

FOR INDEX OF SHEETS, SEE SHEET NO. 2

DESIGN DESIGNATION

LOCAL STREET

TRAFFIC DATA

ADT:

5,656 (2023)

SPEED LIMIT

30 MPH (POSTED)

30 MPH (DESIGN)

THE PROJECT IS LOCATED IN
THE CITY OF CHICAGOSUBSURFACE UTILITY ENGINEERING (SUE)
UTILIZED ON THIS PROJECTMEADE ELECTRIC CO. DISTRICT ONE ELECTRICAL MAINTENANCE
CONTRACTOR LOCATES IDOT ELECTRICAL EQUIPMENT AND
UNDERGROUND CABLES (773) 287-7672STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	FAI 290 22 BRIDGE 3	COOK	161	1

* 161 + 3 = 164 TOTAL SHEETS

** 164 + 2 = 166 TOTAL SHEETS

D-91-323-22

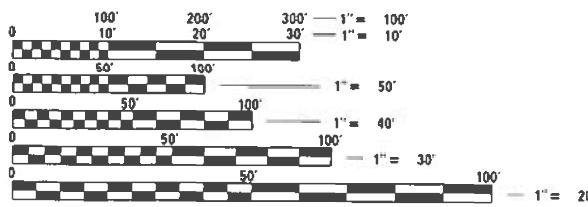
PROPOSED
HIGHWAY PLANS

F.A.I. 290 (INTERSTATE 290)
AT LOOMIS STREET
SECTION FAI 290 22 BRIDGE 3
PROJECT NHPP-1REM(318)
SUPERSTRUCTURE REPLACEMENT
COOK COUNTY

C-91-005-23

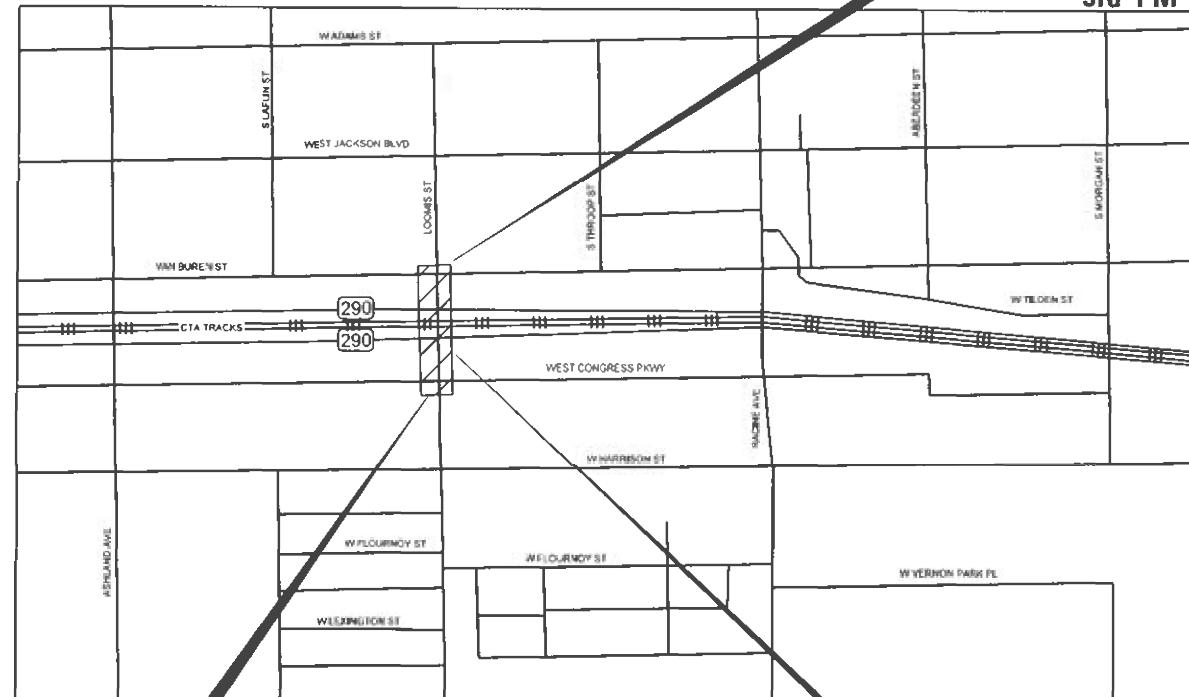
END PROJECT
STA 605 + 25.00

3rd PM



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD
ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT
CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS
ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATORS
1-800-892-0123
OR 811

BEGIN PROJECT
STA 600 + 05.00LOCATION MAP
NOT TO SCALESUPERSTRUCTURE REPLACEMENT
STA. 601 + 08.97 TO STA. 604 + 18.97
S.N. 016-2114

GROSS LENGTH = 520 FT. = 0.10 MILE

PROJECT MANAGER: PRAVEEN KAINI (847) 705-4237

CONTRACT NO. 62U12

8725 W. Higgins Rd, Ste 600, Chicago, IL 60631
P: 773.775.4009 | www.ciorba.com6330 Belmont Road, Suite 4B
Downers Grove, IL 60516CONSULTING
ENGINEERS
EFK-Moen
Civil Engineering DesignCONSULTING
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OF THE STATE OF ILLINOIS

REVISED SHEET 12/31/2025

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUBMITTED *Sept 20 2025*

REGIONAL ENGINEER
December 5, 2025
[Signature]

ENGINEER OF DESIGN AND ENVIRONMENT
December 5, 2025
[Signature]

DIRECTOR OF HIGHWAYS PROJECT IMPLEMENTATION
December 5, 2025
[Signature]



LOCATION OF SECTION INDICATED THUS: -

REV-SEP

GENERAL NOTES (CONTINUED)

26. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER A SATISFACTORY PROGRESS SCHEDULE AND CRITICAL PATH SCHEDULE WHICH SHALL SHOW THE PROPOSED SEQUENCE OF WORK AT THE TIME OF THE PRE-CONSTRUCTION CONFERENCE.
27. ALL ELEVATIONS ARE ON THE U.S.G.S. DATUM NAVD 88.
28. ALL OFFSET LOCATIONS GIVEN ON THE DETAILED PLANS FOR STRUCTURES, BACK OF CURBS, ETC. ARE FROM THE CENTERLINE AS SHOWN ON THE PLANS.
29. ANY LOOSE MATERIAL DEPOSITED IN THE FLOW LINE OF DRAINAGE STRUCTURES WHICH OBSTRUCTS THE NATURAL FLOW OF WATER SHALL BE REMOVED AT THE CLOSE OF EACH WORKING DAY. PRIOR TO ACCEPTANCE OF IMPROVEMENT, ALL DRAINAGE STRUCTURES SHALL BE FREE OF DIRT AND DEBRIS. THIS WORK WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED INCLUDED IN THE COST OF THE PROPOSED DRAINAGE ITEMS.
30. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING FRESH CONCRETE FROM DAMAGE AND VANDALISM. ANY DAMAGED OR VANDALIZED CONCRETE SHALL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
31. THE COST OF PLUGGING EXISTING SEWER SHALL BE INCLUDED IN THE COST OF THE SEWER BEING REMOVED.
32. BACKFILLING STORM SEWER CONSTRUCTED UNDER THE ROADWAY SPECIFIED UNDER ARTICLE 550.07(B,C) OF THE STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION WILL NOT BE ALLOWED.
33. THE CONTRACTOR SHALL COORDINATE WITH THE CTA ON THE CLOSURE OF THE RAMP ON THE PLATFORM.
34. MEADE ELECTRIC CO. DISTRICT ONE ELECTRICAL MAINTENANCE CONTRACTOR LOCATES IDOT ELECTRICAL EQUIPMENT AND UNDERGROUND CABLES 773-287-7672.
35. THE CONTRACTOR SHALL CONTACT KALPANA KANNAN-HOSADURGA, THE DISTRICT ONE TRAFFIC CONTROL SUPERVISOR AT KALPANA.KANNAN-HOSADURGA@ILLINOIS.GOV, A MINIMUM OF 72 HOURS IN ADVANCE OF BEGINNING WORK.

COMMITMENTS

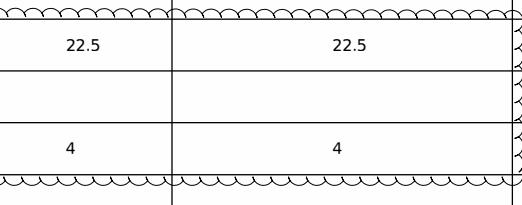
1. COMMITMENTS ARE NOT TO BE ALTERED WITHOUT WRITTEN APPROVAL OF ALL PARTIES.
2. ADJACENT I-290 OVERHEAD BRIDGES SHALL NOT BE CLOSED/DETOURED CONCURRENTLY DURING CONSTRUCTION.

CITY OF CHICAGO NOTES

1. ALL NEW CURB INSTALLATION ADJACENT TO FIRE HYDRANTS MUST BE PAINTED "SAFETY YELLOW" FOR 15 FEET ON EACH SIDE OF THE FIRE HYDRANT EXCEPT WHERE THE 15-FOOT DIMENSION INTERSECTS A CROSSWALK, DRIVEWAY OR SIMILAR FEATURE. THE COST SHALL BE INCLUDED WITH THE COST OF THERMOPLASTIC PAVEMENT MARKING - LINE 24".
2. PERMITS FROM THE DEPARTMENT OF WATER MANAGEMENT - SEWER SECTION ARE REQUIRED FOR RESURFACING WORK INVOLVING ADJUSTMENT OF CITY OF CHICAGO SEWER STRUCTURES. THE PERMIT MUST BE OBTAINED BY A LICENSED SEWER DRAIN LAYER PRIOR TO START OF CONSTRUCTION. THE LICENSED SEWER CONTRACTOR OR SUBCONTRACTOR MUST SUBMIT TWO (2) SETS OF PLANS APPROVED BY THE DEPARTMENT OF SEWERS FOR THE ISSUE OF THE SEWER PERMIT IN SUITE 410 - 333 SOUTH STATE STREET CHICAGO, IL 60604. INSPECTION WILL BE PROVIDED BY THE DEPARTMENT OF SEWERS.
3. IN CASE OF DAMAGE TO CITY OF CHICAGO SEWERS, PRIVATE AND PUBLIC DRAINS, SEWER STRUCTURES AND/OR BENCH MONUMENTS, THE CONTRACTOR SHALL IMMEDIATELY CONTACT THE DEPARTMENT OF WATER MANAGEMENT - SEWER SECTION AT (312) 747-7892 OR (312) 747-7893.
4. THE CONTRACTOR MUST NOTIFY CHICAGO'S OFFICE OF EMERGENCY MANAGEMENT AND COMMUNICATION (OEMC) 48 HOURS BEFORE COMMENCING CONSTRUCTION OR CHANGING TRAFFIC FLOW.
5. CITY OF CHICAGO WATER VALVE VAULTS AND SEWER STRUCTURES SHALL NOT BE CLOSED, COVERED OR OTHERWISE OBSTRUCTED DURING CONSTRUCTION WITHOUT WRITTEN PERMISSION FROM THE CITY OF CHICAGO DEPARTMENT OF WATER AND/OR DEPARTMENT OF WATER MANAGEMENT SEWER SECTION.
6. THE CONTRACTOR SHALL CONTACT THE CHICAGO TRANSIT AUTHORITY AT TRAFFIC.PLANNING@TRANSITCHICAGO.COM OR BY PHONE AT (312) 681-4151 SIX (6) WEEKS PRIOR TO CONSTRUCTION, TO REPORT TRAFFIC IMPACTS.
7. THE CTA REQUIRES THAT NO TWO CONSECUTIVE BUS STOPS IN ONE DIRECTION OF TRAVEL ARE ELIMINATED AT THE SAME TIME. THE CONTRACTOR SHALL SUBSTAGE WORK AS NECESSARY OR PROVIDE TEMPORARY ADA COMPLIANT PEDESTRIAN ACCESS THROUGH THE WORK ZONE TO AN EXISTING OR TEMPORARILY RELOCATED BUS STOP.
8. CTA TRAFFIC PLANNING MUST BE NOTIFIED AT LEAST 2 WEEKS IN ADVANCE OF THE TIME CONSTRUCTION WILL COMMENCE, AND IN ADVANCE OF ANY IMPACT TO BUS STOPS OR BUS OPERATIONS. TRAFFIC.PLANNING@TRANSITCHICAGO.COM
9. CTA OPERATES BUSES ON VAN BUREN STREET AND REQUIRE A MINIMUM OF 11 FOOT LANE WIDTHS. THE CONTRACTOR AND ENGINEER SHALL CONTACT CTA TRAFFIC PLANNING TO DISCUSS IMPACTS SHOULD THERE BE DISRUPTIONS TO THE BUS ROUTING. AT LEAST TWO WEEKS NOTICE IS REQUIRED FOR ANY BUS SERVICE CHANGES. CTA'S POINT OF CONTACT: JACK CHALABIAN - COORDINATOR, TRAFFIC PLANNING. JCHALABIAN@TRANSITCHICAGO.COM OR DIRECT OFFICE LINE AT 312-681-4176
10. EXISTING CROWN CASTLE FACILITIES ARE IN THE AREA AND EXTREME CAUTION SHALL BE USED. HAND TRENCH WITHIN 3 FEET OF CROWN CASTLE FACILITIES TO VISUALLY LOCATED AND A MINIMUM OF 12 INCH CLEARANCE (VERTICAL & HORIZONTAL) FROM EXISTING CROWN CASTLE FACILITIES IS REQUIRED. CONTACT DIGGER FOR LOCATES A MINIMUM OF 48 HOURS BEFORE BEGINNING CONSTRUCTION.
11. THE CONTRACTOR SHALL COORDINATE TRAFFIC CONTROL (INCLUDING LANE CLOSURES, VEHICLE AND EQUIPMENT PLACEMENT, ETC) WITH CDOT-DIVISION OF INFRASTRUCTURE MANAGEMENT PRIOR TO IMPLEMENTATION OF ASSOCIATED WORK STAGES. DETOUR ROUTING TO BE REVIEWED AT THE TIME OF CONSTRUCTION TO RESOLVE ANY SHORT TERM CONFLICTS WITH NEARBY WORK ZONES OR OTHER CLOSURES. CONTRACTOR TO SUBMIT CLOSURE PERMIT REQUEST TO CDOT-PERMITS (THIS TYPE OF ACTIVITY CANNOT BE REQUESTED ONLINE AND MUST BE GENERATED BY CDOT) AND PROVIDE A MINIMUM OF 2 WEEKS NOTICE TO CTA FOR IMPACTS TO BUS SERVICE.
12. THE CONTRACTOR SHALL USE EXTREME CAUTION WHEN WORKING NEAR AND ABOVE TRACK STRUCTURES, NEAR THE TRACKS AND THEIR FOUNDATIONS, AND BELOW EXISTING SIGNAL AND COMMUNICATION CABLES. ALL EXCAVATION AND DEMOLITION WORK, MINIMUM 4' - 6" NEAR EXISTING CTA'S STRUCTURES SHALL BE DONE BY HAND. THE CONTRACTOR SHALL PROTECT ALL CTA'S STRUCTURES AND TRAINS FROM CONSTRUCTION DEBIRS. FLAGMEN MAY BE REQUIRED AT TRACK LEVEL IF ANY TRAINS NEED TO BE STOPPED FOR ANY POTENTIALLY DANGEROUS CRANE PICKS OR ANY OTHER WORK THAT MIGHT AFFECT A TRAIN ROUTE.
13. THE CONTRACTOR SHALL RESTORE ALL DAMAGED STRUCTURES AND UTILITIES TO THE SATISFACTION OF THE CTA. THE CONTRACTOR SHALL FOLLOW THE CTA'S ADJACENT CONSTRUCTION MANUAL, CURRENT ADDITION. COORDINATION OF THE INSURANCE REQUIREMENTS SHALL OCCUR WITH LINDA LEE, CPCU, ARM, AIC, AINS, CSM - CTA RISK COMPLIANCE - LAW DEPARTMENT (PH: 312-681-2921 / EMAIL: LLEE@TRANSITCHICAGO.COM) THIS WORK IS SUBJECT TO THE REQUIREMENTS ESTABLISHED IN THE CTA ADJACENT CONSTRUCTION MANUAL AND "CTA REQUIREMENTS FOR CONTRACTOR'S WORKING ALONG RIGHT-OF-WAY (R.O.W.)" WHICH CAN BE FOUND AT [HTTP://WWW.TRANSITCHICAGO.COM/NEARBYCONSTRUCTION](http://WWW.TRANSITCHICAGO.COM/NEARBYCONSTRUCTION)
14. ALL CONSTRUCTION ACTIVITIES SHALL BE COORDINATED WITH THE CTA WITH ABDIN CARRILLO CONSTRUCTION PROJECT MANAGER III, CAPITAL.
15. PEOPLES GAS MAINS AND SERVICES ARE PRESENT. THE CONTRACTOR SHALL USE CAUTION. CONTACT 811 CHICAGO/DIGGER 312-744-7000 FOR LOCATES 48 HOURS PRIOR TO START OF CONSTRUCTION. USE EXTREME CAUTION NEAR ALL GAS FACILITIES DURING CONSTRUCTION AND RELATED EXCAVATION ACTIVITIES. HAND DIG OR NON-INVASIVE EXCAVATION IS REQUIRED TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF GAS FACILITIES PRIOR TO CROSSING AND WORKING WITHIN 3 FEET OF ALL GAS FACILITIES. A MINIMUM OF 3' HORIZONTAL EDGE TO EDGE CLEARANCE IS REQUIRED FOR GAS FACILITIES WITH DIAMETERS OF 16" OR SMALLER, AND 5FT EDGE TO EDGE CLEARANCE FOR GAS FACILITIES WITH DIAMETERS 18" AND LARGER. MAINTAIN A MINIMUM OF 18" EDGE TO EDGE VERTICAL CLEARANCE WHEN CROSSING GAS FACILITIES 16" OR LESS IN DIAMETER, AND 24" EDGE TO EDGE VERTICAL CLEARANCE WHEN CROSSING 18" AND LARGER DIAMETER GAS FACILITIES.
16. CONTACT 811 CHICAGO/DIGGER 312-744-7000 FOR LOCATES 48 HOURS PRIOR TO START OF CONSTRUCTION. VALVE BASIN FRAMES, COVERS, GAS SHUT-OFF VALVES, ROADWAY BOXES SHALL NOT BE BURIED/COVERED AND REQUIRE UNRESTRICTED ACCESS AT ALL TIMES. FOR VERTICAL ADJUSTMENTS CONTACT PAVING AND RESTORATION AT PGRESTORATION@PEOPLESgasDELIVERY.COM, 4 WEEKS PRIOR TO THE START OF RESTORATION FOR PLANNING & SCHEDULING. ALL GAS FACILITIES ARE TO BE MAINTAINED. ANY DAMAGES TO PEOPLES GAS FACILITIES SHALL BE THE RESPONSIBILITY OF THE INSTALLING UTILITY AND THEIR CONTRACTOR(S). CALL 866-556-6002 IMMEDIATELY FOR ANY DAMAGES TO THE GAS FACILITIES. THE USE OF CONCRETE, FLOW FILL, OR THE LIKE IS PROHIBITED WITHIN 24 INCHES OF ALL GAS FACILITIES, NOR SHALL IT ENCASE ANY GAS FACILITY. A BUFFER OF 24" SAND IS TO BE USED BETWEEN FLOW FILL AND ALL GAS FACILITIES. A MINIMUM OF 6" FA-02 OR FM-02 SAND SHALL BE USED WHEN BACKFILLING OTHER MATERIALS AROUND ANY EXPOSED GAS FACILITY. CONTRACTOR EXPOSING GAS FACILITY IS RESPONSIBLE FOR PROVIDING THE SAND.
17. ANY ISSUES OR WORK NEEDED WHEN CROSSING OR ADJACENT TO ANY COMED ASSET, THE CONTRACTOR SHALL CONTACT ANGELA WILLIAMS AT THREE LINCOLN CENTRE, SUITE 600, OAK BROOK TERRACE, ILLINOIS 60181 / PH: 779-231-1065 / EMAIL: ANGELA.WILLIAMS@COMED.COM

CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE			
				90% FED 10% STATE	90% FED 10% STATE	90% FED 10% STATE	90% FED 10% STATE
				ROADWAY	BRIDGE	IDOT HIGHWAY LIGHTING	TRAFFIC SIGNALS & LIGHTING (CDOT)
				0005	0013	0021	0021
				URBAN	S.N. 016-2114	URBAN	URBAN
70303210	TEMPORARY PAVEMENT MARKING - LINE 24"- MODIFIED URETHANE	FOOT	712	712			
70307120	TEMPORARY PAVEMENT MARKING - LINE 4" - TYPE IV TAPE	FOOT	12,834	12834			
70307125	TEMPORARY PAVEMENT MARKING - LINE 5" - TYPE IV TAPE	FOOT	5,220	5220			
70307130	TEMPORARY PAVEMENT MARKING - LINE 6" - TYPE IV TAPE	FOOT	1,550	1550			
70307140	TEMPORARY PAVEMENT MARKING - LINE 8" - TYPE IV TAPE	FOOT	4,894	4894			
70307160	TEMPORARY PAVEMENT MARKING - LINE 12"- TYPE IV TAPE	FOOT	548	548			
70400100	TEMPORARY CONCRETE BARRIER	FOOT	1,550	1550			
70400125	PINNING TEMPORARY CONCRETE BARRIER	EACH	48	48			
70600255	IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, NARROW), TEST LEVEL 2	EACH	2	2			
70600260	IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, NARROW), TEST LEVEL 3	EACH	4	4			
*	72000100 SIGN PANEL - TYPE 1	SQ FT	71	71			
*	72000300 SIGN PANEL - TYPE 3	SQ FT	170	169.5			
*	73304000 OVERHEAD SIGN STRUCTURE - BRIDGE MOUNTED	FOOT	22.5	22.5			
*	73602000 REMOVE OVERHEAD SIGN STRUCTURE - BRIDGE MOUNTED	EACH	4	4			

* SPECIALTY ITEM



1

REVISED SHEET 12/30/2025



BLA, Inc.
ITASCA, ILLINOIS

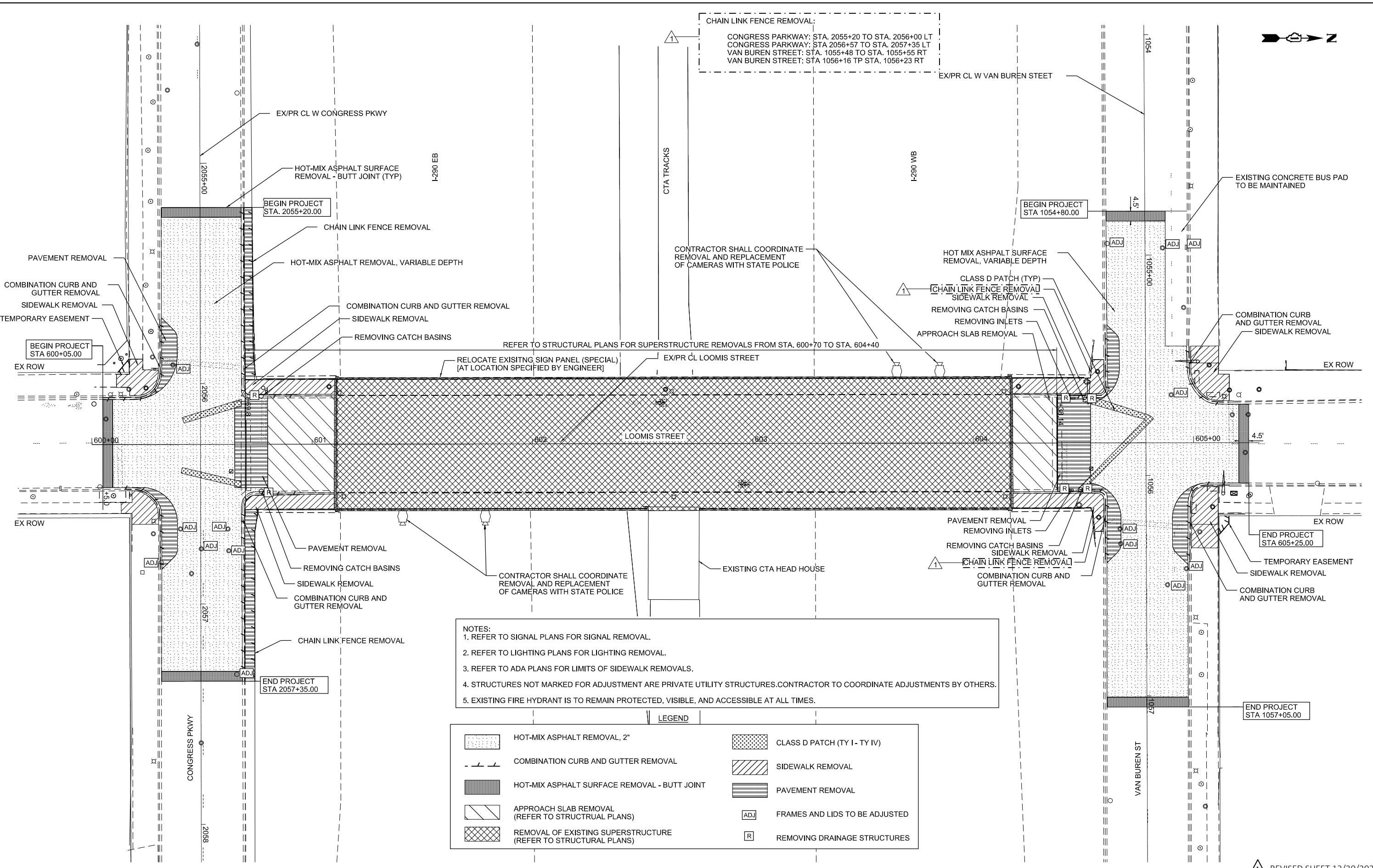
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DRAWN -	REVISED -	
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PLOT DATE = 10/16/2025	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

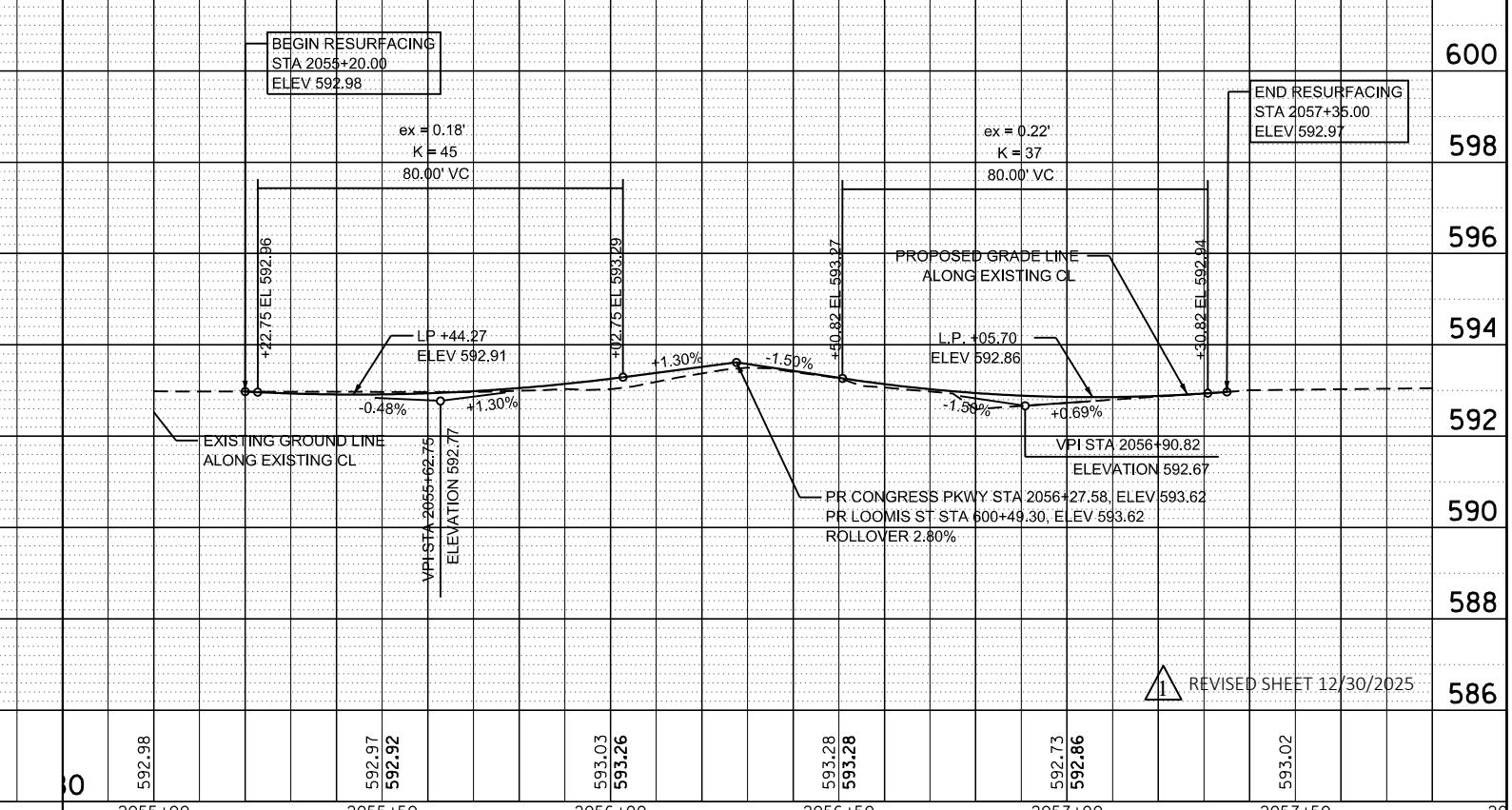
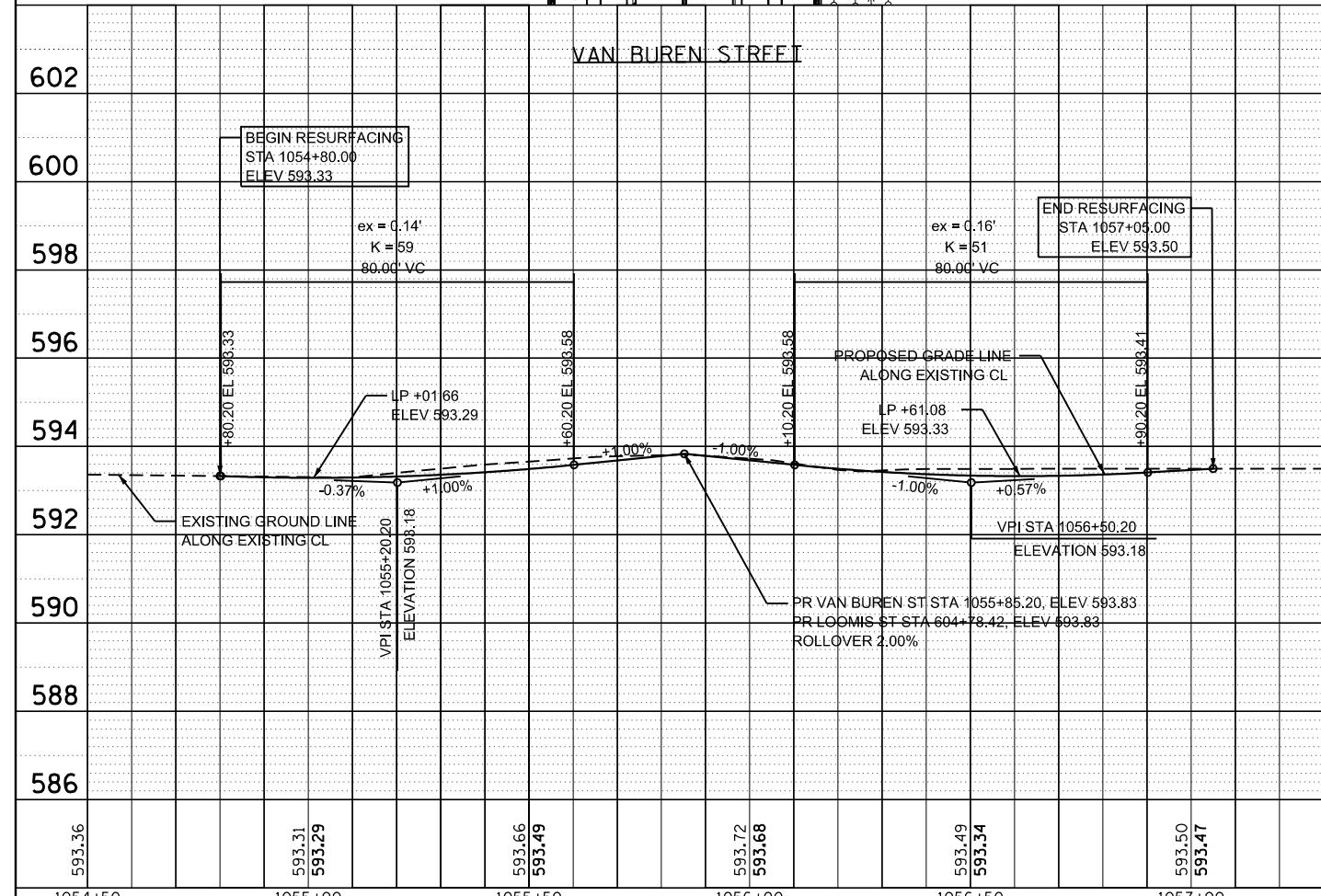
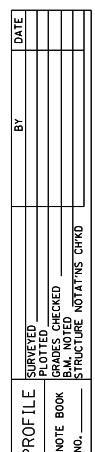
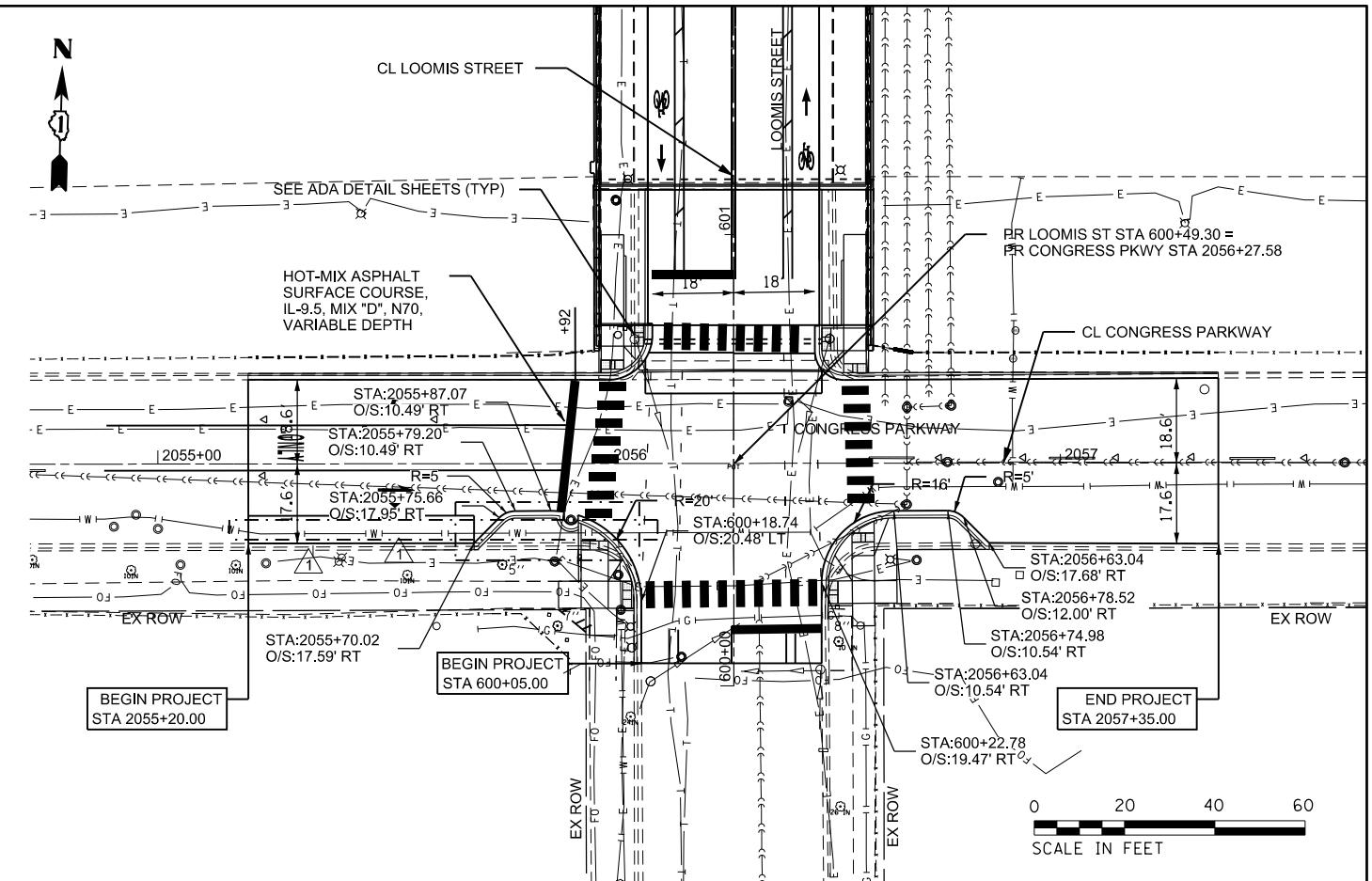
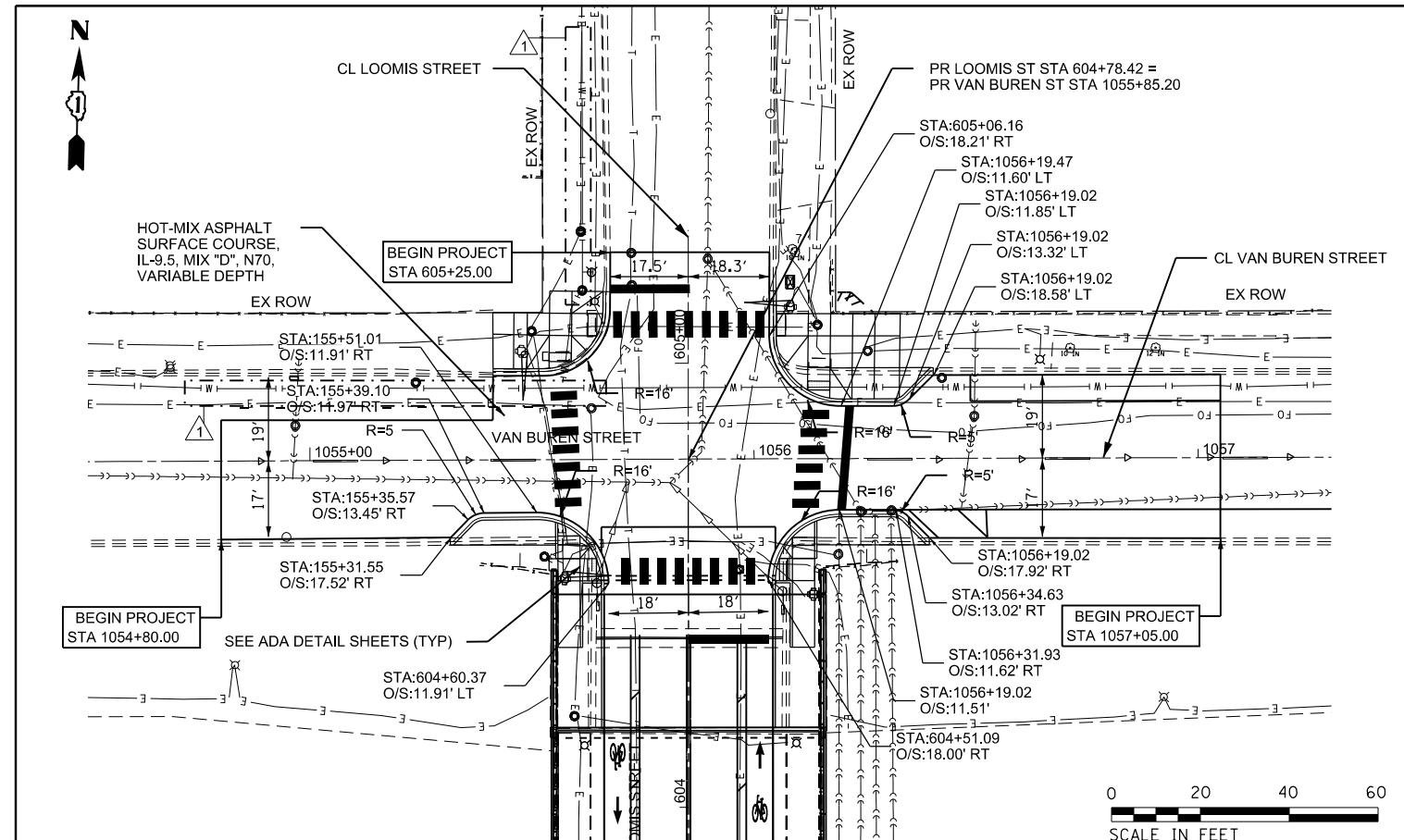
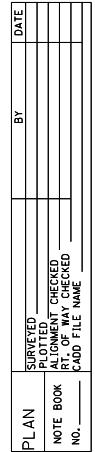
SUMMARY OF QUANTITIES
LOOMIS STREET OVER I-290 (FAI 290)

FAI. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	FAI 290 22 BRIDGE 3	COOK	161	9
				CONTRACT NO. 62U12

ILLINOIS | FED. AID PROJECT



REVISED SHEET 12/30/2025



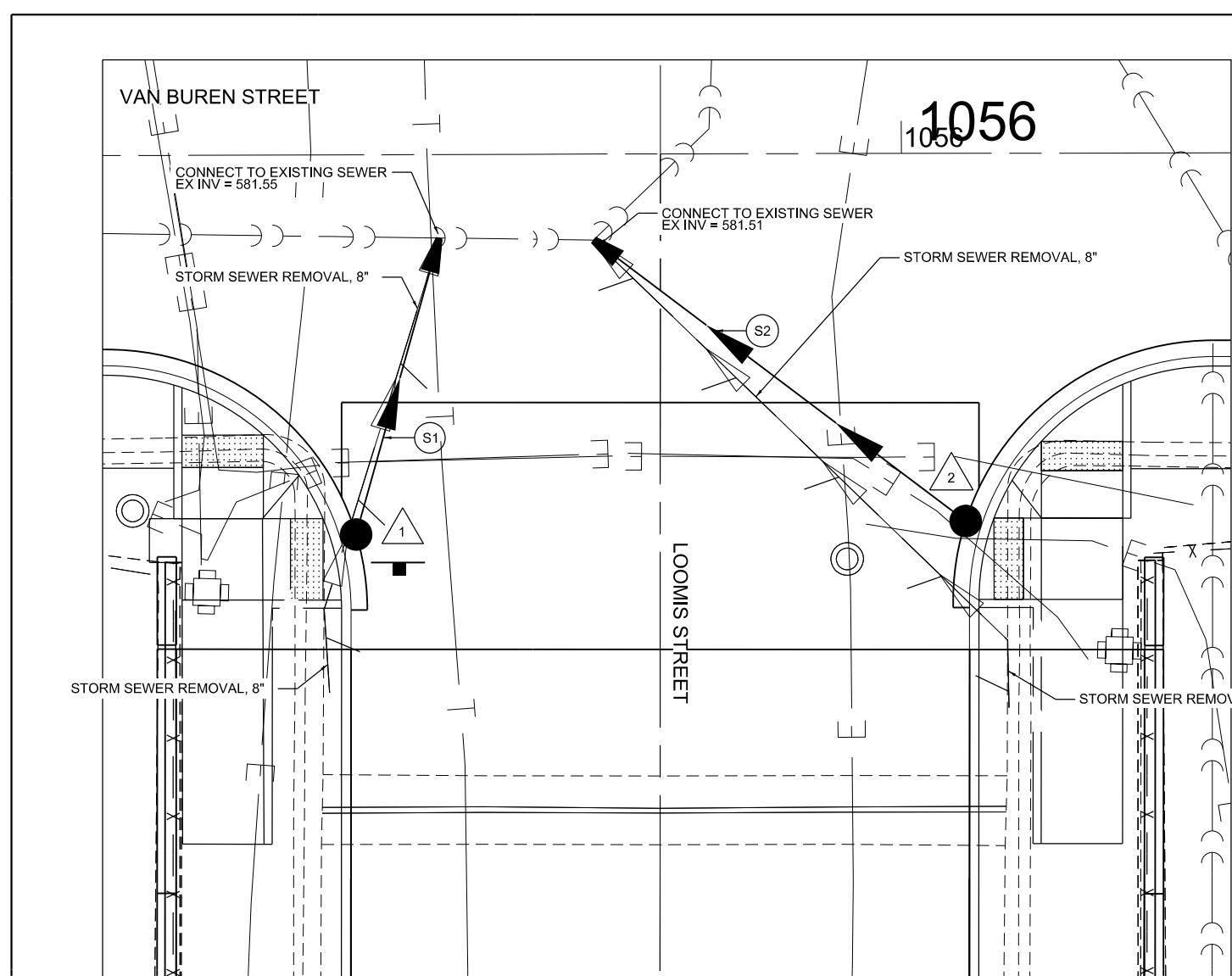
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Default	PLOT DATE = 12/19/2025	DA

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**ROADWAY PLAN AND PROFILE
CONGRESS PARKWAY AND VAN BUREN STREET**

20' | SHEET 2 OF 2 SHEETS STA. TO STA.

2057+00		2057+50		20	
FAI RTE.	SECTION	COUNTY	TOTAL SHEETS	HEET NO.	
2140	FAI 290 22 BRIDGE 3	COOK	161	25	
		CONTRACT NO. 62U12			
	ILLINOIS FED. AID PROJECT				

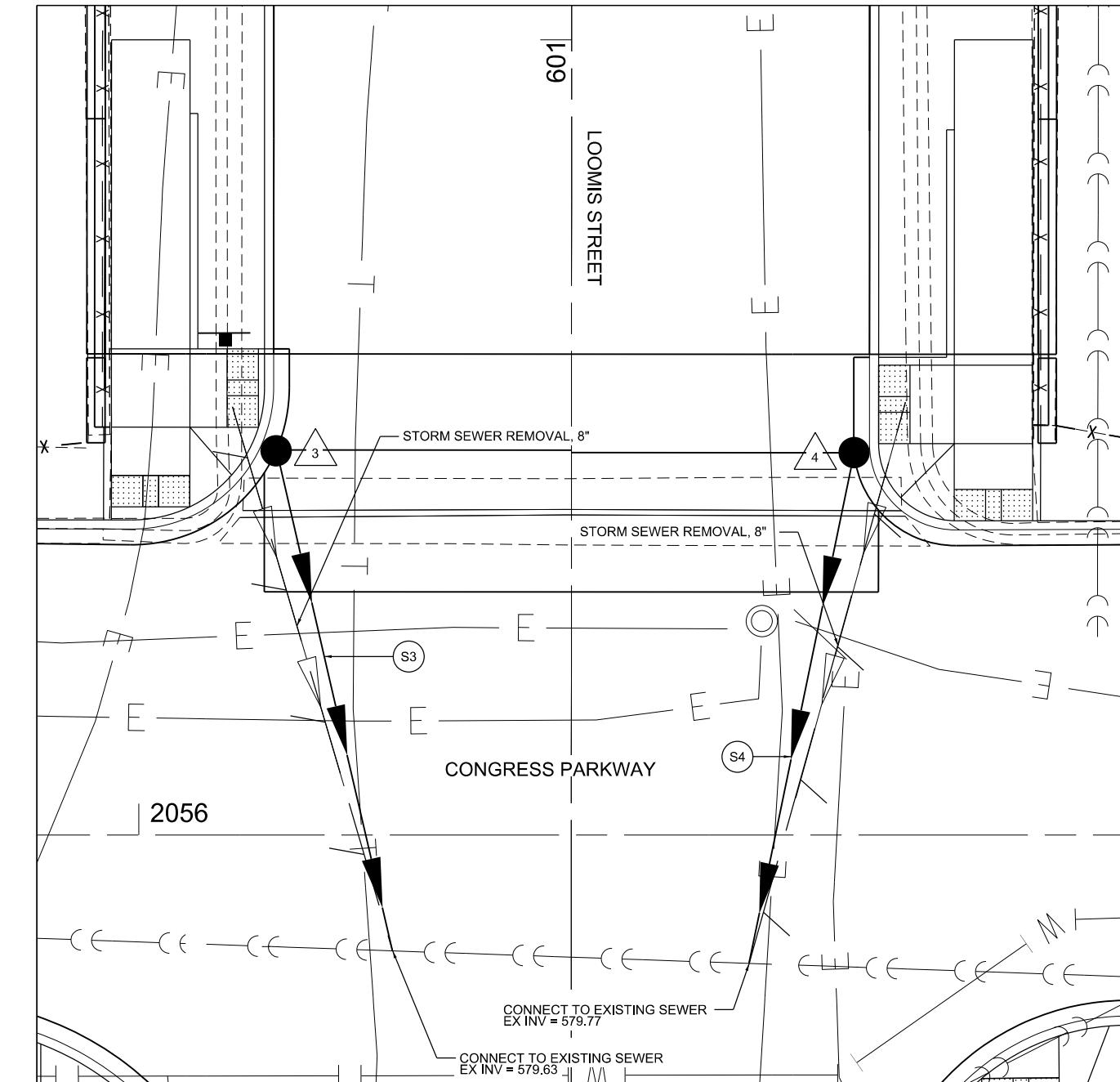


PIPETABLE							
PIPE NO.	STRUCTURE		DESCRIPTION	CLASS	TYPE	DIAMETER (IN)	LENGTH (FT)
	FROM	TO					
S1	1	EX	STORM SEWERS (WMQ)	A	2	12	20
S2	2	EX	STORM SEWERS (WMQ)	A	2	12	29
S3	3	EX	STORM SEWERS (WMQ)	A	2	12	33
S4	4	EX	STORM SEWERS (WMQ)	A	2	12	34

NOTE:
STORM SEWER MATERIAL FOR WATERMAIN QUALITY PIPE FOR S3 AND S4 SHALL BE DUCTILE IRON PIPE

STRUCT. NO.	STATION	OFFSET	RIM ELEVATION	DIAMETER (FT)	TYPE	FRAME AND GRATE	INVERT ELEVATIONS			
							NORTH	EAST	SOUTH	WEST
1	604+55.02	18.68' LT	594.09	4	CATCH BASIN, TYPE A	TYPE24 FRAME AND GRATE - ADA COMPLIANT	590.91			
2	604+55.86	18.76' RT	594.07	4	CATCH BASIN, TYPE A	TYPE24 FRAME AND GRATE - ADA COMPLIANT	591.00			
3	600+73.84	18.80' LT	594.00	4	CATCH BASIN, TYPE A	TYPE24 FRAME AND GRATE - ADA COMPLIANT		591.00		
4	600+73.84	18.02' RT	594.02	4	CATCH BASIN, TYPE A	TYPE24 FRAME AND GRATE - ADA COMPLIANT			591.02	

NOTE:
CONTRACTOR SHALL VERIFY THE EXACT ELEVATION OF THE CONNECTION OF THE PROPOSED STORM SEWER TO THE EXISTING.
THE SLOPE OF THE PROPOSED STORM SEWER PIPE SHALL BE ADJUSTED TO MATCH THE EXISTING INVERT ELEVATION.



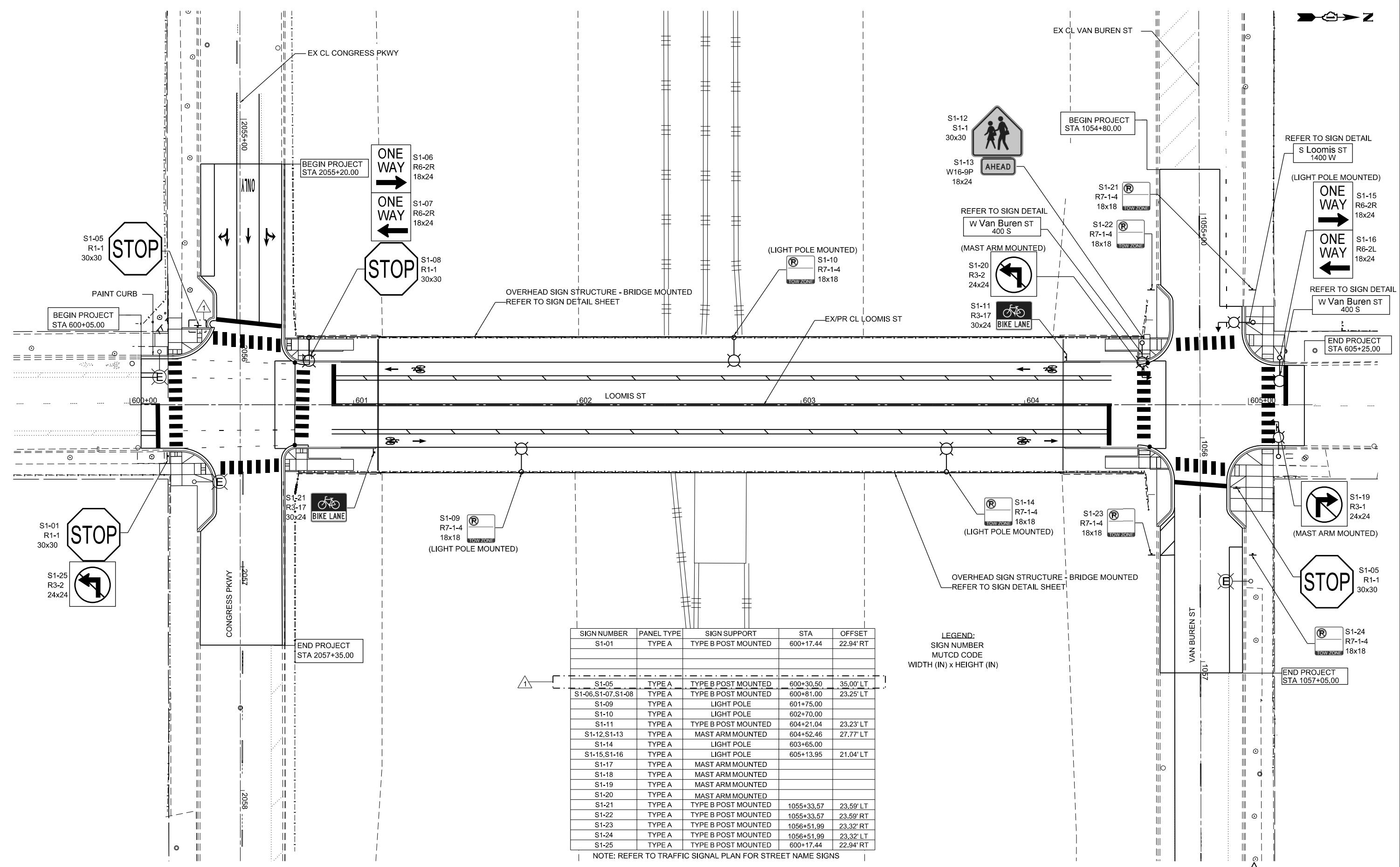
LEGEND		
	STORM SEWER REMOVAL	
	PROPOSED STRUCTURE	
	PROPOSED STORM SEWER	

A REPRESENTATIVE OF THE DWM MUST BE PRESENT DURING THE EXCAVATION AND INSTALLATION OF THE PROPOSED SEWER S4. WHERE IT IS IN CLOSE PROXIMITY TO THE EXISTING 12 INCH WATERMAIN AND THRUST BLOCK. IT IS REQUIRED THAT THE FORCE ACCOUNT CONSTRUCTION MANAGER BE CONTACTED AT FACM@DWMPMO.NET TWO WEEKS PRIOR TO THE ANTICIPATED CONSTRUCTION DATE SO A RESIDENT ENGINEER CAN BE ADDED TO THE PROJECT. THE DWM REPRESENTATIVE WILL ADHERE TO THE SCHEDULE PROVIDED BY IDOT, UNLESS NOTIFIED OTHERWISE. FAILURE TO COMPLY WITH THESE REQUIREMENTS MAY RESULT IN ADDITIONAL EXPENSES TO THE PROPOSED PROJECT TO VERIFY THAT ALL WORK CONFORMS TO DWM'S STANDARDS.

FOR PROPOSED SEWER INSTALLATIONS S3 AND S4, MAX TRENCH WIDTH NOT TO EXCEED 3 FEET.

THE EXISTING CB LATERAL MUST BE CAPPED/PLUGGED AT A DISTANCE FROM THE WATERMAIN THAT IS COMPLIANT WITH IEPA SEPARATION REQUIREMENTS FOR WATER AND SEWER.

REVISED SHEET 12/30/2025



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SIGNING PLAN
LOOMIS STREET OVER I-290 (FAI 290)

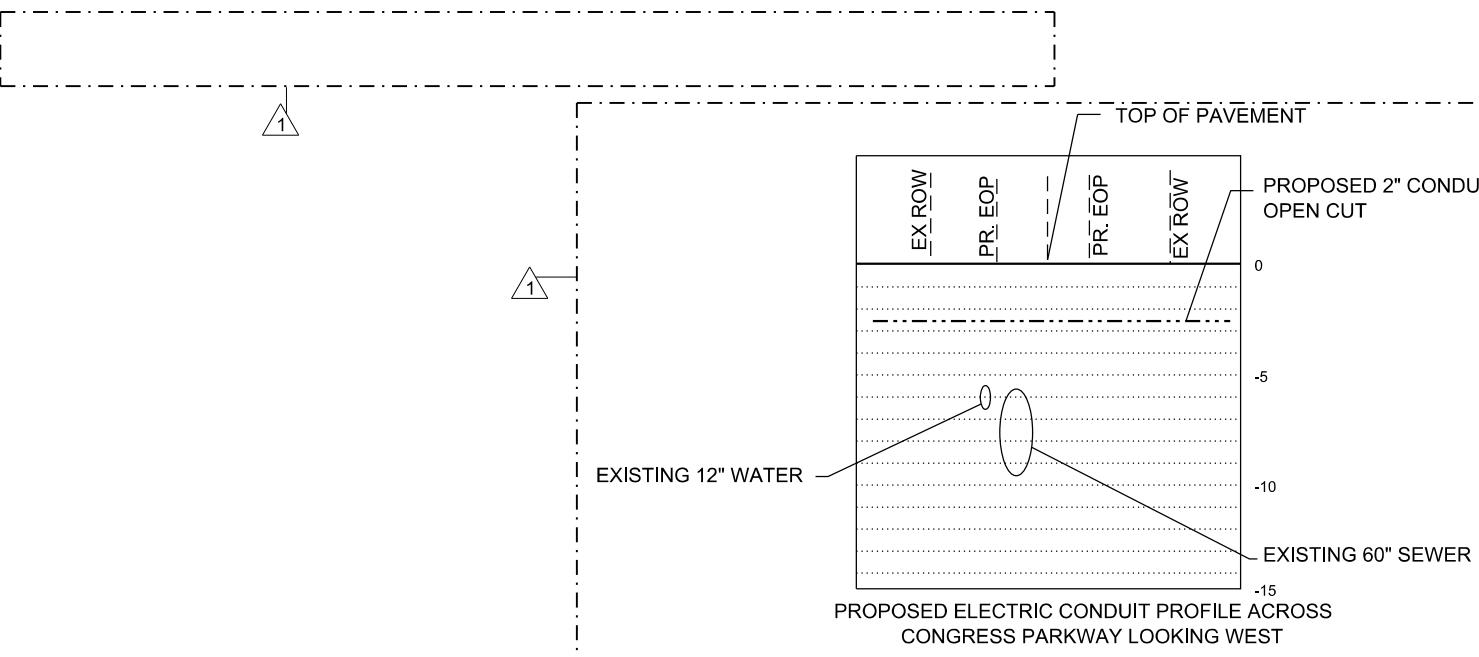
SCALE: 1" = 20' SHEET 1 OF 1 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	HEET NO.
290 FAI 290 22 BRIDGE 3	COOK	161	46	

ILLINOIS FED. AID PROJECT

GENERAL NOTES:

1. THIS PROJECT INCLUDES THE REPLACEMENT OF LOOMIS STREET BRIDGE DECK AND SUPERSTRUCTURE RESULTING IN THE REPLACEMENT OF UNDER DECK AND OVER DECK LIGHTING. THE UNDER DECK LIGHTING OVER I-290 IS OWNED AND MAINTAINED BY IDOT WHERE AS THE OVER DECK LIGHTING ON THE BRIDGE IS OWNED AND MAINTAINED BY CDOT.
2. THE PROJECT LIMITS ALONG LOOMIS STREET ARE BETWEEN W. VAN BUREN STREET AND W. CONGRESS PARKWAY.
3. THE EXISTING UNDERPASS LIGHTING IS CONNECTED TO EXISTING IDOT LIGHTING CONTROLLER "G" WHICH SHALL CONTINUE TO FEED THE PROPOSED LIGHTING LUMINAIRES AFTER CONSTRUCTION.
4. THE EXISTING UNDERPASS LIGHTING SHALL BE REMOVED WHEN THE BRIDGE DECK ALONG WITH BEAMS ARE REMOVED. MAINLINE LIGHTING SHALL PROVIDE LIGHTING FOR THIS PORTION OF THE ROADWAY DURING CONSTRUCTION. THE PROPOSED UNDERPASS LUMINAIRES SHALL BE INSTALLED DURING CONSTRUCTION OF THE NEW DECK.
5. THE CONTRACTOR SHALL NOTIFY J.U.L.I.E. TO LOCATE AND MARK/STAKE ALL UNDERGROUND UTILITIES. MEADE ELECTRIC CO. DISTRICT 1 ELECTRICAL MAINTENANCE CONTRACTOR LOCATES IDOT ELECTRICAL EQUIPMENT AND UNDERGROUND CABLES, CALL MEADE ELECTRIC CO. TRANSFER IDOT MAINTAINED EQUIPMENT TO THE CONTRACTOR BEFORE THE START OF ANY WORK. THEIR PHONE NUMBER IS 773-287-7672. THE CONTRACTOR SHALL ALSO CONTACT 811 CHICAGO (DIGGER) AT 312-744-7000 TO LOCATE CITY OF CHICAGO EQUIPMENT AND UNDERGROUND CABLES.
6. THE LIGHTING SYSTEM INSTALLATION SHALL CONFORM TO THE LATEST NEC, IDOT STANDARDS, CDOT STANDARD AND LOCAL CODES.
7. ALL ELECTRICAL EQUIPMENT AND PRODUCT SHALL BE UL LISTED AND LABELED.
8. THE CONTRACTOR SHALL SUBMIT CDOT EQUIPMENT CATALOG CUTS TO:
CHICAGO DOT
2 N LASALLE ST, SUITE 1110
CHICAGO, IL 60602
CALL (312) 744-3600 TO COORDINATE.
9. THE CONTRACTOR SHALL PICK UP LIGHTING HARDWARE SUPPLIED BY THE CITY AND SALVAGE AT CITY'S DIVISION OF ELECTRICAL OPERATIONS LOCATED AT 2451 S. ASHLAND AVENUE, CHICAGO, IL.
10. THE COST OF DELIVERING SALVAGED LIGHTING HARDWARE TO CITY OF CHICAGO (CDOT) IS INCLUDED IN PAY ITEM 84200500 REMOVAL OF LIGHTING UNIT, SALVAGE.
11. MAINTENANCE OF THE LIGHTING SYSTEMS BENEATH THE BRIDGE SHALL BE INCLUDED IN PAY ITEM X8000003 MAINTENANCE OF LIGHTING SYSTEM.
12. THERE ARE EXISTING UNDERGROUND ELECTRIC CABLE UNDER THE NB LANES OF LOOMIS STREET AND EXISTING TELEPHONE CABLE LINES UNDER THE SB LANES OF LOOMIS STREET. THESE CABLES WILL NEED TO BE REPLACED AND RELOCATED BETWEEN THE EXISTING HANDHOLES AND MANHOLES AT VAN BUREN STREET AND AT CONGRESS PKWY. THE CONTRACTOR SHALL REPLACE THESE CABLE AND CONDUIT PER STRUCTURAL PLANS IN COOPERATION WITH THE ENGINEER AND CITY OF CHICAGO REQUIREMENTS.



BILL OF MATERIALS

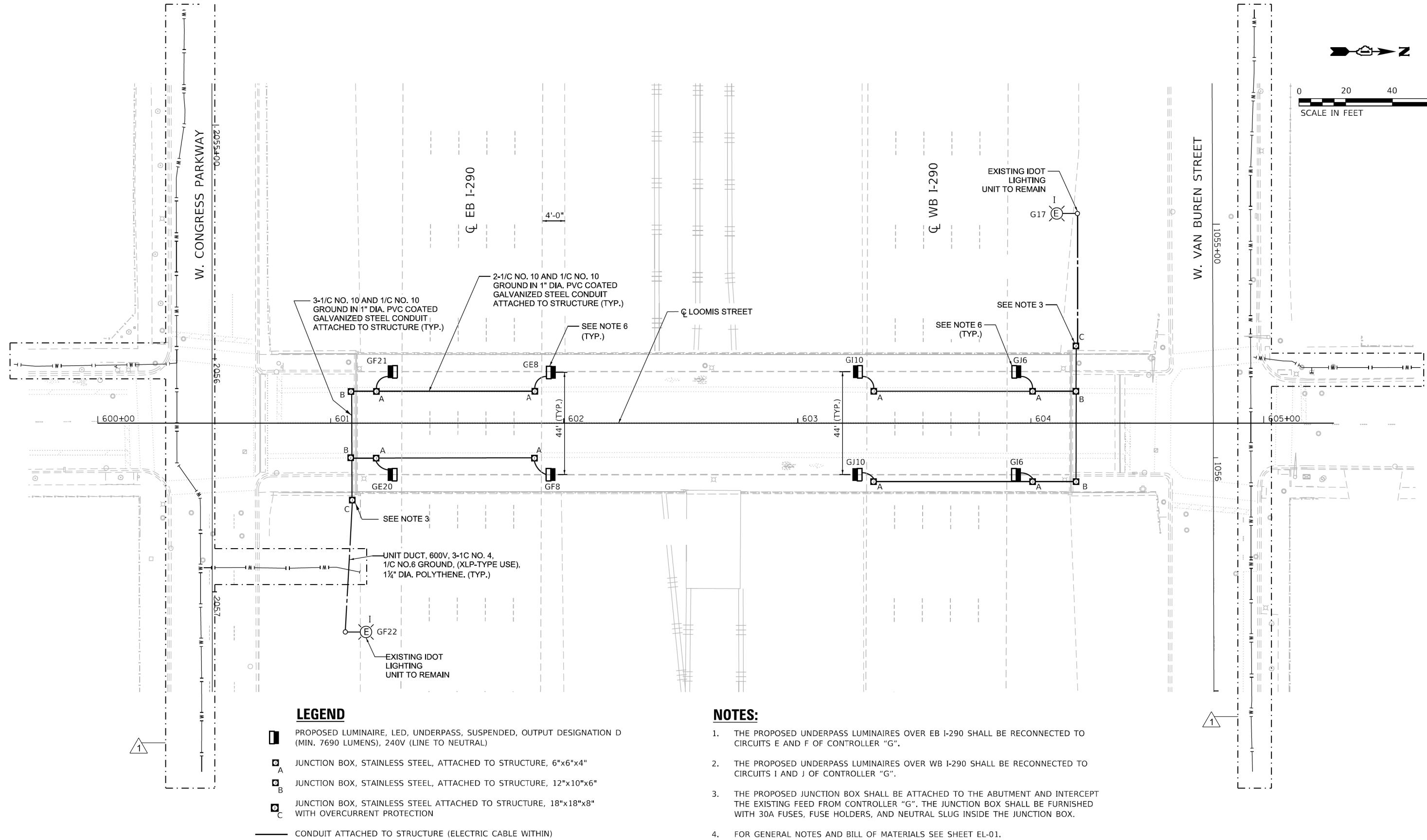
DESCRIPTION	UNIT	TOTAL QUANTITY	UNDERPASS IDOT LIGHTING	CDOT LIGHTING
UNDERGROUND CONDUIT, GALVANIZED STEEL, 2" DIA.	FOOT	106		106
CONDUIT ATTACHED TO STRUCTURE, 1" DIA., PVC COATED GALVANIZED STEEL	FOOT	435	435	
CONDUIT ATTACHED TO STRUCTURE, 3" DIA., PVC COATED GALVANIZED STEEL	FOOT	30	30	
CONDUIT EMBEDDED IN STRUCTURE, 1" DIA., PVC	FOOT	42		42
CONDUIT EMBEDDED IN STRUCTURE, 2" DIA., PVC	FOOT	708		708
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4"	EACH	8	8	
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 12" X 10" X 6"	EACH	4	4	
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 18" X 18" X 8"	EACH	2	2	
JUNCTION BOX EMBEDDED IN STRUCTURE 8" X 8" X 6"	EACH	2		2
UNIT DUCT, 600V, 3-1C NO.4, 1/C NO.6 GROUND, (XLP-TYPE USE), 1 1/4" DIA. POLYETHYLENE	FOOT	130	130	
ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 10	FOOT	1410	1410	
LUMINAIRE, LED, UNDERPASS, SUSPENDED, OUPUT DESIGNATION D	EACH	8	8	
REMOVAL OF LIGHTING UNIT, SALVAGE	EACH	6		6
REMOVAL OF LIGHTING UNIT, NO SALVAGE	EACH	8	8	
CABLE IN CONDUIT, TRIPLEX, 2-1/C NO. 6 AND 1-1/C NO. 8 GROUND	FOOT	881		881
LUMINAIRE (SPECIAL)	EACH	4		4
MAINTENANCE OF LIGHTING SYSTEM	CAL MO	12	12	
LIGHTING UNIT COMPLETE (SPECIAL)	EACH	4		4
MAINTENANCE OF STREET LIGHTING SYSTEM (CITY OF CHICAGO)	L SUM	1		1

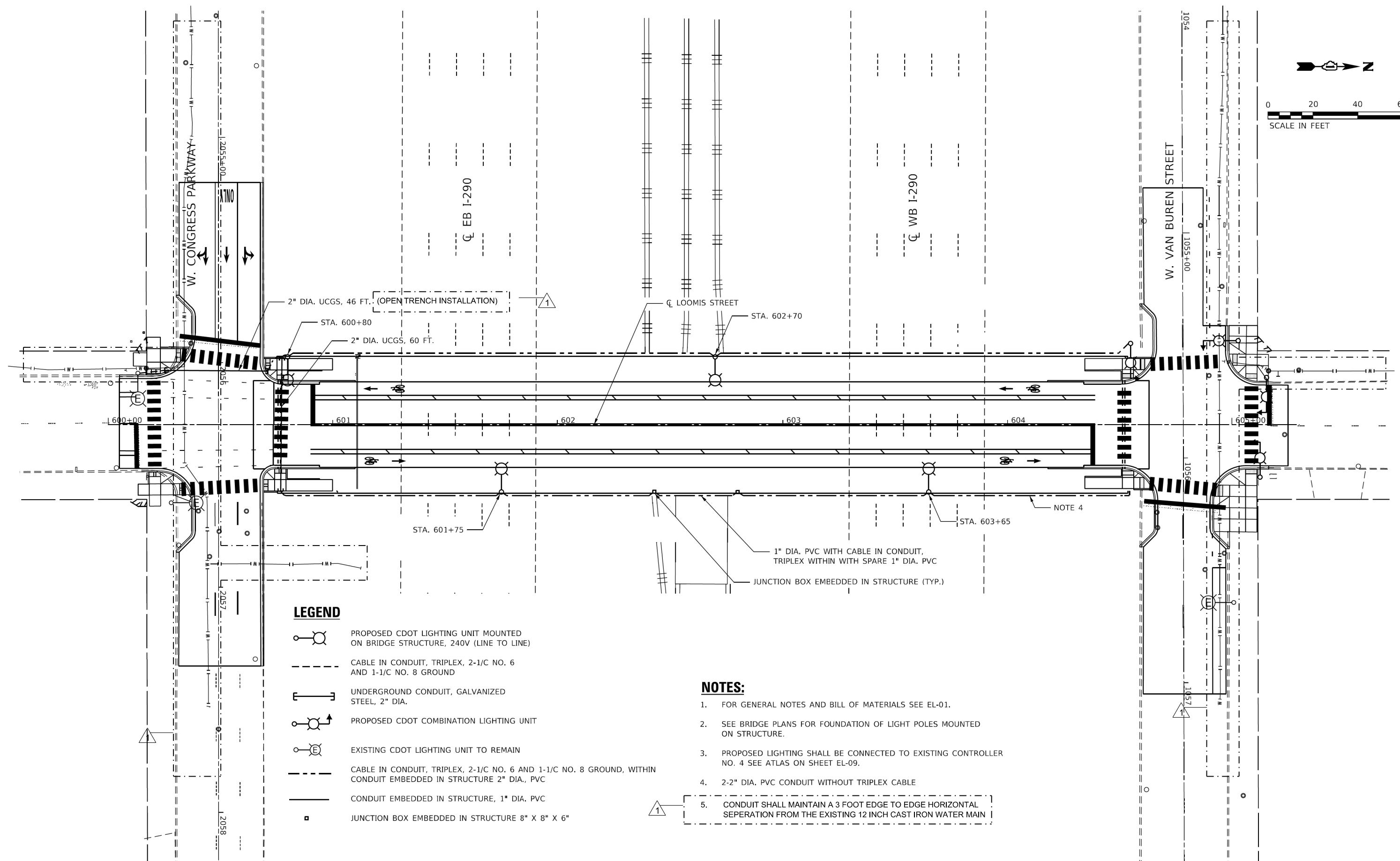
LEGEND

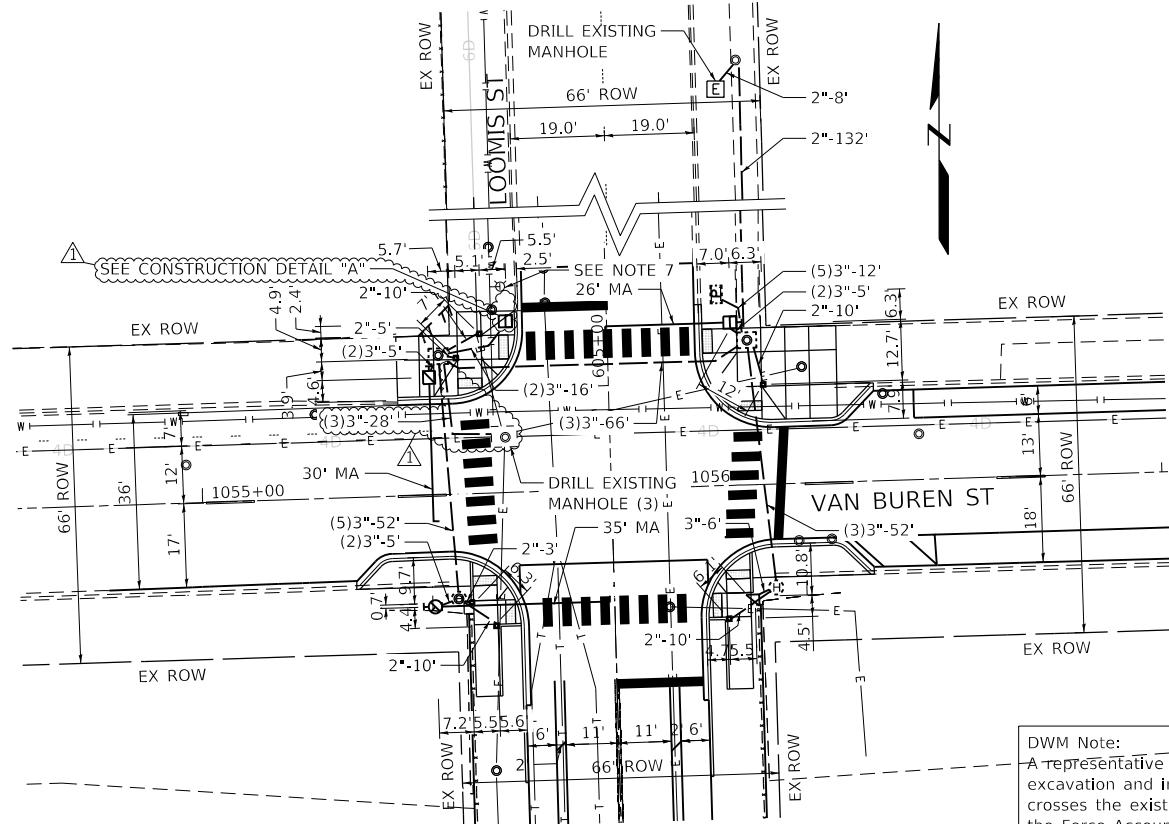
- CABLE IN CONDUIT, TRIPLEX, 2-1/C NO. 6 AND 1-1/C NO. 8 GROUND (CDOT)
- UNIT DUCT, 600V, 3-1C NO. 4, 1/C NO.6 GROUND, (XLP-TYPE USE), 1 1/4" DIA. POLYTHENE.
- EXISTING UNDERPASS LUMINAIRE TO BE REMOVED, NO SALVAGE (IDOT)
- PROPOSED LUMINAIRE, LED, UNDERPASS, SUSPENDED, OUTPUT DESIGNATION D (MIN. 7690 LUMENS), 240V (LINE TO NEUTRAL)
- A JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4" (IDOT)
- B JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 12" X 10" X 6" (IDOT)
- C JUNCTION BOX, STAINLESS STEEL ATTACHED TO STRUCTURE, 18" X 18" X 8" WITH OVERCURRENT PROTECTION (IDOT)
- (R) EXISTING CDOT LIGHTING UNIT TO BE REMOVED AND SALVAGED
- (L) PROPOSED CDOT LIGHTING UNIT MOUNTED ON BRIDGE STRUCTURE, 240V (LINE TO LINE)
- (C) PROPOSED CDOT COMBINATION LIGHTING UNIT
- (E) EXISTING CDOT LIGHTING UNIT TO REMAIN
- (E) EXISTING IDOT LIGHTING UNIT TO REMAIN
- CONDUIT ATTACHED TO STRUCTURE (ELECTRIC CABLE WITHIN) (IDOT)
- UNDERGROUND CONDUIT, GALVANIZED STEEL, 2" DIA. (CDOT)
- CABLE IN CONDUIT, TRIPLEX, 2-1/C NO. 6 AND 1-1/C NO. 8 GROUND, WITHIN CONDUIT EMBEDDED IN STRUCTURE 2" DIA., PVC

REVISED SHEET 12/30/2025

EL-01





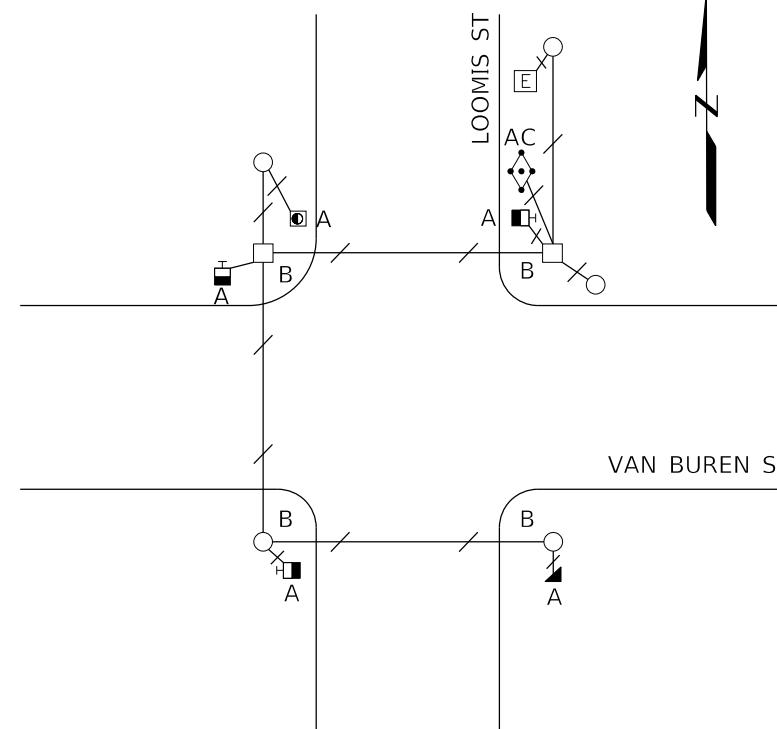


FOUNDATION AND CONDUIT PLAN

1" = 20

DWM Note:

- A representative of the DWM must be present during the excavation and installation of the proposed conduit where it crosses the existing 24-inch feeder main. It is required that the Force Account Construction Manager be contacted at FACM@dwmmpo.net two weeks prior to the anticipated construction date so a resident engineer can be assigned to the project. The DWM representative will adhere to the schedule provided by IDOT, unless notified otherwise. Failure to comply with these requirements may result in additional expenses to the proposed project to verify that all work conforms to DWM's standards.



FOUNDATION AND CONDUIT REMOVAL PLAN

NO SCALI

REMOVAL LEGEND

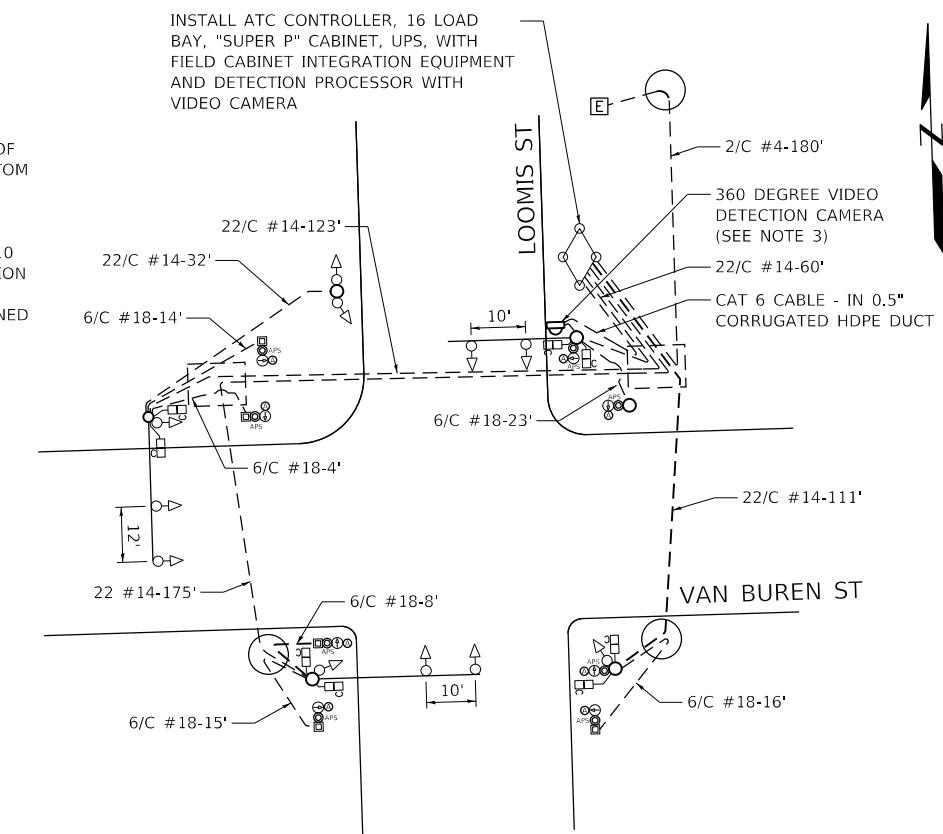
- A - REMOVE FOUNDATION
- B - REMOVE EXISTING HANDHOLE/MANHOLE
- C - REMOVE CONTROLLER AND CABINET
- D - REMOVE POST, JUNCTION BOX, SIGNAL HEAD, AND HARNESS CABLE
- E - REMOVE POLE, JUNCTION BOX, SIGNAL HEAD AND HARNESS CABLE
- - REMOVE EXISTING CABLE
- █ - EXISTING PEDESTRIAN SIGNAL
- - EXISTING TRAFFIC SIGNAL
- ↔ - ABANDON CONDUIT

NOTES

1. SEE STANDARD DRAWING 826 FOR CDOT TRAFFIC SIGNAL AND LIGHTING SYMBOLS.
2. TRAFFIC SIGNAL HEADS MOUNTED ON MAST ARMS SHALL BE A MINIMUM OF 15 FEET ABOVE THE TRAVELED ROADWAY WHEN MEASURED TO THE BOTTOM OF THE SIGNAL HOUSING.
3. THE LOCATION OF THE 360 DEGREE VIDEO DETECTION CAMERA SHALL BE CONFIRMED BY CDOT DEO.
4. IF TWO ACCESSIBLE PEDESTRIAN PUSHBUTTONS ARE PLACED LESS THAN 10 FEET APART OR PLACED ON THE SAME POLE, THE AUDIBLE WALK INDICATION SHALL BE A SPEACH WALK MESSAGE.
5. APS PUSHBUTTONS SHALL BE INSTALLED SO THE TACTILE ARROW IS ALIGNED PARALLEL TO THE DIRECTION OF TRAVEL.
6. ALL CONDUIT SHALL BE SCHEDULE 80.
7. PROTECT EXISTING FIRE HYDRANT.

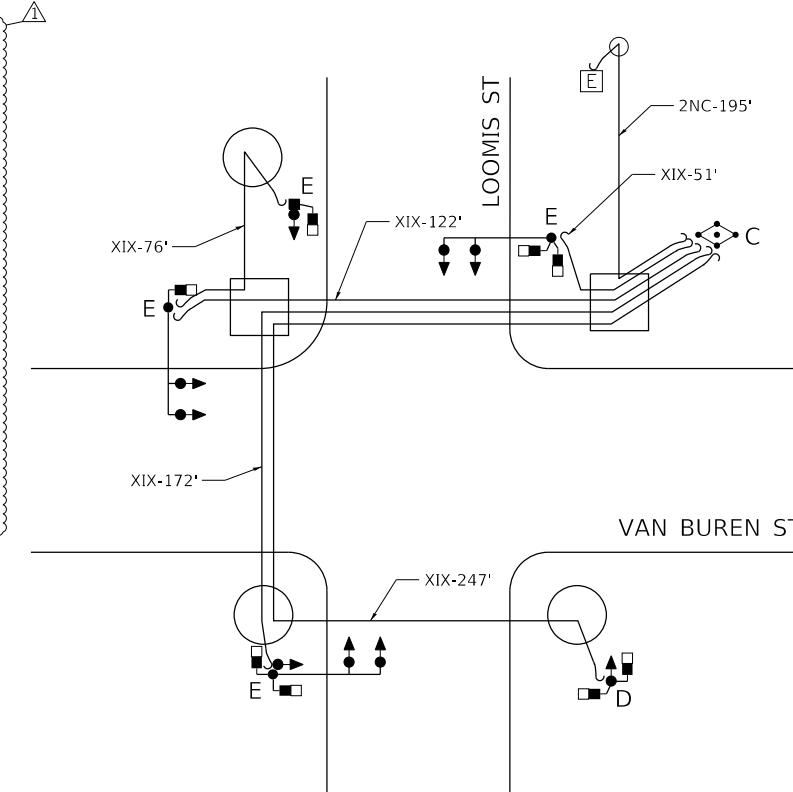
LEGEND

- ► EXISTING TRAFFIC SIGNAL HEAD
- ► PROPOSED TRAFFIC SIGNAL HEAD
- █ EXISTING COUNTDOWN PEDESTRIAN SIGNAL
- █ PROPOSED COUNTDOWN PEDESTRIAN SIGNAL
- █ PROPOSED PUSHBUTTON POST
- AP ➔ PROPOSED ACCESSIBLE PEDESTRIAN PUSH BUTTON
(PUSHBUTTON ARROW SHOULD BE INSTALLED PER DIRECTIONS ON SIGNAL AND CABLE PLAN)
- ⌚ PROPOSED 360 DEGREE VIDEO CAMERA



SIGNAL AND CABLE PLAN

NO SCALE



SIGNAL AND CABLE REMOVAL PLAN

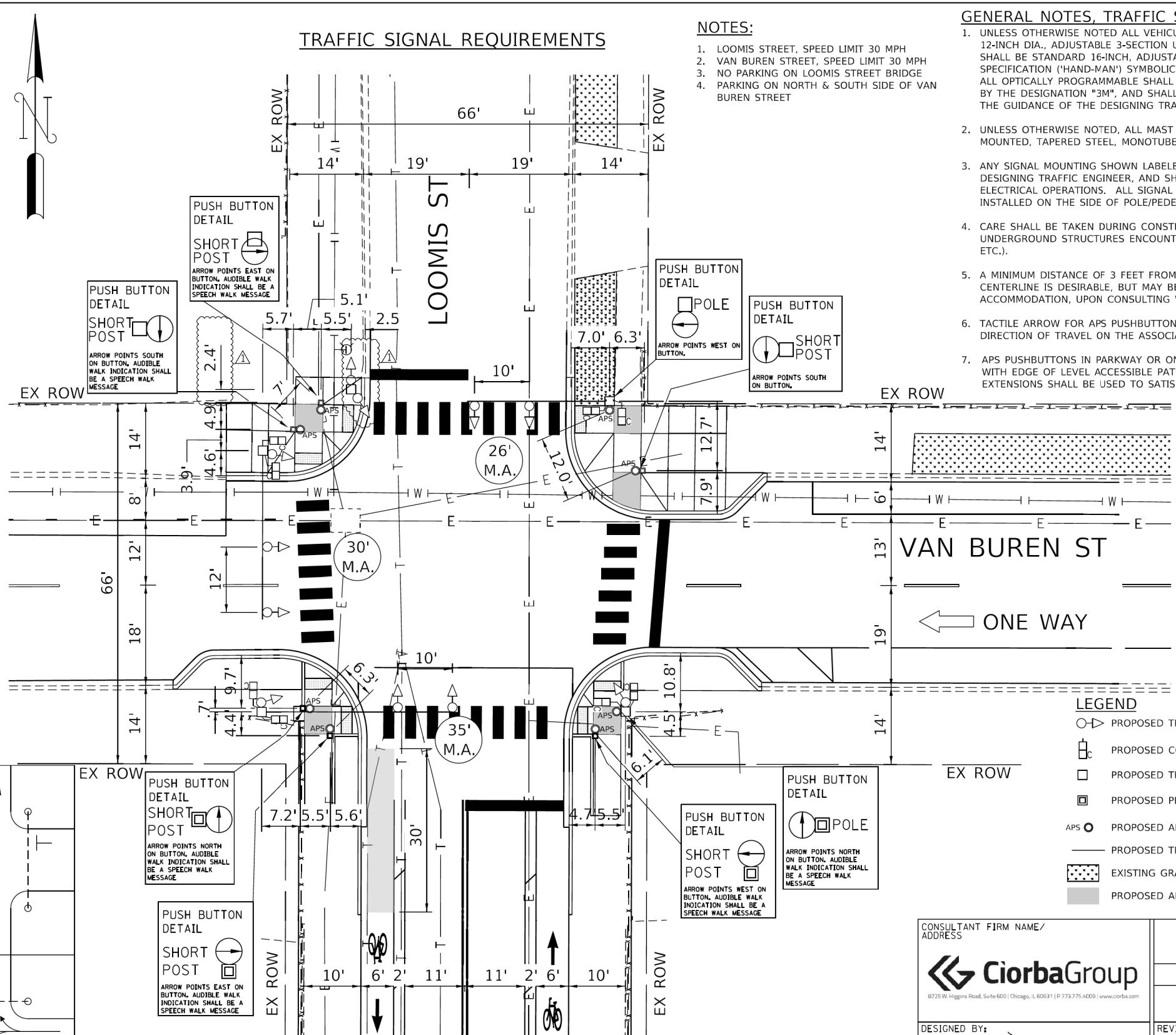
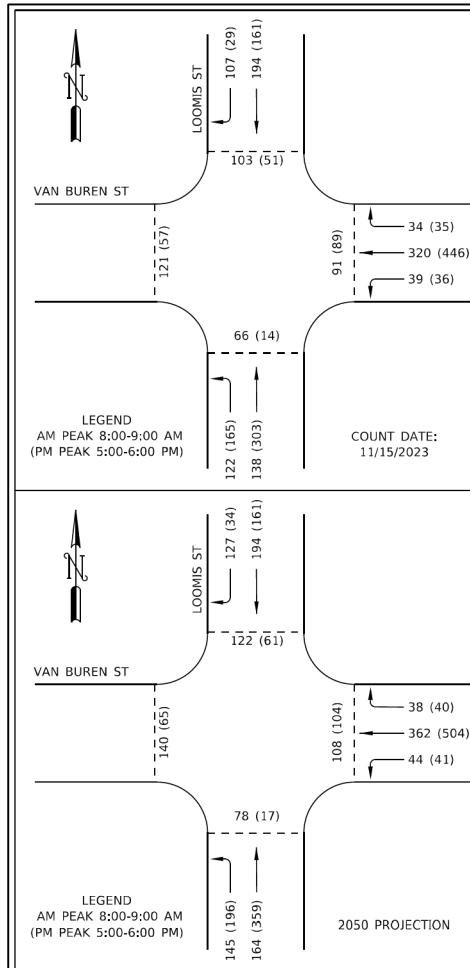
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REVISED SHEET 12/30/2025

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

LOOMIS STREET AT I-290 CDOT TRAFFIC SIGNAL PLAN

S. RTE.	SECTION	COUNTY	SHEETS	NO.	
					290
			CONTRACT NO.		62U12
	ILLINOIS	FED. AID PROJECT			



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

LOOMIS STREET AT I-290
PERMANENT TRAFFIC SIGNAL REQUIREMENTS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

LOOMIS STREET AT I-290
PERMANENT TRAFFIC SIGNAL REQUIREMENTS

SCALE: N.T.S.	sheet	of	sheets	sta.	to sta.	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
SCALE: N.T.S.	sheet	of	sheets	sta.	to sta.	290	FAI 290 22 BRIDGE 3	COOK	161	69
										CONTRACT NO. 62U12

GENERAL NOTES

All new structural steel shall be metallized. See Special Provision for "Metallizing of Structural Steel."

Calculated weight of Structural Steel = 1,004,800 lbs (Gr. 50)
Calculated weight of Structural Steel = 46,710 lbs (Gr. 36)

No field welding is permitted except as specified in the contract documents.

Fasteners shall be ASTM F3125 Grade A325 Type 1, hot-dip galvanized bolts in uncoated metallized areas. Fasteners shall be ASTM F3125 Grade A325 Type 1, mechanically galvanized bolts in coated metallized areas. Bolts $\frac{3}{8}$ in. diameter, holes $\frac{15}{16}$ in. diameter, unless otherwise noted. See special provisions for "Metallizing of Structural Steel."

Reinforcement bars designated (E) shall be epoxy coated.

Slipforming of the parapets is not allowed.

Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of $\frac{1}{8}$ in. (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.

Concrete Sealer shall be applied to the designated areas of the abutments and piers. The Concrete Sealer shall be a "film forming" type for horizontal surfaces.

Anti-Graffiti Coating shall be applied to the exposed surfaces of the abutments and piers.

Plan dimensions and details relative to the existing structure have been taken from existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity furnished at the unit price bid for the work.

The Protective Shield shall extend as shown on Sheet S-01 and as a minimum 2 ft beyond the existing edge of deck.

As a minimum, the following will be required:
2250 Sq. Yd. to remove the existing deck

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to address the presence of lead on this project.

INDEX OF SHEETS

S-01	General Plan and Elevation
S-02	General Notes and Total Bill of Material
S-03	General Data
S-04	Top of Slab Elevations Layout
S-05	Top of Slab Elevations - 1
S-06	Top of Slab Elevations - 2
S-07	Top of Slab Elevations - 3
S-08	Top of Slab Elevations - 4
S-09	Top of Slab Elevations - 5
S-10	Top of South Approach Slab Elevations
S-11	Top of North Approach Slab Elevations
S-12	Deck Plan and Cross Section
S-13	West Sidewalk and Parapet Details
S-14	East Sidewalk and Parapet Details
S-15	Architectural Parapet Details
S-16	Deck Diaphragm Details
S-17	Deck Miscellaneous Details
S-18	Partial Depth Precast Bridge Approach Slab (1 of 4)
S-19	Partial Depth Precast Bridge Approach Slab (2 of 4)
S-20	Partial Depth Precast Bridge Approach Slab (3 of 4)
S-21	Partial Depth Precast Bridge Approach Slab (4 of 4)
S-22	Bridge Fence Railing, Curved (1 of 2)
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S-24	Preformed Joint Strip Seal - Sidewalk (1 of 3)
S-25	Preformed Joint Strip Seal - Sidewalk (2 of 3)
S-26	Preformed Joint Strip Seal - Sidewalk (3 of 3)
S-27	CTA Station Joint Details
S-28	Framing Plan and Elevation
S-29	Girder Details
S-30	Field Splice and Diaphragm Details
S-31	Bearing Details
S-32	South Abutment Removal and Repair
S-33	North Abutment Removal and Repair
S-34	Abutment Seat Modifications
S-35	Wingwall Parapet Modifications
S-36	Pier 1 Removal and Repairs
S-37	Pier 2 Removal and Repairs
S-38	Pier 1 and Pier 2 Cap Modifications

For Existing Bridge Plans, see
Sheets SX-1 thru SX-8 immediately
following Sheet S-38.

SCOPE OF WORK

1. Relocate or Temporarily support existing utilities.
2. Remove existing superstructure and slabs over utility manhole behind abutment.
3. Fill abandoned utility manhole with granular backfill (see roadway plans).
4. Remove abutment backwalls and convert to semi-integral.
5. Remove Graffiti, as needed.
6. Repair existing substructure and apply concrete sealant to exposed surfaces along splashzones.
7. Construct proposed pier cap extensions.
8. Construct proposed superstructure.
9. Construct proposed approach slabs.
10. Utility work.
11. Reinstall sign structures.
12. Remove Graffiti, as needed.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Superstructures	Each	1		1
Concrete Removal	Cu. Yd.			127.0 127.0
Bridge Rail Removal	Foot			134 134
Protective Shield	Sq. Yd.	2,250		2,250
Structure Excavation	Cu. Yd.			128 128
Concrete Structures	Cu. Yd.	37.0	100.0	137.0
Bridge Deck Grooving	Sq. Yd.	1,472		1,472
Protective Coat	Sq. Yd.	2,776		2,776
Furnishing and Erecting Structural Steel	L Sum	1		1
Stud Shear Connectors	Each	7,410		7,410
Reinforcement Bars, Epoxy Coated	Pound	187,110	13,400	200,510
Bridge Fence Railing, Curved	Foot	705		705
Name Plates	Each	1		1
Preformed Joint Strip Seal	Foot	124		124
Elastomeric Bearing Assembly, Type I	Each	30		30
Anchor Bolts, 1"	Each	80		80
Granular Backfill For Structures	Cu. Yd.			128 128
Concrete Sealer	Sq. Ft.			2,493 2,493
Epoxy Crack Injection	Foot	60	60	60
Geocomposite Wall Drain	Sq. Yd.			82 82
Pipe Underdrains For Structures 4"	Foot			302 302
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq. Ft.			165 165
Expansion Joint (Special)	Foot	29		29
Graffiti Removal	Sq. Yd.			108 108
Anti-Graffiti Coating	Sq. Ft.			7,660 7,660
Concrete Wearing Surface, 5"	Sq. Yd.	413		413
Precast Bridge Approach Slab	Sq. Ft.			3,660 3,660
Bar Terminators	Each			272 272
Concrete Superstructure	Cu. Yd.			878.4 878.4

STA. 602+63.97
BUILT 20 BY
STATE OF ILLINOIS
F.A.I. RTE. 290 SEC. FAT 290 22 BRIDGE 3
LOADING HL-93
STR. NO. 016-2114

NAME PLATE

See Std. 515001

Note

Existing Name Plate shall be cleaned and relocated next to new Name Plate. Cost included with Name Plates.

REVISED SHEET 12/31/2025

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL NOTES AND TOTAL BILL OF MATERIAL
STRUCTURE NO. 016-2114

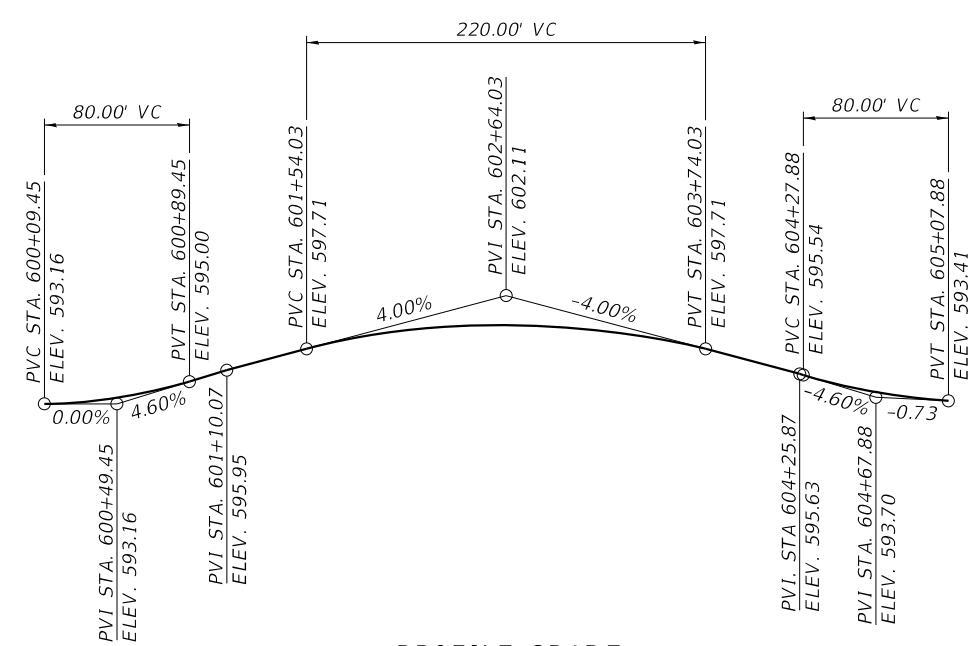
SHEET S-02 OF 38 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	FAT 290 22 BRIDGE 3	COOK	161	94
				CONTRACT NO. 62U12

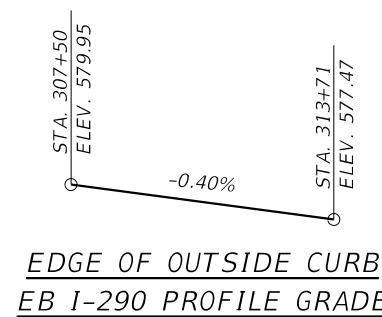
ILLINOIS FED. AID PROJECT

BLA, Inc.

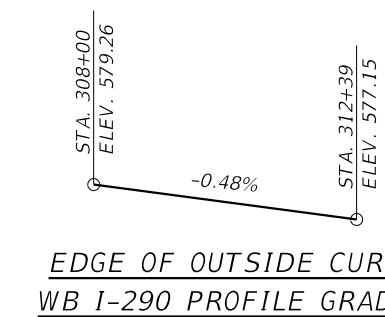
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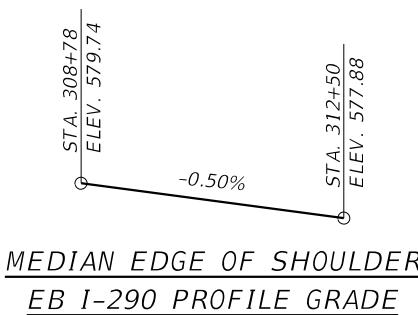
PROFILE GRADE
(Along Loomis Street)



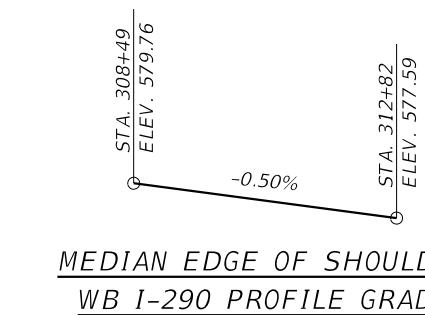
EDGE OF OUTSIDE CURB
EB I-290 PROFILE GRADE



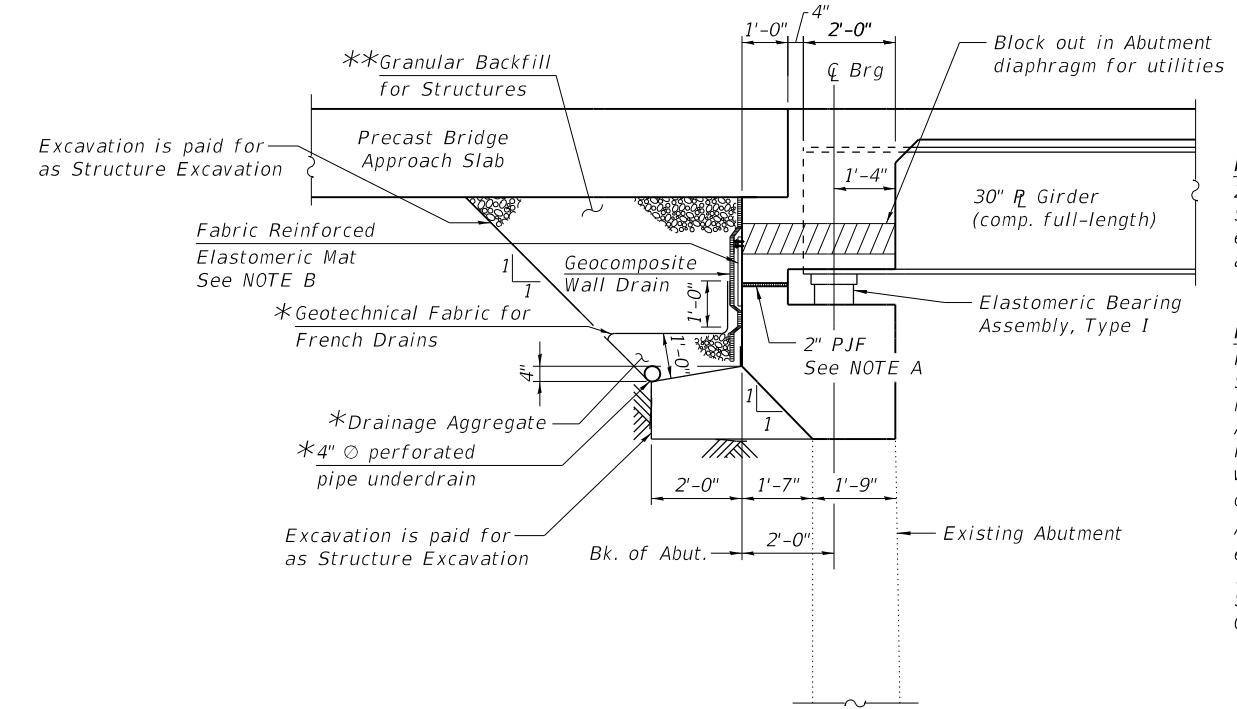
EDGE OF OUTSIDE CURB
WB I-290 PROFILE GRADE



MEDIAN EDGE OF SHOULDER
EB I-290 PROFILE GRADE



MEDIAN EDGE OF SHOULDER
WB I-290 PROFILE GRADE



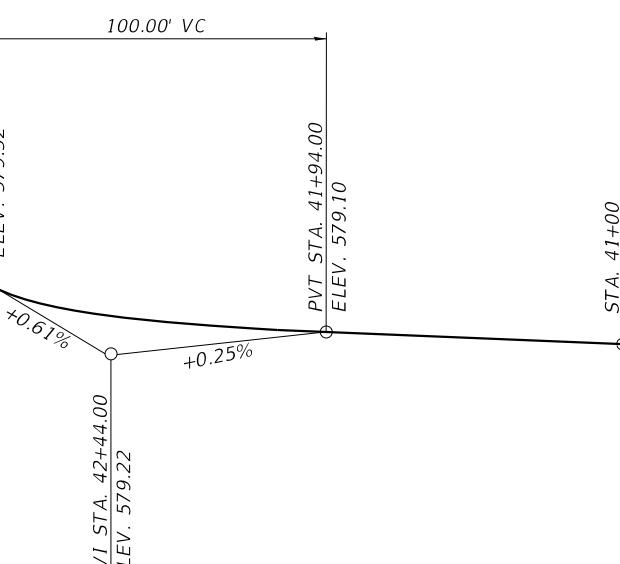
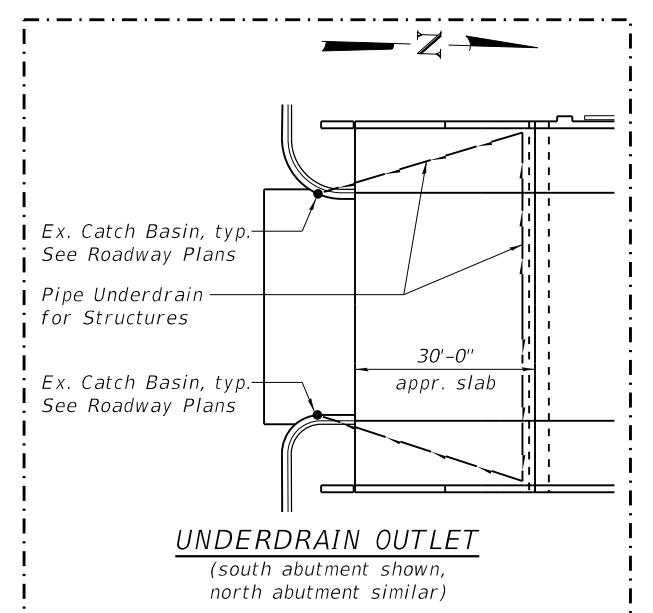
SECTION THRU SEMI-INTEGRAL ABUTMENT

* Included in the cost of Pipe Underdrains for Structures.

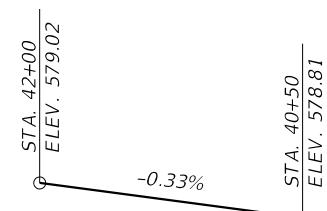
** Granular Backfill for Structures shall follow Standard Specification 586 except the coarse aggregate shall be grade CA7, CA 11, or CA 14.

Note:

Connect the pipe underdrain to the existing catch basin located off the approach slab on each side of the roadway.



TOP OF RAIL
(Track C)



Top of Rail
(Track D)

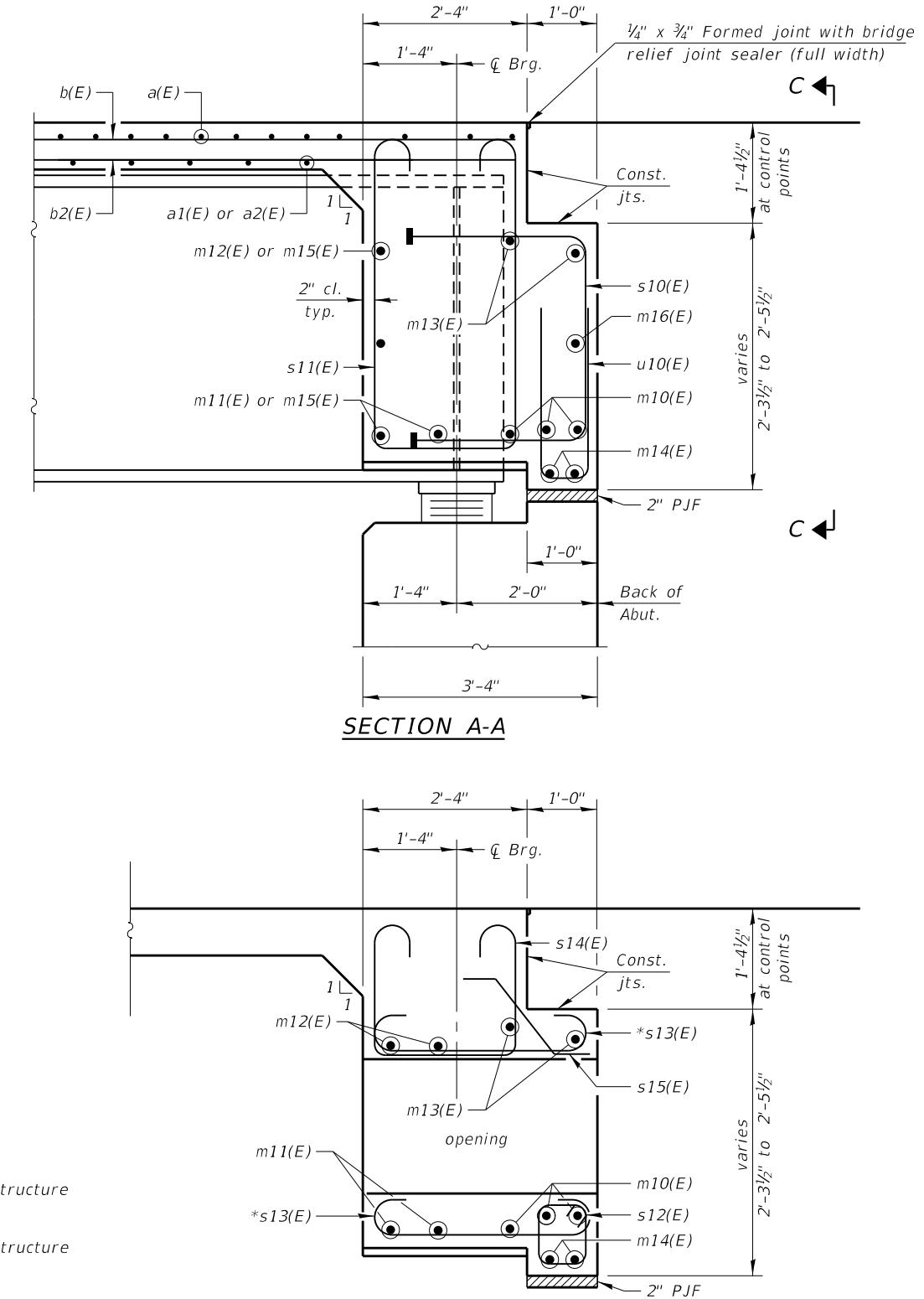
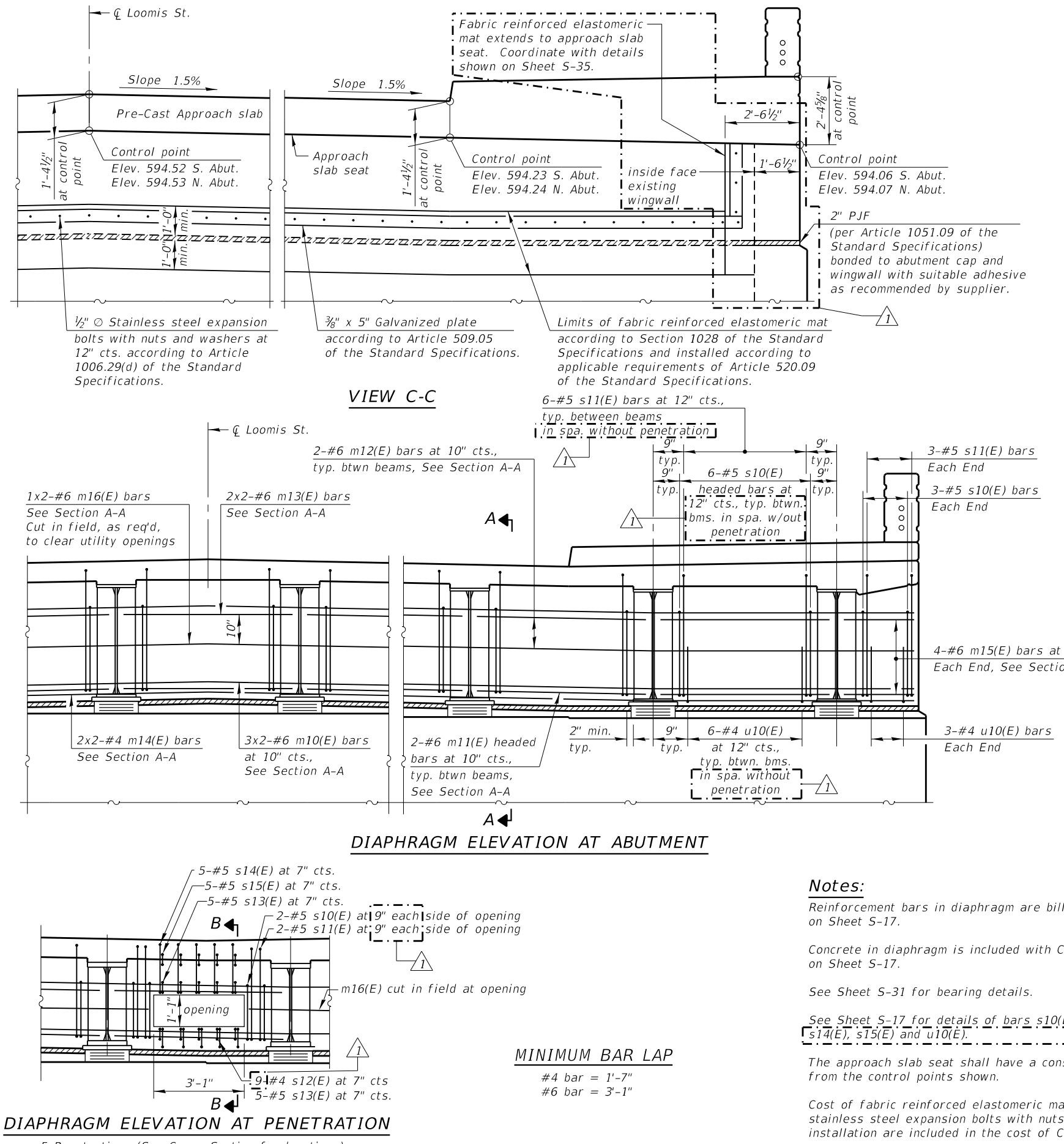
1 REVISED SHEET 12/31/2025

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL DATA
STRUCTURE NO. 016-2114

SHEET S-03 OF 38 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	FAI 290 22 BRIDGE 3	COOK	161	95
				CONTRACT NO. 62U12



* Tilt s13(E) bars as needed to maintain clearances.

REVISED SHEET 12/31/2025

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DECK DIAPHRAGM DETAILS
STRUCTURE NO. 016-2114



BLA, Inc.

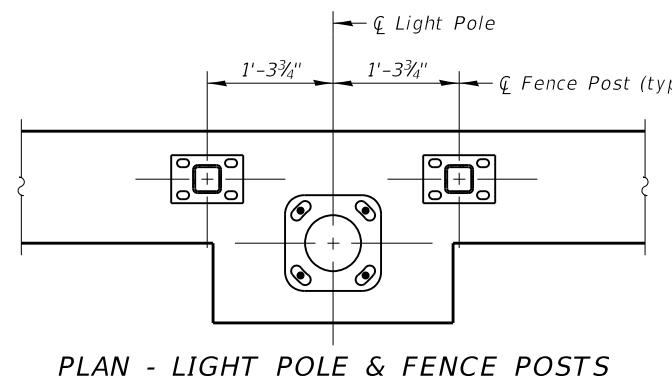
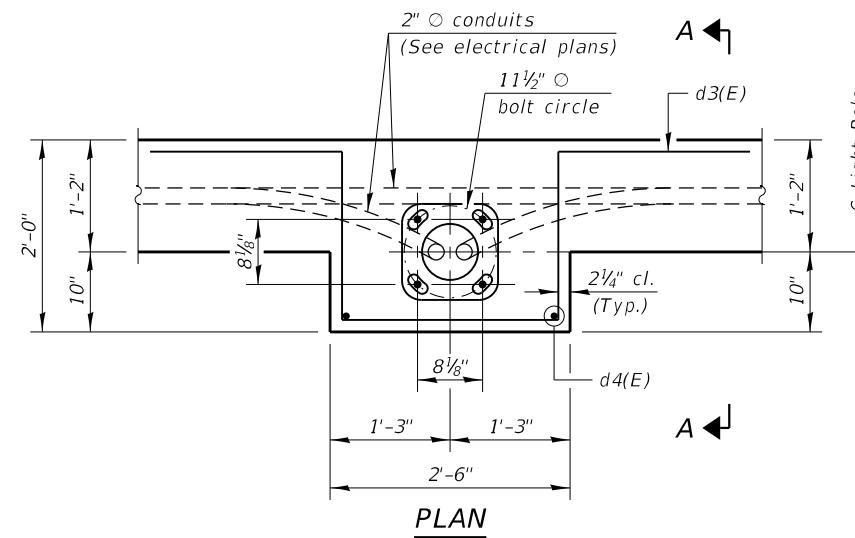
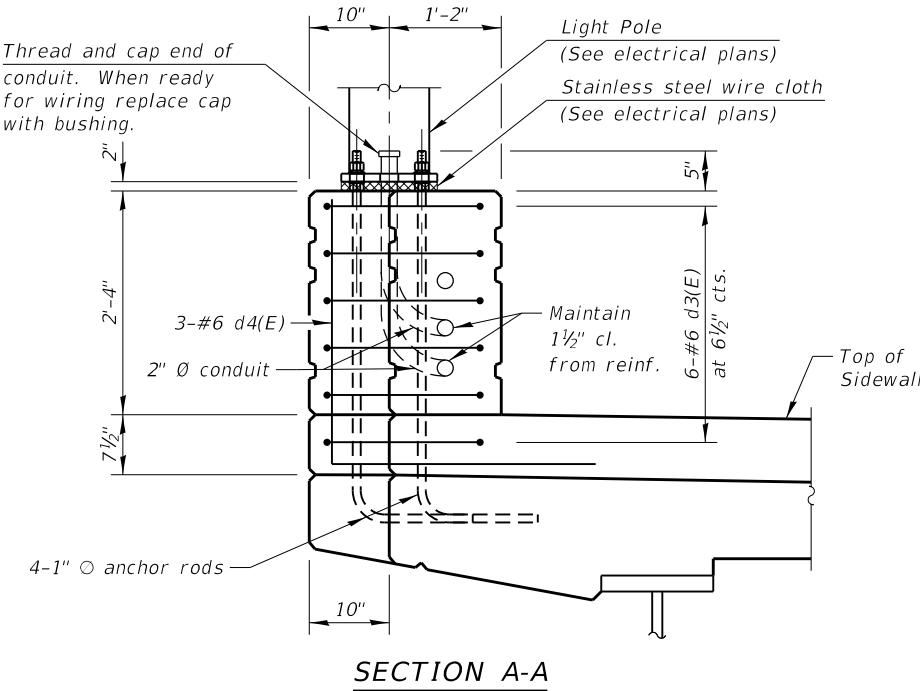
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PLOT DATE = 11/5/2025	CHECKED - PRD	REVISED -

F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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		ILLINOIS	FED. AID PROJECT	CONTRACT NO. 62U12

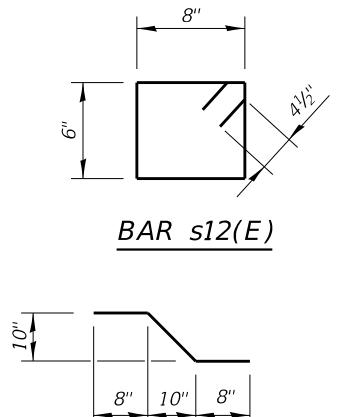
BILL OF MATERIAL

BAR	NO.	SIZE	LENGTH	SHAPE
a(E)	1050	#5	32'-6"	---
a1(E)	362	#5	29'-6"	---
a2(E)	362	#5	35'-6"	---
b(E)	1001	#5	32'-0"	---
b1(E)	372	#8	27'-0"	---
b2(E)	720	#5	29'-6"	---
c(E)	618	#5	2'-4"	l
c1(E)	618	#5	11'-6"	l
d(E)	608	#4	5'-4"	Y
d1(E)	608	#6	4'-6"	Y
d2(E)	130	#4	2'-3"	Y
d3(E)	24	#6	9'-3"	Y
d4(E)	12	#6	5'-5"	Y
e(E)	128	#4	19'-2"	---
e1(E)	64	#4	15'-2"	---
e2(E)	40	#4	17'-8"	---
e3(E)	8	#4	13'-8"	---
e4(E)	8	#4	13'-10"	---
e5(E)	8	#4	15'-9"	---
e6(E)	8	#4	16'-8"	---
m10(E)	12	#6	32'-0"	---
m11(E)	36	#6	5'-9"	---
m12(E)	36	#6	5'-9"	---
m13(E)	8	#6	32'-3"	---
m14(E)	8	#4	32'-0"	---
m15(E)	16	#6	2'-5"	---
m16(E)	4	#6	32'-3"	---
s10(E)	100	#5	7'-9"	---
s11(E)	100	#5	9'-2"	---
s12(E)	90	#4	3'-1"	---
s13(E)	100	#5	4'-2"	---
s14(E)	50	#5	6'-1"	---
s15(E)	50	#5	2'-6"	---
u10(E)	60	#4	3'-8"	---
Reinforcement Bars, Epoxy Coated			Pound	166,400
Concrete Superstructure			Cu. Yd.	822.0
Bridge Deck Grooving			Sq. Yd.	1232
Protective Coat			Sq. Yd.	2322

Notes:
Bar terminators, paid for separately.
See Total Bill of Material on Sheet S-02.

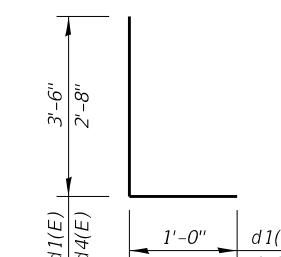
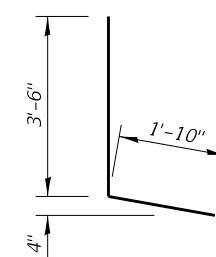
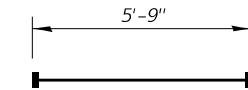
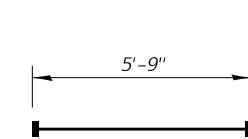


LIGHT POLE PEDESTAL DETAILS

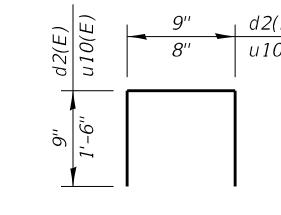


BAR s15(E)

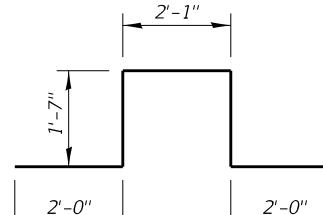
BAR m11(E)
(Headed. 72-#6 Bar terminators)



BARS d1(E)
& **d4(E)**



BARS d2(E)
& **u10(E)**



BAR d3(E)

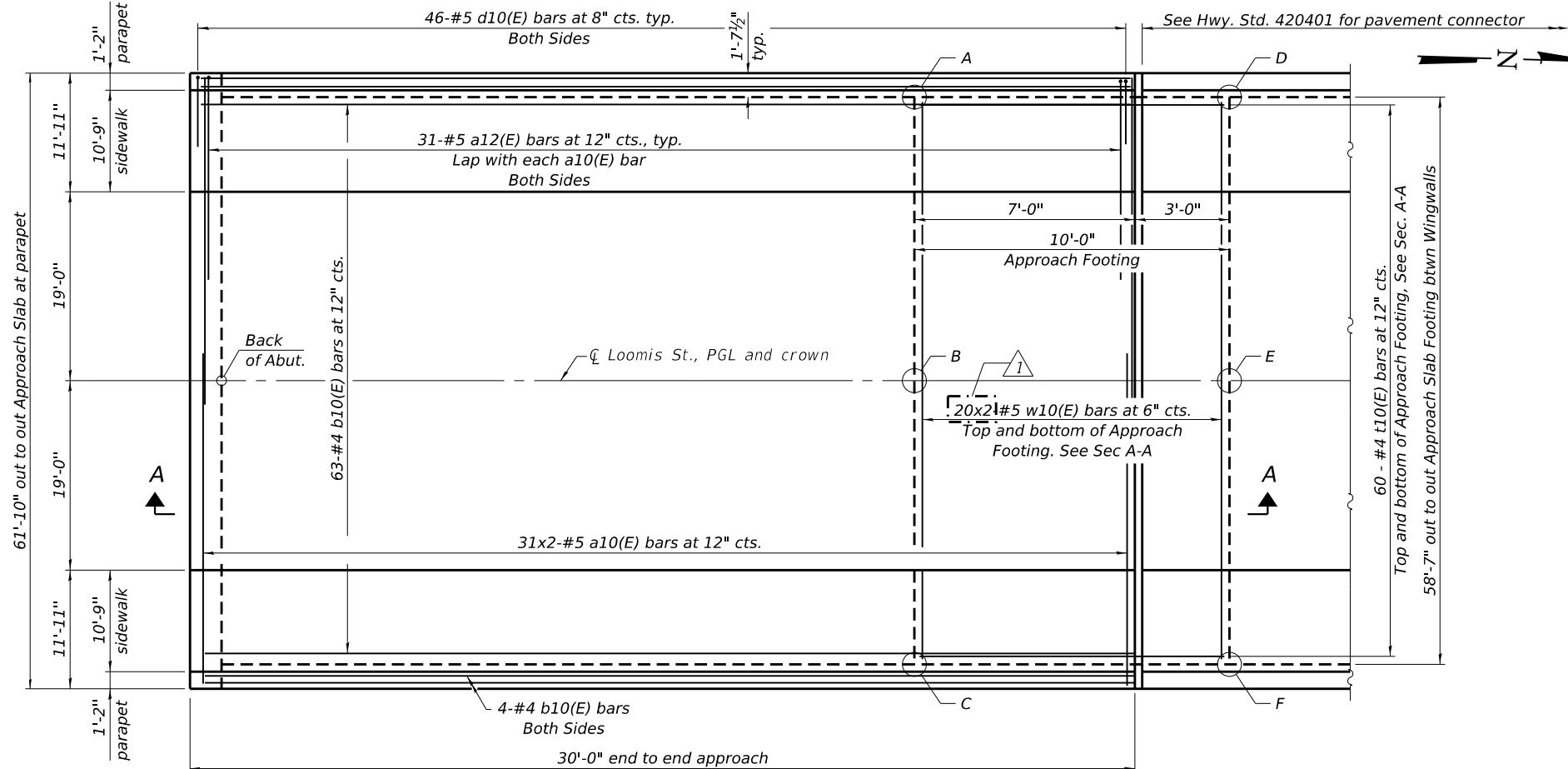
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

DECK MISCELLANEOUS DETAILS STRUCTURE NO. 016-2114

SHEET S-17 OF 38 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	F.A.I. 290 22 BRIDGE 3	COOK	161	109
				CONTRACT NO. 62U12

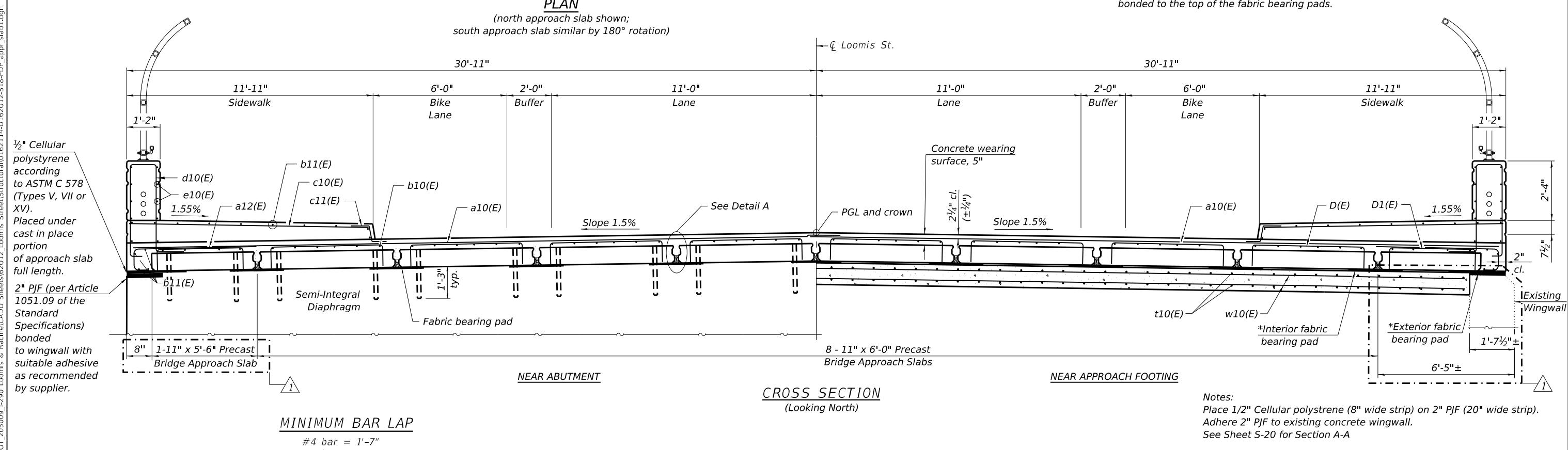
ILLINOIS FED. AID PROJECT



TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING

South Approach			North Approach		
Point/ Location	Top	Bottom	Point/ Location	Top	Bottom
A -	593.24	592.41	A -	593.37	592.54
B -	593.53	592.69	B -	593.65	592.82
C -	593.24	592.41	C -	593.37	592.54
D -	592.84	592.01	D -	592.97	592.14
E -	593.13	592.29	E -	593.25	592.42
F -	592.84	592.01	F -	592.97	592.14

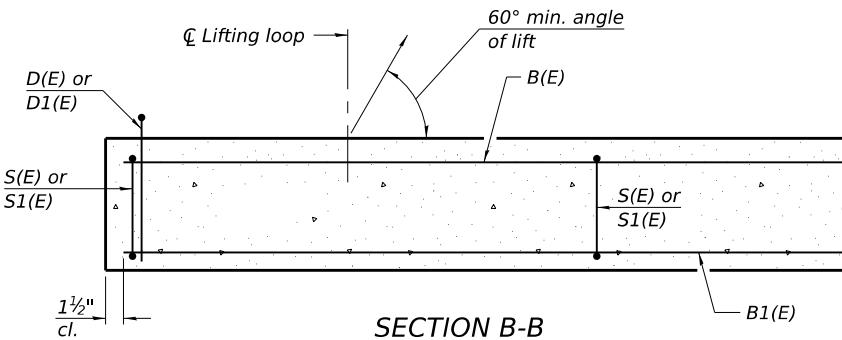
* Fabric bearing pads at the expansion end shall be recessed $\frac{1}{4}$ " into the approach footing and bonded. Adjusting shims, when required, shall be bonded to the top of the fabric bearing pads.



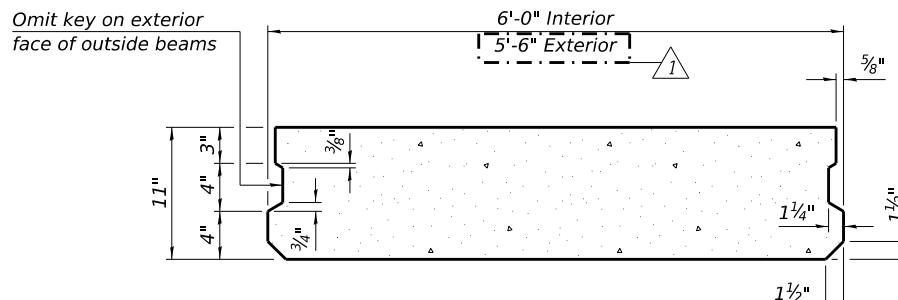
Notes:
Place 1/2" Cellular polystyrene (8" wide strip) on 2" PJF (20" wide strip).
Adhere 2" PJF to existing concrete wingwall.
See Sheet S-20 for Section A-A.

(Sheet 1 of 4)

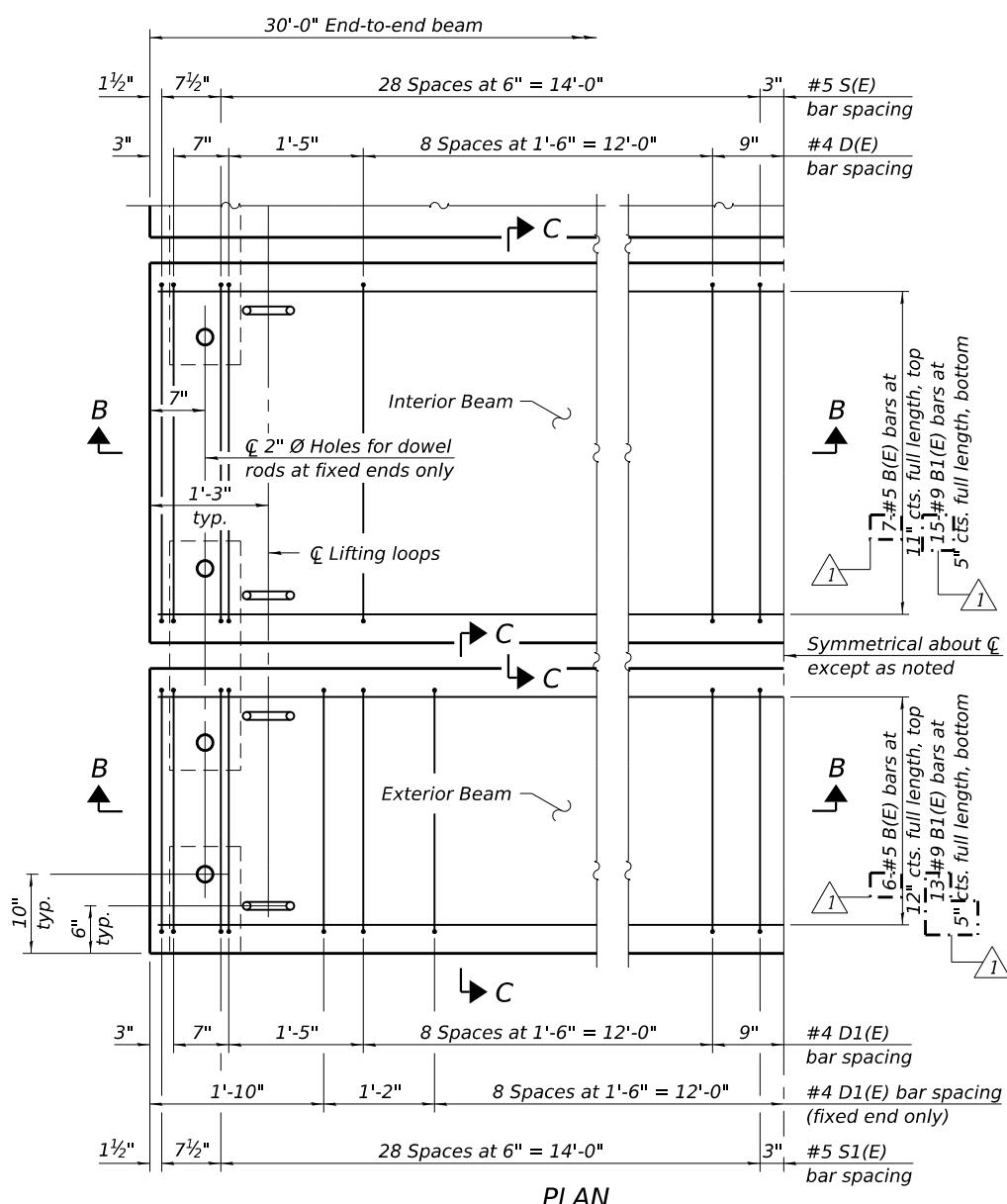
1 REVISED SHEET 12/31/2025



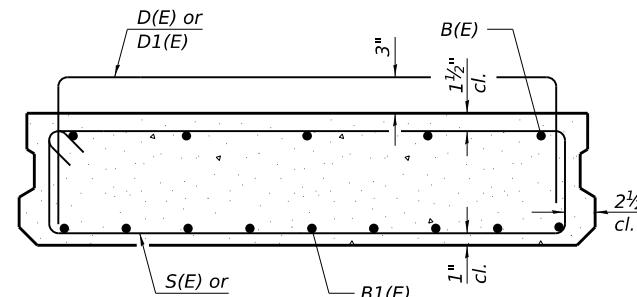
SECTION B-B



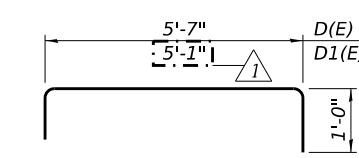
SECTION C-C
(Showing dimensions)



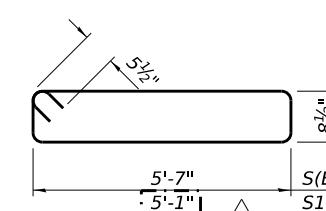
(Beams: 36" min. width; 72" max. width)



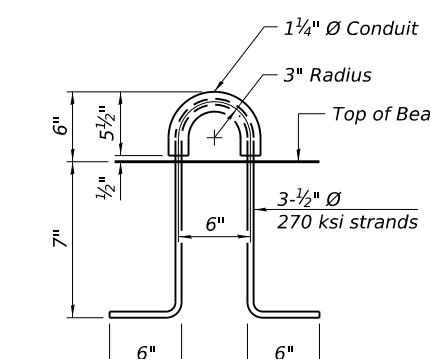
SECTION C-C
(Showing reinforcement)



BARS D(E) & D1(E)



BARS S(E) & S1(E)



LIFTING LOOP DETAIL

(An alternate lifting loop with a Safe Working Load of 6,250 lbs.
(25,000 lbs. Proof Load / Factor of Safety of 4) and utilized according
to the manufacturer's recommendations may be used.)

Notes:

The precast bridge approach slab shall be according to Section 504 of the Standard Specifications and shall be paid for at the contract unit price per square foot for Precast Bridge Approach Slab.

Cast-in-place substitution of Precast Bridge Slab is not allowed.

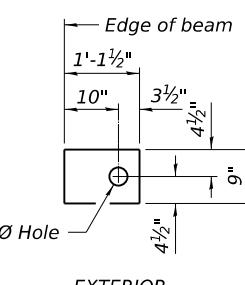
The top surface of precast bridge approach slabs shall be finished similar to precast prestressed deck beams with concrete wearing surface as specified in the IDOT "Manual for Fabrication of Precast Prestressed Concrete Products."

Two 1/2" fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location. Cost included with Precast Bridge Approach Slab.

A minimum 2 1/2" Ø lifting pins shall be used to engage the lifting loops during handling.

Compressive strength of precast concrete, $f'c$ shall be 6,000 psi.

Compressive strength of precast concrete during initial lifting, $f'ci$ shall be 5,000 psi.



FABRIC BEARING PAD
Notes:
Bearing pads at fixed end shall be 1/2" thick and
bearing pads at expansion end shall be 3/4" thick.
Omit holes for fabric bearing pads at approach
slab footing end of beams.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PARTIAL DEPTH PRECAST BRIDGE APPROACH SLAB
STRUCTURE NO. 016-2114

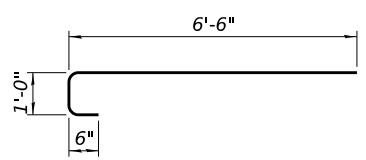
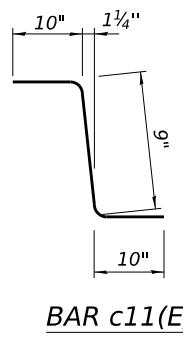
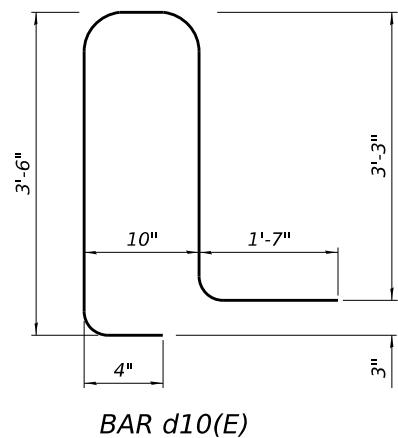
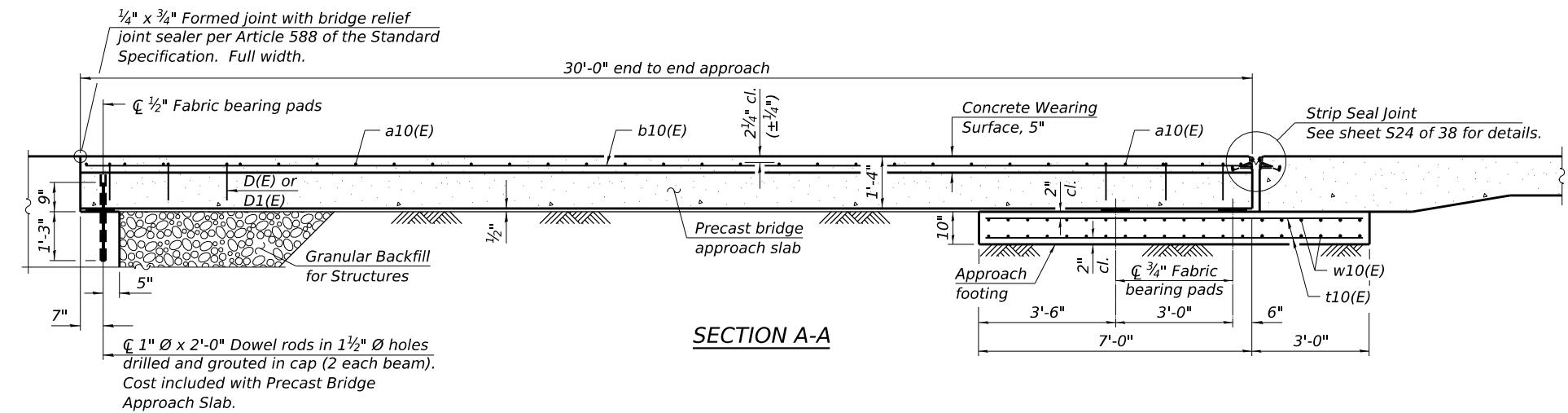
REVISED SHEET 12/31/2025

(Sheet 2 of 4)

BLA, Inc.

USER NAME = idot	DESIGNED - TB	REVISED - 10/24/2025 TB
CHECKED - PRD	REVISED -	
PLOT SCALE = NONE	DRAWN - TB	REVISED -
PLOT DATE = 10/31/2025	CHECKED - PRD	REVISED -

F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	F.A.I. 290 22 BRIDGE 3	COOK	161	111
				CONTRACT NO. 62U12



Notes:

The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.

After precast bridge approach slabs have been erected, holes shall be drilled into abutment and anchor dowels placed. Dowel holes shall be filled with non-shrink grout to top of precast slab and cured according to Article 1020.13(a)(3) or 1020.13(a)(5) of the Standard Specifications for a minimum of 24 hours before casting the shear keys and wearing surface.

Any concrete poured monolithically with the wearing surface, such as curbs, shall not be paid for separately, but will be included in the cost of Concrete Wearing Surface, 5".

Parapet concrete shall be paid for as Concrete Superstructure.

Approach footing concrete shall be paid for as Concrete Structures.

The approach footing maximum applied service bearing pressure (Omax) = 2.0 ksf.

Cost of excavation for approach footing included with Concrete Structures.

For Granular Backfill for Structures and drainage treatment details, see sheet S-03.

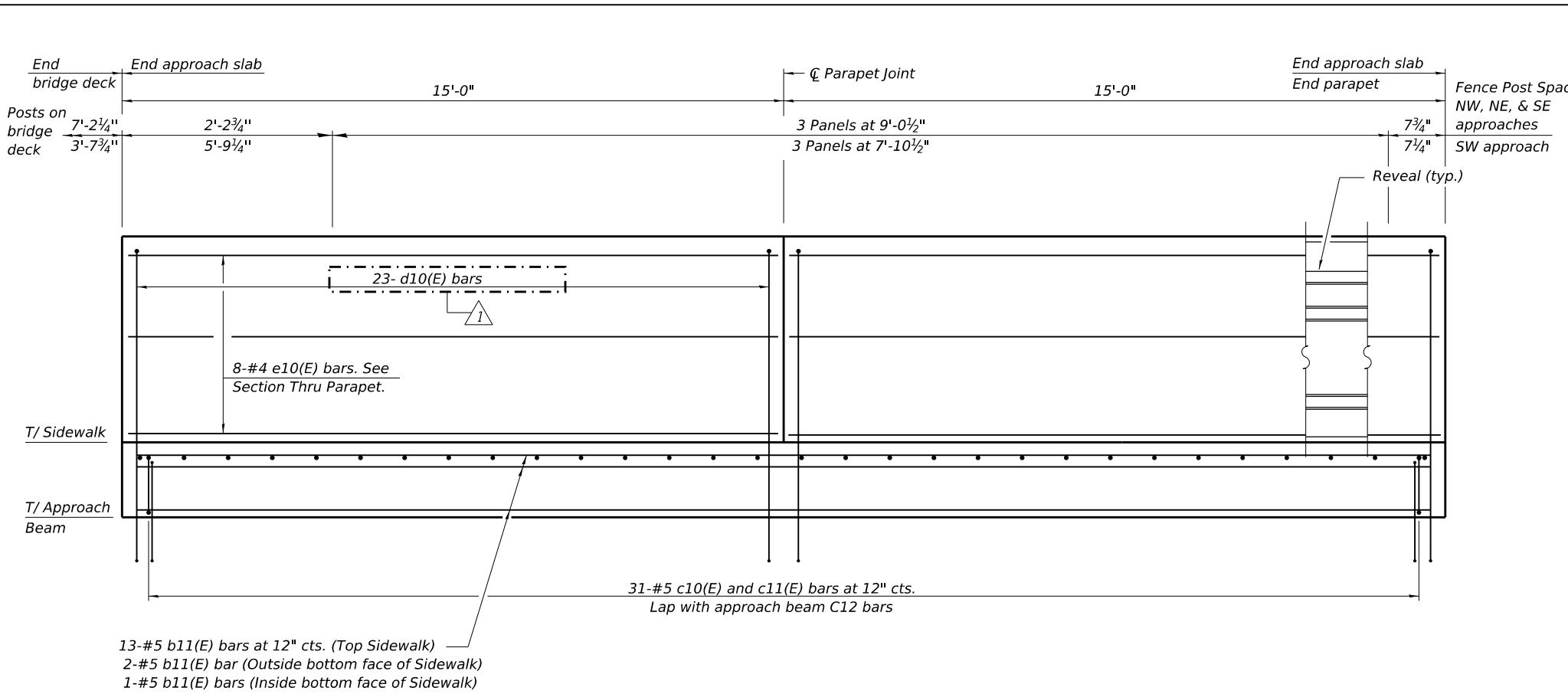
Cost of cellular polystyrene is included with Concrete Superstructure.

TWO APPROACHES
BILL OF MATERIALS

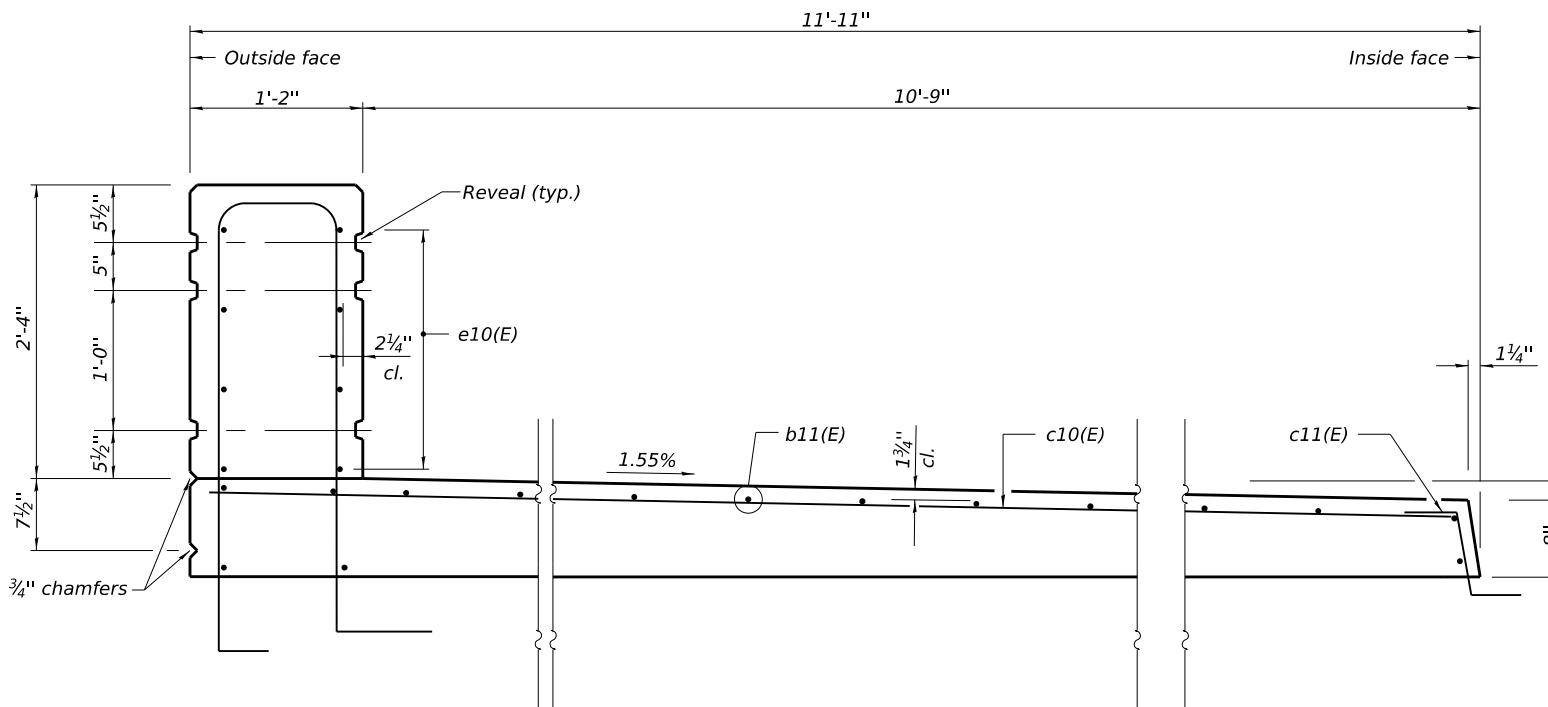
BAR	NO.	SIZE	LENGTH	SHAPE
a10(E)	124	#5	31'-9"	—
a12(E)	124	#5	8'-0"	□
b10(E)	142	#4	29'-6"	—
b11(E)	64	#5	29'-8"	—
c10(E)	124	#5	11'-7"	—
c11(E)	124	#5	2'-5"	□
d10(E)	184	#5	9'-6"	□
e10(E)	64	#4	14'-8"	—
t10(E)	240	#4	9'-8"	—
w10(E)	160	#5	29'-10"	—
Concrete Superstructure		Cu. Yd.	51.9	
Concrete Structures		Cu. Yd.	37.0	
Reinforcement Bars, Epoxy Coated		Pound	20,710	
Precast Bridge Approach Slab		Sq. Ft.	3,660	
Concrete Wearing Surface, 5"		Sq. Yd.	413	
Bridge Deck Grooving		Sq. Yd.	240	
Protective Coat		Sq. Yd.	454	

(Sheet 3 of 4)

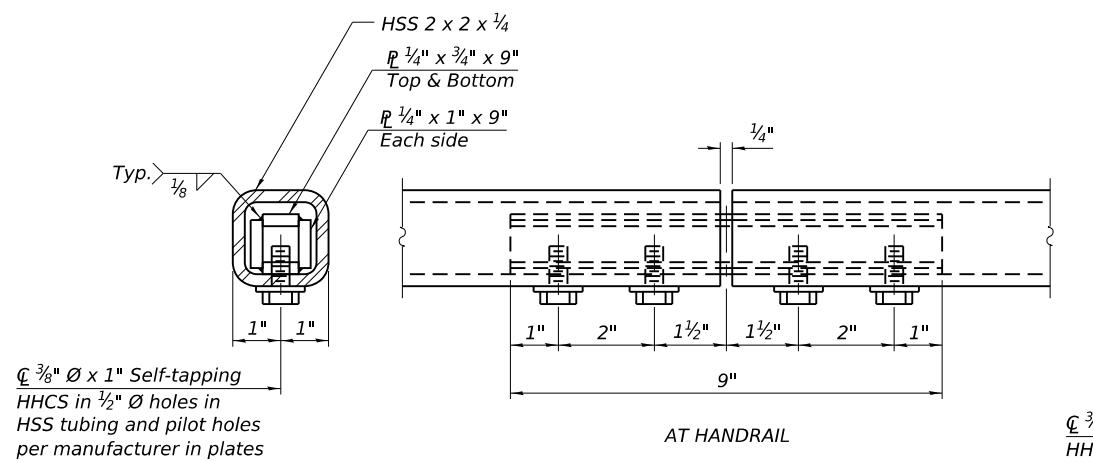
1 REVISED SHEET 12/31/2025



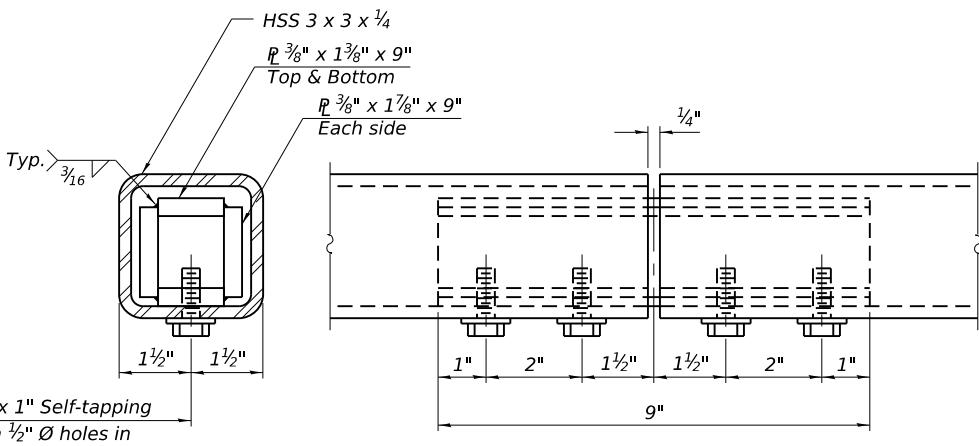
INSIDE ELEVATION OF PARAPET
SECTION C-C



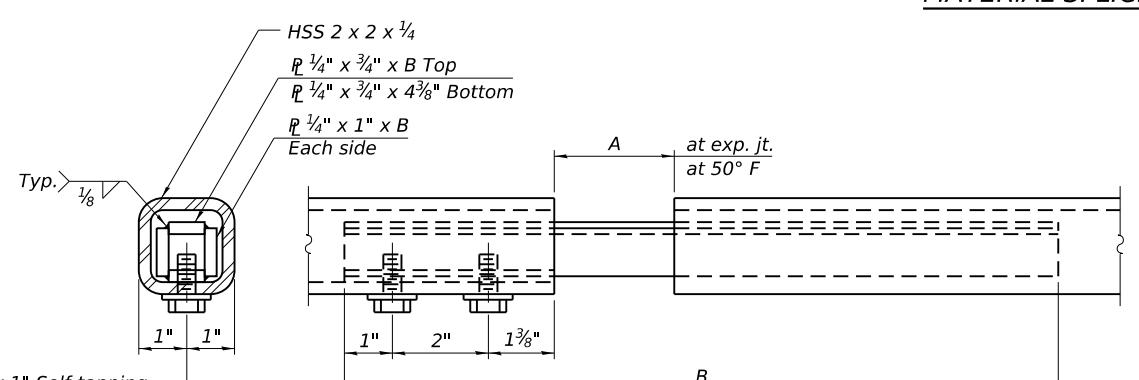
SECTION THRU SIDEWALK AND PARAPET



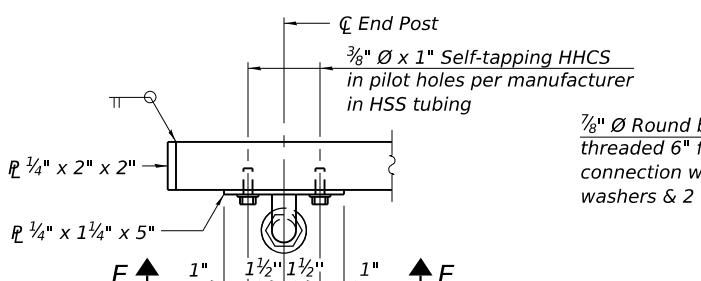
MATERIAL SPLICE



AT RAIL



EXPANSION SPLICE
(Weld bottom plate on bolt side)

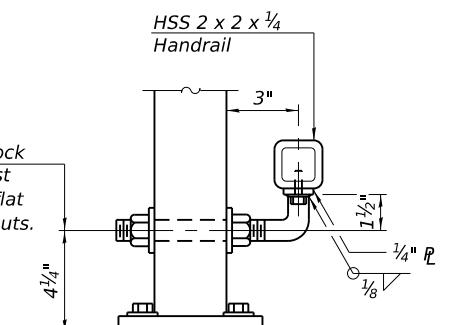


HANDRAIL DETAIL

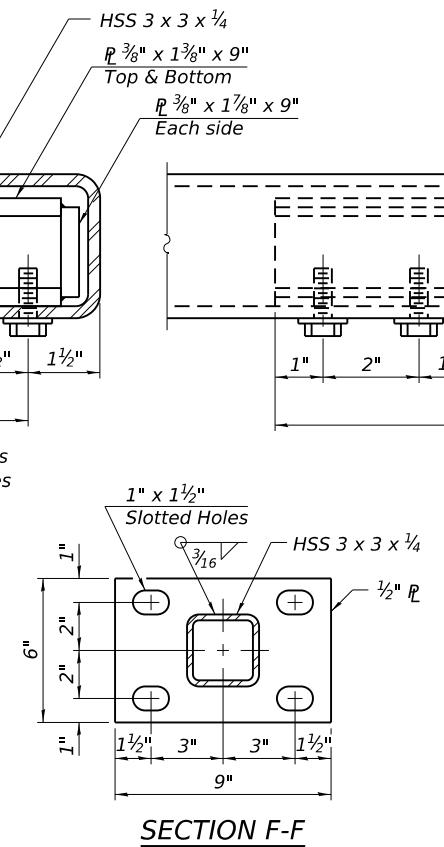
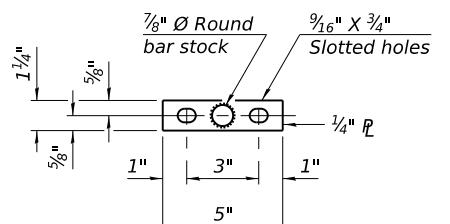
Location	T	A	B	C	D
Over Strip Seal Jt.	≤4"	2 1/2"	1'-5 7/8"	6 1/4"	3 3/4"
Over Finger or Modular Jt.	≤9 1/2"	5 1/2"	1'-11 5/8"	9 3/4"	6"
Over Finger or Modular Jt.	≤15"	8 1/4"	2'-5 1/8"	13 7/8"	8 3/4"
At Light Poles	8"	NA	NA	11 3/4"	10"
At Bridge Relief Joints	NA	Material Splices Per Fabricator			

T= ; total movement based on total temperature range from -20°F to 120°F along centerline of roadway at expansion joint.

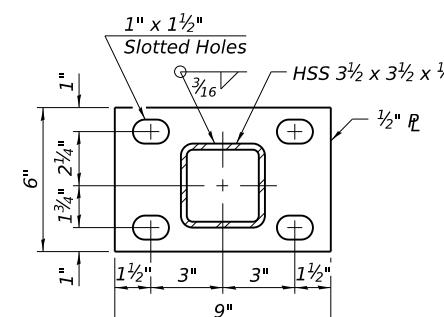
OR
8" hand-hole access for maintenance of light poles (when present;
replace splice with 8" gap)



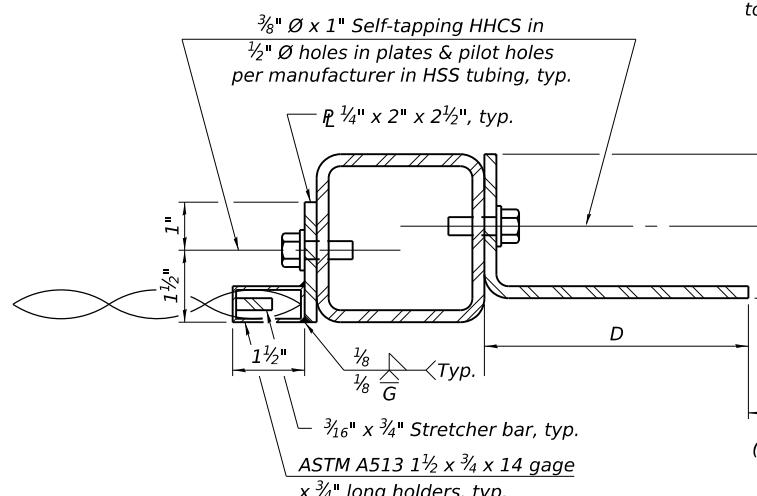
VIEW E-E
(Handrail)



SECTION F-F



SECTION G-G



SECTION B-B

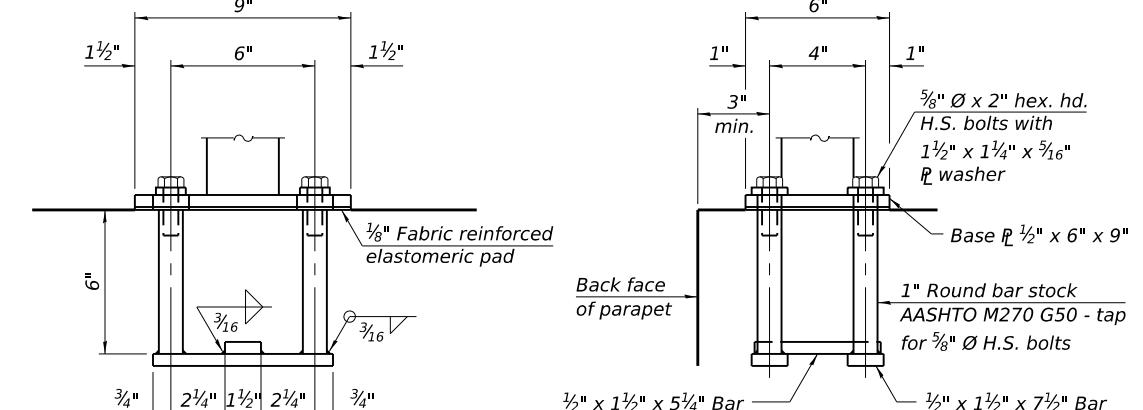
Notes:
Place reinforcement bars to miss anchor rod locations.
CVN testing is not required for the HSS tubing used in the Bridge Fence Railing, Curved.
All HSS tubing used for the Handrail shall be CVN tested according to Article 1006.34(b) of the Standard Specifications.
All heavy hex nuts shall be according to ASTM A 563 grade DH.
All fully threaded anchor rods shall be ASTM F1554 grade 105.
The post base plate shall be fastened to the curb snug tight and given an additional 1/8" turn.

Contractor shall offset posts to miss any aluminum sheet or cork joints for sufficient installation of the anchorage assembly.
Rail splice inserts may be built out of bent plates of the same thicknesses and outside geometry limits as the 4 plate rail splice inserts shown.

When the contract specifies a galvanized railing, all steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications. When the contract specifies a painted railing, all posts, rail, splices, anchor devices and plates of the railing shall be painted according to the paint system for railings as specified in the General Notes.

See sheet 5-24 for dimensions of concrete openings at expansion joints.

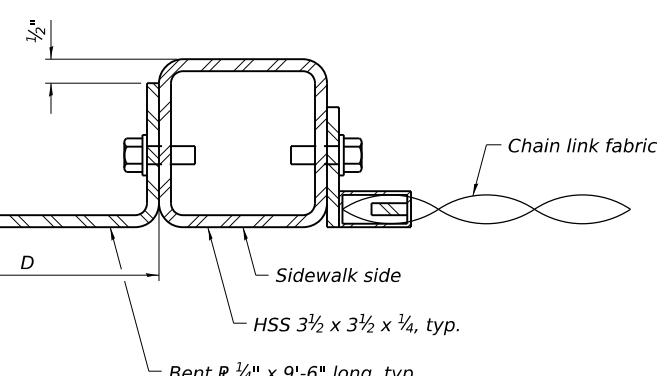
The railing shall be galvanized.



ANCHORAGE ASSEMBLY

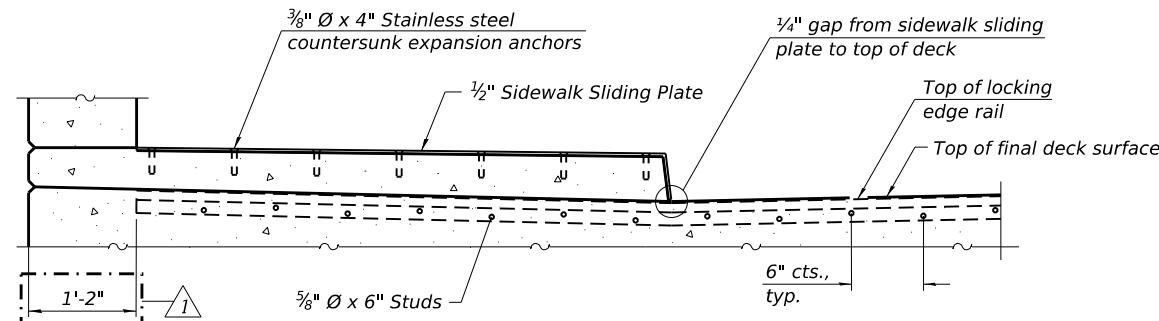
The Bridge Fence Railing, Curved fasteners for end posts near expansion joints may need to be installed prior to installing the bent plates.

In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and setting 5/8" Ø fully threaded anchor rods with the same plate washers as specified above and heavy hex lock nuts according to Article 509.06 of the Standard Specifications. Embedment shall be according to the manufacturer's specifications.

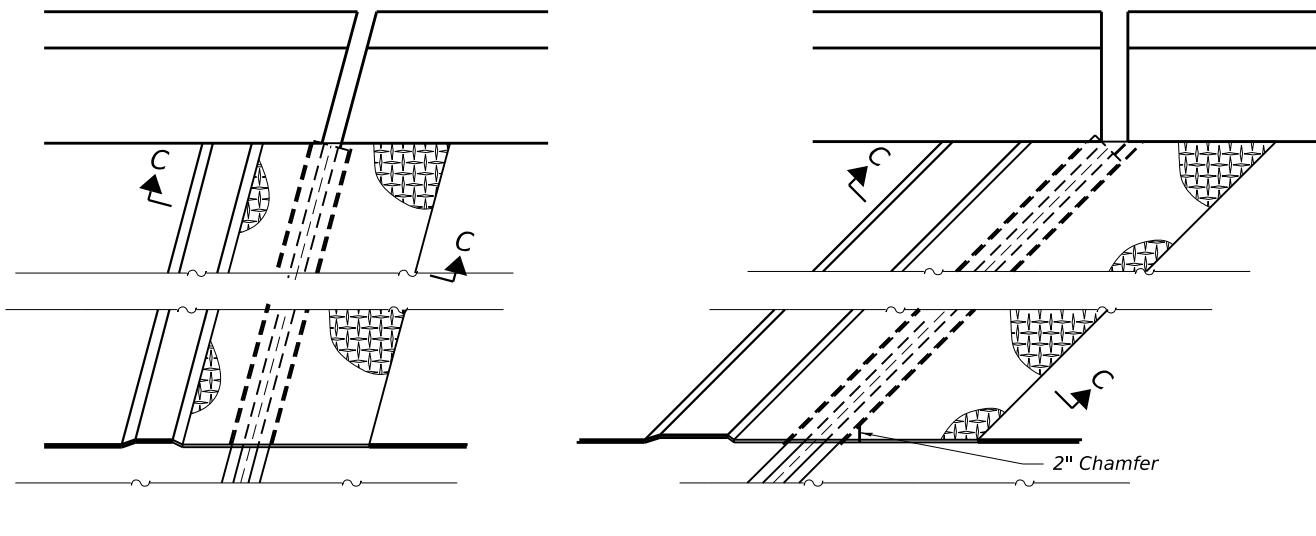


BILL OF MATERIAL

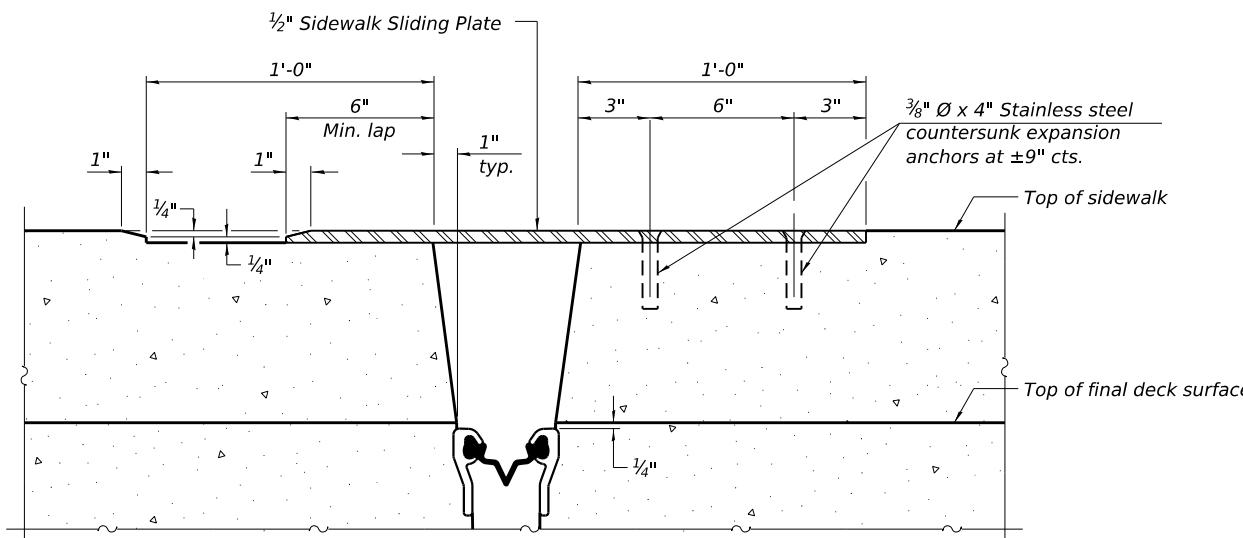
Item	Unit	Quantity
Bridge Fence Railing, Curved	Foot	705



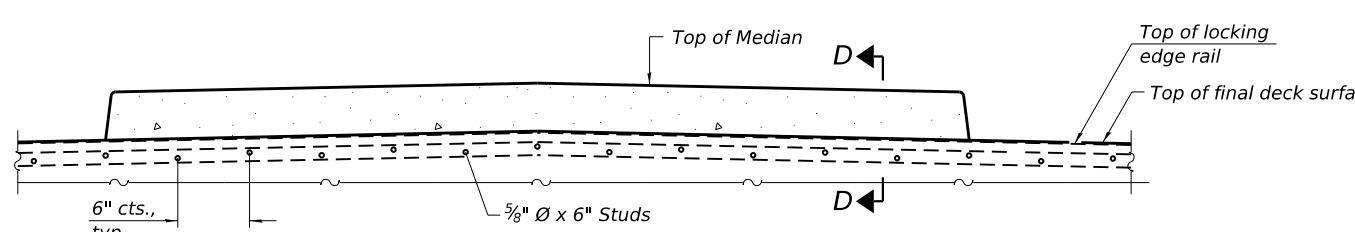
SECTION AT RAISED SIDEWALK



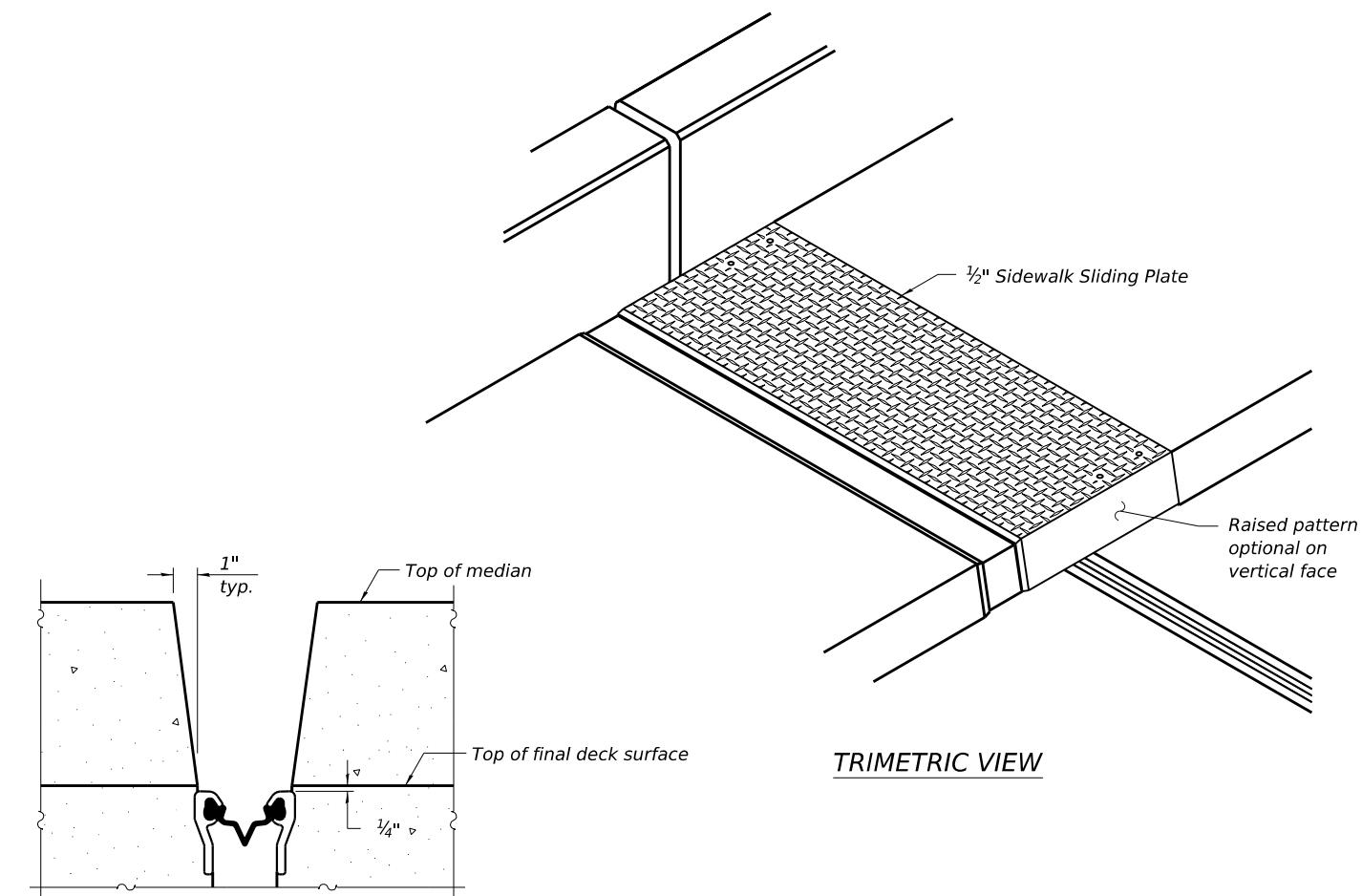
PLAN AT RAISED SIDEWALK



SECTION C-C



SECTION AT MEDIAN
For skews $> 30^\circ$, chamfer acute corners 2" similar to sidewalk.



EJ-SS-S

4-4-2025



INTERIOR GIRDER MOMENT TABLE			
	0.4 Span 1 0.6 Span 3	Pier 1 & Pier 2	0.5 Span 2
I_s (in ⁴)	15761	29001	24338
$I_c(n)$ (in ⁴)	35180	56136	48982
$I_c(3n)$ (in ⁴)	25637	41438	35959
$I_c(cr)$ (in ⁴)	—	33623	—
S_s (in ³)	927	1568	1352
$S_c(n)$ (in ³)	1233	1989	1734
$S_c(3n)$ (in ³)	1119	1803	1573
$S_c(cr)$ (in ³)	—	1666	—
S_x (in ³)	1178	1872	1665
$DC1$ (kip)	0.932	1.096	1.042
M_{DC1} (kip)	472	1212	695
$DC2$ (kip)	0.402	0.402	0.402
M_{DC2} (kip)	220	483	253
DW (kip)	0.095	0.095	0.095
M_{DW} (kip)	52	114	60
$LLDF$	0.538	0.526	0.514
M_{L+IM} (kip)	1066	1291	1208
f_t (Strength I) (ksi)	0.00	0.00	0.00
$M_u + \frac{1}{3} f_t S_x$ (kip)	2809	4550	3387
$\Phi_f M_n$ (kip)	5446	—	7375
$f_s DC1$ (ksi)	6.1	9.3	6.2
$f_s DC2$ (ksi)	2.4	3.5	1.9
$f_s DW$ (ksi)	0.6	0.8	0.5
$f_s (L+IM)$ (ksi)	10.4	9.3	8.4
f_t (Service II) (ksi)	0.0	0.0	0.0
$f_s + f_t/2$ (Service II) (ksi)	22.5	25.7	19.4
Service II Resistance (ksi)	47.5	47.5	47.5
$f_s + f_t/3$ (Strength I) (ksi)	—	33.5	—
$\Phi_f F_n$ (ksi)	—	50.0	—
V_f (k)	24.6	27.8	22.0

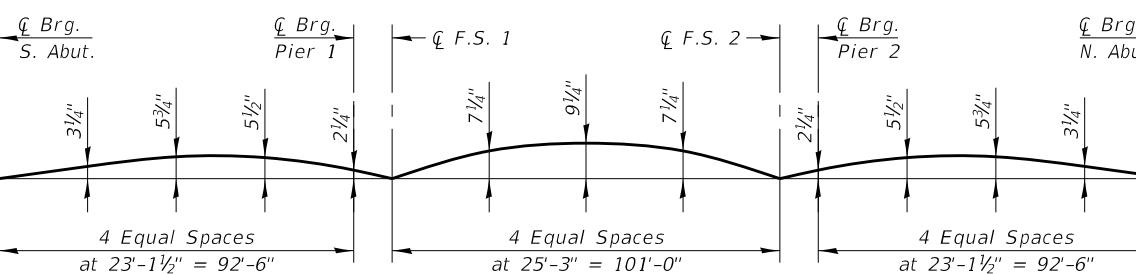
I_s, S_s :	Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in. ⁴ and in. ³).	$f_s DC1$:	Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
$I_c(n), S_c(n)$:	Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in. ⁴ and in. ³).	M_{DC1} / S_s	M_{DC1} / S_s
$I_c(3n), S_c(3n)$:	Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in. ⁴ and in. ³).	$f_s DC2$:	Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
$I_c(cr), S_c(cr)$:	Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in. ⁴ and in. ³).	$M_{DC2} / S_c(n)$ or $M_{DC2} / S_c(cr)$ as applicable.	$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.
S_x :	Section modulus about the major axis of a section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in. ³).	$f_s (L+IM)$:	$M_{L+IM} / S_c(n)$ or $M_{L+IM} / S_c(cr)$ as applicable.
$DC1$:	Un-factored non-composite dead load (kips/ft.).	$f_s + f_t/2$ (Service II):	Sum of stresses as computed below (ksi).
M_{DC1} :	Un-factored moment due to non-composite dead load (kip-ft.).	$f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (L+IM) + f_t/2$	$f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (L+IM) + f_t/2$
$DC2$:	Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).	Composite (0.95R _f F _{yf}) or noncomposite (0.80R _f F _{yf}) stress capacity according to Article 6.10.4.2 (ksi).	Composite (0.95R _f F _{yf}) or noncomposite (0.80R _f F _{yf}) stress capacity according to Article 6.10.4.2 (ksi).
M_{DC2} :	Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).	$f_s + f_t/3$ (Strength I):	Sum of stresses as computed below on non-compact sections (ksi).
DW :	Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).	ΦF_n :	1.25 (f _s DC1 + f _s DC2) + 1.5 f _s DW + 1.75 f _s (L+IM) + f _t /3
M_{DW} :	Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).	Service II Resistance:	Factored nominal flexural resistance of the section as specified in Article 6.10.7.2 or 6.10.8 as applicable (ksi).
$LLDF$:	Live Load Distribution Factor for moment and shear computed according to Article 4.6.2.2 and further IDOT provisions.	R_f :	Maximum factored shear range in span computed according to Article 6.10.10.
M_{L+IM} :	Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).	OCF :	Obtuse Correction Factor according to Article 4.6.2.2.3c or as further simplified by IDOT provisions.
M_u :	Strength I load combination of factored design moments (kip-ft.).	R_{DC1} :	Un-factored reaction due to non-composite dead load (kip).
$1.25(M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{L+IM}$		R_{DC2} :	Un-factored reaction due to long-term composite (superimposed excluding future wearing surface) dead load (kip).
f_t :	Factored calculated flange lateral bending stress as calculated using Article 6.10.1.6 and as further simplified by IDOT provisions (ksi).	R_{DW} :	Un-factored reaction due to long-term composite (superimposed future wearing surface only) dead load (kip).
$\Phi_f M_n$:	Factored nominal flexural resistance of the section determined as specified in Article 6.10.7.1 or A6 as applicable (kip-ft.).	R_L :	Un-factored live load reaction (kip).
		R_{IM} :	Un-factored dynamic load allowance (impact) (kip).
		$R_{Total}(Strength I)(Impact)$:	Strength I load combination of factored design reactions (kip).
		$R_{Total}(Strength I)(No Impact)$:	1.25 (R _{DC1} + R _{DC2}) + 1.5 R _{DW} + 1.75 (R _L + R _{IM})
			Strength I load combination of factored design reactions, not including dynamic load allowance (Impact) (kip).
			1.25 (R _{DC1} + R _{DC2}) + 1.5 R _{DW} + 1.75 (R _L)

GIRDER REACTION TABLE		
	Abut.	Pier
LLDF	0.689	0.689
OCF	—	—
R_{DC1} (kip)	56.0	119.2
R_{DC2} (kip)	19.4	48.2
R_{DW} (kip)	4.6	11.4
R_L (kip)	77.0	121.8
R_{IM} (kip)	18.6	21.6
$R_{Total}(Strength I)(Impact)$ (kip)	268.5	477.3
$R_{Total}(Strength I)(No Impact)$ (kip)	235.9	439.5

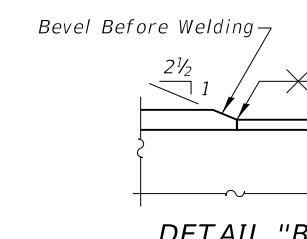
TOP OF WEB ELEVATIONS

(For Fabrication use only)

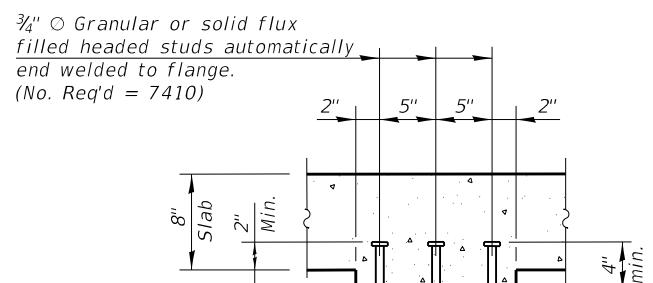
Girder No.	$\frac{1}{4}$ Brdg. S. Abut.	$\frac{1}{4}$ Brdg. Pier 1	F.S. 1	F.S. 2	$\frac{1}{4}$ Brdg. Pier 2	$\frac{1}{4}$ Brdg. N. Abut.
1	594.775	597.853	597.978	597.980	597.855	594.775
2	594.775	597.853	597.978	597.980	597.855	594.775
3	594.827	597.905	598.030	598.032	597.907	594.827
4	594.925	598.003	598.128	598.130	598.005	594.925
5	595.015	598.093	598.218	598.220	598.095	595.015
6	595.015	598.093	598.218	598.220	598.095	595.015
7	594.925	598.003	598.128	598.130	598.005	594.925
8	594.827	597.905	598.030	598.032	597.907	594.827
9	594.775	597.853	597.978	597.980	597.855	594.775
10	594.775	597.853	597.978	597.980	597.855	594.775



CAMBER DIAGRAM



DETAIL "B"



SHEAR CONNECTORS DETAIL

	Span 1 Span 3	Span 2
Δ_{ALLOW} (in)	1.11	1.45
Δ_{L+IM} (in)	1.10	1.35

Δ_{ALLOW} : Maximum allowable Service I live load plus impact deflection according to AASHTO LRFD Bridge Design Specifications 9.5.2.

Δ_{L+IM} : Calculated value Service I live load plus impact deflection.

Notes:

All beams, bearing stiffeners, connection plates, and splice plate material shall be AASHTO M270 Grade 50.

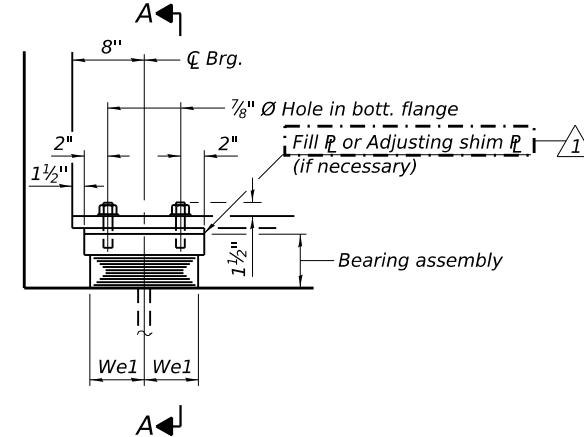
Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Requirements, Zone 2.

All span lengths, end of beam dimensions and diaphragm spaces are along horizontal plane of the structure.

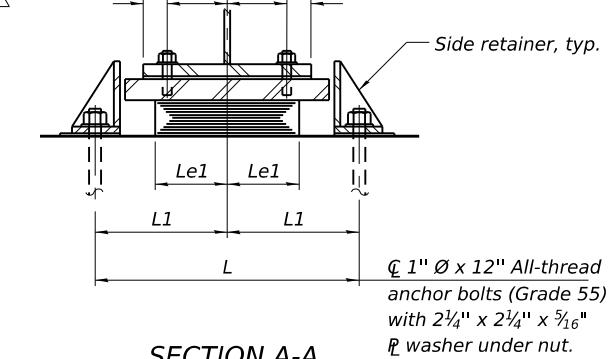
1 REVISED SHEET 12/31/2025

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

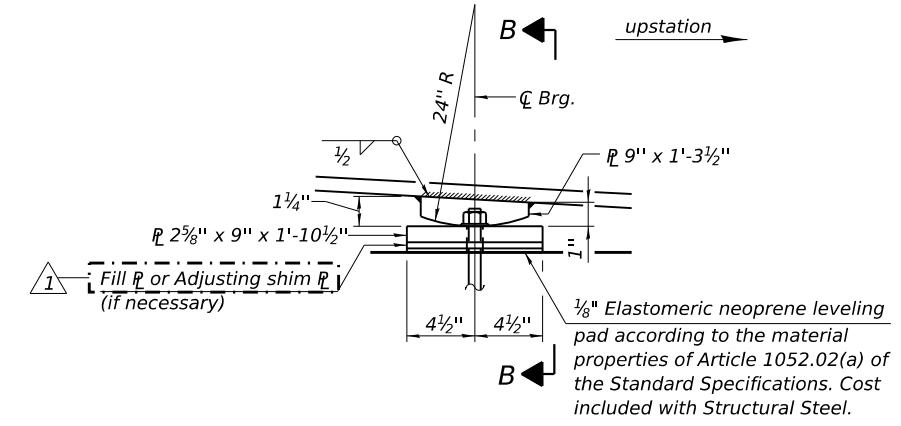
GIRDER DETAILS
STRUCTURE NO. 01



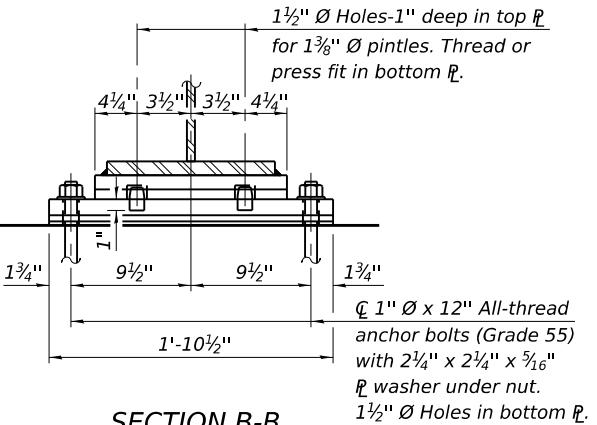
ELEVATION AT ABUT.
Elevation at Pier 1 similar



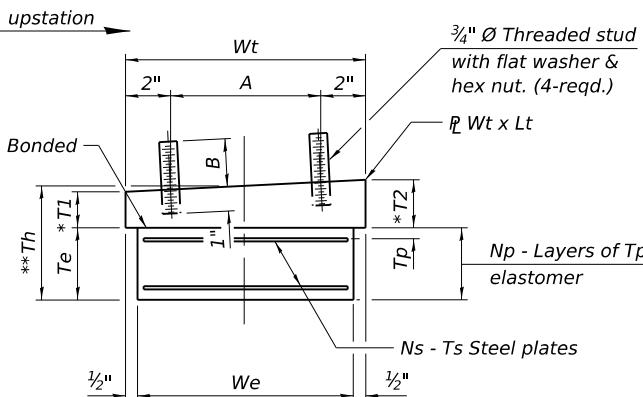
SECTION A-A



ELEVATION

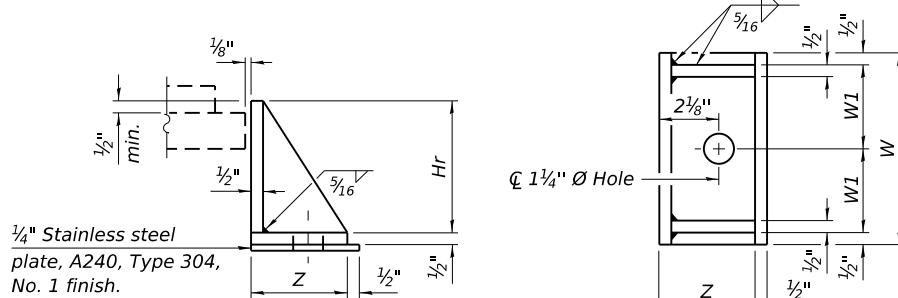


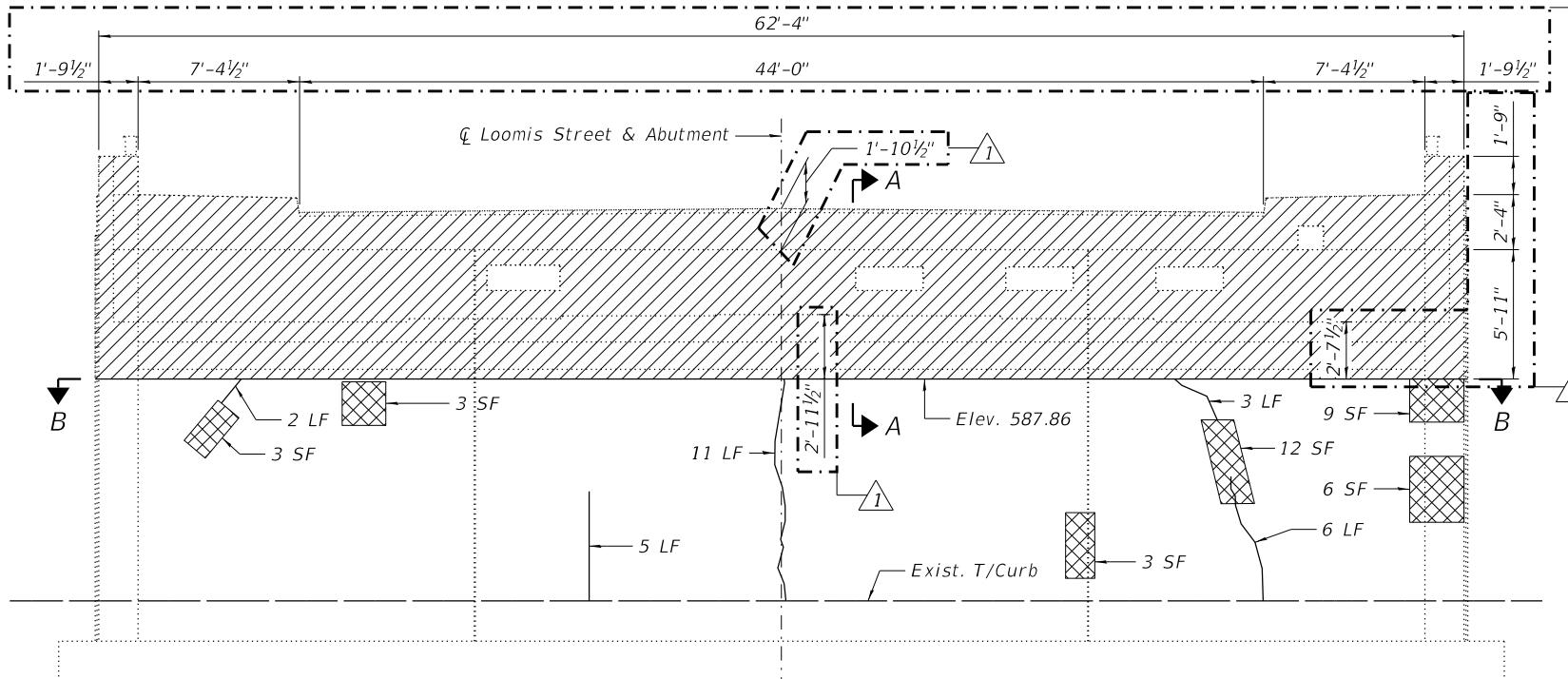
SECTION B-B



BEARING ASSEMBLY

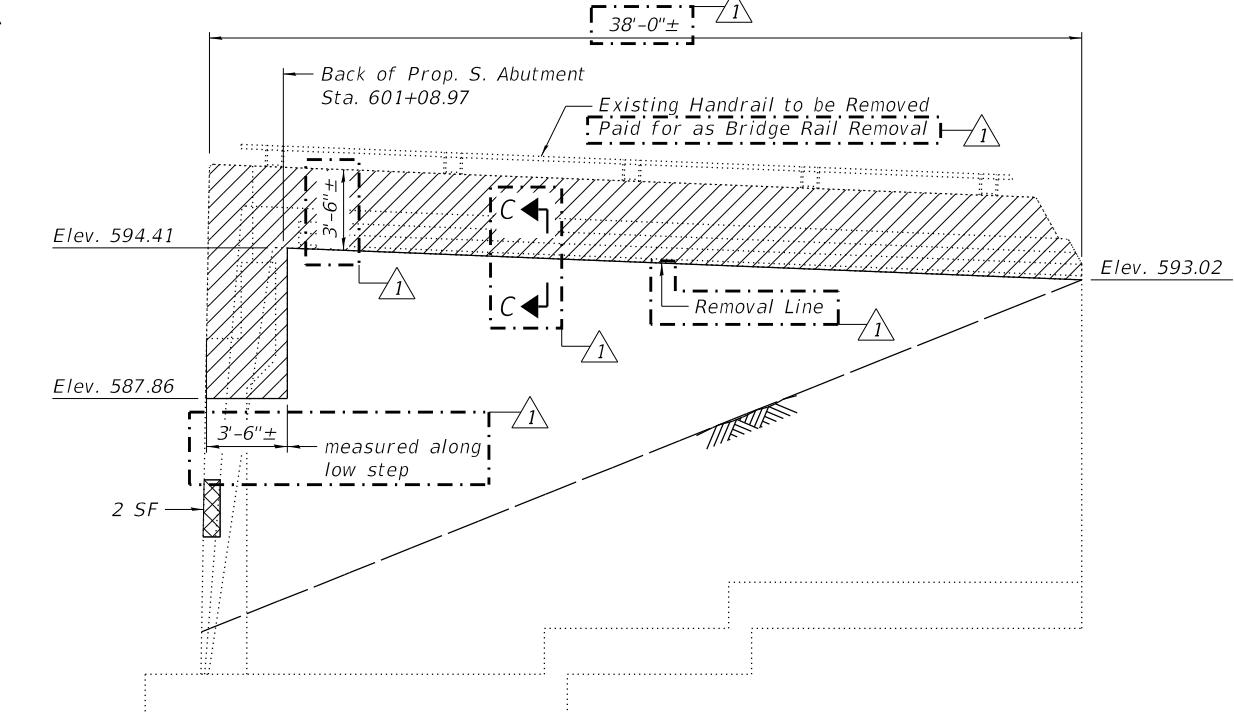
Note
Shim plates shall not be placed under bearing assembly.
* T2 always oriented upstation from T1
** Measured at \mathbb{C} Brdg





SOUTH ABUTMENT ELEVATION

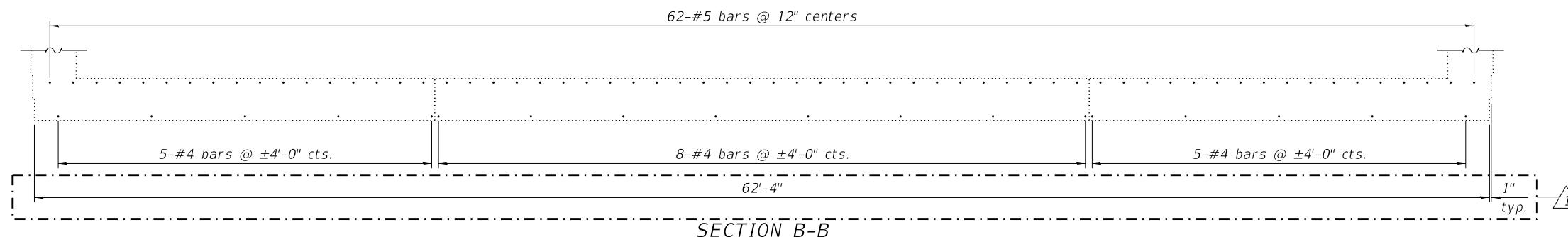
(Looking South)



SOUTHWEST WINGWALL ELEVATION

(Looking East)

(Similar concrete and handrail removal for Southeast Wingwall)



SECTION B-B

(Showing existing reinforcement bars to remain)

NOTES:

Repairs to the existing abutment below the concrete removal shall include but not be limited to the areas shown. The actual areas to be repaired will be determined by the Engineer at the time of construction.

Repairs are to be performed after removal of existing superstructure elements and associated bearings.

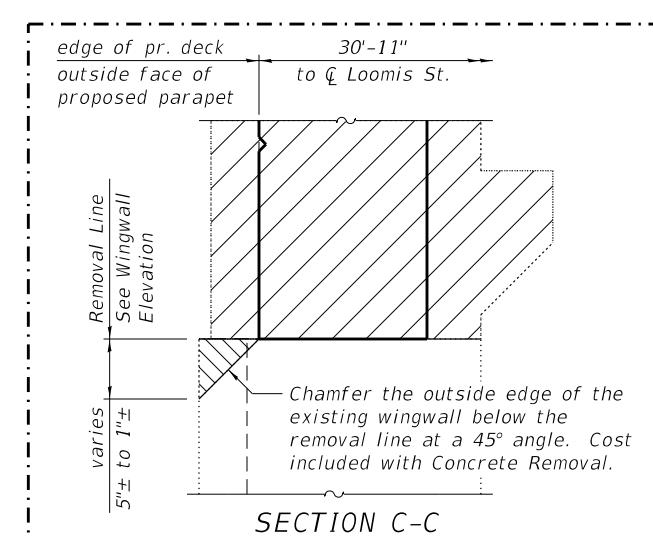
Saw cut to maintain smooth finish on exposed concrete faces. Cost included with Concrete Removal.

Existing reinforcement to remain shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.

Seal exposed rebar at top of wingwall with epoxy. Cost included with Concrete Removal.

See Sheet S-34 for South Abutment Modifications.

Removal of the handrail on the deck parapet is included in the cost of Removal of Existing Superstructures.



SECTION C-C

LEGEND

Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)

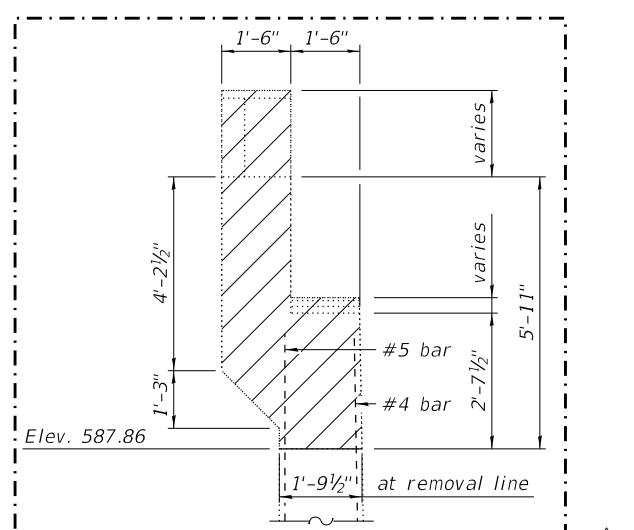
Concrete Removal

Epoxy Crack Injection

LF Linear Feet

SF Square Feet

SY Square Yards



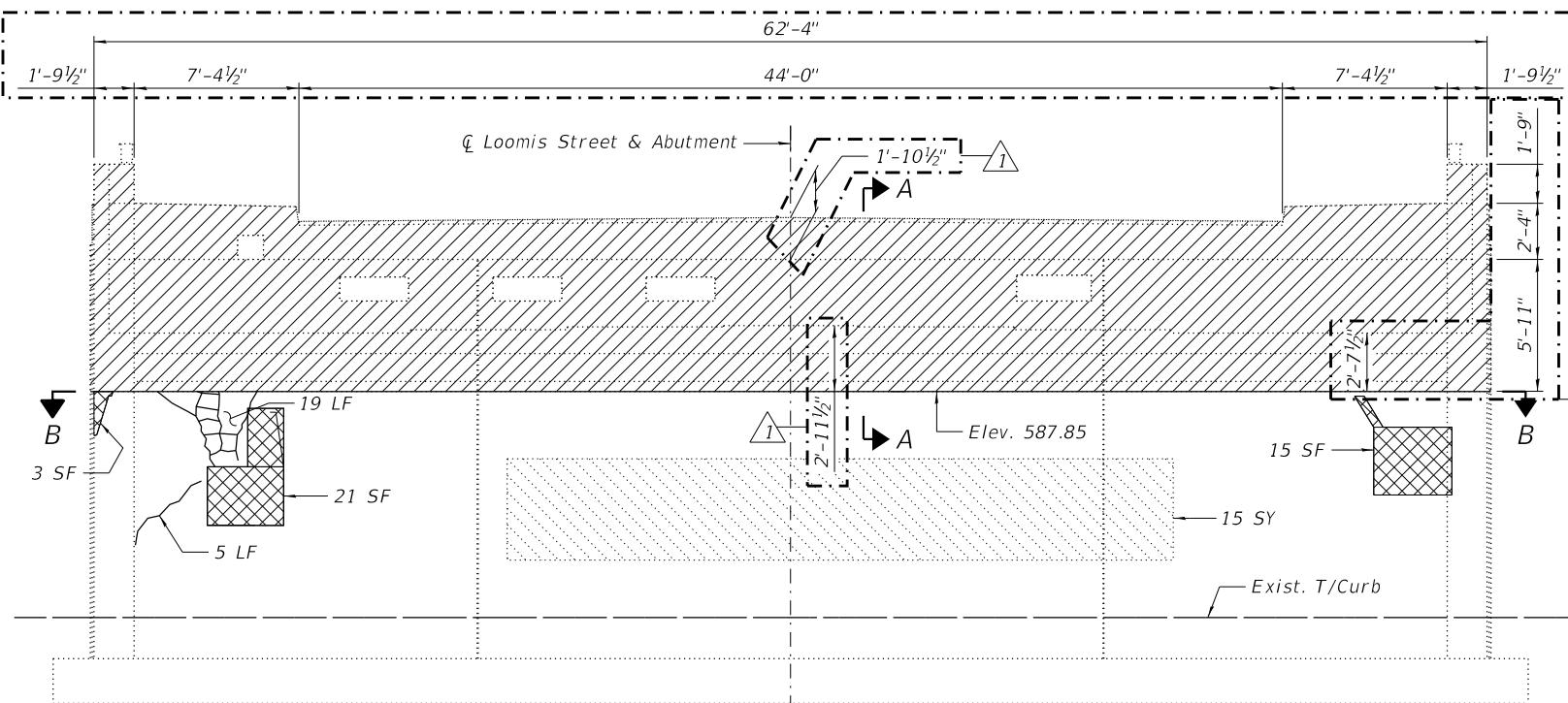
SECTION A-A

(Showing existing reinforcement bars to remain)

BILL OF MATERIAL

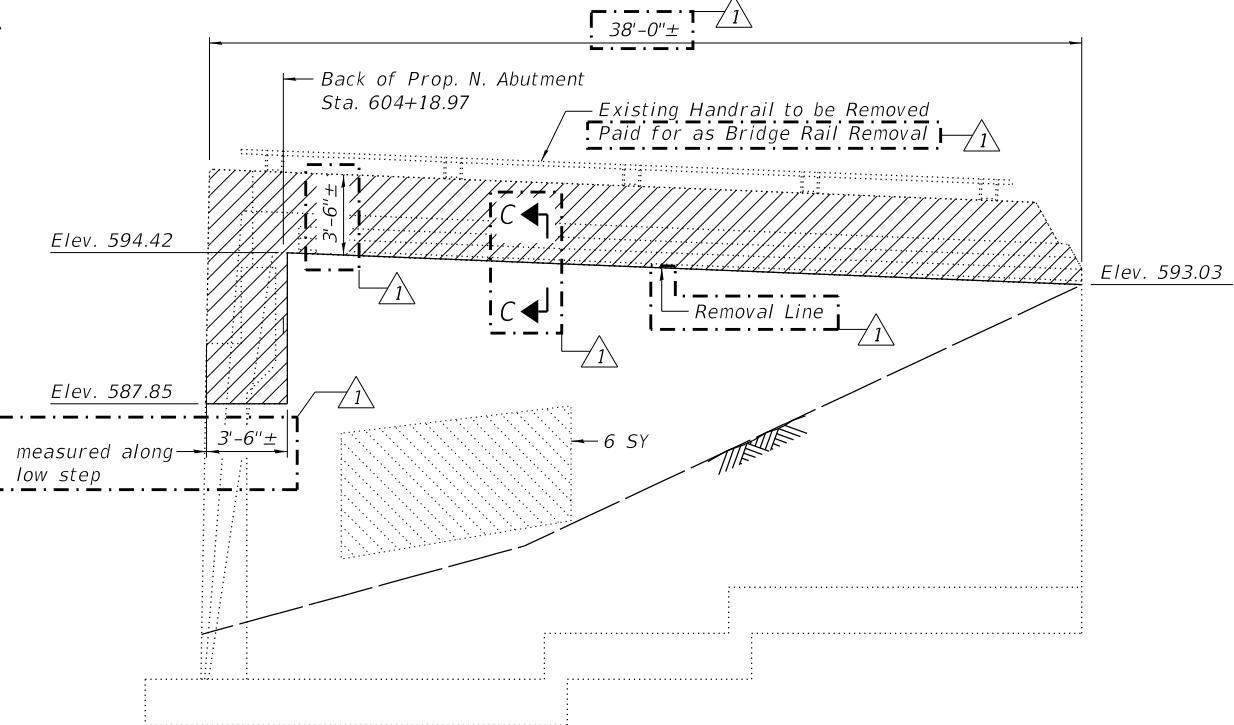
Item	Unit	Total
Concrete Removal	Cu Yd	51
Bridge Rail Removal	Foot	67
Epoxy Crack Injection	Foot	27
Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	38

REVISED SHEET 12/31/2025



NORTH ABUTMENT ELEVATION

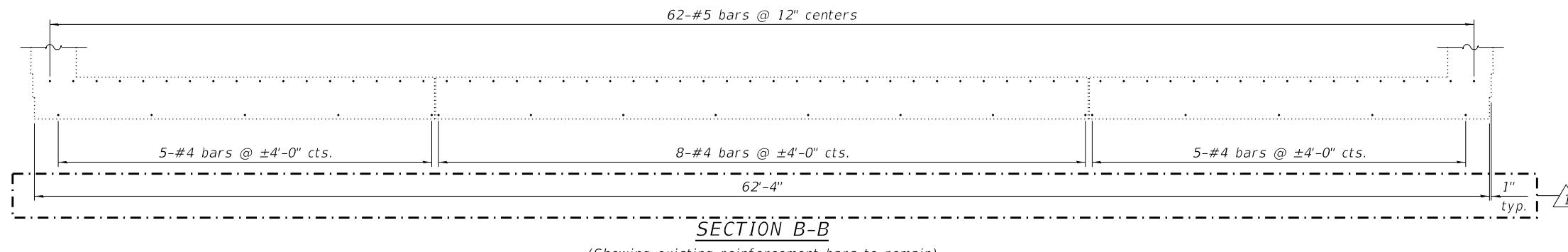
(Looking North)



NORTHEAST WINGWALL ELEVATION

(Looking West)

(Similar concrete and handrail removal for Northwest Wingwall)



SECTION B-B

(Showing existing reinforcement bars to remain)

NOTES:

Repairs to the existing abutment below the concrete removal shall include but not be limited to the areas shown. The actual areas to be repaired will be determined by the Engineer at the time of construction.

Repairs are to be performed after removal of existing superstructure elements and associated bearings.

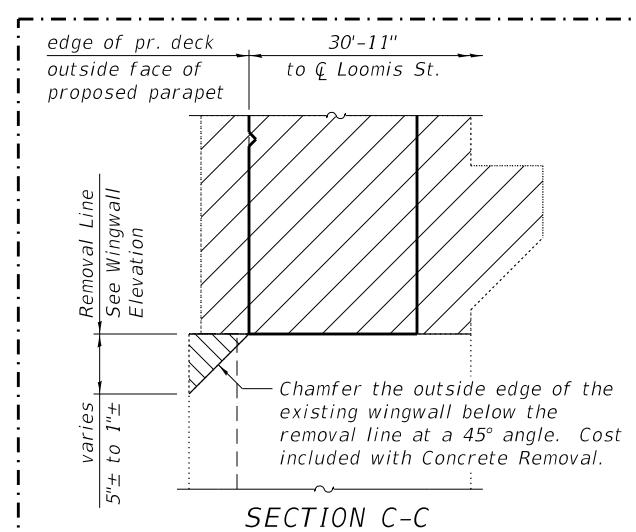
Saw cut to maintain smooth finish on exposed concrete faces. Cost included with Concrete Removal.

Existing reinforcement to remain shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.

Seal exposed rebar at top of wingwall with epoxy. Cost included with Concrete Removal.

See Sheet S-34 for North Abutment Modifications.

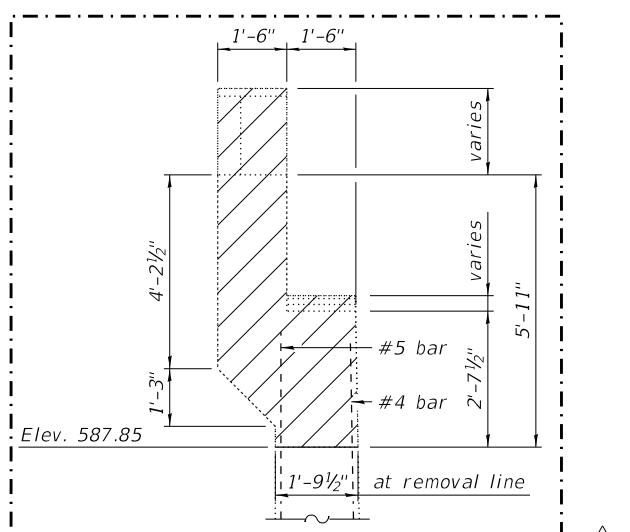
Removal of the handrail on the deck parapet is included in the cost of Removal of Existing Superstructures.



SECTION C-C

LEGEND

- Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)
- Graffiti Removal
- Concrete Removal
- Epoxy Crack Injection
- LF Linear Feet
- SF Square Feet
- SY Square Yards



SECTION A-A

(Showing existing reinforcement bars to remain)

BILL OF MATERIAL

Item	Unit	Total
Concrete Removal	Cu Yd	51
Bridge Rail Removal	Foot	67
Epoxy Crack Injection	Foot	24
Graffiti Removal	Sq Yd	21
Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	39

REVISED SHEET 12/31/2025



BLA, Inc.

USER NAME = idot	DESIGNED - JHZ	REVISED - 10/24/2025 TB
CHECKED - PRD	DRAWN - JHZ	REVISED -
PLOT SCALE = NONE	DRAWN - JHZ	REVISED -

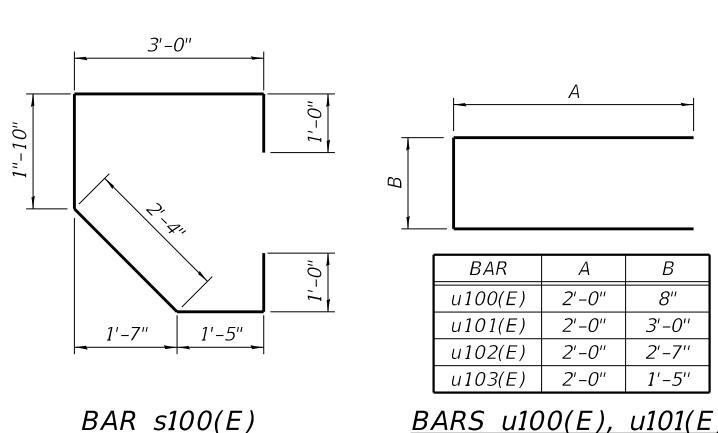
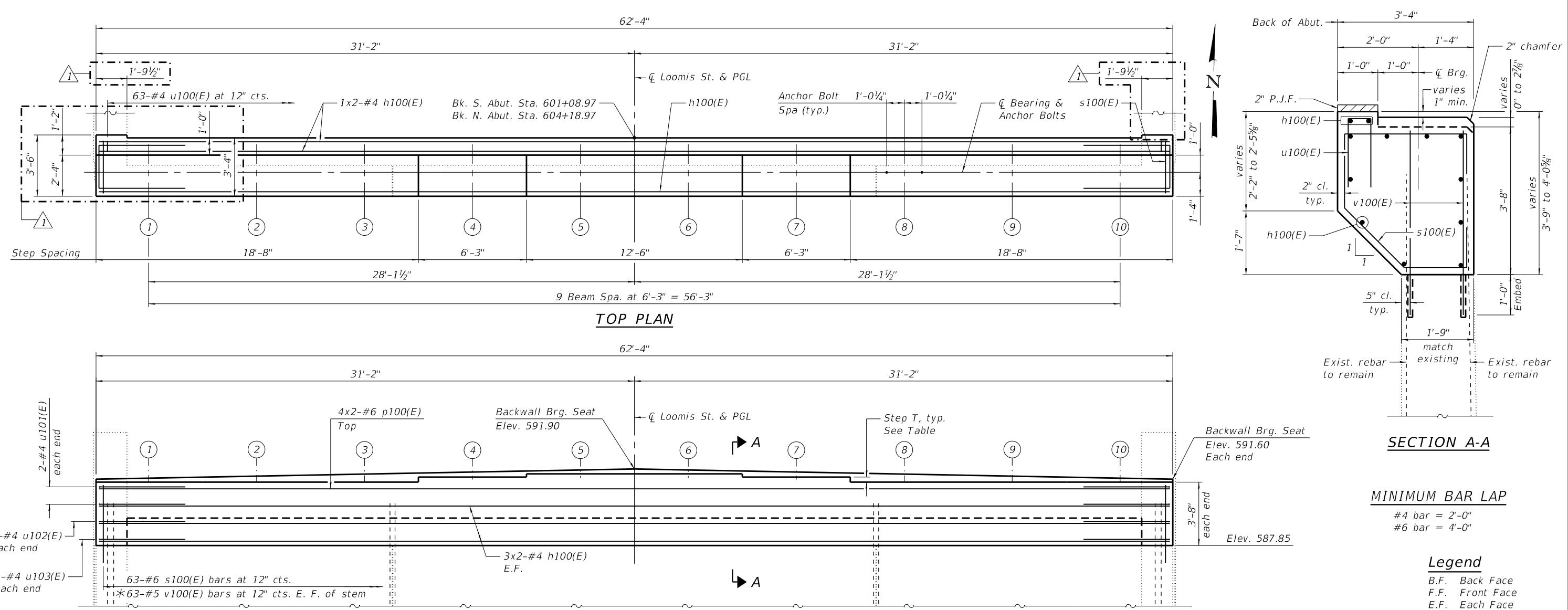
11/7/2025 5:45:55 PM

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT REMOVAL AND REPAIR
STRUCTURE NO. 016-2114

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	F.A.I. 290 22 BRIDGE 3	COOK	161	125
		ILLINOIS	FED. AID PROJECT	

SHEET S-33 OF 38 SHEETS



Beam No.	BRG. SEAT ELEVATION	STEP T- inch
1	591.52	
2	591.52	0
3	591.52	1 1/4
4	591.67	1 1/8
5	591.76	0
6	591.76	1 1/8
7	591.67	1 1/4
8	591.52	
9	591.52	0
10	591.52	

NOTES:

Bars indicated thus 4x2-#7 etc. indicates 4 lines of bars with 2 lengths per line.

See Sheet S-32 for South Abutment Removal and Repair.

See Sheet S-33 for North Abutment Removal and Repair.

See Sheet S-03 for abutment drainage details.

Existing reinforcement to remain shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.

Space reinforcement in cap to miss anchor bolts.

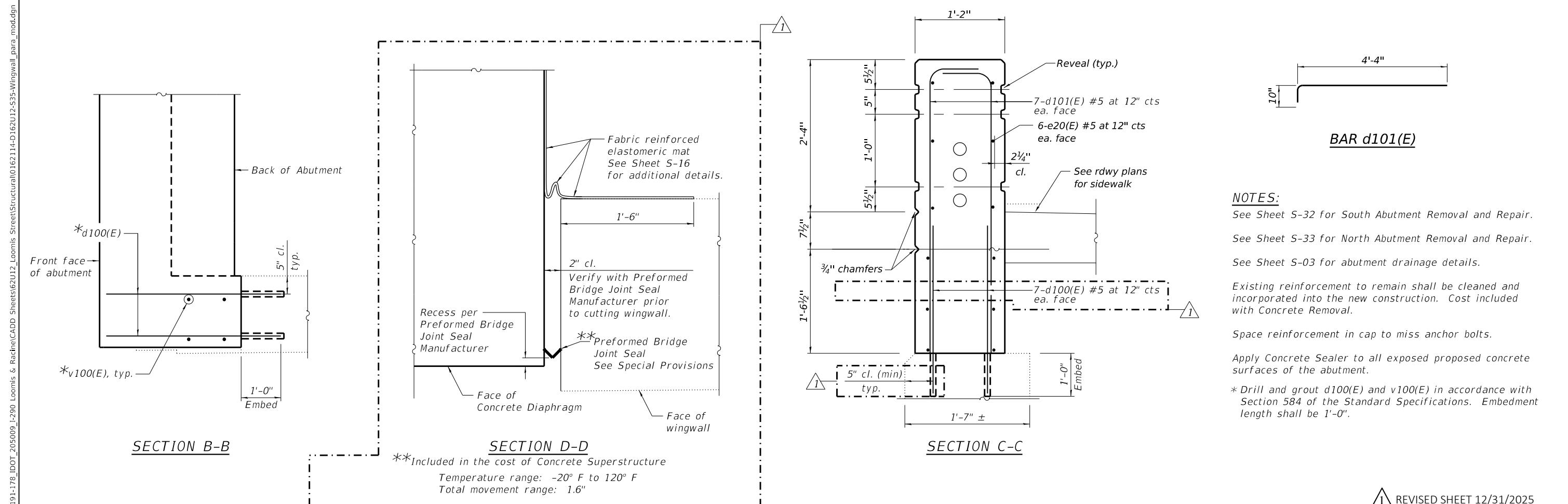
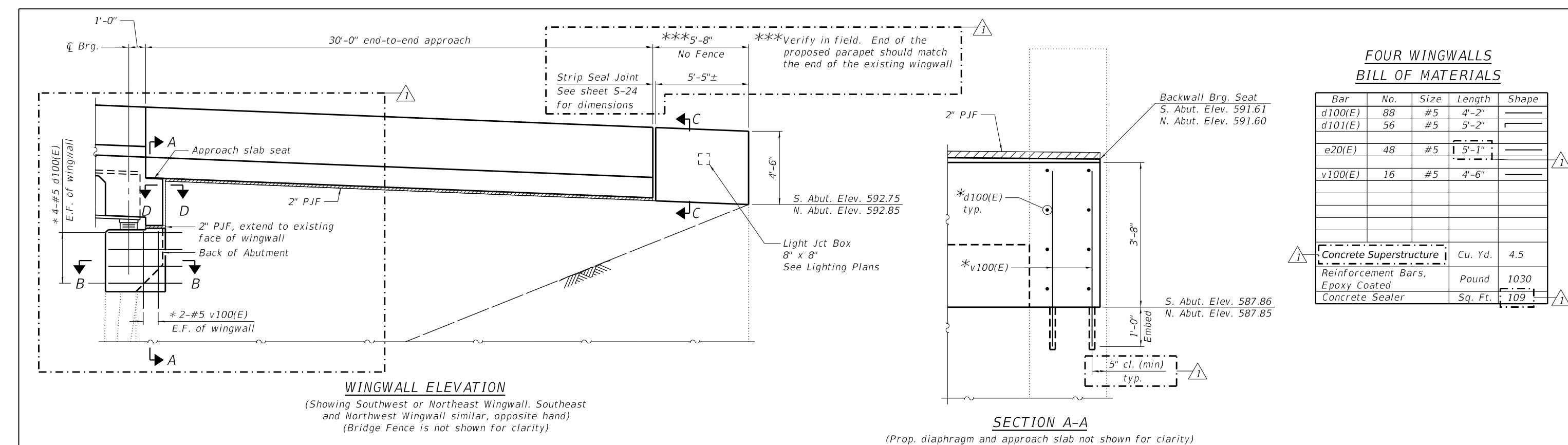
Apply Concrete Sealer to all exposed proposed concrete surfaces of the abutment.

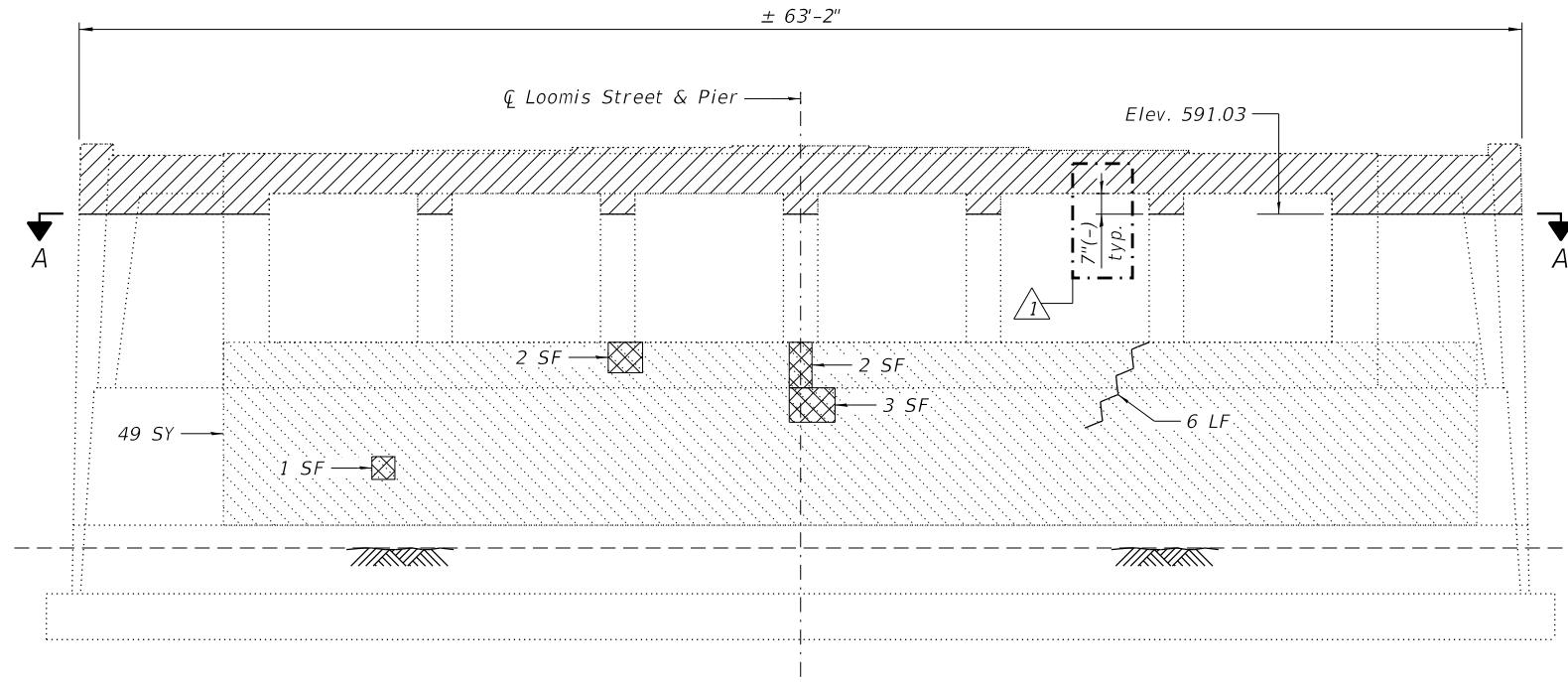
*Drill and grout v100(E) in accordance with Section 584 of the Standard Specifications. Embedment length shall be 1'-0".

TWO ABUTMENTS BILL OF MATERIALS

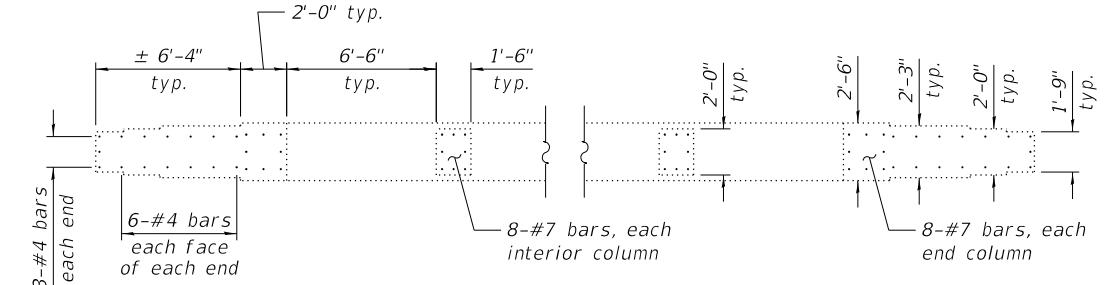
BAR	NO.	SIZE	LENGTH	SHAPE
h100(E)	32	#4	32'-0"	—
p100(E)	16	#6	33'-0"	—
s100(E)	126	#6	10'-7"	C
u100(E)	126	#4	4'-8"	—
u101(E)	8	#4	7'-0"	—
u102(E)	4	#4	6'-7"	—
u103(E)	4	#4	5'-5"	—
v100(E)	252	#5	4'-6"	—
Concrete Structures			Cu. Yd.	56.0
Concrete Sealer			Sq. Ft.	830
Reinforcement Bars, Epoxy Coated			Pound	5,130

REVISED SHEET 12/31/2025





PIER 1 ELEVATION
(Looking South)



SECTION A-A
(Showing existing reinforcement bars to remain)

NOTES:

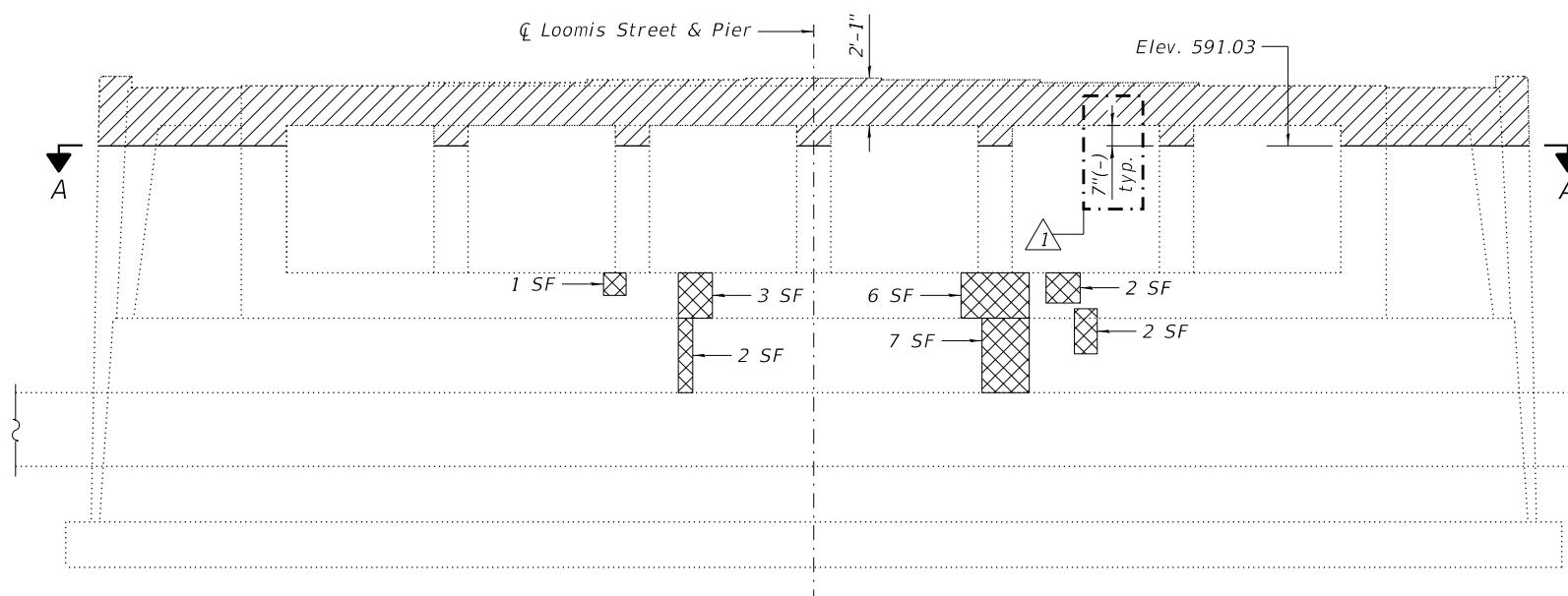
Repairs to the existing pier below the cap shall include but not be limited to the areas shown. The actual areas to be repaired will be determined by the Engineer at the time of construction.

Repairs are to be performed after removal of existing superstructure elements and associated bearings.

Saw cut to maintain smooth finish on exposed concrete faces. Cost included with Concrete Removal.

Existing reinforcement to remain shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.

See Sheet S-38 for Pier 1 Cap Modifications.



PIER 1 ELEVATION
(Looking North)

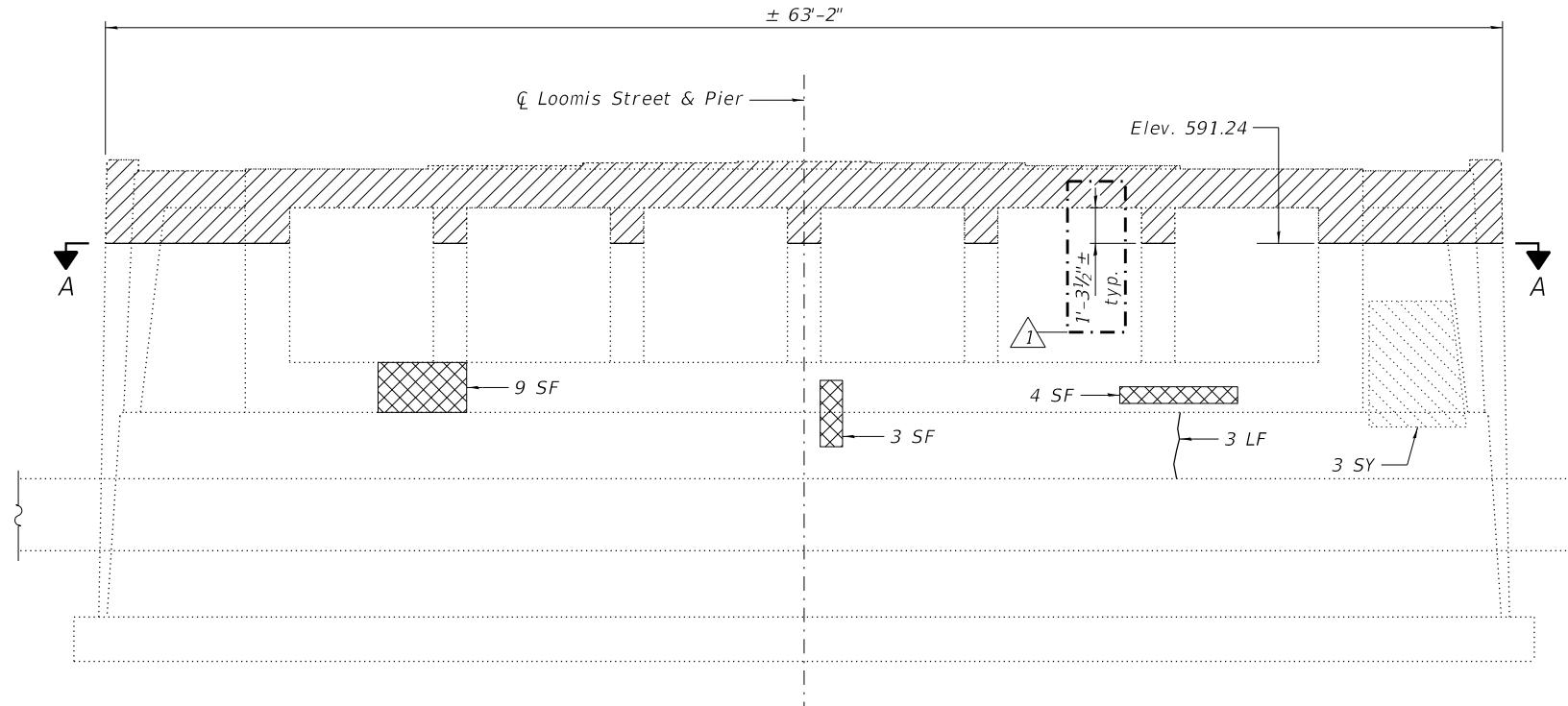
BILL OF MATERIAL

Item	Unit	Total
Concrete Removal	Cu Yd	12
Epoxy Crack Injection	Foot	6
Graffiti Removal	Sq Yd	49
Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	31

LEGEND

- Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)
- Graffiti Removal
- Concrete Removal
- Epoxy Crack Injection
- LF Linear Feet
- SF Square Feet
- SY Square Yards

REVISED SHEET 12/31/2025



NOTES:

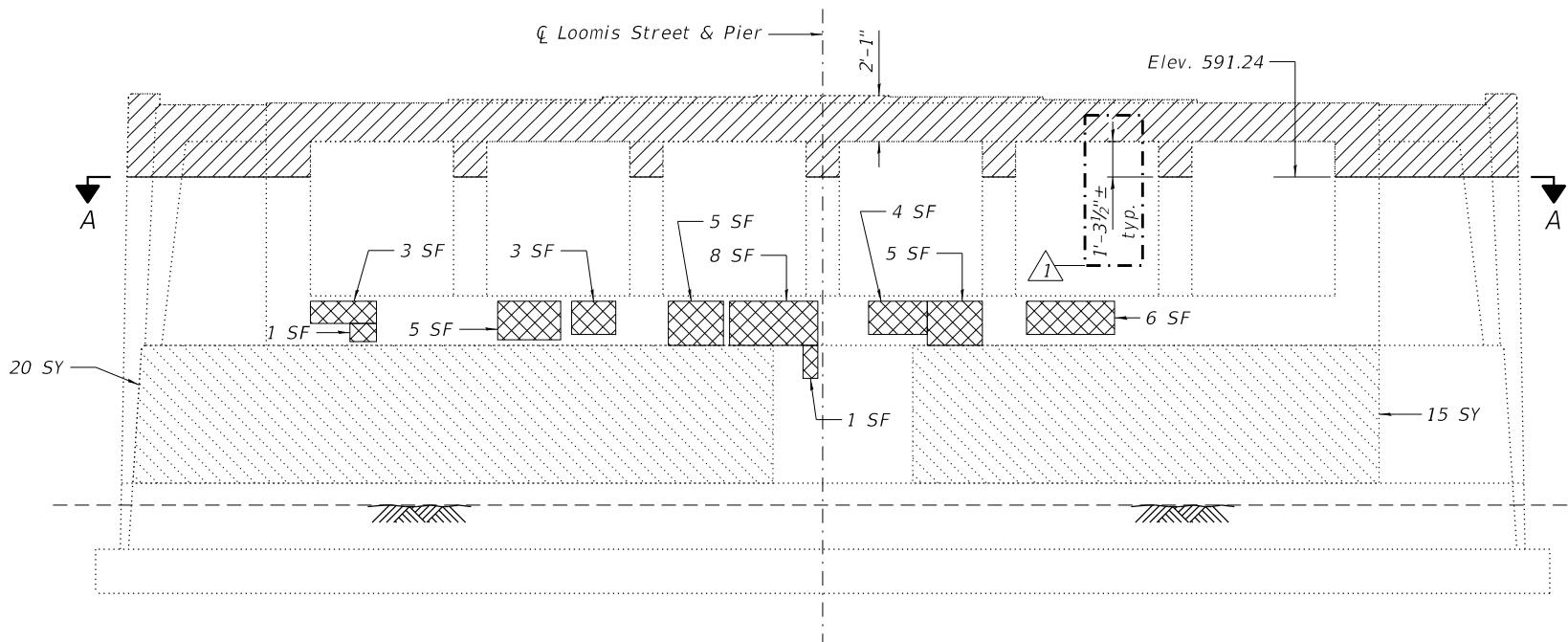
Repairs to the existing pier below the cap shall include but not be limited to the areas shown. The actual areas to be repaired will be determined by the Engineer at the time of construction.

Repairs are to be performed after removal of existing superstructure elements and associated bearings.

Saw cut to maintain smooth finish on exposed concrete faces. Cost included with Concrete Removal.

Existing reinforcement to remain shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.

See Sheet S-38 for Pier 2 Cap Modifications.



BILL OF MATERIAL

Item	Unit	Total
Concrete Removal	Cu Yd	13
Epoxy Crack Injection	Foot	3
Graffiti Removal	Sq Yd	38
Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	57

LEGEND

Structural Repair of Concrete (Depth Equal To Or Less Than 5 Inches)

Graffiti Removal

Concrete Removal

Epoxy Crack Injection

LF Linear Feet

SF Square Feet

SY Square Yards

1 REVISED SHEET 12/31/2025

GENERAL NOTES

SPECIFICATIONS:

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

MINIMUM CLEARANCE: 3" greater than bridge members at all locations. (All Obstructions)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 Structural Welding Code (Steel) and the Standard Specifications.

MATERIALS: All Structural Steel Pipe shall be ASTM A53 Grade B with a minimum yield of 35,000 p.s.i., or A500 Grade B or C with a minimum yield of 46,000 p.s.i. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 (M183, M223 Gr. 50.).

HIGH STRENGTH BOLTS: All bolts, washers, nuts and locknuts shall satisfy the requirements of ASTM designation A307 unless noted as "H.S." which shall require AASHTO M164 (A325), ASTM A449, or approved alternate. All fasteners shall be hot dip galvanized per AASHTO M232 unless otherwise specified.

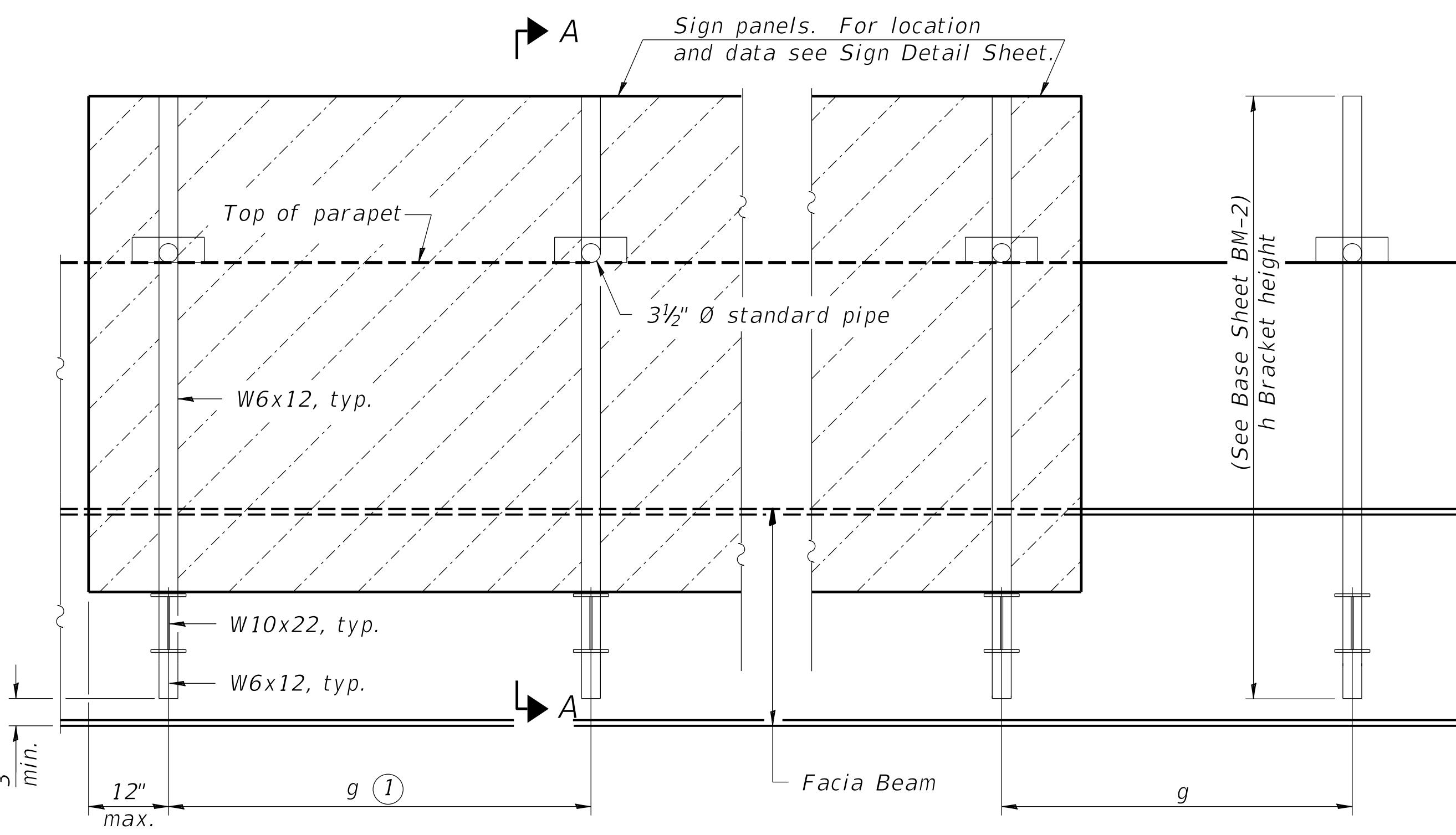
GALVANIZING: All Steel Grating, Plates and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: All-threaded rod shall conform to ASTM F1554 Grade 105, $\frac{3}{4}$ " Ø x 12" long, each with one plate washer and locknut and be hot dip galvanized per AASHTO M232. They shall be either cast into the concrete or epoxy grouted in accordance with Section 584 of the Standard Specifications. Minimum embedment in concrete shall be 9".

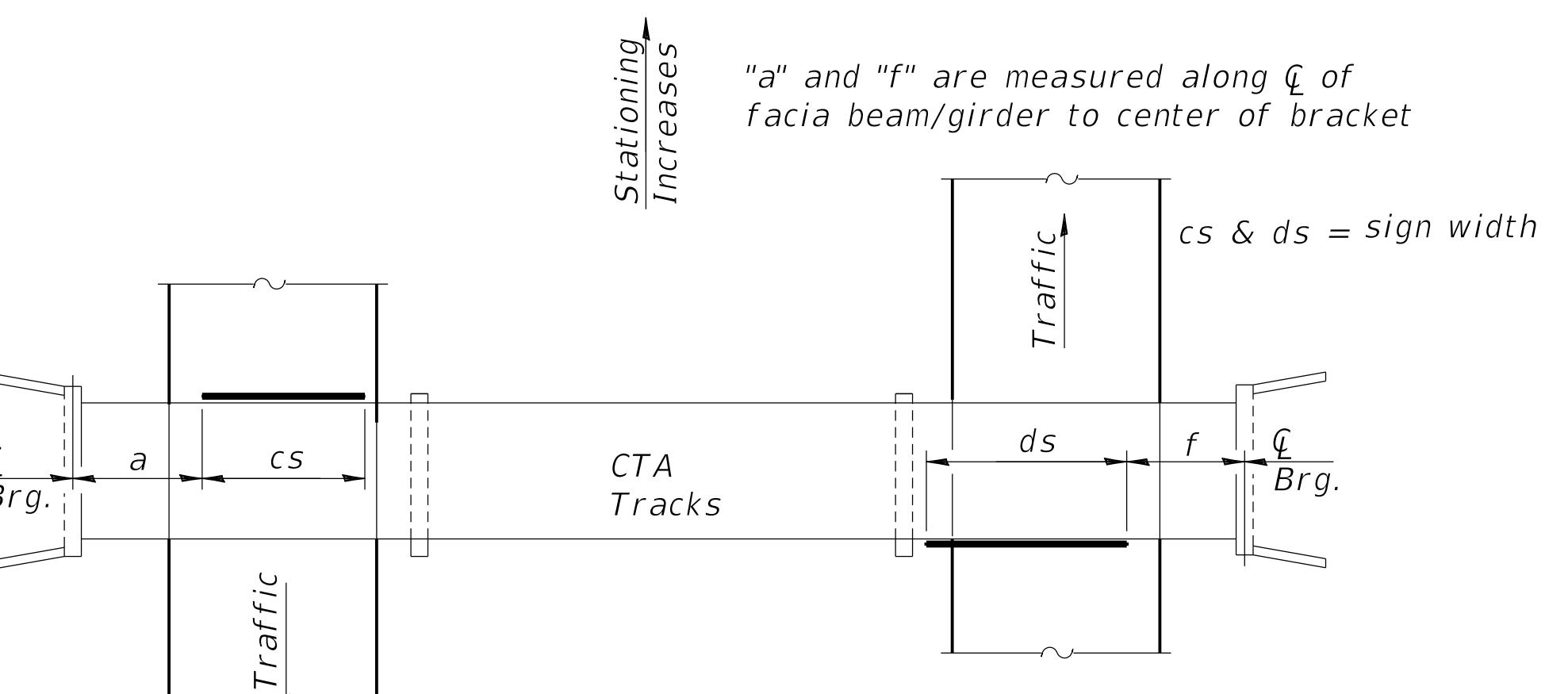
① Bracket spacing $g \leq 6'-0"$, max. Spacing shall be uniform if possible but may vary $\pm 6"$ to miss existing obstruction (rail post, light poles, web stiffeners, splice plates, etc.). Adjust bracket lengths accordingly on skewed structures.

② Any design modifications shall be based on the current version of applicable specifications and submitted for the Engineer's approval.

③ Unit price includes grating, handrail, brackets, supports, anchor bolts, fasteners, fabrication, delivery, erection, field drilling and other necessary items. Limits of payment are based on grating length (cw, dw) unless otherwise specified. For Safety Chain Details and Details D, F and G, see Base Sheet BM-4.



TYPICAL FRONT ELEVATION



PLAN

PLAN SKETCH
(Road plan beneath structure varies.)

Structure Number	Sign Skew Angle (L) or (R)	Bridge Station	Bridge Structure Number	Contract Route Designation	a	cs	f	g	No. of Brackets (Total)
0	0	601+21	016-2114	290 EB	5'-6"	11'-0"	-	4'-6"	3
0		603+99.5	016-2114	290 WB	-	11'-6"	17'-6"	5'-0"	3

Dimensions a, f & g may vary as approved by the Engineer, see ①.

TOTAL BILL OF MATERIAL

③ OVERHEAD SIGN STRUCTURE-BRIDGE MOUNTED	Foot	22.5
--	------	------

① ADDED ENTIRE SHEET 12/31/2025

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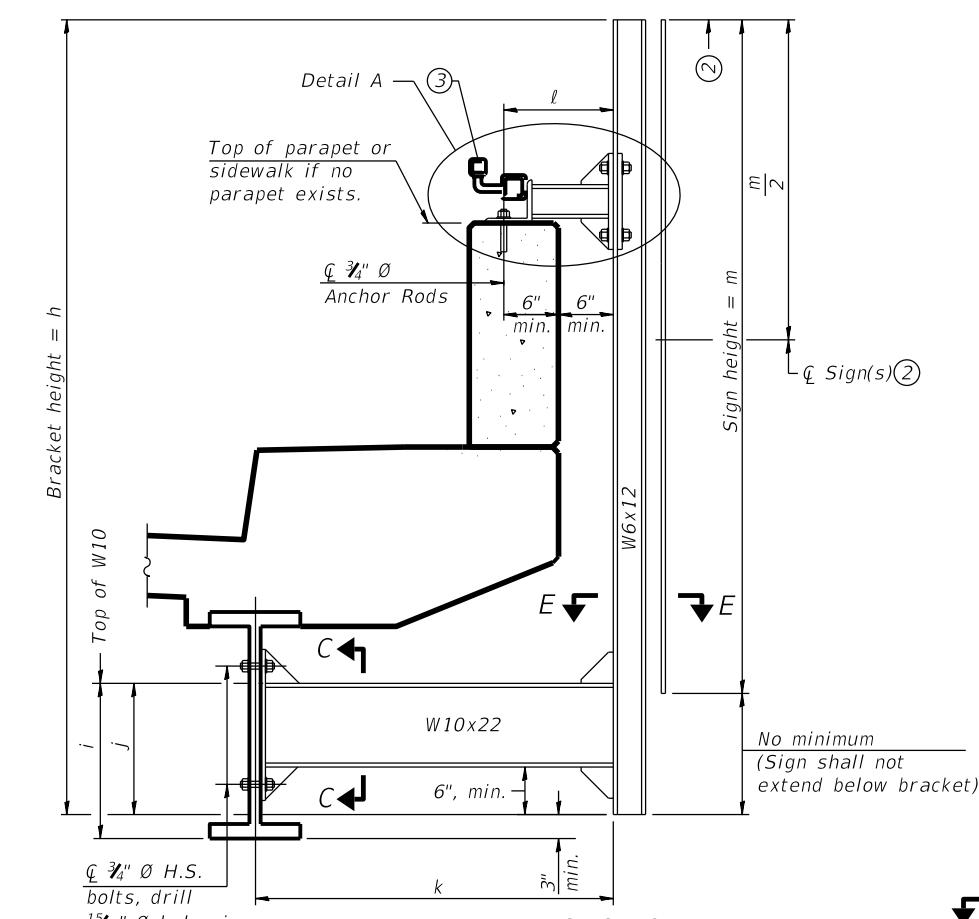
1

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SECTION A-A
Details for mounting to steel beam or girder
& Details for mounting with parapet mounted fence

Notes:
Installations not within dimensional limits shown require special analysis for all components and must be submitted to the Bureau of Bridges and Structures for approval.
Contractor shall field check all pertinent existing bridge dimensions shown on plans before submitting shop drawings.

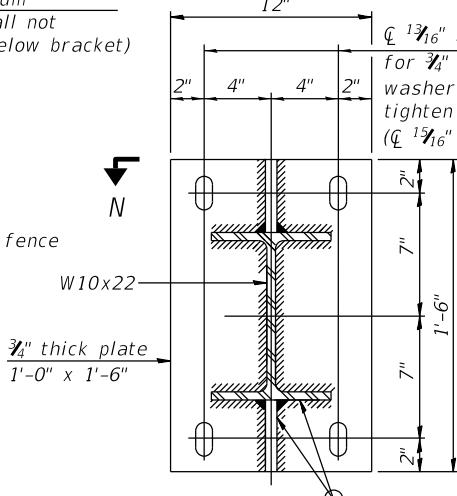
All holes in bridge beams or girders should be located in the middle half of the member. There shall be no holes drilled in the lower quarter of the member's depth. (For R.C. girder, depth = bottom of deck to bottom of the girder.) Proposed exceptions must be approved by the Bureau of Bridges and Structures.

The Engineer may adjust dimension "i" to meet the above condition and to keep the sign level.

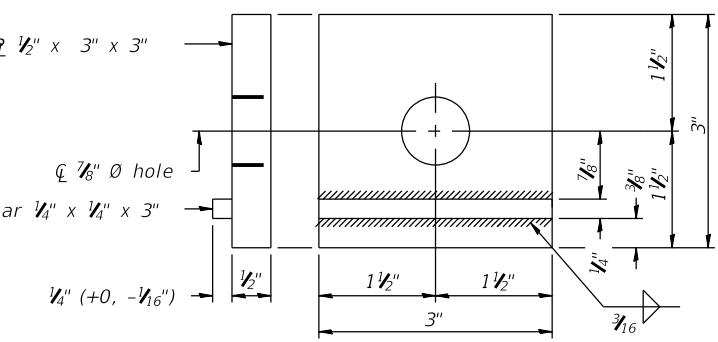
① Holes in new steel members may be drilled in the fabrication shop or in the field. Field drill existing members.

② Sign shall not extend more than 6" above top of bracket, and this dimension may vary to keep sign level if bridge is on grade or vertical curve. Multiple signs of various heights shall share a common horizontal centerline and use equal bracket heights. If no sign is attached to a W6x12 vertical (bracket only supporting walkway), dimension h shall be the same as an adjacent bracket with a sign attached, unless Engineer specifically directs shorter brackets due to locational restraints on future uses. (See Detail A for minimum bracket height.)

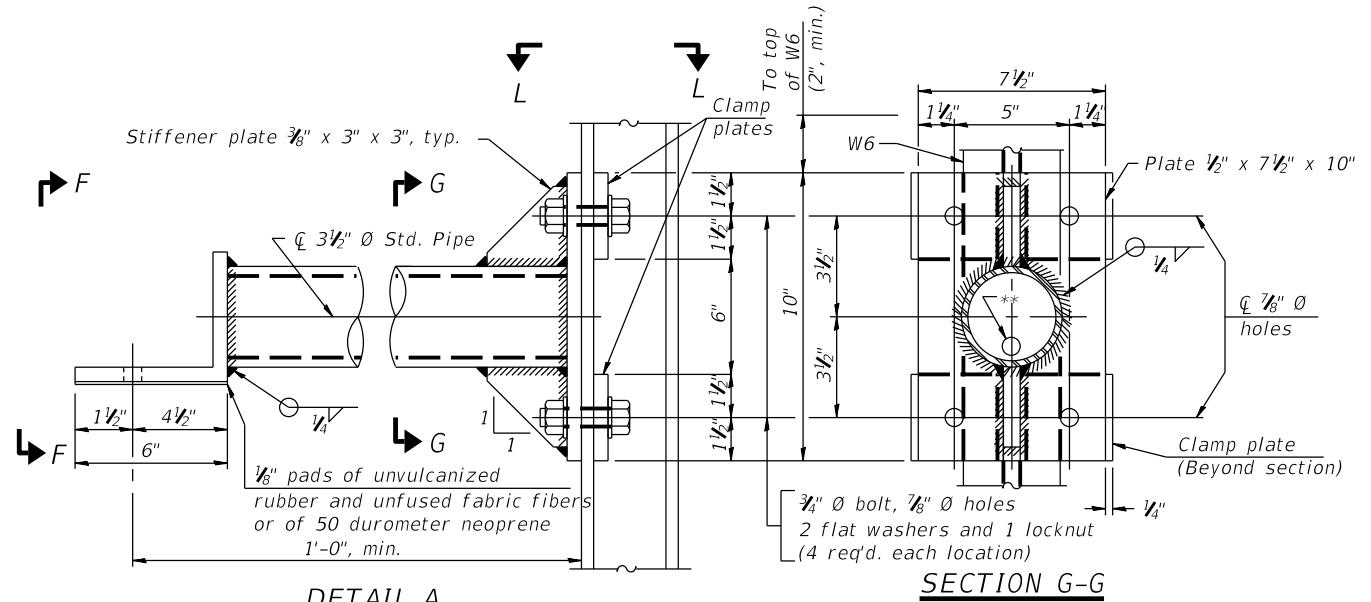
③ For bridge mounted sign structures installed on new bridges with railing, during design, bracket spacing must be coordinated with railing post spacing and the Contractor must install upper brackets prior to fence installation. If it is determined during design that existing railings can't be removed, alternate upper connection details must be developed for the contract plans and approved by the Bureau of Bridges and Structures.



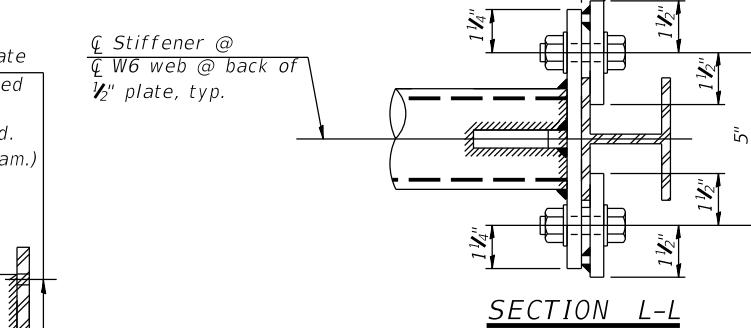
SECTION C-C
Steel beam or girder connection plate details



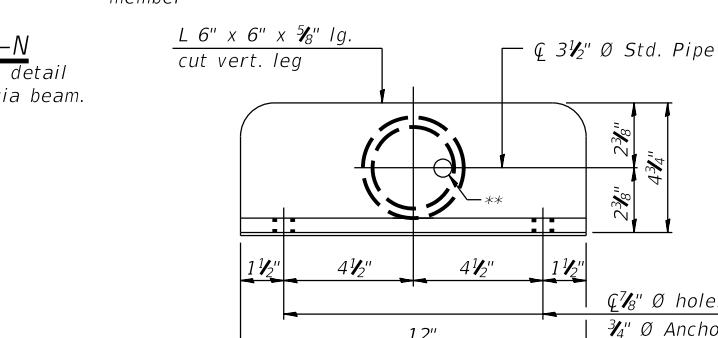
CLAMP PLATE DETAILS



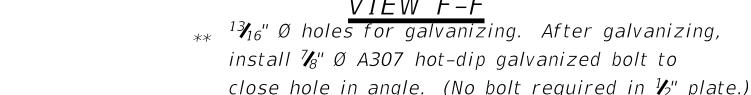
DETAIL A



SECTION L-L

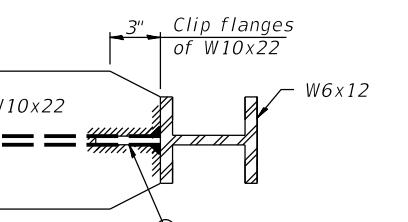


SECTION N-N
Skewed connection detail
for W10x22 to facia beam.

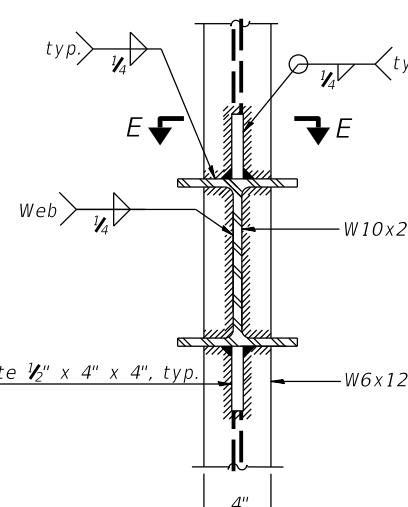


VIEW F-F

** 1/16" Ø holes for galvanizing. After galvanizing, install 1/8" Ø A307 hot-dip galvanized bolt to close hole in angle. (No bolt required in 1/2" plate.)



SECTION E-E



SECTION H-H

Structure Number	Station	h	i	j	k max. (10'-0" max.)	l max. (8'-0" max.)	m (15'-0" max.)
016-2114	601+21	7'-6"	1'-9"	1'-4"	3'-4"	1'-1"	6'-0"
016-2114	603+99.5	10'-6"	1'-9"	1'-4"	3'-4"	1'-1"	9'-0"

ADDED ENTIRE SHEET 12/31/2025

See sheet 105 of 161 for additional dimensions.

MODEL: Default
FILE NAME: W10x178-IDOT_205091-290_Loomis & Racine ADD Sheets6212_Loomis StreetStructural0162144-D16U12-xSign1.dgn

B
BLA, Inc.

USER NAME = *USER*	DESIGNED - TB	REVISED - 10/14/2025 PRD
CHECKED - PRD	REVISED -	
PLOT SCALE = NONE	DRAWN - TB	REVISED -
PLOT DATE = 12/29/2025	CHECKED - PRD	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BRIDGE MOUNT SIGN STRUCTURES DETAILS
STRUCTURE NO. 016-2114

F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
290	FAI 290 22 BRIDGE 3	COOK	161	161B
		ILLINOIS	FED. AID PROJECT	CONTRACT NO. 62U12