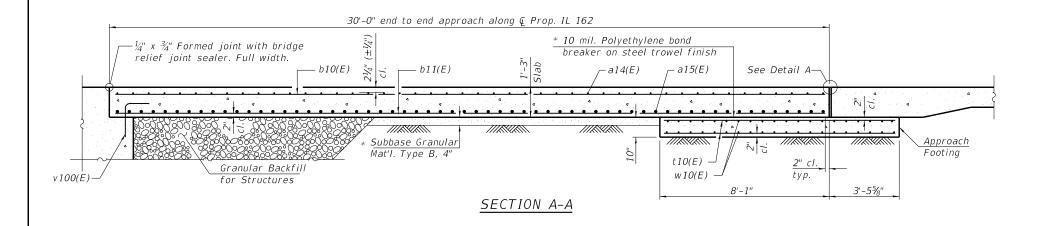


INSIDE ELEVATION OF PARAPET AND CURB



Note

The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.

Parapet concrete shall be paid for as Concrete Superstructure.

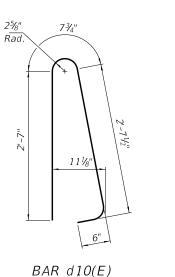
Approach slab shall be paid for as Concrete Superstructure (Approach Slab).

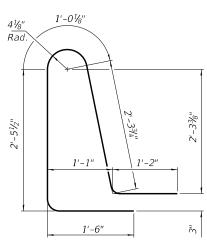
Approach footing concrete shall be paid for as Concrete Structures.

The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.

Cost of excavation for approach footing included with Concrete Structures.

For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 31.

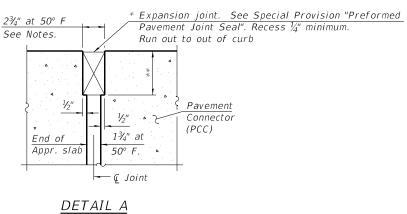




<u>WEST APPROACH</u> <u>BILL OF MATERIAL</u>

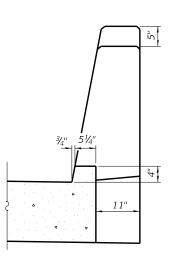
BAR d11(E)

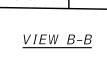
No. 92	Size	Length	Shape
92			
	#5	33'-7"	
20	#8	33'-10"	
46	#5	7'-4"	
83	#5	29'-8"	
32	#9	29'-8"	
8	#5	14'-8"	
1	#4	14'-9"	
1	#4	14'-3"	
46	#5	6'-5"	
46	#5	8'-6"	Ľ
			_
20	#4	14'-8"	
10	#4	10'-11"	
80	#5	32'-7"	
uctur	es	Cu. Yd.	19.2
erstr	ucture	Cu. Yd.	3.8
erstr	ucture	C. Val	76.8
ab)		Cu. Ya.	/ 0.8
nt Bar	·s,	D /	22.260
		Pound	33,260
	446 83 32 8 1 1 1 446 446 20 10 wcturr perstrice perstrice ab)	#5 #83 #5 #83 #5 #82 #9 #8 #5 #1 #4 #4 #6 #5 #6 #5 #6 #5 #7 #6 #5 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7 #7	#5 7'-4" #83 #5 29'-8" #83 #5 14'-8" #84 14'-9" #1 #4 14'-3" #66 #5 6'-5" #66 #5 8'-6" #70 #4 10'-11" #70 #70 #70 #70 #70 #70 #70 #70 #70 #70

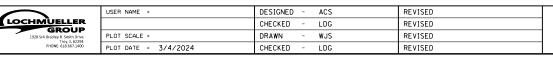


DETAIL A
(at Rt. L's)

- * Cost included with Concrete Superstructure (Approach Slab).
- ** Per manufacturer recommendations







STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WEST BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 060-0241

SHEET NO. 18 OF 31 SHEETS

33'-1"

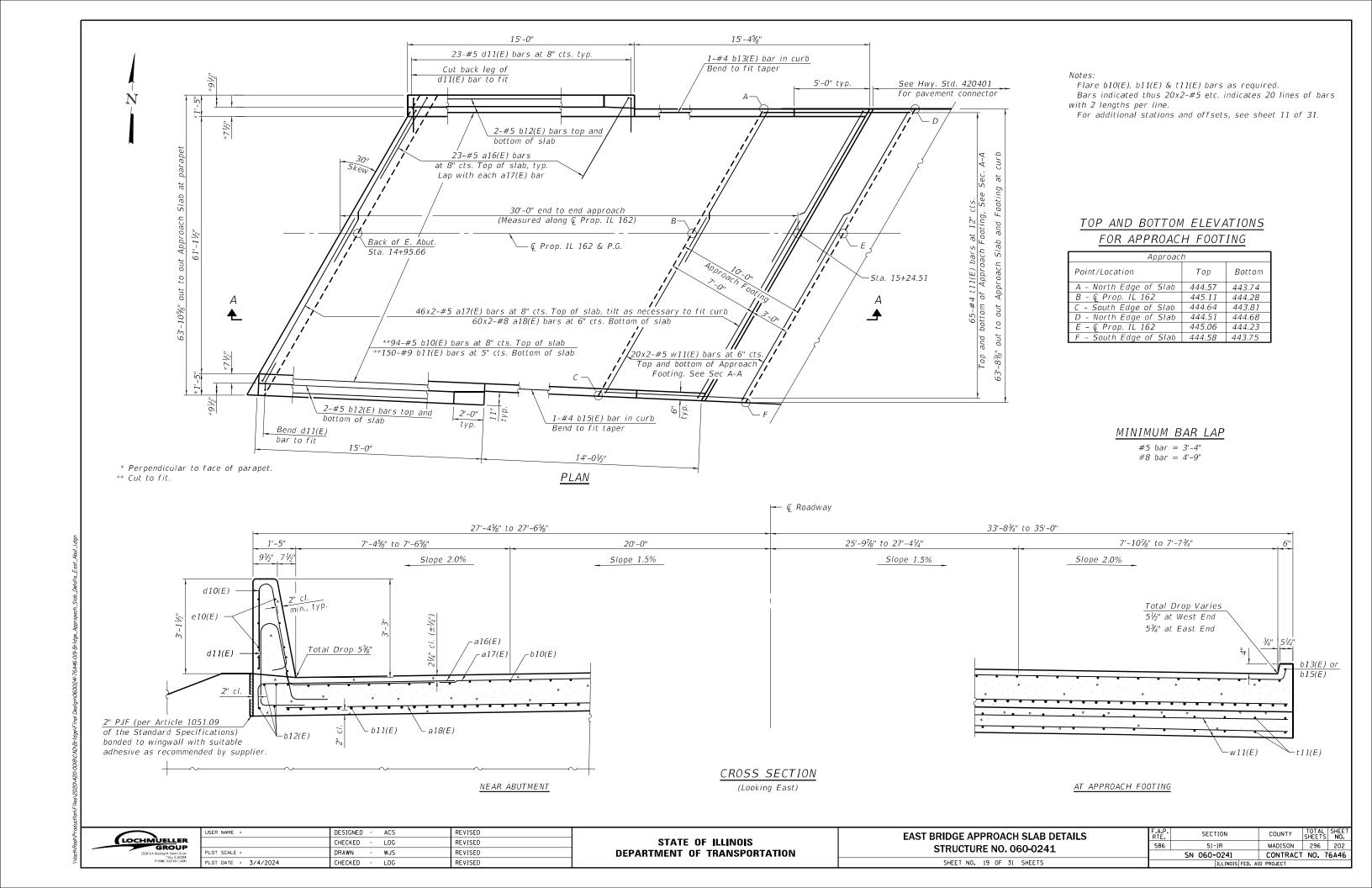
BAR a14(E)

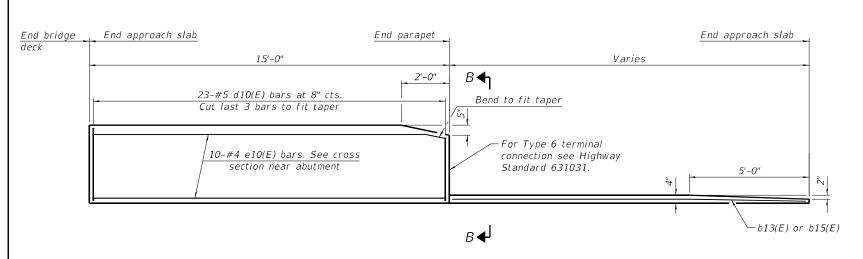
6'-6"

 $BAR \ a16(E)$

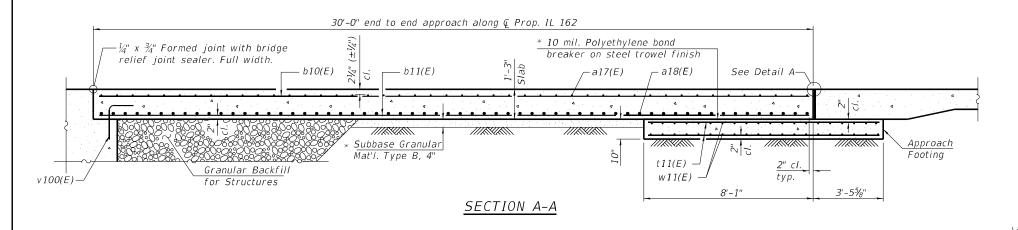
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.						
586	51-1R	MADISON	296	201						
SN 060-0241 CONTRACT NO. 76A46										
TILLINGIS FED. AID PROJECT										

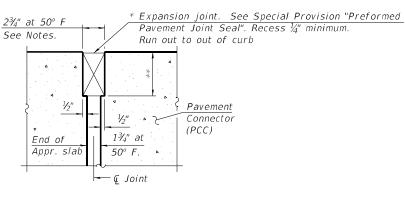
.CAD\Bridge\Final Design\060024I-75A46-0I8-Bridge_Appropach_Siab_Details_West_Abut_2.dgn





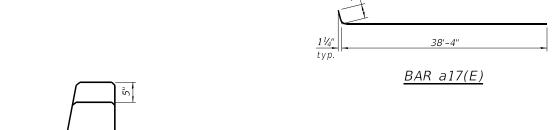
INSIDE ELEVATION OF PARAPET AND CURB





 $\frac{DETAIL A}{(at Rt. L's)}$

- * Cost included with Concrete Superstructure (Approach Slab).
- ** Per manufacturer recommendations



Notes:

The joint opening shall be adjusted for temperature per Article 520.04 of the

length of bridge used to calculate the adjustment shall be equal to half the total

Approach slab shall be paid for as Concrete Superstructure (Approach Slab). Approach footing concrete shall be paid for as Concrete Structures.

Cost of excavation for approach footing included with Concrete Structures.

The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.

For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 31.

bridge length plus the length of the bridge approach slab.

Parapet concrete shall be paid for as Concrete Superstructure.

For a16(E), d10(E) and d11(E) bar details, see sheet 18 of 31.

Standard Specifications. However, since this detail is for jointless structures, the

<u>EAST APPROACH</u> BILL OF MATERIAL

_				
Bar	No.	Size	Length	Shape
a16(E)	46	#5	7'-4"	
a17(E)	92	#5	38'-10"	
a18(E)	120	#8	39'-1"	
b10(E)	94	#5	29'-8"	
b11(E)	150	#9	29'-8"	
b12(E)	8	#5	14'-8"	
b13(E)	1	#4	14'-9"	
b15(E)	1	#4	13'-9"	
d10(E)	46	#5	6'-5"	Λ
d11(E)	46	#5	8'-6"	
e10(E)	20	#4	14'-8"	
t11(E)	130	#4	11'-0"	
w11(E)	80	#5	38'-3"	
Concrete		Cu. Yd.	22.7	
Concrete	Superstr	Cu. Yd.	3.8	
Concrete	Superstr	Cu. Yd.	88.8	
(Approach	Slab)		cu. ru.	00.0
Reinforce	ment Bar	5,	Pound	39,850
Ероху Со	ated		Found	39,030

LOCHMUELLER GROUP
1928 SrA Bradley R. Smith Drive Troy, IL 62294 PHONE: 618.667.1400

CHECKED - LDG REVISED PLOT SCALE = DRAWN - WJS REVISED	
PLOT SCALE = DRAWN - W.IS REVISED	
Dimini 100	
PLOT DATE = 3/4/2024 CHECKED - LDG REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

5½"

VIEW B-B

 					SLAB DETAILS 60-0241
SHEET	NO.	20	OF	31	SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
586	51-1R	MADISON	296	203
	SN 060-0241	CONTRACT	NO. 7	6A46
	TILL INDIC CED	AID DROJECT		

**TOP OF BEAM ELEVATIONS

Location	⊊ Brg. W. Abut.	⊊ Brg. Pier 1	© Splice 1	© Splice 2	⊊ Brg. Pier 2	⊊ Brg. E. Abut.
Beam 1	445.48	445.54	445.56	445.47	445.41	445.20
Beam 2	445.60	445.68	445.70	445.64	445.58	445.37
Beam 3	445.70	445.79	445.82	445.77	445.71	445.50
Beam 4	445.79	445.88	445.91	445.90	445.84	445.63
Beam 5	445.76	445.88	445.91	445.88	445.83	445.64
Beam 6	445.63	445.76	445.80	445.76	445.71	445.53
Beam 7	445.50	445.63	445.67	445.64	445.59	445.41
Beam 8	445.33	445.46	445.50	445.49	445.44	445.27

** "For Fabrication Only"

Notes:

All diaphragms between beams shall be installed with erection pins and bolts in accordance with erection plan approved by the Engineer. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor

For Beam Dimension Tables, see sheet 22 of 31.

For diaphragm details and dimensions, see sheet 23 of 31. For Detail A and B, see sheet 23 of 31.

***DIAPHRAGM LENGTHS

Diaphragm	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	D12	D13	D14	D15	D16
No. of Locations	18	1	3	3	1	3	3	1	1	1	1	1	1	1	1	1
Length	7'-3"	8'-43/4"	8'-41/2"	8'-53/8"	9'-31/8"	8'-4 ¹ / ₂ "	10'-83/4"	7'-41/4"	7'-67/8"	7'-75/8"	7'-83/8"	7'-5¾"	7'-111/8"	7'-111/8"	8'-05/8"	7'-71/8"
Diaphragm	D17	D18	D19	D20	D21	D22	D23	D24	D25	D26	D27	D28	D29	D30	D31	
No. of Locations	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
Length	8'-21/2"	8'-3 ¹ / ₄ "	8'-41/8"	7'-81/4"	8'-5½"	8'-63/8"	8'-71/8"	7'-9½"	8'-87/8"	8'-9¾"	8'-105/8"	7'-11"	9'-11/8"	9'-2"	9'-2 ⁷ /8"	

***Dimensions are taken along diaphragm from <code>Q</code> to <code>Q</code> beam.

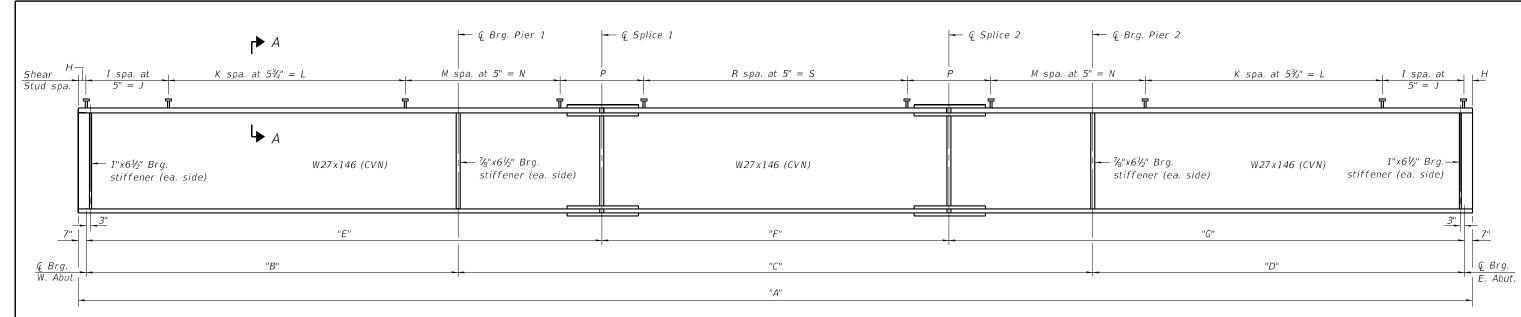


CHECKED - LDG REVISED PLOT SCALE = DRAWN - WJS REVISED	
PLOT SCALE = DRAWN - WJS REVISED	
PLOT DATE = 3/4/2024 CHECKED - LDG REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN STRUCTURE NO. 060-0241		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		51-1R	MADISON	296	204	
		SN 060-0241	CONTRACT	NO. 7	6A46	
SHEET NO. 21 OF 31 SHEETS	ILLINOIS FED. AID PROJECT					

*Perpendicular to fascia beam

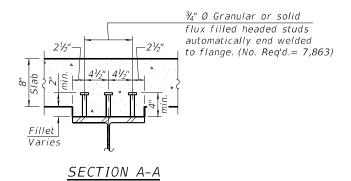


BEAM ELEVATION

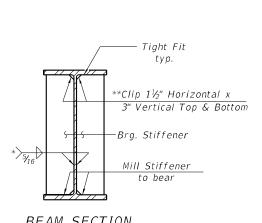
"CVN" denotes Charpy-V-Notch impact energy requirements, zone 2.

BEAM DIMENSION TABLE

Beam	Α	В	С	D	Ε	F	G	Н	I	J	Κ	L	М	N	P	R	5
1	151'-45%"	46'-13/4"	57'-11 ¹ / ₈ "	46'-1¾"	57'-9 ¹ / ₈ "	34'-8¾"	57'-9 ¹ / ₈ "	0'-3¾"	13	5'-5"	71	34'-01/4"	41	17'-1"	3'-01/4"	76	31-8"
2	150'-11"	46'-0"	57'-9"	46'-0"	57'-7"	34'-7"	57'-7"	0'-4¾"	12	5'-0"	71	34'-01/4"	41	17'-1"	3'-4"	75	31-3"
3	150'-11"	46'-0"	57'-9"	46'-0"	57'-7"	34'-7"	57'-7"	0'-43/4"	12	5'-0"	71	34'-01/4"	41	17'-1"	3'-4"	75	31-3"
4	150'-11"	46'-0"	57'-9"	46'-0"	57'-7"	34'-7"	57'-7"	0'-43/4"	12	5'-0"	71	34'-01/4"	41	17'-1"	3'-4"	75	31-3"
5	150'-11"	46'-0"	57'-9"	46'-0"	57'-7"	34'-7"	57'-7"	0'-43/4"	12	5'-0"	71	34'-01/4"	41	17'-1"	3'-4"	75	31-3"
6	149'-85%"	45'-75/8"	57'-3½"	45'-7 ⁵ / ₈ "	57'-1½"	34'-3¾"	57'-1½"	0'-41/8"	12	5'-0"	70	33'-61/2"	41	17'-1"	3'-5¾"	74	30'-10"
7	148'-65/8"	45'-31/4"	56'-10"	45'-3 ¹ / ₄ "	56'-8"	34'-01/2"	56'-8"	0'-23/4"	16	6'-8"	60	28'-9"	48	20'-0"	3'-21/2"	74	30'-10"
8	147'-4 ⁷ / ₈ "	44'-111%"	56'-4¾''	44'-11½"	56'-2 ⁷ / ₈ "	33'-91/4"	56'-2 ⁷ / ₈ "	0'-21/2"	16	6'-8"	59	28'-3½"	48	20'-0"	3'-41/4"	73	30'-5"



Notes: Structural steel for beams and splices shall be AASHTO M270 Grade 50.

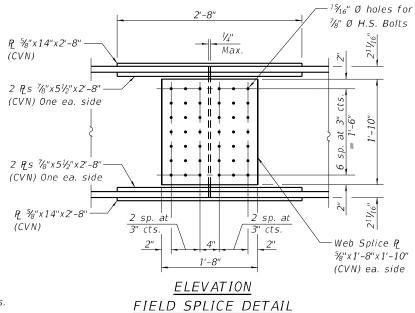


<u>BEAM SECTION</u> <u>AT STIFFENER</u>

- * Terminate $\frac{1}{4}$ " ($\pm\frac{1}{8}$ ") from the end of plate intersects.
- ** Clip may be rounded for ease of shop painting.

¹⁵ / ₁₆ " Ø holes fo ½" Ø H.S. Bolts		1/4" Max.	- Flange S	- = -
= - -	·		===	11-2"
1¾"	sp. at 1¾" cts.	4" 7 alt. sp. at 1¾" cts.	134"	11/2

TOP & BOTTOM

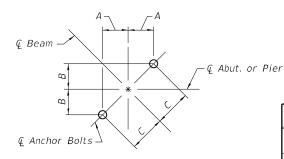


	LOCHMUELLER
_	GROUP
	1928 SrA Bradley R. Smith Drive
	Troy, IL 62294

USER NAME =	DESIGNED - ACS	REVISED
	CHECKED - LDG	REVISED
PLOT SCALE =	DRAWN - WJS	REVISED
PLOT DATE = 3/4/2024	CHECKED - LDG	REVISED

STRUCTURAL STEEL STRUCTURE NO. 060-0241	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO OGO_02/1	586	51-1R	MADISON	296	205
3111001011E NO: 000-0241		SN 060-0241	CONTRACT	NO. 7	76A46
SHEET NO. 22 OF 31 SHEETS		ILLINOIS FED. A	D PROJECT		

(16 Required)



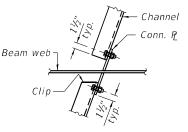
ANCHOR BOLT LAYOUT FOR

FIXED BEARINGS AT

ABUTMENTS AND PIERS

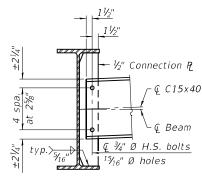
ANCHOR BOLT DIMENSIONS

Location	Beam No.	А	В	C
West & East Abutments	1-8	3"	13/4"	31/2"
	1-5	73/4"	41/2"	9"
Piers 1 & 2	6	77/8"	4¾"	9"
11013 1 0 2	7	77/8"	41/4"	9"
	8	8"	4½"	9"



<u>DETAIL A</u>
(14 required on skew)



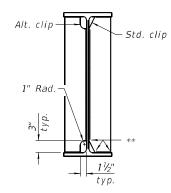


INTERIOR DIAPHRAGM

Conn. R Channel Conn. R Channel Beam web C Diaphragms

DETAIL B

(42 required perpendicular to beam)

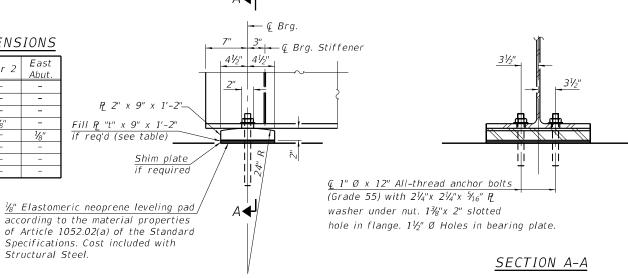


WELD LIMITS AND CLIP DETAILS

** Stop welds $\frac{1}{4}$ " ($\pm\frac{1}{6}$ ") from edges as shown. Typical.

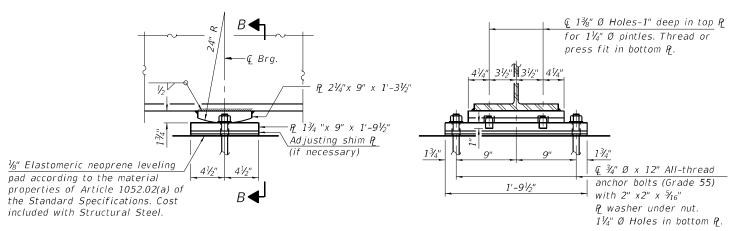
TABLE OF "t" DIMENSIONS

Beam No.	West Abut.	Pier 1	Pier 2	East Abut.
1	-	-	-	-
2	-	-	-	-
3	-	-	-	-
4	11/16"	-	1/8"	-
5	11/16"	-	-	1/8"
6	-	-	-	-
7	-	-	-	-
8	-	-	-	-



ELEVATION AT ABUTMENT

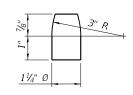
(16 Required)



ELEVATION AT PIER

(16 Required)

FIXED BEARING



Notes

Two $\frac{1}{8}$ in. adjustment shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on the bearing details.

The anchor bolt sizes and grades shown constitute a calculated seismic structural fuse. Substitutions of higher diameter and/or grade anchor bolts will not be allowed.

SECTION B-B

<u>PINTLE</u>

BILL OF MATERIAL

Item	Unit	Total
Anchor Bolts, ¾"	Each	32
Anchor Bolts, 1"	Each	<i>32</i>



USER NAME =	DESIGNED - ACS	REVISED
	CHECKED - LDG	REVISED
PLOT SCALE =	DRAWN - WJS	REVISED
PLOT DATE = 3/4/2024	CHECKED - LDG	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL	F.A. RTE
STRUCTURE NO. 060-0241	58
3111001011L 140: 000-0241	
SHEET NO 23 OF 31 SHEETS	

A.P. SECTION COUNTY TOTAL SHEETS NO. 366 51-1R MADISON 296 206 SN 060-0241 CONTRACT NO. 76A46

INTERIOR BEAM MOMENT TABLE (For Beam Line 6)							
		0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.6 Sp. 3	
Is	(in ⁴)	5,660	5,660	5,660	5,660	5,660	
$I_c(n)$	(in⁴)	17,108	17,108	17,463	17,463	17,750	
$I_c(3n)$	(in4)	12,680	12,680	13,062	13,062	13,380	
$I_{c}(cr)$	(in4)		8,203		8,360		
Ss	(in³)	414	414	414	414	414	
$S_c(n)$	(in³)	633	633	638	638	641	
Sc(3n)	(in³)	<i>575</i>	<i>575</i>	581	581	585	
Sc(cr)	(in³)		492		496		
5x	(in³)						
DC1	(k/')	0.943	1.020	1.020	1.089	1.089	
M _{DC1}	('k)	137	274.3	151	298	164.6	
DC2	(k/')	0.131	0.131	0.131	0.131	0.131	
M _{DC2}	('k)	18.9	35.8	18.8	35.8	18.9	
DW	(k/')	0.39	0.43	0.43	0.46	0.46	
M _{DW}	('k)	54.2	110.3	61.4	122.1	68	
LLDF		0.707	0.717	0.727	0.764	0.801	
M& + IM	('k)	518.9	498.5	558.5	531.2	587.9	
f; (Strength I)	(ksi)	0	0	0	0	0	
$M_U + \frac{1}{3} f_1 S_X$	('k)	1,184	1,425	1,282	1,530	1,360	
$\emptyset_f M_n$	('k)	3,298		3,307		3,325	
f _s DC1	(ksi)	4.0	8.0	4.4	8.6	4.8	
f _s DC2	(ksi)	0.4	0.9	0.4	0.9	0.4	
f _s DW	(ksi)	1.1	2.7	1.3	3.0	1.4	
f _s (4+1M)	(ksi)	9.8	12.2	10.5	12.9	11.0	
fr (Service II)	(ksi)	0	0	0	0	0	
f _s + ^{fl} / ₂ (Service II)	(ksi)	18.3	27.3	19.7	29.2	20.9	
Service II Resistance	(ksi)	47.5	47.5	47.5	47.5	47.5	
f _s + ^{fl} / ₃ (Strength I)	(ksi)	24.4	36.3	26.3	38.8	27.8	
$Ø_f F_n$	(ksi)		50.0		47.8		
Vf	(k)	26.9	32.0	32.0	32.0	28.0	

BEAM REACTION TABLE (For Beam Line 6)								
		W. Abut.	Pier 1	Pier 2	E. Abut.			
LLDF		0.888	0.948	0.948	1.001			
OCF		1.12	1.12	1.12	1.12			
R _{DC1}	(k)	16.4	58.2	62.9	19.2			
R _{DC2}	(k)	2.2	7.6	7.6	2.2			
Row	(k)	6.5	23.4	25.7	7.9			
R ½	(k)	60.3	115.3	122.4	68.0			
Rim	(k)	15.98	26.15	27.75	18.0			
RTotal (Strength I)(Impact)	(k)	166.6	364.9	389.4	189.2			
RTotal (Strength I)(No Impact)	(k)	138.6	319.1	340.9	157.6			

Is, Ss :	Non-composite moment of inertia and section modulus of the
	steel section used for computing $f_s(Total-Strength\ I,\ and$
	Service II) due to non-composite dead loads (in.4 and in.3).

 $I_{c}(n), S_{c}(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing $f_{s}(Total-Strength\ I, and\ Service\ II)$ in uncracked sections due to short-term composite live loads (in.4 and in.3).

 $I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing $f_s(Total-Strength\ I$, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.4 and in.3).

Ic(cr), Sc(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing fs (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.4 and in.3).

Sx: Section modulus about the major axis of a section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.3).

DC1: Un-factored non-composite dead load (kips/ft.).

M DC1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M DC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

Now: Un-factored moment due to long-term composite (superimposed

future wearing surface only) dead load (kip-ft.).

LLDF: Live Load Distribution Factor for moment and shear computed

according to Article 4.6.2.2 and further IDOT provisions. $M \not = + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

 M_U : Strength I load combination of factored design moments (kip-ft.). 1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{\perp} + 1M

f: Factored calculated flange lateral bending stress as calculated using Article 6.10.1.6 and as further simplified by IDOT provisions (ksi).

 $\phi_f M_n$: Factored nominal flexural resistance of the section determined as specified in Article 6.10.7.1 or A6 as applicable (kip-ft.).

 f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

MDCI / S_s

fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

 M_{DC2} / $S_c(3n)$ or M_{DC2} / $S_c(cr)$ as applicable. f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi). M_{DW} / $S_c(3n)$ or M_{DW} / $S_c(cr)$ as applicable.

fs (½ + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).

 $M_{L+IM}/S_c(n)$ or $M_{L+IM}/S_c(cr)$ as applicable.

 f_s + $f_\ell/2$ (Service II): Sum of stresses as computed below (ksi). f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (ℓ + IM) + $f_\ell/2$

Service II Resistance: Composite $(0.95R_hF_{yf})$ or noncomposite $(0.80R_hF_{yf})$ stress capacity according to Article 6.10.4.2 (ksi).

 f_s + $f_t/3$ (Strength I): Sum of stresses as computed below on non-compact sections (ksi).

 $1.25~(f_s~DC1+f_s~DC2)+1.5~f_s~DW+1.75~f_s~(\mbox{ℓ}+\mbox{IM})+f_{\mbox{I}}/3$ $\phi_f F_n$: Factored nominal flexural resistance of the section as

specified in Article 6.10.7.2 or 6.10.8 as applicable (ksi). V_f : Maximum factored shear range in span computed according

to Article 6.10.10.

OCF: Obtuse Correction Factor according to Article 4.6.2.2.3c or as

further simplified by IDOT provisions.

R_{DC1}: Un-factored reaction due to non-composite dead load (kip).

 R_{DC2} : Un-factored reaction due to long-term composite (superimposed

excluding future wearing surface) dead load (kip).

R_DW: Un-factored reaction due to long-term composite (superimposed

future wearing surface only) dead load (kip).
R 4 : Un-factored live load reaction (kip).

R_{IM}: Un-factored dynamic load allowance (impact) (kip).

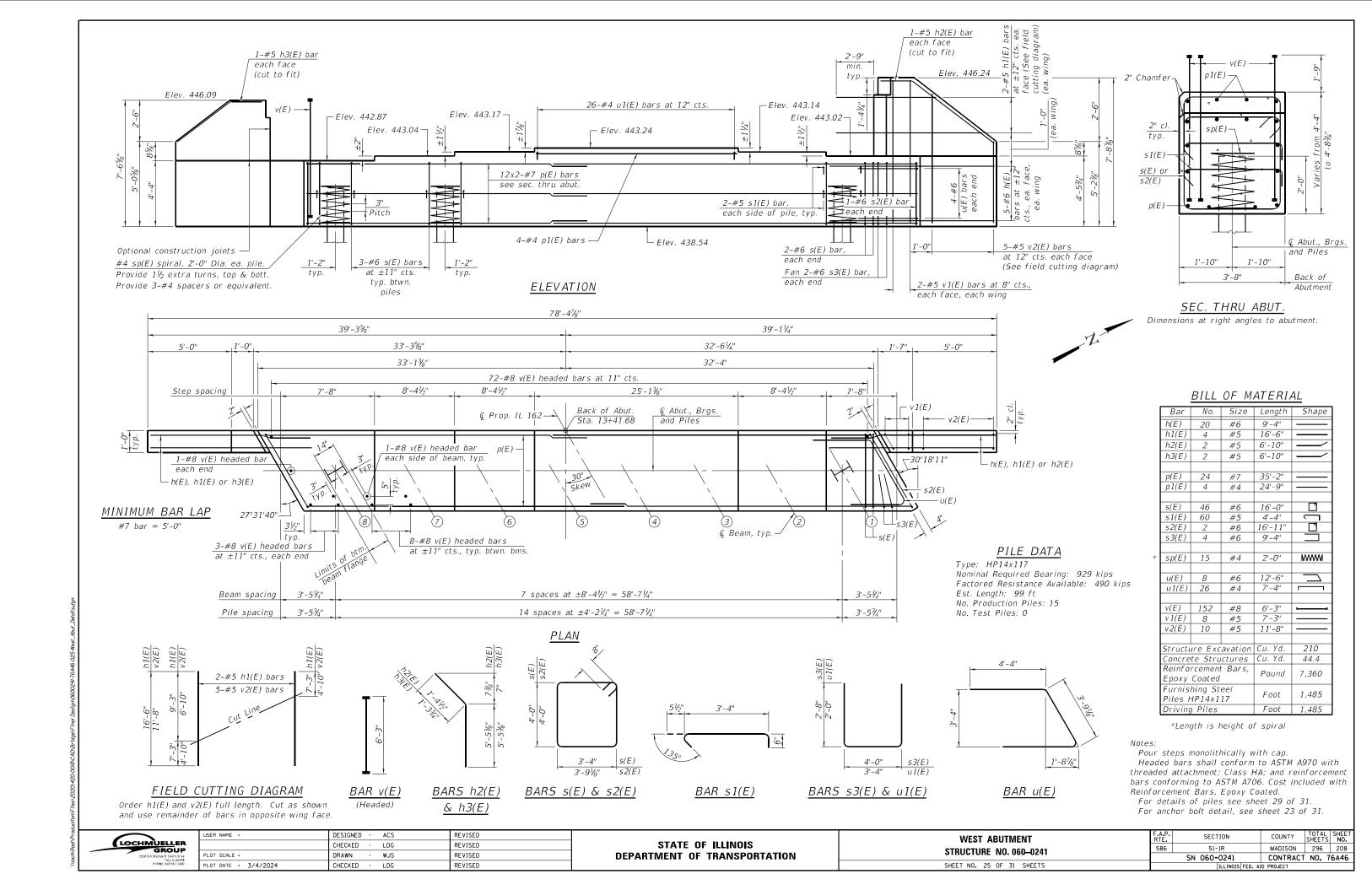
 R_{TM} : on-ractioned dynamic load anomalice (impact) (kip). R_{Total} (Strength I)(Impact): Strength I load combination of factored design reactions (kip).

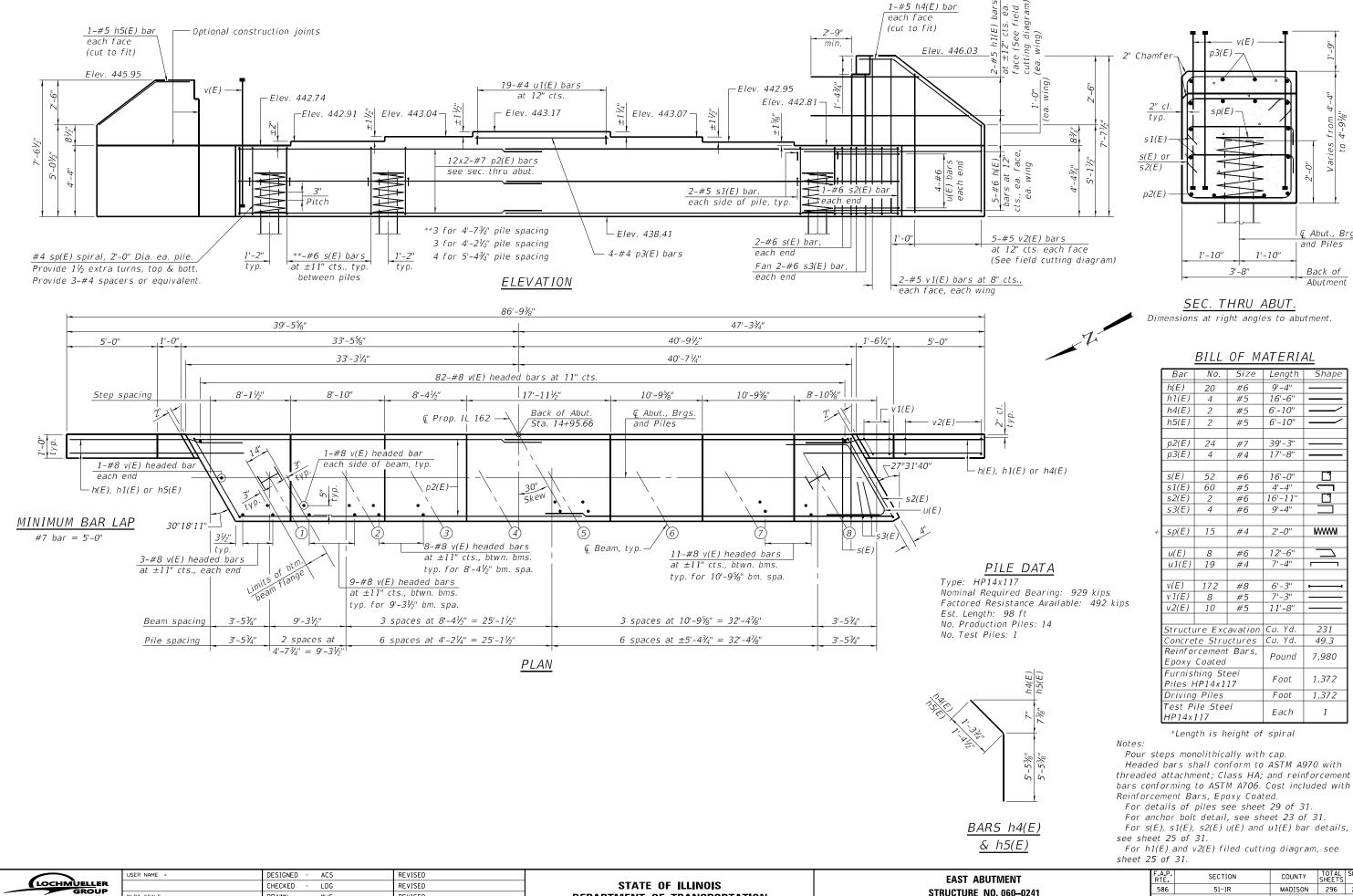
 R_{Total} (Strength I)(No Impact): 1.25 ($R_{DC1} + R_{DC2}$) + 1.5 R_{DW} + 1.75 ($R_{4} + R_{IM}$) Strength I load combination of factored design reactions, not

including dynamic load allowance (Impact) (kip). $1.25 (R_{DCI} + R_{DCZ}) + 1.5R_{DW} + 1.75 (R4)$

LOCHMUELLER	
GROUP	
1928 SrA Bradley R. Smith Drive	
Troy, IL 62294	

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	CHECKED - LDG	REVISED
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STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

CHECKED - LDG

WJS

I DG

DRAWN

CHECKED -

PLOT DATE = 3/4/2024

REVISED

REVISED

REVISED

COUNTY 586 51-1R MADISON 296 209 STRUCTURE NO. 060-0241 SN 060-0241 CONTRACT NO. 76A46 SHEET NO. 26 OF 31 SHEETS

p3(E) -

sp(E)

1'-10"

20

3'-8"

SEC. THRU ABUT.

#6

#5

#4

#6

#4

#4

#5

10

BILL OF MATERIAL

Size Length

9'-4"

16'-6"

6'-10"

6'-10"

39'-3"

17'-8"

16'-0"

4'-4"

16'-11"

9'-4"

2'-0"

12'-6"

7'-4"

6'-3"

7'-3"

11'-8"

Pound

Foot

Foot

Each

1'-10"

€ Abut., Brgs.

and Piles

Back of

Abutment

 \Box

WWW.

231

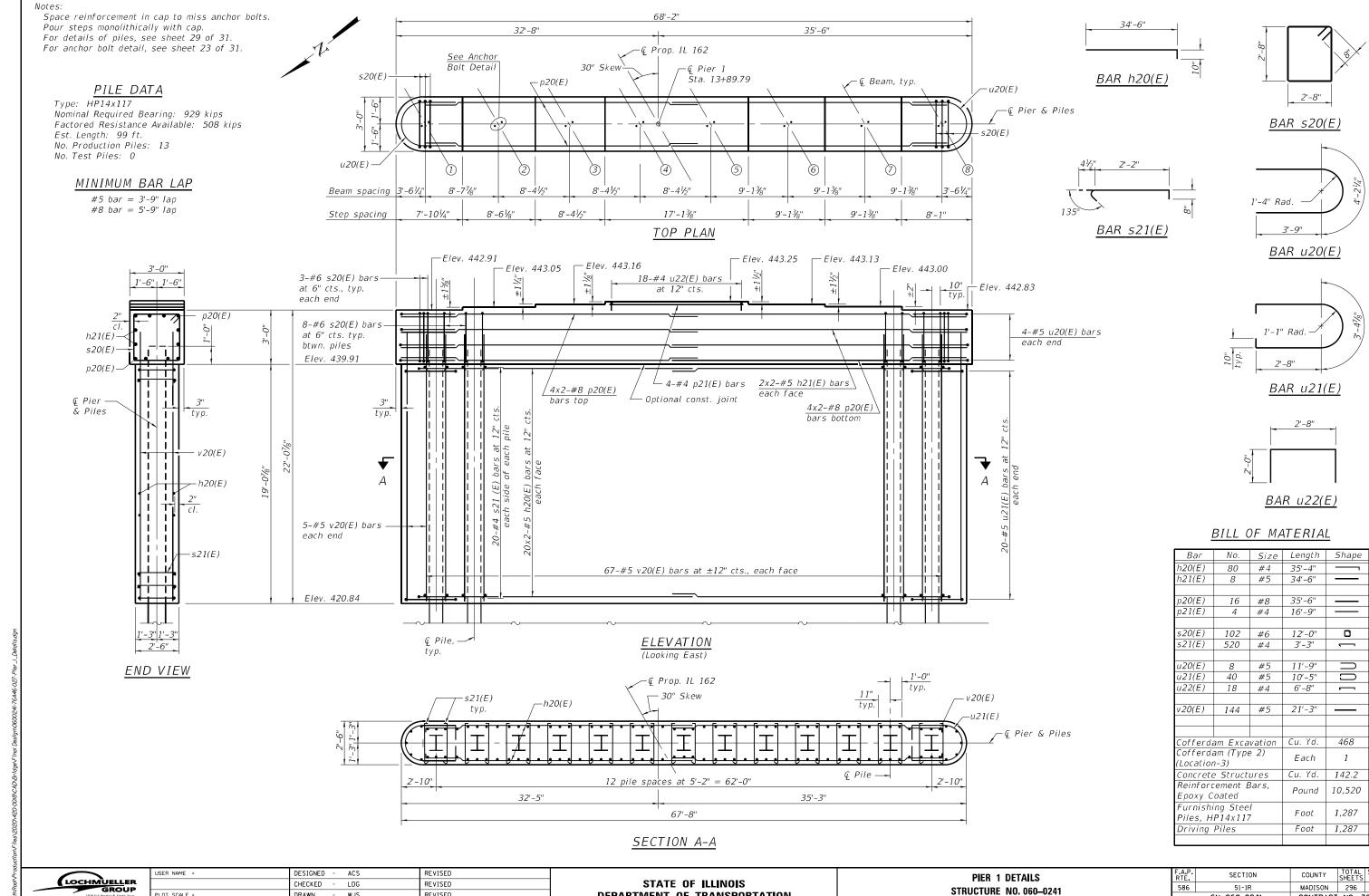
49.3

7,980

1,372

1,372

1

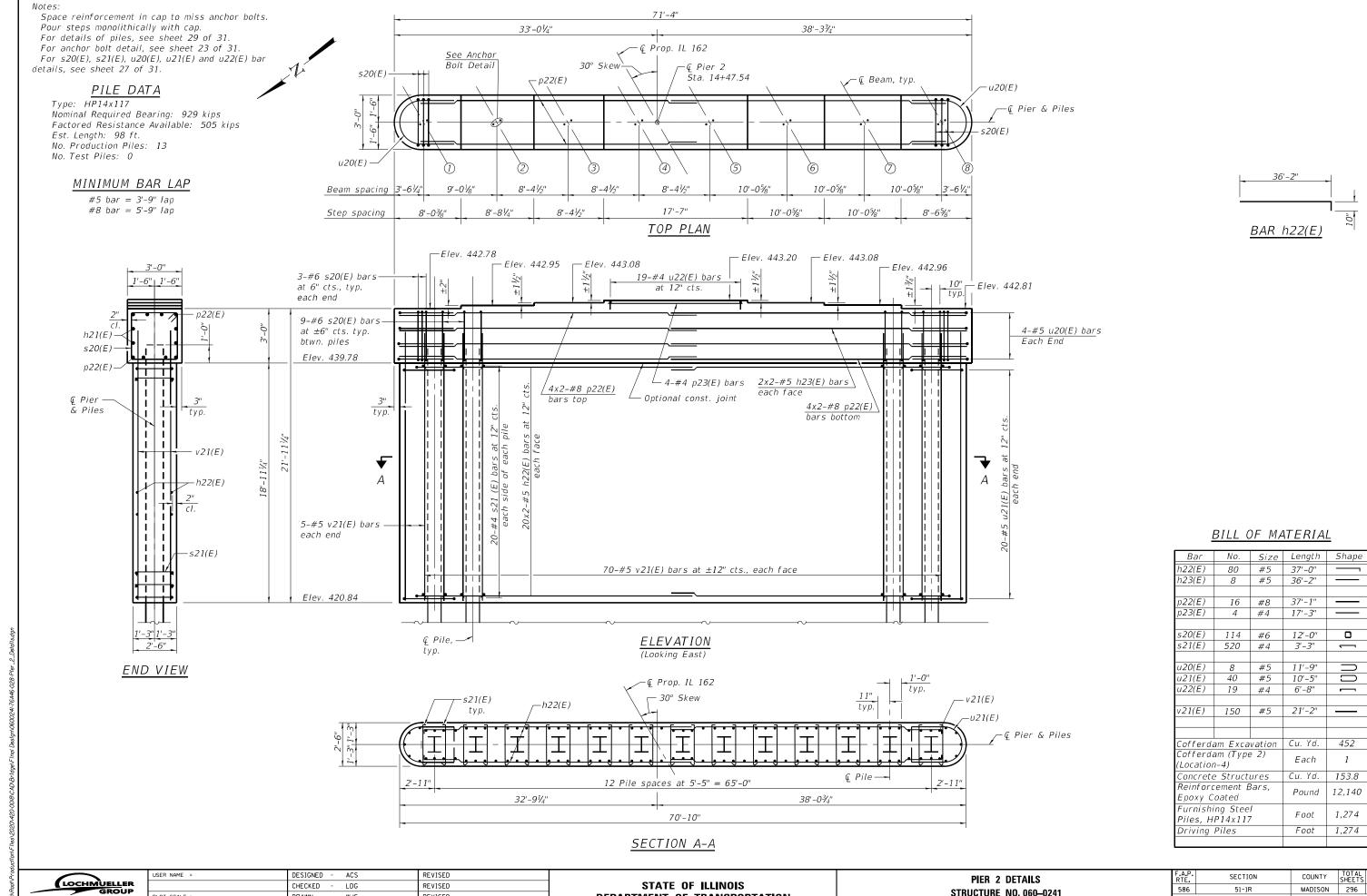


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PLOT DATE = 3/4/2024	CHECKED - LDG	REVISED

DEPARTMENT OF TRANSPORTATION

		PIEF	1	DE	ΓAI	LS	
STRUCTURE				NO	. 06	60-0241	
	SHEET	NO.	27	OF	31	SHEETS	

	RTE.	SECTION	COUNTY	SHEETS	NO.						
	586	51-1R	MADISON	296	210						
_		SN 060-0241	CONTRACT	NO. 7	6A46						
	ILLINOIS FED. AID PROJECT										

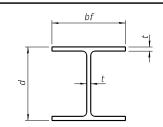


DRAWN WJS REVISED PLOT DATE = 3/4/2024 CHECKED I DG REVISED

DEPARTMENT OF TRANSPORTATION

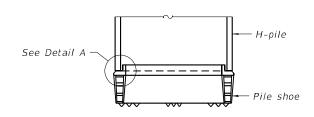
STRUCTURE NO. 060-0241 SHEET NO. 28 OF 31 SHEETS

TOTAL SHEE NO. 296 211 SN 060-0241 CONTRACT NO. 76A46

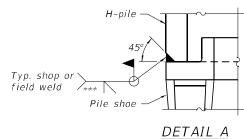


STEEL PILE TABLE

			1	
Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14½"	14 ⁷ / ₈ "	13/ ₁₆ "	30"
x102	14"	14¾"	¹ 1/ ₁₆ "	30"
x89	131/8"	14¾"	5/8"	30"
x73	13%"	145/8"	1/2"	30"
HP 12x84	12½"	121/4"	11/ ₁₆ "	24"
x74	12½"	121/4"	5/8"	24"
x63	12"	121/8"	1/2"	24"
x53	1 1 3/4"	12"	⁷ / ₁₆ "	24"
HP 10x57	10"	101/4"	%16"	24"
x42	9¾"	101/8"	7/ ₁₆ "	24"
HP 8x36	8"	8½"	½ ₁₆ "	18"

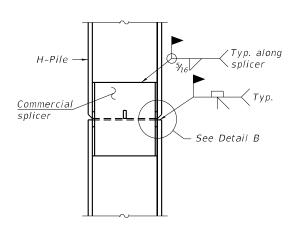


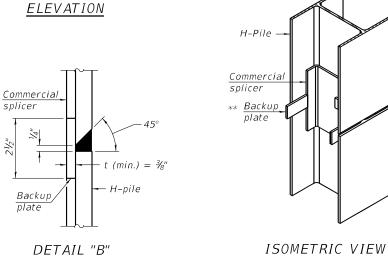
ELEVATION



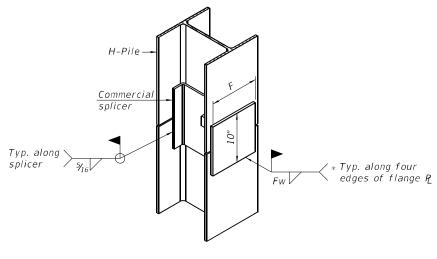
SHOE ATTACHMENT

The steel H-piles shall be according to AASHTO M270 Grade 50.





WELDED COMMERCIAL SPLICE

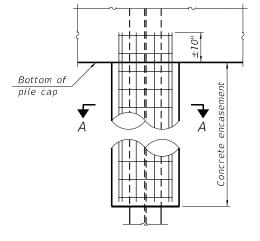


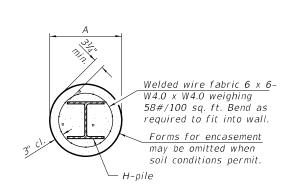
ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- $_*$ Interrupt welds $\frac{1}{4}$ " from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.

*** Weld size per pile shoe manufacturer (5/16" min.).



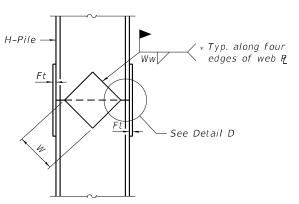


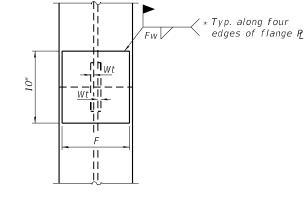
ELEVATION

SECTION A-A

INDIVIDUAL PILE CONCRETE ENCASEMENT

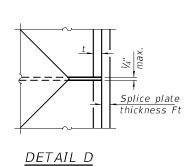
(when specified)





ELEVATION

END VIEW



Designation	F	Ft	Fw	W	Wt	Ww
HP 14×117	121/2"	1"	7/8"	73/4"	5/8"	1/2"
x102	121/2"	7/8"	3/4"	73/4"	5/8"	1/2"
x89	121/2"	3/4"	¹ ½ ₁₆ "	73/4"	5/8"	1/2"
x73	121/2"	5/8"	%16"	73/4"	5/8"	1/2"
HP 12x84	10"	7/8"	11/16"	6½"	5/8"	1/2"
x74	10"	7/8"	11/16"	6½"	5/8"	1/2"
x63	10"	5/8"	1/2"	6½"	1/2"	3/8"
x53	10"	5/8"	1/2"	6½"	1/2"	3/8"
HP 10x57	8"	3/4"	%16"	5½"	1/2"	3/8"
x42	8"	5/8"	%16"	51/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/ ₁₆ "	41/4"	1/2"	3/8"

WELDED PLATE FIELD SPLICE

F-HP

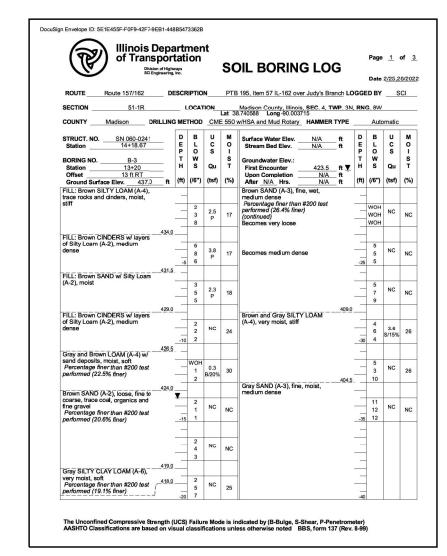
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

HP PILE DETAILS STRUGTERIETNUREDISO-0241 SHEET NO. 29 OF 31 SHEETS

TOTAL SHEE NO. SECTION COUNTY 586 51-1R MADISON 296 212 CONTRACT NO. 76A46 SN 060-0241



of Transport	tati	ner on		sc	OIL BORING LOG	Pa	ge <u>2</u>	of
JOI LITY IN THE ING.	iiio.					Da	te 2/25	,28/2
ROUTE Route 157/162 DE	SCR	IPTIO	N	PTB	195, Item 57 IL-162 over Judy's Branch LOG	GED I	BY	SCI
SECTION 51-1R		OCAT			Madison County, Illinois, SEC. 4, TWP. 3N, F 3.740588 Long-90.003715	RNG. 8	W	
COUNTY Madison DRILLING	MET	THOD	CME	550	w/HSA and Mud Rotary HAMMER TYPE	A	utomati	c
STRUCT. NO. SN 060-0241 Station 14+18.67 BORING NO. B-3 Station 13+20 Offset 13 ft RT	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Stream Bed Elev. N/A ft Groundwater Elev.: First Encounter 423.5 ft ▼ Upon Completion N/A ft	D E L P C T V H S	C S V Qu	
Gray SAND (A-3), fine, moist,	(11)	(,0,)	(tai)	(70)	Gray SILTY CLAY (A-6), moist,	, (/0) (เอเ	10
medium dense (continued)					stiff (continued)			
					Brown SILTY CLAY LOAM (A-6), moist, stiff, w/ sand deposits			
D		13	NC		-	- 3		
Becomes very dense	-45	14 17		NC	_	-65 4	B/201	
	1-					_		
390.0	2				370.0			
Gray SAND (A-2), fine to coarse, moist, medium dense, trace fine gravel	-				Gray CLAY (A-7), moist, very stiff			
387.5		14	1.5 B/20%	20	-	- 4		, 2
Gray SANDY CLAY LOAM (A-6), moist, stiff	-50	5	B/20%		-	-70 E	B/20	76
Gray SILTY CLAY (A-6), moist.	9				-			
stiff	2-				_	_		
	-	5	26		_	- 4		+
	-55	5 8	B/20%	23	_	5 -75 7	B/20	
	- Ja				_			
	0							
	9-					-		
	8	4			-	=		
Becomes brown, trace gravel	-60	4	1.2 B/20%	22	-	_		

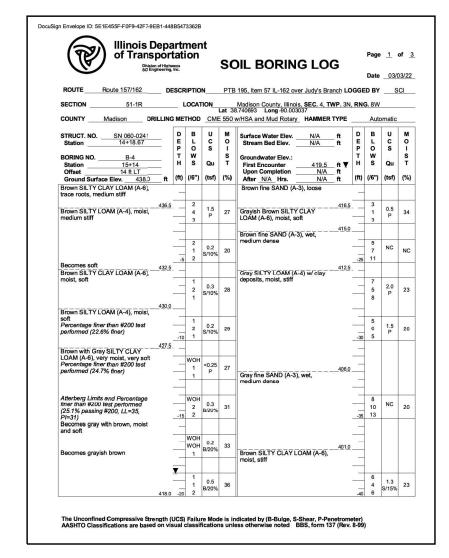
of Trans	porta on of Highways ngineering, inc.			sc	OIL BORING LOG	Page 3
ROUTE Route 157/162	DESC	RIPTIC	ON	PTB	195, Item 57 IL-162 over Judy's Branch LOGO	
SECTION 51-1R		LOCA	TION		Madison County, Illinois, SEC. 4, TWP. 3N, RI 3.740588 Long-90.003715	NG. 8W
COUNTY Madison DI	RILLING M	ETHOD	CME	E 550	w/HSA and Mud Rotary HAMMER TYPE	Autom
STRUCT. NO. SN 060-0241 Station 14+18.67		L	U C S	M 0 1	Surface Water Elev. N/A ft Stream Bed Elev. N/A ft	
BORING NO. B-3 Station 13+20 Offset 13 ft RT	= 1	w	Qu	S T	Groundwater Elev.: First Encounter 423.5 ft Upon Completion N/A ft	
Ground Surface Elev. 437.0 Gray CLAY (A-7), moist, very stiff	ft (f	(/6")	(tsf)	(%)	After N/A Hrs. N/A ft	
(continued)	-					
Gray CLAY LOAM (A-7), moist, hard w/ weathered clayey shale	355.0					
		12	2.6	27	•	
	_4	14 35 19	S/15%	21		
Gray CLAYEY SHALE	351.2	50/5.5 50/2.5	-	13		
Sampler Refusal at 87 feet. Borehole grouted upon completion.	350.0	50/4.5 50/1.5		13		
	_					
		90				
	-					
	-	7				
	_					
	-	1				
		95				
	_					
	_	1				
	_	\exists				
	-10					



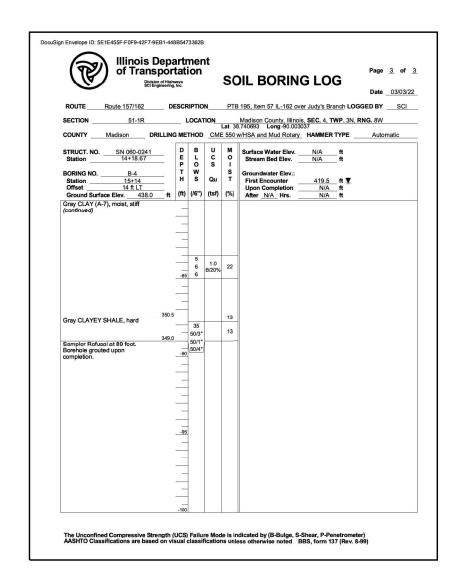
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PLOT SCALE =	DRAWN - WJS	REVISED
PLOT DATE = 3/4/2024	CHECKED - LDG	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORINGS Structure No. 060–0241		F.A.P. SECTION		TOTAL SHEETS	SHEE NO.
		51-1R	MADISON	296	213
		SN 060-0241	CONTRACT	NO.	76A46
SHEET NO. 30 OF 31 SHEETS		ILLINOIS FED. AI	D PROJECT		



Illinois Depa of Transpor	tati	on		sc	OIL BORING LOG			03/0	
ROUTE Route 157/162 DI	ESCR	IPTIO	N	РТВ	195, Item 57 IL-162 over Judy's Branch Li	OGGE	D BY	S	CI
SECTION 51-1R	_ L	OCA1	TION		Madison County, Illinois, SEC. 4, TWP. 38 3.740693	, RNG	. 8W		
COUNTY Madison DRILLING	3 ME	THOD			w/HSA and Mud Rotary HAMMER TYPE			omatic	
STRUCT. NO.	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. Ni/A ft	D E P T H	B L O W S	U C S Qu (tsf)	
Brown SILTY CLAY LOAM (A-6), moist, stiff (continued)	-	- 1	, ,		Gray CLAY (A-7), moist, stiff (continued)				ľ
	_				Gray SILTY CLAY LOAM (A-6), moist, stiff				
w/ fine sand and clay deposits	_45	3 4 6	1.8 P	25		-65	4 4 4	0.3 B/20%	
w/ fine aand deposit, no clay deposit	-50	4 3 4	1.5 P	27			2 4 5	1.3 B/20%	3
Becomes gray		2 4 5	0.9 S/15%	23	Becomes very stiff, w/ trace gravel		5 12 16	1.0 S/10%	
Gray CLAY (A-7), moist, stiff	=				Gray CLAY (A-7), moist, stiff	2			
	-60	5 4 7	1.9 B/20%	23		-80	WOH 4 4	0.9 B/20%	



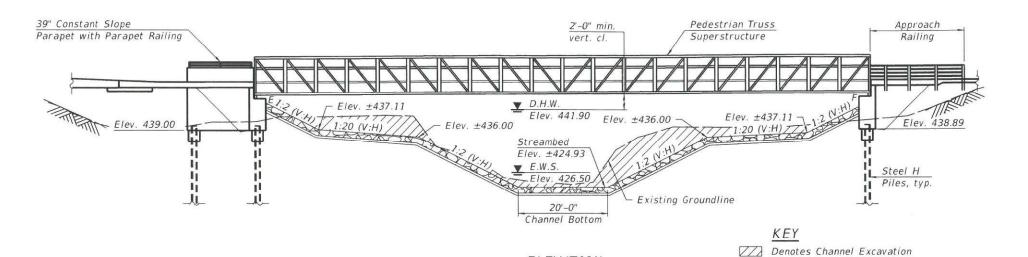
LOCHMUELLER
GROUP
1928 SrA Bradley R. Smith Drive
Troy, IL 62294
PHDNE: 618.667.1400

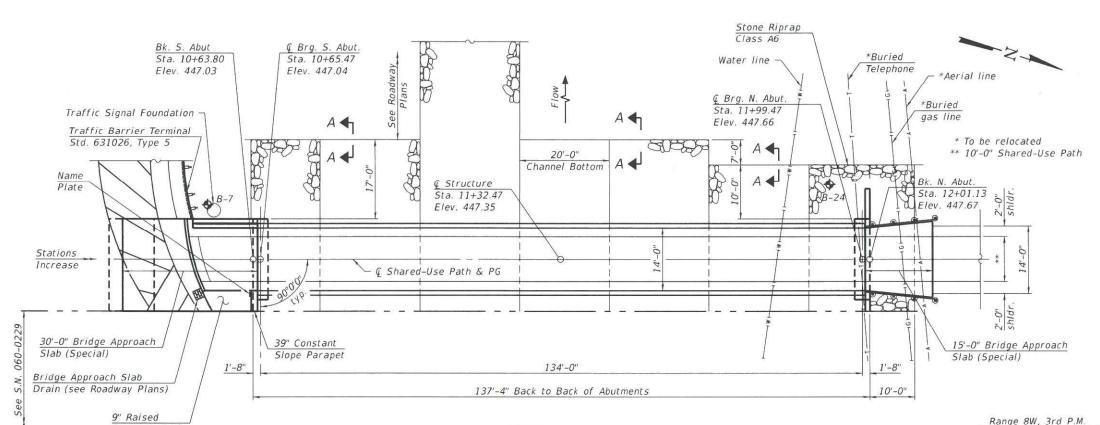
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PLOT DATE = 3/4/2024	CHECKED - LDG	REVISED

SOIL BORINGS STRUCTURE NO. 060–0241		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		51-1R	MADISON	296	214
		SN 060-0241	CONTRACT	NO. 7	76A46
SHEET NO. 31 OF 31 SHEETS		ILLINOIS FED	. AID PROJECT		

Benchmark: Cut "p", with +, on top of parapet wall at Northwest corner of IL-157 Bridge over Judys Branch (S.N. 060-0087). Elev. 449.226.

Existing Structure: None.





ELEVATION

Note: For Section A-A, see sheet 2 of 19.

Concrete Curb

APPROVED
For Structural Adequacy Only
Engineer of Bridges & Structures

PLAN



(See Roadway Plans)

Glen Rie 5 Br. Maryville Burdick Br. Maryville De CATION SKETCH

INDEX OF SHEETS

- 1. General Plan and Elevation
- 2. General Data and Details
- 3. Top of Slab Elevations
- 4. Top of South Bridge Approach Slab Elevations
- 5. Top of North Bridge Approach Slab Elevations
- 6-8. South Bridge Approach Slab Details
- 9. North Bridge Approach Slab Details
- 10. Parapet Railing Details
- 11. Approach Railing Details
- 12. Pedestrian Truss Details
- 13-14. South Abutment Details
- 15. North Abutment Details
- 16. Steel H-Pile Details
- 17-19. Boring Logs

-0.	70% 8.	.33%	1.60%	4.90%	0.47%	
446.55	Sta. 10+49.94 446.52	Sta. 10+50.54	446.5/ Sta. 10+55.54	446.65 Sta. 10+63.30	447.03	Sta. 12+45.00 447.88
Elev.	1.P.1. Elev.	-	/.P.I.	EIev. /.P.I.	Elev.	1.P.C.

PROFILE GRADE

(Along & Shared-Use Path)

<u>LOADING H-10</u> Live Load = 90 psf uniform load

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications, 9th. Edition.

2009 AASHTO LRFD Guide Specifications For Design of Pedestrian Bridges

DESIGN STRESSES

FIELD UNITS

f'c = 4,000 psi (Superstructure) f'c = 3,500 psi (Substructure)

fy = 60,000 psi (Reinforcement)

PRE-ENGINEERED BRIDGE UNITS fy = 50,000 psi (M270 Grade50W)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 2
Design Spectral Acceleration at 1.0 sec. (SD1) = 0.23g
Design Spectral Acceleration at 0.2 sec. (SDS) = 0.53g

Soil Site Class = D

GENERAL PLAN & ELEVATION

SHARED-USE PATH OVER JUDYS BRANCH

F.A.P. ROUTE 586

SECTION 51-1R

MADISON COUNTY

STATION 11+32.47

STRUCTURE NO. 060-7003

ENGINEERING WWW. FUHRMANN-ENG.COM

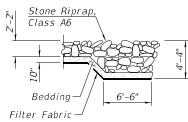
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NN		CHECKED -	E.M. Lagemann	REVISED -
NG	PLOT SCALE =	DRAWN -	A.H. Morinaga Mansilla	REVISED -
OM	PLOT DATE =	CHECKED -	E.M. Lagemann	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

A film forming Concrete Sealer shall be applied to the horizontal surfaces of the bridge seats of both Abutments. A penetrating Concrete Sealer shall be applied to the exposed vertical surfaces of both abutments, including the backwall and front face of abutments. Concrete Sealers at the abutments shall be applied

205.06 of the Standard Specifications.

Bridge Deck Grooving shall be applied only to the roadway portion of the South Bridge Approach Slab.



SECTION A-A

Note:

Reinforcement bars designated (E) shall be epoxy coated. Slipforming of the parapets is not allowed.

prior to setting bearings and beams.

Granular Backfill behind the abutments shall be compacted according to Article

For location of section A-A, see sheet 1 of 19.

SECTION THRU ABUTMENT

Notes:

* Included in the cost of Pipe

Underdrains for Structures.

-Granular Backfill for Structures

Bridge Approach Slab (Special)

* Geotechnical Fabric for French Drains *Drainage Aggregate

⊧4" Ø Perforated

pipe underdrain

<u>Geocomposit</u>e

Wall Drain

Steel H Piles

→ Bk. of Abut

Section Thru North Abutment shown, South Abutment similar. All drainage system components shall extend 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

WATERWAY INFORMATION

Excavation is paid for

as Structure Excavation

DESIGN SCOUR ELEVATION TABLE Event / Limit Design Scour Elevations (ft. S. Abut. N. Abut. Item 113 Q100 439.00 438.92 439.00 438.92 0200 439.00 438.92 Check 439.00 438.92

Concrete Deck

[/]Stone Riprap,

Class A6

Stringer

End Post

U U

& Brg., Abut. & Piles

Concrete

Prefabricated

Truss

Drainage Area = 8.6 sq. mi. Existing Overtopping Elev. = 445.83 at Sta. 582+50.00 Proposed Overtopping Elev. = 446.42 at Sta. 581+42.36									
Flood Event	Freq.	Discharge, Q	Waterway 0	pening – Ft²	Natural	Head	- Ft.	Headwater E	levation – Ft.
Frood Event	Yr.	C.F.S.	Existing	Proposed	H.W.E Ft.	Existing	Proposed	Existing	Proposed
Design	50	5750	883	1146	441.9	0.5	0.3	442.4	442.2
Base	100	6590	941	1214	442.4	1.5	0.4	443.9	442.8
Scour Design Check	200	7600	987	1287	443.0	1.3	0.6	444.3	443.7
Overtop Existing	N/A								
Overtop Proposed	N/A								
Max. Calc.	500	8920	998	1367	443.7	1.3	1.6	445.0	445.3

10 Year Velocity Through Existing Bridge = N/A

10 Year Velocity Through Proposed Bridge = 4.46 ft/s

7 3 7 7 12 2 1 2 2	0, ,,,,		=	
ITEM	UNIT	SUPER	SUB	TOTAL
Stone Riprap, Class A6	Sq. Yd.			532
Filter Fabric	Sq. Yd.			563
Structure Excavation	Cu. Yd.		42	42
Concrete Structures	Cu. Yd.		39.6	39.6
Concrete Superstructure	Cu. Yd.	43.4		43.4
Bridge Deck Grooving	Sq. Yd.	34		34
Concrete Encasement	Cu. Yd.		8.6	8.6
Protective Coat	Sq. Yd.	317		317
Concrete Superstructure (Approach Slab)	Cu. Yd.	35.0		35.0
Reinforcement Bars, Epoxy Coated	Pound	12,980	5,270	18,250
Parapet Railing	Foot	13.3		13.3
Furnishing Steel Piles HP14x117	Foot		935	935
Driving Piles	Foot		935	935
Name Plates	Each			1
Granular Backfill For Structures	Cu. Yd.		91	91
Concrete Sealer	Sq. Ft.		127.3	127.3
Geocomposite Wall Drain	Sq. Yd.		44	44
Pipe Underdrains For Structures 4"	Foot		95	95
Pedestrian Truss Superstructure	Sq. Ft.	1,920.7		1,920.7
Wood Rail	Foot	32.0		32.0

TOTAL BILL OF MATERIAL

STA. 11+32.47 BUILT 202X BY STATE OF ILLINOIS F.A.P. Rt. 586 Sec. 51-1R LOADING H-10 STR. NO. 060-7003

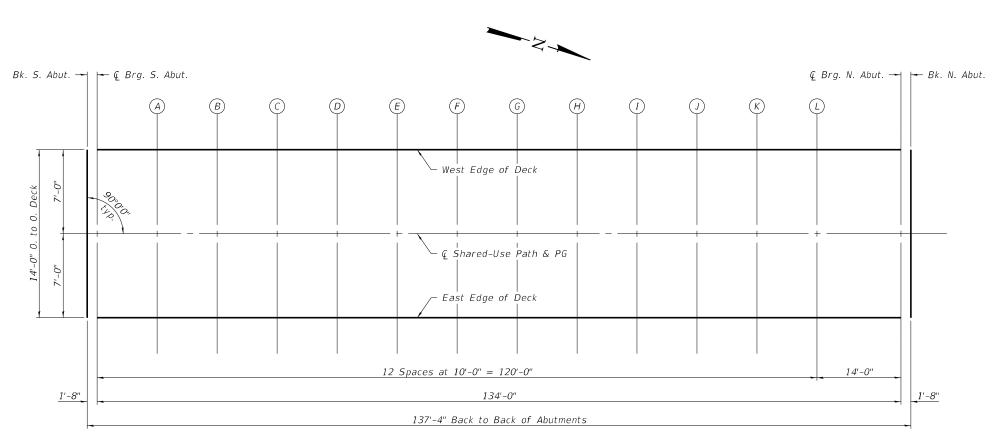
> NAME PLATE See Std. 515001

FUHRMANN ENGINEERING WWW. FUHRMANN-ENG .COM PLOT DATE =

DESIGNED - A.H. Morinaga Mansilla REVISED -CHECKED - E.M. Lagemann REVISED -A.H. Morinaga Mansilla REVISED CHECKED - E.M. Lagemann REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** **GENERAL DATA AND DETAILS** STRUCTURE NO. 060-7003 SHEET 2 OF 19 SHEETS

SECTION COUNTY 51-1R MADISON 296 216 CONTRACT NO. 76A46 ILLINOIS FED. AID PROJECT



Prefabricated pedestrian truss manufacturer shall determine Theoretical Deck Elevations Adjusted for Dead Load Deflection and take into account the concrete deck when determining bridge

<u>PLAN</u>

WEST EDGE OF DECK

Location	Station	Offset	Theoretical Grade Elevations
Bk. S. Abut.	10+63.80	-7.00	446.93
ℚ Brg. S. Abut.	10+65.47	-7.00	446.94
A B C D E F G H I J K L	10+75.47 10+85.47 10+95.47 11+05.47 11+15.47 11+25.47 11+35.47 11+45.47 11+55.47 11+65.47 11+75.47	-7.00 -7.00 -7.00 -7.00 -7.00 -7.00 -7.00 -7.00 -7.00 -7.00 -7.00	446.98 447.03 447.08 447.12 447.17 447.22 447.26 447.31 447.36 447.40 447.45 447.50
ℚ Brg. N. Abut.	11+99.47	-7.00	447.56
Bk. N. Abut.	12+01.13	-7.00	447.57

© SHARED-USE PATH & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations
Bk. S. Abut.	10+63.80	0.00	447.03
₡ Brg. S. Abut.	10+65.47	0.00	447.04
A B C D E F G H I J	10+75.47 10+85.47 10+95.47 11+05.47 11+15.47 11+25.47 11+35.47 11+45.47 11+55.47	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	447.09 447.13 447.18 447.23 447.27 447.32 447.37 447.42 447.46 447.51
K L	11+75.47 11+85.47	0.00 0.00	447.56 447.60
€ Brg. N. Abut.	11+99.47	0.00	447.67
Bk. N. Abut.	12+01.13	0.00	447.68

EAST EDGE OF DECK

Location	Station	Offset	Theoretica Grade Elevations
Bk. S. Abut.	10+63.80	7.00	447.14
ℚ Brg. S. Abut.	10+65.47	7.00	447.15
A B C D E F G H I J K L	10+75.47 10+85.47 10+95.47 11+05.47 11+15.47 11+25.47 11+35.47 11+45.47 11+55.47 11+65.47 11+75.47	7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00	447.19 447.24 447.29 447.33 447.43 447.43 447.52 447.57 447.61 447.66 447.71
ℚ Brg. N. Abut.	11+99.47	7.00	447.77
Bk. N. Abut.	12+01.13	7.00	447.78

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350	USER NAME =	DESIGNED -	E.M. Lagemann	REVISED -
V		CHECKED -	T.S. Friederich	REVISED -
G	PLOT SCALE =	DRAWN -	E.M. Lagemann	REVISED -
	PLOT DATE =	CHECKED -	T.S. Friederich	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS STRUCTURE NO. 060-7003		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
		51-1R		MADISON	296	217
31100101L 110: 000-1003				CONTRA	CT NO. 7	76A46
SHEET 3 OF 19 SHEETS		ILLINOIS	FED. A	D PROJECT		

WEST EDGE OF APPROACH SLAB

Location	Station	0ffset	Theoretical Grade Elevations
S. End S. Appr. Slab	10+34.80	-9.00	446.35
A	10+39.80	-9.00	446.38
В	10+42.75	-9.00	446.39
С	10+48.54	-9.00	446.42
D	10+49.12	-9.00	446.46
Ε	10+55.54	-9.00	446.52
F	10+59.80	-9.00	446.72
N. End S. Appr. Slab	10+64.80	-9.00	446.90

INSIDE FACE OF PARAPET

Location	Station	Offset	Theoretical Grade Elevations
S. End S. Appr. Slab	10+34.80	-7.00	446.39
A B C D E F	10+39.80 10+43.27 10+48.69 10+49.27 10+55.54 10+59.80	-7.00 -7.00 -7.00 -7.00 -7.00 -7.00	446.41 446.43 446.45 446.49 446.55 446.75
N. End S. Appr. Slab	10+64.80	-7.00	446.93

Location	Station	Offset	Theoretical Grade Elevations
S. End S. Appr. Slab	10+34.80	0.00	446.47
A B C D E F	10+39.80 10+45.65 10+49.94 10+50.54 10+55.54 10+59.80	0.00 0.00 0.00 0.00 0.00 0.00	446.51 446.55 446.52 446.57 446.65 446.86
N. End S. Appr. Slab	10+64.80	0.00	447.04

INSIDE FACE OF CURB

Location	Station	Offset	Theoretical Grade Elevations		
S. End S. Appr. Slab	10+34.80	7.00	446.51		
A B C D E F	10+39.80 10+48.99 10+52.40 10+53.04 10+55.54 10+59.80	7.00 7.00 7.00 7.00 7.00 7.00	446.53 446.58 446.54 446.58 446.76 446.96		
N. End S. Appr. Slab	10+64.80	7.00	447.14		

EAST EDGE OF APPROACH SLAB

Location	Station	0ffset	Theoretical Grade Elevations
S. End S. Appr. Slab	10+34.80	11.55	446.53
A	10+39.80	11.55	446.55
B	10+51.75	11.55	446.59
C	10+54.76	11.55	446.55
D	10+55.44	11.55	446.59
E	10+55.54	11.55	446.76
F	10+59.80	11.55	446.96
N. End S. Appr. Slab	10+64.80	11.55	447.14

S. End S. Appr. Slab	5'-0"	2'-11 ³ / ₈ " 5'-9	3½" 7" -	6'-5"	4'-31/8"	5'-0"	N. End S. Appr. Slab
	(,	A) B			E F Edge of Approa	ach Slab	
2'-0"				/	 		
70"		R	65.00' R =	45.00' = 44.42'	Inside Fa	ace of Parapet	~ ~
20'-6%" O. to O. Approach Slab	2000,		1.5	7"		<u> </u>	
%" 0. to 0. Ap			/ //		Ç Shared	d-Use Path & PG	
20'-6%					_ Inside Fa	ace of Curb	
4-6%"	– East Ed	ge of Approach Slab			 		
-	5'-0"	11'-1		3'-01%" 81%"	_1½" 4'-3½"	5'-0"	
ļ			30'-0"			-	
			<u>PLAN</u>	•			

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	USER NAME =	DESIGNED	-	E.M. Lagemann	REVISED -	
N		CHECKED	-	T.S. Friederich	REVISED -	
G	PLOT SCALE =	DRAWN	-	E.M. Lagemann	REVISED -	
ı	PLOT DATE =	CHECKED	-	T.S. Friederich	REVISED -	

WEST EDGE OF APPROACH SLAB

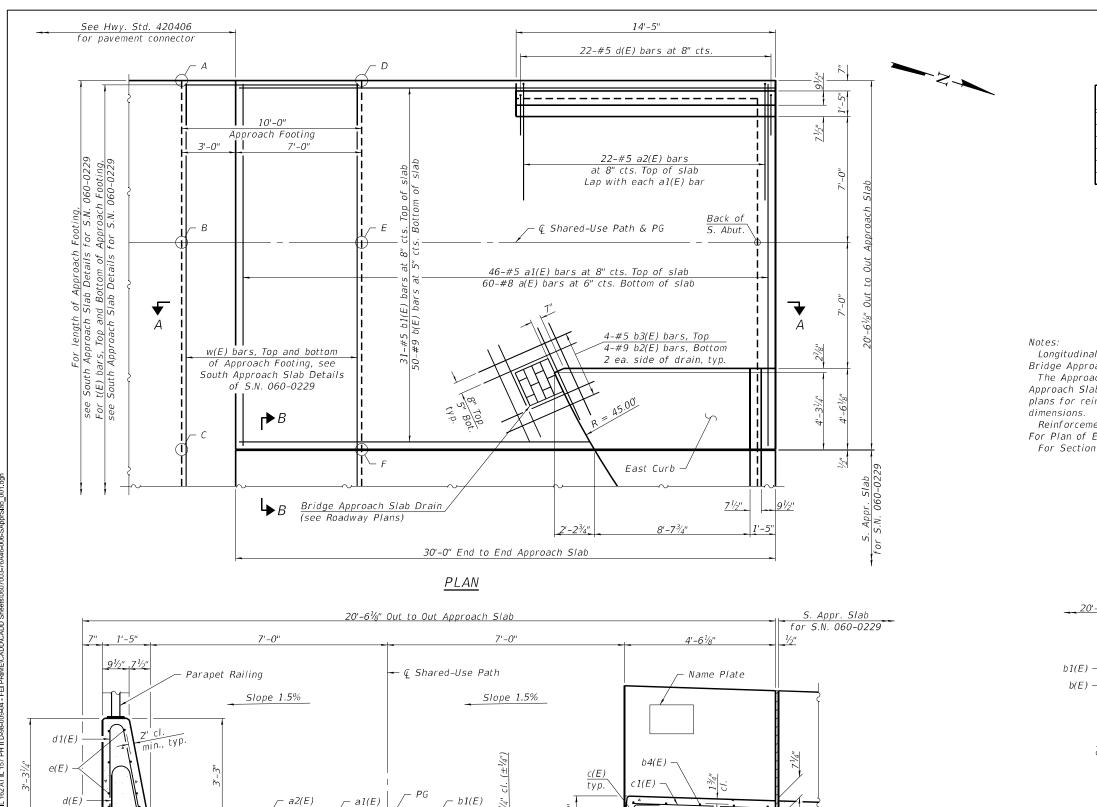
Location	Station	Offset	Theoretical Grade Elevations
S. End N. Appr. Slab	12+00.13	-7.00	447.57
A B	12+05.13 12+10.13	-7.50 -8.00	447.58 447.60
N. End N. Appr. Slab	12+15.13	-8.50	447.62

<u>Ç SHARED-USE PATH & PROFILE GRADE</u>

Location	Station	Offset	Theoretical Grade Elevations
S. End N. Appr. Slab	12+00.13	0.00	447.67
A B	12+05.13 12+10.13	0.00 0.00	447.70 447.72
N. End N. Appr. Slab	12+15.13	0.00	447.74

EAST EDGE OF APPROACH SLAB

Location	Station	Offset	Theoretica Grade Elevations
S. End N. Appr. Slab	12+00.13	7.00	447.78
A B	12+05.13 12+10.13	7.50 8.00	447.81 447.84
N. End N. Appr. Slab	12+15.13	8.50	447.87



TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

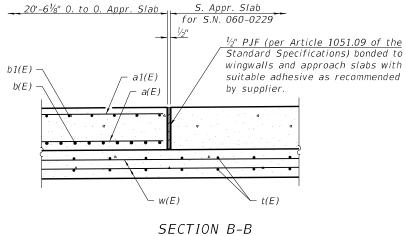
Point/ Location	Station	Offset	Тор	Bottom
А	10+31.80	-9.00 ft.	445.09	444.26
В	10+31.80	0.00 ft.	445.20	444.37
С	10+31.80	11.55 ft.	445.27	444.44
D	10+41.80	-9.00 ft.	445.14	444.31
Ε	10+41.80	0.00 ft.	445.27	444.44
F	10+41.80	11.55 ft.	445.30	444.47

Longitudinal and Transverse reinforcement shall be field cut around the Bridge Approach Slab Drain. A minimum clearance of 2" shall be provided.

The Approach Footing is a continuous footing supporting the South Approach Slabs of both this structure and S.N. 060-0229. See S.N. 060-0229 plans for reinforcement and concrete quantities, and Approach Footing dimensions.

Reinforcement details for the East Curb and Parapet not shown for clarity. For Plan of East Curb, see sheet 7 of 19.

For Section A-A, see sheet 7 of 19.



CROSS SECTION - NEAR ABUTMENT

a(E)

(Looking North)

.: ::		USER NAME =	DESIGNED -	-	E.M. Lagemann	REVISED	-
ME	FUHRMANN ENGINEERING		CHECKED -	-	A.H. Morinaga Mansilla	REVISED	-
		PLOT SCALE =	DRAWN -	-	E.M. Lagemann	REVISED	-
빌	WWW. FUHRMANN-ENG .COM	PLOT DATE =	CHECKED -	-	A.H. Morinaga Mansilla	REVISED	-

b(E)

SOUTH	BRIDGE A	4PF	ROA	4CH	I SLAB DETAILS I	
STRUCTURE NO. 060-7003						
	CHEET	c	ΩE	10	CHEETE	

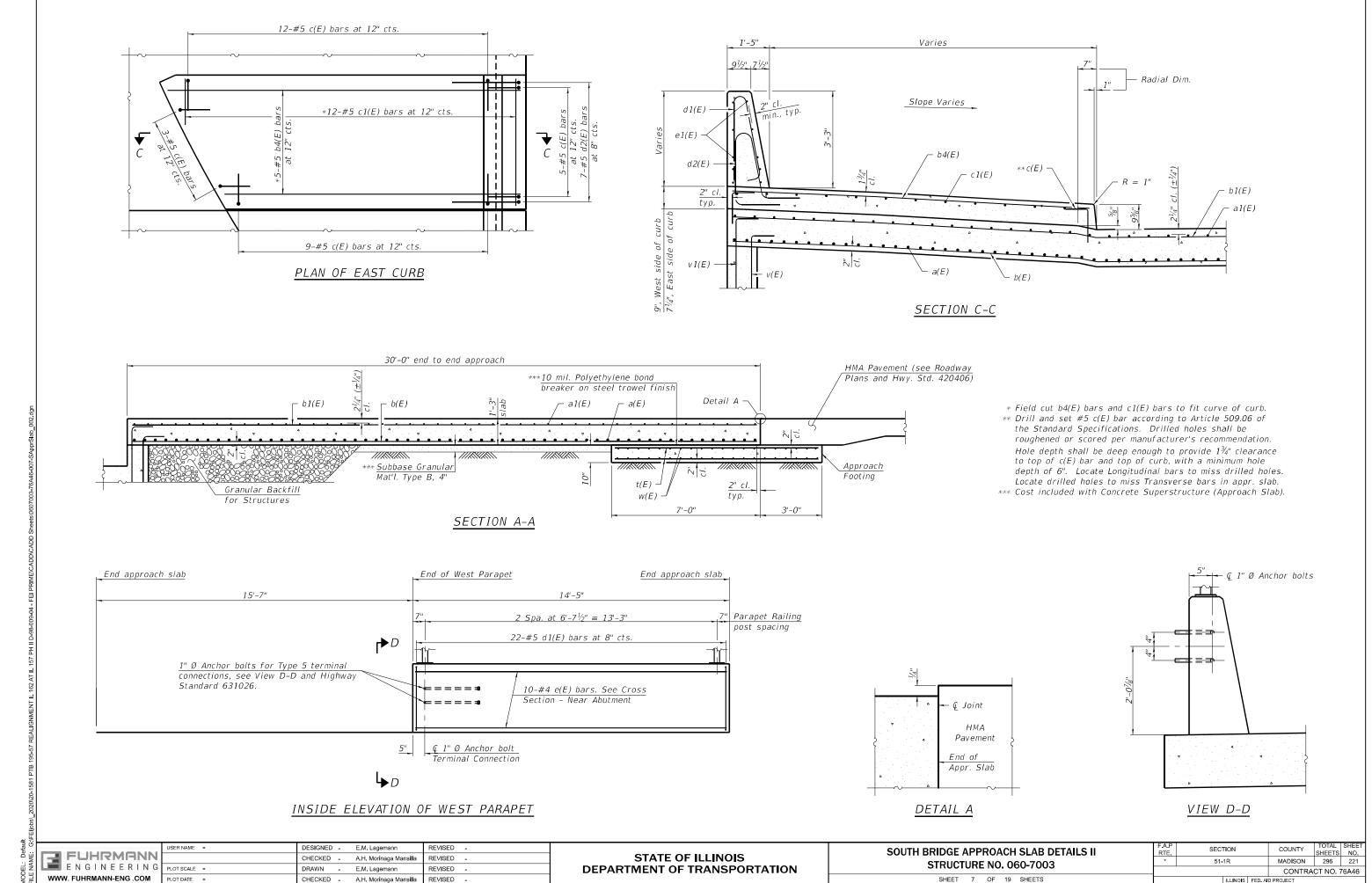
½" PJF (per Article 1051.09 of the Standard Specifications) bonded to wingwalls and approach slabs with suitable adhesive as recommended

by supplier.

		nioto	EED 41	CONTRA D PROJECT	CTNO.	/ bA46
				0011701	OT 110	
*	51-	51-1R		MADISON	296	220
RTE.	SEC	SECTION		COUNTY	SHEETS	
F.A.P					TOTAL	SHEE

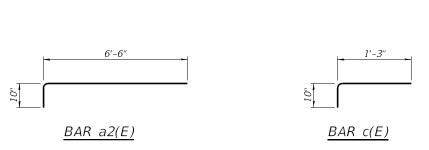
v5(E)

v6(E)

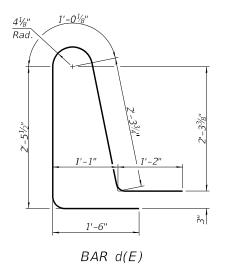


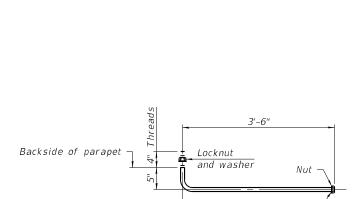
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INSIDE ELEVATION OF EAST PARAPET



* Cost included with Concrete Superstructure (Approach Slab).





*1" Ø ANCHOR BOLT

(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications)

Notes:

11½'

 $BAR \ d1(E)$

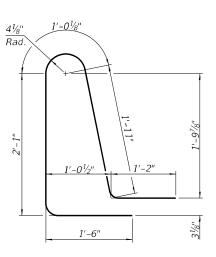
Parapet and curb concrete shall be paid for as Concrete Superstructure.

Approach slab shall be paid for as Concrete Superstructure (Approach Slab).

Approach footing concrete shall be paid for as Concrete Structures. Quantity is included in S.N. 060-0229 plans.

Approach footing reinforcement quantity is included in S.N. 060-0229 plans. The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf. Cost of excavation for approach footing included with Concrete Structures. Quantity is included in S.N. 060-0229 plans.

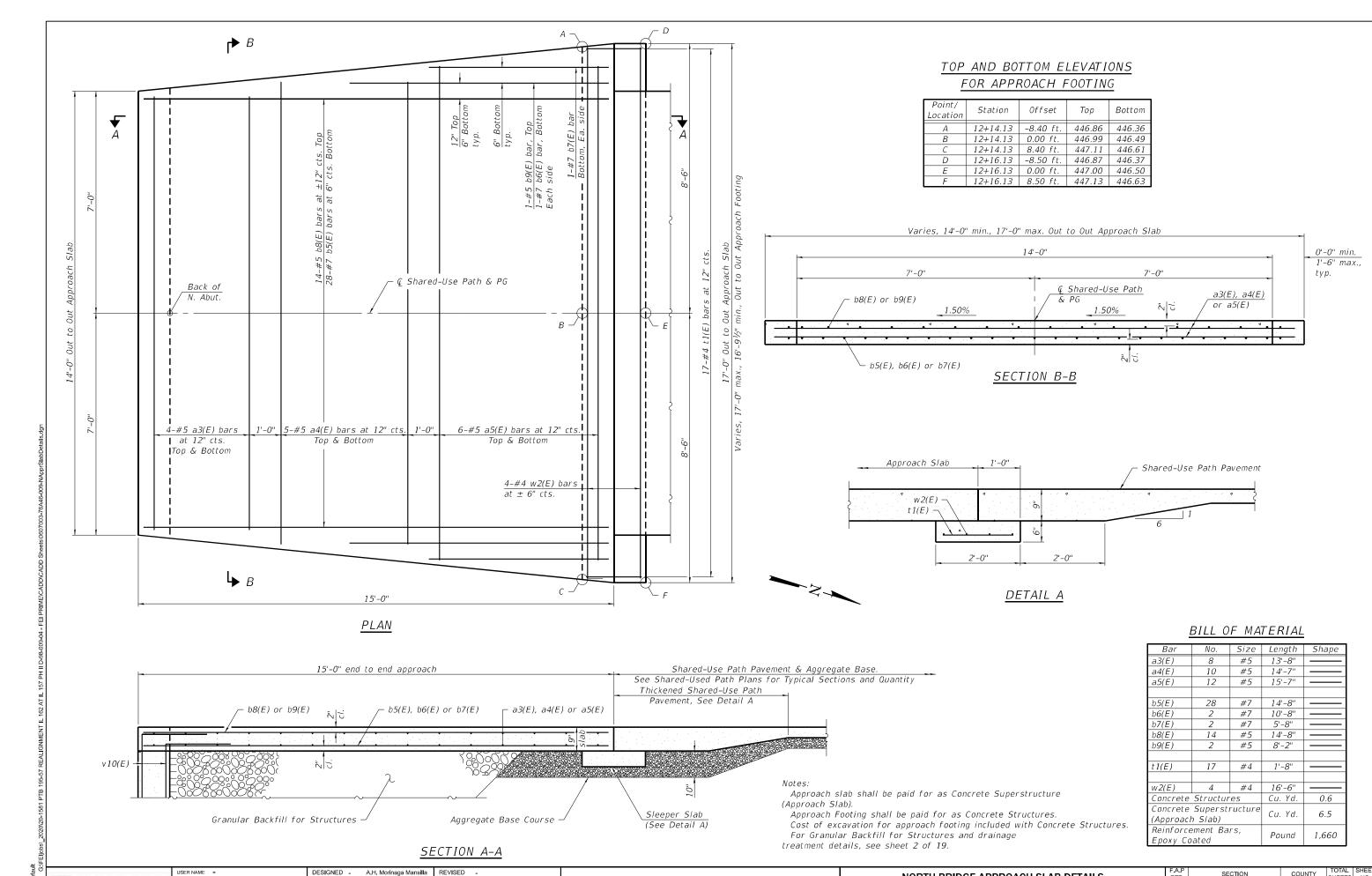
For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 19.



BAR d2(E)

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	60	#8	20'-2"	
a1(E)	46	#5	20'-2"	
a2(E)	22	#5	7'-4"	
b(E)	50	#9	29'-8"	
b1(E)	31	#5	29'-8"	
b2(E)	8	#9	5'-8"	
b3(E)	8	#5	5'-8"	
b4(E)	5	#5	12'-0"	
c(E)	29	#5	2'-1"	_
c1(E)	12	#5	4'-2"	
d(E)	22	#5	8'-6"	<u> </u>
d1(E)	29	#5	6'-5"	Ŋ
d2(E)	7	#5	7'-8"	<u>\</u>
e(E)	10	#4	14'-1"	
e1(E)	10	#4	4'-2"	
Concrete	e Superst	ructure	Cu. Yd.	5.8
Bridge L	Deck Groo	ving	Sq. Yd.	34
	Superst.	ructure	Cu. Yd.	28.5
(Approac			ca. ra.	20.5
	ement Ba	rs,	Pound	11,320
Ероху С	oated			/



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FUHRMANN ENGINEERING

CHECKED -

E.M. Lagemann

CHECKED - E.M. Lagemann

A.H. Morinaga Mansilla

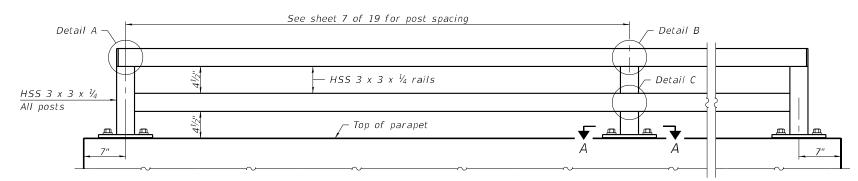
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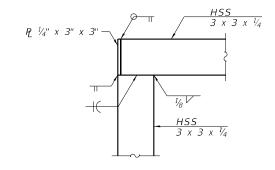
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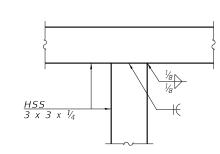
REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION NORTH BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 060-7003

SHEET 9 OF 19 SHEETS







ELEVATION PARAPET RAILING

min.

(Inside face)

 $\frac{5}{8}$ " Ø x 2" hex. hd. H.S. bolts with

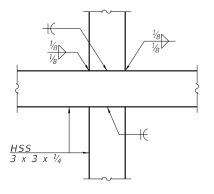
 $1\frac{1}{2}$ " x $1\frac{1}{4}$ " x $\frac{5}{16}$ " R washer

 $\frac{1"}{AASHT0}$ M270 G50 - tap for $\frac{5}{8}$ " Ø H.S. bolts

Base R ½" x 6" x 9"

DETAIL A

DETAIL B



DETAIL C

ANCHORAGE ASSEMBLY

Back face of

½" x 1½" x 5¼" Bar

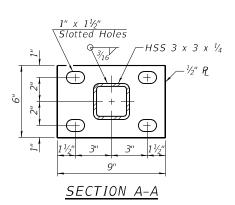
parapet

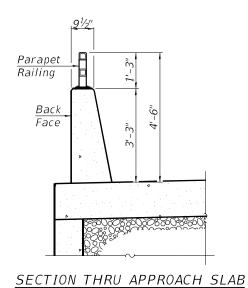
⅓" Fabric

reinforced

elastomeric pad

In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and setting $\frac{5}{6}$ " Ø fully threaded anchor rods with the same plate washers as specified above and heavy hex lock nuts according to Article 509.06 of the Standard Specifications. Embedment shall be according to the manufacturer's specifications.





Notes:

Place reinforcement bars to miss anchor rod locations.

All HSS tubing used for the Parapet Railing shall be CVN tested according to Article 1006.34(b) of the Standard Specifications.

All HSS tubing used for the Parapet Railing shall be ASTM A500.

All HSS tubing used for the Parapet Railing shall be ASTM A500 grade C.

All base plates used for the Parapet Railing shall be AASHTO M270 grade 50.

All heavy hex nuts shall be according to ASTM A 563 grade DH. All fully threaded anchor rods shall be ASTM F1554 grade 105.

The post base plate shall be fastened to the parapet snug tight and given an additional $\frac{1}{8}$ " turn.

All steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.

RAILING CRITERIA

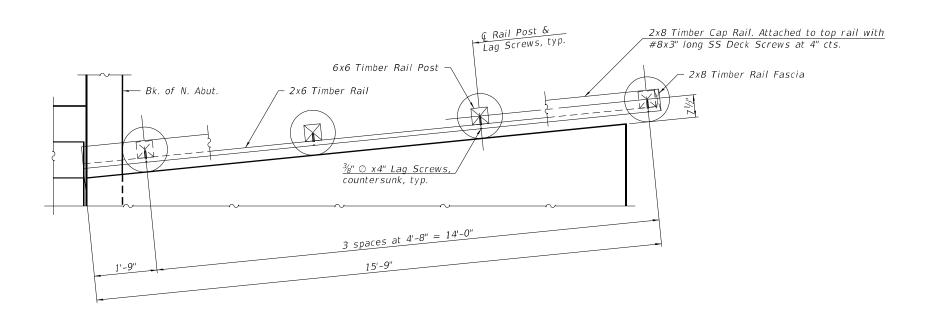
11/2"

MASH 2016 Test Level	4
Parapet Railing Weight (plf)	25
Max Post Spacing	10'-0"

<u>BILL OF MATERIAL</u>

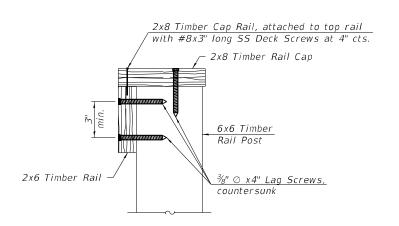
Item	Unit	Quantity
Parapet Railing	Foot	13.3

DESIGNED - E.M. Lagemann REVISED -SECTION COUNTY PARAPET RAILING DETAILS FUHRMANN ENGINEERING **STATE OF ILLINOIS** CHECKED - A.H. Morinaga Mansilla REVISED -MADISON 296 224 51-1R **STRUCTURE NO. 060-7003 DEPARTMENT OF TRANSPORTATION** E.M. Lagemann REVISED CONTRACT NO. 76A46 WWW. FUHRMANN-ENG .COM SHEET 10 OF 19 SHEETS CHECKED - A.H. Morinaga Mansilla REVISED .



PART PLAN

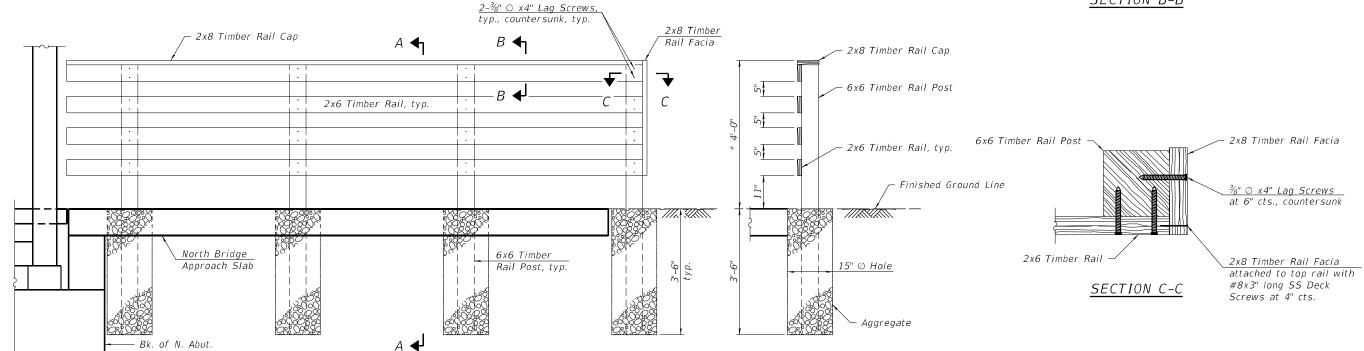
ELEVATION



SECTION B-B

* Top of top timber rail shall be installed a minimum 4'-0" above

the finished ground line. Adjust post length accordingly in field.



SECTION A-A

TIMBER FENCE RAIL DETAIL

Note:

No splices allowed on end railing.

BILL OF MATERIAL

Item	Unit	Quantity
Wood Rail	Foot	32.0

FUHRMANN
ENGINEERING
PLOT SCALE =

WWW. FUHRMANN-ENG.COM
PLOT DATE =

 USER NAME
 =
 DESIGNED
 A.H. Morinaga Mansilla
 REVISED

 CHECKED
 E.M. Lagemann
 REVISED

 PLOT SCALE
 =
 DRAWN
 A.H. Morinaga Mansilla
 REVISED

 PLOT DATE
 =
 CHECKED
 E.M. Lagemann
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

APPROACH RAILING DETAILS STRUCTURE NO. 060-7003

SHEET 11 OF 19 SHEETS

F.A.P. SECTION COUNTY TOTAL SHEET NO.

* 51-1R MADISON 296 225

CONTRACT NO. 76A46

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PEDESTRIAN TRUSS GENERAL NOTES

The substructure is designed per AASHTO LRFD, and based on the assumed truss loads (including deck) as shown on this sheet.

Truss Manufacturer shall camber the truss as necessary to provide allowance for dead load deflection.

Bridge bearing seat elevations are subject to revision based on the approved pedestrian truss superstructure shop drawings.

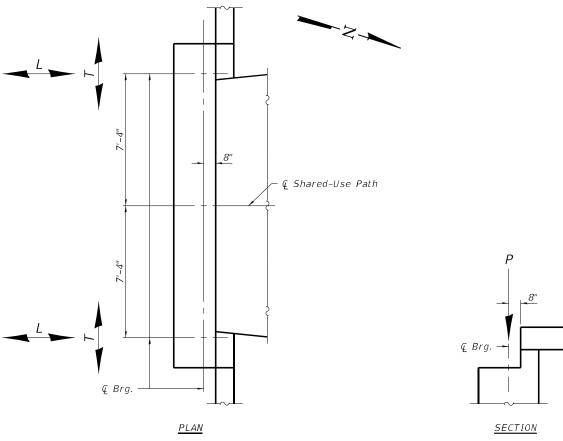
Design of pedestrian bridge shall accommodate anticipated dead and live load deflections so that no sag occurs within the bridge span. Pedestrian bridge profile must be either flat (when fully loaded), or have a slight upward camber.

Truss Manufacturer shall provide the reinforced concrete deck design. Concrete deck to utilize galvanized stay-in-place forms. Reinforcement shall be epoxy coated. Contractor shall place the concrete deck after truss is set. Cost included with Pedestrian Truss Superstructure.

Truss Manufacturer shall provide ADA compliant joint covers for joints between abutment backwall and steel truss.

Depth of pedestrian bridge from top of deck to bottom of bottom chord shall provide a minimum of 2'-0" vertical clearance above "Design Highwater Elevation".

Quantity of Concrete Superstructure is estimated for the aid of bidding. Actual quantity to be determined by Contractor based off of design provided by Truss Manufacturer.

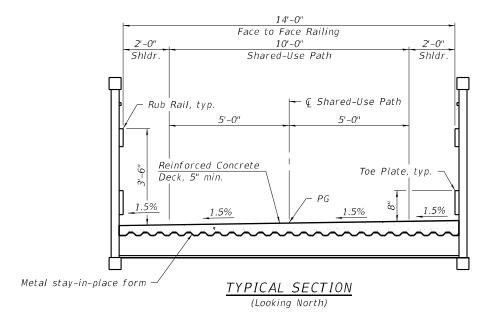


TRUSS LOAD LOCATIONS

Notes:

Details provided to indicate to Truss Manufacturer where loads are applied for abutment design. Truss Manufacturer and Contractor shall coordinate with Engineer if approved truss design differs from what is shown.

North Abutment shown, South Abutment similar.



UNFACTORED TRUSS REACTIONS

LOAD TY	PE	P (kips)	T (kips)	L (kips)
Dead Load, DC		78.42		
Vehicular Live Load	', LL	9.79		
Pedestrian Live Loa	ad, PL	42.71		
Uniform Temperatui	e, TU			7.75
Wind Load, WS	Windward	-15.66	24.18	0.80
Willu Loau, W3	Leeward	5.22	24.10	0.80
Seismic, EQ			15	.68

"P" = Vertical load, each bearing

"T" = Transverse load, each bearing

"L" = Longitudinal load, each bearing

Notes:

Dead load reaction includes estimated weight of truss and concrete deck. Truss Manufacturer to verify reactions.

"+" values indicate downward load.

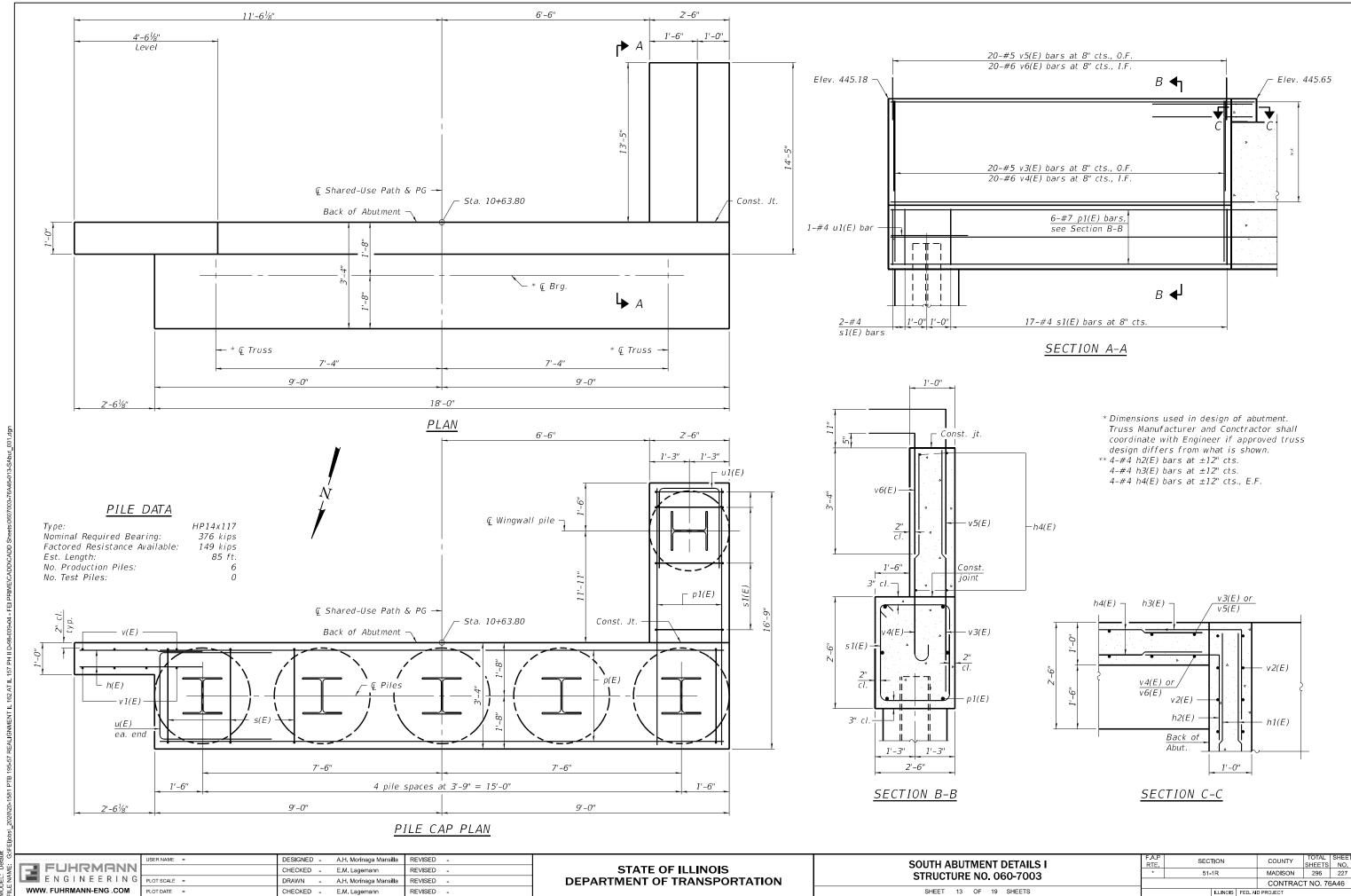
"-" values indicate upward load.

BILL OF MATERIAL

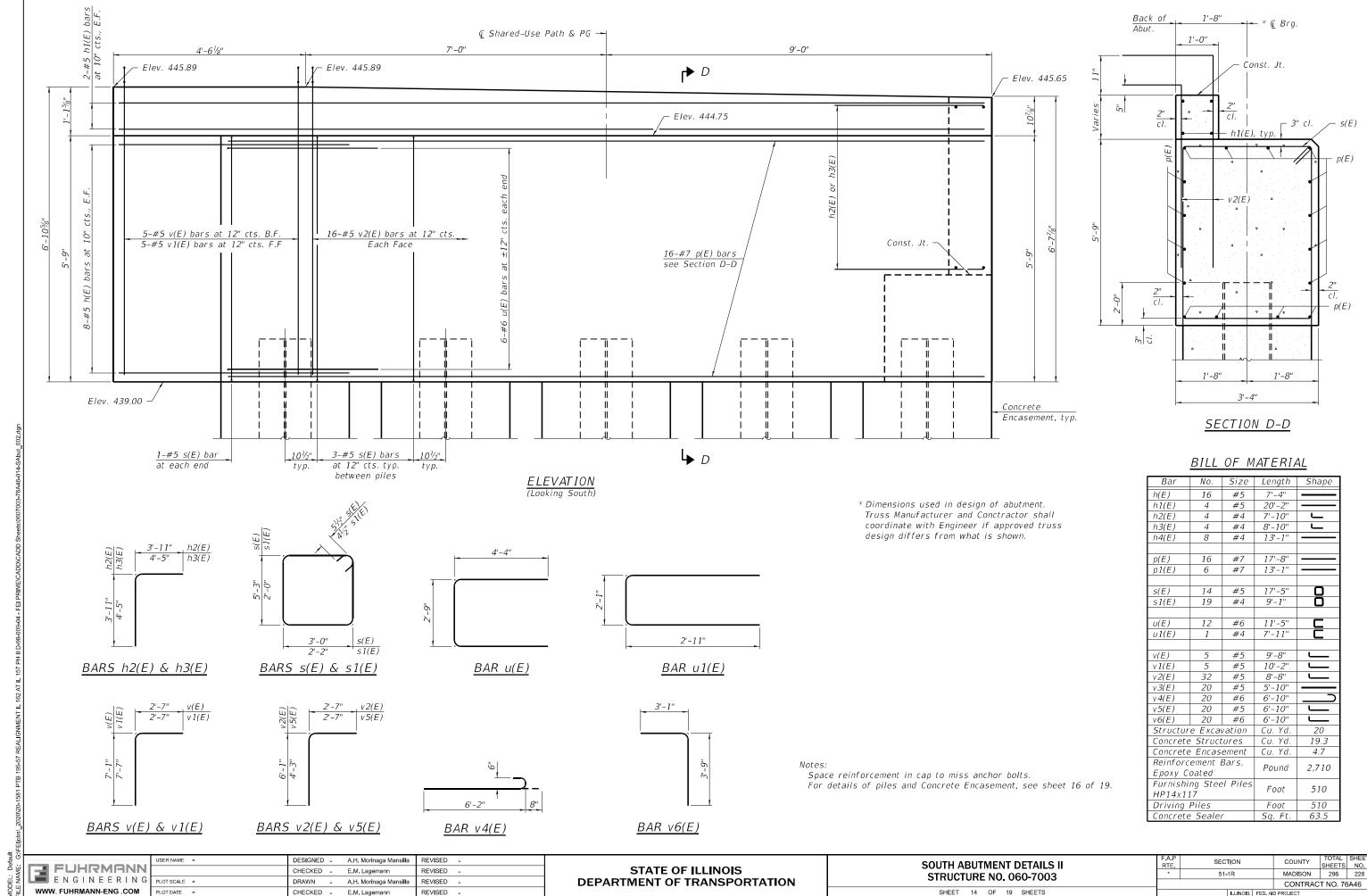
Item	Unit	Quantity
Concrete Superstructure	Cu. Yd.	37.6
Pedestrian Truss	Sa. Ft.	1.920.7
Superstructure	34.11.	1,920.7



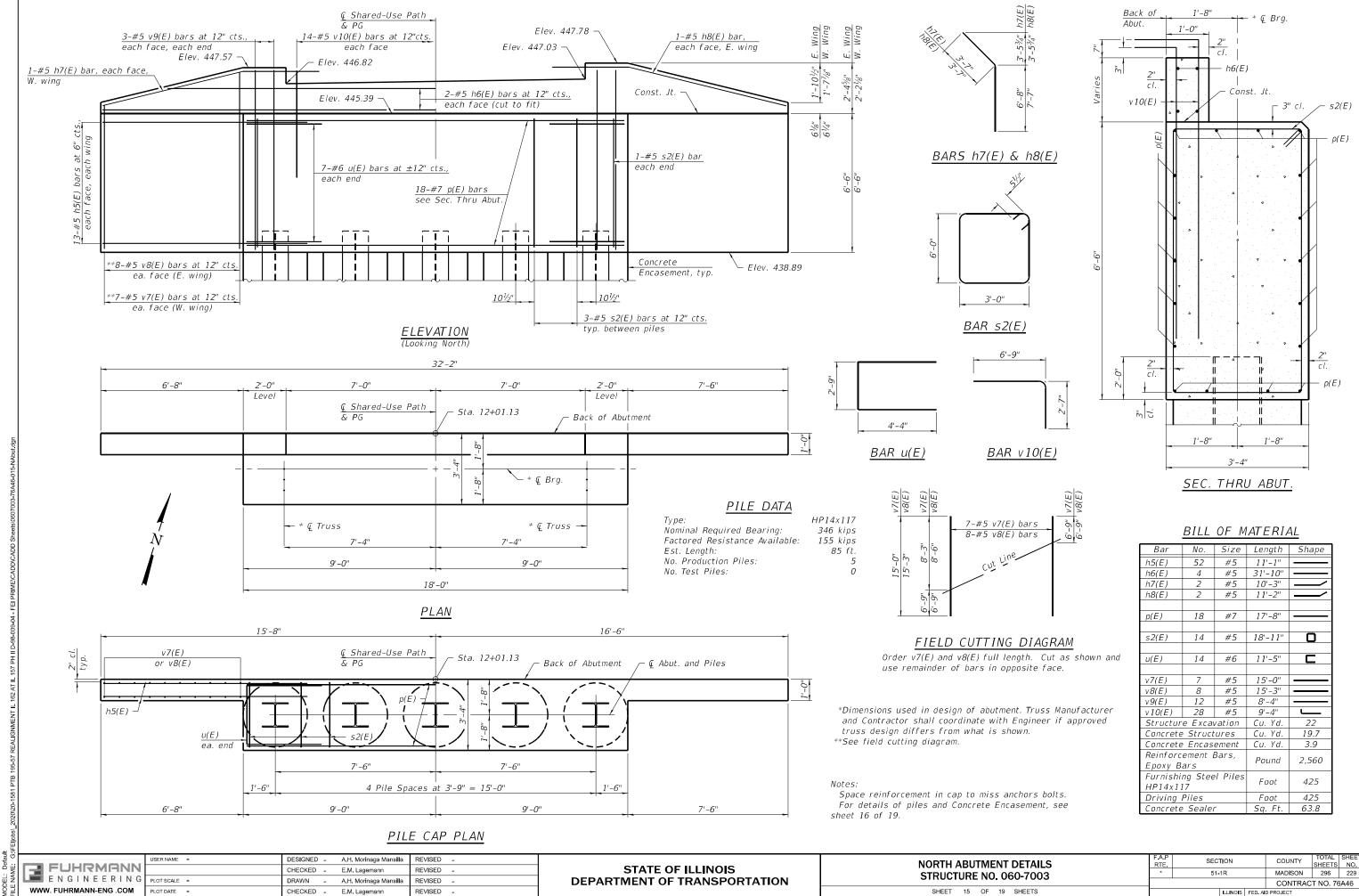
A.P TE.	SEC ⁻	TION		COUNTY	TOTAL	SHI
*	51-	1R		MADISON	296	2:
				CONTRA	CT NO. 7	76A
		ILLINOIS	FED. A	D PROJECT		



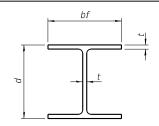
3/5/2024 7:57:03 AM



3/5/2024 8:00:45 AM

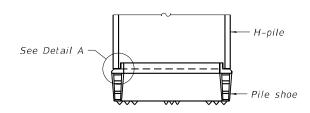


3/8/2024 11:25:56 AM

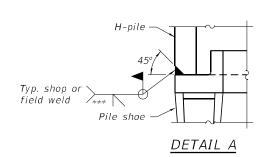


STEEL PILE TABLE

Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14×117	141/4"	147/8"	13/ ₁₆ "	30"
x102	14"	14¾"	¹¹ / ₁₆ "	30"
x89	13 ⁷ / ₈ "	143/4"	5/8"	30"
x73	13%"	145/8"	1/2"	30"
HP 12x84	121/4"	121/4"	¹ 1/ ₁₆ "	24"
x74	12½"	121/4"	5/8"	24"
x63	12"	121/8"	1/2"	24"
x53	1 1 3/4"	12"	⁷ / ₁₆ "	24"
HP 10x57	10"	101/4"	%16"	24"
x42	9¾"	101/8"	⁷ /16"	24"
HP 8x36	8"	81/8"	7/16"	18"



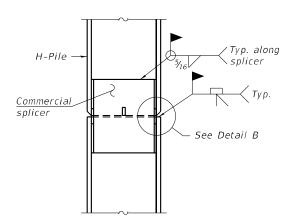
ELEVATION

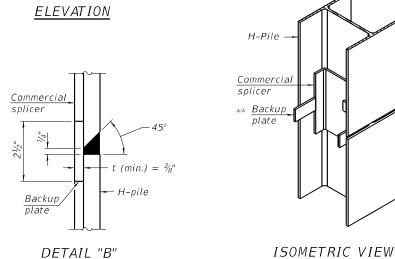


SHOE ATTACHMENT

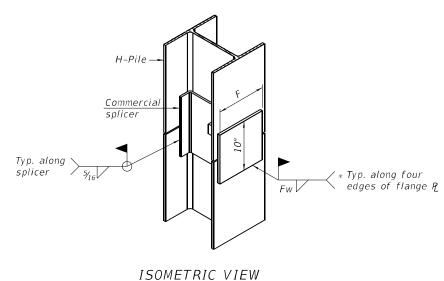
Note

The steel H-piles shall be according to AASHTO M270 Grade 50.





WELDED COMMERCIAL SPLICE

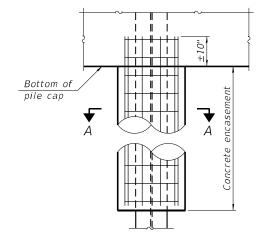


ISUMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- st Interrupt welds $^{1}\!\!/_{4}$ " from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.

*** Weld size per pile shoe manufacturer (5/16" min.).



Welded wire fabric 6 x 6-W4.0 x W4.0 weighing 58#/100 sq. ft. Bend as required to fit into wall.

Forms for encasement may be omitted when soil conditions permit.

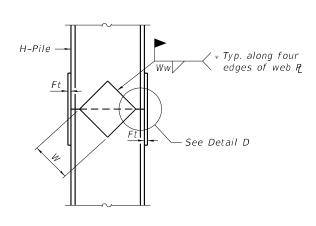
H-pile

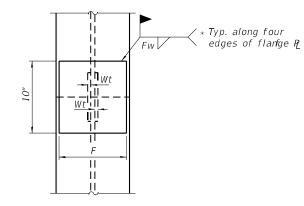
ELEVATION

SECTION A-A

INDIVIDUAL PILE CONCRETE ENCASEMENT

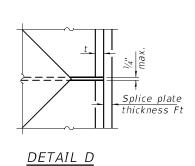
(when specified)





ELEVATION

END VIEW



Designation	F	Ft	Fw	W	Wt	Ww
HP 14×117	121/2"	1"	7/8"	73/4"	5/8"	1/2"
x102	12½"	7/8"	3/4"	73/4"	5/8"	1/2"
x89	12½"	3/4"	¹¹ / ₁₆ "	73/4"	5/8"	1/2"
x73	12½"	5/8"	%16"	73/4"	5/8"	1/2"
HP 12x84	10"	7/8"	11/16"	6½"	5/8"	1/2"
x74	10"	7/8"	¹ ½16"	6½"	5/8"	1/2"
x63	10"	5/8"	1/2"	6½"	1/2"	3/8"
x53	10"	5/8"	1/2"	6½"	1/2"	3/8"
HP 10x57	8"	3/4"	%16"	51/4"	1/2"	3/8"
x42	8"	5/8"	%16"	51/4"	1/2"	3/8"
HP 8x36	7"	5/8"	⁷ / ₁₆ "	41/4"	1/2"	3/8"

WELDED PLATE FIELD SPLICE

 F_-HP

2-1-2023

FUHRMANN
ENGINEERING
WWW. FUHRMANN-ENG.COM

	USER NAME =	DESIGNED -	A.H. Morinaga Mansilla	REVISED	-
V		CHECKED -	E.M. Lagemann	REVISED	-
G	PLOT SCALE =	DRAWN -	E.M. Lagemann	REVISED	-
	PLOT DATE =	CHECKED -	A.H. Morinaga Mansilla	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HP PILE DETAILS
STRUCTURE NO. 060-7003
SHEET 16 OF 19 SHEETS

3/5/2024 8:13:02 AM

SOIL BORING LOG

Page 1 of 3

Date 3/1,2/2022

DESCRIPTION ROUTE Route 157/162 LOGGED BY SCI Shared Use Path over Judy's Branch Madison County, Illinois, SEC. 4, TWP. 3N, RNG. 8W
Lat 38.740837 Long-90.002113 SECTION COUNTY DRILLING METHOD CME 550 w/HSA and Mud Rotary HAMMER TYPE Madison SN 060-0229/ SN UCS STRUCT, NO. Surface Water Elev. 060-7003 E LO 584+70.00 and 11+32.26 Station Stream Bed Elev. N/A 0 P S W W BORING NO. Groundwater Elev.: S Qu S Qu Station 17+86 First Encounter 425.0 ft ▼ Offset 50 ft LT **Upon Completion** N/A ft (ft) (/6") (tsf) (%) (ft) (/6") (tsf) (%) Ground Surface Elev. 438.0 After N/A Hrs. N/A ft FILL: CINDERS (A-1) Gray fine SAND (A-3), wet, medium dense 3.0 NC FILL: Dark brown to brown SILTY 23 P LOAM (A-4), stiff 0.9 1.5 Gray SILTY LOAM (A-4), moist, S/10% 412.5 Brown SILTY LOAM (A-4), very Gray fine SAND (A-3), wet, moist, very soft WOH Percentage finer than #200 test <0.25 NC WOH 12 NC performed (21.6% passing) WOH 13 Percentage finer than #200 test WOH NC performed (19.9% passing) NC WOH Brown to reddish brown SANDY LOAM (A-3) w/ slit, very moist, very loose Percentage finer than #200 test WOH NC performed (19.2% passing) WOH Gray CLAY (A-7), moist, stiff 1 Percentage finer than #200 test performed (20.3% passing) 2.0 <0.25 31 23 B/20% 423.0 -15 Gray and Brown SILTY CLAY (A-6), moist, very soft WOH 0.5 1 30 S/15% Gray CLAY (A-7), moist, soft Atterberg Limits test performed (LL=34, PI=18) 2.5 23 21 B/20% B/20% 418.0 -20

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Page 2 of 3

Date 3/1,2/2022

ROUTE	Route 157/162	DESC	CRIPTIC	N	Sha	red Use Path over Judy's Branch L	ogg	ED B	/ <u>s</u>	CI
SECTION	51-1R		LOCA	ΠON	Lant 38	Madison County, Illinois, SEC. 4, TWP. 38 3.740837 Long -90.002113	N, RN	IG. 8V	V	
COUNTY	Madison D	RILLING M	IETHOD			w/HSA and Mud Rotary HAMMER TYPE		Auto	omatic	
BORING NO.	SN 060-0229/ SI 060-7003 584+70.00 and 11+3 B-7 17+86		L O W	U C S Qu	M O I S T	Surface Water Elev. N/A ft Stream Bed Elev. N/A ft Groundwater Elev.: First Encounter 425.0 ft	D E P T H	B L O W S	U C S	M O I S T
Offset Ground Surfa	50 ft LT ace Elev. 438.0	ft (f	t) (/6")	(tsf)	(%)	Upon Completion N/A ft After N/A Hrs. N/A ft	(ft)	(/6")	(tsf)	(%)
Gray CLAY (A (continued)	-7), moist, stiff	396.0			()	Gray CLAY LOAM (A-7), moist, stiff (continued)			(7	(10)
Gray SANDY moist, stiff (fill)	CLAY LOAM (A-6),					Gray CLAY (A-7), moist, very stiff	_			
		-	3 4 45 7	2.0 B/20%	18		-65	6 6 9	3.0 B/20%	22
		-					_			
		-	3 4	1.6	17	Becomes brown and stiff		6	2.7	32
			₅₀ 6	B/20%			-70	5	B/20%	
Gray CLAY LO	DAM (A-7), moist,	384.5	5	2.8 B/20%	22	Becomes very stiff		4 7	2.1 B/20%	28
			55 8				75	9		
		-								
			3 4 60 4	1.4 B/20%	28	Becomes gray 358.5	-80	4 5 7	1.4 B/20%	28

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)

DESIGNED - A.H. Morinaga Mansilla REVISED -CHECKED - E.M. Lagemann REVISED -A.H. Morinaga Mansilla REVISED CHECKED - E.M. Lagemann

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION **BORING LOGS** 51-1R **STRUCTURE NO. 060-7003** SHEET 17 OF 19 SHEETS

COUNTY

MADISON 296 231

CONTRACT NO. 76A46

SOIL BORING LOG

Page <u>3</u> of <u>3</u>

Date 3/1,2/2022

ROUTE Route 157/162	ESCI	₹Р∏О	N	Sha	ared Use Path over Judy's Branch LOGGED BY S
SECTION 51-1R		LOCAT	TON		Madison County, Illinois, SEC. 4, TWP. 3N, RNG. 8W
COUNTY Madison DRILLIN	IG ME	THOD			8.740837 Long-90.002113 w/HSA and Mud Rotary HAMMER TYPE Automatic
SN 060-0229/ SN					
STRUCT. NO. 060-7003	D	В	ū	M	Surface Water Elev. N/A ft
Station 584+70.00 and 11+32.26	E	L	C	0	Stream Bed Elev. N/A ft
SORING NO. B-7	ΙŢ	w	3	s	Groundwater Elev.:
Station 17+86	H	S	Qu	T	First Encounter 425.0 ft \(\bigvee\)
Offset 50 ft LT				20000000	Upon Completion N/A ft
Ground Surface Elev. 438.0 ft	(ft)	(/6")	(tsf)	(%)	After N/A Hrs. N/A ft
Gray weathered CLAYEY SHALE, nard (continued)	_				
		-			
	_	-			
	-	+			
	-	6			-
	_	11	1.7	21	
	-85	20	S/10%	100000	
	-				
	-	50/5.5		11	
	_	50/2"		11	_
350. Sampler Refusal at 87.5 feet.	<u> </u>	50/1"		11.	-
Borehole grouted upon	-	(
ompletion.					
	-				
	-90	1			
	-	-			
		1			
	<u> </u>	Ī			
	·				
	_	-			
	- 50	-			
	-95				
	-50				
		-			
	_	-			
	-	+			
		1			
	10				
	400	1			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Page 1 of 3 Date 2/22,23/2022

ROUTE	Route 157/162	DE	SCF	₹РПО	N	Sha	ared Use Path Over Judy	's Branch	L0	OGGI	ED BY	s	CI
SECTION	51-1R		1	LOCAT	ПОМ	Madis Lat अ	on County, Illinois, SEC. 3.741190 Long-90.0023	4, TWP. 3N	, RNG.	BW			
COUNTY	Madison DR	RILLING	ME	THOD			w/HSA and Mud Rotary		TYPE		Auto	omatic	
STRUCT. NO. Station	SN 060-7003 11+32.26		D E P	B L O	U C S	M 0 I	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft ft	D E P	B L O	UCS	M 0 1
Station	B-24 585+27 95 ft LT ace Elev. 437.0	_ 	H (ft)	W S (/6")	Qu (tsf)	S T (%)	Groundwater Elev.: First Encounter Upon Completion After 24 Hrs.	N/A N/A 427.6	_ft _ft _ft ⊻	T H (ft)	W S (/6")	Qu (tsf)	S T (%)
FILL: Gray CL	AY (A-7), moist,	n	()	(44)	(10.7	(70)			416.5		(0)	()	(70)
medium stiff				4	0.4 S/10%	28	Gray CLAY (A-7) w/ sa deposits, moist, mediu	m stiff	415.0	_	WOH 2	0.5 P	26
		434.0		4	0/10/0		Gray fine SAND (A-3), loose	moist,			6		
Gray SILTY CI soft	LAY (A-6), moist,			2	0.2		Becomes medium den	se		_	8	NC	
			-5	2	S/15%	18				-25	9 10		
			_	1 2	0.2 S/15%	21	w/ sandy loam deposits	s			7 6	NC	20
Grav SILTV CI	LAY (A-7), trace	429.0	_	1						_	11		
brown, moist, s	soft		<u>▼</u> -10	1 1 1	0.1 S/20%	20	No loam, trace coal fra	gments		-30	5 6 13	NC	32
Becomes gray	and brown			WOH						_			
Document gray			_	WOH 2	NC	25							
soft, sand is fir		424.0	_	WOH		28			403.0		8		
(LL=32, PI=10)		422.0	-15	2	NC	30	Brown CLAY (A-7), mo	oist, stiff	-100.0	-35	5 6	1.6 B/20%	32
Brown and gra (A-6), soft, mo	y SILTY CLAY ist			1		24				_			
			_	1 3	2.3 P	48				_			
	LAY LOAM (A-6) w/ moist, medium stiff	4 <u>19.0</u>		3			Becomes medium stiff	. w/ siltv		_	3		
			-20	2	0.5 B/20%	28	loam deposits	, 		-40	2 5	1.0 B/20%	26

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)

AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)

BORING LOGS	F.A.P RTE	SECTION	COUNTY	TOTAL SHEETS	SHE
STRUCTURE NO. 060-7003	*	51-1R	MADISON	296	23:
3111001011E 110: 000-1003			CONTRA	CT NO. 7	76A4
SHEET 18 OF 19 SHEETS		ILLINOIS FED. /	ID PROJECT		

SOIL BORING LOG

Page 2 of 3

Date 2/22,23/2022

SECTION	51-1R		_ LC	CAT	ION	اد اسما	Madison County, Illinois, SEC. 4, TV 3.741190 Long-90.002303	VP. 3N,	RNG	8. 8W		
COUNTY Mad	dison Di	RILLING	METH	HOD			w/HSA and Mud Rotary HAMMER	TYPE		Auto	omatic	
STRUCT. NO	SN 060-7003 11+32.26		D E P	B L O	U C S	M 0	Surface Water Elev. N/A Stream Bed Elev. N/A	ft ft	D E P	B	U C S	h C
BORING NO Station Offset	585+27	_	T H	W S	Qu	S T	Groundwater Elev.: First Encounter N/A Upon Completion N/A	-	H	W S	Qu	
Ground Surface E	lev. 437.0	ft	(ft) (/6")	(tsf)	(%)	After 24 Hrs. 427.6		(ft)	(/6")	(tsf)	(9
Brown CLAY (A-7), (continued)	moist, stiff	-	-				Brown and gray CLAY LOAM (A-7), moist, stiff (till) (continued)		_			
		-					Brown CLAY (A-7), moist, very stiff	<u>375.0</u>	_			
Becomes gray and	stiff	1.4	-45	4 5 7	3.7 B/20%	20			-85	5 8 13	3.1 B/20%	2
		-										
Gray SILTY CLAY I moist, stiff	OAM (A-6),	390.0	_						_			
		-	-50	5 6 7	1.7 B/20%	24	Grayish brown SILTY CLAY (A-6), trace organics, moist, medium stiff	368.0	-/0	3 3 4	0.7 B/20%	3
		=						No market consists	_			
Gray and brown SA LOAM (A-6), trace g very stiff (glacial till)	ravel, moist.	<u>385.0</u>					Grayish brown CLAY (A-7), trace organics, moist, medium stiff	<u>365.0</u>	_			
			-55	7 7 11	2.7 B/20%	15			-75	3 3 4	1.4 B/20%	2
		-										
Brown and gray CL (A-7), moist, stiff (til	AY LOAM I)	380.0							_			
		-	-60	6 5 7	3.3 B/20%	33	Grayish brown CLAY LOAM (A-7), trace gravel, moist, very stiff	<u>358.0</u>	-80	4 5 13	1.6 B/20%	2

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Page 3 of 3

Date 2/22,23/2022

SECTION 51-1R		1	LOCAT	TON	Isant 3.0	Madison County, Illinois 3.741190 Long-90.0023	S. SEC. 4, TWP. 3N, F	NG. 8W
COUNTY Madison E	RILLING	ME	THOD			w/HSA and Mud Rotary		
STRUCT. NO. SN 060-7003 Station 11+32.26		D E P	B L O	UCS	M 0 I	Surface Water Elev. Stream Bed Elev.	N/A ft	
BORING NO. B-24 Station 585+27 Offset 95 ft LT	_	T H	w	Qu	S	Groundwater Elev.: First Encounter Upon Completion	N/A ft	
Ground Surface Elev. 437.0	ft	(ft)	(/6")	(tsf)	(%)	After 24 Hrs.	427.6 ft ¥	
Grayish brown CLAY LOAM (A-7) trace gravel, moist, very stiff (continued)	l _i e	_						
Brown CLAY (A-7), moist, very	354.5	_						
		-	20		26			
Grayish brown SANDY CLAY	352.5	-85	12	5.0 S/20%	26			
LOÁM (A-6), trace gravel, moist, very stiff								
		-	-					
Gray CLAYEY SHALE, hard	348.5		50/2"					
under contrata ♥ 0, de la reconstante de la contrata del contrata de la contrata de la contrata del contrata de la contrata del la contrata del la contrata de la contrata del la contrata de la contrata del la contrata			50/1"					
	346.8	-90						
Sampler Refusal at 90.25 feet. Borehole grouted upon completion.	7,0,0	_	50/2" 50/1"					
completion.		-						
		-						
		-95						
		-						
		-						
		_						
			-					
		-100						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)

AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)

PROPOSED IL ROUTE 157 CURVES C50 AND C51

STATION	LEFT EDGE	SLOPE %	CENTERLINE	SLOPE %	RIGHT EDGE
572+24.35	452.13	-1.33	452.29	-0.92	452.18
572+50.00	452.26	-0.23	452.29	-1.15	452.15
572+75.00	452.39	0.84	452.29	-1.36	452.13
572+90.35	452,47	1.50	452.29	-1.50	452.11
573+00.00	452.53	1.94	452.30	-1.94	452.06
573+01.35	452.54	2.00	452.30	-2.00	452.06
573+50.00	452.51	2.00	452.27	-2.00	452.03
574+00.00	452,32	2.00	452,08	-2.00	451.84
574+50.00	451.96	2.00	451.71	-2.00	451.46
		FULL SU	PERELEVATION		
576+50.00	450.05	2.00	449.72	-2.00	449.39
577+00.00	449.57	2.00	449.22	-2.00	448.87
577+50.00	449.09	2.00	448.72	-2.00	448.35
577+75.11	448.85	2.00	448.47	-2.00	448.09
577+93.11	448,59	1.50	448.30	-1.50	447.99
578+00.00	448.48	1.32	448.23	-1.50	447.91
578+25.00	448.11	0.65	447.98	-1.50	447.65
578+50.00	447.74	-0.02	447.75	-1.50	447.39
578+75.00	447.40	-0.69	447.53	-1.50	447.16
579+00.00	447,07	-1,36	447,34	-1.50	446,94
579+05.11	447.00	-1.50	447,30	-1.50	446.90
579+50.00	446.71	-1.50	447.01	-1.50	446.57
580+00.00	446.45	-1.50	446.75	-1.50	446.28
580+50.00	446.27	-1.50	446.57	-1.50	446.09
		NOR	MAL CROWN		
591+50.00	450,51	-1.50	450,78	-1.50	450,33
592+00.00	451,03	-1.50	451,30	-1.50	450.85
592+50.00	451,50	-1.64	451.79	-1.04	451.41
592+80.47	451.70	-1.72	452.01	-0.76	451.86

PROPOSED IL ROUTE 162 CURVES J12 AND J2

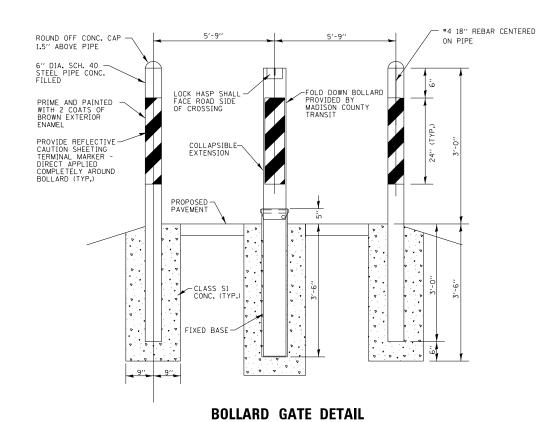
STATION	LEFT EDGE	SLOPE %	CENTERLINE	SLOPE %	RIGHT EDGE
6+91.56	438.26	-1.66	438.46	-1.25	438.31
7+00.00	438.27	-1.63	438.47	-1.29	438.31
7+42.06	438.34	-1.50	438.52	-1.50	438.34
7+50.00	438.39	-1.19	438.54	-1.50	438.36
7+75.00	438.60	-0.22	438.62	-1.50	438.44
8+00.00	438.84	0.76	438.75	-1.50	438.57
8+19.06	439.06	1.50	438.88	-1.50	438.70
8+25.00	439.13	1.73	438,92	-1.73	438.71
8+50.00	439.45	2.71	439.13	-2.71	438.80
8+75.00	439.82	3.68	439.38	-3.68	438.93
9+00.00	440.22	4.65	439.66	-4.65	439.11
9+25.00	440.67	5.63	439.99	-5.63	439.32
9+34.56	440.85	6.00	440.13	-6.00	439,41
9+50.00	441.08	6.00	440,36	-6.00	439,64
10+00.00	441.96	6.00	441.21	-6.00	440.47
10+50.00	443.03	6.00	442.23	-6.00	441.42
10+56.48	443.18	6.00	442.37	-6.00	441.56
10+75.00	443,54	5,44	442,79	-5.44	442,03
11+00.00	444.03	4.68	443.35	-4.68	442.68
11+25.00	444.47	3.92	443.89	-3.92	443.30
11+50.00	444.47	3.92	444.38	-3.17	443.89
					443.69
11+75.00	445.20	2.41	444.82	-2.41	
12+00.00	445,49	1,65	445,22	-1.65	444,95
12+04.98	445.54	1,50	445.29	-1.50	445.05
12+25.00	445.72	0.89	445.57	-1.50	445.32
12+50.00	445.90	0.14	445.88	-1.50	445.62
12+75.00	446.03	-0.62	446.14	-1.50	445.87
13+00.00	446.10	-1.38	446.35	-1.50	446.08
13+03.98	446.11	-1.50	446.38	-1.50	446.11
13+50.00	446.35	-1.50	446.64	-1.50	446.35
14+00.00	446.45	-1.50	446.75	-1.50	446.42
14+50.00	446.38	-1.50	446.68	-1.50	446.31
		NORI	MAL CROWN		
20+50.00	445.95	-1.50	446.43	-1.50	446.09
21+00.00	446.20	-1.50	446.68	-1.50	446.38
21+50.00	446.50	-1.50	446.98	-1.50	446.68
21+95.74	446.75	-1.50	447.23	-1.50	446.93
22+00.00	446.81	-1.39	447.25	-1.50	446.95
22+50.00	447.45	-0.08	447.47	-1.50	447.18
23+00.00	447.97	1.22	447.65	-1.50	447.37
23+10.74	448.06	1.50	447.69	-1.50	447.41
23+50.00	448.36	2.47	447.81	-2.47	447.36
24+00.00	448.67	3.71	447.96	-3.71	447.31
24+50.00	448.95	4.95	448.13	-4.95	447.32
24+68,24	449,10	5.40	448.23	-5.40	447.36
25+00.00	449.29	5.40	448.45	-5.40	447,61
25+50.00	449.71	5.40	448.93	-5.40	448.14
26+00.00	450.29	5.40	449.56	-5.40	448.83
26+49.12	451.01	5.40	450.33	-5.40	449.66
	451.02	5.37	450.35	-5.42	449.67
26+50.00	431.02) 3.37			

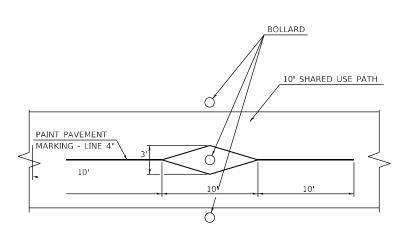
LOCHMUELLEI	R
GROU	•
1928 SrA Bradley R. Smith Driv	ne
Troy, IL 6225	

USER NAME = alex.huelskamp	DESIGNED - ESW	REVISED _
MODEL NAME = SE 01	DRAWN - LEC	REVISED _
PLOT SCALE = 40.0000 '/in.	CHECKED - BRM	REVISED _
PLOT DATE = 3/5/2024	DATE - 10/8/2021	REVISED -

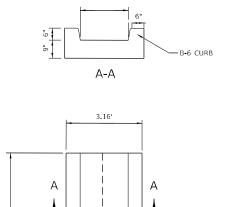
SUPERELEVATION TABLES								
	L ROUTE 162							
SCALE: NONE	SHEET	1	OF	1	SHEETS	Superelevation		

BOLLARD GATE PLAN VIEW





TYPICAL PAVEMENT MARKINGS AT BOLLARDS



OUTLET DETAIL N.T.S.

FUHRMANN
ENGINEERING
WWW. FUHRMANN-ENG. COM

USER NAME = ggayt	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 40.0000 / in.	CHECKED -	REVISED -
PLOT DATE = 6/26/2024	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

				F.A.P. SECTION				COUNTY TOTAL SHEET NO.			
DETAIL SHEET					*	* 51-1R			MADISON	296	235
									CONTRACT	NO. 76	5A46
SCALE: NTS	SHEET 1	OF 1	SHEETS	STA.	TO STA.	ILLINOIS FED. AID PROJECT					

