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2 SUMMARY OF QUANTITIES AND COMMITMENTS
3-73 PROPOSED STEEL AND BEARING FABRICATION PLANS



BENESCH
AMEE L. GIBSON
062-066832

Amee Gibson

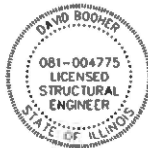
DATE: 07-21-2025
SHEETS: 1-2
LICENSE EXPIRATION DATE: 11-30-2025



BENESCH
JOHN GISLASON
081-006703

John Gislason

DATE: 07-21-2025
SHEETS: 3-29, 37-73
LICENSE EXPIRATION DATE: 11-30-2026



OURG ENGINEERING
DAVID BOOMER
081-004775

David Boomer

DATE: 07-21-2025
SHEETS: 30-36
LICENSE EXPIRATION DATE: 11-30-2026

DESIGN DESIGNATION

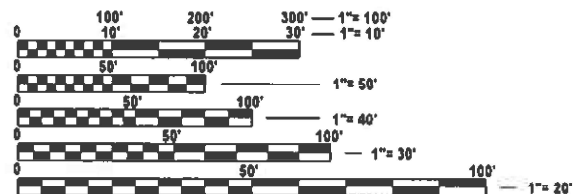
US 20: 4,345 (40) OTHER PRINCIPAL ARTERIAL 12 (JPC-20)
DESIGN SPEED: 45MPH POSTED SPEED: 45MPH
PV: 90% MU: 5% SU: 5%

STRUCTURES

US 20 OVER KISHWAUKEE RIVER
EXISTING SN 101-0073 /0074 (EB /WB)
PROPOSED SN 101-0225 /0226 (EB /WB)

TOWNSHIPS: CHERRY VALLEY

SECTIONS: 1



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123
OR 811

PROJECT MANAGER: ROBERT BARTON
SECTION ENGINEER: ANDREW LEE

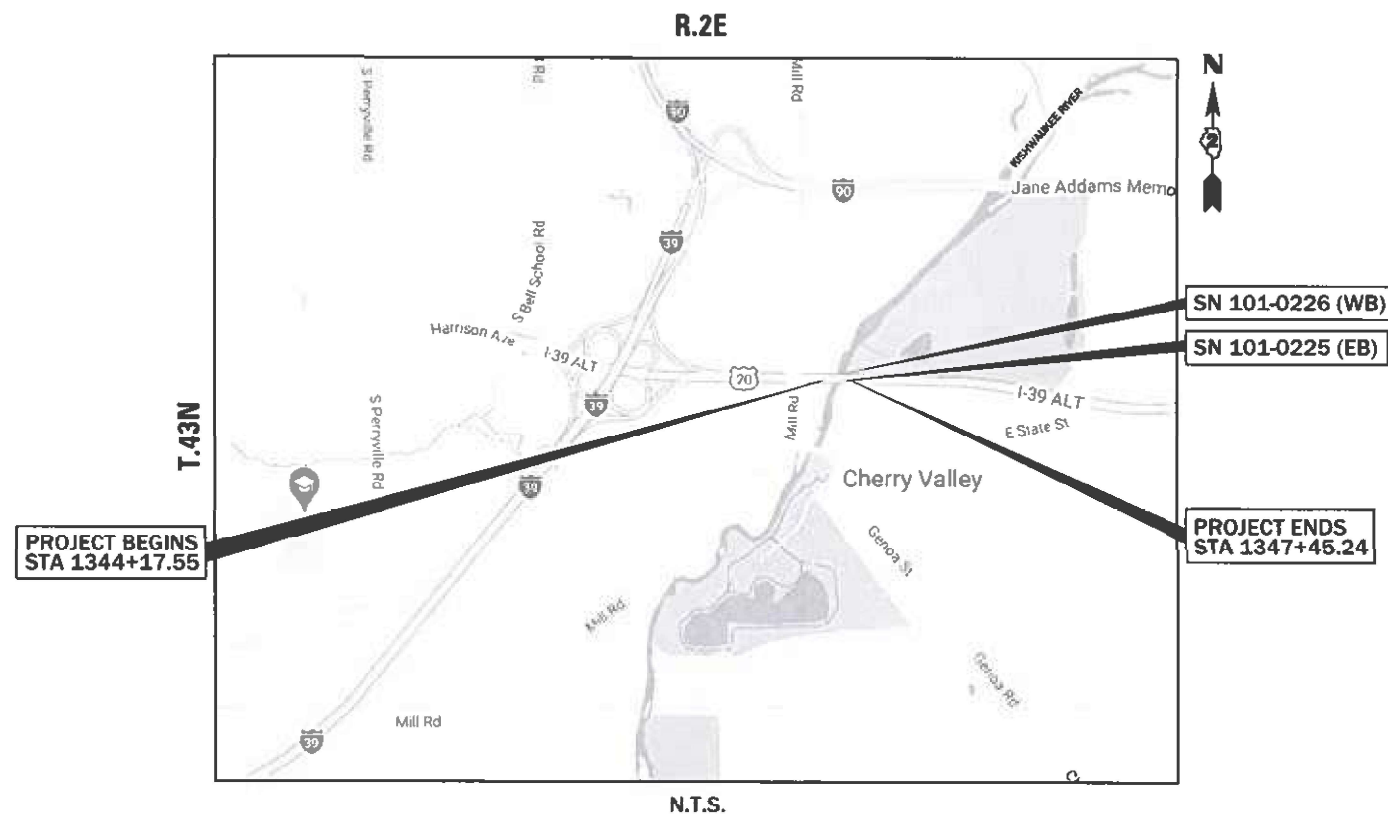
CONTRACT NO. 64U98

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PROPOSED HIGHWAY PLANS

FAP ROUTE 525 (US 20)
SECTION 6BF
PROJECT NHPP-EM1Y(611)
BRIDGE FABRICATION:
US 20 AT KISHWAUKEE RIVER
WINNEBAGO COUNTY

C-92-002-26



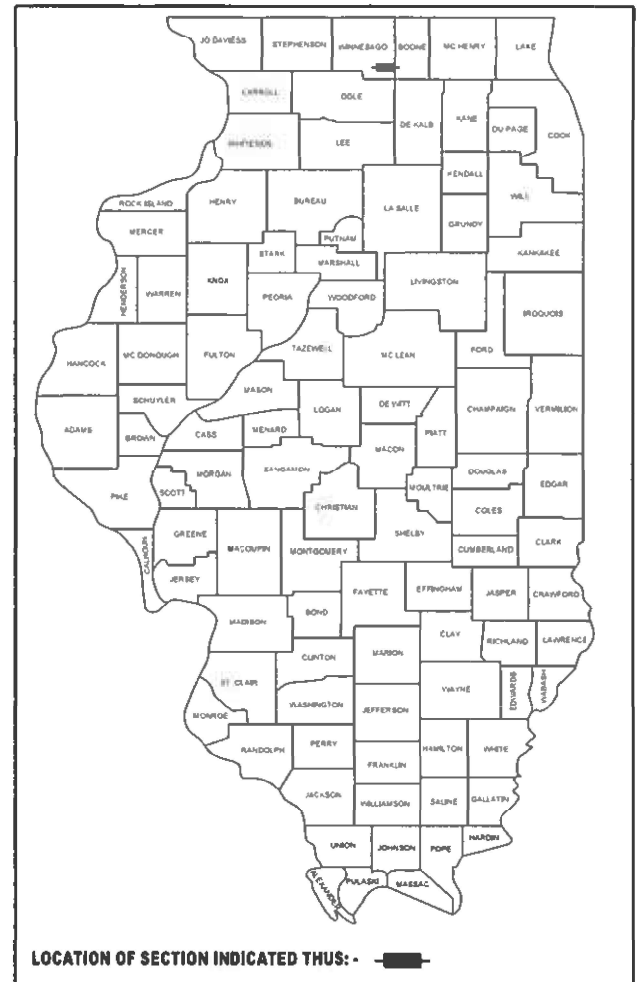
GROSS LENGTH = 327.69 FT. = 0.062 MILE
NET LENGTH = 327.69 FT. = 0.062 MILE



Alfred Benesch & Company
35 West Wacker Drive, Suite 3300
Chicago, Illinois 60601
312-565-0450

F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	1
6BF		ILLINOIS	CONTRACT NO. 64U98	

D-92-112-26



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUBMITTED August 6, 2025
Julia Dun REGIONAL ENGINEER

October 3, 2025
See E. A. Etk ENGINEER OF DESIGN AND ENVIRONMENT

October 3, 2025
Cherry DIRECTOR OF HIGHWAYS PROJECT IMPLEMENTATION

PRINTED BY THE AUTHORITY
OF THE STATE OF ILLINOIS

TREES THREE (3) INCHES OR GREATER IN DIAMETER AT BREAST HEIGHT CAN BE CLEARED FROM AUGUST 1ST THROUGH OCTOBER 15TH OF ANY GIVEN YEAR TO PROTECT THE QUEENS BEFORE THEY RETURN TO THEIR OVER-WINTERING BURROWS. THE US FISH AND WILDLIFE SERVICE CONCURRED WITH OUR DETERMINATION AND DATE RESTRICTION ON TREE CLEARING.


ALL TREE IMPACTS WILL BE MITIGATED UNDER INTER GOVERNMENTAL AGREEMENT NO. 21-IDNR-TREES-D2 WITH IDNR, THEREFORE NO TREE REPLACEMENT IS INCLUDED WITH THIS PROJECT. THIS AGREEMENT COVERS ALL PROJECTS FUNDED FROM FY 2025 THROUGH FY 2028.

[illegible]

Existing Structure: SN 101-0073 (EB) and SN 101-0074 (WB) were constructed in 1962 as Project F-284(18) under Section No. 6-B for FA Route 194. Each structure is a three (3) span bridge with rolled shape steel girders and a non-composite concrete deck. The superstructure is supported by stub abutments founded on concrete piles and trapezoidal piers founded on timber piles. Each existing structure is 230'-0" in length (back-to-back of abutments) and 35'-8" wide (out-to-out of deck). Traffic is to be maintained utilized stage construction.

Diagram illustrating the cross-section of a bridge structure, showing various components and elevations:

- 42" Web R Girder (composite full length)**: The main structural member.
- 44" Constant-Slope Parapet**: The outer edge of the bridge deck.
- 2'-0" typ.**: Typical width of the parapet base.
- Elev. 730.39 (WB)** and **Elev. 730.47 (EB)**: Elevation points on the left abutment.
- Elev. 712.00±**: Elevation of the top of the pier.
- 1:2 (V:H) typ.***: Slope of the pier face.
- Elev. 709.50**: Elevation of the base of the pier.
- Streambed Elev. 712.00±**: Elevation of the streambed.
- E.W.S. Elev. 716.75**: Elevation of the water surface.
- D.H.W. Elev. 727.50**: Elevation of the high water mark.
- 6'-7" min. vert. clr.**: Minimum vertical clearance.
- Elev. 730.02 (WB)** and **Elev. 730.10 (EB)**: Elevation points on the right abutment.
- Elev. 712.00±**: Elevation of the top of the pier.
- Elev. 709.50**: Elevation of the base of the pier.
- Min. Pile Tip Elev. 694.52, typ. at piers**: Minimum pile tip elevation.
- Stone Riprap, Class A5, typ.**: Material used for the streambed.
- Type 2 Cofferdam with Seal Coat, typ.**: Structure used for foundation work.
- Metal Shell Piles with Pile Shoes, 14"Ø with 0.312" walls, typ. at abutments**: Foundation piles at the abutments.
- Metal Shell Piles with Pile Shoes, 14"Ø with 0.312" walls, typ. at piers**: Foundation piles at the piers.

 Channel Excavation (See Roadway Plans for quantity)

* at Rt. L's

ELEVATION

APPROVED
For Structural Adequacy Only
James F. Schaff
Engineer of Bridges & Structures

2020 AASHTO LRFD Bridge Design
Specifications, 9th Edition

Allow 50#/sq. ft. for future wearing surface

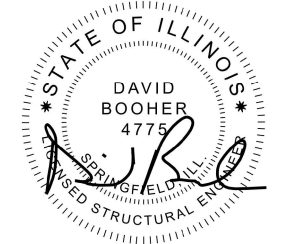
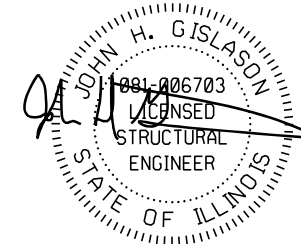
FIELD UNITS

$f'_c = 4,000 \text{ psi}$ (Superstructure)
 $f'_c = 3,500 \text{ psi}$ (Substructure)
 $f_y = 60,000 \text{ psi}$ (Reinforcement)
 $f_y = 50,000 \text{ psi}$ (M270 Grade 50) **

**** All structural steel shall be metallized.**

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration at 1.0 sec (SD1) = 0.079g
Design Spectral Acceleration at 0.2 sec (SDS) = 0.135g
Soil Site Class = D



SHEETS: 1-27 & 35-71 OF 71

SHEETS: 28-34 OF 71

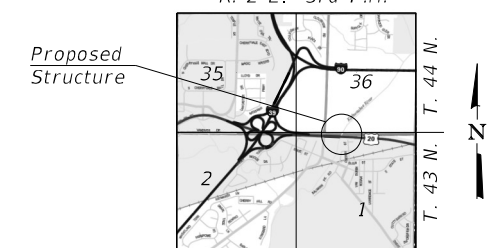
VPT Sta. 1343+51.00
Elev. 740.08

-0.14%

Sta. 1359+60.00
Elev. 737.86

Along U.S. 20 EB/WB edge of inside lane/shoulder
Note: The profile grade shows the final
elevations after grinding. Up to 1/4" may be
ground off the bridge deck and the bridge
approach slabs.

R. 2 E. 3rd P.M.



LOCATION SKETCH

GENERAL PLAN & ELEVATION

U.S. ROUTE 20 OVER KISHWAUKEE RIVER

F.A.P. ROUTE 525 - SECTION 6BF

WINNEBAGO COUNTY

STATION 1345+82.83

STRUCTURE NO. 101-0225 (EB)

STRUCTURE NO. 101-0226 (WB)

NOTES:

1. See Sheet 3 of 71 for Section A-A and B-B.
2. Elevations in plan represent elevations after grinding.
3. Protective shielding shall be required along the full length of the existing middle span and 15 ft beyond the pier of the existing end spans during removal of the existing deck.



benesch
 Alfred Benesch & Company
 35 W Wacker Drive, Suite 3300
 Chicago, Illinois 60601
 312-565-0450 Job No. 10800

USER NAME =	DESIGNED - JPM	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE =	DRAWN - RMG	REVISED -
PLOT DATE =	CHECKED - JHG	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

SHEET 1 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	3
CONTRACT NO. 64U98				
ILLINOIS		EED. AND PROJECT		

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GENERAL NOTES:

- 1. Fasteners shall be ASTM F 3125 Grade A325 Type 1, hot dip galvanized bolts in metallized areas. Bolts 7/8" diameter, holes 15/16" diameter, unless otherwise noted. See Special Provision for "Metallizing of Structural Steel".
- 2. Calculated weight of Structural Steel (Grade 50) = 870,000 lbs. and Structural Steel (Grade 36) = 50,000 lbs.
- 3. All structural steel shall be metallized. See Special Provision for "Metallizing of Structural Steel."
- 4. It is anticipated that the delivery of the structural steel and bearings will be required by September 8, 2026 for Stage 1 and June 1, 2027 for Stage 2. The delivery dates shall be coordinated with IDOT and the Contractor responsible for Contract No. 64R72. Shop drawings for both stages shall be submitted for approval at the same time prior to Stage 1 fabrication.
- 5. These plans are for fabrication and storage of the structural steel and bearings. All work shown related to the erection and installation of the structural steel and bearings is for information only and is to be included in Contract 64R72.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Furnishing Structural Steel	L Sum	1		1
Storage of Structural Steel and Bearings - Stage 1	Cal Da	365		365
Storage of Structural Steel and Bearings - Stage 2	Cal Da	365		365

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Alfred Benesch & Company

35 W Wacker Drive, Suite 3300

Chicago, Illinois 60601

312.465.4150

Job No. 10800

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		CHECKED	-	JHG	REVISED	-
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PLOT DATE	=	CHECKED	-	JHG	REVISED	-

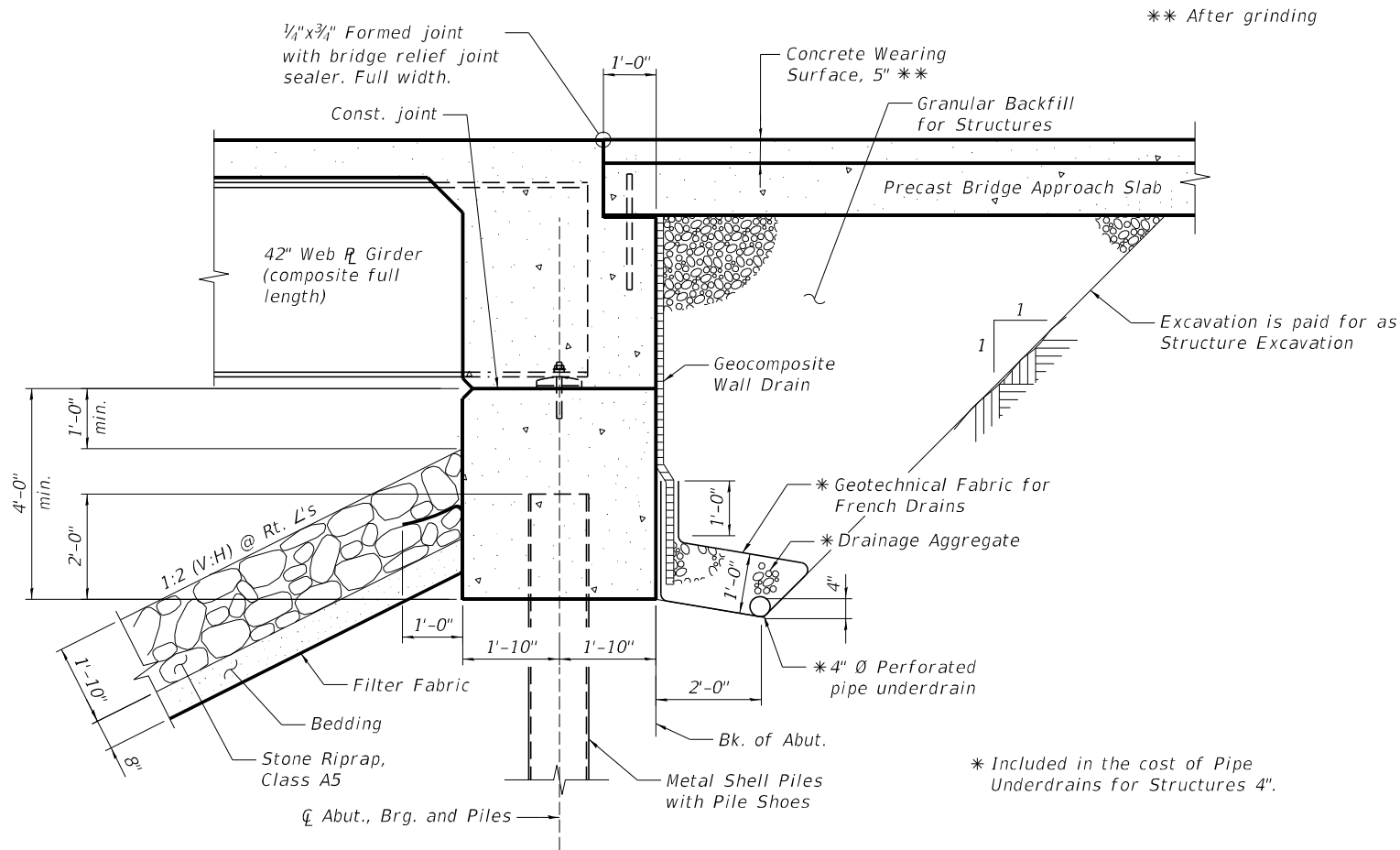
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL NOTES, BILL OF MATERIALS AND INDEX OF SHEETS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 2 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	4
		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

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SECTION THRU INTEGRAL ABUTMENT

(Horiz. dim. @ Rt. L's)

Note: All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

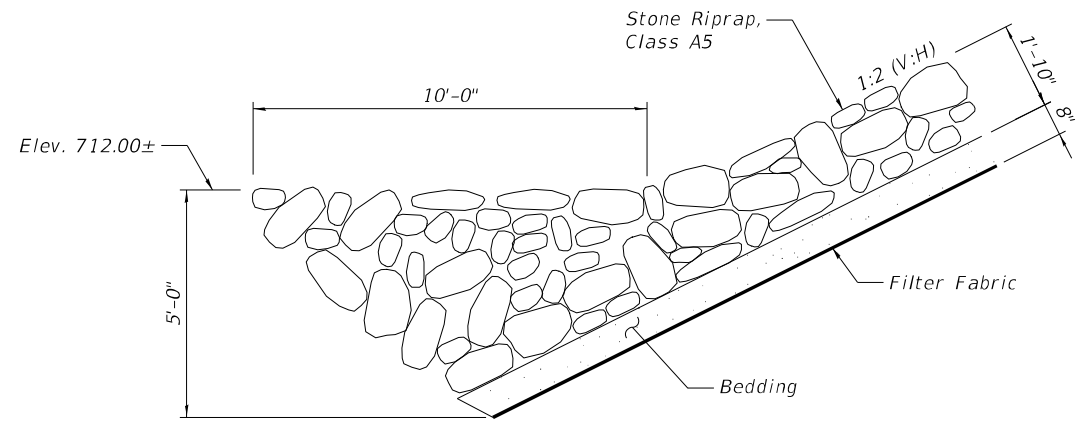
WATERWAY INFORMATION

Drainage Area = 628.0 sq. mi.		Exist. Overtopping Elevation = 737.0 at Station 1359+50 Prop. Overtopping Elevation = 737.0 at Station 1359+50							
Flood	Freq. Yr.	Q (C.F.S.)	Opening (Ft ²)		Natural H.W.E.	Head (Ft)		Headwater El.	
			Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
Design	10	10,380	1,836	2,058	725.3	0.2	0.1	725.5	725.4
Base	50	16,100	2,220	2,535	727.5	0.5	0.2	728.0	727.7
Check	100	18,800	2,382	2,735	728.4	0.6	0.3	729.0	728.7
Max. Calc.	200	21,500	2,567	2,962	729.4	0.7	0.4	730.1	729.8
	500	25,100	2,851	3,309	730.9	0.8	0.4	731.7	731.3

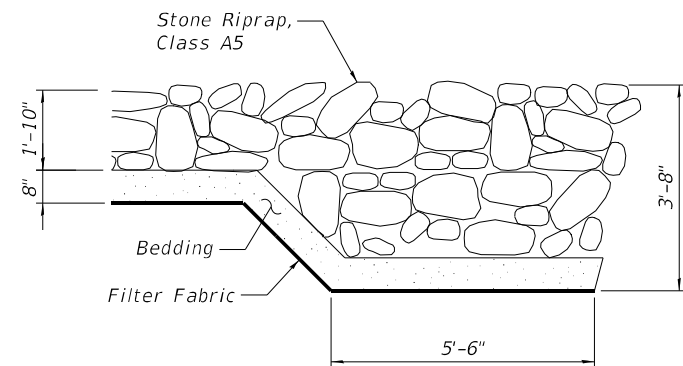
10-year velocity through existing bridge = 5.7 ft/sec
10-year velocity through proposed bridge = 4.7 ft/sec

DESIGN SCOUR ELEVATION TABLE

Event / Limit	Design Scour Elevations (ft.)				Item 113
	W. Abut.	Pier 1	Pier 2	E. Abut.	
Q100	730.32	704.52	704.52	729.96	5
Q200	730.32	701.84	701.84	729.96	
Design	730.32	704.52	704.52	729.96	
Check	730.32	701.84	701.84	729.96	



SECTION A-A



SECTION B-B

STATION 1345+82.83
BUILT 202_ BY
STATE OF ILLINOIS
F.A.P. RT. 525
SEC. (15X)RC & 5RS
LOADING HL-93
STR. NO. 101-0225

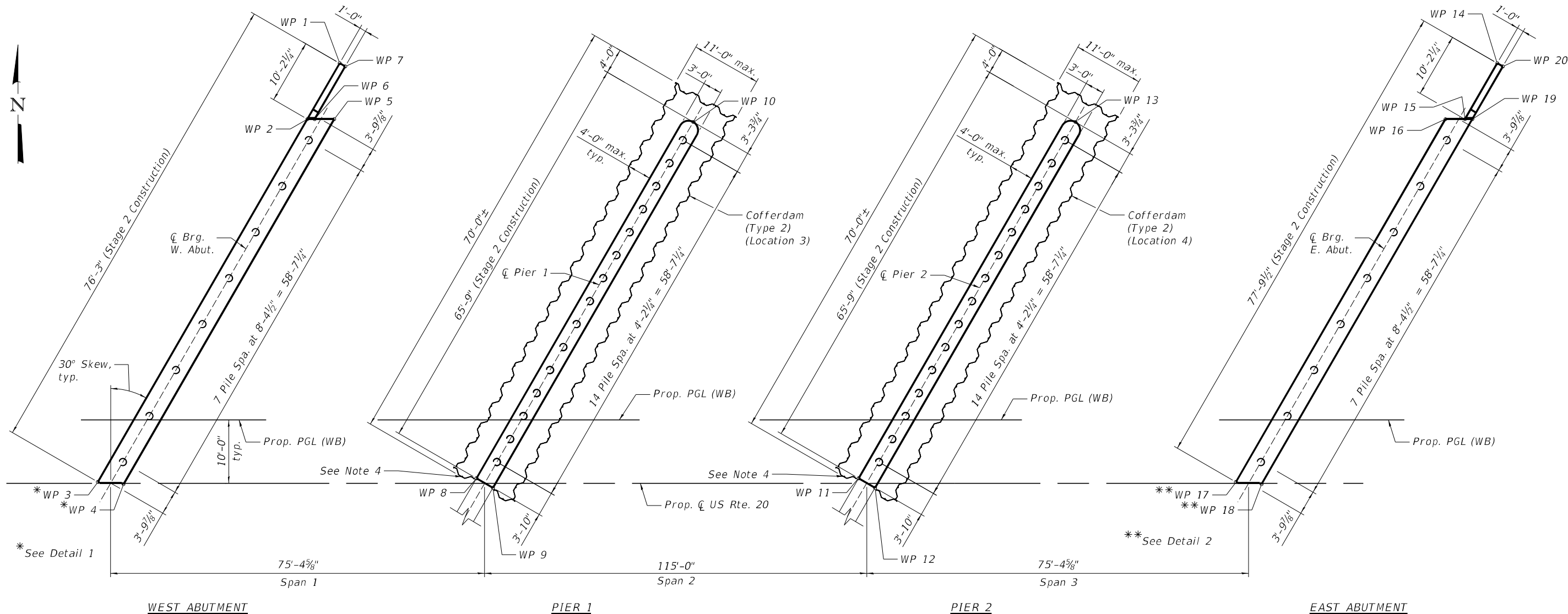
NAME PLATE (EB)
See Std. 515001

STATION 1345+82.83
BUILT 202_ BY
STATE OF ILLINOIS
F.A.P. RT. 525
SEC. (15X)RC & 5RS
LOADING HL-93
STR. NO. 101-0226

NAME PLATE (WB)
See Std. 515001

FOR INFORMATION ONLY

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FOUNDATION LAYOUT PLAN (WESTBOUND)

W. ABUT. WORK POINTS

W.P.	Station	Offset
1	1344+84.64	66.24' Lt.
2	1344+79.54	57.42' Lt.
3	1344+46.51	00.21' Lt.
4	1344+50.55	00.13' Rt.
5	1344+83.77	57.42' Lt.
6	1344+80.69	57.42' Lt.
7	1344+85.50	65.74' Lt.

PIER 1 WORK POINTS

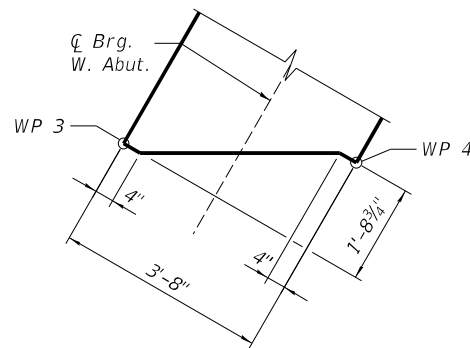
W.P.	Station	Offset
8	1345+22.61	00.79' Lt.
9	1345+25.21	00.71' Rt.
10	1345+56.79	56.98' Lt.

PIER 2 WORK POINTS

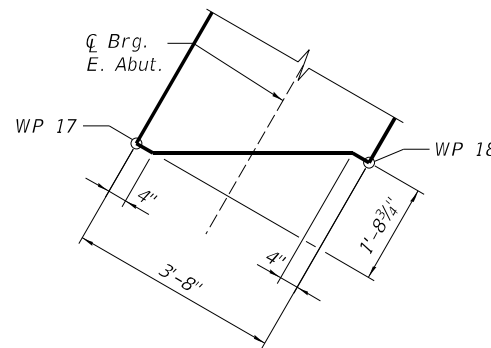
W.P.	Station	Offset
11	1346+37.61	00.79' Lt.
12	1346+40.21	00.71' Rt.
13	1346+71.79	56.98' Lt.

E. ABUT. WORK POINTS

W.P.	Station	Offset
14	1347+53.48	66.24' Lt.
15	1347+48.39	57.42' Lt.
16	1347+45.31	57.42' Lt.
17	1347+12.28	00.21' Lt.
18	1347+16.32	00.13' Rt.
19	1347+49.54	57.42' Lt.
20	1347+54.35	65.74' Lt.



DETAIL 1



DETAIL 2

LEGEND

○ = Vertical Pile

NOTES:

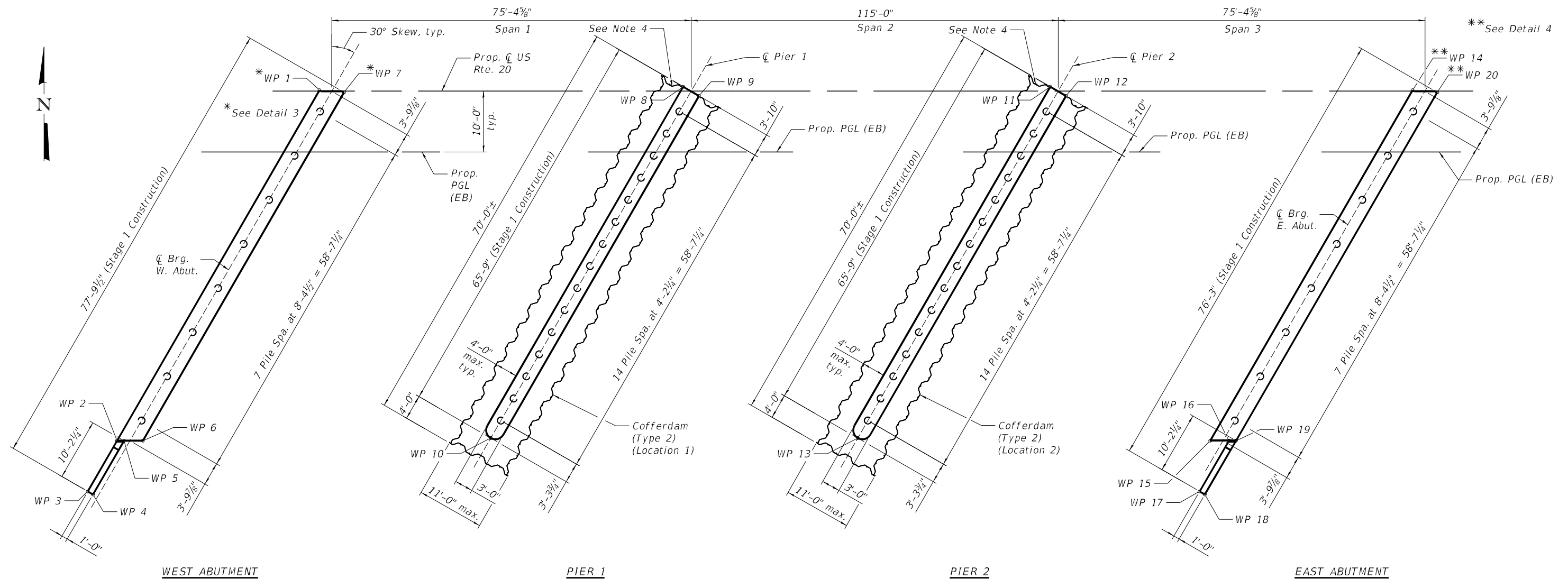
- See Sheets 57 to 61 of 71 for pier details.
- See Sheets 52 to 56 of 71 for abutment details.
- All stations and offsets are measured from Prop. ∇ US Rte. 20.
- The Contractor is responsible for determining how the cofferdam will be constructed and used between Stage 1 and Stage 2 construction.
- The distance from face of proposed pier to outside face of cofferdam shall be 4'-0" maximum due to potential conflicts with the existing pier foundations. Removal of existing pier footings or piles shall be at no additional cost to the State.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FOUNDATION LAYOUT PLAN (1 OF 2)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 4 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	6
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				



FOUNDATION LAYOUT PLAN (EASTBOUND)

W. ABUT. WORK POINTS

W.P.	Station	Offset
1	1344+46.46	00.12' Lt.
2	1344+13.24	57.42' Rt.
3	1344+08.43	65.74' Rt.
4	1344+09.30	66.24' Rt.
5	1344+14.40	57.42' Rt.
6	1344+17.47	57.42' Rt.
7	1344+50.50	00.21' Rt.

PIER 1 WORK POINTS

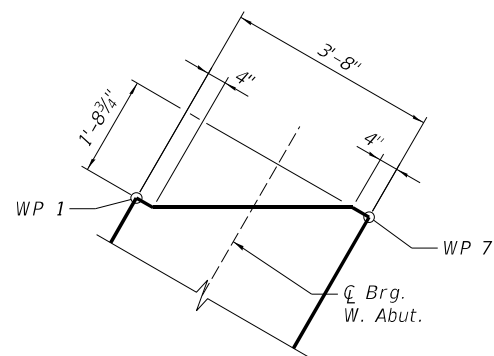
W.P.	Station	Offset
8	1345+22.57	00.71' Lt.
9	1345+25.17	00.79' Rt.
10	1344+90.99	56.98' Rt.

E. ABUT. WORK POINTS

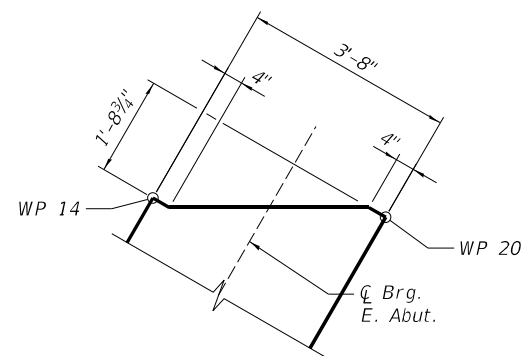
W.P.	Station	Offset
14	1347+12.23	00.12' Lt.
15	1346+79.01	57.42' Rt.
16	1346+82.09	57.42' Rt.
17	1346+77.28	65.74' Rt.
18	1346+78.14	66.24' Rt.
19	1346+83.24	57.42' Rt.
20	1347+16.27	00.21' Rt.

PIER 2 WORK POINTS

W.P.	Station	Offset
11	1346+37.57	00.71' Lt.
12	1346+40.17	00.79' Rt.
13	1346+05.99	56.98' Rt.



DETAIL 3



DETAIL 4

LEGEND

⊙ = Vertical Pile

NOTES:

- See Sheets 57 to 61 of 71 for pier details.
- See Sheets 52 to 56 of 71 for abutment details.
- All stations and offsets are measured from Prop. ζ US Rte. 20.
- The Contractor is responsible for determining how the cofferdam will be constructed and used between Stage 1 and Stage 2 construction.
- The distance from face of proposed pier to outside face of cofferdam shall be 4'-0" maximum due to potential conflicts with the existing pier foundations. Removal of existing pier footings or piles shall be at no additional cost to the State.

FOR INFORMATION ONLY

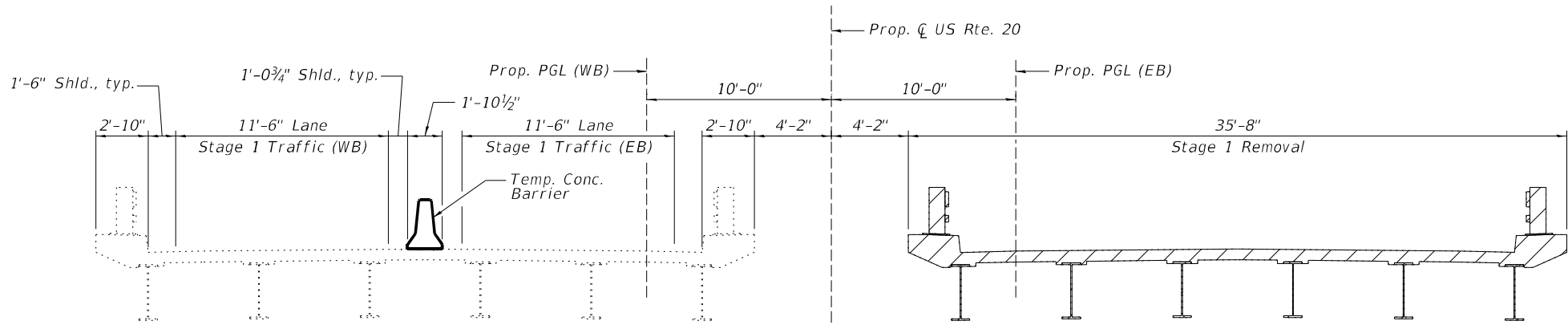
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FOUNDATION LAYOUT PLAN (2 OF 2)
STRUCTURE NO. 101-0225 & 101-0226

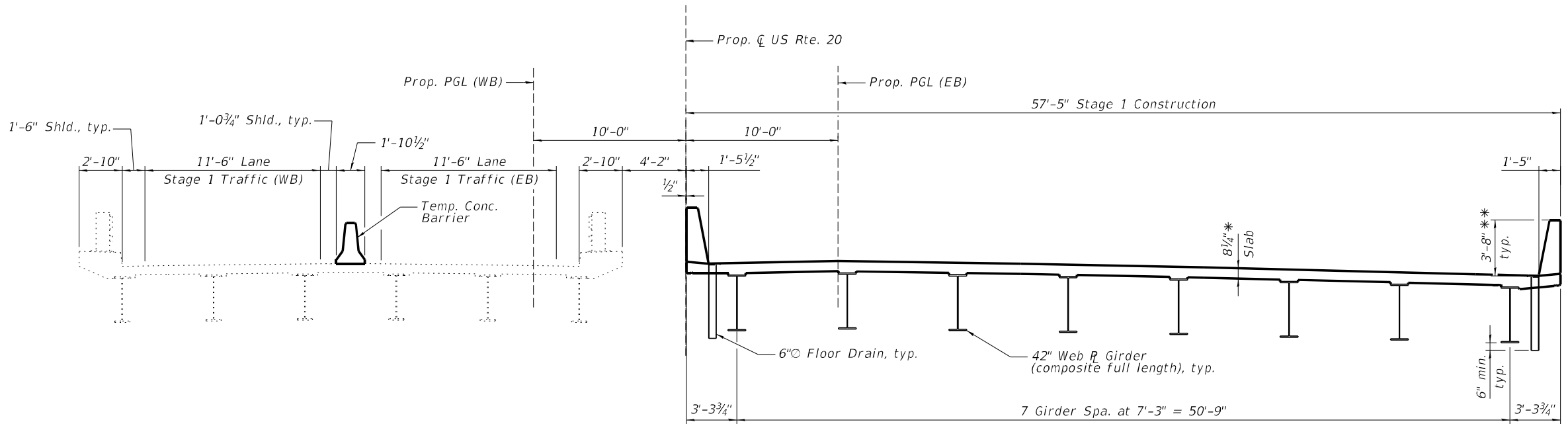
SHEET 5 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	7
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

USER NAME =	DESIGNED - JPM	REVISED -
PLOT SCALE =	CHECKED - JHG	REVISED -
PLOT DATE =	DRAWN - RMG	REVISED -
	CHECKED - JHG	REVISED -



STAGE 1 REMOVAL
(Looking East)



STAGE 1 CONSTRUCTION
(Looking East)

* Prior to grinding
** After grinding

- NOTES:**
1. See Sheets 10 and 11 of 71 for substructure removal lines.
 2. For quantity of Temporary Concrete Barrier, see Roadway Plans.
 3. Hatched area indicates Removal of Existing Structures No. 1 (EB) or No. 2 (WB).
 4. See Sheet 9 of 71 for Temporary Concrete Barrier details.

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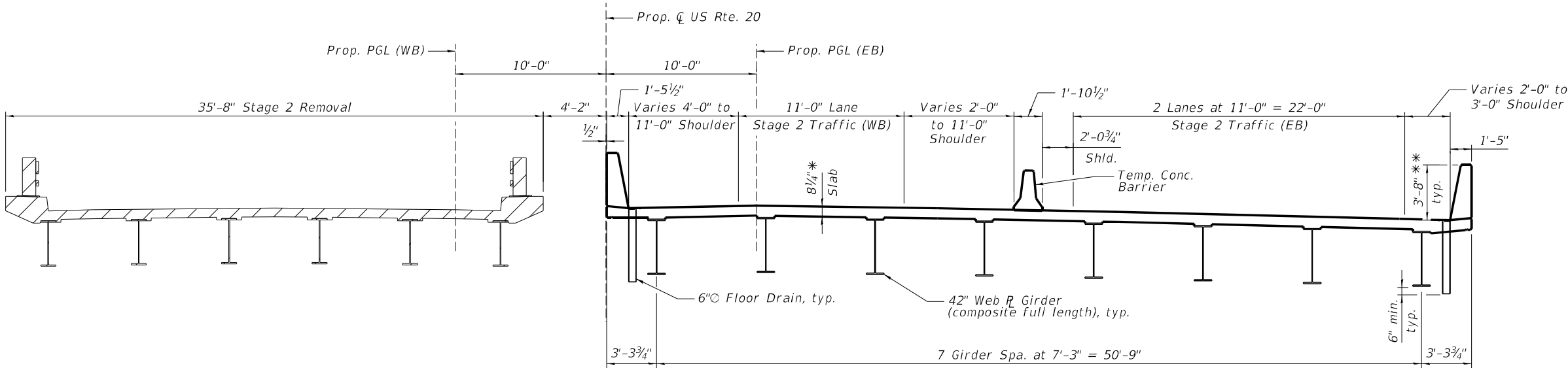
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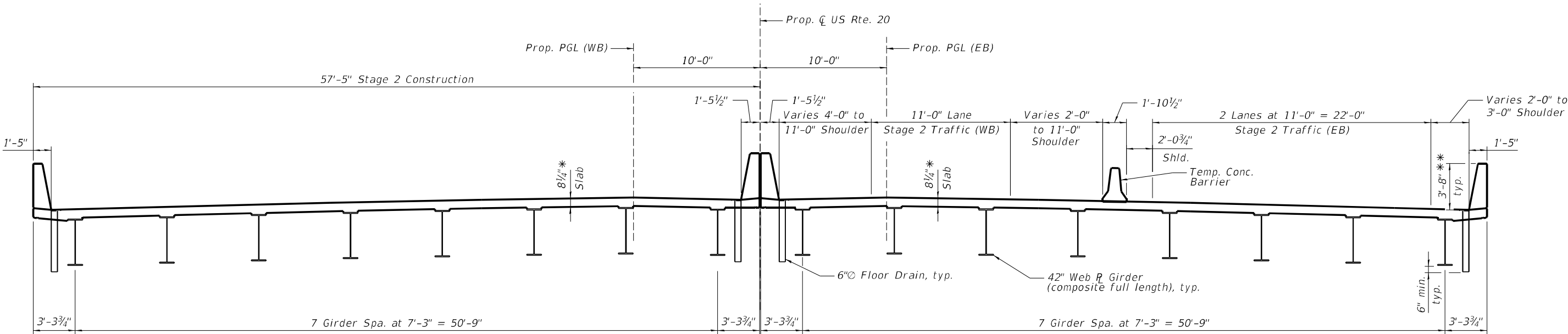
**STAGE CONSTRUCTION DETAILS (1 OF 3)
STRUCTURE NO. 101-0225 & 101-0226**

SHEET 6 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	8
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



STAGE 2 REMOVAL
(Looking East)



STAGE 2 CONSTRUCTION
(Looking East)

* Prior to grinding
** After grinding

- NOTES:
1. See Sheets 10 and 11 of 71 for substructure removal lines.
 2. For quantity of Temporary Concrete Barrier, see Roadway Plans.
 3. Hatched area indicates Removal of Existing Structures No. 1 (EB) or No. 2 (WB).
 4. See Sheet 9 of 71 for Temporary Concrete Barrier details.

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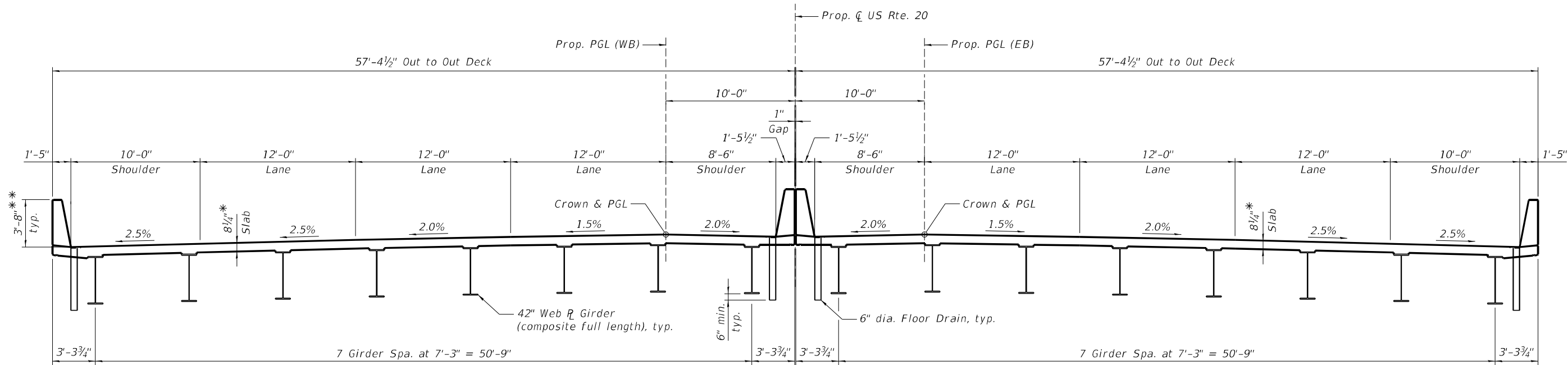
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STAGE CONSTRUCTION DETAILS (2 OF 3)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 7 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	9
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



CROSS SECTION - FINAL CONDITION
(Looking East)

* Prior to grinding
** After grinding

- NOTES:**
- 1. See Sheets 10 and 11 of 71 for substructure removal lines.
 - 2. For quantity of Temporary Concrete Barrier, see Roadway Plans.
 - 3. Hatched area indicates Removal of Existing Structures No. 1 (EB) or No. 2 (WB).
 - 4. See Sheet 9 of 71 for Temporary Concrete Barrier details.

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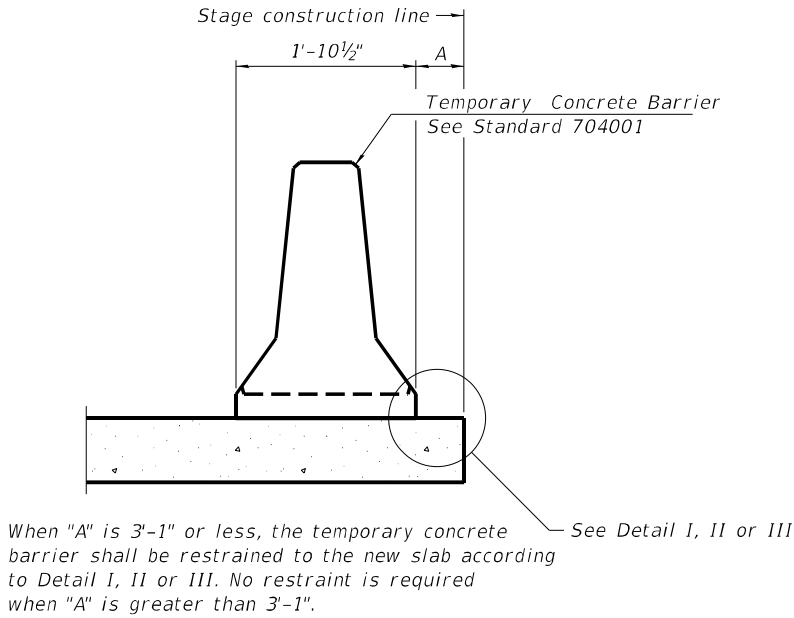
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STAGE CONSTRUCTION DETAILS (3 OF 3)
STRUCTURE NO. 101-0225 & 101-0226

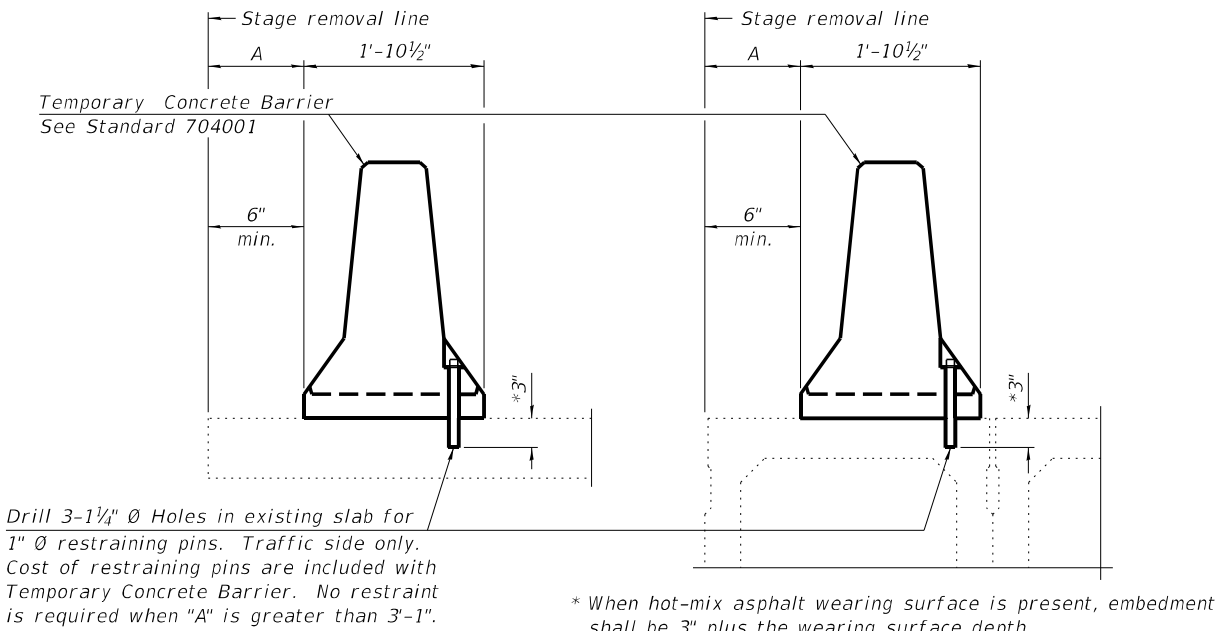
SHEET 8 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	10
CONTRACT NO. 64U98				
		ILLINOIS	FED. AID PROJECT	

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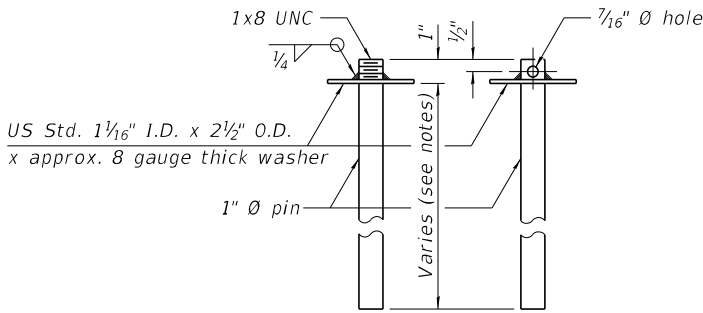


NEW SLAB OR NEW DECK BEAM



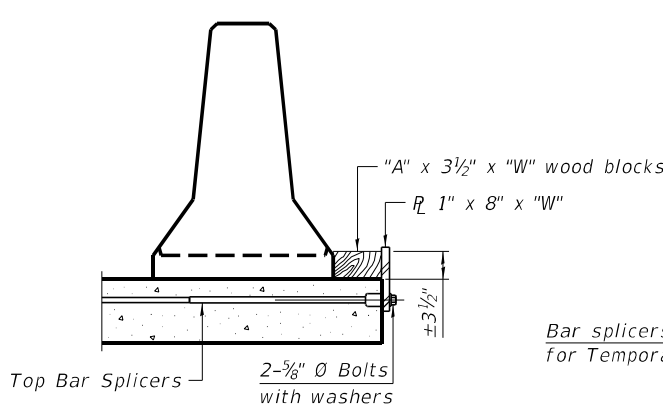
EXISTING SLAB

EXISTING DECK BEAM

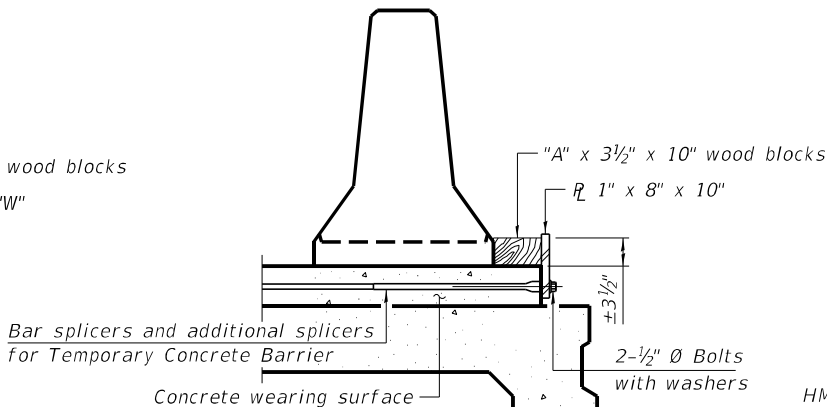


RESTRAINING PIN

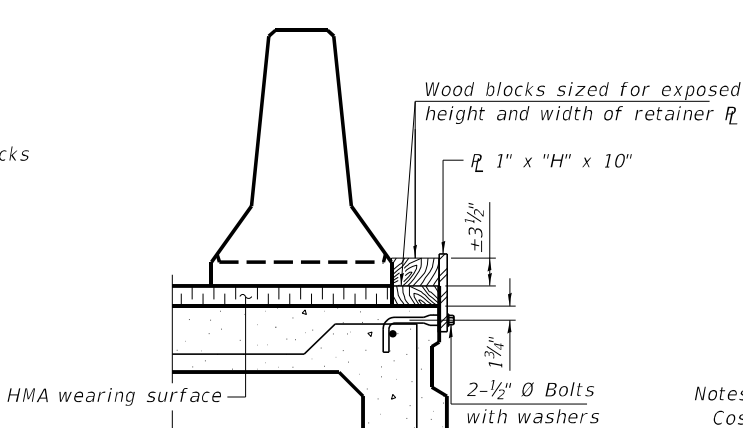
SECTIONS THRU SLAB OR DECK BEAM



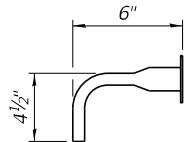
DETAIL I



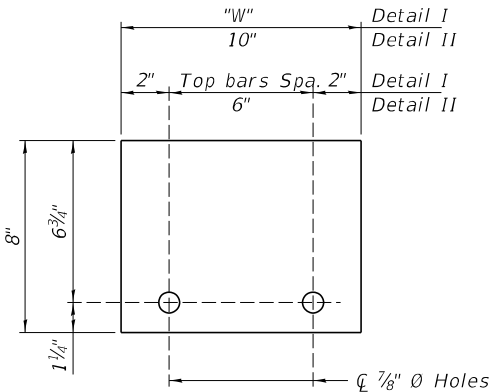
DETAIL II



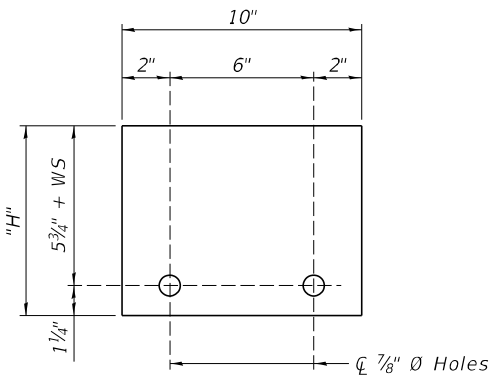
DETAIL III



BAR SPLICER FOR #4 BAR - DETAIL III



STEEL RETAINER R 1" x 8" x "W"
(Detail I and II)



STEEL RETAINER R 1" x "H" x 10"
(Detail III)

Notes:
Cost of retainer assembly is included with Temporary Concrete Barrier.
A retainer assembly shall be located at the approximate □ of each temporary concrete barrier.
The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.
When the 'A' dimension is less than 1 1/2", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

Detail I - Installation for a new bridge deck or bridge slab.

Detail II - Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

RAILING CRITERIA

NCHRP 350 Test Level	3
Railing Weight (plf)	440

R-27

5-15-2023

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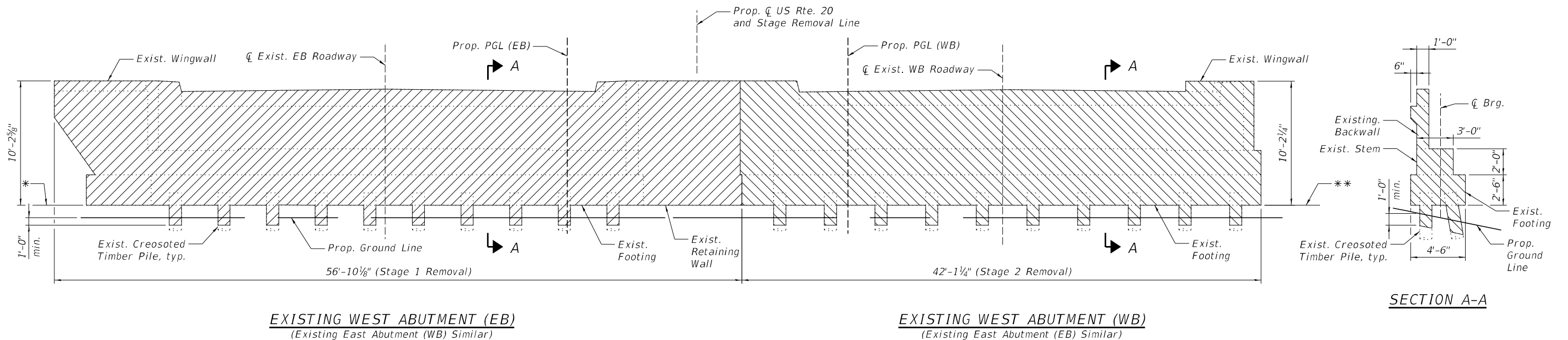
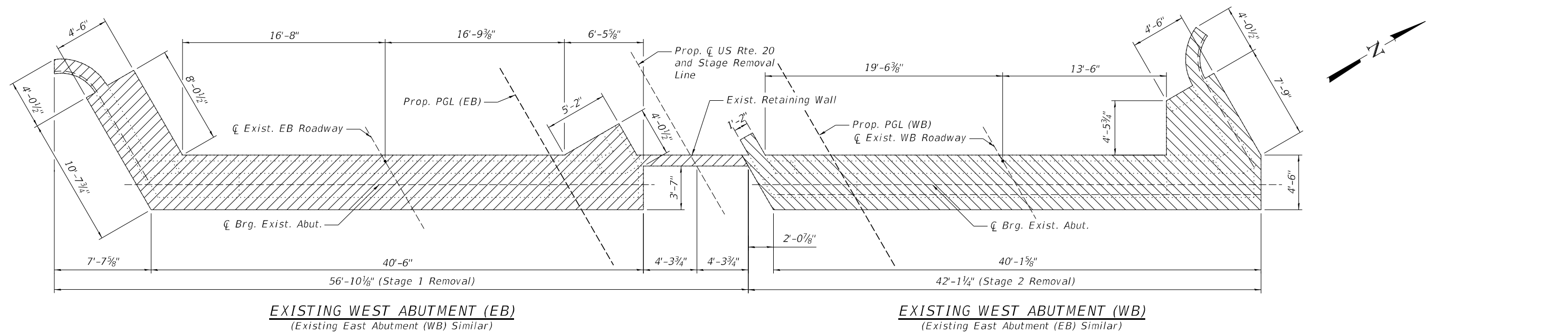
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PLOT DATE =	CHECKED - JHG	REVISED -

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TEMPORARY CONCRETE BARRIER
STRUCTURE NO. 101-0225 & 101-0226

SHEET 9 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	11
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				



* Elev. 730.47 (West Abut. EB)
Elev. 730.11 (East Abut. EB)
** Elev. 730.43 (West Abut. WB)
Elev. 730.07 (East Abut. WB)

LEGEND

- Stage 1 Removal
- Stage 2 Removal

NOTES:

1. All removal dimensions and details are approximate based on the existing plans and shall be confirmed by the Contractor prior to beginning removal.
2. Removal of Existing Structures No. 1 is for EB and Removal of Existing Structures No. 2 is for WB.
3. Removal limits of the superstructure and substructure differ. Work this sheet with superstructure removal limits and staging shown on Sheets 6 to 8 of 71.

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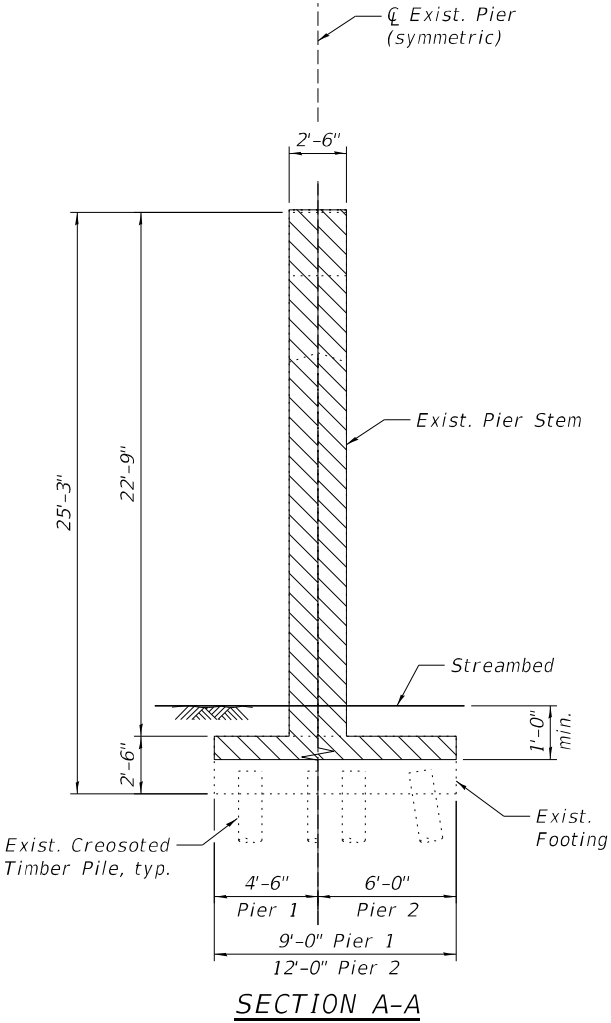
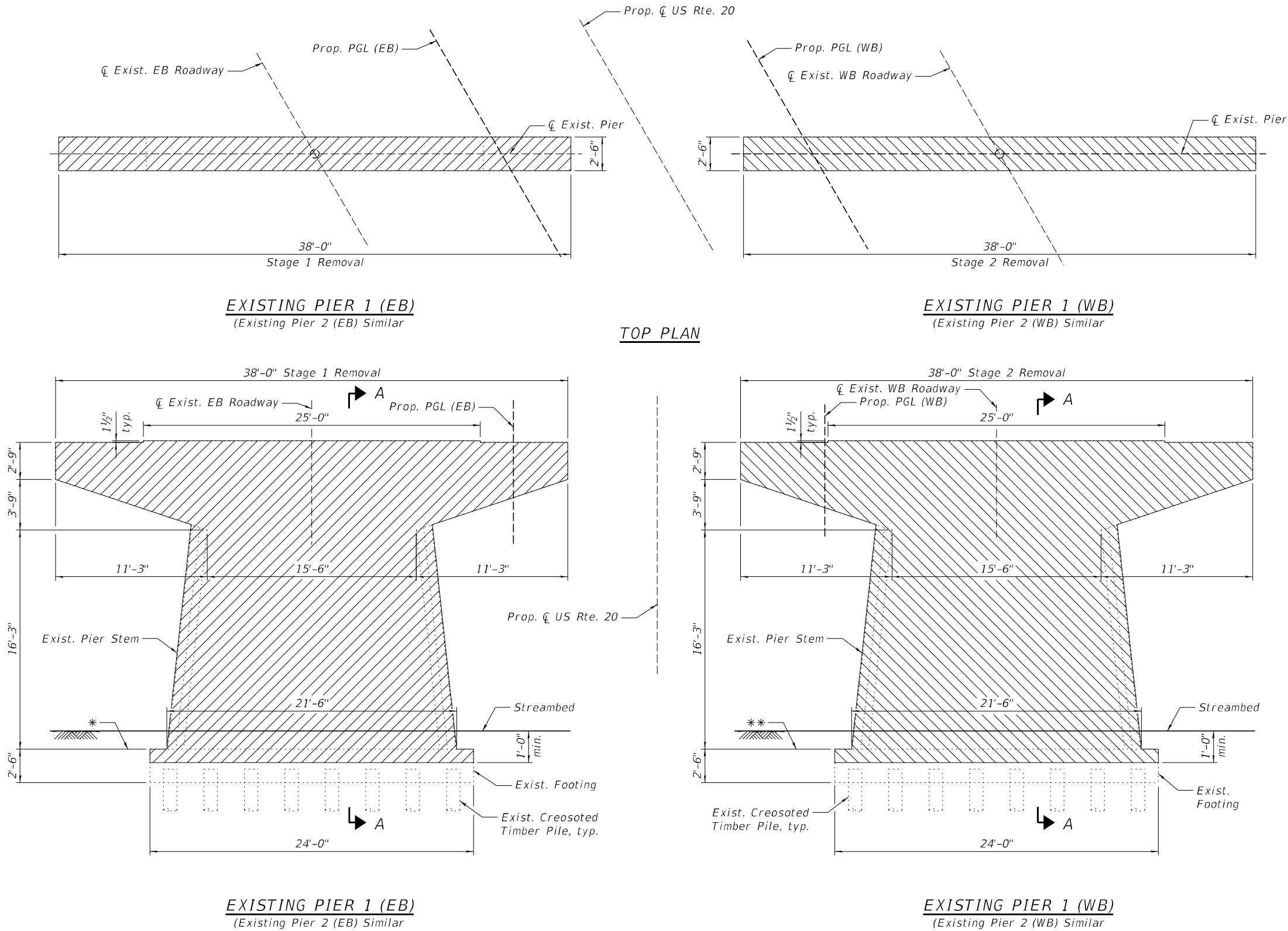
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PIER REMOVAL DETAILS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 11 OF 71 SHEETS

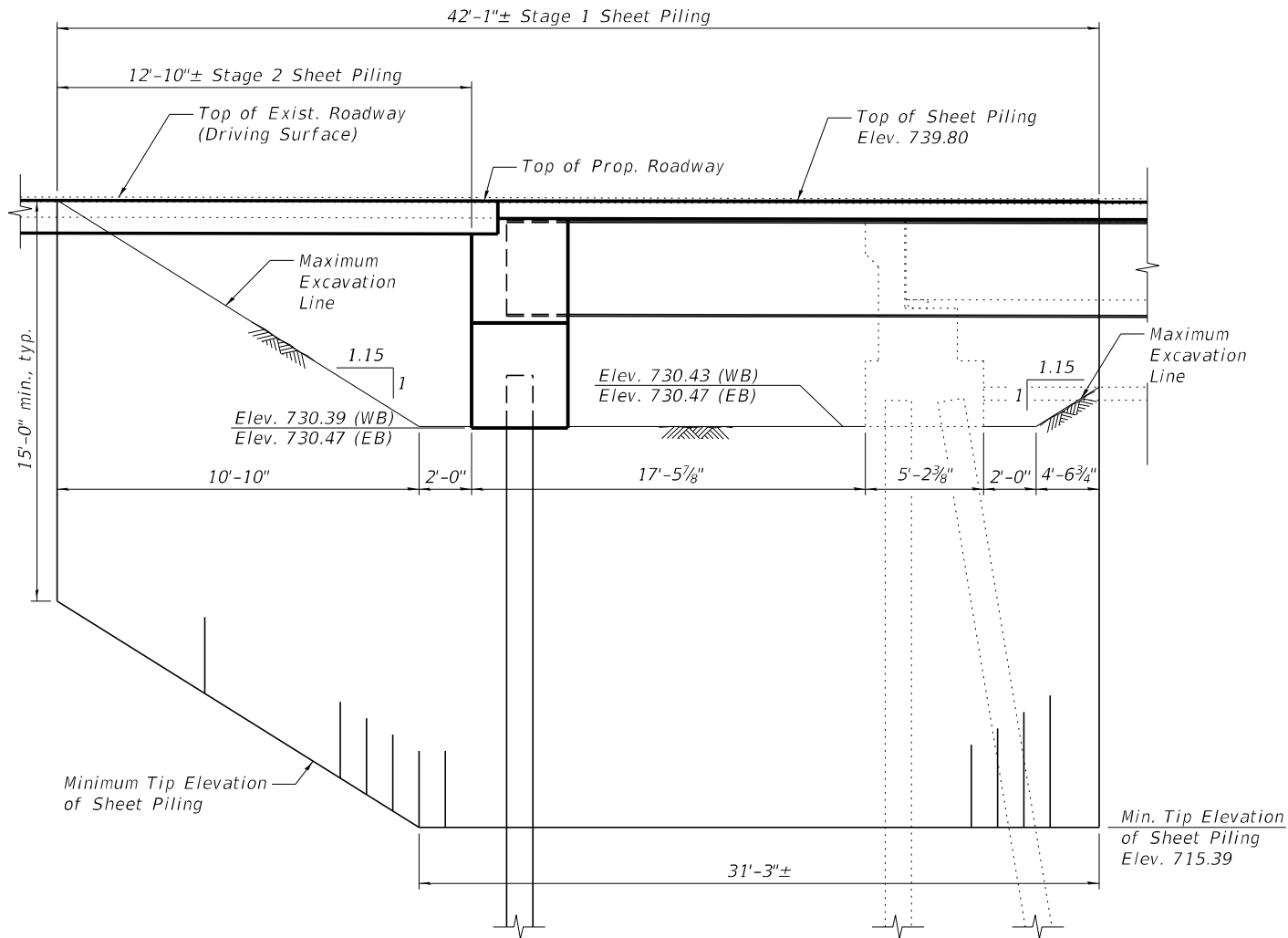
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	13
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



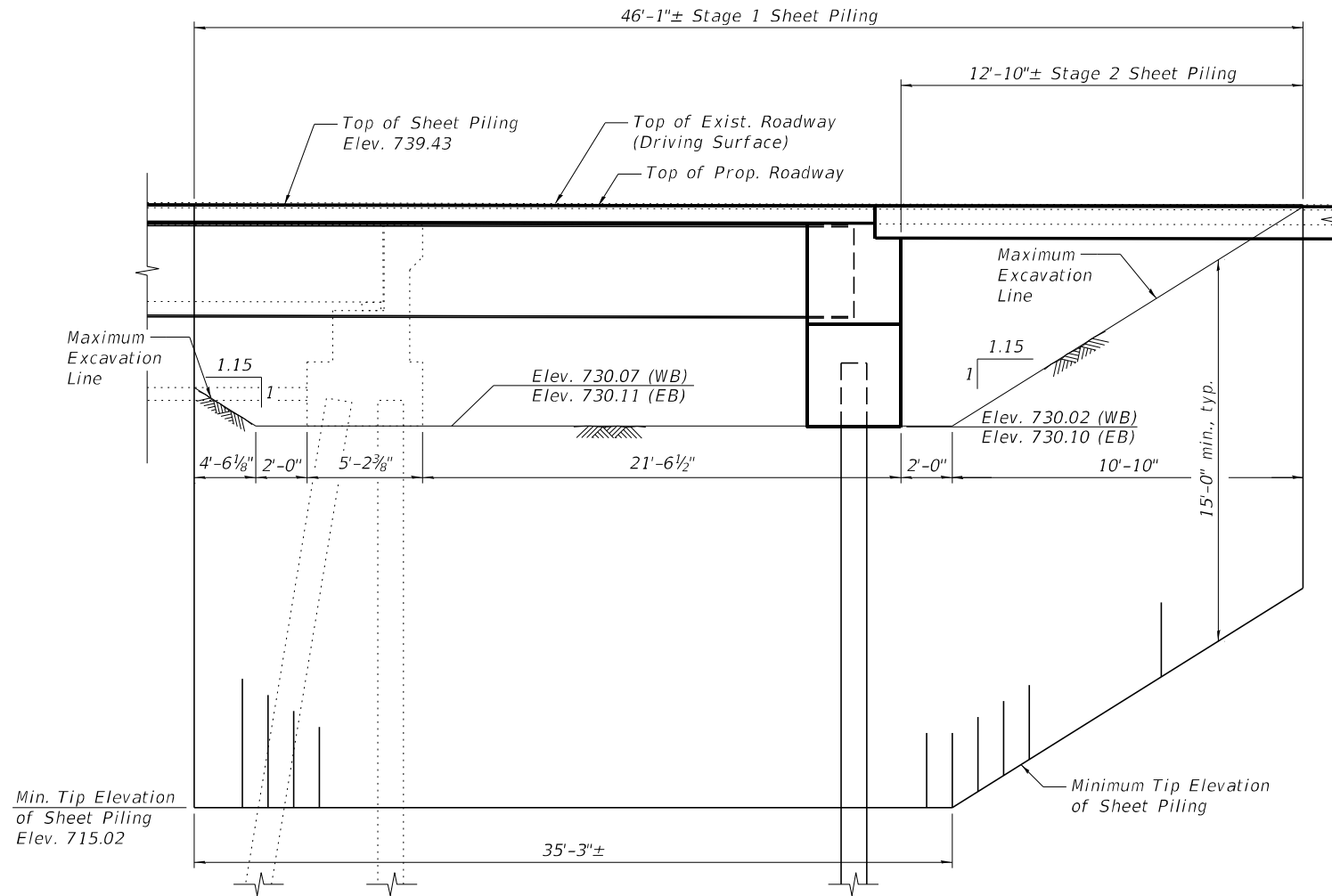
- NOTES:**
- All removal dimensions and details are approximate based on the existing plans and shall be confirmed by the Contractor prior to beginning removal.
 - Removal of Existing Structures No. 1 is for EB and Removal of Existing Structures No. 2 is for WB.
 - Removal limits of the superstructure and substructure differ. Work this sheet with superstructure removal limits and staging shown on Sheets 6 to 8 of 71.

LEGEND	
	Stage 1 Removal
	Stage 2 Removal

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TEMPORARY SHEET PILING - WEST ABUTMENT
(Looking North)
(Horizontal dimensions given along the skew)



TEMPORARY SHEET PILING - EAST ABUTMENT
(Looking North)
(Horizontal dimensions given along the skew)

NOTES:

- See Sheet 1 of 71 for plan view of Temporary Sheet Piling.
- If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer.
- The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling.
- The minimum section modulus for the Temporary Sheet Piling shall be 18.1 in. /ft.

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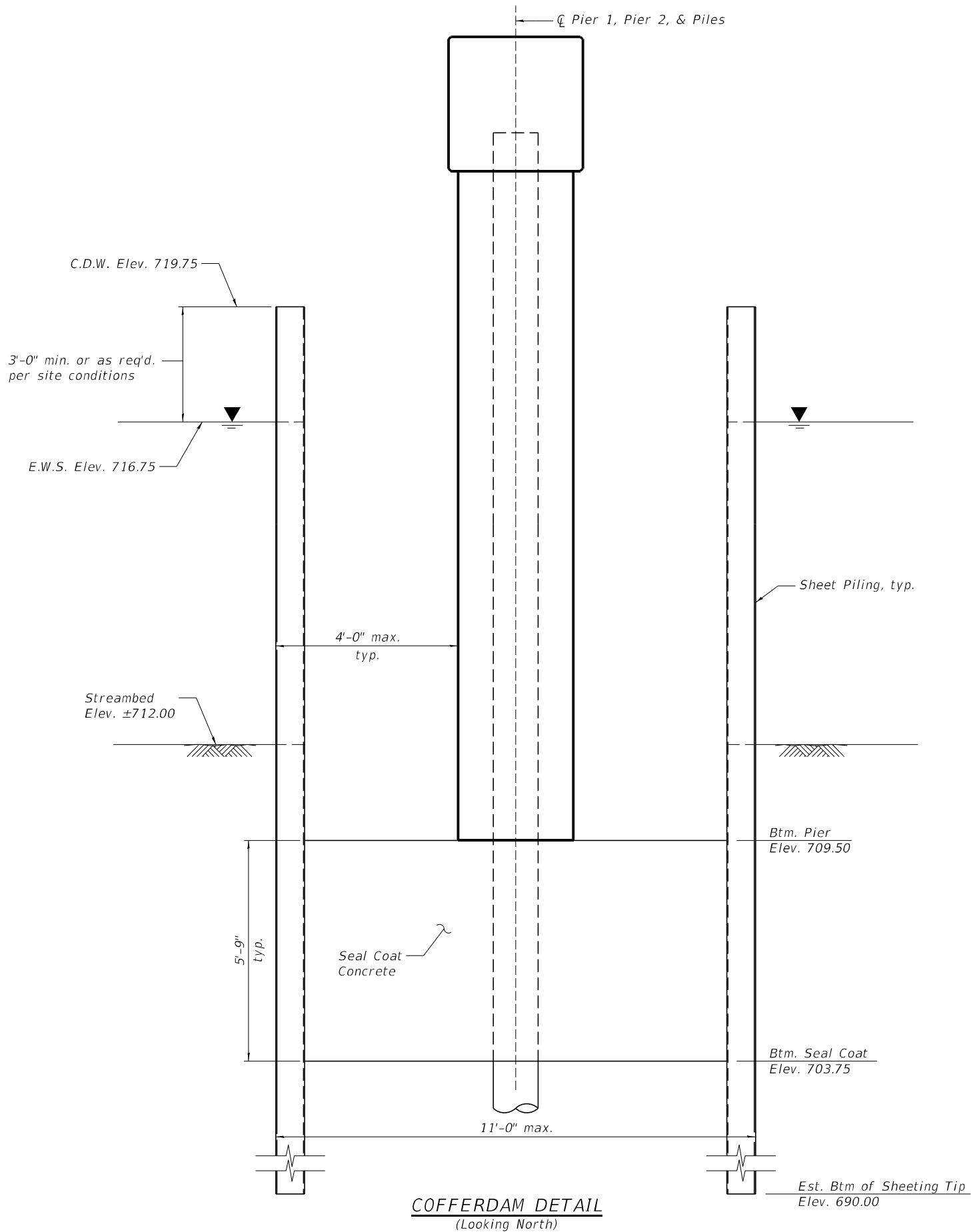
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DEPARTMENT OF TRANSPORTATION**

**TEMPORARY SHEET PILING DETAILS
STRUCTURE NO. 101-0225 & 101-0226**

SHEET 12 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	14
CONTRACT NO. 64U98				
		ILLINOIS	FED. AID PROJECT	

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NOTES:
See General Note 8 on Sheet 2 of 71 .

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COFFERDAM DETAILS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 13 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	15
CONTRACT NO. 64U98				
		ILLINOIS	FED. AID PROJECT	

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Job No. 10800

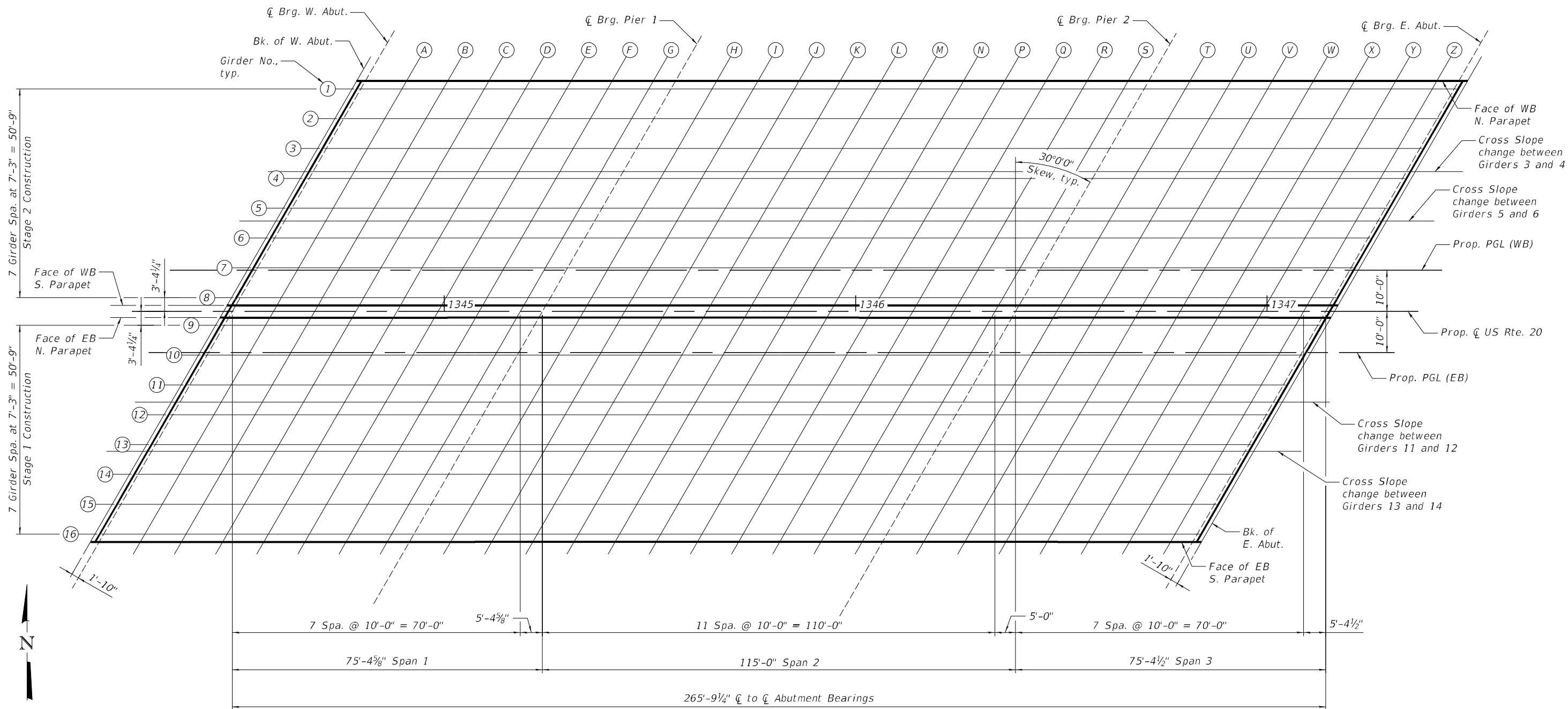
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PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS - PLAN
STRUCTURE NO. 101-0225 & 101-0226

SHEET 14 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	16
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



PLAN

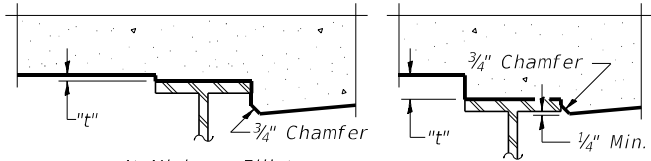
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FACE OF WB. N. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+78.72	-56.00	738.93	738.95
CL. BRG. W. ABUT.	1344+80.84	-56.00	738.93	738.95
A	1344+90.84	-56.00	738.92	738.95
B	1345+00.84	-56.00	738.90	738.95
C	1345+10.84	-56.00	738.89	738.94
D	1345+20.84	-56.00	738.88	738.92
E	1345+30.84	-56.00	738.86	738.89
F	1345+40.84	-56.00	738.85	738.87
G	1345+50.84	-56.00	738.83	738.85
CL. BRG. PIER 1	1345+56.22	-56.00	738.83	738.85
H	1345+66.22	-56.00	738.81	738.86
I	1345+76.22	-56.00	738.80	738.87
J	1345+86.22	-56.00	738.78	738.89
K	1345+96.22	-56.00	738.77	738.91
L	1346+06.22	-56.00	738.76	738.91
M	1346+16.22	-56.00	738.74	738.90
N	1346+26.22	-56.00	738.73	738.88
P	1346+36.22	-56.00	738.72	738.84
Q	1346+46.22	-56.00	738.70	738.79
R	1346+56.22	-56.00	738.69	738.75
S	1346+66.22	-56.00	738.67	738.71
CL. BRG. PIER 2	1346+71.22	-56.00	738.67	738.69
T	1346+81.22	-56.00	738.65	738.67
U	1346+91.22	-56.00	738.64	738.66
V	1347+01.22	-56.00	738.63	738.66
W	1347+11.22	-56.00	738.61	738.65
X	1347+21.22	-56.00	738.60	738.64
Y	1347+31.22	-56.00	738.58	738.63
Z	1347+41.22	-56.00	738.57	738.60
CL. BRG. E. ABUT.	1347+46.61	-56.00	738.56	738.58
BK. E. ABUT.	1347+48.72	-56.00	738.56	738.58

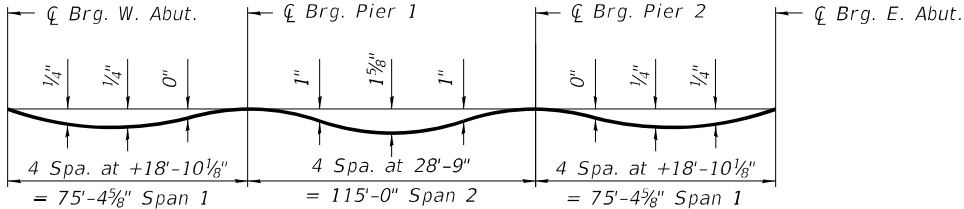
GIRDER 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+77.63	-54.10	738.98	739.00
CL. BRG. W. ABUT.	1344+79.74	-54.10	738.98	739.00
A	1344+89.74	-54.10	738.97	739.00
B	1344+99.74	-54.10	738.95	739.00
C	1345+09.74	-54.10	738.94	738.99
D	1345+19.74	-54.10	738.92	738.96
E	1345+29.74	-54.10	738.91	738.94
F	1345+39.74	-54.10	738.90	738.92
G	1345+49.74	-54.10	738.88	738.90
CL. BRG. PIER 1	1345+55.13	-54.10	738.88	738.90
H	1345+65.13	-54.10	738.86	738.90
I	1345+75.13	-54.10	738.85	738.92
J	1345+85.13	-54.10	738.83	738.94
K	1345+95.13	-54.10	738.82	738.96
L	1346+05.13	-54.10	738.81	738.96
M	1346+15.13	-54.10	738.79	738.95
N	1346+25.13	-54.10	738.78	738.93
P	1346+35.13	-54.10	738.76	738.89
Q	1346+45.13	-54.10	738.75	738.84
R	1346+55.13	-54.10	738.74	738.80
S	1346+65.13	-54.10	738.72	738.76
CL. BRG. PIER 2	1346+70.13	-54.10	738.72	738.74
T	1346+80.13	-54.10	738.70	738.72
U	1346+90.13	-54.10	738.69	738.71
V	1347+00.13	-54.10	738.68	738.71
W	1347+10.13	-54.10	738.66	738.70
X	1347+20.13	-54.10	738.65	738.69
Y	1347+30.13	-54.10	738.63	738.67
Z	1347+40.13	-54.10	738.62	738.65
CL. BRG. E. ABUT.	1347+45.51	-54.10	738.61	738.63
BK. E. ABUT.	1347+47.63	-54.10	738.61	738.63



To determine "t": After all structural steel has been erected, elevations of the top flanges of the girders shall be taken at intervals shown on Sheet 14 of 71 . These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection and Grinding" shown below and on Sheets 16 thru 23 of 71, minus the initial slab thickness prior to grinding, equals the fillet heights "t" above top flange of girders. The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown below and on Sheets 16 thru 23 of 71. For grinding the deck, see Special Provisions.

FILLET HEIGHTS



DEAD LOAD DEFLECTION DIAGRAM - GIRDERS 1 THRU 16
(Includes weight of concrete only.)

Note:
The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown above and on Sheets 16 thru 23 of 71.

FOR INFORMATION ONLY



USER NAME	=	DESIGNED	-	JPM	REVISED	-
		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS (1 OF 9)
STRUCTURE NO. 101-0225 & 101-0226

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	17
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		

SHEET 15 OF 71 SHEETS

MODEL: Default
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FOR INFORMATION ONLY

GIRDER 2				
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+73.44	-46.85	739.17	739.19
CL. BRG. W. ABUT.	1344+75.56	-46.85	739.17	739.19
A	1344+85.56	-46.85	739.15	739.19
B	1344+95.56	-46.85	739.14	739.18
C	1345+05.56	-46.85	739.12	739.17
D	1345+15.56	-46.85	739.11	739.15
E	1345+25.56	-46.85	739.10	739.13
F	1345+35.56	-46.85	739.08	739.10
G	1345+45.56	-46.85	739.07	739.09
CL. BRG. PIER 1	1345+50.94	-46.85	739.06	739.08
H	1345+60.94	-46.85	739.05	739.09
I	1345+70.94	-46.85	739.03	739.11
J	1345+80.94	-46.85	739.02	739.13
K	1345+90.94	-46.85	739.01	739.14
L	1346+00.94	-46.85	738.99	739.14
M	1346+10.94	-46.85	738.98	739.13
N	1346+20.94	-46.85	738.97	739.11
P	1346+30.94	-46.85	738.95	739.07
Q	1346+40.94	-46.85	738.94	739.03
R	1346+50.94	-46.85	738.92	738.98
S	1346+60.94	-46.85	738.91	738.94
CL. BRG. PIER 2	1346+65.94	-46.85	738.90	738.92
T	1346+75.94	-46.85	738.89	738.91
U	1346+85.94	-46.85	738.88	738.90
V	1346+95.94	-46.85	738.86	738.89
W	1347+05.94	-46.85	738.85	738.89
X	1347+15.94	-46.85	738.83	738.88
Y	1347+25.94	-46.85	738.82	738.86
Z	1347+35.94	-46.85	738.81	738.84
CL. BRG. E. ABUT.	1347+41.32	-46.85	738.80	738.82
BK. E. ABUT.	1347+43.44	-46.85	738.80	738.82

GIRDER 3				
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+69.26	-39.60	739.36	739.38
CL. BRG. W. ABUT.	1344+71.37	-39.60	739.35	739.37
A	1344+81.37	-39.60	739.34	739.38
B	1344+91.37	-39.60	739.33	739.37
C	1345+01.37	-39.60	739.31	739.36
D	1345+11.37	-39.60	739.30	739.34
E	1345+21.37	-39.60	739.28	739.31
F	1345+31.37	-39.60	739.27	739.29
G	1345+41.37	-39.60	739.26	739.27
CL. BRG. PIER 1	1345+46.76	-39.60	739.25	739.27
H	1345+56.76	-39.60	739.24	739.28
I	1345+66.76	-39.60	739.22	739.29
J	1345+76.76	-39.60	739.21	739.31
K	1345+86.76	-39.60	739.19	739.33
L	1345+96.76	-39.60	739.18	739.33
M	1346+06.76	-39.60	739.17	739.32
N	1346+16.76	-39.60	739.15	739.30
P	1346+26.76	-39.60	739.14	739.26
Q	1346+36.76	-39.60	739.13	739.21
R	1346+46.76	-39.60	739.11	739.17
S	1346+56.76	-39.60	739.10	739.13
CL. BRG. PIER 2	1346+61.76	-39.60	739.09	739.11
T	1346+71.76	-39.60	739.08	739.09
U	1346+81.76	-39.60	739.06	739.08
V	1346+91.76	-39.60	739.05	739.08
W	1347+01.76	-39.60	739.04	739.08
X	1347+11.76	-39.60	739.02	739.07
Y	1347+21.76	-39.60	739.01	739.05
Z	1347+31.76	-39.60	738.99	739.02
CL. BRG. E. ABUT.	1347+37.14	-39.60	738.99	739.01
BK. E. ABUT.	1347+39.26	-39.60	738.98	739.00

CROSS SLOPE CHANGE BETWEEN GIRDERS 3 AND 4				
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+66.02	-34.00	739.50	739.52
CL. BRG. W. ABUT.	1344+68.14	-34.00	739.50	739.52
A	1344+78.14	-34.00	739.48	739.52
B	1344+88.14	-34.00	739.47	739.52
C	1344+98.14	-34.00	739.46	739.50
D	1345+08.14	-34.00	739.44	739.48
E	1345+18.14	-34.00	739.43	739.46
F	1345+28.14	-34.00	739.41	739.43
G	1345+38.14	-34.00	739.40	739.42
CL. BRG. PIER 1	1345+43.52	-34.00	739.39	739.41
H	1345+53.52	-34.00	739.38	739.42
I	1345+63.52	-34.00	739.37	739.44
J	1345+73.52	-34.00	739.35	739.46
K	1345+83.52	-34.00	739.34	739.47
L	1345+93.52	-34.00	739.32	739.47
M	1346+03.52	-34.00	739.31	739.47
N	1346+13.52	-34.00	739.30	739.44
P	1346+23.52	-34.00	739.28	739.40
Q	1346+33.52	-34.00	739.27	739.36
R	1346+43.52	-34.00	739.26	739.31
S	1346+53.52	-34.00	739.24	739.27
CL. BRG. PIER 2	1346+58.52	-34.00	739.24	739.26
T	1346+68.52	-34.00	739.22	739.24
U	1346+78.52	-34.00	739.21	739.23
V	1346+88.52	-34.00	739.19	739.22
W	1346+98.52	-34.00	739.18	739.22
X	1347+08.52	-34.00	739.17	739.21
Y	1347+18.52	-34.00	739.15	739.19
Z	1347+28.52	-34.00	739.14	739.17
CL. BRG. E. ABUT.	1347+33.90	-34.00	739.13	739.15
BK. E. ABUT.	1347+36.02	-34.00	739.13	739.15

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Alfred Benesch & Company
35 W Wacker Drive, Suite 3300
Chicago, Illinois 60601
312.465.4150 Job No. 10800

USER NAME	=	DESIGNED	-	JPM	REVISED	-
		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS (3 OF 9)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 17 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	19
		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

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Alfred Benesch & Company

35 W Wacker Drive, Suite 3300

Chicago, Illinois 60601

312.465.4150 Job No. 10800

USER NAME	=	DESIGNED	-	JPM	REVISED	-
		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS (4 OF 9)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 18 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	20
		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

GIRDER 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+56.70	-17.85	739.82	739.84
CL. BRG. W. ABUT.	1344+58.82	-17.85	739.81	739.83
A	1344+68.82	-17.85	739.80	739.83
B	1344+78.82	-17.85	739.79	739.83
C	1344+88.82	-17.85	739.77	739.82
D	1344+98.82	-17.85	739.76	739.80
E	1345+08.82	-17.85	739.74	739.77
F	1345+18.82	-17.85	739.73	739.75
G	1345+28.82	-17.85	739.72	739.73
CL. BRG. PIER 1	1345+34.20	-17.85	739.71	739.73
H	1345+44.20	-17.85	739.70	739.74
I	1345+54.20	-17.85	739.68	739.75
J	1345+64.20	-17.85	739.67	739.77
K	1345+74.20	-17.85	739.65	739.79
L	1345+84.20	-17.85	739.64	739.79
M	1345+94.20	-17.85	739.63	739.78
N	1346+04.20	-17.85	739.61	739.76
P	1346+14.20	-17.85	739.60	739.72
Q	1346+24.20	-17.85	739.58	739.67
R	1346+34.20	-17.85	739.57	739.63
S	1346+44.20	-17.85	739.56	739.59
CL. BRG. PIER 2	1346+49.20	-17.85	739.55	739.57
T	1346+59.20	-17.85	739.54	739.55
U	1346+69.20	-17.85	739.52	739.54
V	1346+79.20	-17.85	739.51	739.54
W	1346+89.20	-17.85	739.50	739.53
X	1346+99.20	-17.85	739.48	739.52
Y	1347+09.20	-17.85	739.47	739.51
Z	1347+19.20	-17.85	739.45	739.48
CL. BRG. E. ABUT.	1347+24.58	-17.85	739.45	739.47
BK. E. ABUT.	1347+26.70	-17.85	739.44	739.46

GIRDER 7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+52.51	-10.60	739.93	739.95
CL. BRG. W. ABUT.	1344+54.63	-10.60	739.93	739.95
A	1344+64.63	-10.60	739.91	739.95
B	1344+74.63	-10.60	739.90	739.95
C	1344+84.63	-10.60	739.89	739.93
D	1344+94.63	-10.60	739.87	739.91
E	1345+04.63	-10.60	739.86	739.89
F	1345+14.63	-10.60	739.84	739.86
G	1345+24.63	-10.60	739.83	739.85
CL. BRG. PIER 1	1345+30.01	-10.60	739.82	739.84
H	1345+40.01	-10.60	739.81	739.85
I	1345+50.01	-10.60	739.80	739.87
J	1345+60.01	-10.60	739.78	739.89
K	1345+70.01	-10.60	739.77	739.90
L	1345+80.01	-10.60	739.75	739.90
M	1345+90.01	-10.60	739.74	739.90
N	1346+00.01	-10.60	739.73	739.87
P	1346+10.01	-10.60	739.71	739.83
Q	1346+20.01	-10.60	739.70	739.79
R	1346+30.01	-10.60	739.69	739.74
S	1346+40.01	-10.60	739.67	739.70
CL. BRG. PIER 2	1346+45.01	-10.60	739.66	739.69
T	1346+55.01	-10.60	739.65	739.67
U	1346+65.01	-10.60	739.64	739.66
V	1346+75.01	-10.60	739.62	739.65
W	1346+85.01	-10.60	739.61	739.65
X	1346+95.01	-10.60	739.60	739.64
Y	1347+05.01	-10.60	739.58	739.62
Z	1347+15.01	-10.60	739.57	739.60
CL. BRG. E. ABUT.	1347+20.40	-10.60	739.56	739.58
BK. E. ABUT.	1347+22.51	-10.60	739.56	739.58

PROPOSED PGL (WB)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+52.16	-10.00	739.94	739.96
CL. BRG. W. ABUT.	1344+54.28	-10.00	739.94	739.96
A	1344+64.28	-10.00	739.92	739.96
B	1344+74.28	-10.00	739.91	739.95
C	1344+84.28	-10.00	739.90	739.94
D	1344+94.28	-10.00	739.88	739.92
E	1345+04.28	-10.00	739.87	739.90
F	1345+14.28	-10.00	739.85	739.87
G	1345+24.28	-10.00	739.84	739.86
CL. BRG. PIER 1	1345+29.66	-10.00	739.83	739.85
H	1345+39.66	-10.00	739.82	739.86
I	1345+49.66	-10.00	739.81	739.88
J	1345+59.66	-10.00	739.79	739.90
K	1345+69.66	-10.00	739.78	739.91
L	1345+79.66	-10.00	739.76	739.91
M	1345+89.66	-10.00	739.75	739.90
N	1345+99.66	-10.00	739.74	739.88
P	1346+09.66	-10.00	739.72	739.84
Q	1346+19.66	-10.00	739.71	739.80
R	1346+29.66	-10.00	739.69	739.75
S	1346+39.66	-10.00	739.68	739.71
CL. BRG. PIER 2	1346+44.66	-10.00	739.67	739.70
T	1346+54.66	-10.00	739.66	739.68
U	1346+64.66	-10.00	739.65	739.67
V	1346+74.66	-10.00	739.63	739.66
W	1346+84.66	-10.00	739.62	739.66
X	1346+94.66	-10.00	739.61	739.65
Y	1347+04.66	-10.00	739.59	739.63
Z	1347+14.66	-10.00	739.58	739.61
CL. BRG. E. ABUT.	1347+20.05	-10.00	739.57	739.59
BK. E. ABUT.	1347+22.16	-10.00	739.57	739.59

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GIRDER 8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+48.33	-3.35	739.81	739.83
CL. BRG. W. ABUT.	1344+50.44	-3.35	739.81	739.83
A	1344+60.44	-3.35	739.80	739.83
B	1344+70.44	-3.35	739.78	739.83
C	1344+80.44	-3.35	739.77	739.82
D	1344+90.44	-3.35	739.75	739.79
E	1345+00.44	-3.35	739.74	739.77
F	1345+10.44	-3.35	739.73	739.75
G	1345+20.44	-3.35	739.71	739.73
CL. BRG. PIER 1	1345+25.83	-3.35	739.71	739.73
H	1345+35.83	-3.35	739.69	739.73
I	1345+45.83	-3.35	739.68	739.75
J	1345+55.83	-3.35	739.66	739.77
K	1345+65.83	-3.35	739.65	739.79
L	1345+75.83	-3.35	739.64	739.79
M	1345+85.83	-3.35	739.62	739.78
N	1345+95.83	-3.35	739.61	739.76
P	1346+05.83	-3.35	739.59	739.72
Q	1346+15.83	-3.35	739.58	739.67
R	1346+25.83	-3.35	739.57	739.63
S	1346+35.83	-3.35	739.55	739.59
CL. BRG. PIER 2	1346+40.83	-3.35	739.55	739.57
T	1346+50.83	-3.35	739.53	739.55
U	1346+60.83	-3.35	739.52	739.54
V	1346+70.83	-3.35	739.51	739.54
W	1346+80.83	-3.35	739.49	739.53
X	1346+90.83	-3.35	739.48	739.52
Y	1347+00.83	-3.35	739.46	739.50
Z	1347+10.83	-3.35	739.45	739.48
CL. BRG. E. ABUT.	1347+16.21	-3.35	739.44	739.46
BK. E. ABUT.	1347+18.33	-3.35	739.44	739.46

WB S. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+47.26	-1.50	739.78	739.80
CL. BRG. W. ABUT.	1344+49.37	-1.50	739.77	739.79
A	1344+59.37	-1.50	739.76	739.80
B	1344+69.37	-1.50	739.75	739.79
C	1344+79.37	-1.50	739.73	739.78
D	1344+89.37	-1.50	739.72	739.76
E	1344+99.37	-1.50	739.70	739.73
F	1345+09.37	-1.50	739.69	739.71
G	1345+19.37	-1.50	739.68	739.69
CL. BRG. PIER 1	1345+24.76	-1.50	739.67	739.69
H	1345+34.76	-1.50	739.66	739.70
I	1345+44.76	-1.50	739.64	739.72
J	1345+54.76	-1.50	739.63	739.74
K	1345+64.76	-1.50	739.61	739.75
L	1345+74.76	-1.50	739.60	739.76
M	1345+84.76	-1.50	739.59	739.75
N	1345+94.76	-1.50	739.57	739.72
P	1346+04.76	-1.50	739.56	739.69
Q	1346+14.76	-1.50	739.55	739.64
R	1346+24.76	-1.50	739.53	739.59
S	1346+34.76	-1.50	739.52	739.55
CL. BRG. PIER 2	1346+39.76	-1.50	739.51	739.53
T	1346+49.76	-1.50	739.50	739.51
U	1346+59.76	-1.50	739.48	739.50
V	1346+69.76	-1.50	739.47	739.50
W	1346+79.76	-1.50	739.46	739.50
X	1346+89.76	-1.50	739.44	739.49
Y	1346+99.76	-1.50	739.43	739.47
Z	1347+09.76	-1.50	739.41	739.44
CL. BRG. E. ABUT.	1347+15.14	-1.50	739.41	739.43
BK. E. ABUT.	1347+17.26	-1.50	739.40	739.42

EB N. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+45.52	1.50	739.78	739.80
CL. BRG. W. ABUT.	1344+47.64	1.50	739.78	739.80
A	1344+57.64	1.50	739.76	739.80
B	1344+67.64	1.50	739.75	739.79
C	1344+77.64	1.50	739.73	739.78
D	1344+87.64	1.50	739.72	739.76
E	1344+97.64	1.50	739.71	739.74
F	1345+07.64	1.50	739.69	739.71
G	1345+17.64	1.50	739.68	739.70
CL. BRG. PIER 1	1345+23.02	1.50	739.67	739.69
H	1345+33.02	1.50	739.66	739.70
I	1345+43.02	1.50	739.64	739.72
J	1345+53.02	1.50	739.63	739.74
K	1345+63.02	1.50	739.62	739.75
L	1345+73.02	1.50	739.60	739.76
M	1345+83.02	1.50	739.59	739.75
N	1345+93.02	1.50	739.58	739.73
P	1346+03.02	1.50	739.56	739.69
Q	1346+13.02	1.50	739.55	739.64
R	1346+23.02	1.50	739.53	739.59
S	1346+33.02	1.50	739.52	739.55
CL. BRG. PIER 2	1346+38.02	1.50	739.51	739.53
T	1346+48.02	1.50	739.50	739.52
U	1346+58.02	1.50	739.49	739.51
V	1346+68.02	1.50	739.47	739.50
W	1346+78.02	1.50	739.46	739.50
X	1346+88.02	1.50	739.44	739.49
Y	1346+98.02	1.50	739.43	739.47
Z	1347+08.02	1.50	739.42	739.45
CL. BRG. E. ABUT.	1347+13.41	1.50	739.41	739.43
BK. E. ABUT.	1347+15.52	1.50	739.41	739.43

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Alfred Benesch & Company

35 W Wacker Drive, Suite 3300

Chicago, Illinois 60601

312.465.4150 Job No. 10800

USER NAME	=	DESIGNED	-	JPM	REVISED	-
		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS (6 OF 9)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 20 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	22
		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

GIRDER 9

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+44.45	3.35	739.82	739.84
CL. BRG. W. ABUT.	1344+46.57	3.35	739.81	739.84
A	1344+56.57	3.35	739.80	739.84
B	1344+66.57	3.35	739.79	739.83
C	1344+76.57	3.35	739.77	739.82
D	1344+86.57	3.35	739.76	739.80
E	1344+96.57	3.35	739.75	739.77
F	1345+06.57	3.35	739.73	739.75
G	1345+16.57	3.35	739.72	739.74
CL. BRG. PIER 1	1345+21.95	3.35	739.71	739.73
H	1345+31.95	3.35	739.70	739.74
I	1345+41.95	3.35	739.68	739.76
J	1345+51.95	3.35	739.67	739.78
K	1345+61.95	3.35	739.66	739.79
L	1345+71.95	3.35	739.64	739.80
M	1345+81.95	3.35	739.63	739.79
N	1345+91.95	3.35	739.61	739.76
P	1346+01.95	3.35	739.60	739.73
Q	1346+11.95	3.35	739.59	739.68
R	1346+21.95	3.35	739.57	739.63
S	1346+31.95	3.35	739.56	739.59
CL. BRG. PIER 2	1346+36.95	3.35	739.55	739.57
T	1346+46.95	3.35	739.54	739.55
U	1346+56.95	3.35	739.52	739.55
V	1346+66.95	3.35	739.51	739.54
W	1346+76.95	3.35	739.50	739.54
X	1346+86.95	3.35	739.48	739.53
Y	1346+96.95	3.35	739.47	739.51
Z	1347+06.95	3.35	739.46	739.48
CL. BRG. E. ABUT.	1347+12.34	3.35	739.45	739.47
BK. E. ABUT.	1347+14.45	3.35	739.45	739.47

PROPOSED PGL (EB)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+40.62	10.00	739.96	739.98
CL. BRG. W. ABUT.	1344+42.73	10.00	739.95	739.97
A	1344+52.73	10.00	739.94	739.97
B	1344+62.73	10.00	739.93	739.97
C	1344+72.73	10.00	739.91	739.96
D	1344+82.73	10.00	739.90	739.94
E	1344+92.73	10.00	739.88	739.91
F	1345+02.73	10.00	739.87	739.89
G	1345+12.73	10.00	739.86	739.87
CL. BRG. PIER 1	1345+18.12	10.00	739.85	739.87
H	1345+28.12	10.00	739.84	739.88
I	1345+38.12	10.00	739.82	739.89
J	1345+48.12	10.00	739.81	739.91
K	1345+58.12	10.00	739.79	739.93
L	1345+68.12	10.00	739.78	739.93
M	1345+78.12	10.00	739.77	739.92
N	1345+88.12	10.00	739.75	739.90
P	1345+98.12	10.00	739.74	739.86
Q	1346+08.12	10.00	739.72	739.81
R	1346+18.12	10.00	739.71	739.77
S	1346+28.12	10.00	739.70	739.73
CL. BRG. PIER 2	1346+33.12	10.00	739.69	739.71
T	1346+43.12	10.00	739.68	739.69
U	1346+53.12	10.00	739.66	739.68
V	1346+63.12	10.00	739.65	739.68
W	1346+73.12	10.00	739.64	739.67
X	1346+83.12	10.00	739.62	739.66
Y	1346+93.12	10.00	739.61	739.65
Z	1347+03.12	10.00	739.59	739.62
CL. BRG. E. ABUT.	1347+08.50	10.00	739.59	739.61
BK. E. ABUT.	1347+10.62	10.00	739.58	739.60

GIRDER 10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+40.27	10.60	739.95	739.97
CL. BRG. W. ABUT.	1344+42.39	10.60	739.94	739.97
A	1344+52.39	10.60	739.93	739.97
B	1344+62.39	10.60	739.92	739.96
C	1344+72.39	10.60	739.90	739.95
D	1344+82.39	10.60	739.89	739.93
E	1344+92.39	10.60	739.88	739.90
F	1345+02.39	10.60	739.86	739.88
G	1345+12.39	10.60	739.85	739.87
CL. BRG. PIER 1	1345+17.77	10.60	739.84	739.86
H	1345+27.77	10.60	739.83	739.87
I	1345+37.77	10.60	739.81	739.88
J	1345+47.77	10.60	739.80	739.90
K	1345+57.77	10.60	739.79	739.92
L	1345+67.77	10.60	739.77	739.92
M	1345+77.77	10.60	739.76	739.91
N	1345+87.77	10.60	739.74	739.89
P	1345+97.77	10.60	739.73	739.85
Q	1346+07.77	10.60	739.72	739.81
R	1346+17.77	10.60	739.70	739.76
S	1346+27.77	10.60	739.69	739.72
CL. BRG. PIER 2	1346+32.77	10.60	739.68	739.70
T	1346+42.77	10.60	739.67	739.68
U	1346+52.77	10.60	739.65	739.67
V	1346+62.77	10.60	739.64	739.67
W	1346+72.77	10.60	739.63	739.67
X	1346+82.77	10.60	739.61	739.66
Y	1346+92.77	10.60	739.60	739.64
Z	1347+02.77	10.60	739.59	739.61
CL. BRG. E. ABUT.	1347+08.15	10.60	739.58	739.60
BK. E. ABUT.	1347+10.27	10.60	739.57	739.60

GIRDER 11

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+36.08	17.85	739.84	739.86
CL. BRG. W. ABUT.	1344+38.20	17.85	739.84	739.86
A	1344+48.20	17.85	739.83	739.86
B	1344+58.20	17.85	739.81	739.86
C	1344+68.20	17.85	739.80	739.85
D	1344+78.20	17.85	739.79	739.83
E	1344+88.20	17.85	739.77	739.80
F	1344+98.20	17.85	739.76	739.78
G	1345+08.20	17.85	739.74	739.76
CL. BRG. PIER 1	1345+13.58	17.85	739.74	739.76
H	1345+23.58	17.85	739.72	739.77
I	1345+33.58	17.85	739.71	739.78
J	1345+43.58	17.85	739.70	739.80
K	1345+53.58	17.85	739.68	739.81
L	1345+63.58	17.85	739.67	739.82
M	1345+73.58	17.85	739.65	739.81
N	1345+83.58	17.85	739.64	739.79
P	1345+93.58	17.85	739.63	739.75
Q	1346+03.58	17.85	739.61	739.70
R	1346+13.58	17.85	739.60	739.66
S	1346+23.58	17.85	739.59	739.62
CL. BRG. PIER 2	1346+28.58	17.85	739.58	739.60
T	1346+38.58	17.85	739.56	739.58
U	1346+48.58	17.85	739.55	739.57
V	1346+58.58	17.85	739.54	739.57
W	1346+68.58	17.85	739.52	739.56
X	1346+78.58	17.85	739.51	739.55
Y	1346+88.58	17.85	739.50	739.54
Z	1346+98.58	17.85	739.48	739.51
CL. BRG. E. ABUT.	1347+03.97	17.85	739.47	739.50
BK. E. ABUT.	1347+06.08	17.85	739.47	739.49

CROSS SLOPE CHANGE BETWEEN GIRDERS 11 AND 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+33.69	22.00	739.79	739.81
CL. BRG. W. ABUT.	1344+35.81	22.00	739.78	739.80
A	1344+45.81	22.00	739.77	739.80
B	1344+55.81	22.00	739.75	739.80
C	1344+65.81	22.00	739.74	739.79
D	1344+75.81	22.00	739.73	739.77
E	1344+85.81	22.00	739.71	739.74
F	1344+95.81	22.00	739.70	739.72
G	1345+05.81	22.00	739.69	739.70
CL. BRG. PIER 1	1345+11.19	22.00	739.68	739.70
H	1345+21.19	22.00	739.66	739.71
I	1345+31.19	22.00	739.65	739.72
J	1345+41.19	22.00	739.64	739.74
K	1345+51.19	22.00	739.62	739.76
L	1345+61.19	22.00	739.61	739.76
M	1345+71.19	22.00	739.60	739.75
N	1345+81.19	22.00	739.58	739.73
P	1345+91.19	22.00	739.57	739.69
Q	1346+01.19	22.00	739.55	739.64
R	1346+11.19	22.00	739.54	739.60
S	1346+21.19	22.00	739.53	739.56
CL. BRG. PIER 2	1346+26.19	22.00	739.52	739.54
T	1346+36.19	22.00	739.51	739.52
U	1346+46.19	22.00	739.49	739.51
V	1346+56.19	22.00	739.48	739.51
W	1346+66.19	22.00	739.46	739.50
X	1346+76.19	22.00	739.45	739.49
Y	1346+86.19	22.00	739.44	739.48
Z	1346+96.19	22.00	739.42	739.45
CL. BRG. E. ABUT.	1347+01.57	22.00	739.42	739.44
BK. E. ABUT.	1347+03.69	22.00	739.41	739.43

GIRDER 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+31.90	25.10	739.73	739.75
CL. BRG. W. ABUT.	1344+34.01	25.10	739.72	739.74
A	1344+44.01	25.10	739.71	739.74
B	1344+54.01	25.10	739.70	739.74
C	1344+64.01	25.10	739.68	739.73
D	1344+74.01	25.10	739.67	739.71
E	1344+84.01	25.10	739.65	739.68
F	1344+94.01	25.10	739.64	739.66
G	1345+04.01	25.10	739.63	739.64
CL. BRG. PIER 1	1345+09.40	25.10	739.62	739.64
H	1345+19.40	25.10	739.60	739.65
I	1345+29.40	25.10	739.59	739.66
J	1345+39.40	25.10	739.58	739.68
K	1345+49.40	25.10	739.56	739.70
L	1345+59.40	25.10	739.55	739.70
M	1345+69.40	25.10	739.54	739.69
N	1345+79.40	25.10	739.52	739.67
P	1345+89.40	25.10	739.51	739.63
Q	1345+99.40	25.10	739.49	739.58
R	1346+09.40	25.10	739.48	739.54
S	1346+19.40	25.10	739.47	739.50
CL. BRG. PIER 2	1346+24.40	25.10	739.46	739.48
T	1346+34.40	25.10	739.45	739.46
U	1346+44.40	25.10	739.43	739.45
V	1346+54.40	25.10	739.42	739.45
W	1346+64.40	25.10	739.40	739.44
X	1346+74.40	25.10	739.39	739.43
Y	1346+84.40	25.10	739.38	739.42
Z	1346+94.40	25.10	739.36	739.39
CL. BRG. E. ABUT.	1346+99.78	25.10	739.36	739.38
BK. E. ABUT.	1347+01.90	25.10	739.35	739.37

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Alfred Benesch & Company
35 W Wacker Drive, Suite 3300
Chicago, Illinois 60601
312.465.4150 Job No. 10800

USER NAME	=	DESIGNED	-	JPM	REVISED	-
		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS (7 OF 9)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 21 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	23
		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

GIRDER 13

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+27.71	32.35	739.59	739.61
CL. BRG. W. ABUT.	1344+29.83	32.35	739.58	739.60
A	1344+39.83	32.35	739.57	739.61
B	1344+49.83	32.35	739.56	739.60
C	1344+59.83	32.35	739.54	739.59
D	1344+69.83	32.35	739.53	739.57
E	1344+79.83	32.35	739.51	739.54
F	1344+89.83	32.35	739.50	739.52
G	1344+99.83	32.35	739.49	739.50
CL. BRG. PIER 1	1345+05.21	32.35	739.48	739.50
H	1345+15.21	32.35	739.47	739.51
I	1345+25.21	32.35	739.45	739.52
J	1345+35.21	32.35	739.44	739.54
K	1345+45.21	32.35	739.42	739.56
L	1345+55.21	32.35	739.41	739.56
M	1345+65.21	32.35	739.40	739.55
N	1345+75.21	32.35	739.38	739.53
P	1345+85.21	32.35	739.37	739.49
Q	1345+95.21	32.35	739.36	739.44
R	1346+05.21	32.35	739.34	739.40
S	1346+15.21	32.35	739.33	739.36
CL. BRG. PIER 2	1346+20.21	32.35	739.32	739.34
T	1346+30.21	32.35	739.31	739.32
U	1346+40.21	32.35	739.29	739.31
V	1346+50.21	32.35	739.28	739.31
W	1346+60.21	32.35	739.27	739.31
X	1346+70.21	32.35	739.25	739.30
Y	1346+80.21	32.35	739.24	739.28
Z	1346+90.21	32.35	739.22	739.25
CL. BRG. E. ABUT.	1346+95.59	32.35	739.22	739.24
BK. E. ABUT.	1346+97.71	32.35	739.21	739.23

CROSS SLOPE CHANGE BETWEEN GIRDERS 13 AND 14

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+26.76	34.00	739.55	739.58
CL. BRG. W. ABUT.	1344+28.88	34.00	739.55	739.57
A	1344+38.88	34.00	739.54	739.57
B	1344+48.88	34.00	739.52	739.57
C	1344+58.88	34.00	739.51	739.56
D	1344+68.88	34.00	739.50	739.54
E	1344+78.88	34.00	739.48	739.51
F	1344+88.88	34.00	739.47	739.49
G	1344+98.88	34.00	739.46	739.47
CL. BRG. PIER 1	1345+04.26	34.00	739.45	739.47
H	1345+14.26	34.00	739.43	739.48
I	1345+24.26	34.00	739.42	739.49
J	1345+34.26	34.00	739.41	739.51
K	1345+44.26	34.00	739.39	739.53
L	1345+54.26	34.00	739.38	739.53
M	1345+64.26	34.00	739.37	739.52
N	1345+74.26	34.00	739.35	739.50
P	1345+84.26	34.00	739.34	739.46
Q	1345+94.26	34.00	739.32	739.41
R	1346+04.26	34.00	739.31	739.37
S	1346+14.26	34.00	739.30	739.33
CL. BRG. PIER 2	1346+19.26	34.00	739.29	739.31
T	1346+29.26	34.00	739.28	739.29
U	1346+39.26	34.00	739.26	739.28
V	1346+49.26	34.00	739.25	739.28
W	1346+59.26	34.00	739.23	739.27
X	1346+69.26	34.00	739.22	739.26
Y	1346+79.26	34.00	739.21	739.25
Z	1346+89.26	34.00	739.19	739.22
CL. BRG. E. ABUT.	1346+94.64	34.00	739.19	739.21
BK. E. ABUT.	1346+96.76	34.00	739.18	739.20

GIRDER 14

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+23.52	39.60	739.42	739.44
CL. BRG. W. ABUT.	1344+25.64	39.60	739.42	739.44
A	1344+35.64	39.60	739.40	739.44
B	1344+45.64	39.60	739.39	739.43
C	1344+55.64	39.60	739.37	739.42
D	1344+65.64	39.60	739.36	739.40
E	1344+75.64	39.60	739.35	739.38
F	1344+85.64	39.60	739.33	739.35
G	1344+95.64	39.60	739.32	739.34
CL. BRG. PIER 1	1345+01.02	39.60	739.31	739.33
H	1345+11.02	39.60	739.30	739.34
I	1345+21.02	39.60	739.28	739.36
J	1345+31.02	39.60	739.27	739.38
K	1345+41.02	39.60	739.26	739.39
L	1345+51.02	39.60	739.24	739.39
M	1345+61.02	39.60	739.23	739.38
N	1345+71.02	39.60	739.22	739.36
P	1345+81.02	39.60	739.20	739.32
Q	1345+91.02	39.60	739.19	739.28
R	1346+01.02	39.60	739.17	739.23
S	1346+11.02	39.60	739.16	739.19
CL. BRG. PIER 2	1346+16.02	39.60	739.15	739.17
T	1346+26.02	39.60	739.14	739.16
U	1346+36.02	39.60	739.13	739.15
V	1346+46.02	39.60	739.11	739.14
W	1346+56.02	39.60	739.10	739.14
X	1346+66.02	39.60	739.08	739.13
Y	1346+76.02	39.60	739.07	739.11
Z	1346+86.02	39.60	739.06	739.09
CL. BRG. E. ABUT.	1346+91.41	39.60	739.05	739.07
BK. E. ABUT.	1346+93.52	39.60	739.05	739.07

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Alfred Benesch & Company

35 W Wacker Drive, Suite 3300

Chicago, Illinois 60601

312.465.4150 Job No. 10800

USER NAME	=	DESIGNED	-	JPM	REVISED	-
		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS (8 OF 9)
STRUCTURE NO. 101-0225 & 101-0226

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	24
		CONTRACT NO. 64U98		
SHEET 22 OF 71 SHEETS		ILLINOIS	FED. AID PROJECT	

GIRDER 15

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+19.34	46.85	739.24	739.26
CL. BRG. W. ABUT.	1344+21.46	46.85	739.24	739.26
A	1344+31.46	46.85	739.23	739.26
B	1344+41.46	46.85	739.21	739.26
C	1344+51.46	46.85	739.20	739.25
D	1344+61.46	46.85	739.19	739.22
E	1344+71.46	46.85	739.17	739.20
F	1344+81.46	46.85	739.16	739.18
G	1344+91.46	46.85	739.14	739.16
CL. BRG. PIER 1	1344+96.84	46.85	739.14	739.16
H	1345+06.84	46.85	739.12	739.17
I	1345+16.84	46.85	739.11	739.18
J	1345+26.84	46.85	739.10	739.20
K	1345+36.84	46.85	739.08	739.21
L	1345+46.84	46.85	739.07	739.22
M	1345+56.84	46.85	739.05	739.21
N	1345+66.84	46.85	739.04	739.18
P	1345+76.84	46.85	739.03	739.15
Q	1345+86.84	46.85	739.01	739.10
R	1345+96.84	46.85	739.00	739.06
S	1346+06.84	46.85	738.99	739.02
CL. BRG. PIER 2	1346+11.84	46.85	738.98	739.00
T	1346+21.84	46.85	738.96	738.98
U	1346+31.84	46.85	738.95	738.97
V	1346+41.84	46.85	738.94	738.97
W	1346+51.84	46.85	738.92	738.96
X	1346+61.84	46.85	738.91	738.95
Y	1346+71.84	46.85	738.90	738.94
Z	1346+81.84	46.85	738.88	738.91
CL. BRG. E. ABUT.	1346+87.22	46.85	738.87	738.90
BK. E. ABUT.	1346+89.34	46.85	738.87	738.89

GIRDER 16

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+15.15	54.10	739.07	739.09
CL. BRG. W. ABUT.	1344+17.27	54.10	739.07	739.09
A	1344+27.27	54.10	739.05	739.09
B	1344+37.27	54.10	739.04	739.08
C	1344+47.27	54.10	739.02	739.07
D	1344+57.27	54.10	739.01	739.05
E	1344+67.27	54.10	739.00	739.02
F	1344+77.27	54.10	738.98	739.00
G	1344+87.27	54.10	738.97	738.99
CL. BRG. PIER 1	1344+92.65	54.10	738.96	738.98
H	1345+02.65	54.10	738.95	738.99
I	1345+12.65	54.10	738.93	739.01
J	1345+22.65	54.10	738.92	739.03
K	1345+32.65	54.10	738.91	739.04
L	1345+42.65	54.10	738.89	739.05
M	1345+52.65	54.10	738.88	739.04
N	1345+62.65	54.10	738.86	739.02
P	1345+72.65	54.10	738.85	738.98
Q	1345+82.65	54.10	738.84	738.93
R	1345+92.65	54.10	738.82	738.88
S	1346+02.65	54.10	738.81	738.84
CL. BRG. PIER 2	1346+07.65	54.10	738.80	738.82
T	1346+17.65	54.10	738.79	738.80
U	1346+27.65	54.10	738.78	738.80
V	1346+37.65	54.10	738.76	738.79
W	1346+47.65	54.10	738.75	738.79
X	1346+57.65	54.10	738.73	738.78
Y	1346+67.65	54.10	738.72	738.76
Z	1346+77.65	54.10	738.71	738.74
CL. BRG. E. ABUT.	1346+83.04	54.10	738.70	738.72
BK. E. ABUT.	1346+85.15	54.10	738.70	738.72

FACE OF EB S. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection & Grinding
BK. W. ABUT.	1344+14.06	56.00	739.02	739.04
CL. BRG. W. ABUT.	1344+16.18	56.00	739.02	739.04
A	1344+26.18	56.00	739.01	739.04
B	1344+36.18	56.00	738.99	739.04
C	1344+46.18	56.00	738.98	739.03
D	1344+56.18	56.00	738.96	739.00
E	1344+66.18	56.00	738.95	738.98
F	1344+76.18	56.00	738.94	738.96
G	1344+86.18	56.00	738.92	738.94
CL. BRG. PIER 1	1344+91.56	56.00	738.92	738.94
H	1345+01.56	56.00	738.90	738.94
I	1345+11.56	56.00	738.89	738.96
J	1345+21.56	56.00	738.87	738.98
K	1345+31.56	56.00	738.86	739.00
L	1345+41.56	56.00	738.85	739.00
M	1345+51.56	56.00	738.83	738.99
N	1345+61.56	56.00	738.82	738.97
P	1345+71.56	56.00	738.81	738.93
Q	1345+81.56	56.00	738.79	738.88
R	1345+91.56	56.00	738.78	738.84
S	1346+01.56	56.00	738.76	738.80
CL. BRG. PIER 2	1346+06.56	56.00	738.76	738.78
T	1346+16.56	56.00	738.74	738.76
U	1346+26.56	56.00	738.73	738.75
V	1346+36.56	56.00	738.72	738.75
W	1346+46.56	56.00	738.70	738.74
X	1346+56.56	56.00	738.69	738.73
Y	1346+66.56	56.00	738.67	738.72
Z	1346+76.56	56.00	738.66	738.69
CL. BRG. E. ABUT.	1346+81.94	56.00	738.65	738.67
BK. E. ABUT.	1346+84.06	56.00	738.65	738.67

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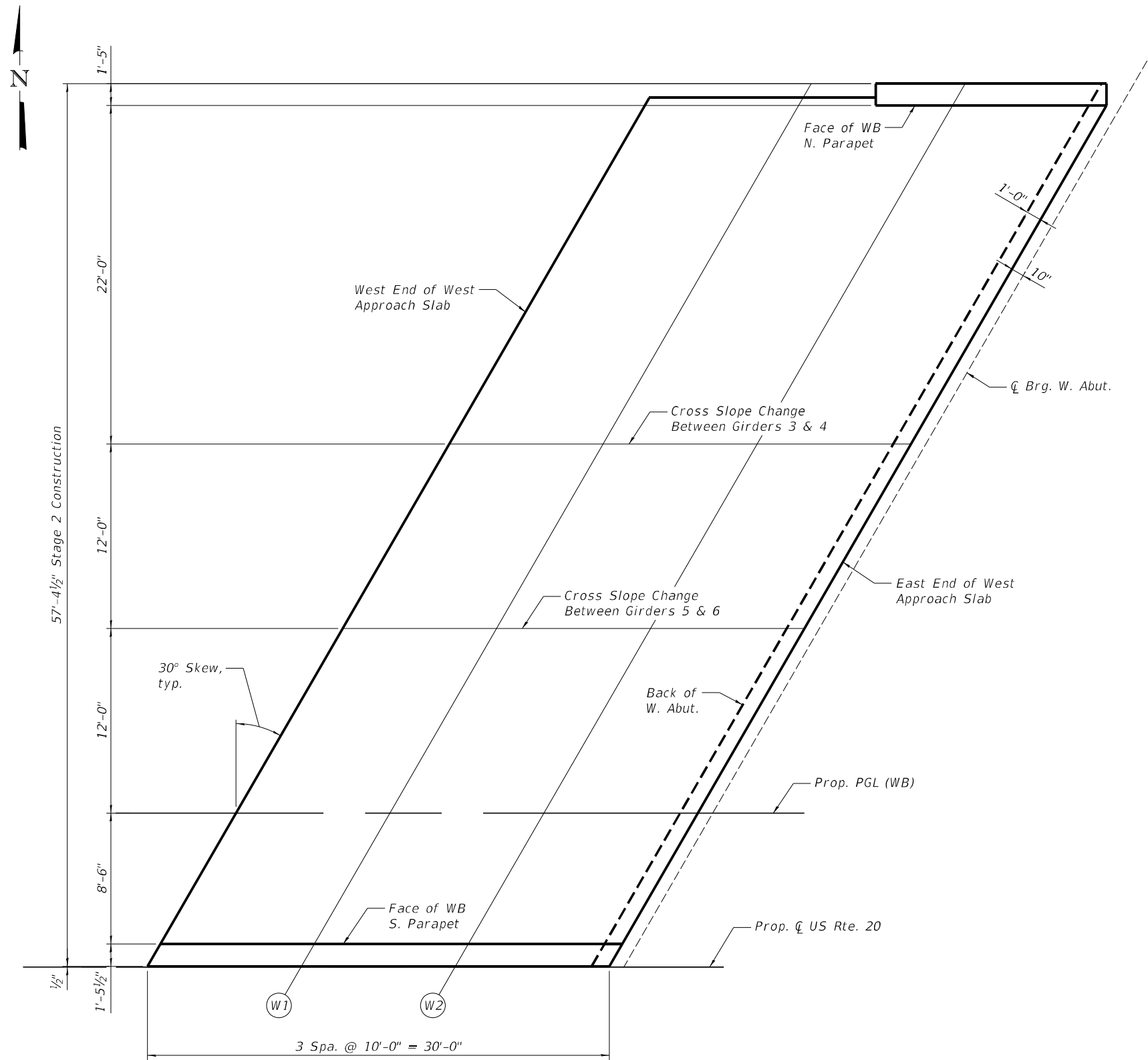
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS (9 OF 9)
STRUCTURE NO. 101-0225 & 101-0226

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	25
		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

SHEET 23 OF 71 SHEETS



PLAN - WESTBOUND TOP OF WEST APPROACH SLAB ELEVATIONS

FACE OF WB N. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+49.88	-56.00	738.97	738.99
W1	1344+59.88	-56.00	738.96	738.98
W2	1344+69.88	-56.00	738.95	738.97
E. END OF W. APPR. SLAB	1344+79.88	-56.00	738.93	738.95

CROSS SLOPE CHANGE BETWEEN GIRDERS 3 & 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+37.17	-34.00	739.54	739.56
W1	1344+47.17	-34.00	739.53	739.55
W2	1344+57.17	-34.00	739.51	739.53
E. END OF W. APPR. SLAB	1344+67.17	-34.00	739.50	739.52

CROSS SLOPE CHANGE BETWEEN GIRDERS 5 & 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+30.25	-22.00	739.79	739.81
W1	1344+40.25	-22.00	739.78	739.80
W2	1344+50.25	-22.00	739.76	739.78
E. END OF W. APPR. SLAB	1344+60.25	-22.00	739.75	739.77

PROP. PGL (WB)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+23.32	-10.00	739.98	740.00
W1	1344+33.32	-10.00	739.97	739.99
W2	1344+43.32	-10.00	739.95	739.97
E. END OF W. APPR. SLAB	1344+53.32	-10.00	739.94	739.96

FACE OF WB S. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+18.41	-1.50	739.82	739.84
W1	1344+28.41	-1.50	739.80	739.82
W2	1344+38.41	-1.50	739.79	739.81
E. END OF W. APPR. SLAB	1344+48.41	-1.50	739.77	739.80

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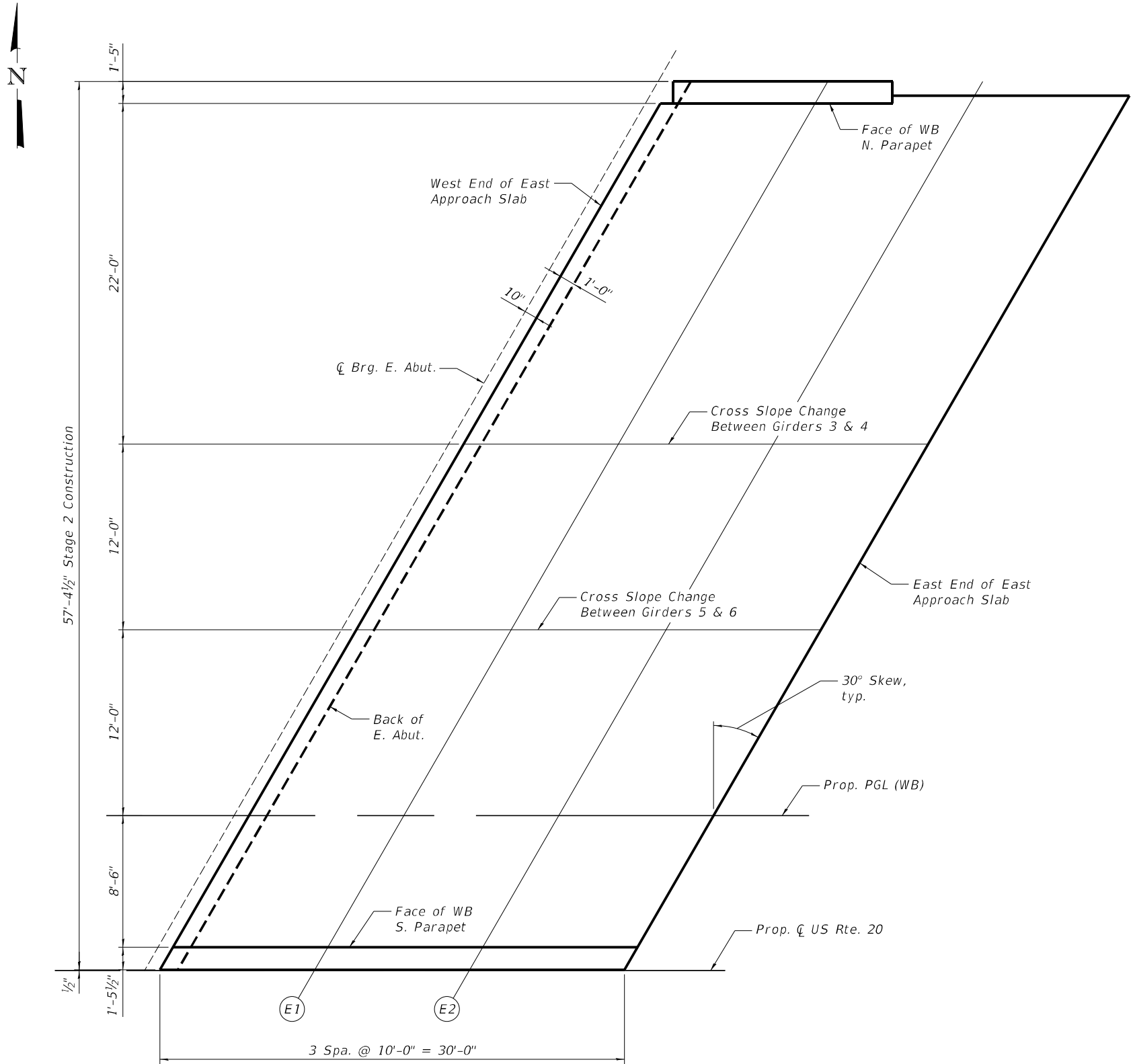
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WESTBOUND TOP OF WEST APPROACH SLAB ELEVATIONS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 24 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	26
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

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PLAN - WESTBOUND TOP OF EAST APPROACH SLAB ELEVATIONS

FACE OF WB N. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+47.57	-56.00	738.56	738.58
E1	1347+57.57	-56.00	738.55	738.57
E2	1347+67.57	-56.00	738.53	738.56
E. END OF E. APPR. SLAB	1347+77.57	-56.00	738.52	738.54

CROSS SLOPE CHANGE BETWEEN GIRDERS 3 & 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+34.87	-34.00	739.13	739.15
E1	1347+44.87	-34.00	739.12	739.14
E2	1347+54.87	-34.00	739.10	739.12
E. END OF E. APPR. SLAB	1347+64.87	-34.00	739.09	739.11

CROSS SLOPE CHANGE BETWEEN GIRDERS 5 & 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+27.94	-22.00	739.38	739.40
E1	1347+37.94	-22.00	739.37	739.39
E2	1347+47.94	-22.00	739.35	739.37
E. END OF E. APPR. SLAB	1347+57.94	-22.00	739.34	739.36

PROP. PGL (WB)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+21.01	-10.00	739.57	739.59
E1	1347+31.01	-10.00	739.56	739.58
E2	1347+41.01	-10.00	739.54	739.56
E. END OF E. APPR. SLAB	1347+51.01	-10.00	739.53	739.55

FACE OF WB S. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+16.10	-1.50	739.41	739.43
E1	1347+26.10	-1.50	739.39	739.41
E2	1347+36.10	-1.50	739.38	739.40
E. END OF E. APPR. SLAB	1347+46.10	-1.50	739.36	739.39

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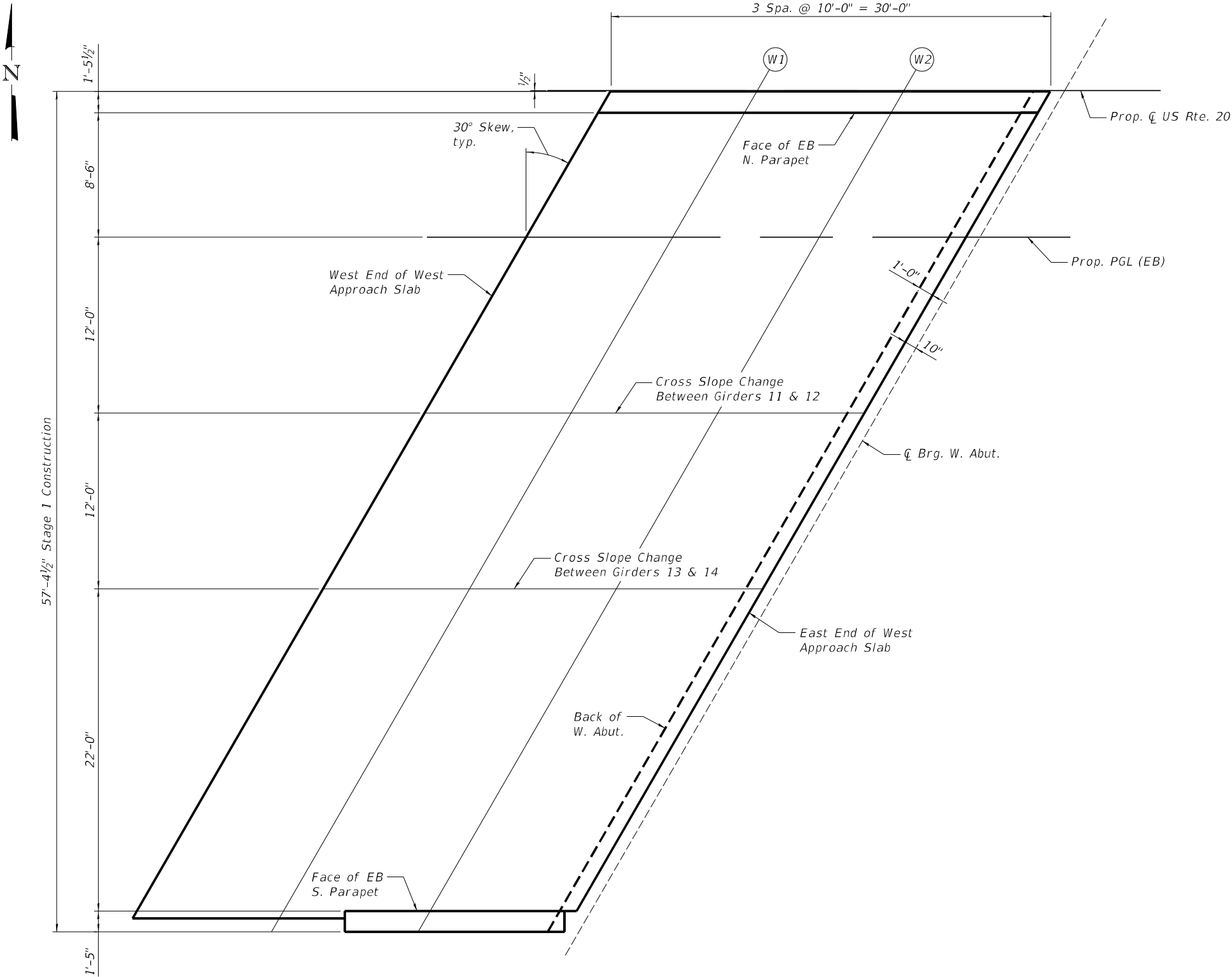
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WESTBOUND TOP OF EAST APPROACH SLAB ELEVATIONS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 25 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	27
		CONTRACT NO. 64U98		
		ILLINOIS FED. AID PROJECT		



PLAN - EASTBOUND TOP OF WEST APPROACH SLAB ELEVATIONS

FACE OF EB N. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+16.68	1.50	739.82	739.84
W1	1344+26.68	1.50	739.80	739.83
W2	1344+36.68	1.50	739.79	739.81
E. END OF W. APPR. SLAB	1344+46.68	1.50	739.78	739.80

PROP. PGL (EB)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+11.77	10.00	740.00	740.02
W1	1344+21.77	10.00	739.98	740.00
W2	1344+31.77	10.00	739.97	739.99
E. END OF W. APPR. SLAB	1344+41.77	10.00	739.95	739.97

CROSS SLOPE CHANGE BETWEEN GIRDERS 11 & 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1344+04.84	22.00	739.83	739.85
W1	1344+14.84	22.00	739.81	739.83
W2	1344+24.84	22.00	739.80	739.82
E. END OF W. APPR. SLAB	1344+34.84	22.00	739.78	739.80

CROSS SLOPE CHANGE BETWEEN GIRDERS 13 & 14

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1343+97.91	34.00	739.59	739.62
W1	1344+07.91	34.00	739.58	739.60
W2	1344+17.91	34.00	739.57	739.59
E. END OF W. APPR. SLAB	1344+27.91	34.00	739.55	739.57

FACE OF EB S. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF W. APPR. SLAB	1343+85.21	56.00	739.06	739.08
W1	1343+95.21	56.00	739.05	739.07
W2	1344+05.21	56.00	739.03	739.06
E. END OF W. APPR. SLAB	1344+15.21	56.00	739.02	739.04

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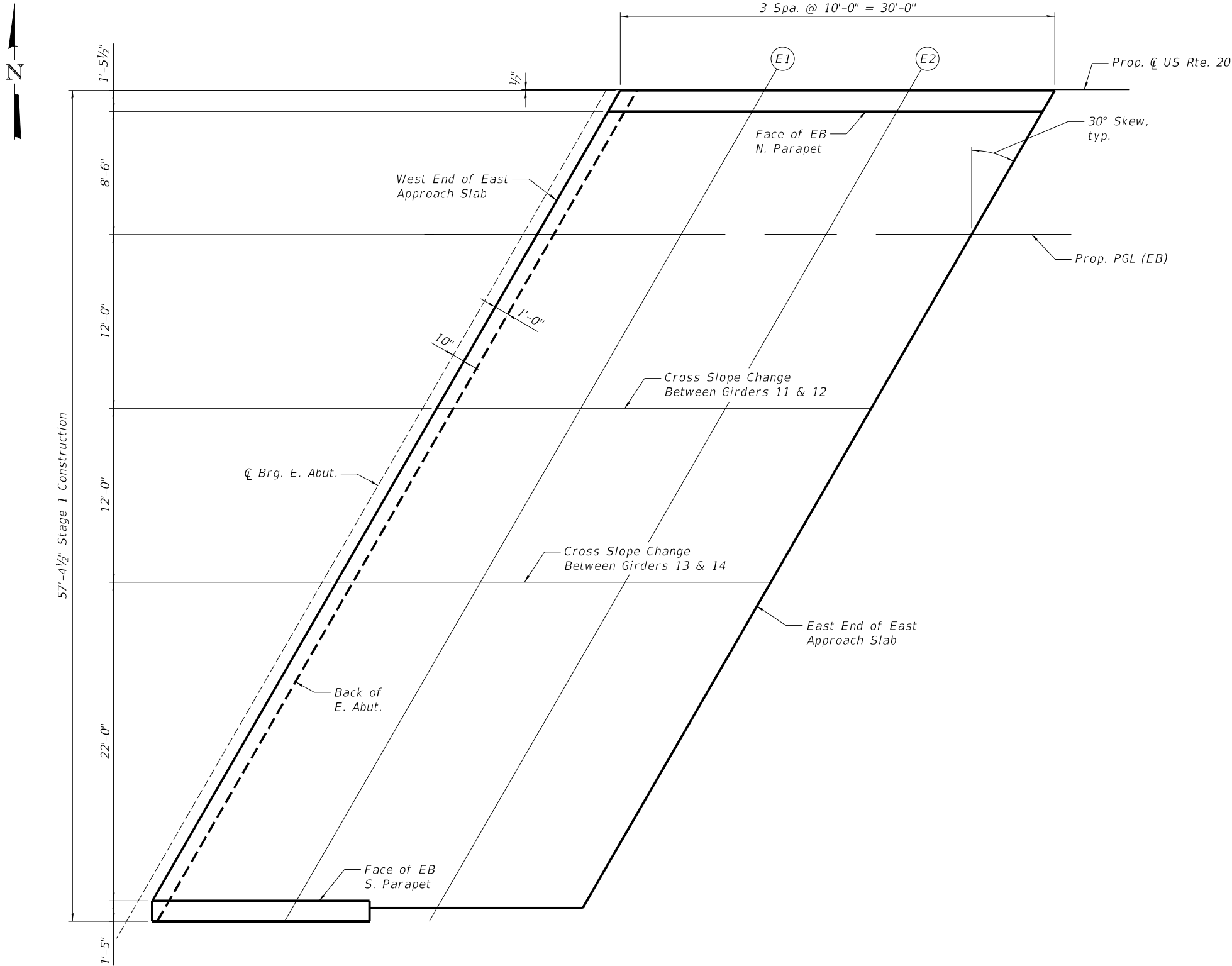
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EASTBOUND TOP OF WEST APPROACH SLAB ELEVATIONS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 26 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	28
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				



PLAN - EASTBOUND TOP OF EAST APPROACH SLAB ELEVATIONS

FACE OF EB N. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+14.37	1.50	739.41	739.43
E1	1347+24.37	1.50	739.39	739.42
E2	1347+34.37	1.50	739.38	739.40
E. END OF E. APPR. SLAB	1347+44.37	1.50	739.37	739.39

PROP. PGL (EB)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+09.46	10.00	739.58	739.61
E1	1347+19.46	10.00	739.57	739.59
E2	1347+29.46	10.00	739.56	739.58
E. END OF E. APPR. SLAB	1347+39.46	10.00	739.54	739.56

CROSS SLOPE CHANGE BETWEEN GIRDERS 11 & 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1347+02.53	22.00	739.41	739.44
E1	1347+12.53	22.00	739.40	739.42
E2	1347+22.53	22.00	739.39	739.41
E. END OF E. APPR. SLAB	1347+32.53	22.00	739.37	739.39

CROSS SLOPE CHANGE BETWEEN GIRDERS 13 & 14

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1346+95.61	34.00	739.18	739.20
E1	1347+05.61	34.00	739.17	739.19
E2	1347+15.61	34.00	739.16	739.18
E. END OF E. APPR. SLAB	1347+25.61	34.00	739.14	739.16

FACE OF EB S. PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
W. END OF E. APPR. SLAB	1346+82.90	56.00	738.65	738.67
E1	1346+92.90	56.00	738.64	738.66
E2	1347+02.90	56.00	738.62	738.64
E. END OF E. APPR. SLAB	1347+12.90	56.00	738.61	738.63

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Alfred Benesch & Company
35 W Wacker Drive, Suite 2300
Chicago, Illinois 60601
312.465.4150 Job No. 10800

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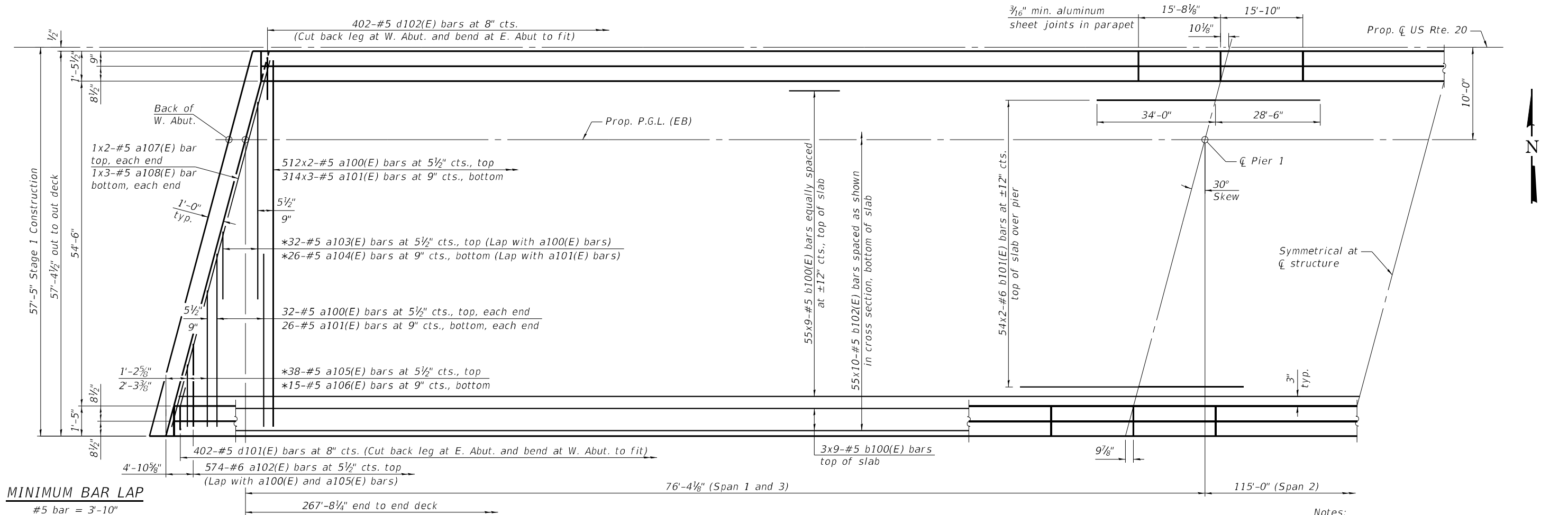
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EASTBOUND TOP OF EAST APPROACH SLAB ELEVATIONS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 27 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	29
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		

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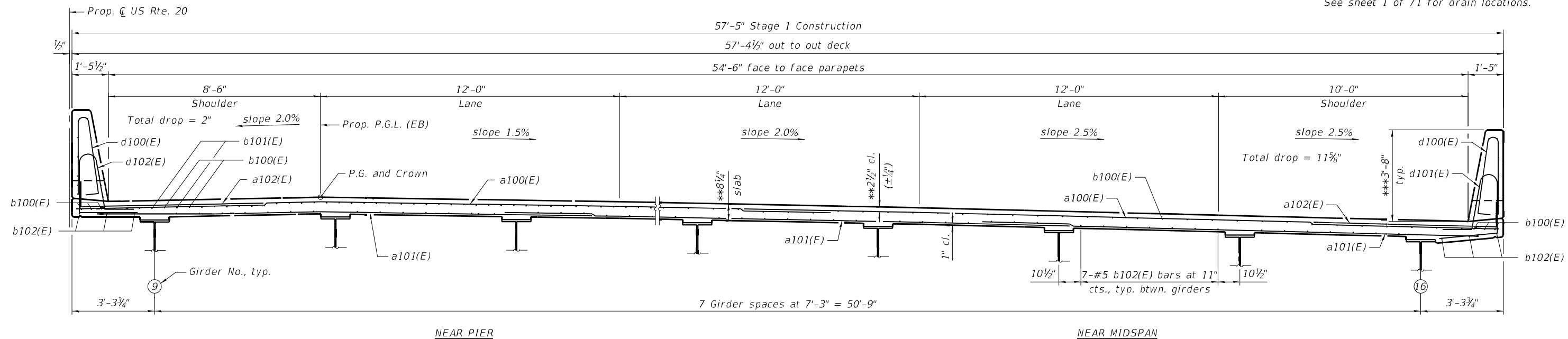
MINIMUM BAR LAP

#5 bar = 3'-10"
#6 bar = 4'-10"

- * See Field Cutting Diagram on sheet 34 of 71.
- ** Prior to grinding.
- *** After grinding.

PARTIAL PLAN - EASTBOUND

Notes:
See sheet 31 of 71 for superstructure and drain details.
See sheet 34 of 71 for additional details and Bill of Material.
Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
See sheet 1 of 71 for drain locations.



CROSS SECTION

(Looking East)

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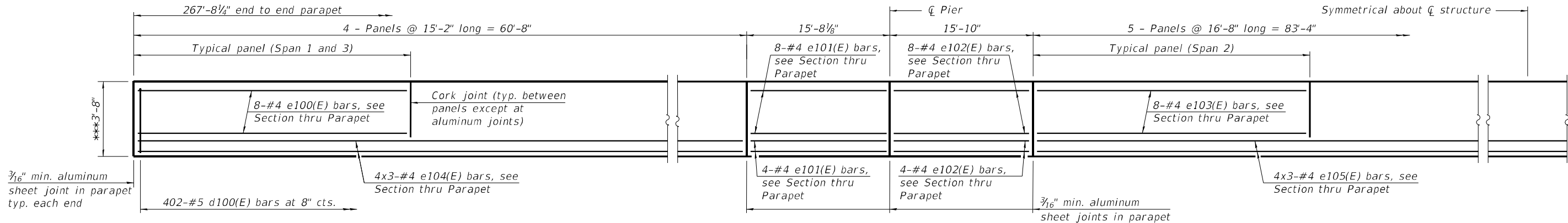
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EASTBOUND SUPERSTRUCTURE
STRUCTURE NO. 101-0225 & 101-0226

SHEET 29 OF 71 SHEETS

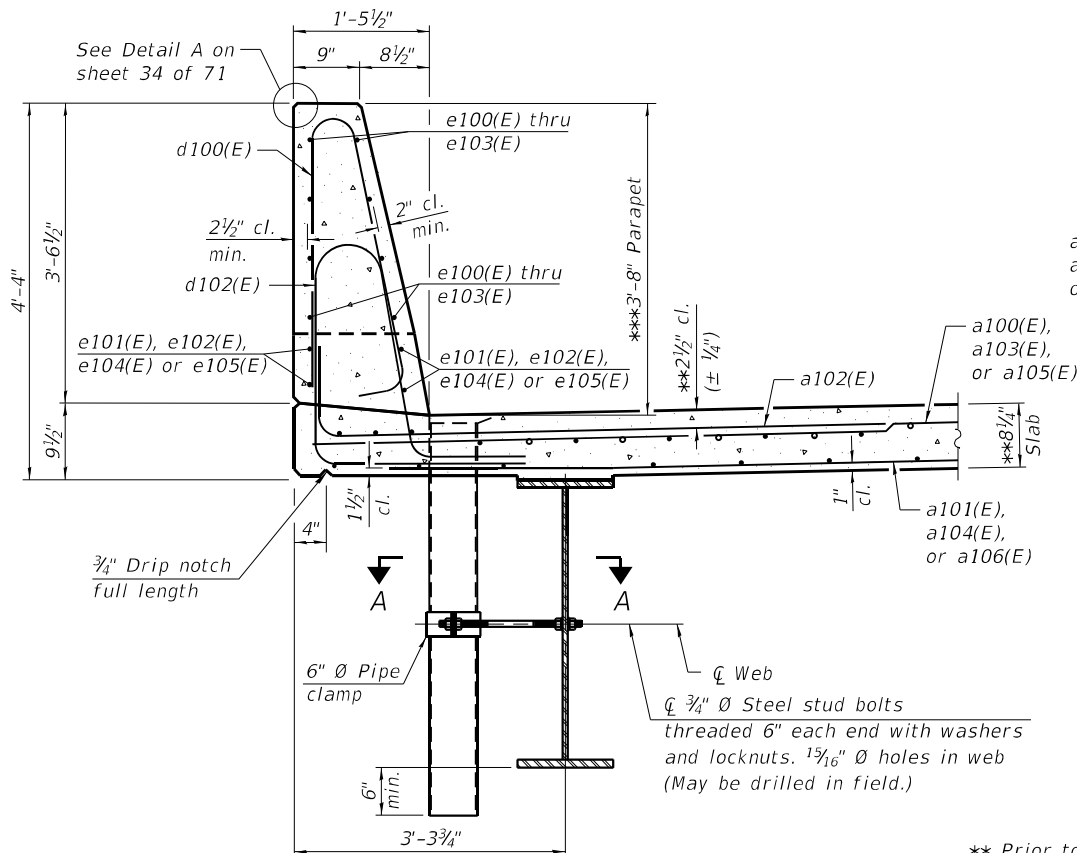
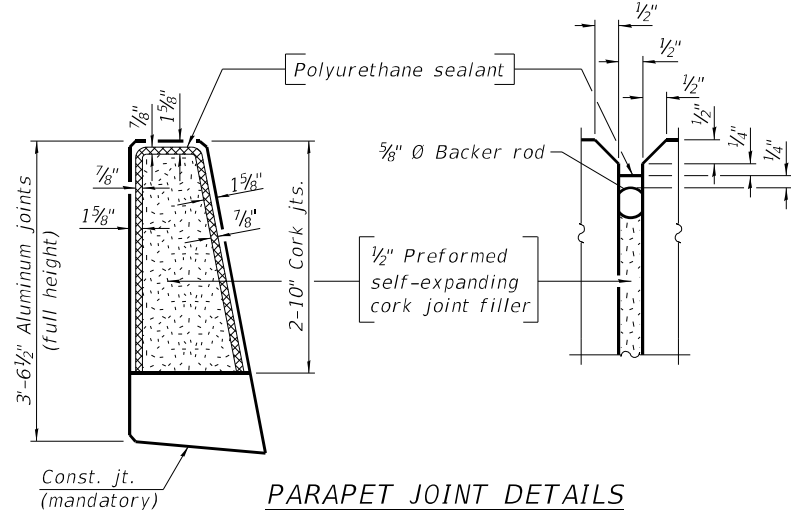
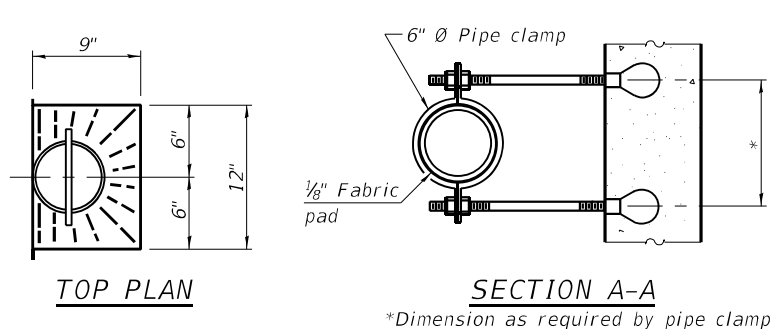
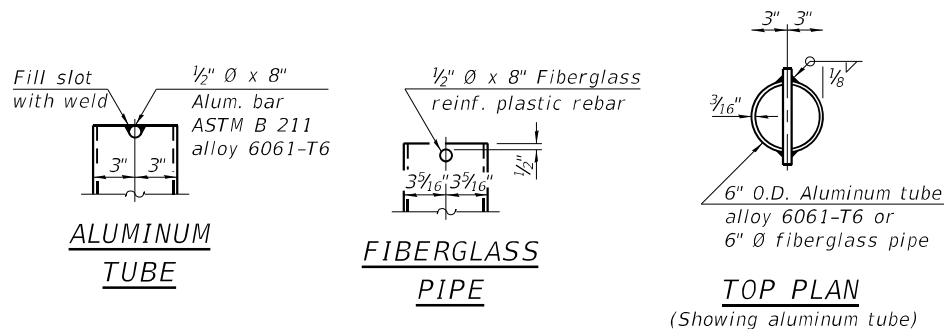
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	31
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

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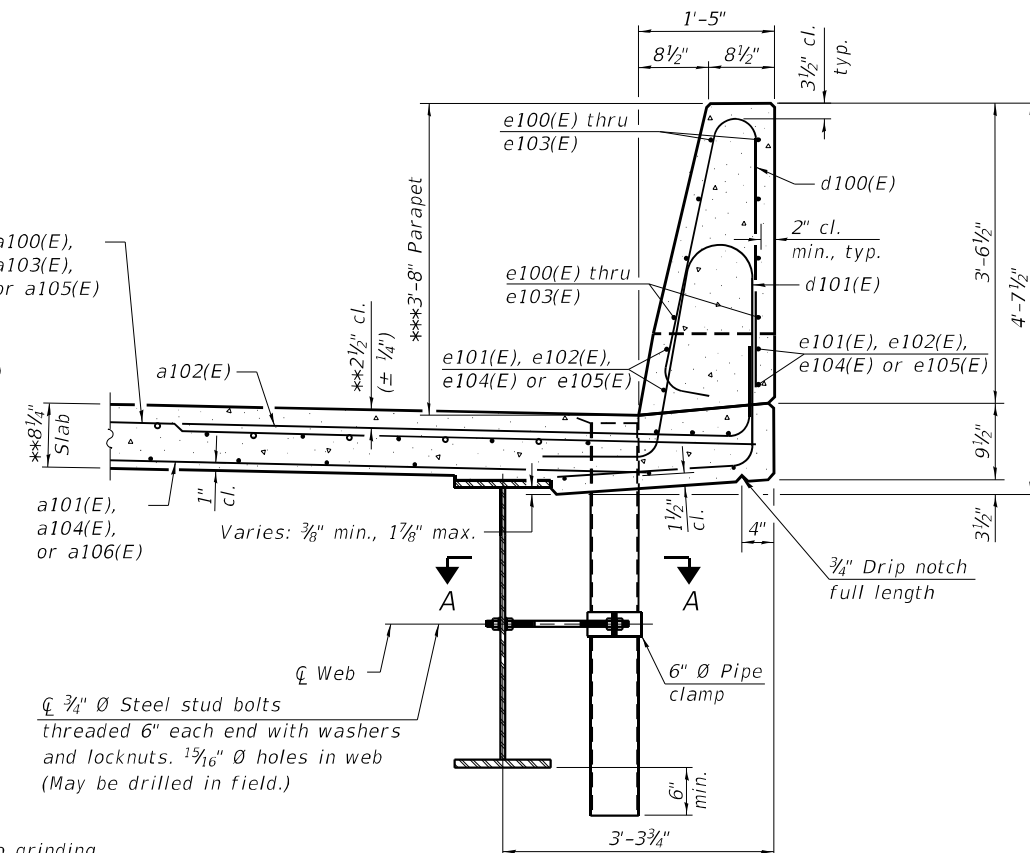
INSIDE ELEVATION OF PARAPET - EASTBOUND

MINIMUM BAR LAP
#4 bar = 2'-5"



SECTION THRU NORTH PARAPET

** Prior to grinding.
*** After grinding.



SECTION THRU SOUTH PARAPET

Notes:
Fiberglass pipe shall conform to ASTM D2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.
The exterior surfaces of the floor drains shall be painted according to Article 506 with the finish coat as specified. The exterior surfaces of the drains shall be cleaned according to the Society of Protective Coatings Spec. SSPC-SP1 prior to painting.
The top portion of aluminum floor drains shall be coated with 5 mils of either bitumen paint or epoxy paint to minimize reaction with wet concrete.
The clamping device shall be galvanized according to AASHTO M 232. Cost of clamping device included with Floor Drains.
The 3/16" min. aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated with 5 mils of either bitumen paint or epoxy paint to minimize reaction with wet concrete. Cost included with Concrete Superstructure.
The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.
Bars indicated thus 20 x 3-#5 etc. indicated 20 lines of bars with 3 lengths per line.
See sheet 34 of 71 for additional details and Bill of Material.
See sheet 1 of 71 for drain locations.

FOR INFORMATION ONLY



QUIGG ENGINEERING INC

USER NAME =	ZDavidson	DESIGNED -	ZLD	REVISED -	
D264R72-1010225_0226-SHT-031-EB Superstructure Details.dgn		DESIGNED -	DRB	REVISED -	
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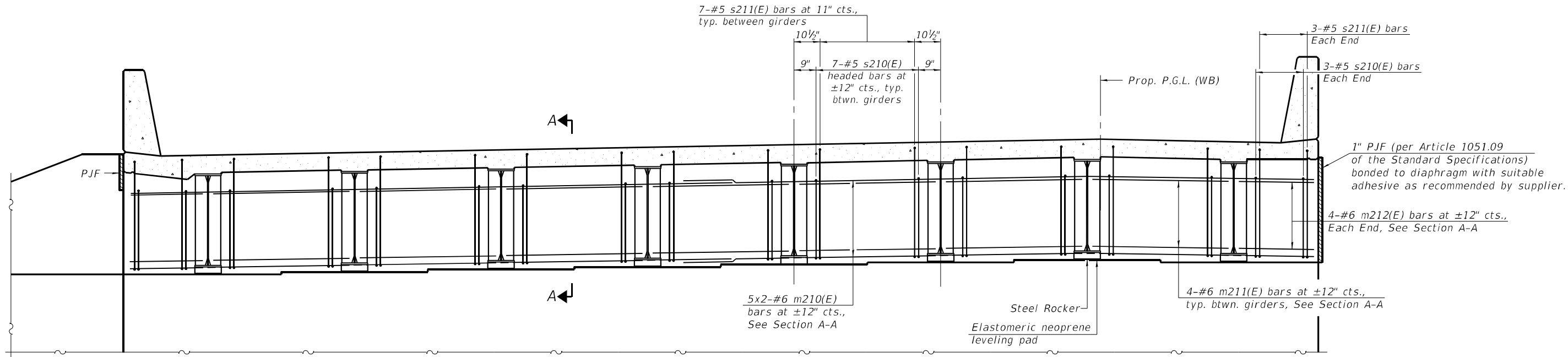
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EASTBOUND SUPERSTRUCTURE DETAILS
STRUCTURE NO. 101-0225 & 101-0226

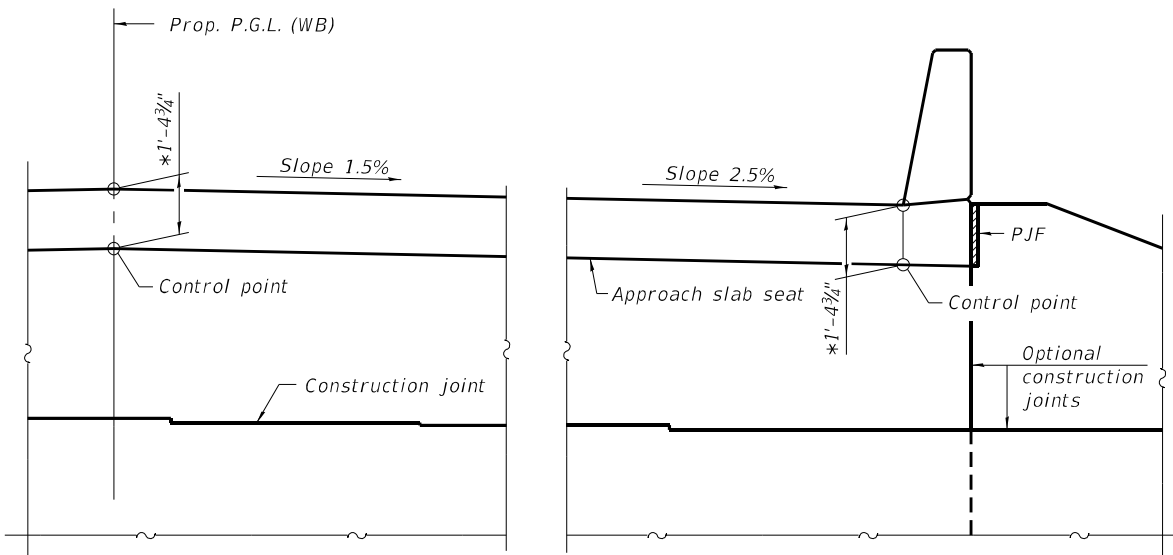
SHEET 31 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

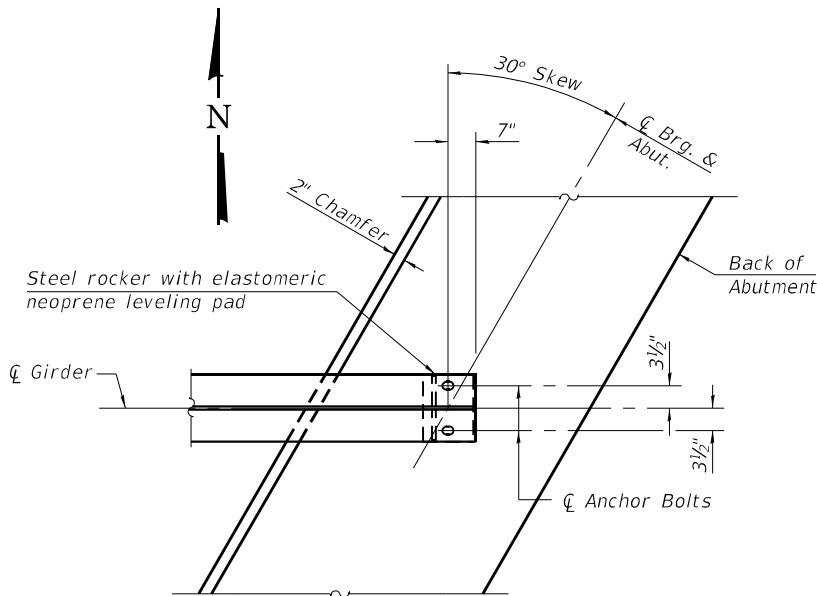
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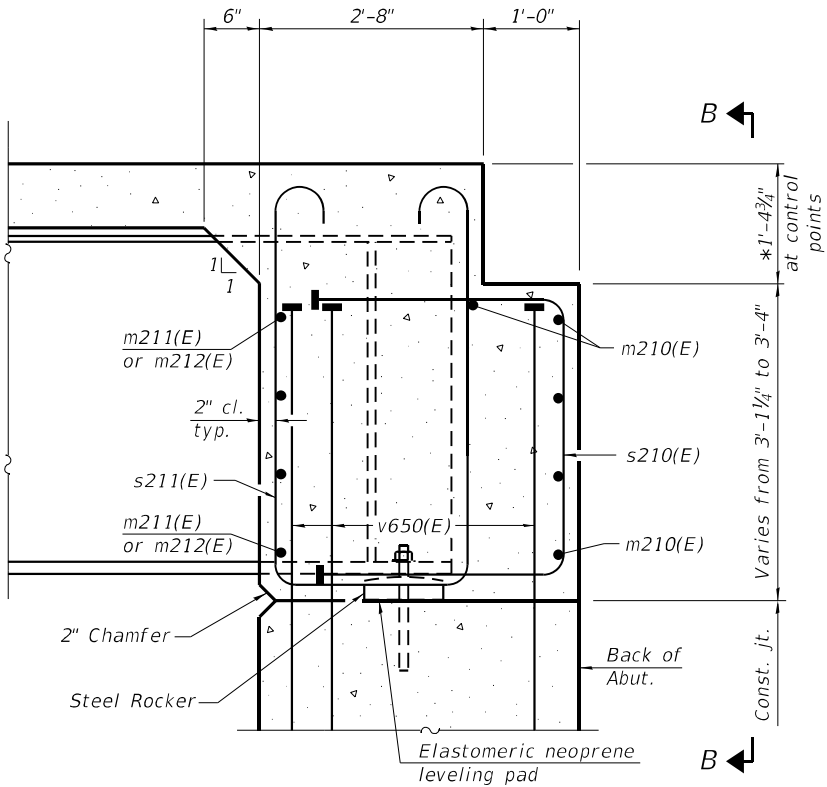
DIAPHRAGM AT ABUTMENT
(Looking East)
(East Abutment shown, West Abutment similar)



VIEW B-B



PLAN AT ABUTMENT
(Showing bottom flange of beam)
(East Abutment shown, West Abutment similar)



SECTION A-A
(at Rt. L's)

Notes:
See sheet 30 of 71 for superstructure details.
See sheet 34 of 71 for additional details and Bill of Material.
See sheet 39 of 71 for PJF details.
The s210(E) and s211(E) bars shall be placed parallel to the girders.
Spacing for these bars shall be at right angles to the girders.
The approach slab seat shall have a constant slope determined from the control points shown.
Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.

* Prior to grinding

MINIMUM BAR LAP
#6 bar = 3'-0"

FOR INFORMATION ONLY

DIA-SB-L

2-1-2023



QUIGG ENGINEERING INC

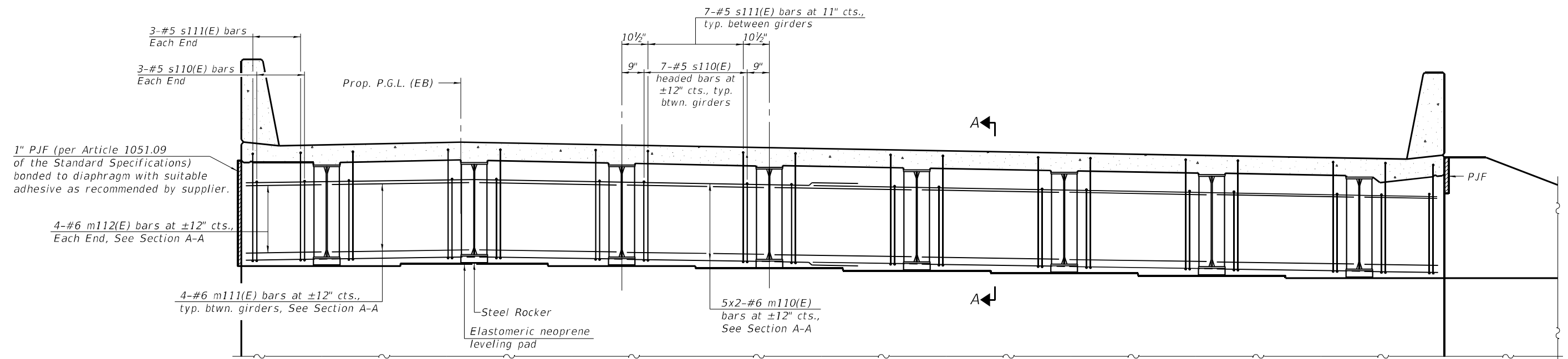
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

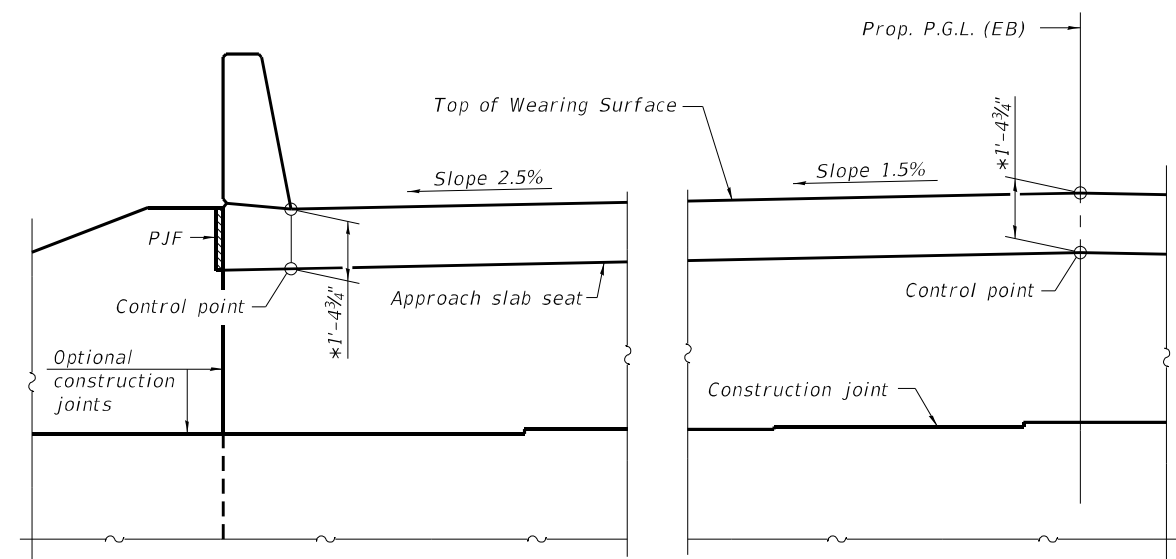
**WESTBOUND DIAPHRAGM DETAILS
STRUCTURE NO. 101-0225 & 101-0226**

SHEET 32 OF 71 SHEETS

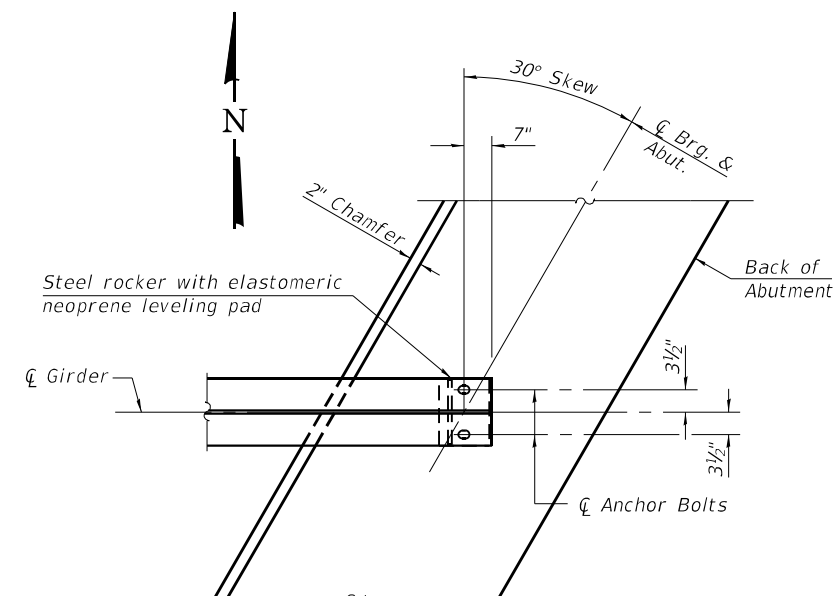
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	34
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				



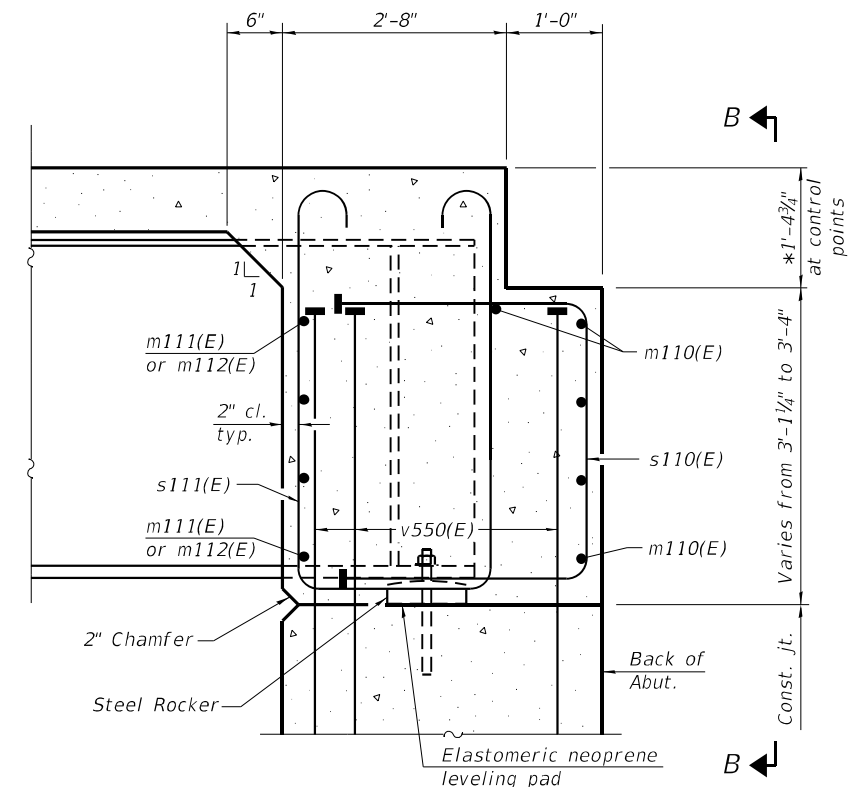
DIAPHRAGM AT ABUTMENT
(Looking East)
(East Abutment shown, West Abutment similar)



VIEW B-B



PLAN AT ABUTMENT
(Showing bottom flange of beam)
(East Abutment shown, West Abutment similar)



SECTION A-A
(at Rt. \angle 's)

Notes:

- See sheet 31 of 71 for superstructure details.
- See sheet 34 of 71 for additional details and Bill of Material.
- See sheet 40 of 71 for PJF details.
- The s110(E) and s111(E) bars shall be placed parallel to the girders.
- Spacing for these bars shall be at right angles to the girders.
- The approach slab seat shall have a constant slope determined from the control points shown.
- Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.

* Prior to grinding

MINIMUM BAR LAP
#6 bar = 3'-0"

FOR INFORMATION ONLY

DIA-SB-L

2-1-2023

USER NAME = ZDavidson	DESIGNED - ZLD	REVISED -
D264R72-1010225_0226-SHT-033-EB Diaphragm, CH	CHECKED - DRB	REVISED -
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PLOT DATE =	CHECKED - MDC	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

EASTBOUND DIAPHRAGM DETAILS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 33 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	35
		CONTRACT NO. 64U98		
		ILLINOIS	FEED	AID PROJECT

ILLINOIS	FED. AID PROJECT
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FOR INFORMATION ONLY



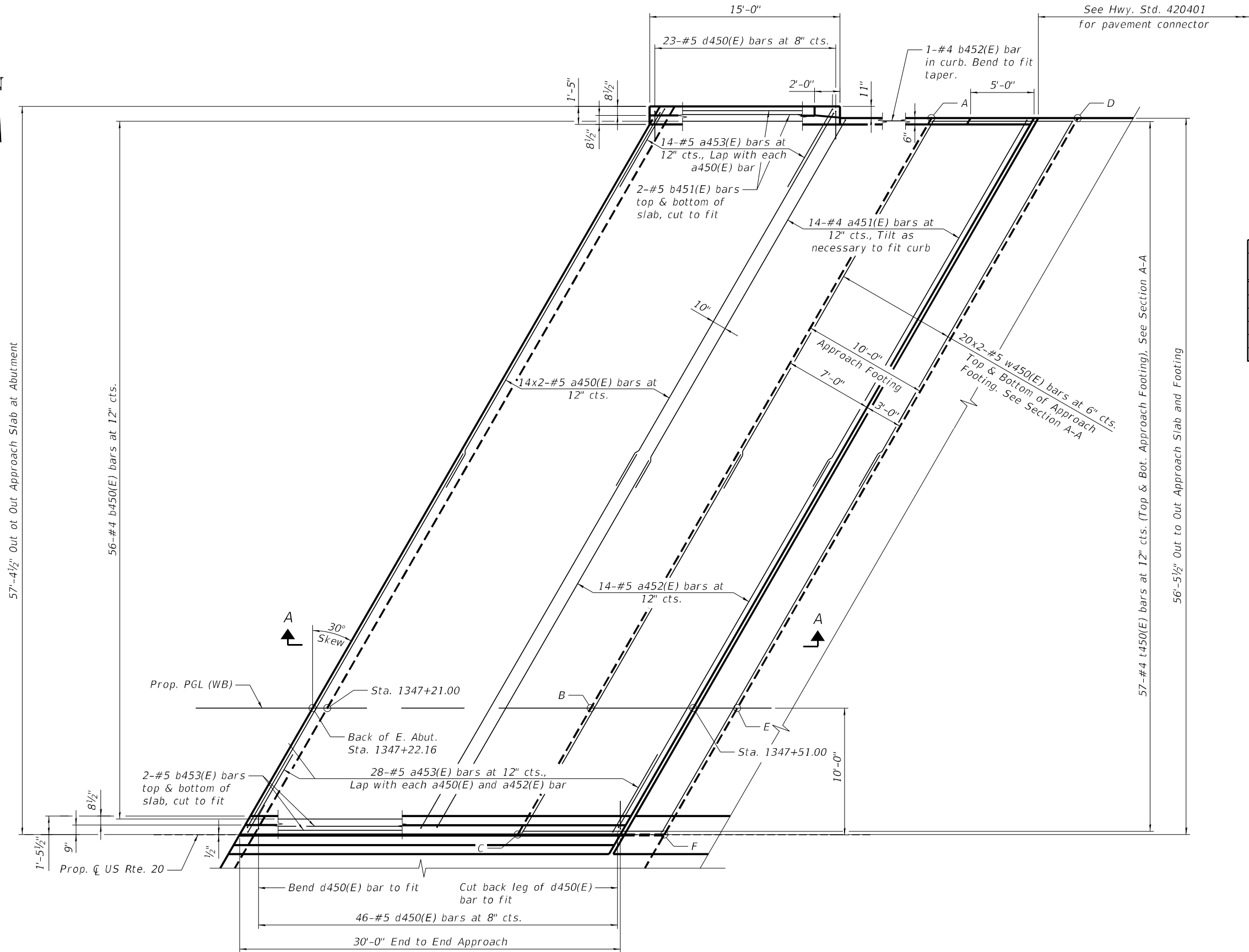
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EAST PRECAST BRIDGE APPROACH SLAB (WESTBOUND)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 36 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	38
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



PLAN

TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING

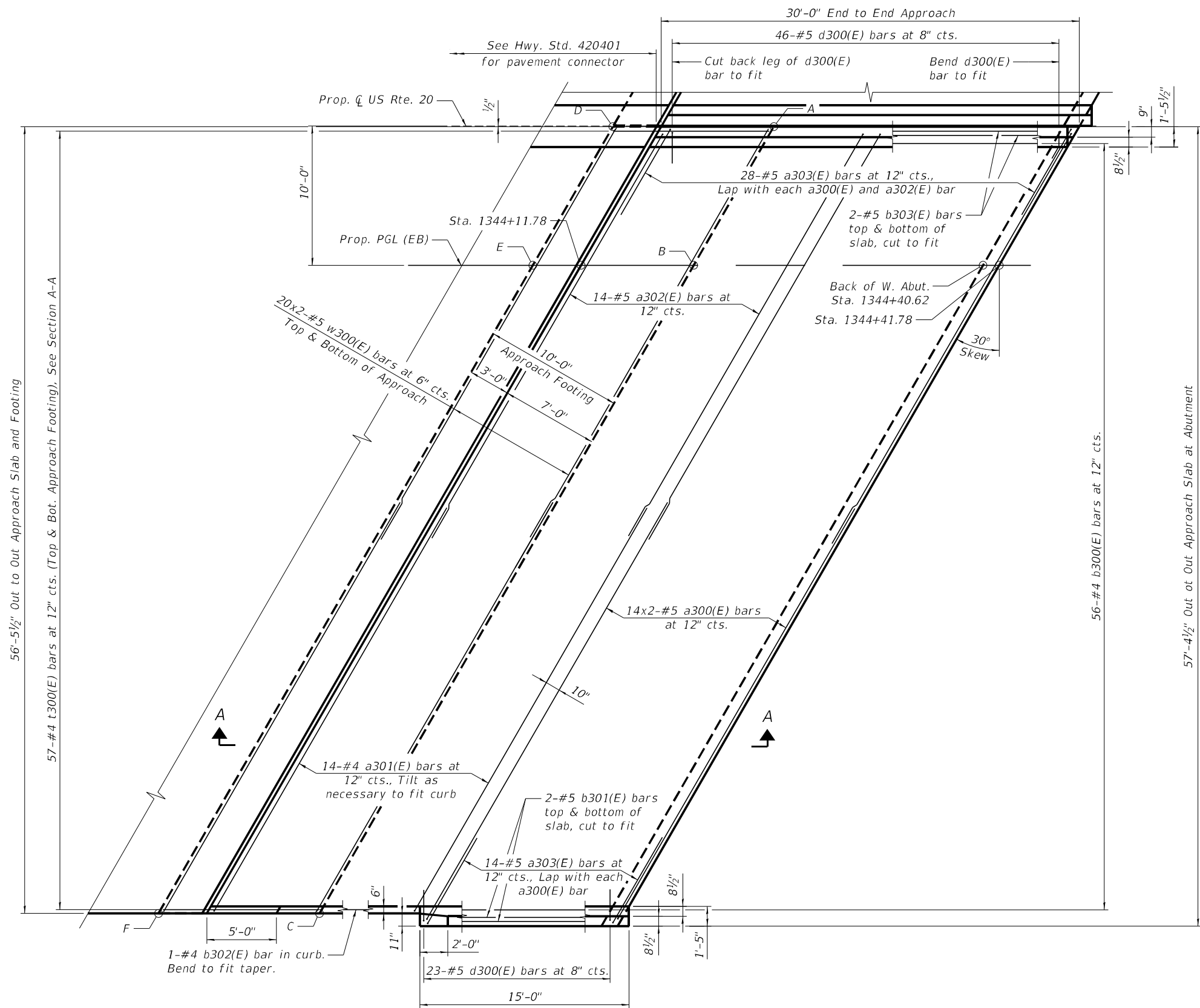
E. Approach (WB)				
Point/ Location	Station	Offset	Top	Bottom
A	1347+69.77	-56.50 ft	737.12	736.29
B	1347+42.93	-10.00 ft	738.14	737.31
C	1347+37.18	-0.04 ft	737.95	737.12
D	1347+81.32	-56.50 ft	737.11	736.27
E	1347+54.47	-10.00 ft	738.13	737.29
F	1347+48.72	-0.04 ft	737.94	737.10

MINIMUM BAR LAP

#4 bar = 1'-7"
#5 bar = 2'-0"

NOTE:

- See Sheet 42 of 71 for Section A-A.
- See Sheet 43 of 71 for bar bends and bill of materials.



PLAN

TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING

W. Approach (EB)				
Point/ Location	Station	Offset	Top	Bottom
A	1344+25.60	0.04 ft	738.38	737.55
B	1344+19.85	10.00 ft	738.59	737.76
C	1343+93.01	56.50 ft	737.64	736.81
D	1344+14.06	0.04 ft	738.40	737.56
E	1344+08.31	10.00 ft	738.60	737.77
F	1343+81.46	56.50 ft	737.66	736.83

MINIMUM BAR LAP

#4 bar = 1'-7"
#5 bar = 2'-0"

NOTE:

- See Sheet 42 of 71 for Section A-A.
- See Sheet 43 of 71 for bar bends and bill of materials.

FOR INFORMATION ONLY

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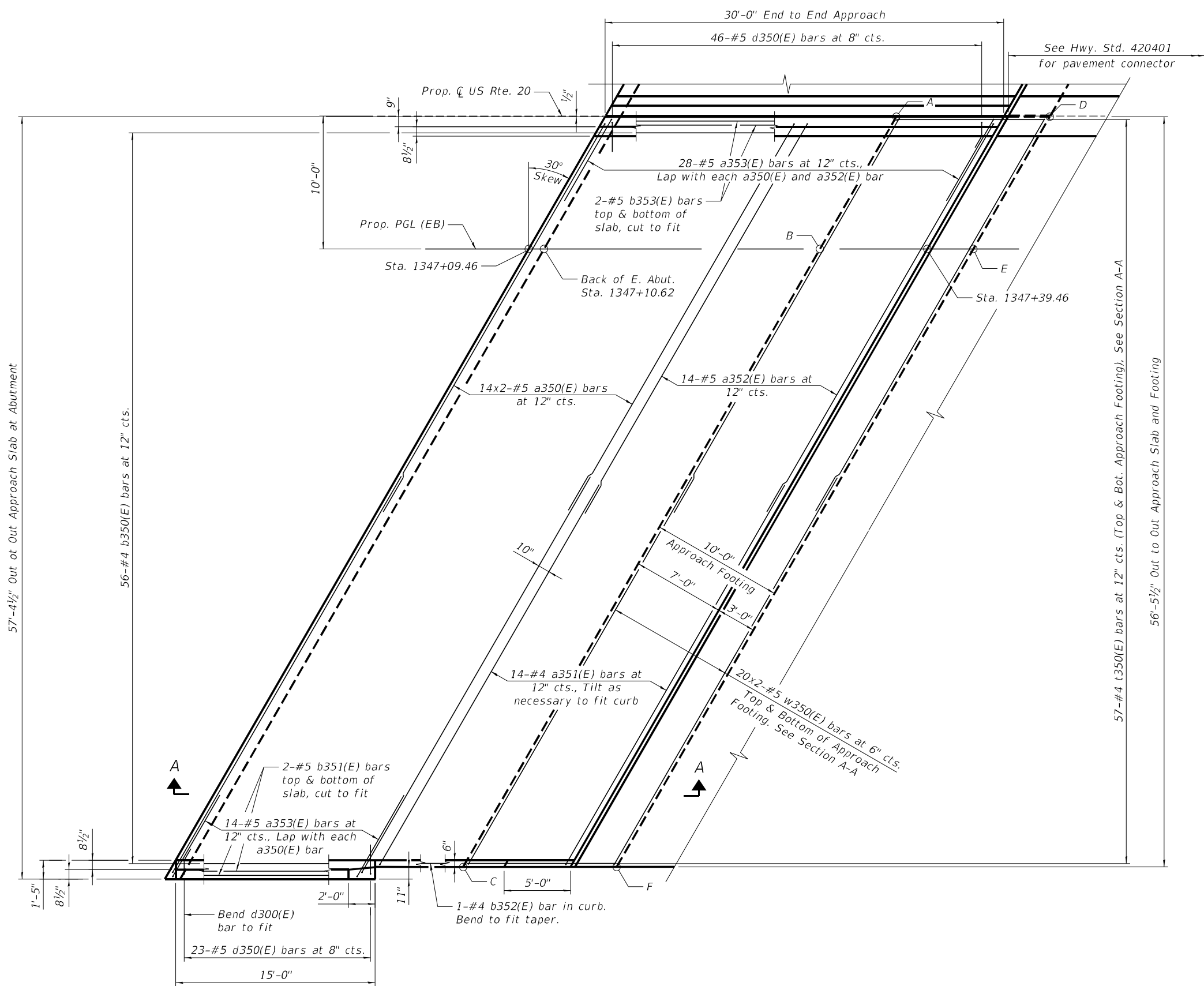
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PLOT DATE	=	CHECKED	-	KMP	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WEST PRECAST BRIDGE APPROACH SLAB (EASTBOUND)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 37 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	39
CONTRACT NO. 64U98				
		ILLINOIS	FED. AID PROJECT	



TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING

E. Approach (EB)				
Point/ Location	Station	Offset	Top	Bottom
A	1347+37.13	0.04 ft	737.95	737.12
B	1347+31.38	10.00 ft	738.16	737.33
C	1347+04.53	56.50 ft	737.21	736.38
D	1347+48.68	0.04 ft	737.94	737.10
E	1347+42.93	10.00 ft	738.14	737.31
F	1347+16.08	56.50 ft	737.20	736.36

MINIMUM BAR LAP

#4 bar = 1'-7"
#5 bar = 2'-0"

PLAN

FOR INFORMATION ONLY

- NOTE:
- See Sheet 42 of 71 for Section A-A.
 - See Sheet 43 of 71 for bar bends and bill of materials.

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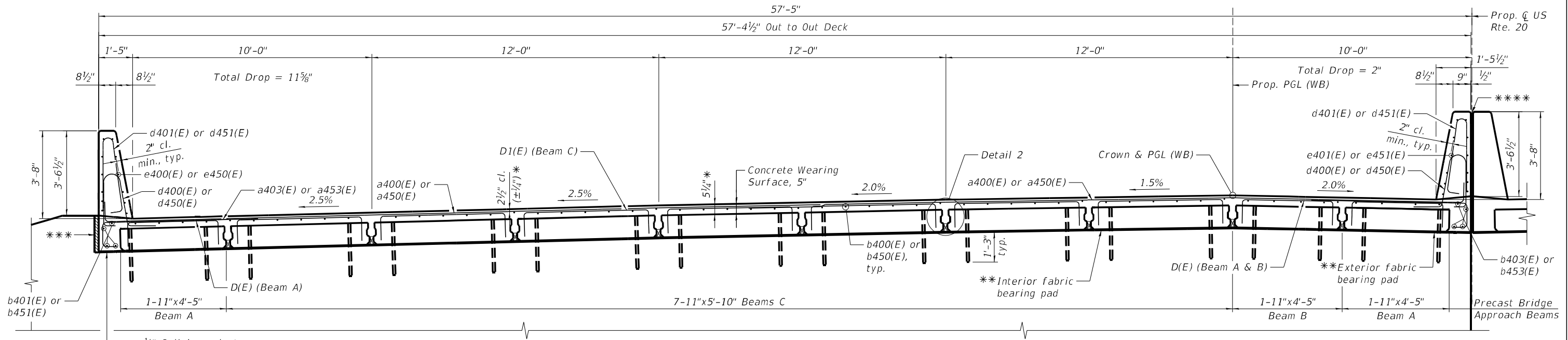
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EAST PRECAST BRIDGE APPROACH SLAB (EASTBOUND)
STRUCTURE NO. 101-0225 & 101-0226

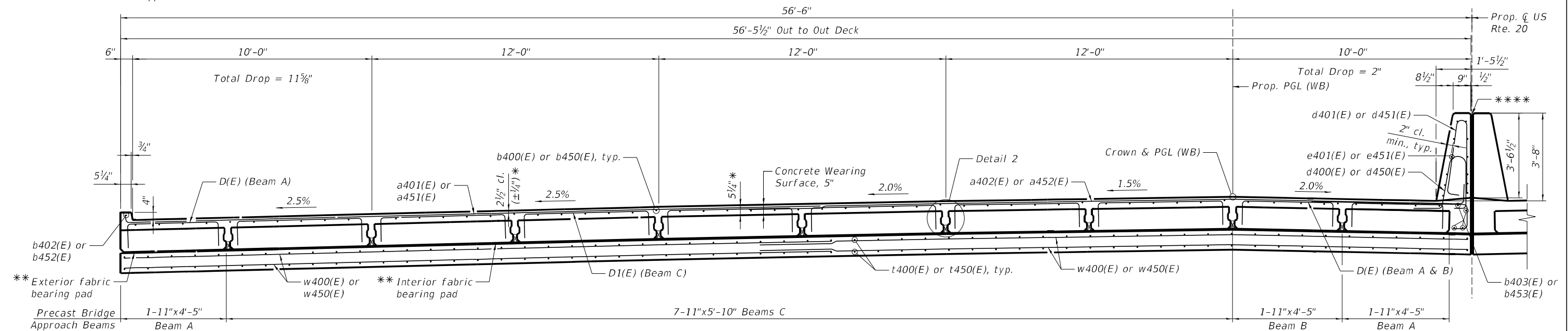
SHEET 38 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	40
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

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WESTBOUND APPROACH CROSS SECTION AT ABUTMENT
(Looking East)



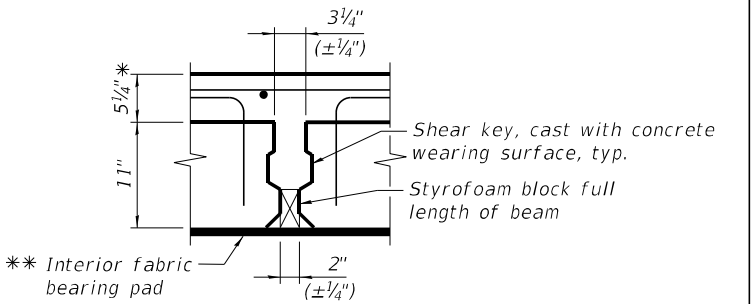
WESTBOUND APPROACH CROSS SECTION AT APPROACH FOOTING
(Looking East)

- * Prior to grinding
- ** Fabric bearing pads at the expansion end shall be recessed 1/4" into the approach footing and bonded. Adjusting shims, when required, shall be bonded to the top of the fabric bearing pads.
- *** 2" PJF (per Article 1051.09 of the Standard Specifications) bonded to wingwall with suitable adhesive as recommended by supplier.
- **** 1" PJF full height and length of parapet, approach slab, and approach footing at median (per Article 1051.09 of the Standard Specifications) bonded to concrete with suitable adhesive as recommended by supplier.

MINIMUM BAR LAP

#4 bar = 1'-7"
#5 bar = 2'-0"

NOTE:
See Sheet 43 of 71 for bar bends and bill of materials.



DETAIL 2

FOR INFORMATION ONLY



USER NAME	=	DESIGNED	-	AED	REVISED	-
PLOT SCALE	=	CHECKED	-	KMP	REVISED	-
PLOT DATE	=	DRAWN	-	RMG	REVISED	-
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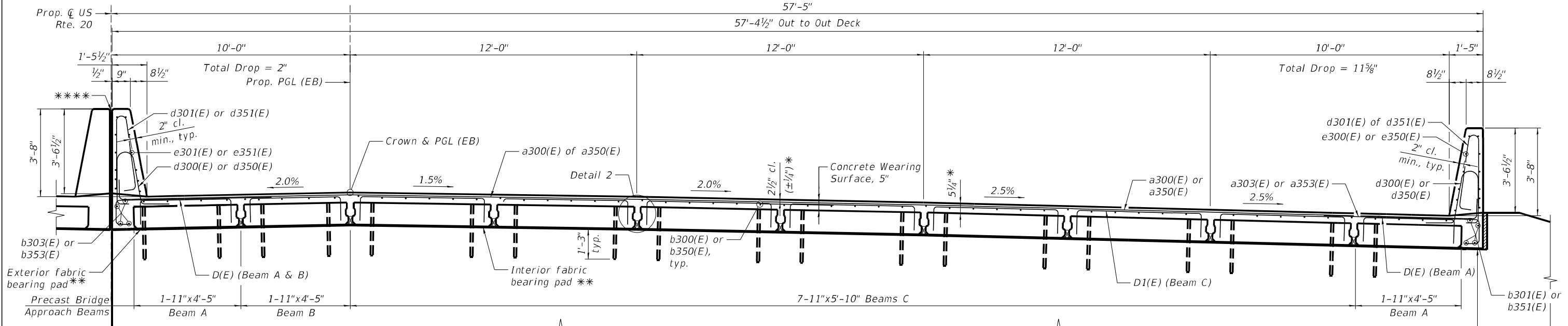
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PRECAST BRIDGE APPROACH SLAB DETAILS (1 OF 5)
STRUCTURE NO. 101-0225 & 101-0226**

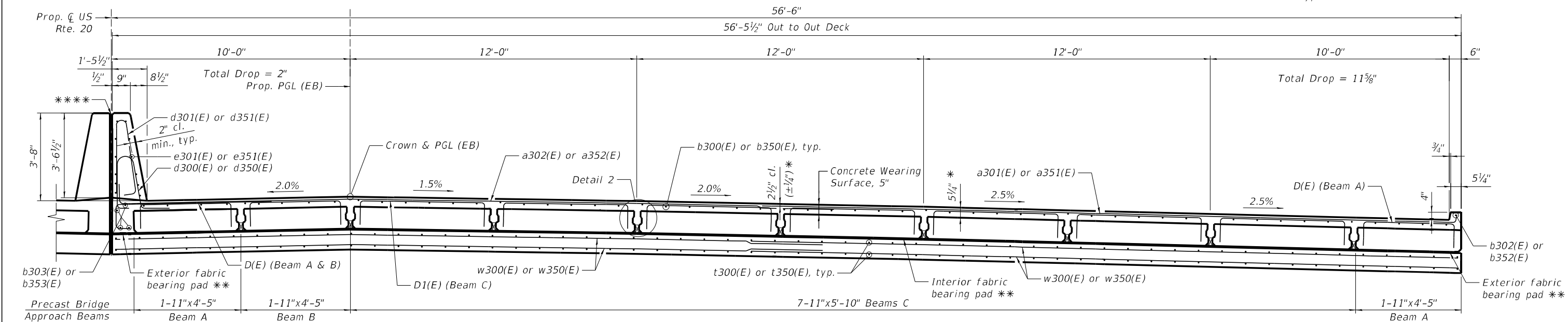
SHEET 39 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	41
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

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EASTBOUND APPROACH CROSS SECTION AT ABUTMENT

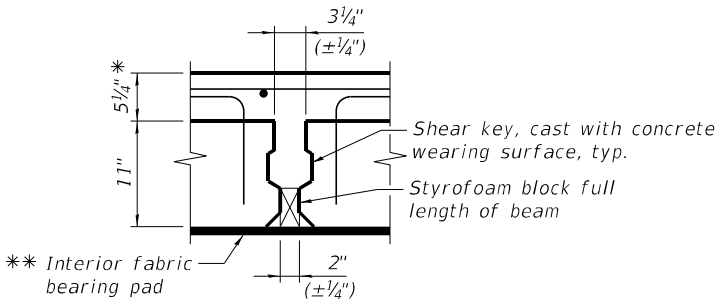


EASTBOUND APPROACH CROSS SECTION AT APPROACH FOOTING
(Looking East)

- * Prior to grinding
- ** Fabric bearing pads at the expansion end shall be recessed 1/4" into the approach footing and bonded. Adjusting shims, when required, shall be bonded to the top of the fabric bearing pads.
- *** 2" PJF (per Article 1051.09 of the Standard Specifications) bonded to wingwall with suitable adhesive as recommended by supplier.
- **** 1" PJF full height and length of parapet, approach slab, and approach footing at median (per Article 1051.09 of the Standard Specifications) bonded to concrete with suitable adhesive as recommended by supplier.

MINIMUM BAR LAP
#4 bar = 1'-7"
#5 bar = 2'-0"

NOTE:
See Sheet 43 of 71 for bar bends and bill of materials.



DETAIL 2

FOR INFORMATION ONLY



USER NAME	=	DESIGNED	-	AED	REVISED	-
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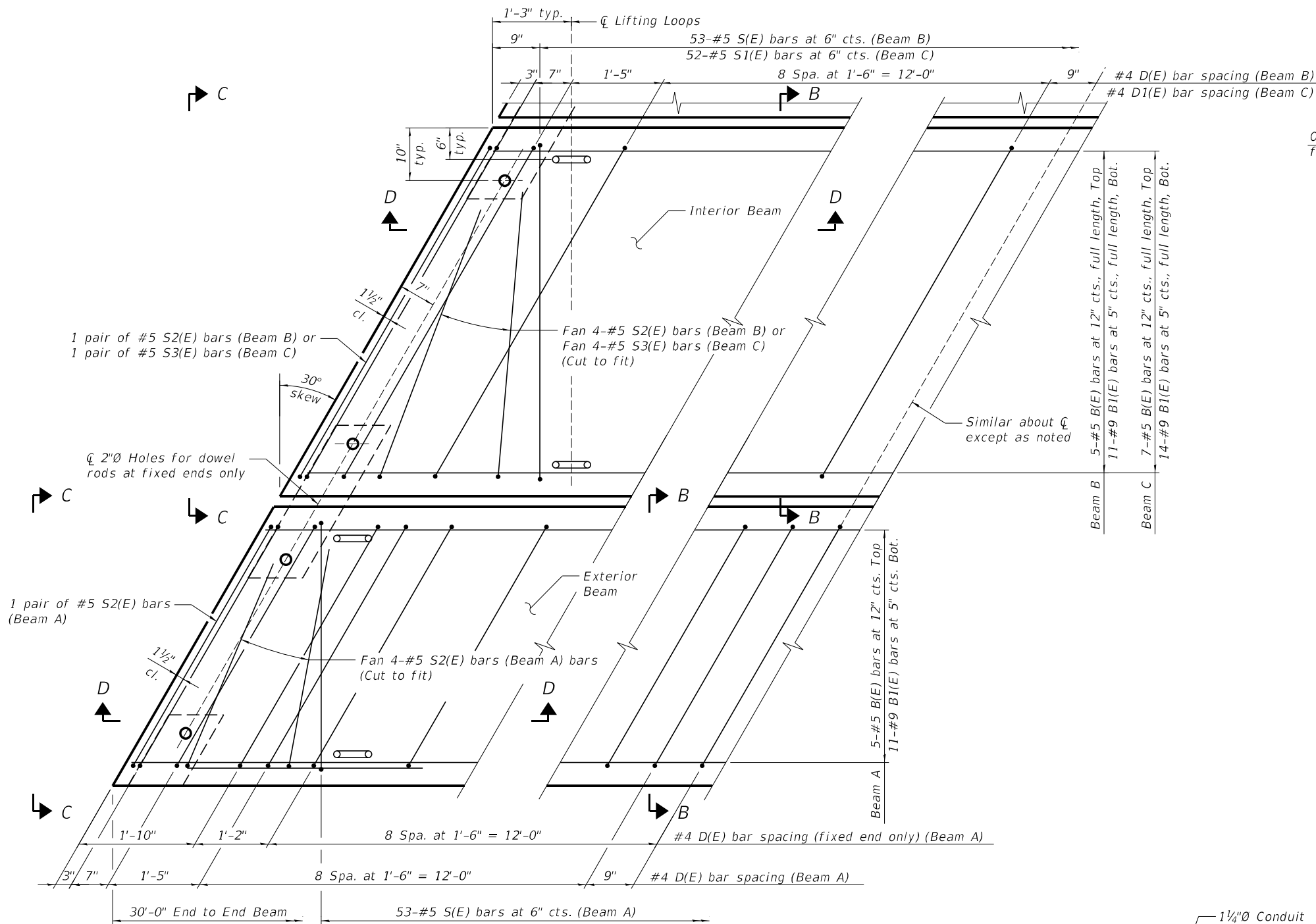
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PRECAST BRIDGE APPROACH SLAB DETAILS (2 OF 5)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 40 OF 71 SHEETS

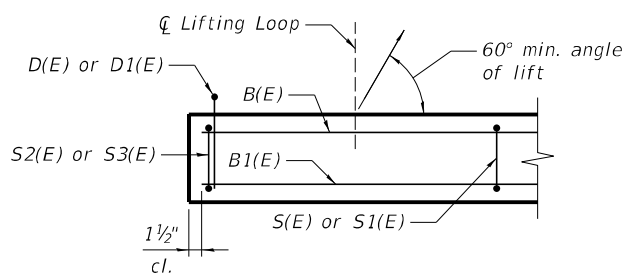
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	42
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

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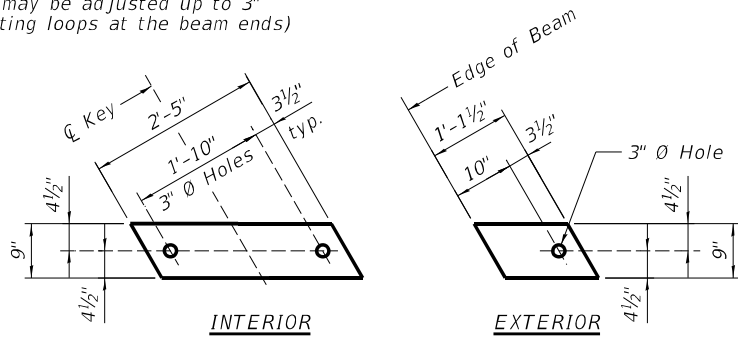


PLAN VIEW

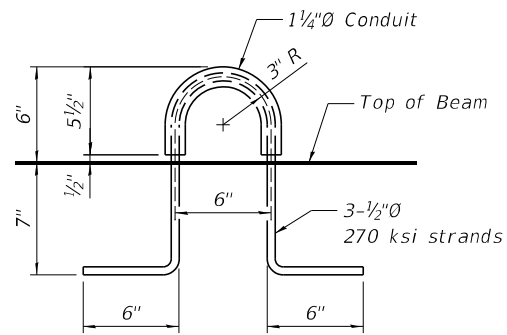
(showing precast bridge approach beams)
(Spacing of D(E) and D1(E) bars may be adjusted up to 3" to miss the dowel rod holes and lifting loops at the beam ends)



SECTION D-D

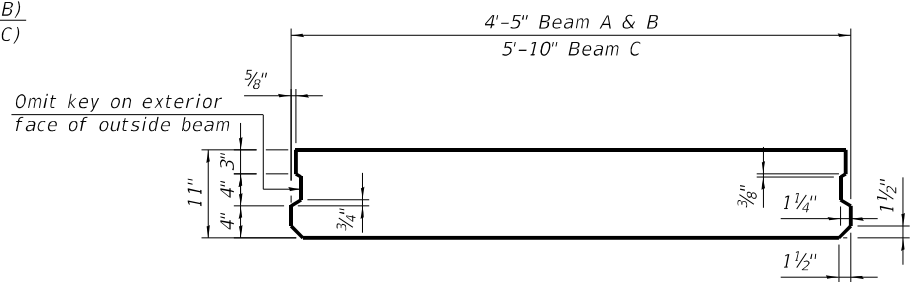


FABRIC BEARING PAD



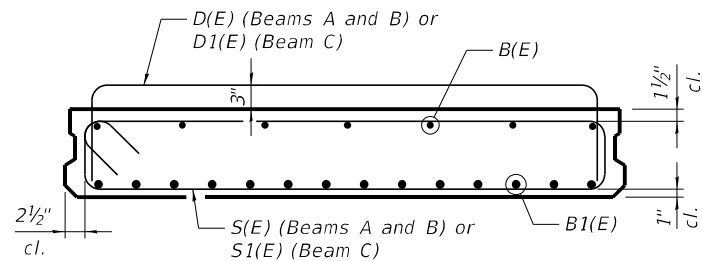
LIFTING LOOP DETAIL

(An alternate lifting loop with a proof load of 25,000 lbs. and utilized according to the manufacturer's recommendations may be used)



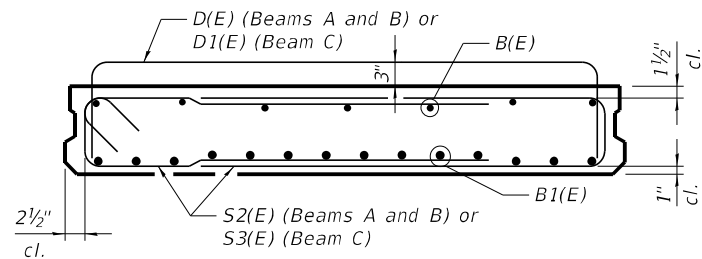
SECTION B-B

(Showing dimensions)



SECTION B-B

(Reinforcement for Beam C shown, Reinforcement for Beams A and B noted)



VIEW C-C

(Reinforcement for Beam C shown, Reinforcement for Beams A and B noted)

NOTES:

1. The precast bridge approach slab shall be according to Section 504 of the Standard Specifications and shall be paid for at the contract unit price per square foot for Precast Bridge Approach Slab.
2. Cast-in-place substitution of Precast Bridge Approach Slab is not allowed.
3. The top surface of precast bridge approach slabs shall be finished similar to precast prestressed deck beams with concrete wearing surface as specified in the IDOT "Manual for Fabrication of Precast Prestressed Concrete Products."
4. Two 1/8" fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location. Cost included with Precast Bridge Approach Slab.
5. A minimum 1/2" Ø lifting pins shall be used to engage the lifting loops during handling.
6. Compressive strength of precast concrete, f'c shall be 6,000 psi.
7. Compressive strength of precast concrete during initial lifting, f'ci shall be 5,000 psi.
8. Bearing pads at fixed ends shall be 1/2" thick and bearing pads at expansion end shall be 3/4" thick.
9. Omit holes for fabric bearing pads at approach slab footing end of beams.
10. See Sheet 43 of 71 for bar bends and bill of materials.

FOR INFORMATION ONLY



Alfred Benesch & Company
35 W Wacker Drive, Suite 3300
Chicago, Illinois 60601
312.465.4150 Job No. 10800

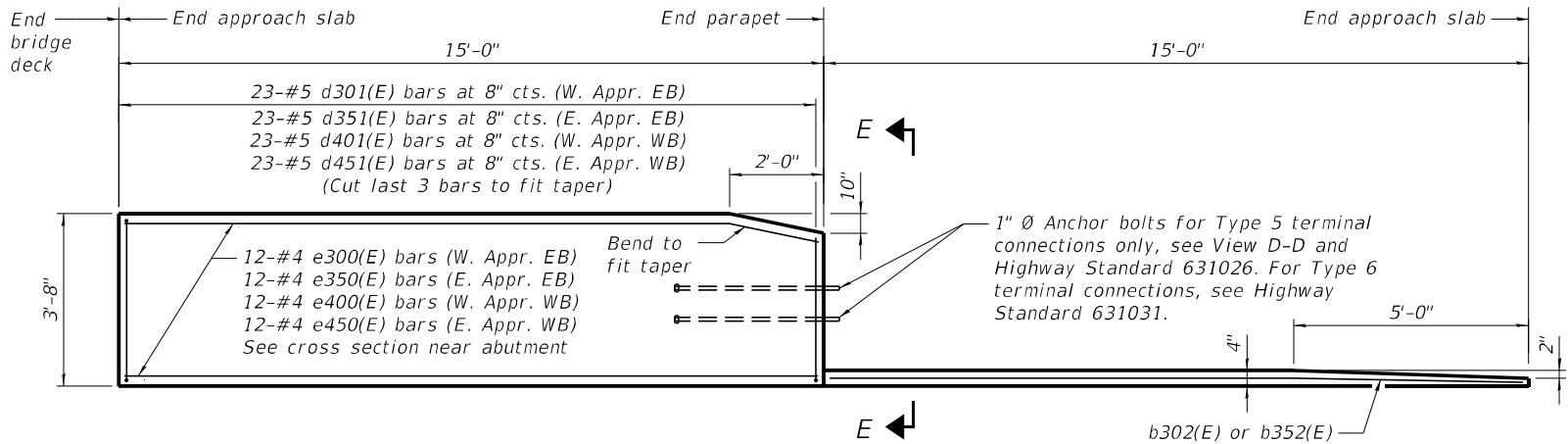
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PLOT DATE =	CHECKED - KMP	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

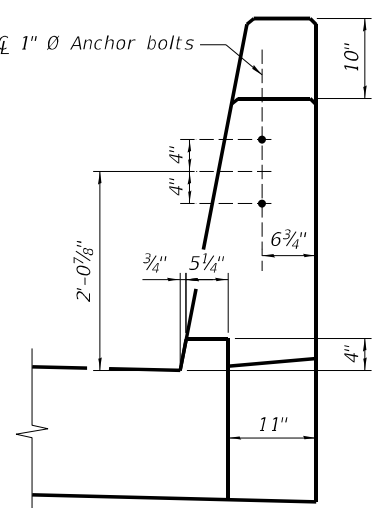
PRECAST BRIDGE APPROACH SLAB DETAILS (3 OF 5)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 41 OF 71 SHEETS

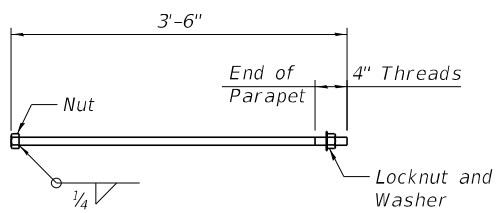
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	43
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				



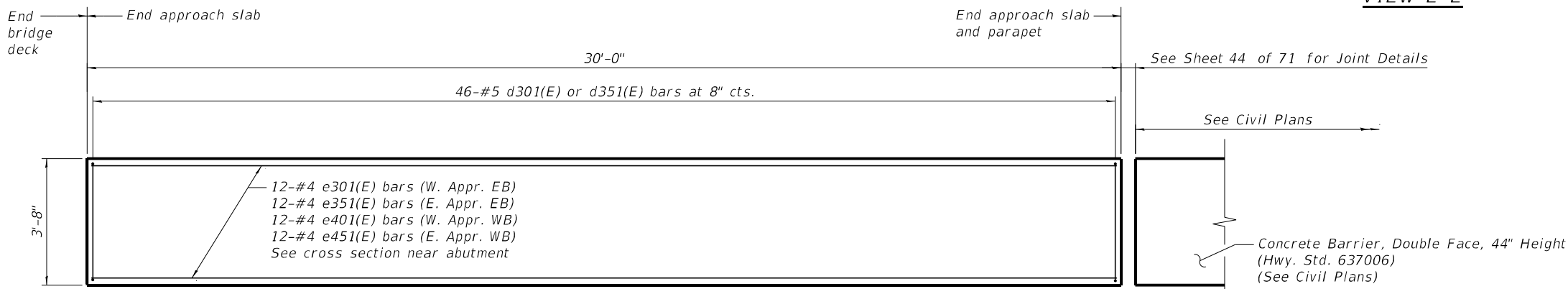
INSIDE ELEVATION OF PARAPET AND CURB
(Outside Parapet Elevation)



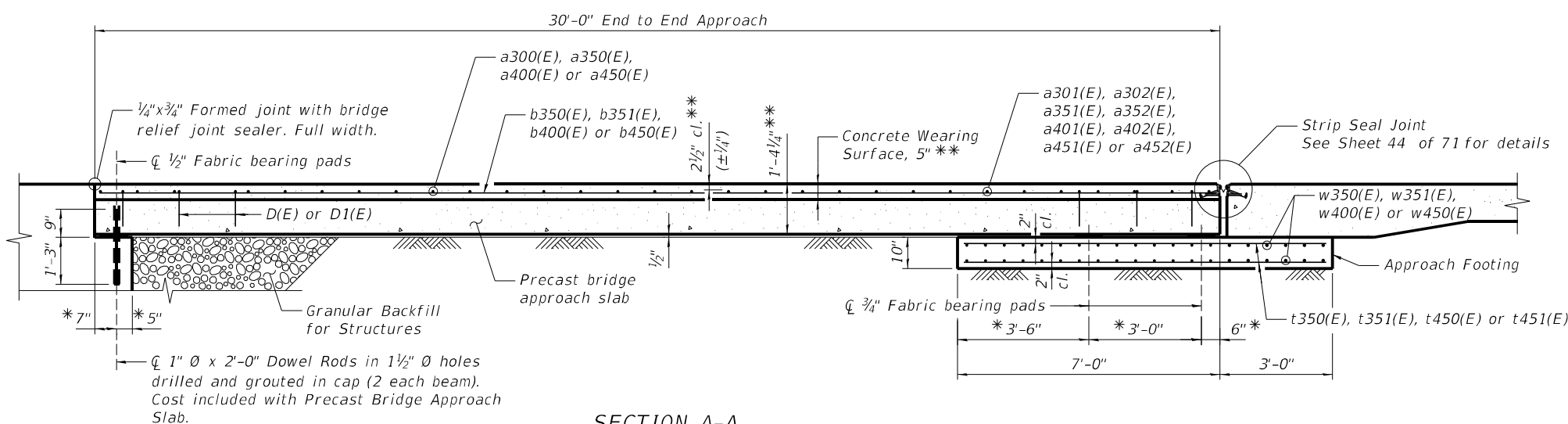
VIEW E-E



1" Ø ANCHOR BOLT
(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications. Cost of anchor bolt assemblies included with Concrete Superstructure)



INSIDE ELEVATION OF MEDIAN PARAPET
(Median Parapet Elevation)



SECTION A-A

* At right angles

** Prior to grinding

NOTES:

1. The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
2. After precast bridge approach slabs have been erected, holes shall be drilled into abutment and anchor dowels placed. Dowel holes shall be filled with non-shrink grout to top of precast slab and cured according to Article 1020.13(a)(3) or 1020.13(a)(5) of the Standard Specifications for a minimum of 24 hours before casting the shear keys and wearing surface.
3. Any concrete poured monolithically with the wearing surface, such as curbs, shall not be paid for separately, but will be included in the cost of Concrete Wearing Surface, 5".
4. The strip seal shall extend 6" beyond the edge of the approach slab on curb end.
5. Parapet concrete shall be paid for as Concrete Superstructure.
6. Approach footing concrete shall be paid for as Concrete Structures.
7. The approach footing maximum applied service bearing pressure (Q_{max}) = 2.0 ksf.
8. Cost of excavation for approach footing included with Concrete Structures.
9. For Granular Backfill for Structures and drainage treatment details, see sheet 3 of 71.
10. Cost of cellular polystyrene is included with Concrete Superstructure.
11. See Sheet 43 of 71 for bar bends and bill of materials.

FOR INFORMATION ONLY

MODEL: Default
FILE NAME: c:\pwwork\benesch projects\projects\dms65234\1010225_0226-sht-approdtls-004.dgn



Alfred Benesch & Company
35 W Wacker Drive, Suite 3300
Chicago, Illinois 60601
312.465.4050 Job No. 10880

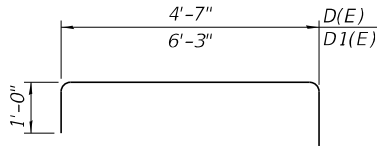
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

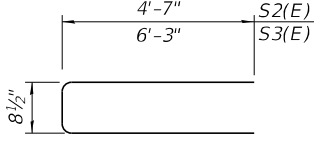
**PRECAST BRIDGE APPROACH SLAB DETAILS (4 OF 5)
STRUCTURE NO. 101-0225 & 101-0226**

SHEET 42 OF 71 SHEETS

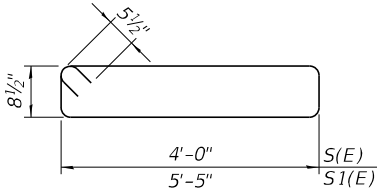
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CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



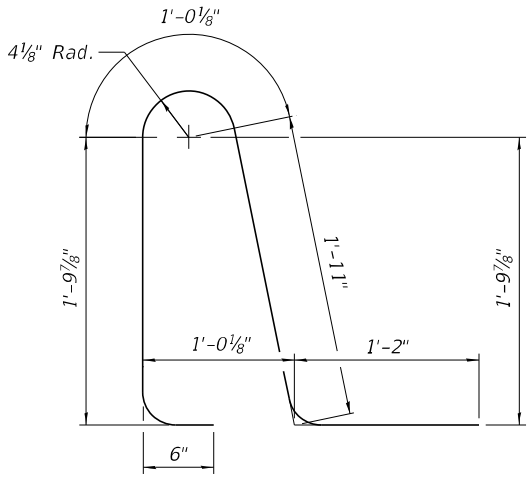
BARS D(E) & D1(E)



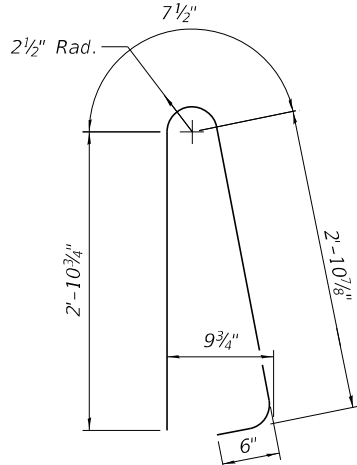
BARS S2(E) & S3(E)



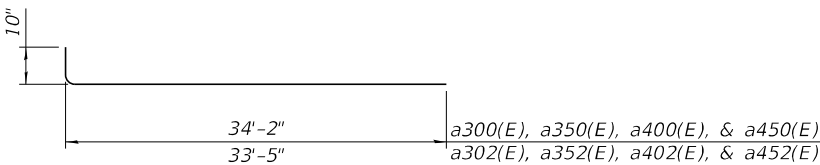
BARS S(E) & S1(E)



BARS d300(E), d350(E),
d400(E) & d450(E)



BARS d301(E), d351(E),
d401(E) & d451(E)



BARS a300(E), a302(E), a350(E),
a352(E), a400(E), a402(E),
a450(E) & a452(E)

BAR LIST
EACH BEAM A

(For information only)

Bar	No.	Size	Length	Shape
B(E)	5	#5	29'-8"	—
B1(E)	11	#9	29'-8"	—
D(E)	32	#4	6'-7"	┐
S(E)	53	#5	10'-4"	┐
S2(E)	12	#5	9'-11"	┐

BILL OF MATERIAL
EAST APPROACH
EB (SN 101-0225)

Bar	No.	Size	Length	Shape
a300(E)	28	#5	35'-0"	┐
a301(E)	14	#4	33'-10"	┐
a302(E)	14	#5	34'-6"	┐
a303(E)	42	#5	8'-2"	┐
b300(E)	56	#4	29'-8"	—
b301(E)	4	#5	14'-8"	—
b302(E)	1	#4	14'-8"	—
b303(E)	4	#4	29'-8"	—
d300(E)	69	#5	6'-5"	┐
d301(E)	69	#5	7'-0"	┐
e300(E)	12	#4	14'-8"	—
e301(E)	12	#4	29'-8"	—
t300(E)	114	#4	11'-2"	—
w300(E)	80	#5	33'-10"	—

BAR LIST
EACH BEAM B

(For information only)

Bar	No.	Size	Length	Shape
B(E)	5	#5	29'-8"	—
B1(E)	11	#9	29'-8"	—
D(E)	22	#4	6'-7"	┐
S(E)	53	#5	10'-4"	┐
S2(E)	12	#5	9'-11"	┐

BILL OF MATERIAL
WEST APPROACH
WB (SN 101-0226)

Bar	No.	Size	Length	Shape
a400(E)	28	#5	35'-0"	┐
a401(E)	14	#4	33'-10"	┐
a402(E)	14	#5	34'-6"	┐
a403(E)	42	#5	8'-2"	┐
b400(E)	56	#4	29'-8"	—
b401(E)	4	#5	14'-8"	—
b402(E)	1	#4	14'-8"	—
b403(E)	4	#4	29'-8"	—
d400(E)	69	#5	6'-5"	┐
d401(E)	69	#5	7'-0"	┐
e400(E)	12	#4	14'-8"	—
e401(E)	12	#4	29'-8"	—
t400(E)	114	#4	11'-2"	—
w400(E)	80	#5	33'-10"	—

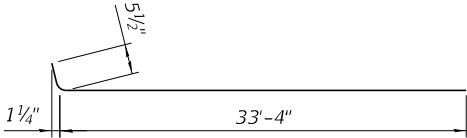
BAR LIST
EACH BEAM C

(For information only)

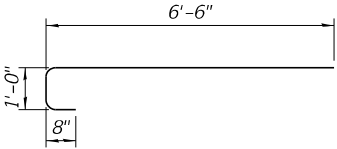
Bar	No.	Size	Length	Shape
B(E)	7	#5	29'-8"	—
B1(E)	14	#9	29'-8"	—
D1(E)	22	#4	8'-3"	┐
S1(E)	52	#5	13'-2"	┐
S3(E)	12	#5	13'-3"	┐

BILL OF MATERIAL
EAST APPROACH
WB (SN 101-0226)

Bar	No.	Size	Length	Shape
a450(E)	28	#5	35'-0"	┐
a451(E)	14	#4	33'-10"	┐
a452(E)	14	#5	34'-6"	┐
a453(E)	42	#5	8'-2"	┐
b450(E)	56	#4	29'-8"	—
b451(E)	4	#5	14'-8"	—
b452(E)	1	#4	14'-8"	—
b453(E)	4	#4	29'-8"	—
d450(E)	69	#5	6'-5"	┐
d451(E)	69	#5	7'-0"	┐
e450(E)	12	#4	14'-8"	—
e451(E)	12	#4	29'-8"	—
t450(E)	114	#4	11'-2"	—
w450(E)	80	#5	33'-10"	—



BARS a301(E), a351(E),
a401(E), & a451(E)



BARS a303(E), a353(E),
a403(E), & a453(E)

NOTE:
Bars indicated thus 1 x 2-#5 etc. indicates
1 line of bars with 2 lengths per line.

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MODEL: Default
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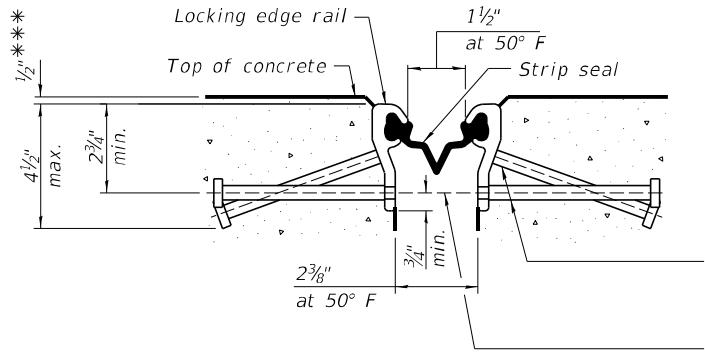
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PRECAST BRIDGE APPROACH SLAB DETAILS (5 OF 5)
STRUCTURE NO. 101-0225 & 101-0226

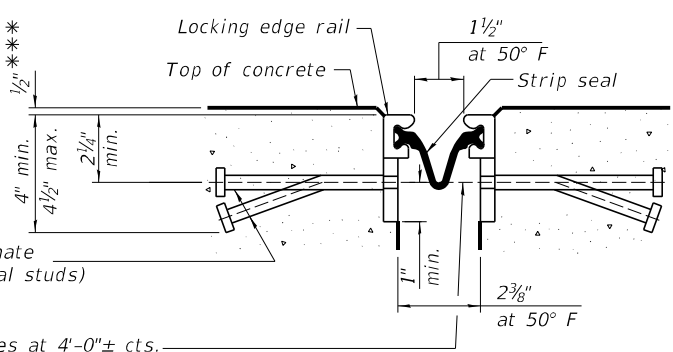
SHEET 43 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

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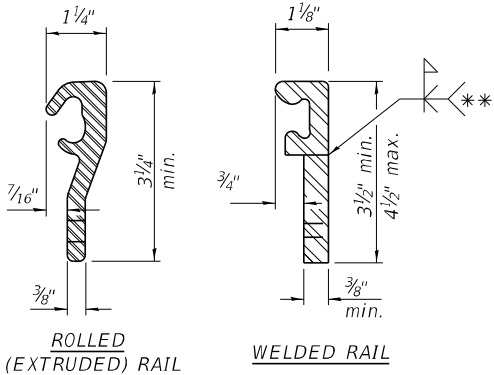
SHOWING ROLLED RAIL JOINT



SHOWING WELDED RAIL JOINT

DETAILS

- * Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.
- *** Prior to grinding.

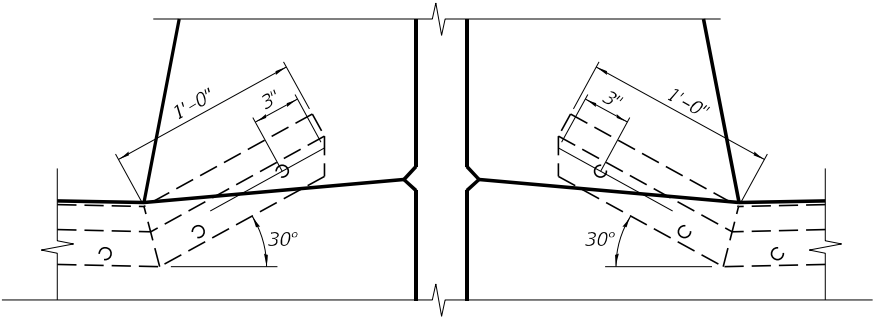


LOCKING EDGE RAILS

- ** Back gouge not required if complete joint penetration is verified by mock-up.

LOCKING EDGE RAIL SPLICE

- The inside of the locking edge rail groove shall be free of weld residue.
- Rolled rail shown, welded rail similar.



STRIP SEAL END DETAIL AT MEDIAN PARAPET

Notes:

The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4 1/2" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be 3/16" and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of anchorage studs included with Preformed Joint Strip Seal.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

FOR INFORMATION ONLY

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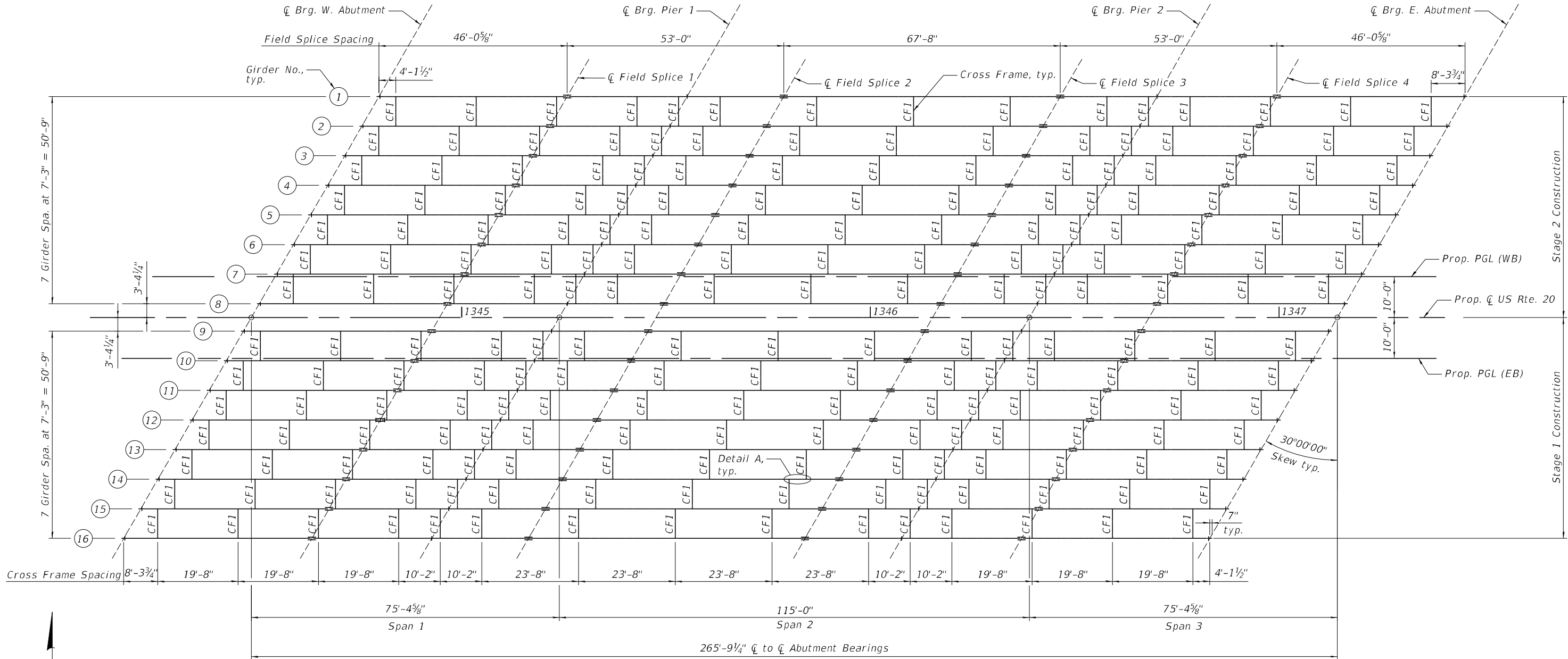
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PREFORMED JOINT STRIP SEAL
STRUCTURE NO. 101-0225 & 101-0226

SHEET 44 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	46
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

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FRAMING PLAN

NOTES:

- All cross frames shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames at supports may be temporarily disconnected to install bearing anchor bolts.
- Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
- See Sheet 46 of 71 for beam elevation.
- See Sheet 50 of 71 for steel cross frame, girder details, and Detail A.
- See Sheet 49 of 71 for field splice details.



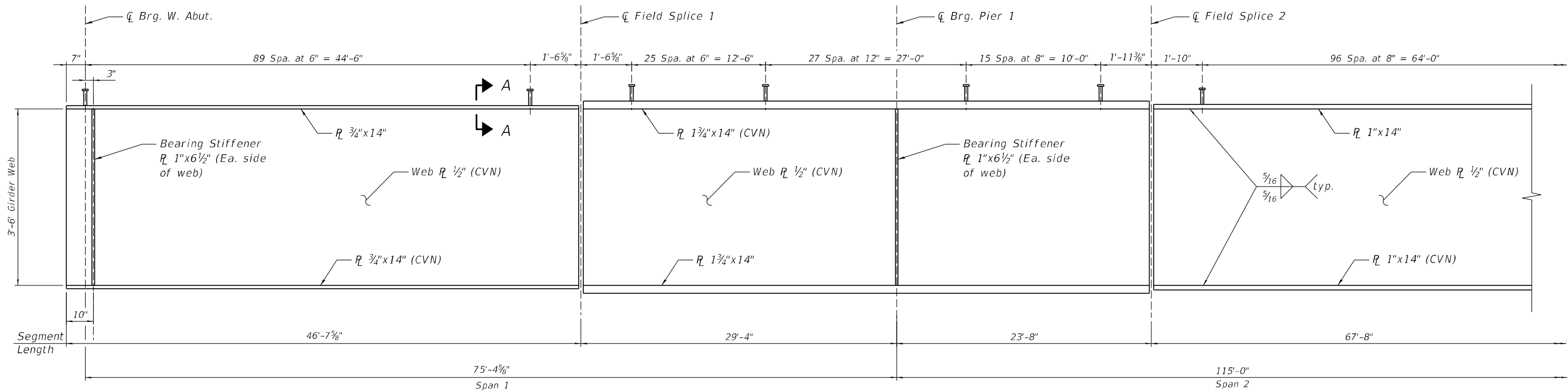
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PLOT DATE	=	CHECKED - JHG	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

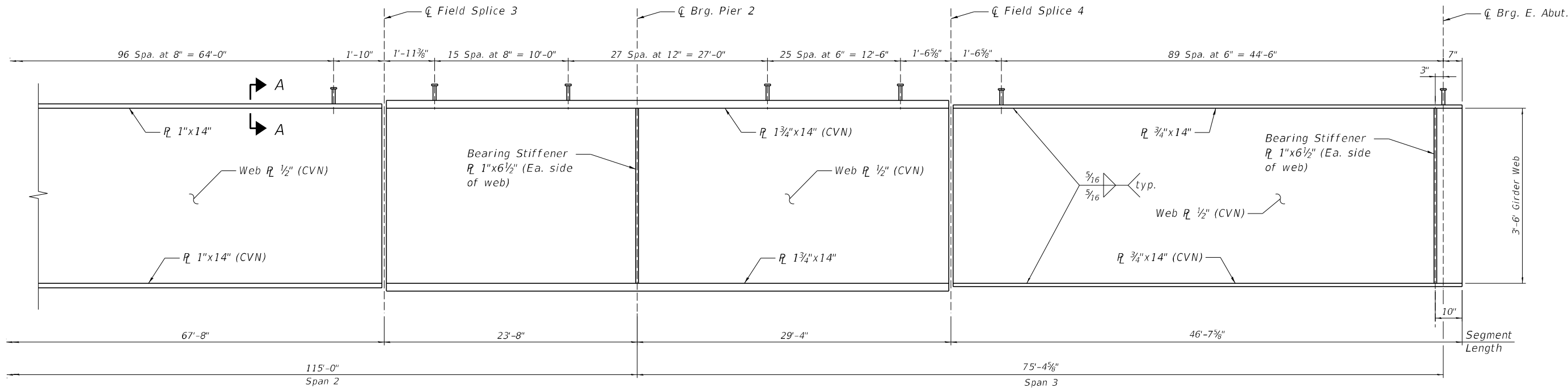
FRAMING PLAN
STRUCTURE NO. 101-0225 & 101-0226

SHEET 45 OF 71 SHEETS

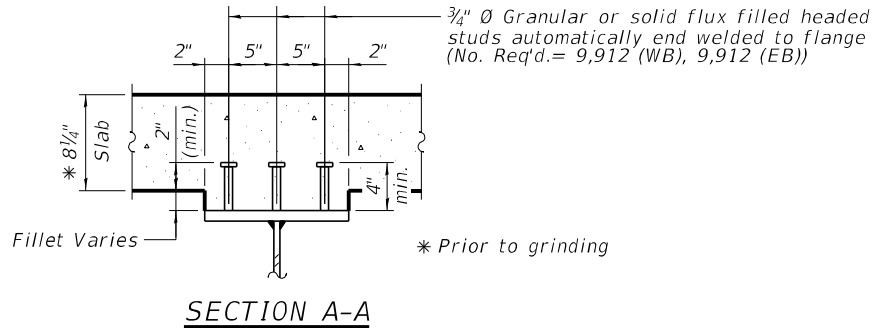
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	47
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



GIRDER ELEVATION
(Cross Frame Connection Plates not shown for clarity)



GIRDER ELEVATION
(Cross Frame Connection Plates not shown for clarity)



- NOTES:**
1. Structural Steel for girder plates, connection plates, and bearing stiffeners shall be AASHTO M270, Grade 50.
 2. Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
 3. See Sheet 45 of 71 for framing plan.
 4. See Sheet 50 of 71 for cross frame and girder details.
 5. See Sheet 49 of 71 for field splice details.

MODEL: Default
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<div></div> <div>Alfred Benesch & Company 35 W Wacker Drive, Suite 3300 Chicago, Illinois 60601 312.465.4150 Job No. 10800</div>	USER NAME =		DESIGNED - JPM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GIRDER ELEVATION STRUCTURE NO. 101-0225 & 101-0226		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			SHEET 46 OF 71 SHEETS									

MODEL: Default
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INTERIOR GIRDER MOMENT TABLE			
	0.4 Sp. 1/ 0.6 Sp. 3	Pier 1/ Pier 2	0.5 Sp. 2
Is	(in ⁴) 12,683	26,547	16,032
Ic(n)	(in ⁴) 32,295	-----	38,138
Ic(3n)	(in ⁴) 24,601	-----	28,948
Ic(cr)	(in ⁴) -----	32,193	-----
Ss	(in ³) 583	1,167	729
Sc(n)	(in ³) 819	-----	979
Sc(3n)	(in ³) 754	-----	904
Sc(cr)	(in ³) -----	1,260	-----
Sx	(in ³) 788	1,234	914
DC1	(k/')	0.908	1.006
MDC1	('k)	228	1,074
DC2	(k/')	0.143	0.143
MDC2	('k)	37	154
DW	(k/')	0.341	0.341
MDW	('k)	87	366
LLDF		0.618	0.594
M _ℓ + I _M	('k)	939	1,342
fℓ (Strength I)	(ksi)	0.0	0.0
Mu + 1/3 fℓ S _{xc}	('k)	2,105	4,431
Øf Mn	('k)	4,267	-----
fs DC1	(ksi)	4.7	11.0
fs DC2	(ksi)	0.6	1.5
fs DW	(ksi)	1.4	3.5
fs (ℓ+IM)	(ksi)	13.8	12.8
fℓ (Service II)	(ksi)	0.0	0.0
fs+ ^{fℓ} /2 (Service II)	(ksi)	24.5	32.6
Service II Resistance(ksi)		47.5	47.5
fs + ^{fℓ} /3 (Strength I)	(ksi)	32.8	43.2
Øf Fn	(ksi)	-----	50.0
Vf	(k)	40.1	63.3

INTERIOR GIRDER REACTION TABLE		
	W. Abut./ E. Abut.	Pier 1/ Pier 2
LLDF	0.850	0.850
OCF	1.115	1.115
RDC1	(k) 22.2	109.2
RDC2	(k) 3.4	15.7
RDW	(k) 8.0	37.3
R _ℓ	(k) 70.4	140.0
R _{IM}	(k) 17.0	27.6
RTotal (Strength I) (Impact)	(k) 197.1	505.3
RTotal (Strength I) (No Impact)	(k) 167.3	457.1

EXTERIOR GIRDER MOMENT TABLE			
	0.4 Sp. 1/ 0.6 Sp. 3	Pier 1/ Pier 2	0.5 Sp. 2
Is	(in ⁴) 12,683	26,547	16,032
Ic(n)	(in ⁴) 32,008	-----	37,783
Ic(3n)	(in ⁴) 24,295	-----	28,598
Ic(cr)	(in ⁴) -----	31,841	-----
Ss	(in ³) 583	1,167	729
Sc(n)	(in ³) 817	-----	976
Sc(3n)	(in ³) 751	-----	901
Sc(cr)	(in ³) -----	1,255	-----
Sx	(in ³) 785	1,230	910
DC1	(k/')	0.876	0.974
MDC1	('k)	236	1,111
DC2	(k/')	0.143	0.143
MDC2	('k)	37	154
DW	(k/')	0.341	0.341
MDW	('k)	87	366
LLDF		0.618	0.594
M _ℓ + I _M	('k)	939	1,341
fℓ (Strength I)	(ksi)	0.0	0.0
Mu + 1/3 fℓ S _{xc}	('k)	2,115	4,477
Øf Mn	('k)	4,254	-----
fs DC1	(ksi)	4.9	11.4
fs DC2	(ksi)	0.6	1.5
fs DW	(ksi)	1.4	3.5
fs (ℓ+IM)	(ksi)	13.8	12.8
fℓ (Service II)	(ksi)	0.0	0.0
fs+ ^{fℓ} /2 (Service II)	(ksi)	24.8	33.1
Service II Resistance(ksi)		47.5	47.5
fs + ^{fℓ} /3 (Strength I)	(ksi)	33.0	43.8
Øf Fn	(ksi)	-----	50.0
Vf	(k)	37.9	63.5

EXTERIOR GIRDER REACTION TABLE		
	W. Abut./ E. Abut.	Pier 1/ Pier 2
LLDF	0.671	0.671
OCF	1.115	1.115
RDC1	(k) 23.0	112.9
RDC2	(k) 3.4	15.7
RDW	(k) 8.0	37.3
R _ℓ	(k) 55.6	110.5
R _{IM}	(k) 13.5	21.8
RTotal (Strength I) (Impact)	(k) 165.8	448.2
RTotal (Strength I) (No Impact)	(k) 142.3	410.1

OCF: Obtuse Correction Factor according to Article 4.6.2.2.3c or as further simplified by IDOT provisions.

R_{DC1}: Un-factored reaction due to non-composite dead load (kip).

R_{DC2}: Un-factored reaction due to long-term composite (superimposed excluding future wearing surface) dead load (kip).

R_{DW}: Un-factored reaction due to long-term composite (superimposed future wearing surface only) dead load (kip).

R_ℓ: Un-factored live load reaction (kip).

R_{IM}: Un-factored dynamic load allowance (impact) (kip).

R_{TOTAL} (Strength I)(Impact): Strength I load combination of factored design reactions (kip).
1.25 (R_{DC1} + R_{DC2}) + 1.5 R_{DW} + 1.75 (R_ℓ + R_{IM})

R_{TOTAL} (Strength I)(No Impact): Strength I load combination of factored design reactions, not including dynamic load allowance (Impact) (kip).
1.25 (R_{DC1} + R_{DC2}) + 1.5 R_{DW} + 1.75 (R_ℓ)

Is, Ss: Non-composite moment of inertia and section modulus of the steel section used for computing fs (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).

Ic(n), Sc(n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing fs (Total-Strength I, and Service II) in uncracked sections due to short term composite live loads (in.⁴ and in.³).

Ic(3n), Sc(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing fs (Total-Strength I, and Service II) in uncracked sections due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

Ic(cr), Sc(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing fs (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.⁴ and in.³).

Sx: Section modulus about the major axis of section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.³).

DC1: Un-factored non-composite dead load (kips/ft.).

MDC1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

LLDF: Live Load Distribution Factor for moment and shear computed according to Article 4.6.2.2 and other IDOT provisions.

M_ℓ + I_M: Un-factored live load moment plus dynamic load allowance (impact)(kip-ft.).

Mu: Strength I load combination of factored design moments (kip-ft.).
1.25 (MDC1+ MDC2) + 1.5 MDW + 1.75 M_ℓ + I_M

fℓ: Factored calculated flange lateral bending stress as calculated using Article 6.10.1.6 and as further simplified by IDOT provisions (ksi).

Øf Mn: Factored nominal flexural resistance of the section determined as specified in Article 6.10.7.1 or A6 as applicable (kip-ft)

fs DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
MDC1 / Ss

fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
MDC2 / Sc(3n) or MDC2 / Sc(cr) as applicable.

fs DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
MDW / Sc(3n) or MDW / Sc(cr) as applicable.

fs (ℓ+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live plus impact loads as calculated below (ksi).
M_ℓ + I_M / Sc(n) or M_ℓ + I_M / Sc(cr) as applicable.

fs + ^{fℓ}/2 (Service II): Sum of stresses as computed below (ksi).
fsDC1 + fsDC2 + fsDW + 1.3 fs(ℓ+ IM) + ^{fℓ}/2

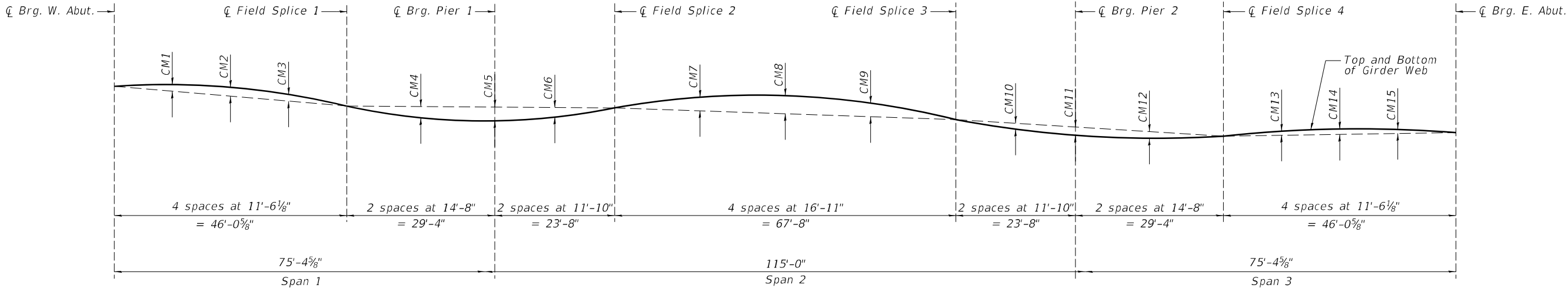
Service II Resistance: Composite (0.95RhFyf) or noncomposite (0.80RhFyf) stress capacity according to Article 6.10.4.2 (ksi).

fs + ^{fℓ}/3 (Strength I): Sum of stresses as computed below on non-compact section (ksi).
1.25 (fsDC1 + fsDC2) + 1.5 fsDW + 1.75 fs(ℓ + IM) + ^{fℓ}/3

Øf Fn: Factored nominal flexural resistance of the section as specified in Article 6.10.7.2 or 6.10.8 as applicable (ksi).

Vf: Maximum factored shear range in span computed according to Article 6.10.10.

5/21/2025 8:30:35 AM



CAMBER DIAGRAM

CAMBER DIMENSIONS

Girders	CM1	CM2	CM3	CM4	CM5	CM6	CM7	CM8	CM9	CM10	CM11	CM12	CM13	CM14	CM15
1 thru 16	$\frac{9}{16}$ "	$\frac{3}{4}$ "	$\frac{9}{16}$ "	$\frac{3}{8}$ "	$\frac{3}{4}$ "	$\frac{1}{2}$ "	$1\frac{1}{8}$ "	$1\frac{1}{2}$ "	$1\frac{1}{8}$ "	$\frac{1}{2}$ "	$\frac{3}{4}$ "	$\frac{3}{8}$ "	$\frac{9}{16}$ "	$\frac{3}{4}$ "	$\frac{9}{16}$ "

TOP OF WEB ELEVATIONS*

Location	Girder 1	Girder 2	Girder 3	Girder 4	Girder 5	Girder 6	Girder 7	Girder 8
⊘ Brg. W. Abut.	738.12	738.31	738.50	738.68	738.83	738.96	739.07	738.96
⊘ Field Splice 1	737.97	738.16	738.34	738.52	738.67	738.80	738.92	738.80
⊘ Brg. Pier 1	737.97	738.15	738.34	738.52	738.67	738.80	738.91	738.80
⊘ Field Splice 2	737.97	738.15	738.34	738.52	738.67	738.80	738.91	738.80
⊘ Field Splice 3	737.87	738.06	738.24	738.42	738.57	738.70	738.82	738.70
⊘ Brg. Pier 2	737.81	737.99	738.18	738.36	738.51	738.64	738.75	738.64
⊘ Field Splice 4	737.73	737.91	738.10	738.28	738.43	738.56	738.68	738.56
⊘ Brg. E. Abut.	737.76	737.95	738.13	738.31	738.46	738.59	738.71	738.59

Location	Girder 9	Girder 10	Girder 11	Girder 12	Girder 13	Girder 14	Girder 15	Girder 16
⊘ Brg. W. Abut.	738.96	739.09	738.99	738.87	738.73	738.56	738.39	738.21
⊘ Field Splice 1	738.81	738.93	738.83	738.71	738.57	738.41	738.23	738.06
⊘ Brg. Pier 1	738.80	738.93	738.83	738.71	738.57	738.40	738.23	738.05
⊘ Field Splice 2	738.80	738.93	738.82	738.71	738.57	738.40	738.22	738.05
⊘ Field Splice 3	738.71	738.84	738.73	738.61	738.47	738.31	738.13	737.96
⊘ Brg. Pier 2	738.64	738.77	738.67	738.55	738.41	738.24	738.07	737.89
⊘ Field Splice 4	738.56	738.69	738.59	738.47	738.33	738.16	737.99	737.81
⊘ Brg. E. Abut.	738.59	738.72	738.62	738.50	738.36	738.20	738.02	737.84

*For fabrication only.

NOTE:

- See Sheet 46 of 71 for girder segment lengths.

MODEL: Default
FILE NAME: c:\pwworkin\benesch_projects\projects\dms65234\1010225_0226-shl-camber.dgn



Alfred Benesch & Company
35 W Wacker Drive, Suite 3300
Chicago, Illinois 60601
312.465.4150 Job No. 10800

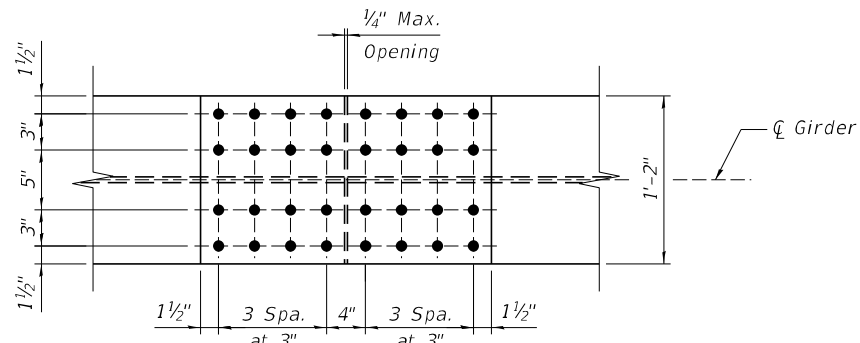
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		CHECKED - JHG	REVISED -
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PLOT DATE	=	CHECKED - JHG	REVISED -

STATE OF ILLINOIS
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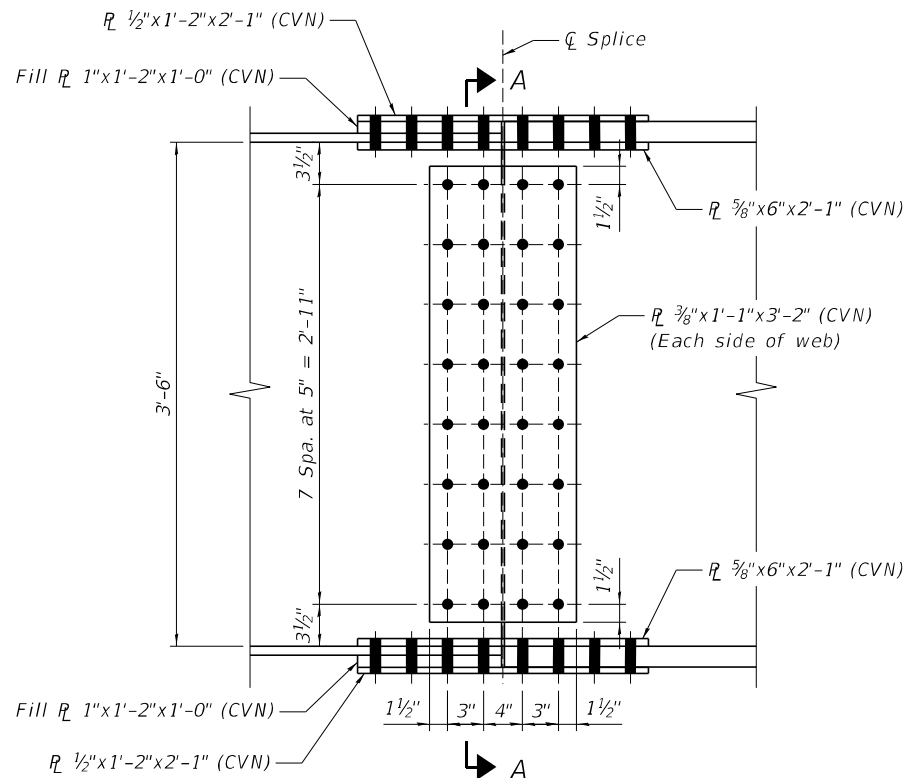
GIRDER TOP OF WEB ELEVATIONS AND CAMBER
STRUCTURE NO. 101-0225 & 101-0226

SHEET 48 OF 71 SHEETS

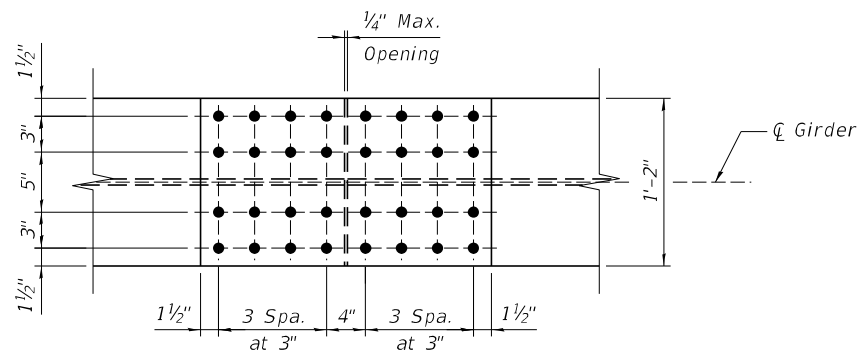
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	50
CONTRACT NO. 64U98				
		ILLINOIS	FED. AID PROJECT	



TOP FLANGE SPLICE



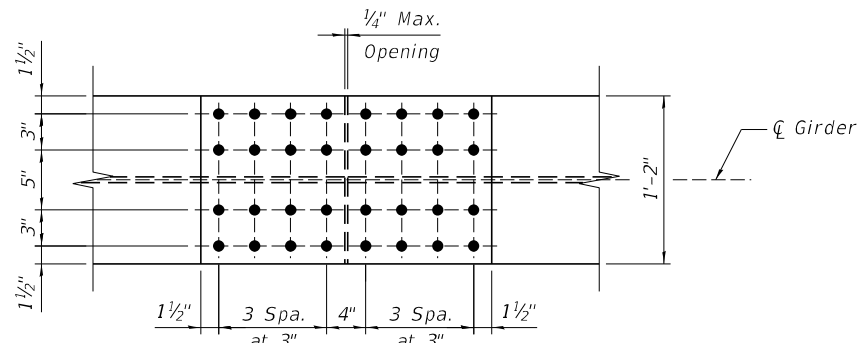
WEB SPLICE - ELEVATION



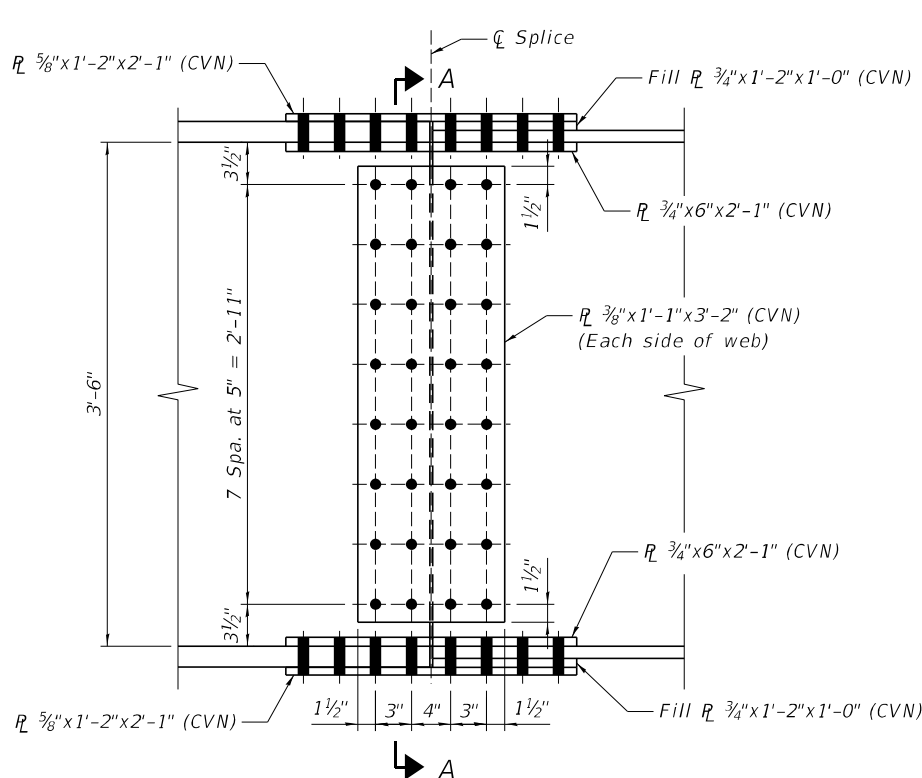
BOTTOM FLANGE SPLICE

FIELD SPLICE 1 & 4

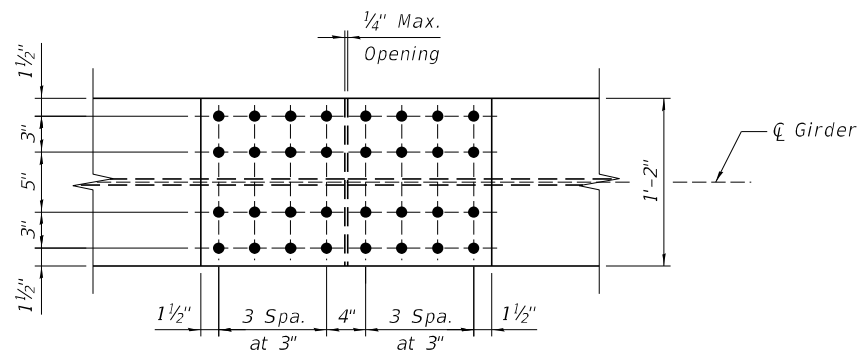
(No. Req'd = 16 (WB), 16 (EB))



TOP FLANGE SPLICE



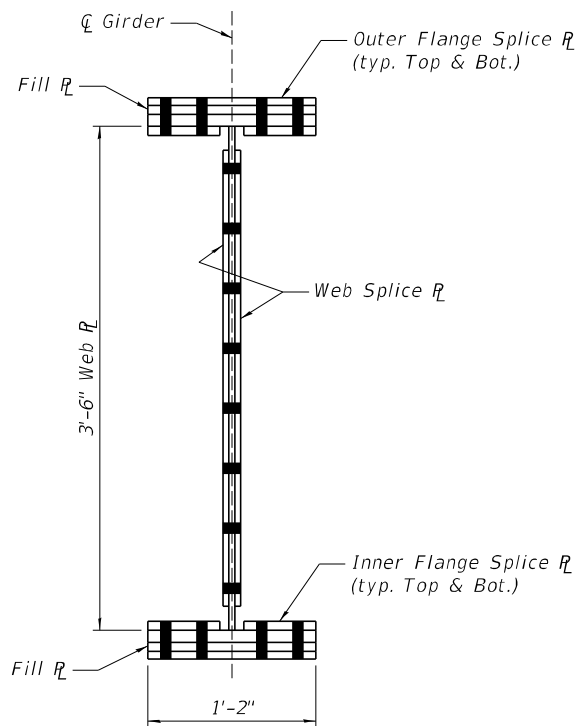
WEB SPLICE - ELEVATION



BOTTOM FLANGE SPLICE

FIELD SPLICE 2 & 3

(No. Req'd = 16 (WB), 16 (EB))



SECTION A-A

NOTES:

- All splice plates shall be AASHTO M270 Grade 50.
- Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
- Fasteners shall be ASTM F3125 Grade A325 Type 1, hot dipped galvanized bolts. Bolts 7/8" diameter, holes 15/16" diameter. See Special Provision for "Metallizing of Structural Steel".

MODEL: Default
FILE NAME: c:\pwword\benesch projects\projects\dms65234\1010225_0226-shl-splice.dgn



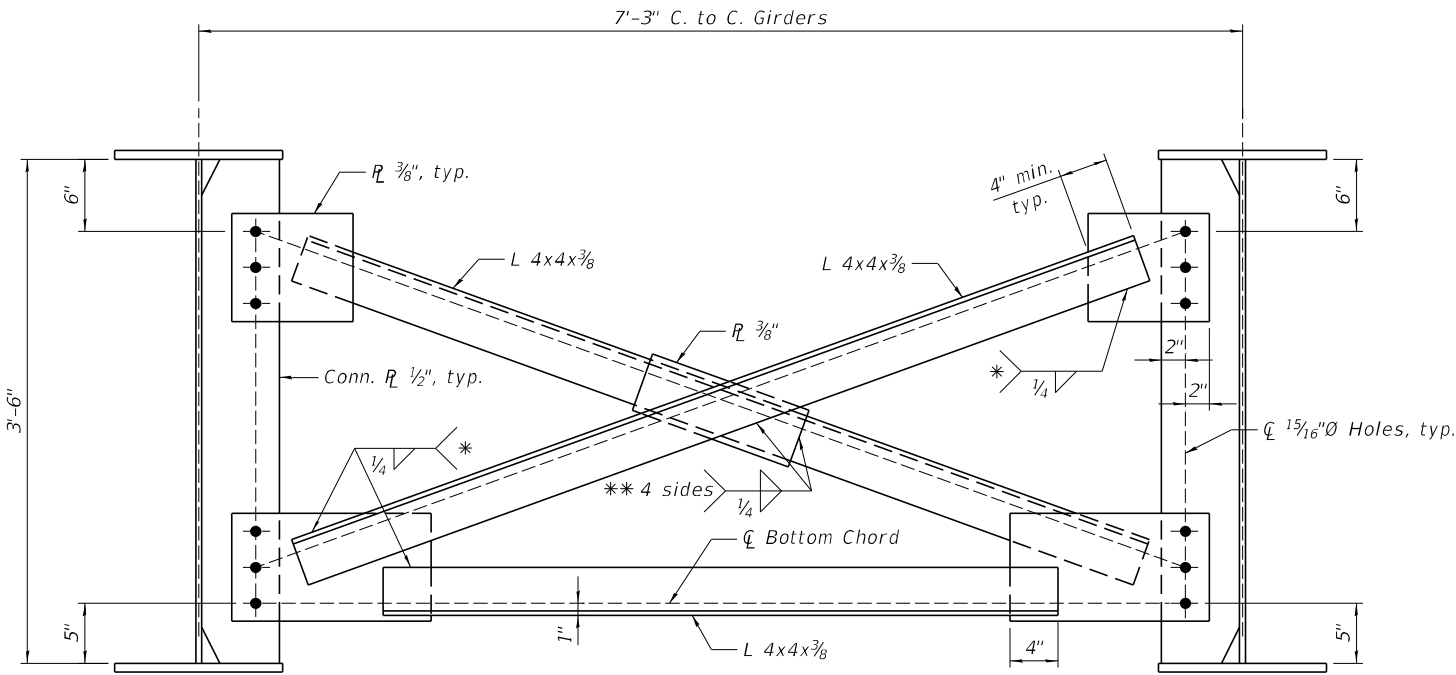
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PLOT SCALE =	CHECKED - JHG	REVISED -
PLOT DATE =	DRAWN - RMG	REVISED -
	CHECKED - JHG	REVISED -

STATE OF ILLINOIS
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FIELD SPLICE DETAILS
STRUCTURE NO. 101-0225 & 101-0226

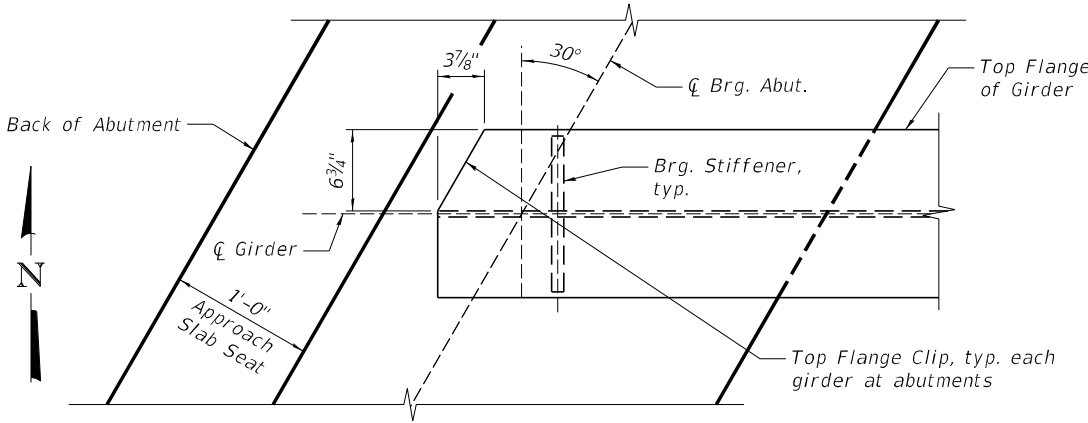
SHEET 49 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	51
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

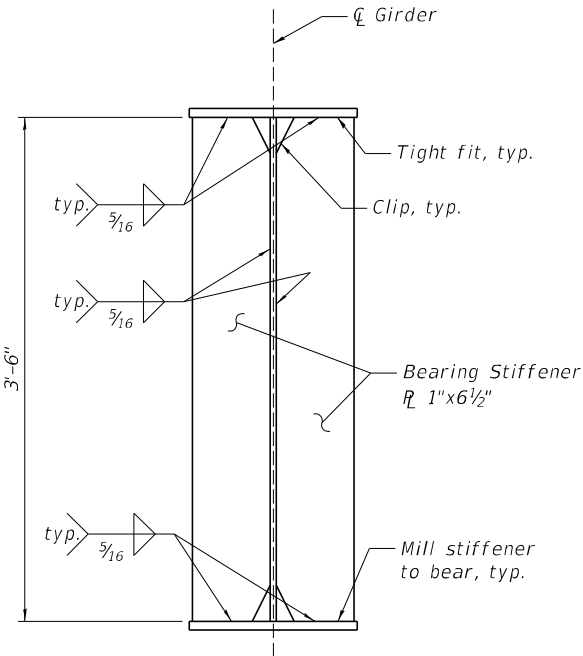


CROSS FRAME DETAIL - CF1
(No. Req'd. = 105 (WB), 105 (EB))

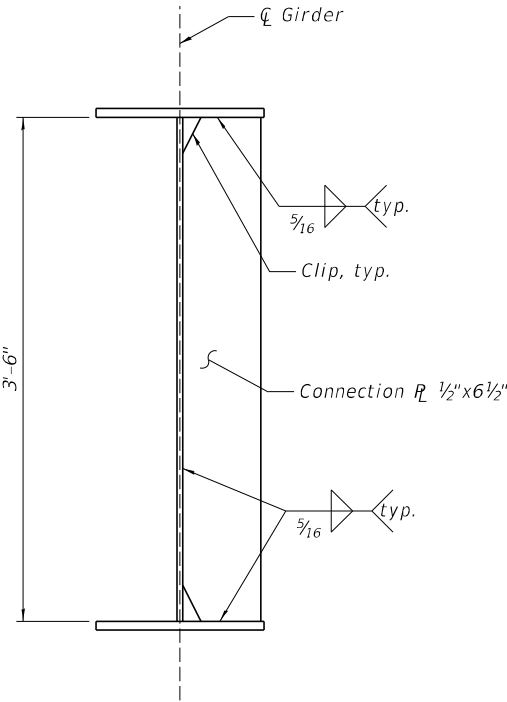
* Fillet weld angles along 3 sides on one face of gusset plate; however, if cross-frames are galvanized, weld all-around.
** If cross-frames are galvanized, weld all-around.



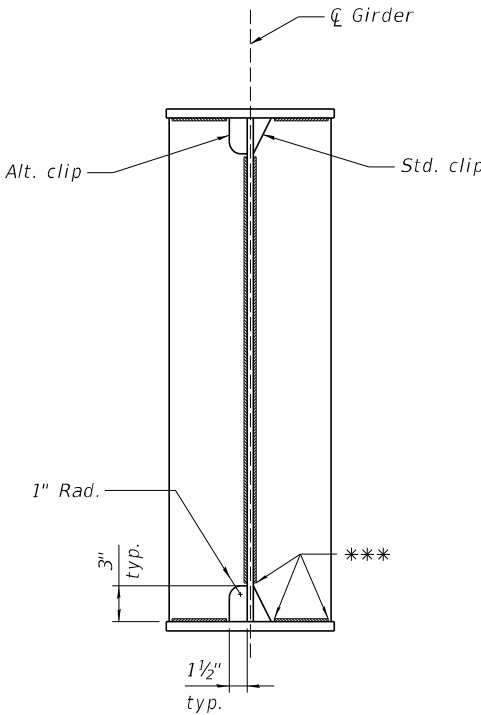
TOP FLANGE PLAN - CLIPPED
(W. Abut. shown, E. Abut. opposite hand)



BEARING STIFFENER DETAIL

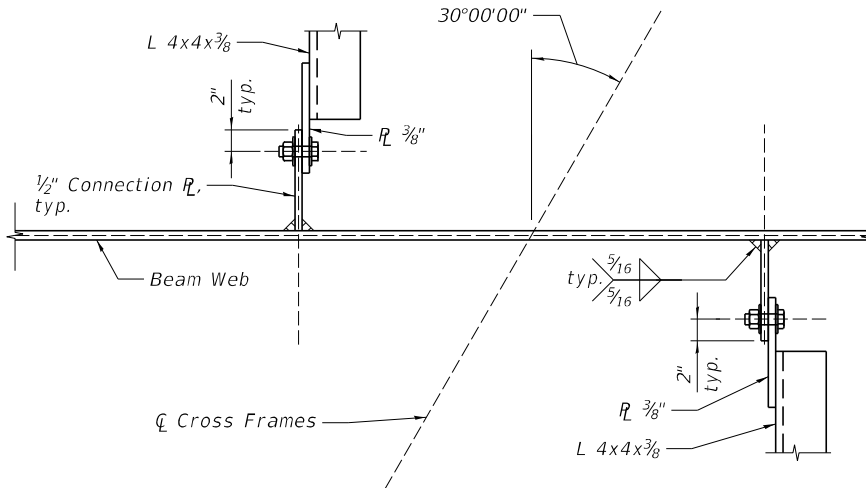


CONNECTION PLATE DETAIL
(No. Req'd. = 210 (WB), 210 (EB))



WELD LIMITS AND CLIP DETAILS

*** Stop welds 1/4" (±1/8") from edges as shown, typ.



DETAIL A
(Bearing stiffener not shown for Pier locations)

NOTES:
1. Two hardened washers required for each set of oversized holes and slotted holes.
2. See Sheet 45 of 71 for location of cross frames.

MODEL: Default
FILE NAME: c:\pwwork\benesch_projects\projects\dms65234\1010225_0226-shl-crossframes.dgn



USER NAME =	DESIGNED - JPM	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE =	DRAWN - RMG	REVISED -
PLOT DATE =	CHECKED - JHG	REVISED -

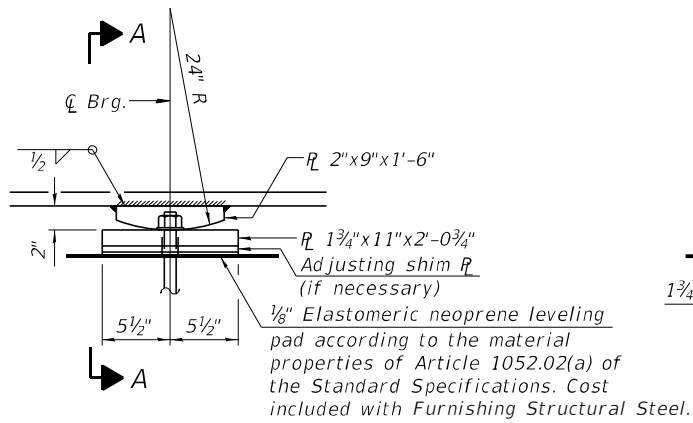
**STATE OF ILLINOIS
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**CROSS FRAME AND GIRDER DETAILS
STRUCTURE NO. 101-0225 & 101-0226**

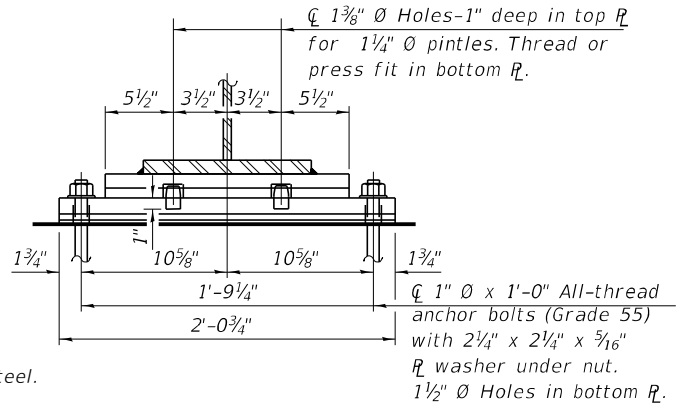
SHEET 50 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	52
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

5/21/2025 8:30:47 AM

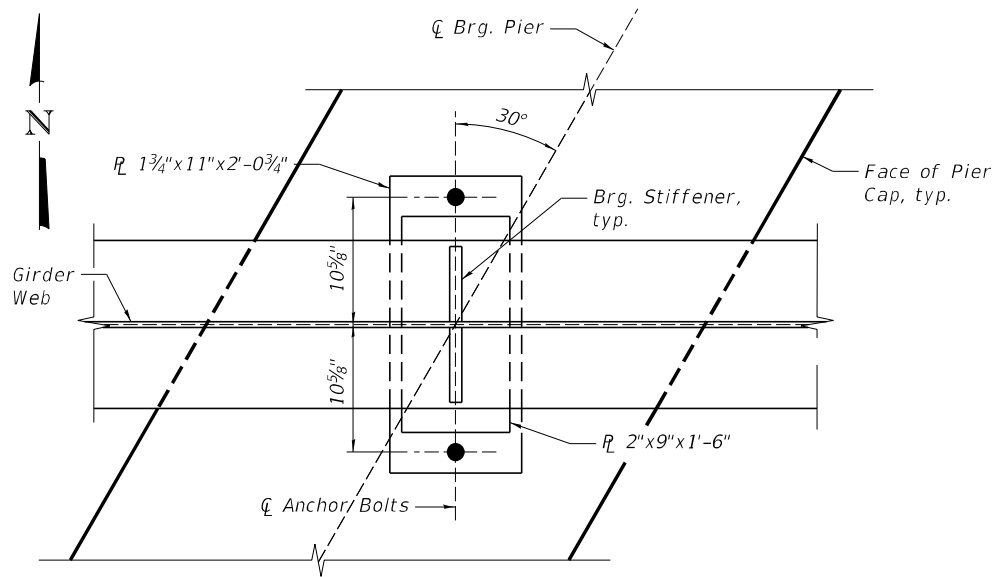


ELEVATION AT PIER

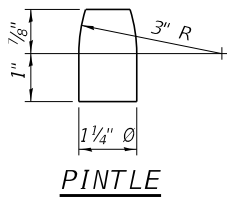


SECTION A-A

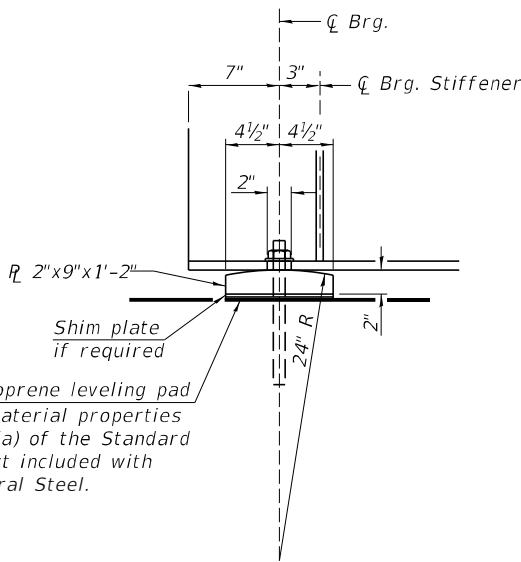
FIXED BEARING - PIERS 1 AND 2



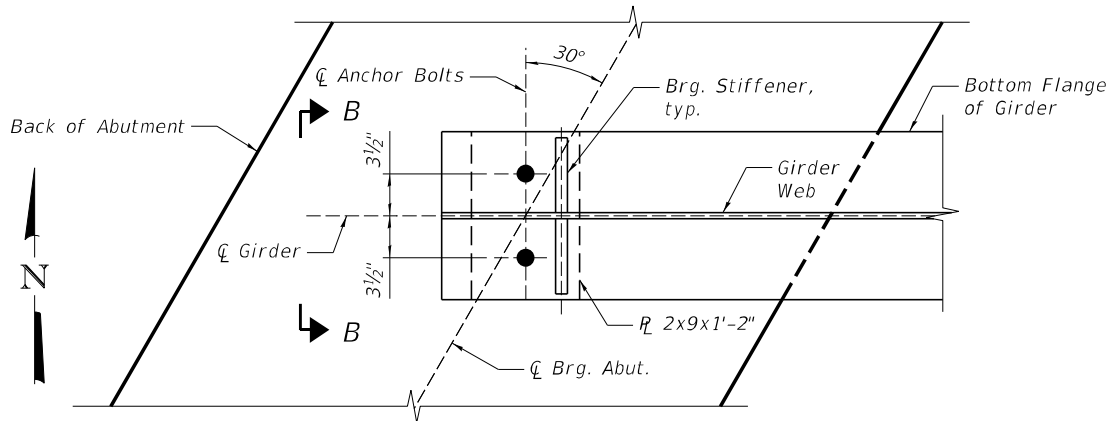
PLAN VIEW - PIERS 1 AND 2
(Connection Plates and Cross Frames not shown for clarity)



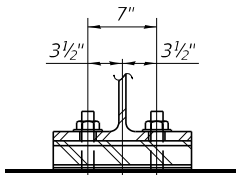
PINTLE



ELEVATION AT ABUTMENT
(W. Abut. looking north, E. Abut. opposite hand)



PLAN AT ABUTMENT
(W. Abut. shown, E. Abut. opposite hand)



1" \varnothing x 12" All-thread anchor bolts (Grade 55) with 2 1/4" x 2 1/4" x 5/16" R washer under nut. 1 3/8" x 2" slotted hole in flange. 1 1/2" \varnothing Holes in bearing plate.

VIEW B-B

FIXED BEARING - ABUTMENT

NOTES:

1. All bearings plates, shims, leveling pads, and pintles shall be included in the cost of Furnishing Structural Steel.
2. The structural steel plates of the bearing and the pintles shall conform to the requirements of AASHTO M270 Grade 50.
3. Two 1/8" adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.
4. All (embedded and separate) bearing plates, anchor bolts, nuts, washers, and pintles shall be galvanized according to AASHTO M111 or M232 as applicable.
5. Anchor bolts at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

MODEL: Default
FILE NAME: c:\pwwork\benesch_projects\projects\dms65234\1010225_0226-shl-bearings.dgn



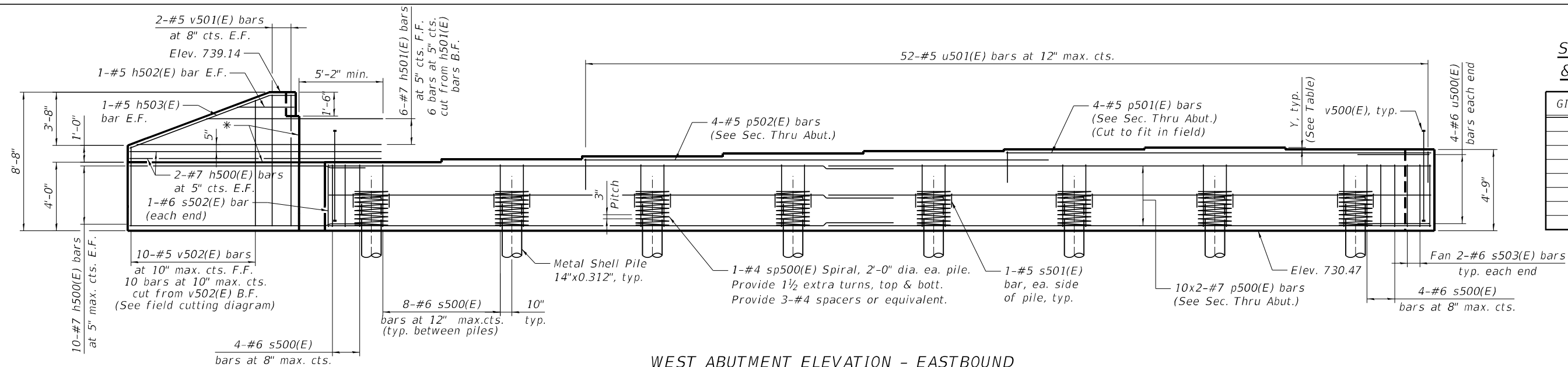
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PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
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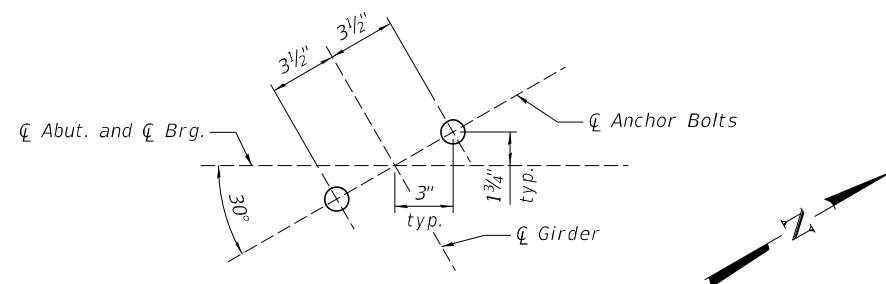
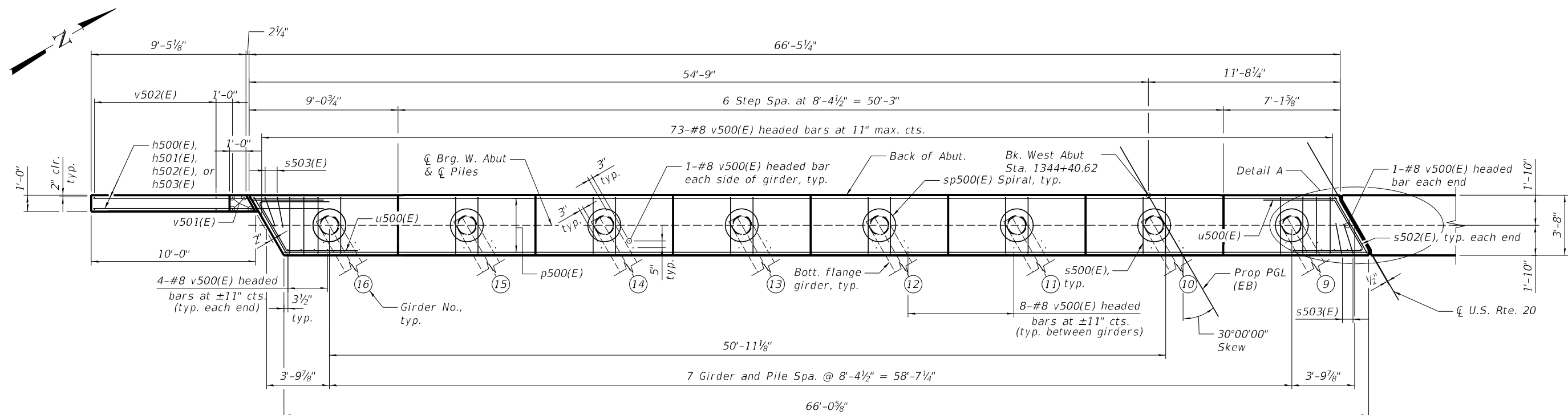
FIXED BEARING DETAILS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 51 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	53
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				



* *Optional construction joints*



MIN BAR LAP
#7 = 5'-2"

NOTES:

1. Pour steps monolithically with cap.
2. Bars indicated thus 10x2-#7 etc. indicates 10 lines of bars with 2 lengths per line.
3. See Sheet 52 of 71 for Detail A.

SEAT ELEVATIONS & STEP HEIGHTS

GIRDER	ELEV.	Y
9	735.22	—
10	735.35	1½"
11	735.25	1¼"
12	735.13	1⅜"
13	734.99	1⅝"
14	734.82	2"
15	734.65	2⅛"
16	734.47	2⅞"

WEST ABUTMENT ELEVATION - EASTBOUND
(Looking West)

WEST ABUTMENT PLAN - EASTBOUND

FOR INFORMATION ONLY



Alfred Benesch & Company
35 W Wacker Drive, Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 10800

USER NAME =	DESIGNED - JPM	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE =	DRAWN - RMG	REVISED -
PLOT DATE =	CHECKED - JHG	REVISED -

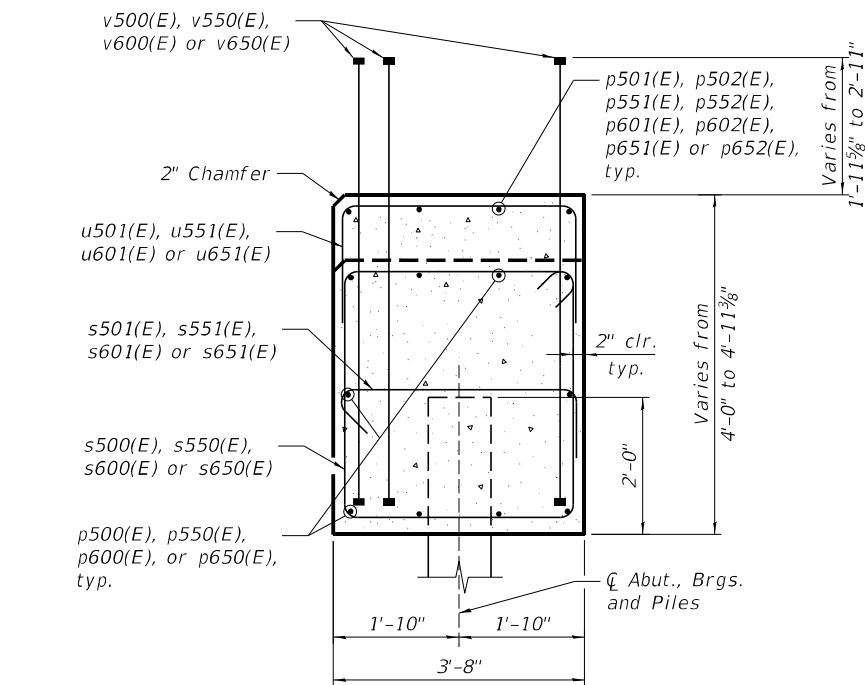
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

WEST ABUTMENT DETAILS (EASTBOUND)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 53 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	55
		CONTRACT NO. 64U98		
ILLINOIS		FED. AID PROJECT		

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\1010225_0226-shr-abutdetails.dgn



SEC. THRU ABUT.

(Dimensions at right angles to abutment.)

WEST ABUTMENT BILL OF MATERIAL WB (SN 101-0226)

Bar	No.	Size	Length	Shape
h600(E)	24	#7	15'-3"	=====
h601(E)	6	#7	25'-1"	=====
h602(E)	2	#5	3'-0"	=====
h603(E)	2	#5	10'-4"	=====
p600(E)	20	#7	35'-10"	=====
p601(E)	4	#5	25'-3"	=====
p602(E)	4	#5	30'-2"	=====
s600(E)	64	#6	15'-4"	=====
s601(E)	16	#5	4'-4"	=====
s602(E)	2	#6	16'-4"	=====
s603(E)	4	#6	7'-8"	=====
* sp600(E)	8	#4	2'-0"	=====
u600(E)	8	#6	12'-4"	=====
u601(E)	52	#5	6'-4"	=====
v600(E)	155	#8	6'-7"	=====
v601(E)	4	#5	8'-4"	=====
v602(E)	10	#5	12'-7"	=====

WEST ABUTMENT BILL OF MATERIAL EB (SN 101-0225)

Bar	No.	Size	Length	Shape
h500(E)	24	#7	15'-3"	=====
h501(E)	6	#7	25'-1"	=====
h502(E)	2	#5	3'-0"	=====
h503(E)	2	#5	10'-4"	=====
p500(E)	20	#7	35'-10"	=====
p501(E)	4	#5	25'-3"	=====
p502(E)	4	#5	30'-2"	=====
s500(E)	64	#6	15'-4"	=====
s501(E)	16	#5	4'-4"	=====
s502(E)	2	#6	16'-4"	=====
s503(E)	4	#6	7'-8"	=====
* sp500(E)	8	#4	2'-0"	=====
u500(E)	8	#6	12'-4"	=====
u501(E)	52	#5	6'-4"	=====
v500(E)	155	#8	6'-7"	=====
v501(E)	4	#5	8'-4"	=====
v502(E)	10	#5	12'-7"	=====

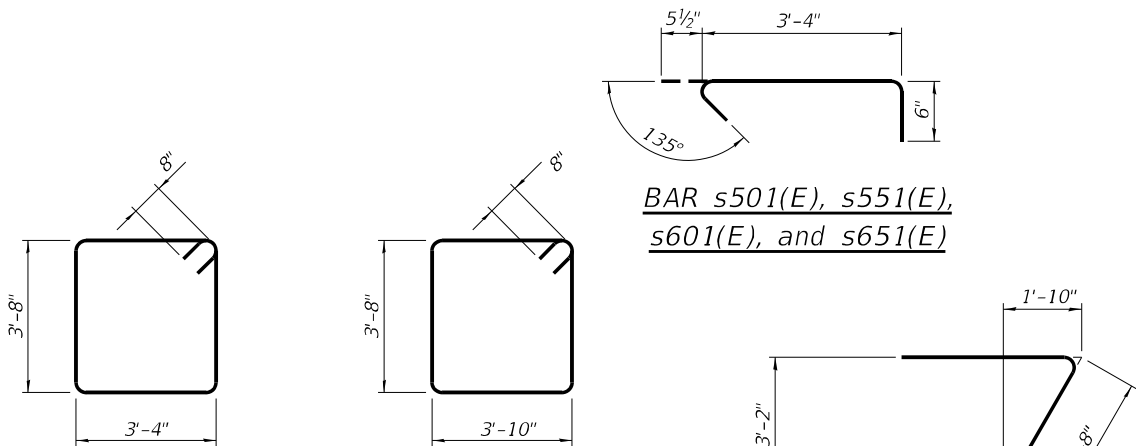
EAST ABUTMENT BILL OF MATERIAL WB (SN 101-0226)

Bar	No.	Size	Length	Shape
h650(E)	24	#7	15'-3"	=====
h651(E)	6	#7	25'-1"	=====
h652(E)	2	#5	3'-0"	=====
h653(E)	2	#5	10'-4"	=====
p650(E)	20	#7	35'-10"	=====
p651(E)	4	#5	25'-3"	=====
p652(E)	4	#5	30'-2"	=====
s650(E)	64	#6	15'-4"	=====
s651(E)	16	#5	4'-4"	=====
s652(E)	2	#6	16'-4"	=====
s653(E)	4	#6	7'-8"	=====
* sp650(E)	8	#4	2'-0"	=====
u650(E)	8	#6	12'-4"	=====
u651(E)	52	#5	6'-4"	=====
v650(E)	155	#8	6'-7"	=====
v651(E)	4	#5	8'-4"	=====
v652(E)	10	#5	12'-7"	=====

EAST ABUTMENT BILL OF MATERIAL EB (SN 101-0225)

Bar	No.	Size	Length	Shape
h550(E)	24	#7	15'-3"	=====
h551(E)	6	#7	25'-1"	=====
h552(E)	2	#5	3'-0"	=====
h553(E)	2	#5	10'-4"	=====
p550(E)	20	#7	35'-10"	=====
p551(E)	4	#5	25'-3"	=====
p552(E)	4	#5	30'-2"	=====
s550(E)	64	#6	15'-4"	=====
s551(E)	16	#5	4'-4"	=====
s552(E)	2	#6	16'-4"	=====
s553(E)	4	#6	7'-8"	=====
* sp550(E)	8	#4	2'-0"	=====
u550(E)	8	#6	12'-4"	=====
u551(E)	52	#5	6'-4"	=====
v550(E)	155	#8	6'-7"	=====
v551(E)	4	#5	8'-4"	=====
v552(E)	10	#5	12'-7"	=====

* Length is height of spiral



BAR s501(E), s551(E), s601(E), and s651(E)

BAR s500(E), s550(E), s600(E) and s650(E)

BAR s502(E), s552(E), s602(E) and s652(E)

BAR u500(E), u550(E), u600(E), and u650(E)

BAR s503(E), s553(E), s603(E), s653(E) u501(E), u551(E), u601(E), and u651(E)

PILE DATA - W. ABUT. - WESTBOUND

Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 513 kips
Factored Resistance Available: 314 kips
Est. Length: 37 feet
No. Production Piles: 7
No. Test Piles: 1

PILE DATA - E. ABUT. - WESTBOUND

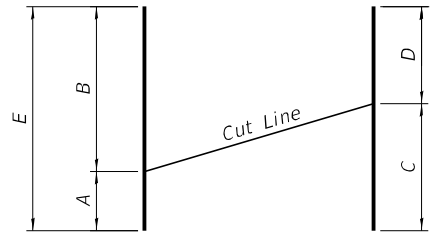
Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 513 kips
Factored Resistance Available: 314 kips
Est. Length: 26 feet
No. Production Piles: 7
No. Test Piles: 1

PILE DATA - W. ABUT. - EASTBOUND

Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 513 kips
Factored Resistance Available: 314 kips
Est. Length: 39 feet
No. Production Piles: 7
No. Test Piles: 1

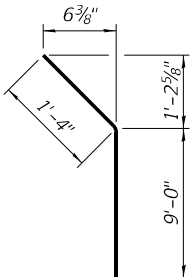
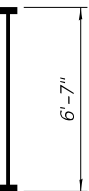
PILE DATA - E. ABUT. - EASTBOUND

Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 513 kips
Factored Resistance Available: 314 kips
Est. Length: 33 feet
No. Production Piles: 7
No. Test Piles: 1



Bar	No.	Size	A	B	C	D	E
h501(E)	6	#7	10'-2"	14'-11"	14'-11"	10'-2"	25'-1"
h551(E)	6	#7	10'-2"	14'-11"	14'-11"	10'-2"	25'-1"
h601(E)	6	#7	10'-2"	14'-11"	14'-11"	10'-2"	25'-1"
h651(E)	6	#7	10'-2"	14'-11"	14'-11"	10'-2"	25'-1"
v502(E)	10	#5	4'-8"	7'-11"	7'-11"	4'-8"	12'-7"
v552(E)	10	#5	4'-8"	7'-11"	7'-11"	4'-8"	12'-7"
v602(E)	10	#5	4'-8"	7'-11"	7'-11"	4'-8"	12'-7"
v652(E)	10	#5	4'-8"	7'-11"	7'-11"	4'-8"	12'-7"

Bar	F	G
s503(E)	3'-8"	2'-0"
s553(E)	3'-8"	2'-0"
s603(E)	3'-8"	2'-0"
s653(E)	3'-8"	2'-0"
u501(E)	3'-4"	1'-6"
u551(E)	3'-4"	1'-6"
u601(E)	3'-4"	1'-6"
u651(E)	3'-4"	1'-6"



NOTES:

1. Bar terminators, paid for separately. See Total Bill of Materials.
2. For details of piles see Sheet 62 of 71.

FIELD CUTTING DIAGRAM

Order h501(E), h551(E), h601(E), h651(E), v502(E), v552(E), v602(E), and v652(E) full length. Cut as shown and use remainder of bars on the opposite face of the wingwall.

FOR INFORMATION ONLY

BAR v500(E), v550(E), v600(E), and v650(E)

(Headed. 1240-#8 Bar Terminators)

BAR h503(E), h553(E), h603(E), and h653(E)



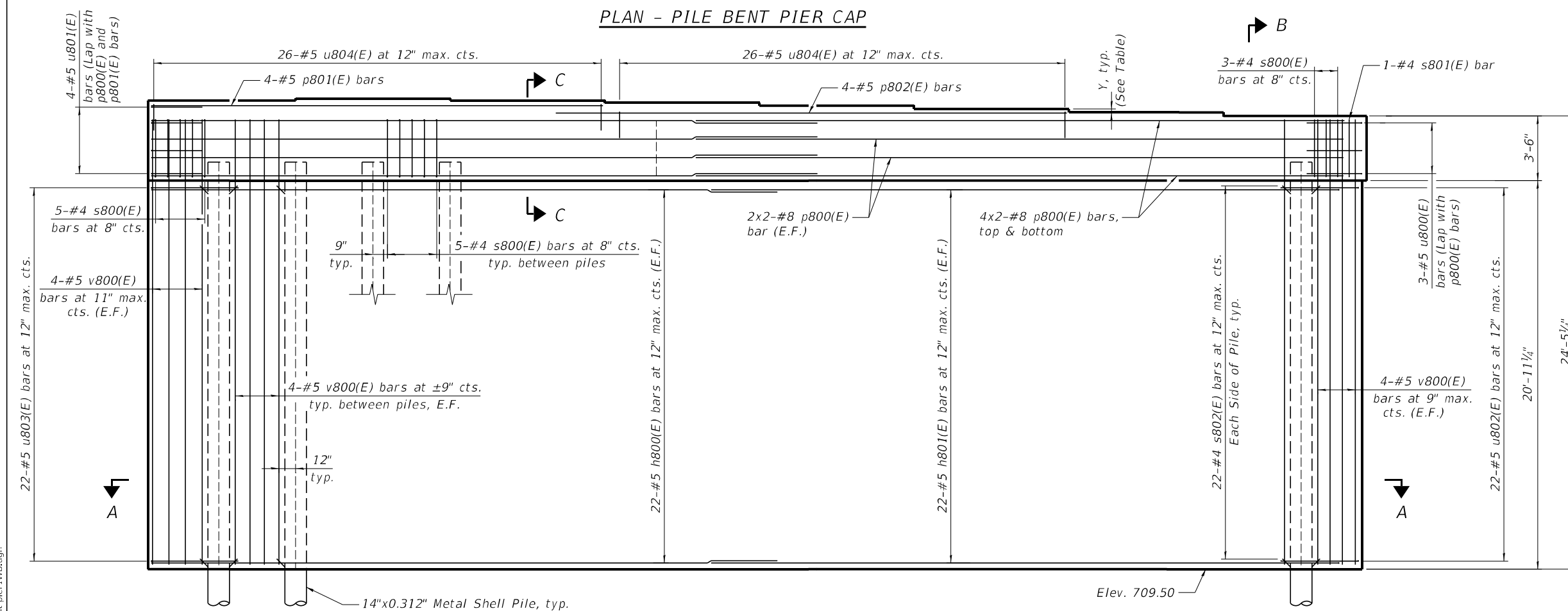
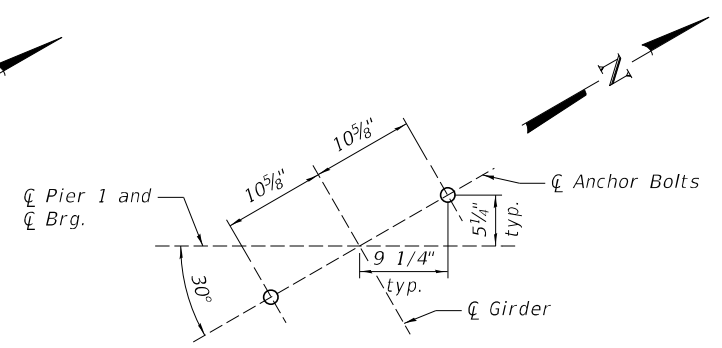
USER NAME =	DESIGNED - JPM	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE =	DRAWN - RMG	REVISED -
PLOT DATE =	CHECKED - JHG	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ABUTMENT DETAILS STRUCTURE NO. 101-0225 & 101-0226

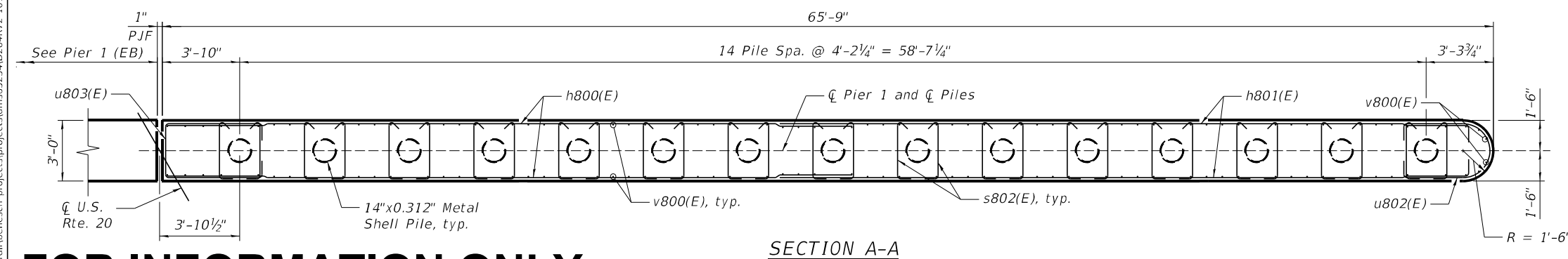
SHEET 56 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	58
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				



<i>GIRDER</i>	<i>ELEV.</i>	<i>Y</i>
1	733.94	—
2	734.12	2¼"
3	734.31	2¼"
4	734.49	2½"
5	734.64	3¾"
6	734.77	1½"
7	734.88	1¾"
8	734.77	1¾"

#5 = 3'-7"
#8 = 5'-9"



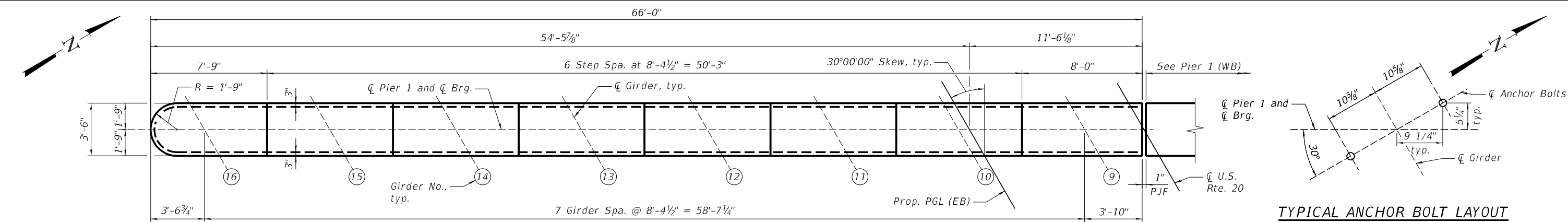
Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 570 kips
Factored Resistance Available: 314 kips
Est. Length: 38 feet
No. Production Piles: 14
No. Test Piles: 1

1. *Four steps monolithically with cap.*
2. *For Sections B-B and C-C, see Sheet 61 of 71.*
3. *Bars indicated thus 4x2-#8 etc., indicates 4 lines of bars with 2 lengths per line.*

FOR INFORMATION ONLY

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	59
		CONTRACT NO. 64U98		
		ILLINOIS	FED. AID PROJECT	

MODEL: Default
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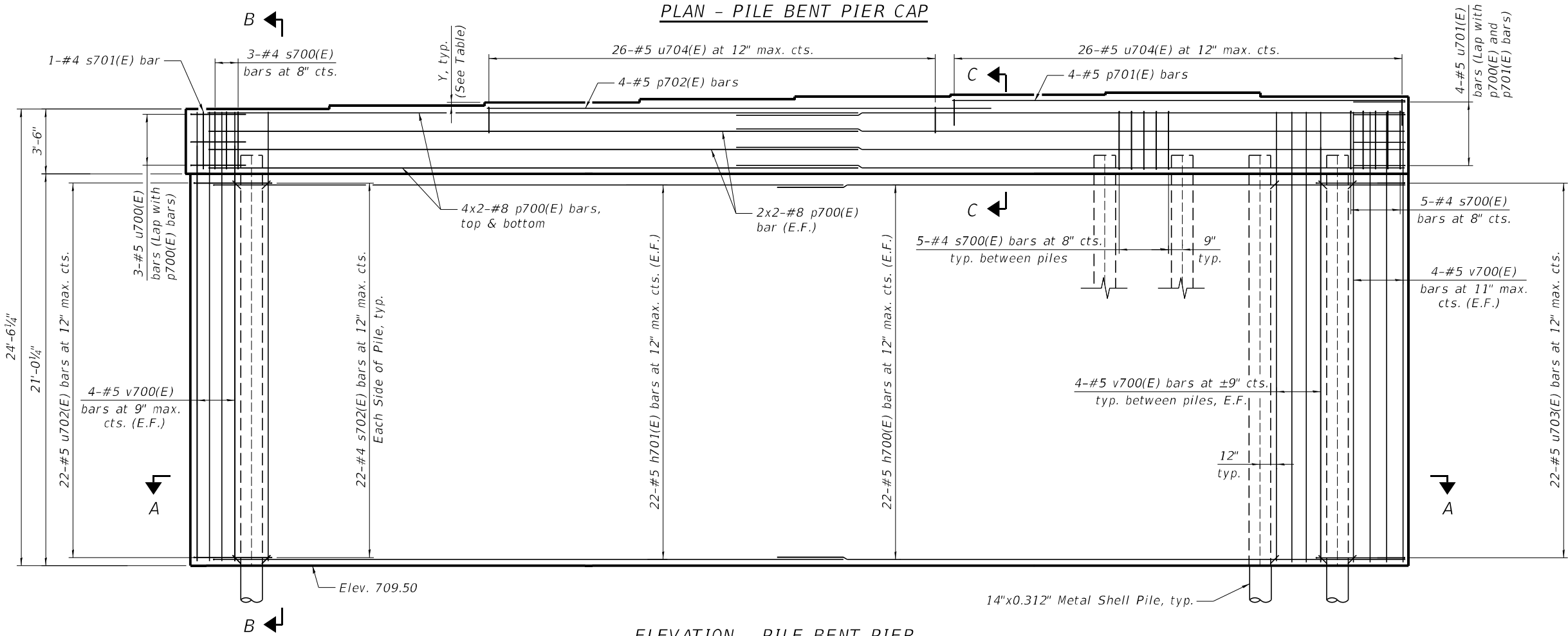
TYPICAL ANCHOR BOLT LAYOUT

SEAT ELEVATIONS & STEP HEIGHTS

GIRDER	ELEV.	Y
9	734.77	—
10	734.90	1 1/2"
11	734.80	1 1/4"
12	734.68	1 3/8"
13	734.54	1 5/8"
14	734.37	2"
15	734.20	2 1/8"
16	734.02	2 1/2"

MIN BAR LAP

#5 = 3'-7"
#8 = 5'-9"



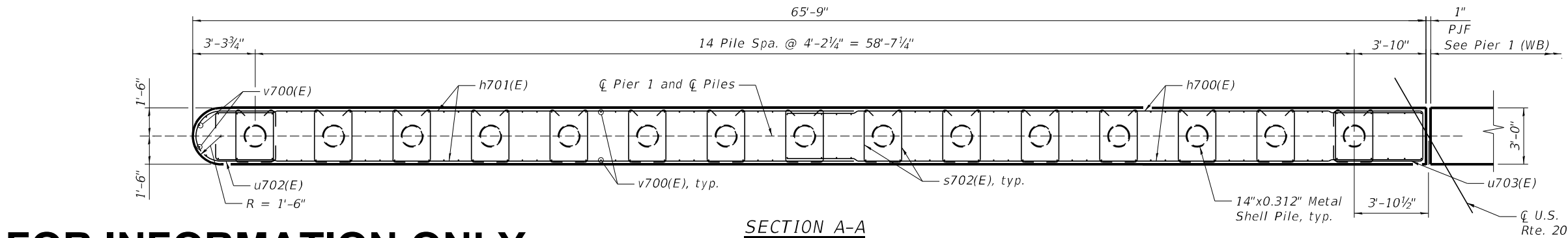
ELEVATION - PILE BENT PIER (Looking West)

PILE DATA - PIER 1 - EASTBOUND

Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 570 kips
Factored Resistance Available: 314 kips
Est. Length: 38 feet
No. Production Piles: 14
No. Test Piles: 1

NOTES:

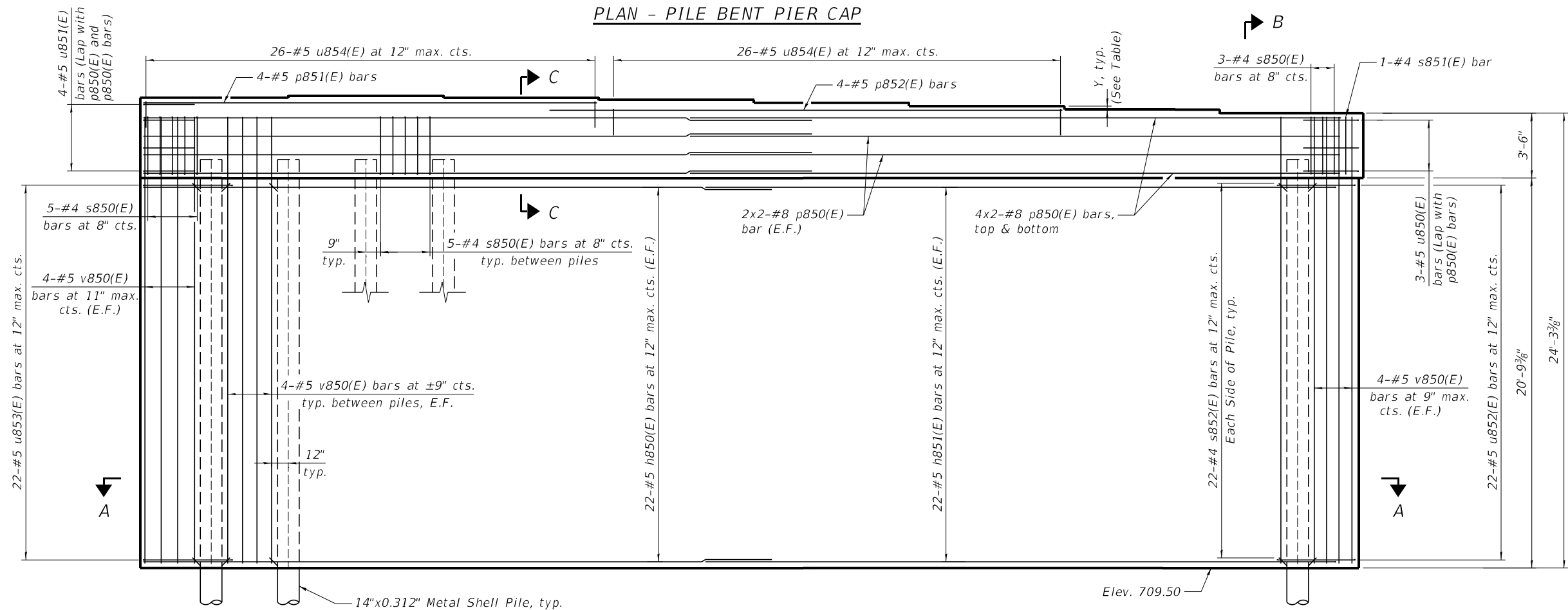
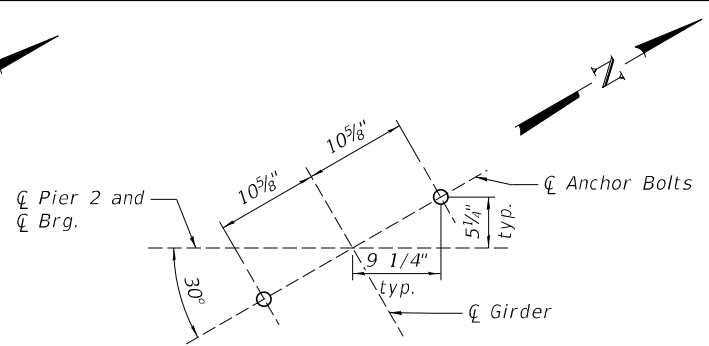
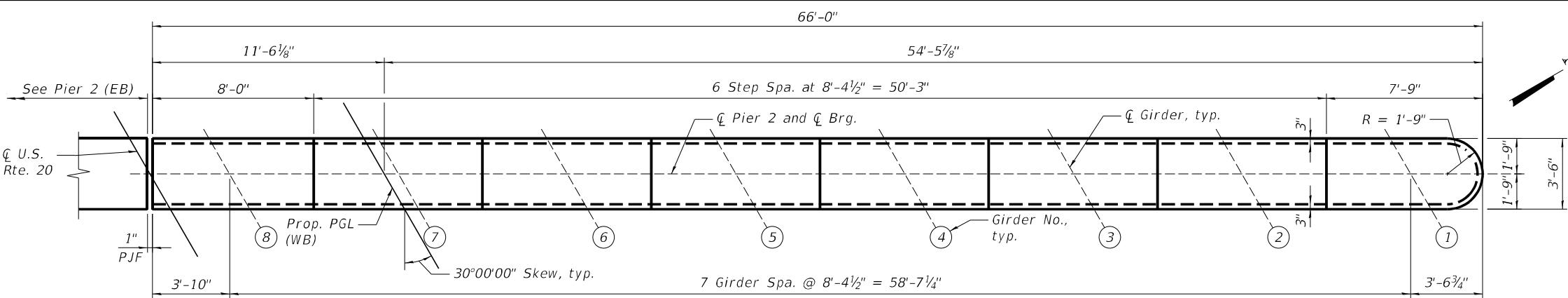
- Pour steps monolithically with cap.
- For Sections B-B and C-C, see Sheet 61 of 71.
- Bars indicated thus 4x2-#8 etc., indicates 4 lines of bars with 2 lengths per line.



FOR INFORMATION ONLY

 Alfred Benesch & Company 35 W Wacker Drive, Suite 3300 Chicago, Illinois 60601 312.465.4150 Job No. 10800	USER NAME =	DESIGNED - JPM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PIER 1 DETAILS (EASTBOUND) STRUCTURE NO. 101-0225 & 101-0226	F.A.P. RTE. =	SECTION =	COUNTY =	TOTAL SHEETS =	SHEET NO. =
	CHECKED - JHG	REVIS	REVISED -			525	6BF	WINNEBAGO	73	60
	PLOT SCALE =	DRAWN - RMG	REVISED -			CONTRACT NO. 64U98				
	PLOT DATE =	CHECKED - JHG	REVISED -			ILLINOIS FED. AID PROJECT				

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\1010225_0226-sht-2\pier2WB.dgn



SEAT ELEVATIONS & STEP HEIGHTS

GIRDER	ELEV.	Y
1	733.78	—
2	733.96	2 1/4"
3	734.15	2 1/4"
4	734.33	2 1/8"
5	734.48	1 3/4"
6	734.61	1 1/2"
7	734.72	1 3/8"
8	734.61	1 3/8"

MIN BAR LAP

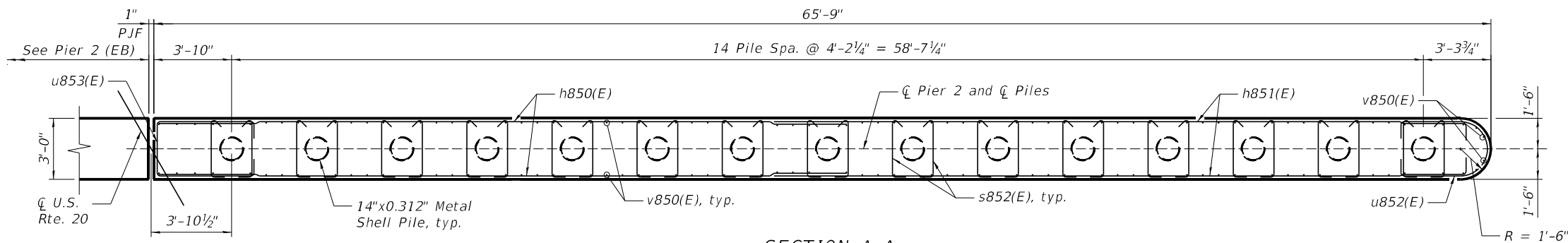
#5 = 3'-7"
#8 = 5'-9"

PILE DATA - PIER 2 - WESTBOUND

Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 570 kips
Factored Resistance Available: 314 kips
Est. Length: 74 feet
No. Production Piles: 14
No. Test Piles: 1

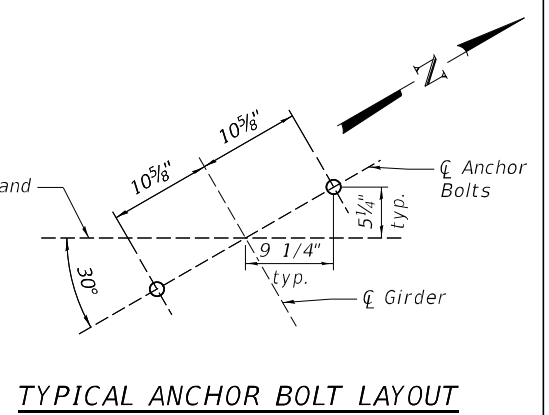
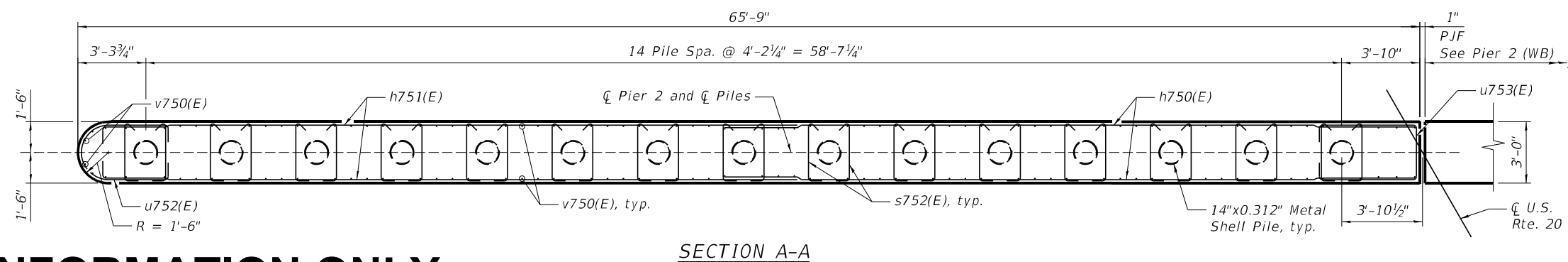
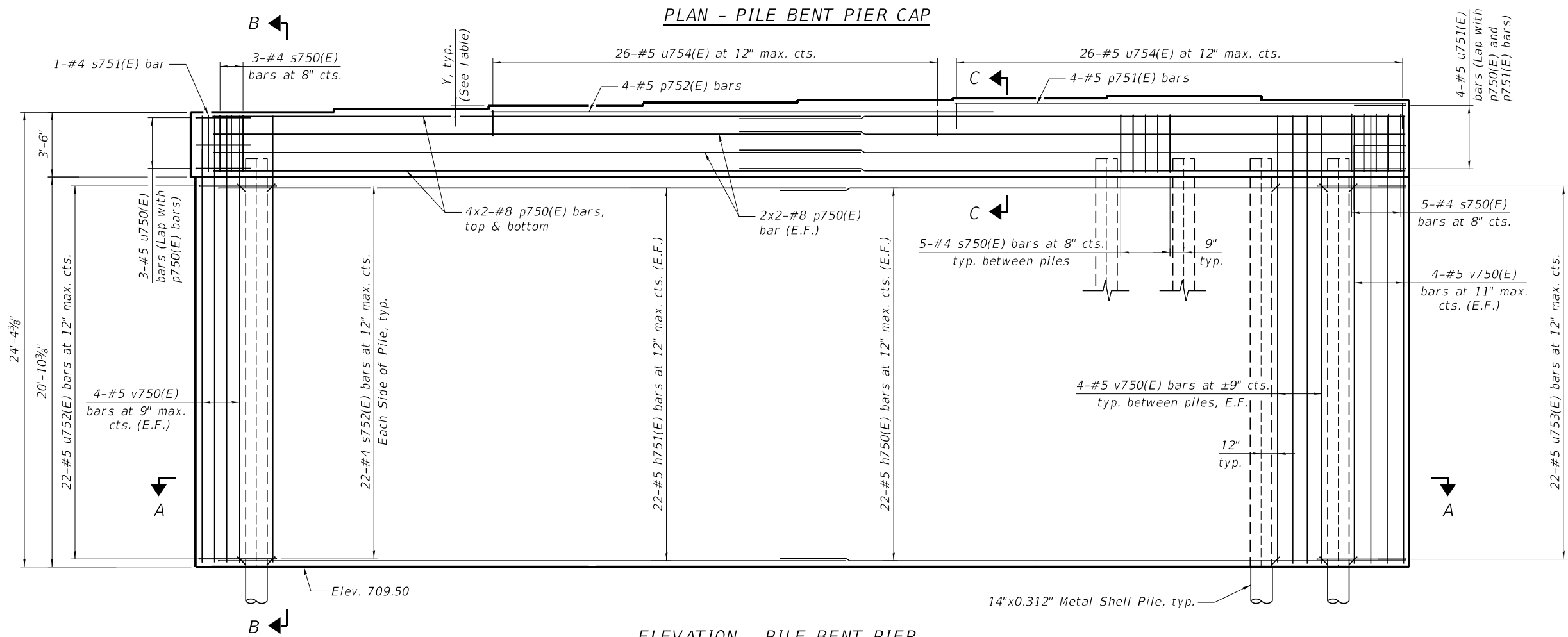
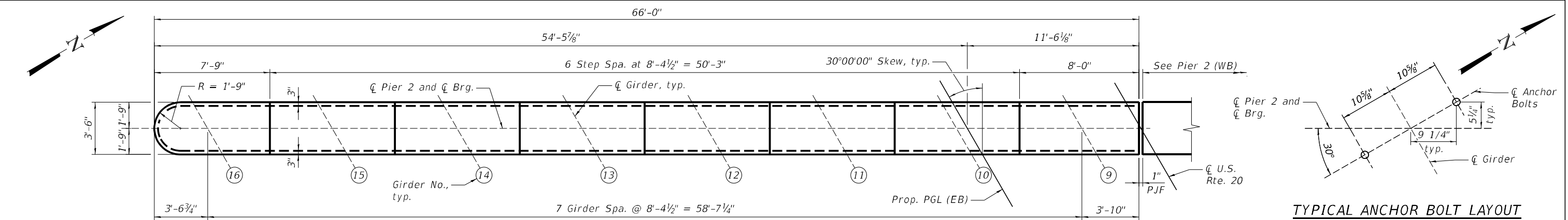
NOTES:

- Pour steps monolithically with cap.
- For Sections B-B and C-C, see Sheet 61 of 71.
- Bars indicated thus 4x2-#8 etc., indicates 4 lines of bars with 2 lengths per line.



FOR INFORMATION ONLY

<div></div> <div>Alfred Benesch & Company 35 W Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-465-4050 Job No. 10800</div>	USER NAME =	DESIGNED - JPM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PIER 2 DETAILS (WESTBOUND) STRUCTURE NO. 101-0225 & 101-0226	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED - JHG	REVISED -			525	6BF	WINNEBAGO	73	61
	PLOT SCALE =	DRAWN - RMG	REVISED -			CONTRACT NO. 64U98				
	PLOT DATE =	CHECKED - JHG	REVISED -			SHEET 59 OF 71 SHEETS				
						ILLINOIS FED. AID PROJECT				



SEAT ELEVATIONS & STEP HEIGHTS

<i>GIRDER</i>	<i>ELEV.</i>	<i>Y</i>
9	734.61	—
10	734.74	1½"
11	734.64	1¼"
12	734.52	1⅜"
13	734.38	1⅝"
14	734.21	2"
15	734.04	2⅛"
16	733.86	2½"

MIN BAR LAP

#5 = 3'-7"
#8 = 5'-9"

PILE DATA - PIER 2 - EASTBOUND

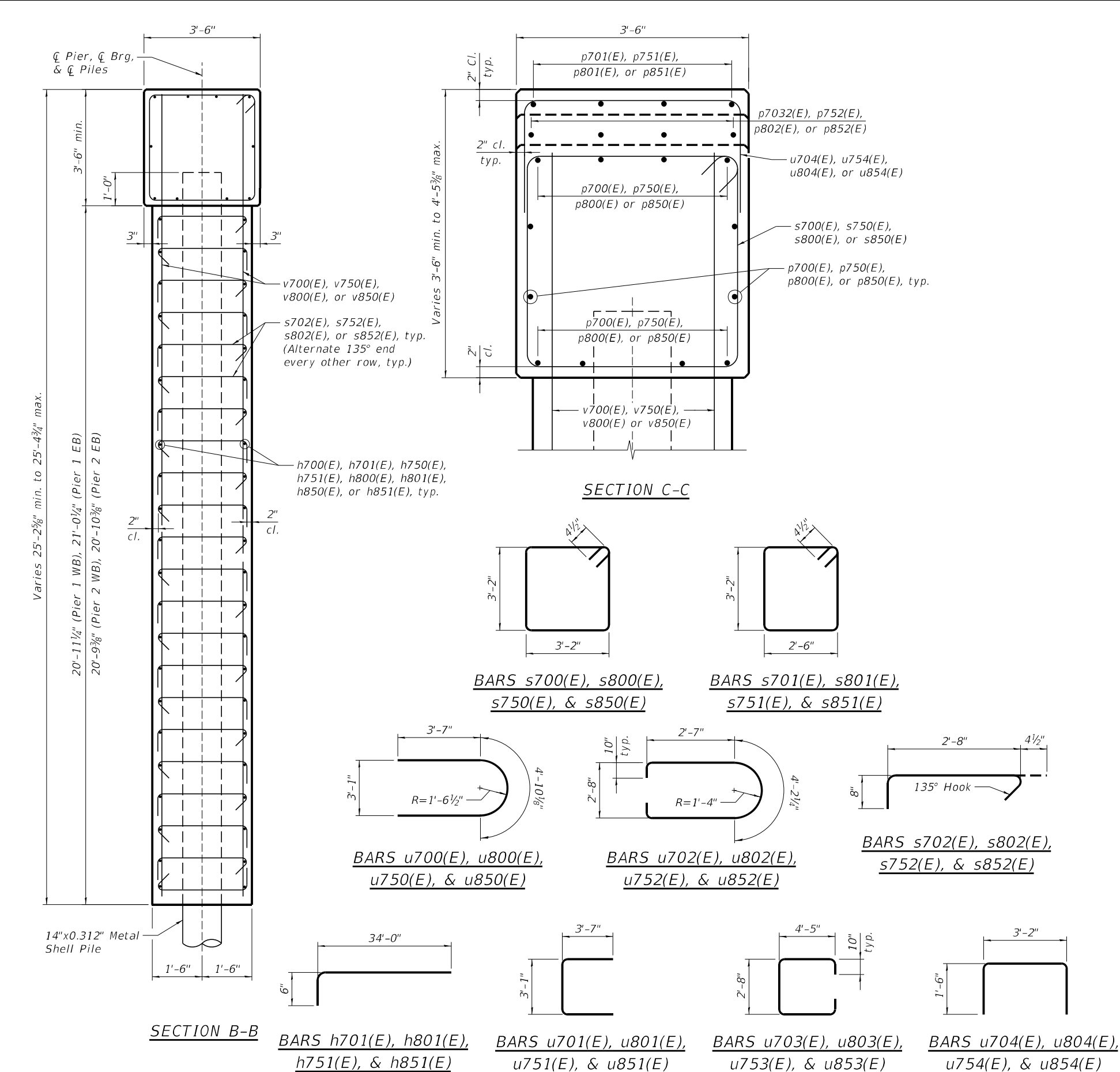
Type: Metal Shell Piles 14"x0.312" w/ Pile Shoes
Nominal Required Bearing: 570 kips
Factored Resistance Available: 314 kips
Est. Length: 38 feet
No. Production Piles: 14
No. Test Piles: 1

NOTES:

1. *Four steps monolithically with cap.*
2. *For Sections B-B and C-C, see Sheet 61 of 71.*
3. *Bars indicated thus 4x2-#8 etc., indicates 4 lines of bars with 2 lengths per line.*

FOR INFORMATION ONLY

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\264r72-1010225_0226-shr-pierdetails.dgn



PIER 1 BILL OF MATERIAL
WB (SN 101-0226)

Bar	No.	Size	Length	Shape
h800(E)	44	#5	34'-0"	—
h801(E)	44	#5	34'-6"	└
p800(E)	24	#8	35'-0"	—
p801(E)	4	#5	24'-5"	—
p802(E)	4	#5	28'-1"	—
s800(E)	78	#4	13'-5"	□
s801(E)	1	#4	12'-1"	□
s802(E)	660	#4	3'-9"	└
u800(E)	3	#5	12'-0"	└
u801(E)	4	#5	10'-3"	└
u802(E)	22	#5	11'-0"	└
u803(E)	22	#5	13'-2"	└
u804(E)	52	#5	6'-2"	└
v800(E)	128	#5	24'-1"	—

PIER 1 BILL OF MATERIAL
EB (SN 101-0225)

Bar	No.	Size	Length	Shape
h700(E)	44	#5	34'-0"	—
h701(E)	44	#5	34'-6"	└
p700(E)	24	#8	35'-0"	—
p701(E)	4	#5	24'-5"	—
p702(E)	4	#5	28'-1"	—
s700(E)	78	#4	13'-5"	□
s701(E)	1	#4	12'-1"	□
s702(E)	660	#4	3'-9"	└
u700(E)	3	#5	12'-0"	└
u701(E)	4	#5	10'-3"	└
u702(E)	22	#5	11'-0"	└
u703(E)	22	#5	13'-2"	└
u704(E)	52	#5	6'-2"	└
v700(E)	128	#5	24'-1"	—

PIER 2 BILL OF MATERIAL
WB (SN 101-0226)

Bar	No.	Size	Length	Shape
h850(E)	44	#5	34'-0"	—
h851(E)	44	#5	34'-6"	└
p850(E)	24	#8	35'-0"	—
p851(E)	4	#5	24'-5"	—
p852(E)	4	#5	28'-1"	—
s850(E)	78	#4	13'-5"	□
s851(E)	1	#4	12'-1"	□
s852(E)	660	#4	3'-9"	└
u850(E)	3	#5	12'-0"	└
u851(E)	4	#5	10'-3"	└
u852(E)	22	#5	11'-0"	└
u853(E)	22	#5	13'-2"	└
u854(E)	52	#5	6'-2"	└
v850(E)	128	#5	23'-11"	—

PIER 2 BILL OF MATERIAL
EB (SN 101-0225)

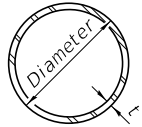
Bar	No.	Size	Length	Shape
h750(E)	44	#5	34'-0"	—
h751(E)	44	#5	34'-6"	└
p750(E)	24	#8	35'-0"	—
p751(E)	4	#5	24'-5"	—
p752(E)	4	#5	28'-1"	—
s750(E)	78	#4	13'-5"	□
s751(E)	1	#4	12'-1"	□
s752(E)	660	#4	3'-9"	└
u750(E)	3	#5	12'-0"	└
u751(E)	4	#5	10'-3"	└
u752(E)	22	#5	11'-0"	└
u753(E)	22	#5	13'-2"	└
u754(E)	52	#5	6'-2"	└
v750(E)	128	#5	23'-11"	—

FOR INFORMATION ONLY

NOTE:

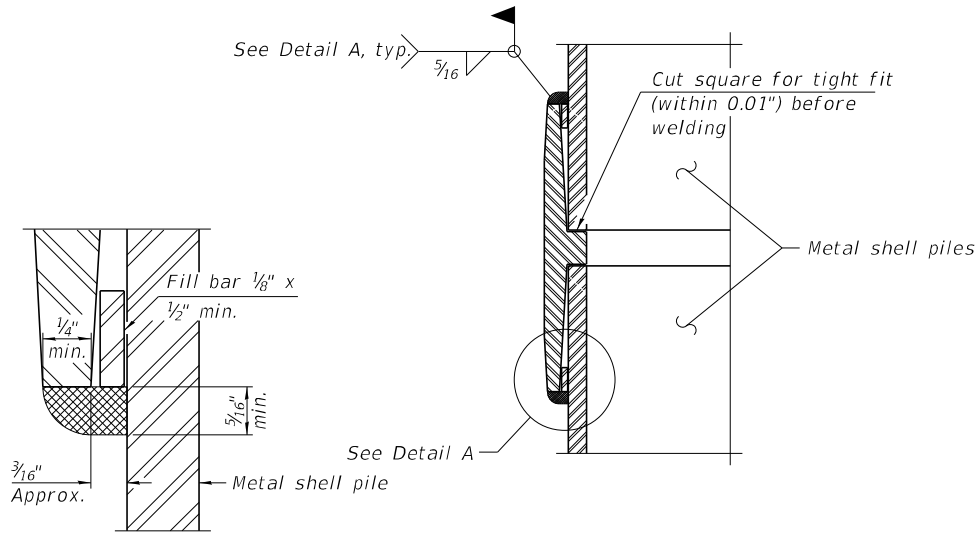
1. For details of metal shell piles, see Sheet 62 of 71.

 Alfred Benesch & Company 35 W Wacker Drive, Suite 3300 Chicago, Illinois 60601 312.465.4050 Job No. 10800	USER NAME =	DESIGNED - JPM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PIER DETAILS STRUCTURE NO. 101-0225 & 101-0226	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	CHECKED - JHG	REVIS	REVIS			525	6BF	WINNEBAGO	73	63
	PLOT SCALE =	DRAWN - RMG	REVISED -			CONTRACT NO. 64U98				
	PLOT DATE =	CHECKED - JHG	REVISED -			ILLINOIS FED. AID PROJECT				

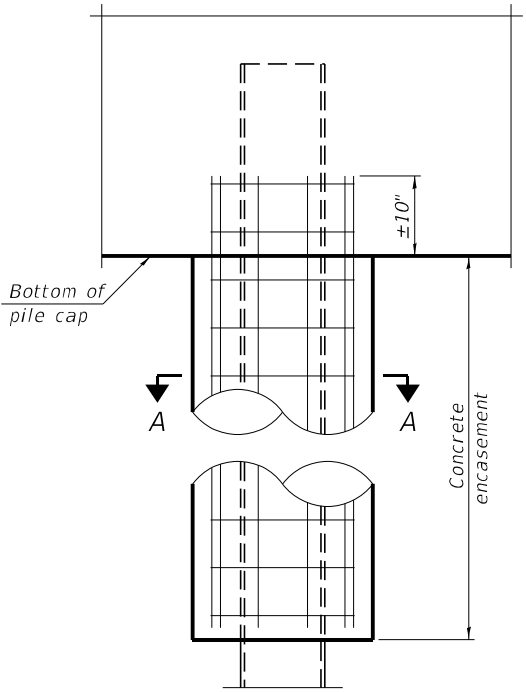


METAL SHELL PILE TABLE

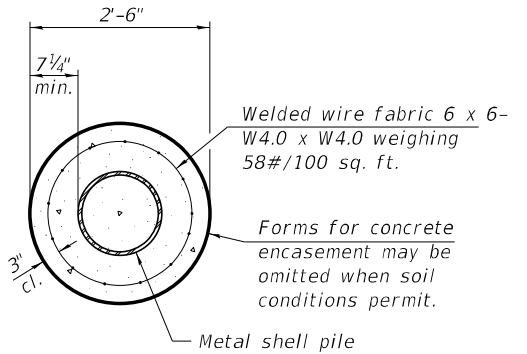
Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. ³ /ft.)
PP12	0.250"	31.40	0.0267
PP14	0.250"	36.75	0.0368
PP14	0.312"	45.65	0.0361
PP16	0.312"	52.32	0.0478
PP16	0.375"	62.64	0.0470



DETAIL A

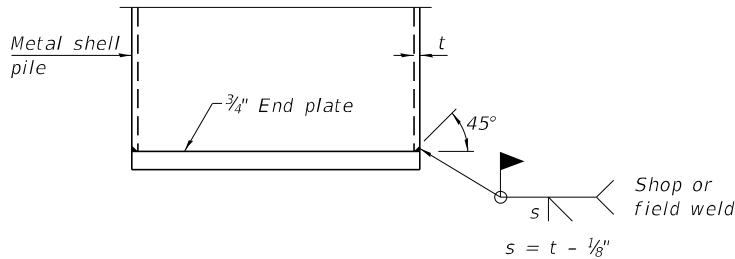


ELEVATION

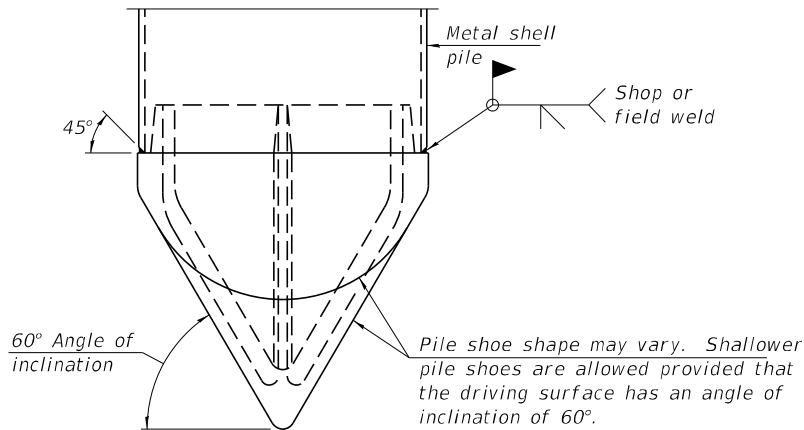


SECTION A-A

INDIVIDUAL PILE
CONCRETE ENCASEMENT
(When specified)



END PLATE ATTACHMENT

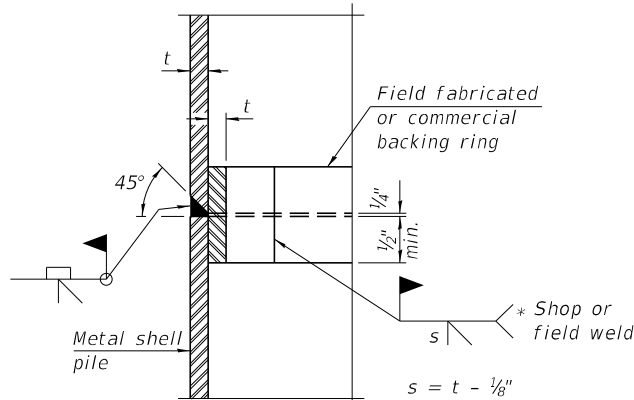


PILE SHOE ATTACHMENT

(When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A 148 Grade 80-50 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld).

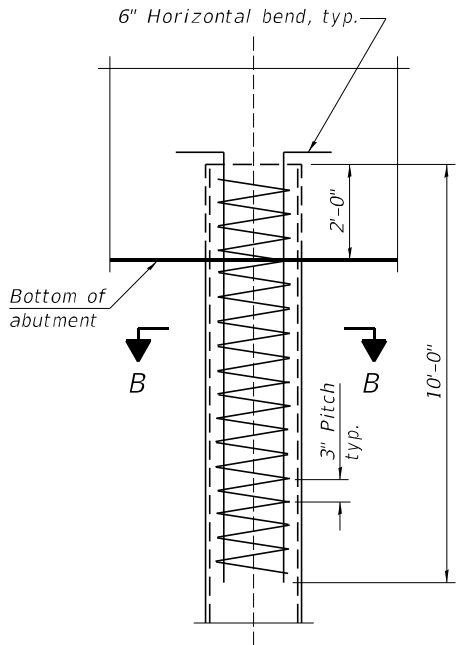
WELDED COMMERCIAL SPLICE

Notes:
The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them.
Pile segments shall be driven to solid contact with splicer before welding.

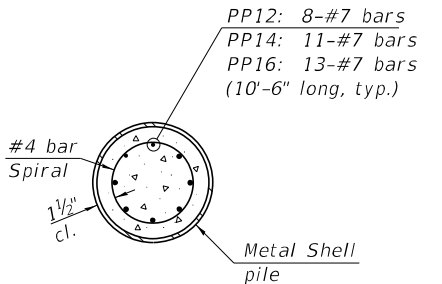


COMPLETE PENETRATION WELD SPLICE

* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.



ELEVATION



SECTION B-B

REINFORCEMENT AT ABUTMENTS
(Omit when concrete encasement is specified)

Note:
The metal shell piles shall be according to Article 1006.05 of the Standard Specifications.

FOR INFORMATION ONLY

F-MS

5-15-2023



USER NAME	=	DESIGNED	-	JPM	REVISED	-
PLOT SCALE	=	CHECKED	-	JHG	REVISED	-
PLOT DATE	=	DRAWN	-	RMG	REVISED	-
		CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

METAL SHELL PILE DETAILS
STRUCTURE NO. 101-0225 & 101-0226

SHEET 62 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	64
CONTRACT NO. 64U98				
ILLINOIS FED. AID PROJECT				

MODEL: Default
FILE NAME: c:\pwword\benesch projects\projects\dms65234\1010225_0226-shl-pile-details.dgn

5/21/2025 8:31:32 AM

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\1010225_0226-shl-slipform.dgn

FOR INFORMATION ONLY



Alfred Benesch & Company
35 W Wacker Drive, Suite 3300
Chicago, Illinois 60601
312.465.4150 Job No. 10800

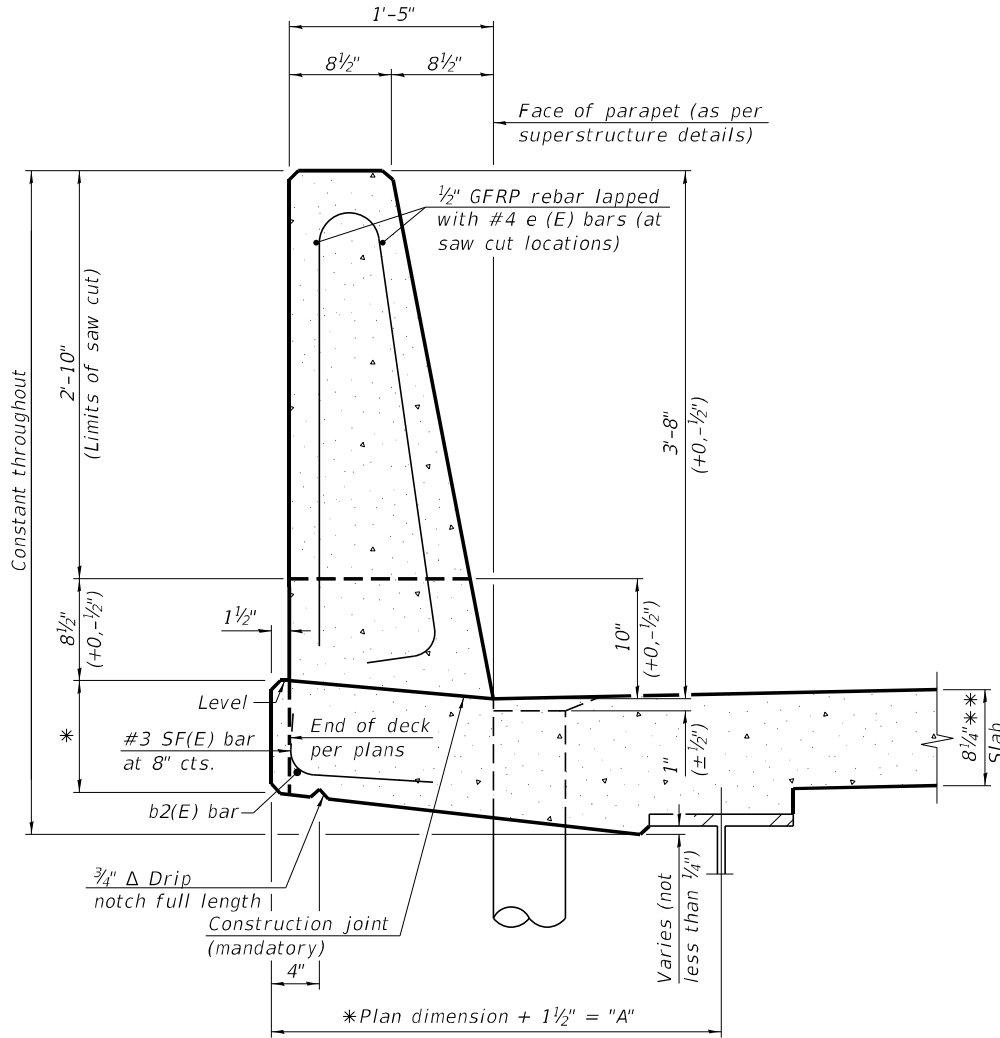
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		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CONCRETE PARAPET SLIPFORMING OPTION
STRUCTURE NO. 101-0225 & 101-0226

SHEET 63 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	65
CONTRACT NO. 64U98				
		ILLINOIS	FED. AID PROJECT	



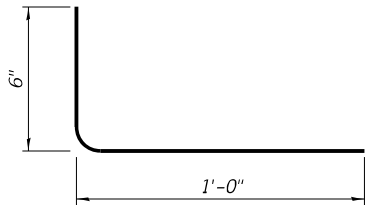
* See Superstructure Details

** Prior to grinding

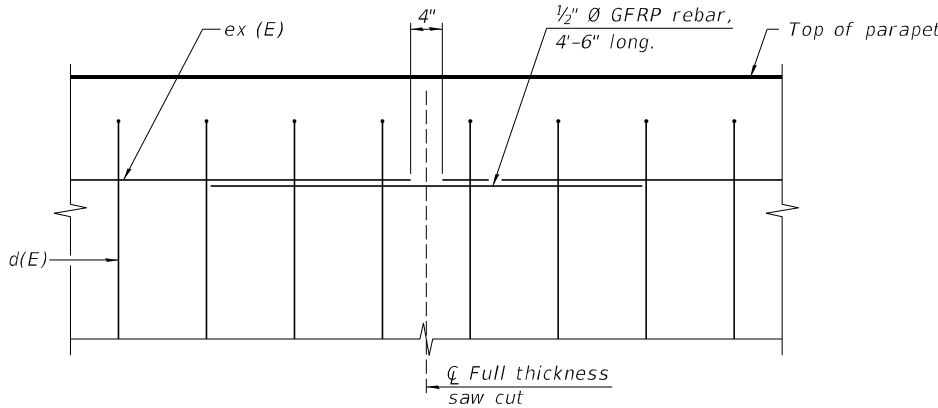
44" CONSTANT-SLOPE

PARAPET SECTION

(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)



SF(E) BAR



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section
at each parapet joint location.)

NOTES:

- All dimensions shall remain the same as shown on superstructure details, except dimension "A" which is to be revised as shown. Additional concrete needed to revise dimension "A" = 0.00348 cu. yds./ft.
- Place full depth aluminum sheets as shown on superstructure details.
- Replace all cork joint filler locations with a full thickness saw cut.

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\ID264R72-1010225_0226-shl-boring-001.dgn

FOR INFORMATION ONLY



USER NAME =	DESIGNED - JPM	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE =	DRAWN - RMG	REVISED -
	CHECKED - JHG	REVISED -
PLOT DATE =		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS (1 OF 8)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 64 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	66
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		



Illinois Department
of Transportation
Division of Highways

SOIL BORING LOG

Page 1 of 4

Date 7/20/21

ROUTE FAP Route 301 DESCRIPTION PTB 193-20 I-39 and U.S. 20 D2 LOGGED BY KEG

SECTION (201-3)K&(4-1.5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO. 101-0074 Station 1345+79.08		D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft Stream Bed Elev. _____ ft	D E P T H	B L O W S	U C S Qu	M O I S T
BORING NO. SB-01 Station 1344+57.6 Offset 30.0 ft LT Ground Surface Elev. 739.90	ft	(ft)	(/6")	(tsf)	(%)	ft	ft	(ft)	(/6")	(tsf) (%)
ASPHALT PAVEMENT - 3" _____ CONCRETE PAVEMENT - 13.5" _____	739.7 738.5					SANDY CLAY FILL - Black, Stiff, Moist (continued) Some Organics				
CLAYEY SAND FILL- Dark Brown, Medium-Grained, Medium-Dense, with Gravel and Pebbles			2 4 4		8			4 4 3		36
SAND FILL - Brown, Medium-Grained, Medium-Dense, with Gravel and Pebbles	736.4					SAND - Gray, Medium-to-Coarse Grained, Medium-Dense, Moist				
			8 10 14		7			6 13 13		16
			8 11 15		8	SANDY GRAVEL - Gray, Medium-Grained, Medium-Dense, Moist				
	731.9							10 13 11		12
SANDY LOAM FILL - Black, Medium-Dense, with Gravel and Cobbles, Wet Diesel Smell			4 3 9		18	No Recovery				
								11 11 15		
			5 12 12		15					
			6 6 11		29	COARSE SAND - Gray, Medium-Dense, with Gravel, Moist				
Becomes Moist, No More Diesel Smell								9 10 12		13
	723.4		4 6 8	2.0 P	23					
SANDY CLAY FILL - Black, Stiff, Moist										
			4 4 8	3.0 P	29	Poor Recovery				
								12 12 11		7

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



Illinois Department
of Transportation
Division of Highways

SOIL BORING LOG

Page 2 of 4

Date 7/20/21

ROUTE FAP Route 301 DESCRIPTION PTB 193-20 I-39 and U.S. 20 D2 LOGGED BY KEG

SECTION (201-3)K&(4-1.5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO. 101-0074		D	B	U	M	Surface Water Elev.	ft	D	B	U	M
Station 1345+79.08		E	L	C	O	Stream Bed Elev.	ft	E	L	C	O
		P	O	S	I			P	O	S	I
		T	W	Qu	S			T	W	Qu	S
		H	S		T			H	S		T
BORING NO. SB-01						Groundwater Elev.:					
Station 1344+57.6						First Encounter	ft				
Offset 30.0 ft LT						Upon Completion	ft				
Ground Surface Elev. 739.90	ft	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(/6")	(tsf)	(%)
COARSE SAND - Gray, Medium-Dense, with Gravel, Moist (continued)						FINE SAND - Gray, Very-Dense, Moist (continued)					
Poor Recovery			11			Becomes Fine			19		
With Limestone Fragments			15		11				31		20
			9						32		
			-45						-65		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



Illinois Department
of Transportation
Division of Highways

SOIL BORING LOG

Page 3 of 4

Date 7/20/21

ROUTE FAP Route 301 DESCRIPTION PTB 193-20 I-39 and U.S. 20 D2 LOGGED BY KEG

SECTION (201-3)K&(4-1.5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO. 101-0074					Surface Water Elev. _____ ft					
Station 1345+79.08					Stream Bed Elev. _____ ft					
BORING NO. SB-01					Groundwater Elev.: _____ ft					
Station 1344+57.6					First Encounter _____ ft					
Offset 30.0 ft LT					Upon Completion _____ ft					
Ground Surface Elev. 739.90	ft	(ft)	(/6")	(tsf)	(%)	ft	(ft)	(/6")	(tsf)	(%)
FINE SAND - Gray, Dense, Moist (continued)					FINE SAND - Gray, Dense, Moist (continued)					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)

Page 1 of 3Date 7/22/21

ROUTE FAP Route 301 DESCRIPTION PTB 193-20 I-39 and U.S. 20 D2 LOGGED BY KEG

SECTION (201-3)K&(4-1,5)R **LOCATION** US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 2 of 3

Date 7/22/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R LOCATION US-20 over Kishwaukee River

COUNTY	Winnebago	DRILLING METHOD	MUD ROTARY	HAMMER TYPE	AUTO
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The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 3 of 3Date 7/22/21

ROUTE	<u>FAP Route 301</u>	DESCRIPTION	<u>PTB 193-20 I-39 and U.S. 20 D2</u>	LOGGED BY	<u>KEG</u>
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SECTION (201-3)K&(4-1,5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

MODEL: Default
FILE NAME: c:\pwwordin\benesch_projects\projects\dms65234\ID264R72-1010225_0226-shf-boring-003.dgn

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USER NAME =	DESIGNED - JPM	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE =	DRAWN - RMG	REVISED -
PLOT DATE =	CHECKED - JHG	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

SOIL BORING LOGS (3 OF 8)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 66 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	68
		CONTRACT NO. 64U98		
ILLINOIS		FED. AID PROJECT		

5/21/2025 8:31:57 AM

Page 1 of 3Date 7/15/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R **LOCATION** US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO.	101-0073	D	B	U	M	Surface Water Elev.	ft	D	B	U	M
Station	1345+79.08	E	L	O	O	Stream Bed Elev.	ft	E	L	O	O
		P	W	S	I			P	W	S	I
BORING NO.	SB-04	T	L		S	Groundwater Elev.:		T	L		S
Station	1344+98.4	H	S	Qu	T	First Encounter	ft	H	S	Qu	T
Offset	30.0 ft RT					Upon Completion	ft				
Ground Surface Elev.	739.96	ft	(ft)	(/6")	(tsf)	After	Hrs.	ft	(ft)	(/6")	(tsf)
					(%)						(%)

[illegible]

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 2 of 3Date 7/15/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R **LOCATION** US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO.	101-0073	D	B	U	M	Surface Water Elev.	ft	D	B	U	M
Station	1345+79.08	E	L	C	O	Stream Bed Elev.	ft	E	L	C	O
		P	W	S	I			P	W	S	I
BORING NO.	SB-04	T	O	Q	T	Groundwater Elev.:		T	O	Q	T
Station	1344+98.4	H	S			First Encounter	ft	H	S		
Offset	30.0 ft RT					Upon Completion	ft				
Ground Surface Elev.	739.96	ft	(ft)	(/6")	(tsf)	After Hrs.	ft	(ft)	(/6")	(tsf)	(%)

Soil Type	Soil Description	Soil Color	Soil Texture	Soil Moisture	Soil Temperature	Soil pH	Soil Density	Soil Weight	Soil Volume	Soil Area	Soil Depth	Soil Thickness	Soil Width	Soil Height	Soil Length	Soil Circumference	Soil Perimeter	Soil Surface Area	Soil Volume	Soil Weight	Soil Density	Soil Temperature	Soil pH	Soil Moisture	Soil Texture	Soil Color	Soil Type
GRAVELLY SAND - Gray, Medium-Dense, Coarse-Grained, Moist (continued) Becomes Wet	9																										
	7																										
	6			18																							
	11																										
	15			20																							
	9																										
	13																										
	28			12																							
	28																										
	26																										
34																											
32																											
SAND - Gray, Dense, Very-Fine Grained, Moist	18																										
	24			17																							
	26																										
	13																										
	28			12																							
	28																										
	26																										
	34																										
	32																										
	15			20																							
9																											
7																											
6			18																								
11																											
15			20																								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 3 of 3Date 7/15/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO. 101-0073	D	B	U	M	Surface Water Elev.	ft
Station 1345+79.08	E	L	C	O	Stream Bed Elev.	ft
	P	O	S	I		
BORING NO. SB-04	T	W		S	Groundwater Elev.:	
Station 1344+98.4	H	S	Qu	T	First Encounter	ft
Offset 30.0 ft RT					Upon Completion	ft
Ground Surface Elev. 739.96	ft	(ft)	(/6")	(tsf)	After Hrs.	ft

	DEPTH Feet	SOUNDINGS	WATER LEVEL
SILT - Gray, Very-Stiff, Moist <i>(continued)</i>			
	658.0		
SAND - Gray, Very-Dense, Very-Fine Grained			
	20		
	28	17	
	27		
	-85		
	24		
	39	22	
	39		
Becomes Medium-Grained	-90		
	16		
	20	23	
	23		
	-95		
	14		
	21	20	
	24		
640.0	+100		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\ID264R72-1010225_0226-shl-boring-004.dgn

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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

SOIL BORING LOGS (4 OF 8)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 67 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	69
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		

5/21/2025 8:32:06 AM

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\ID264R72-1010225_0226-silt-boring-006.dgn

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USER NAME	=	DESIGNED - JPM	REVISED -
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		CHECKED - JHG	REVISED -
PLOT DATE			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS (6 OF 8)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 69 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	71
CONTRACT NO. 64U98				
		ILLINOIS	FED. AID PROJECT	



Illinois Department
of Transportation
Division of Highways

SOIL BORING LOG

Page 1 of 3

Date 7/14/21

ROUTE FAP Route 301 DESCRIPTION PTB 193-20 I-39 and U.S. 20 D2 LOGGED BY KEG

SECTION (201-3)K&(4-1.5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO.	101-0073	D	B	U	M	Surface Water Elev.	ft	D	B	U	M
Station	1345+79.08	E	L	C	O	Stream Bed Elev.	ft	E	L	C	O
		P	O	S	I			P	O	S	I
		T	W	S	S			T	W	S	S
BORING NO.	SB-06	H	S	Qu	T	Groundwater Elev.:	ft	H	S	Qu	T
Station	1346+24.6					First Encounter	ft				
Offset	30.0 ft RT					Upon Completion	ft				
Ground Surface Elev.	739.91	ft	(ft)	(/6")	(tsf)	(%)		(ft)	(/6")	(tsf)	(%)

Note: Drilling Through Bridge Deck
CONCRETE PAVEMENT - 8.25"
Blind Drilling
Total Casing used = 40 ft

Blind Drilling
Total Casing used = 40 ft
(continued)

GRAVELLY SAND - Brown,
Coarse-Grained, Loose

SAND - Brown, Fine-to-Coarse
Grained, Loose, with Gravel
Sample Refusal on Limestone
Fragments at 33.5'

GRAVELLY SAND - Brown,
Medium-Dense, Coarse-Grained,
with Rubbles and Pebbles, Wet

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department
of Transportation
Division of Highways

SOIL BORING LOG

Page 2 of 3

Date 7/14/21

ROUTE FAP Route 301 DESCRIPTION PTB 193-20 I-39 and U.S. 20 D2 LOGGED BY KEG

SECTION (201-3)K&(4-1.5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO.	101-0073	D	B	U	M	Surface Water Elev.	ft	D	B	U	M
Station	1345+79.08	E	L	C	O	Stream Bed Elev.	ft	E	L	C	O
		P	O	S	I			P	O	S	I
		T	W	S	S			T	W	S	S
BORING NO.	SB-06	H	S	Qu	T	Groundwater Elev.:	ft	H	S	Qu	T
Station	1346+24.6					First Encounter	ft				
Offset	30.0 ft RT					Upon Completion	ft				
Ground Surface Elev.	739.91	ft	(ft)	(/6")	(tsf)	(%)		(ft)	(/6")	(tsf)	(%)

GRAVELLY SAND - Brown,
Medium-Dense, Coarse-Grained,
with Rubbles and Pebbles, Wet
(continued)
Becomes Dense

SANDY GRAVEL - Brown-Gray,
Dense, Fine-to-Coarse Grained,
Moist

SAND - Gray, Medium-Dense,
Very-Fine Grained, with Trace
Gravel and Silt, Moist

SILT - Gray, Very-Stiff, Moist

SILT - Gray, Very-Stiff, Moist
(continued)

SAND - Gray- Brown, Dense,
Very-Fine Grained, Moist

Becomes Very Dense

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department
of Transportation
Division of Highways

SOIL BORING LOG

Page 3 of 3

Date 7/14/21

ROUTE FAP Route 301 DESCRIPTION PTB 193-20 I-39 and U.S. 20 D2 LOGGED BY KEG

SECTION (201-3)K&(4-1.5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

STRUCT. NO.	101-0073	D	B	U	M	Surface Water Elev.	ft	D	B	U	M
Station	1345+79.08	E	L	C	O	Stream Bed Elev.	ft	E	L	C	O
		P	O	S	I			P	O	S	I
		T	W	S	S			T	W	S	S
BORING NO.	SB-06	H	S	Qu	T	Groundwater Elev.:	ft	H	S	Qu	T
Station	1346+24.6					First Encounter	ft				
Offset	30.0 ft RT					Upon Completion	ft				
Ground Surface Elev.	739.91	ft	(ft)	(/6")	(tsf)	(%)		(ft)	(/6")	(tsf)	(%)

SAND - Gray- Brown, Dense,
Very-Fine Grained, Moist
(continued)
Becomes Medium-Grained

SILT - Gray, Hard, Moist

SAND - Gray, Very-Dense,
Medium-Grained, with Sandstone
Fragments, Moist

3" Seam of Silt

End of Boring
The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

Page 1 of 2Date 7/19/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R **LOCATION** US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 2 of 2

Date 7/19/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R **LOCATION** US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 1 of 4Date 7/12/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

MODEL: Default
FILE NAME: c:\pwword\benesch_projects\projects\dms65234\ID264R72-1010225_0226-shl-boring-007.dgn

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS (7 OF 8)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 70 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	72
CONTRACT NO. 64U98				
ILLINOIS		FED. AID PROJECT		

5/21/2025 8:32:35 AM

Page 2 of 4Date 7/12/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R **LOCATION** US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 3 of 4Date 7/12/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R **LOCATION** US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Page 4 of 4Date 7/12/21

ROUTE	FAP Route 301	DESCRIPTION	PTB 193-20 I-39 and U.S. 20 D2	LOGGED BY	KEG
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SECTION (201-3)K&(4-1,5)R LOCATION US-20 over Kishwaukee River

COUNTY Winnebago DRILLING METHOD MUD ROTARY HAMMER TYPE AUTO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

FOR INFORMATION ONLY



USER NAME	=	DESIGNED	-	JPM	REVISED	-
		CHECKED	-	JHG	REVISED	-
PLOT SCALE	=	DRAWN	-	RMG	REVISED	-
PLOT DATE	=	CHECKED	-	JHG	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS (8 OF 8)
STRUCTURE NO. 101-0225 & 101-0226

SHEET 71 OF 71 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
525	6BF	WINNEBAGO	73	73
		CONTRACT NO. 64U98		
ILLINOIS		FED. AID PROJECT		

5/21/2025 8:32:44 AM