICHTER
DATE - December 9, 2025

EXAMINED
WITH MATERIAL PASSED

EXAMINED
WITH MATERIAL PASSED

EXAMINED

WITH MATERIAL PASSED

EXAMINED

EXAMINED

WITH MATERIAL PASSED

EXAMINED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

 SOIL BORING LOGS
 FAP. RTE.
 SECTION
 COUNTY SHEETS NO.
 TOTAL SHEETS NO.

 STRUCTURE NO. 058-0137
 714
 (128BR-1) B-1
 MACON
 54
 40

 SHEET
 25
 0F
 26
 SHEETS
 ILLINOIS FED. AID PROJECT

ROUTE FAP 714	ESCRI	PTION			Abutments	LOGG	ED BY	. San	ds
SECTION (12BR-1)B-1	ι	.OCAT	ION _	Mosq	ulto Creek, SEC. 14, TWP. 15N, RNG. Ide N 39.723206, Longitude W 89.	. 1E, 3rd F	PM,		
COUNTY Macon DRILLI					em auger & split spoon_ HAMMER 1			140#	ŀ
STRUCT. NO. 058-0129 (E) Station 058-0137 (P) Station 058-0137 (P) Station 058-0137 (P) Station 058-120.8 (E) 382+15.8 (P) BORING NO.2 South Abutment (South) Station 058-120.9 (E) 381+74.8 Offset 8.0 ft west Ground Surface Elev. 606.78	T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev.   591.40	ft E P T ft.▼ H ft.▽	L O W S	U C S Qu (tsf)	
9" Asphalt Pavement. 606.	•   • /	00,	(10.7)	(/0)	Stiff, moist, grey, SILTY CLAY	. IL <u>¥</u>   (**	1	1.2	t
6" Concrete Pavement. 605. Brown SILTY CLAY					LOAM (continued)  Soft, moist, grey, SANDY LOAM	584.78	0	В	ŀ
	_				Sort, moist, grey, SANDT LOAM	_	1 1	0.3 B	t
	_	2						В	l
Medium,moist dark grey.	<u>-5</u>	3 2	0.6 B	27	Very soft	-25	5 5 8 14	0.2 S	l
Stiff.		1 1 2	1.4 B	32	57 Very stiff, moist, grey, SANDY CLAY LOAM TILL	79.78 ▼ - -	12 10 13	3.9 S	
With organics.	<u>-10</u>	1 2 2	1.2 S	35	Hard, sample reassembled for testing.	<u>-30</u>	29 50 5 15/1	11.5 6" S	l
		1 2 4	1.4 B	25		-	50 <b>4-1</b> /8		
Soft. Grey.	<u>⊽</u> -15	1 2 3	1.6 B	28	Poor sample condition. Sample reassembled for testing.	-35	50 5-5/8	5.7 S	
		1 1 2	0.4 B	33		-	50 4-3/8		
587.	28 -20	1			Sample reassembled for testing.	-4 <u>-</u>	50		

	Division of Highways IDOT					Date _8/30
ROUTE		DESCRIPTION				GED BYE. Sands
SECTION	(12BR-1)B-1	LOCATI	ON Mo: Lat	squito Creek, SEC. 14, TWP. 1 titude N 39 723206, Longitu	L5N, RNG. 1E, 3rd I de W 89.098092	2Μ,
COUNTY		ING METHOD	Hollow	stem auger & split spoon H	AMMER TYPE	Auto 140#
	058-0029 (E) 058-0137 (P) 32+20.8 (E) 382+15.8 ( South Abutment (South	)   P   O   T   W	U N C C S I	Stream Bed Elev.	591.40 ft 591.02 ft	
Station	8.0 ft west	H S	Qu T	Upon Completion	579.8 ft.▼ 589.3 ft.▽	
Ground Surf	ace Elev. <u>606.78</u>	ft (ft) (/6")	(tsf) (%		<u>592.3</u> ft∑	
Extent of expl		5.78 50 4-1/4	S	_		
Benchmark: NE Headwall Sta. 382+46, End of Boring	BM 100 brass disk in of Str. No. 058-0029, 18.6' RT.	-1/2' -1/2' -45				
		-50    				
		<u>-55</u>				
		-60				

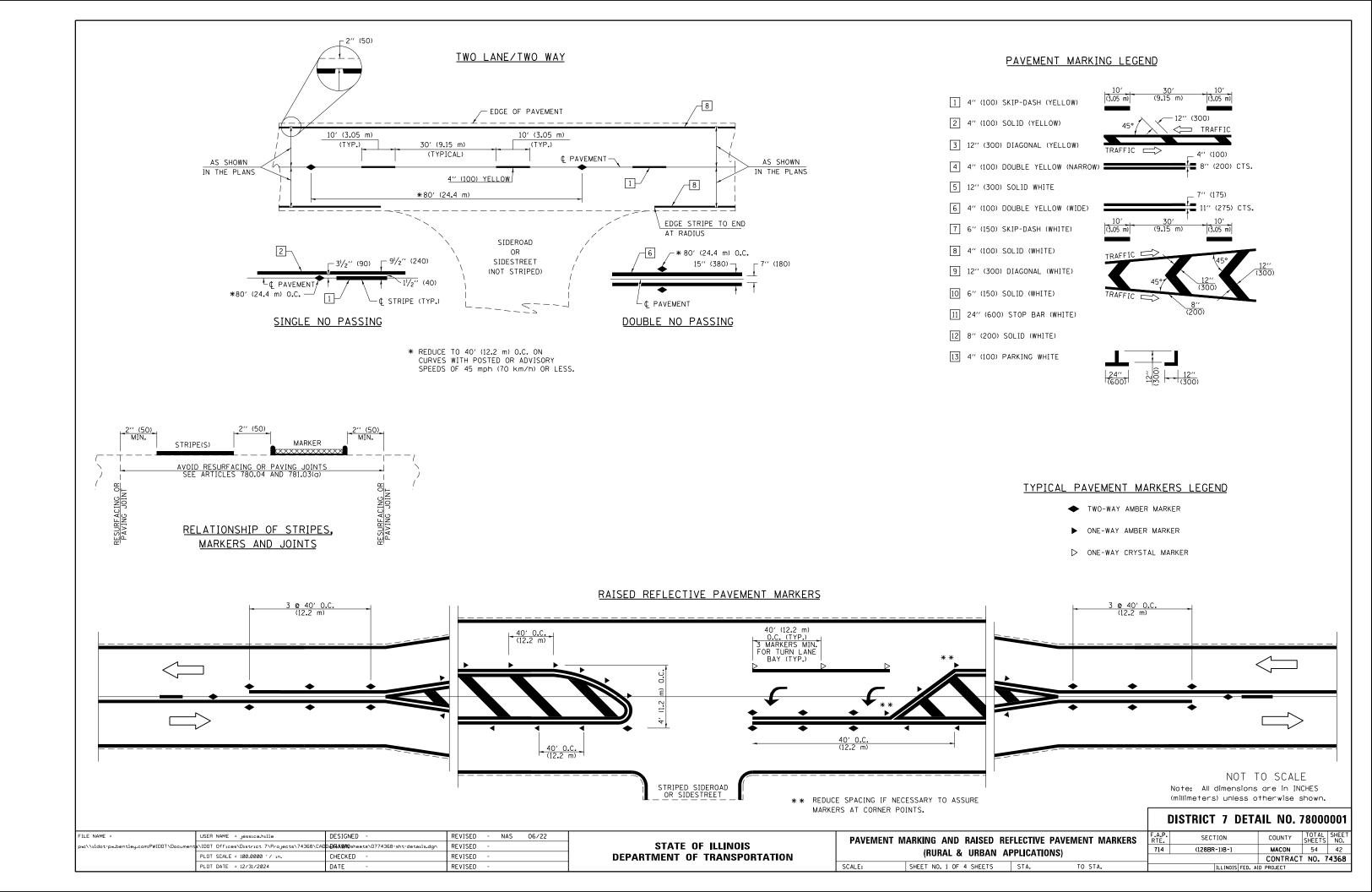
DESIGNED - RYAN P. NEGANGARD EXAMINED DATE - December 9, 2025 CHECKED - DAVID H. RICHTER REVISED -DRAWN - GLENN W. STOVER PASSED S H CHECKED - R.P.N./G.R.A. REVISED -

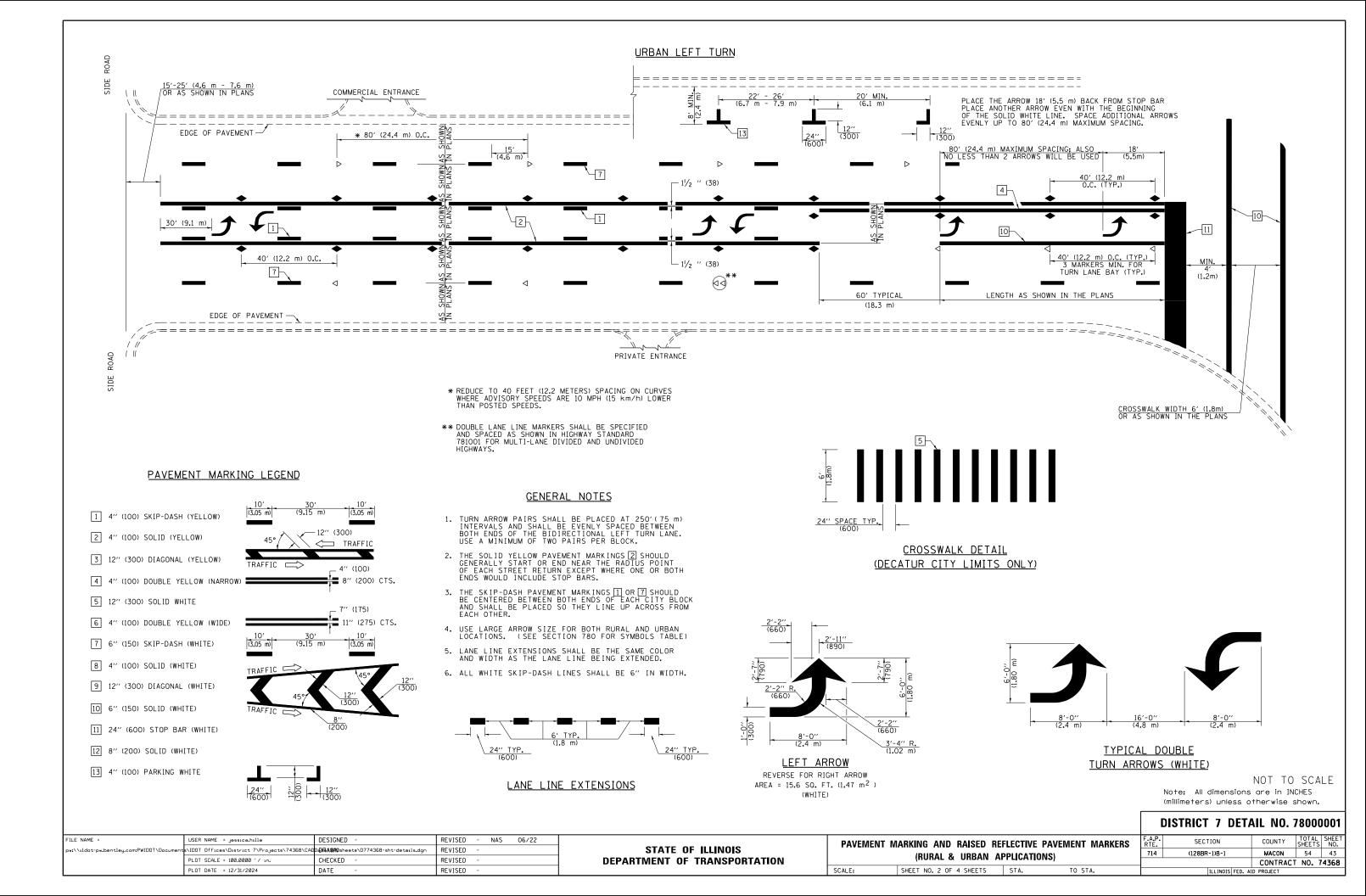
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

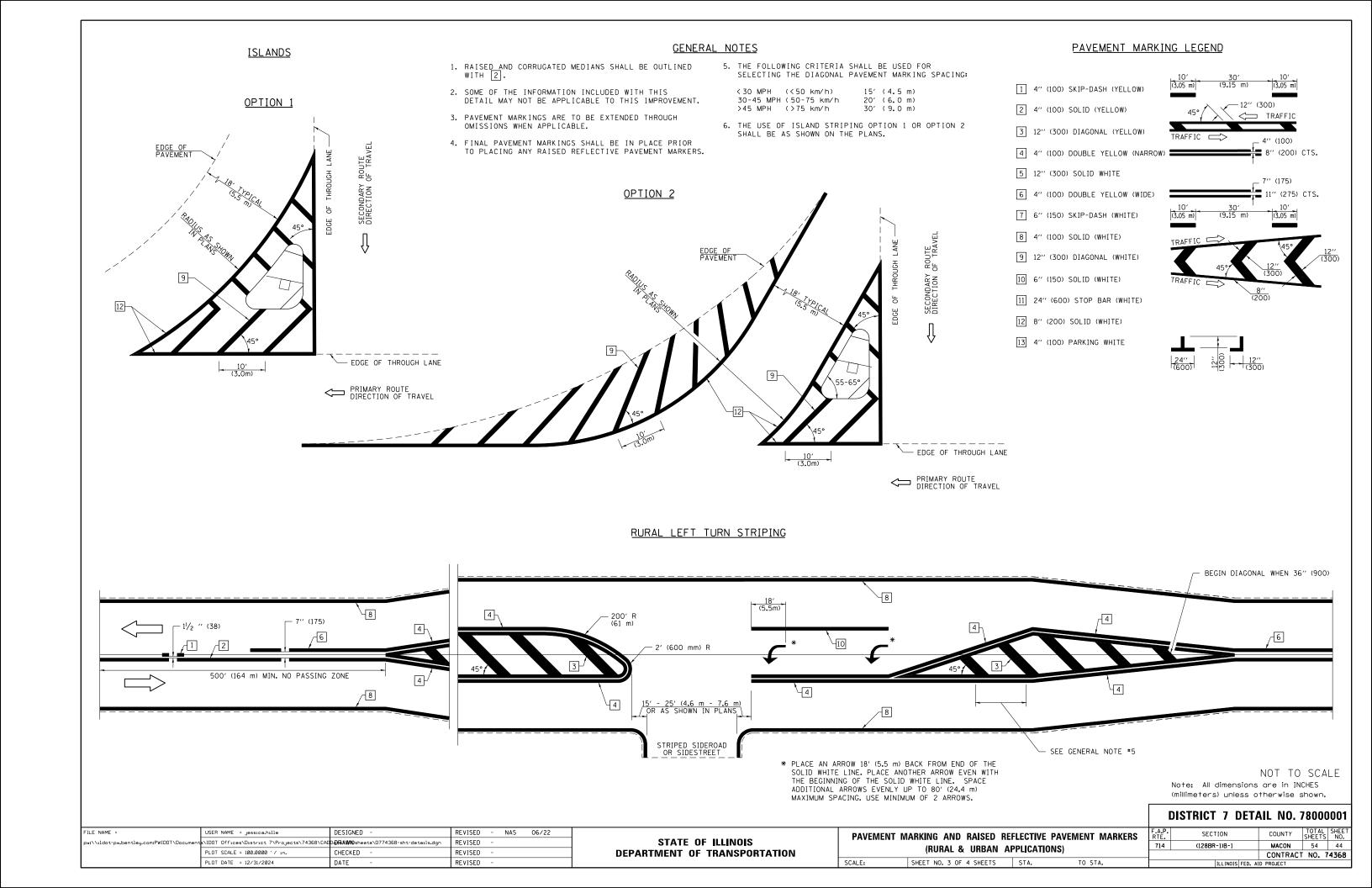
F.A.P. RTE. 714 COUNTY TOTAL SHEET NO.

MACON 54 41 SECTION SOIL BORING LOGS (128BR-1) B-1 **STRUCTURE NO. 058-0137** CONTRACT NO. 74368 SHEET 26 OF 26 SHEETS

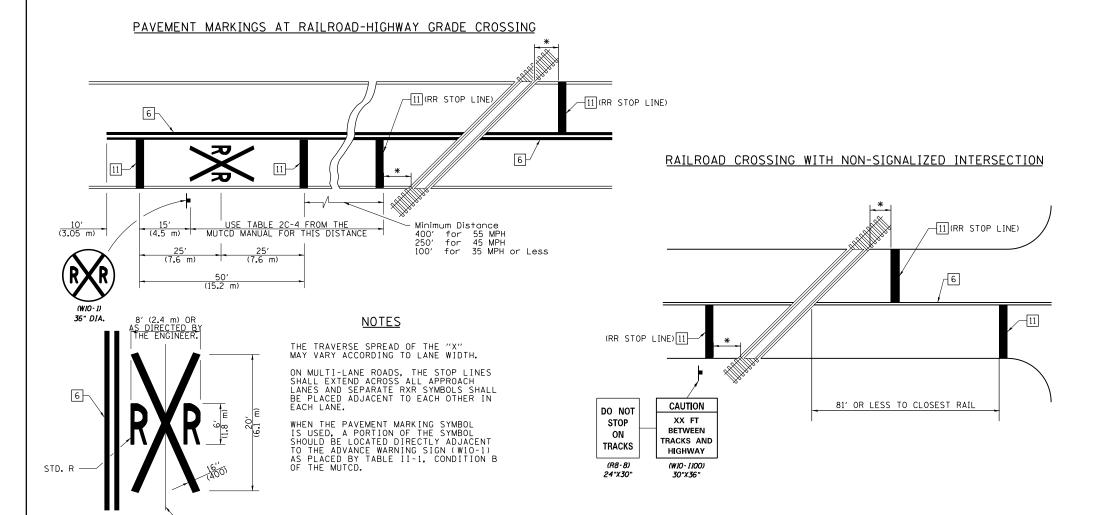
BBS, form 137 (Rev. 8-99)







### SUPPLEMENTAL PAVEMENT MARKING TREATMENT FOR RAILROAD-HIGHWAY GRADE CROSSING

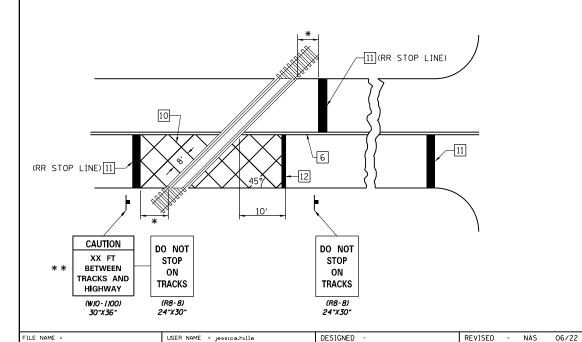


#### RAILROAD CROSSING WITH INTERCONNECT AND PRE-SIGNALS RAILROAD CROSSING WITH INTERCONNECT ONLY

REVISED

REVISED

REVISED



PLOT SCALE = 100.0000 '/ 10.

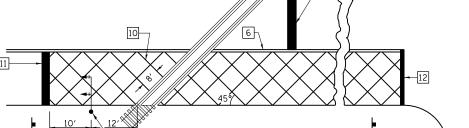
PLOT DATE = 12/31/2024

\IDOT\_Offices\District\_7\Projects\74368\CADDAGAWADsheets\D774368-sht-details.dom

CHECKED

DATE

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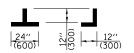
#### PRE-SIGNALS STOP DO NOT DO NOT TURN HERE ON STOP STOP RED ON RED ON ON TRACKS TRACKS (RIO-11b) 24"X30"

(R8-8)

24"X30"

#### PAVEMENT MARKING LEGEND

- 1 4" (100) SKIP-DASH (YELLOW)
- 2 4" (100) SOLID (YELLOW)
- 3 12" (300) DIAGONAL (YELLOW
- 4 4" (100) DOUBLE YELLOW (NARROW)
- 5 12" (300) SOLID WHITE
- 6 4" (100) DOUBLE YELLOW (WIDE)
- 7 6" (150) SKIP-DASH (WHITE)
- 8 4" (100) SOLID (WHITE)
- 9 12" (300) DIAGONAL (WHITE)
- 10 6" (150) SOLID (WHITE)
- 11 24" (600) STOP BAR (WHITE)
- 12 8" (200) SOLID (WHITE)
- 13 4" (100) PARKING WHITE



#### GENERAL NOTES

- SUPPLEMENTAL PAVEMENT MARKINGS TO BE INSTALLED ONLY ON APPROACHES TO INTERSECTIONS CONTROLLED BY TRAFFIC SIGNALS WHICH ARE INTERCONNECTED WITH THE RAILROAD WARNING SIGNALS.
- 2. EXTEND PAVEMENT MARKINGS TO THE INTERSECTION ONLY WHERE PRE-SIGNALS ARE USED.
- 3. WHEN PEDESTRIAN SIGNALS ARE PRESENT WITH INTERCONNECTED SIGNALS, WARNING SIGN W10-I101 (18"X24") SHALL BE PLACED NEAR EACH PEDESTRIAN SIGNAL HEAD. COUNTDOWN PEDESTRIAN SIGNAL HEADS SHALL NOT BE UTILIZED ALONG WITH INTERCONNECTED SIGNALS.

CAUTION WALK TIME SHORTENED WHEN TRAIN APPROACHES

<── TRAFFI

4" (100)

7" (175)

**3** 8" (200) CTS.

(W10-1101)

- PLEASE REFER TO THE IDOT BUREAU OF OPERATION MEMO OPS T-06 DATED DECEMBER 1, 2020 FOR ADDITIONAL INFORMATION
- 15' FROM NEAR RAIL OR 8' FROM AND PARALLEL TO GATE IF PRESENT

TO STA.

\*\* WARNING SIGN W10-I100 SHALL BE USED AS AN INTERIM MEASURE AT INTERCONNECTED SIGNAL LOCATIONS WHERE PRE-SIGNALS ARE TO BE INSTALLED IN THE FUTURE. THIS SIGN SHALL BE REMOVED WHEN THE PRE-SIGNALS ARE INSTALLED AND THE PAVEMENT MARKINGS ARE EXTENDED TO THE INTERSECTION.

NOT TO SCALE

MACON 54 45

CONTRACT NO. 74368

COUNTY

Note: All dimensions are in INCHES (millimeters) unless otherwise shown.

DISTRICT	7	DETAIL	NO.	78000001
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SECTION

(128BR-1)B-1

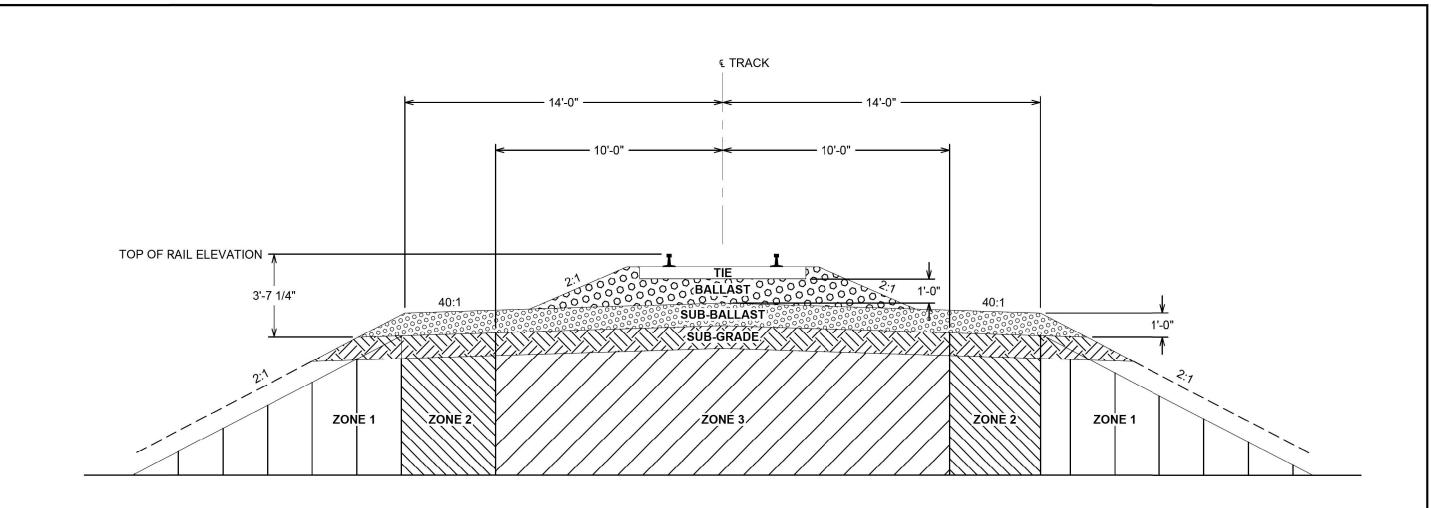
714

PAVEMENT MARKING AND RAISED REFLECTIVE PAVEMENT MARKERS STATE OF ILLINOIS (RURAL & URBAN APPLICATIONS) **DEPARTMENT OF TRANSPORTATION** SHEET NO. 4 OF 4 SHEETS STA.

-11 (RR STOP LINE)

(R8-8)

24"X30"



## **ROADBED PROFILE - SHORING REQUIREMENTS (NTS)**

- ZONE 1 EXCAVATION WITHIN ZONE 1 WILL REQUIRE SHORING FOR THE PROTECTION OF THE RAILROAD
- ZONE 2 EXCAVATION WITHIN ZONE 2 WILL REQUIRE SHORING CONSISTING OF INTERLOCKING SHEETING FOR THE PROTECTION OF THE RAILROAD
- ZONE 3 NO EXCAVATIONS WILL BE ALLOWED IN ZONE 3

#### NOTES:

- EXCAVATIONS OUTSIDE OF ZONE 1 MAY REQUIRE SHORING FOR SAFETY.
   LATERAL PRESSURES DUE TO TRAIN LOADINGS DO NOT NEED TO BE
   INCORPORATED IN SHORING DESIGNS WHERE SHORING/EXCAVATION DOES
   NOT ENCROACH IN ZONES DETAILED ON THIS DRAWING FOR ANY TRACK.
- 2. REFER TO PUBLIC PROJECTS MANUAL APPENDIX H, SECTION H.1.6. (OVERHEAD BRIDGE) OR SECTION H.2.8 (UNDERPASS BRIDGE) AND APPENDIX H FOR ADDITIONAL SHORING LOCATION REQUIREMENTS.



PUBLIC PROJECTS MANUAL TYPICAL DRAWINGS

SHORING DESIGN GUIDE SHORING REQUIREMENTS

REF. NO.: SEC 1 - OHB - 5 - SHT 4

DATE: JANUARY 1, 2022 DRAWING NO.: 4

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

REVISIONS

DATE LTR. DESCRIPTION

MODEL: Default FILE NAME: pw:\\ildot-pw.b

