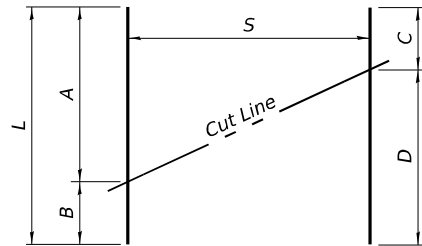


INSIDE ELEVATION OF S. PARAPET - SN 025-0115

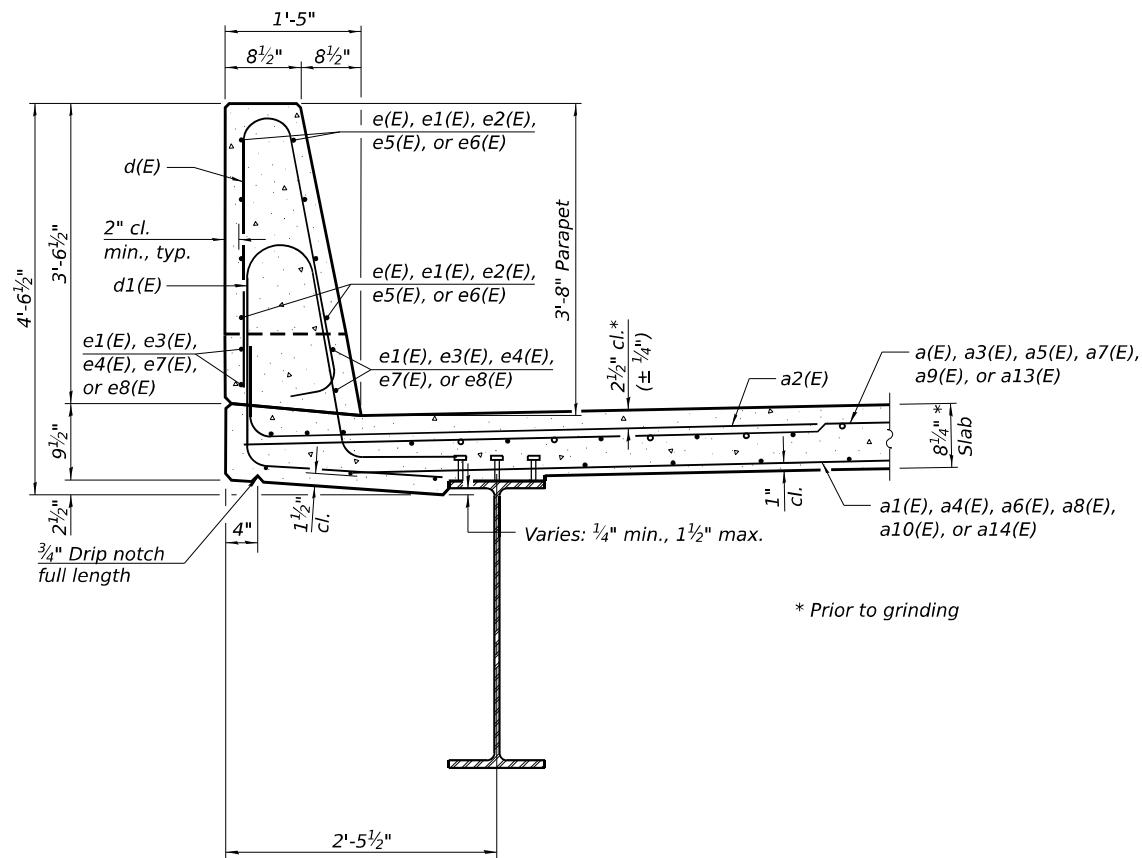
FILE NAME: DWG  License No. 184-000813 © Copyright CMT, Inc.	USER NAME = mcordova	DESIGNED - MAC	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE DETAILS - EB - I STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		DRAWN - BLH	REVISED -			70	25-1HB-3	EFFINGHAM	747	402
	PLOT SCALE = N/A	CHECKED - JTH	REVISED -						CONTRACT NO. 74893	
	PLOT DATE = 8/5/2024	DATE - 08/06/2024	REVISED -							
						SCALE:	SHEET 17 OF 46 SHEETS	STA.	TO STA.	



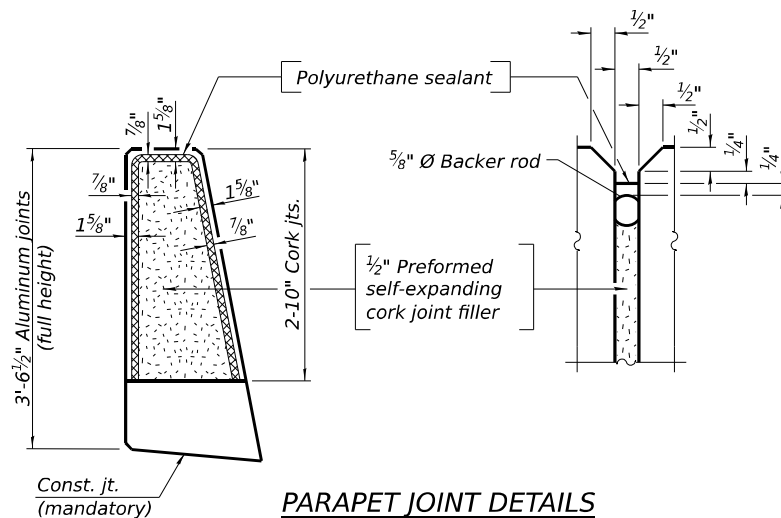
FIELD CUTTING DIAGRAM

Order bars full length.
Cut as shown and use remainder
of bars as indicated in Plan
on sheet 16 of 46.

Bar	S	A	B	C	D	L
a9(E)	13 spa. @ 7"	45'-4"	2'-11"	24'-11"	23'-4"	48'-3"
a10(E)	8 spa. @ 10"	43'-4"	5'-2"	25'-5"	23'-2"	48'-7"
a13(E)	14 spa. @ 7"	47'-9"	1'-8"	25'-6"	23'-11"	49'-5"
a14(E)	8 spa. @ 10"	43'-11"	5'-4"	25'-9"	23'-6"	49'-3"



SECTION THRU PARAPET

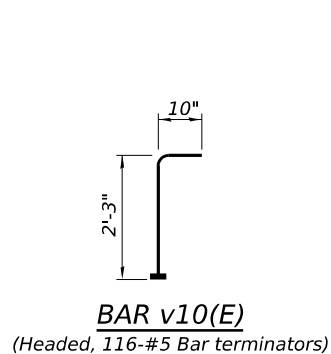


PARAPET JOINT DETAILS

SN 025-0115 SUPERSTRUCTURE

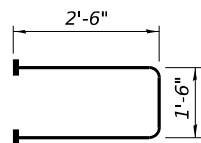
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	223	#5	40'-0"	—
a1(E)	158	#5	19'-7"	—
a2(E)	487	#6	8'-4"	—
a3(E)	75	#5	20'-0"	—
a4(E)	54	#5	39'-7"	—
a5(E)	70	#5	21'-3"	—
a6(E)	50	#5	40'-10"	—
a7(E)	78	#5	22'-6"	—
a8(E)	54	#5	42'-1"	—
a9(E)	14	#5	48'-3"	—
a10(E)	9	#5	48'-7"	—
a11(E)	4	#5	30'-11"	—
a12(E)	4	#5	33'-3"	—
a13(E)	15	#5	49'-5"	—
a14(E)	9	#5	49'-3"	—
b(E)	378	#5	27'-1"	—
b1(E)	112	#6	33'-1"	—
b2(E)	280	#5	26'-8"	—
b3(E)	112	#5	16'-3"	—
d(E)	454	#5	6'-11"	—
d1(E)	434	#5	7'-10"	—
e(E)	32	#4	17'-6"	—
e1(E)	96	#4	6'-10"	—
e2(E)	24	#4	14'-8"	—
e3(E)	8	#4	35'-5"	—
e4(E)	8	#4	23'-7"	—
e5(E)	32	#4	17'-3"	—
e6(E)	24	#4	14'-5"	—
e7(E)	8	#4	34'-11"	—
e8(E)	8	#4	23'-3"	—
m10(E)	8	#6	31'-2"	—
m11(E)	21	#6	5'-5"	—
m12(E)	9	#6	3'-8"	—
m13(E)	6	#6	2'-3"	—
m20(E)	8	#6	33'-6"	—
m21(E)	21	#6	5'-5"	—
m22(E)	9	#6	5'-3"	—
m23(E)	6	#6	2'-3"	—
s10(E)	60	#5	6'-6"	—
s11(E)	60	#5	9'-0"	—
s20(E)	66	#5	6'-6"	—
s21(E)	66	#5	9'-0"	—
v10(E)	116	#5	3'-1"	—
Concrete Superstructure			Cu. Yds.	298.7
Reinforcement Bars, Epoxy Coated			Pound	71,680



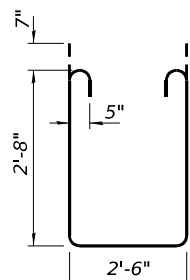
BAR v10(E)

(Headed, 116-#5 Bar terminators)

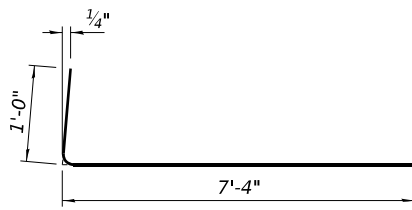


BARS s10(E) & s20(E)

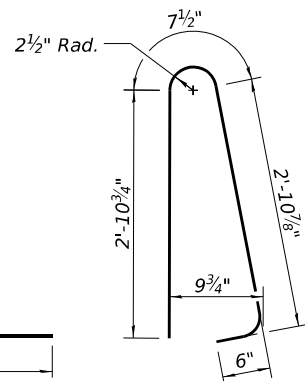
(Headed, 252-#5 Bar terminators)



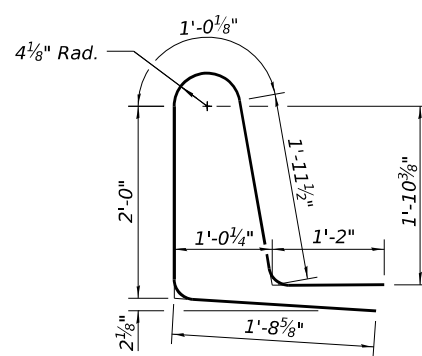
BARS s11(E) & s21(E)



BAR a2(E)



BAR d(E)



BAR d1(E)

Notes:

- The 3/16" min. aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated with 5 mils of either bitumen paint or epoxy paint to minimize reaction with wet concrete. Cost included with Concrete Superstructure.
- The polyurethane sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.
- Bar terminators, paid for separately. See Total Bill of Material.

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DRAWN	- BLH	REVIS	-	REVIS	-
PLOT SCALE	= N/A	CHECKED	- JTH	REVIS	-
PLOT DATE	= 8/5/2024	DATE	- 08/06/2024	REVIS	-

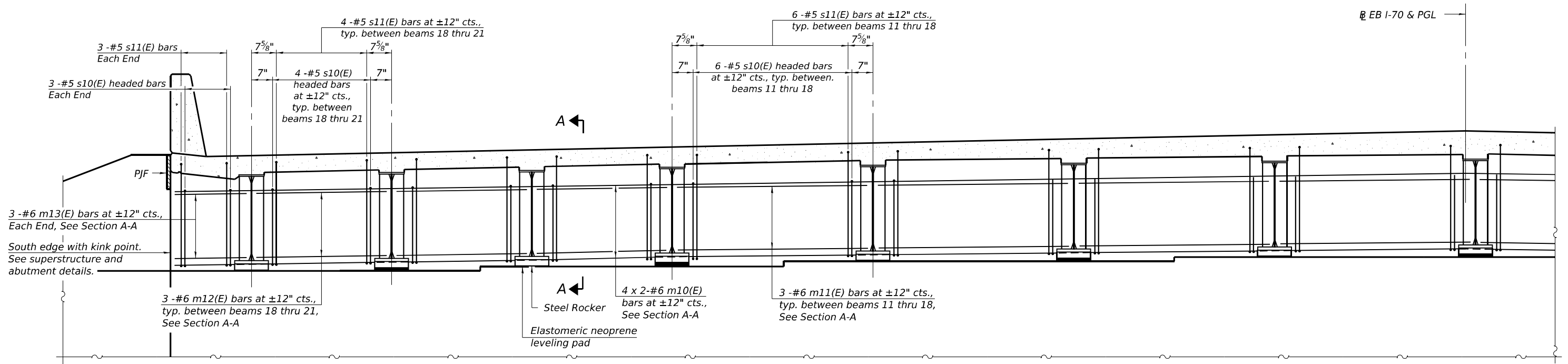
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS - EB - II
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

SCALE: SHEET 18 OF 46 SHEETS STA. TO STA.

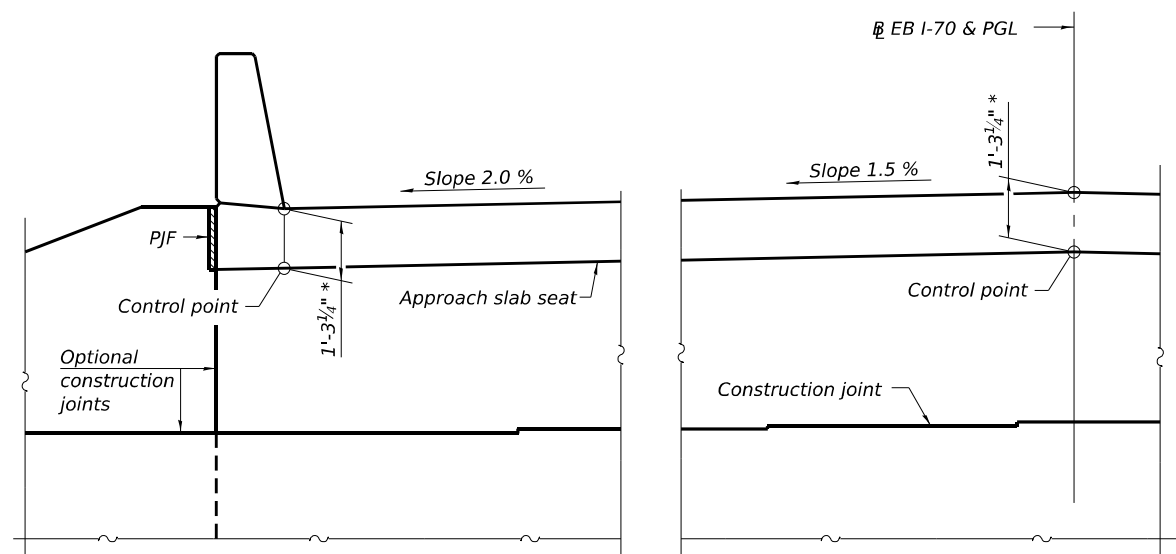
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	403
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

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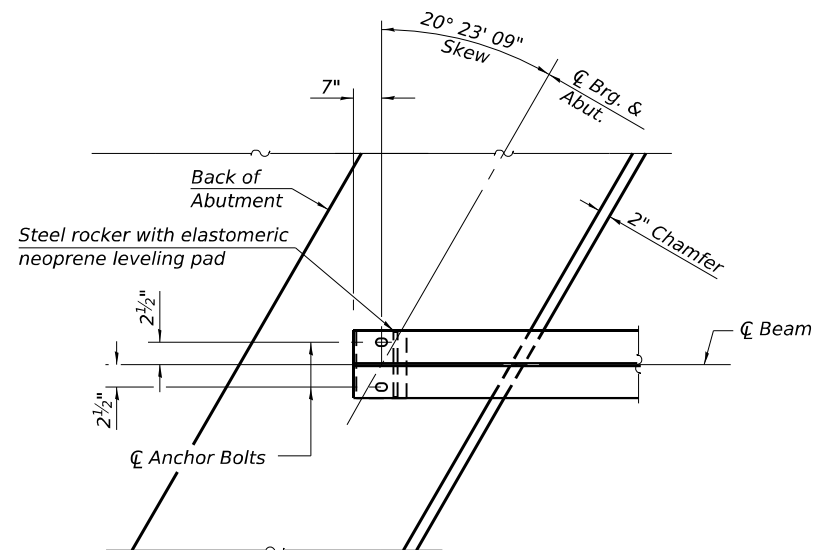


MINIMUM BAR LAP
#6 bar = 4'-0"

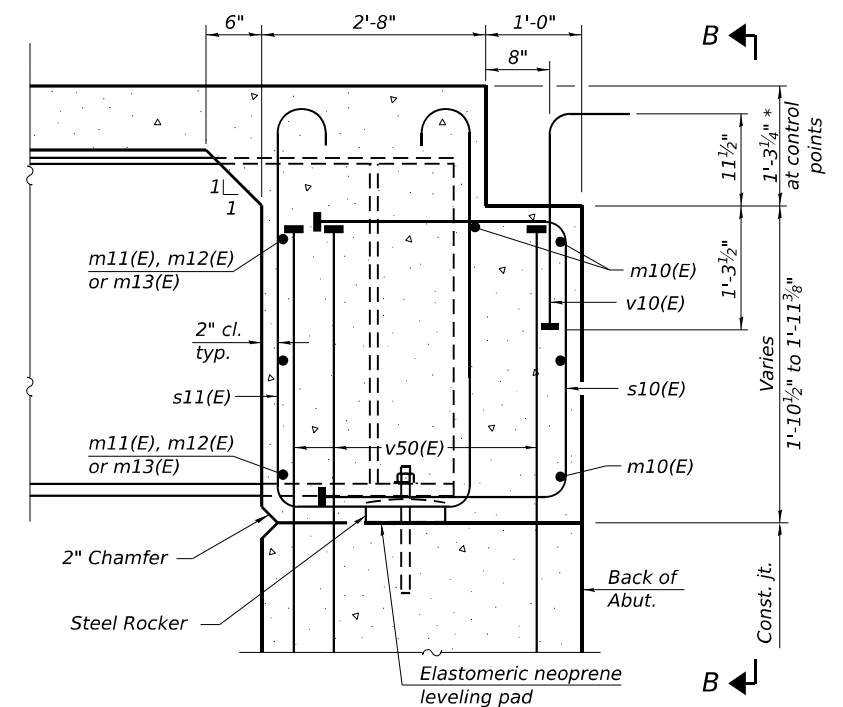
DIAPHRAGM AT WEST ABUTMENT
(Looking west)



VIEW B-B



PLAN AT ABUTMENT
(Showing bottom flange of beam)



SECTION A-A
(at Rt. L's)

* Prior to grinding

- Notes:
1. See sheet 18 of 46 for superstructure details and Bill of Material.
 2. See sheets 25 and 26 of 46 for P/JF details.
 3. The s10(E) and s11(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.
 4. The approach slab seat shall have a constant slope determined from the control points shown.
 5. See Framing Plan on sheet 30 of 46 for beam numbering.
 6. Bars indicated thus 4 x 2-#6, etc. indicates 4 lines of bars with 2 lengths per line.



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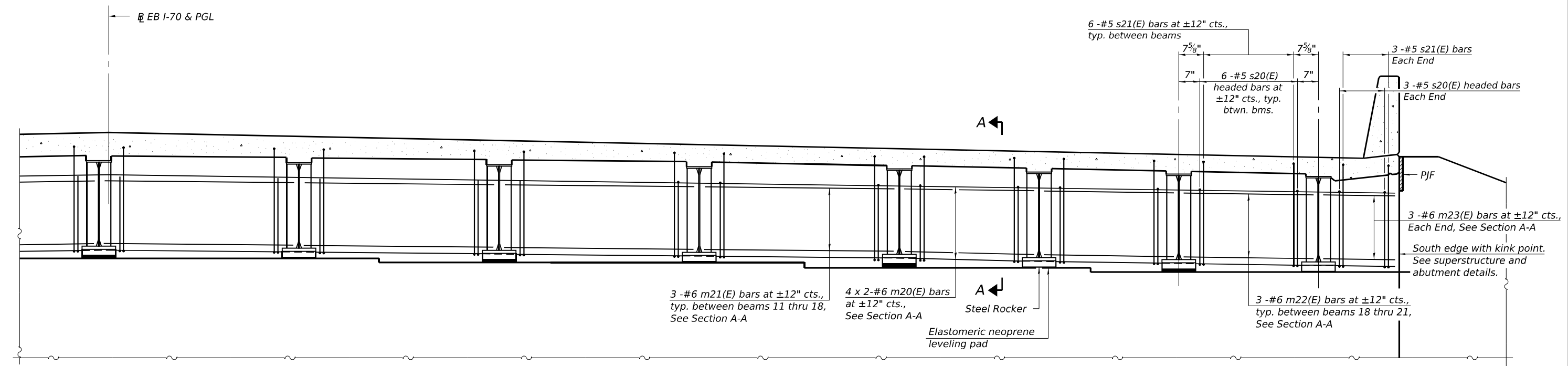
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DRAWN -	BLH	REVISED -			
PLOT SCALE	= N/A	CHECKED -	JTH	REVISED -	
PLOT DATE	= 8/5/2024	DATE -	08/06/2024	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DIAPHRAGM DETAILS - EB - I
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

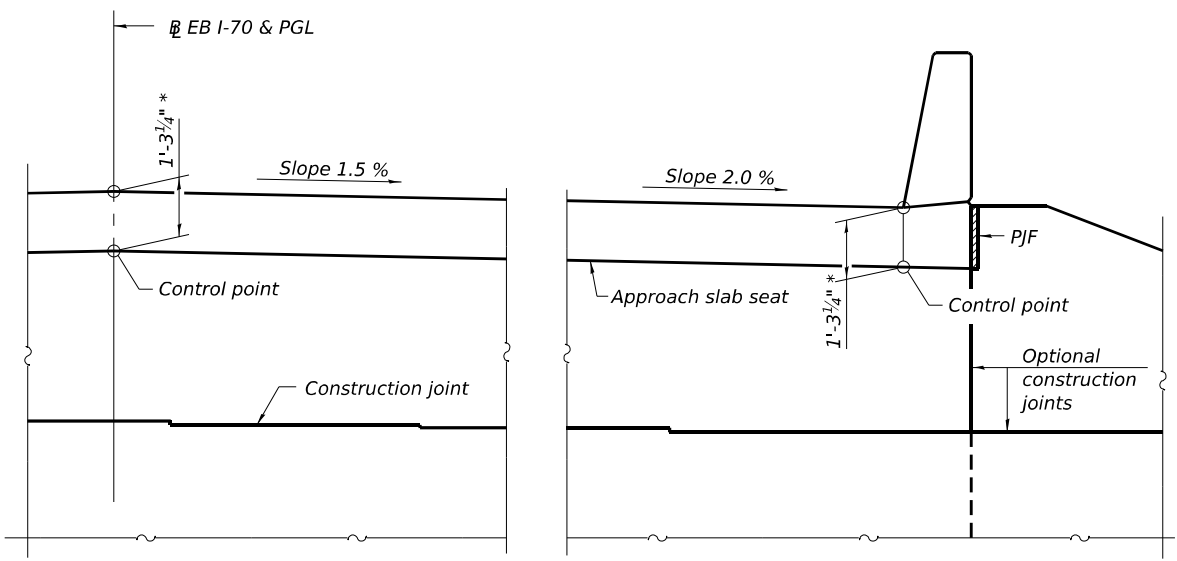
SCALE: SHEET 20 OF 46 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 74893				
ILLINOIS		FED. AID PROJECT		

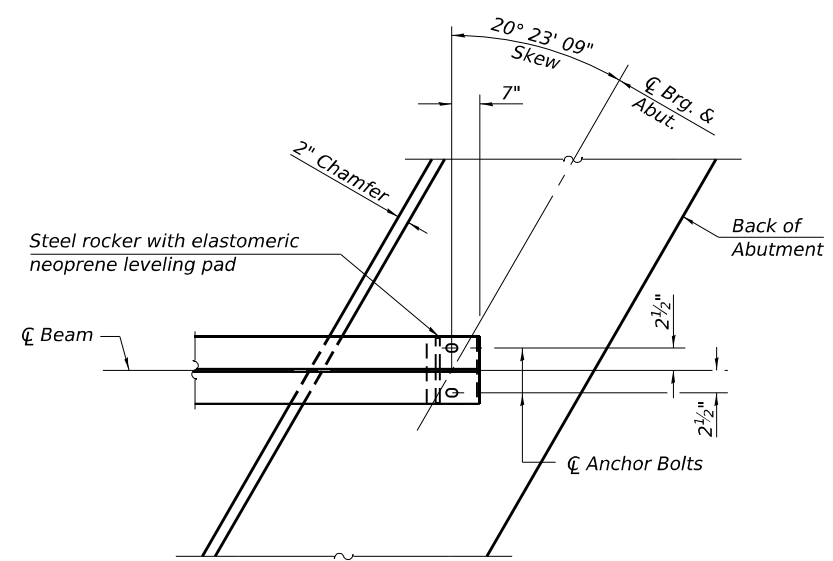


DIAPHRAGM AT EAST ABUTMENT

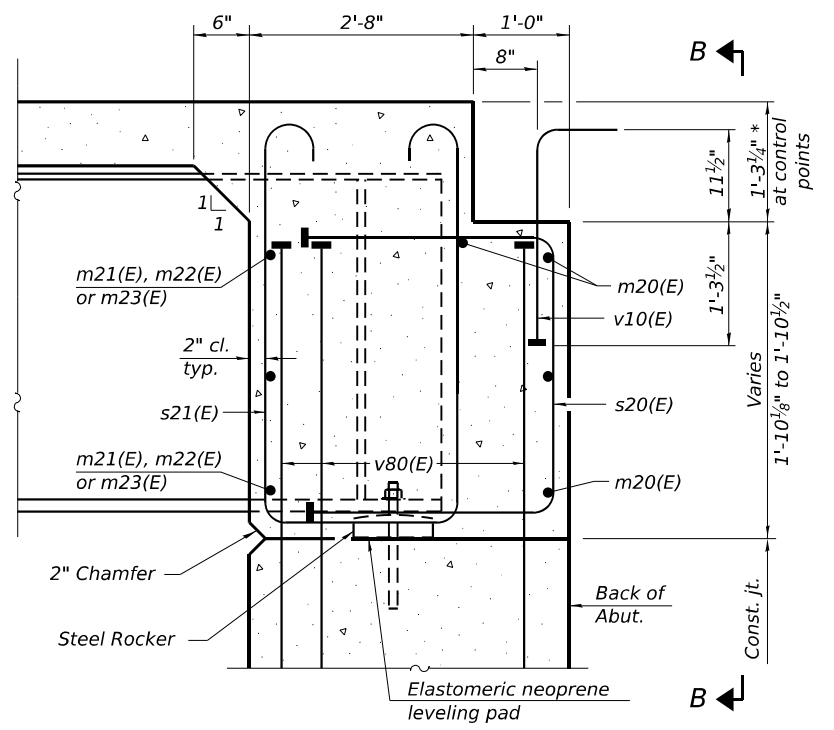
MINIMUM BAR LAP
#6 bar = 4'-0"



VIEW B-B



PLAN AT ABUTMENT
(Showing bottom flange of beam)



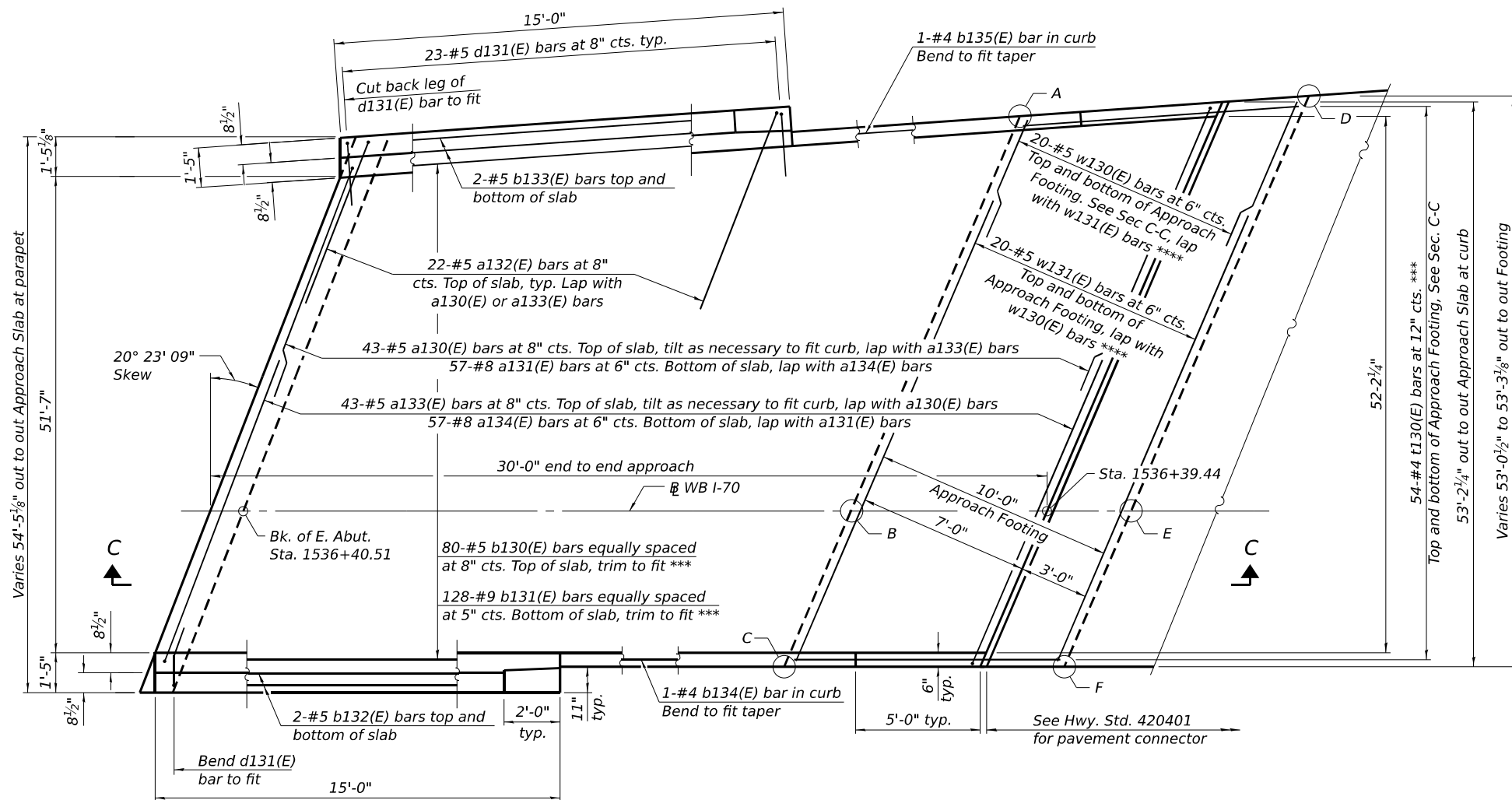
SECTION A-A
(at Rt. L's)

* Prior to grinding

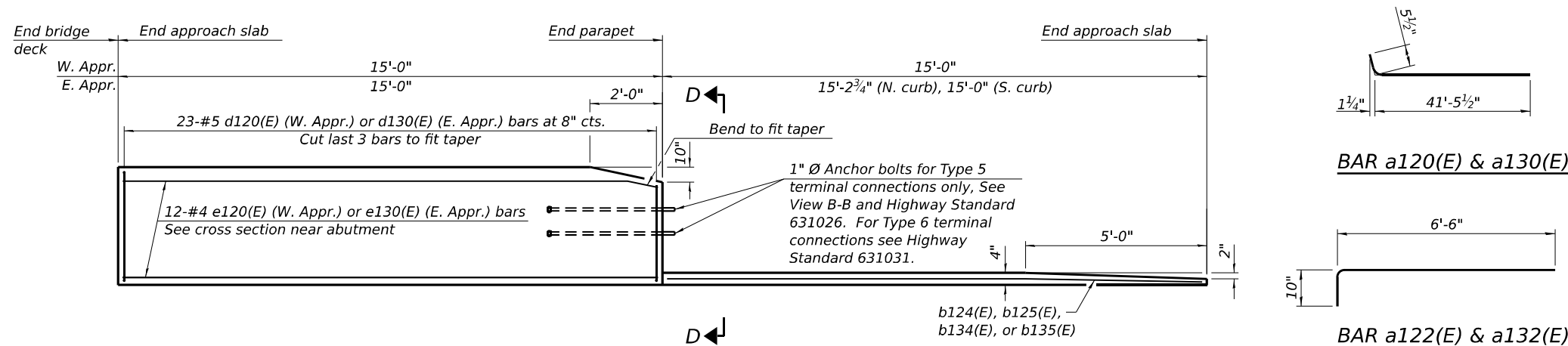
- Notes:**
1. See sheet 18 of 46 for superstructure details and Bill of Material.
 2. See sheets 25 and 26 of 46 for P.J.F. details.
 3. The s20(E) and s21(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.
 4. The approach slab seat shall have a constant slope determined from the control points shown.
 5. See Framing Plan on sheet 30 of 46 for beam numbering.
 6. Bars indicated thus 4 x 2-#6, etc. indicates 4 lines of bars with 2 lengths per line.

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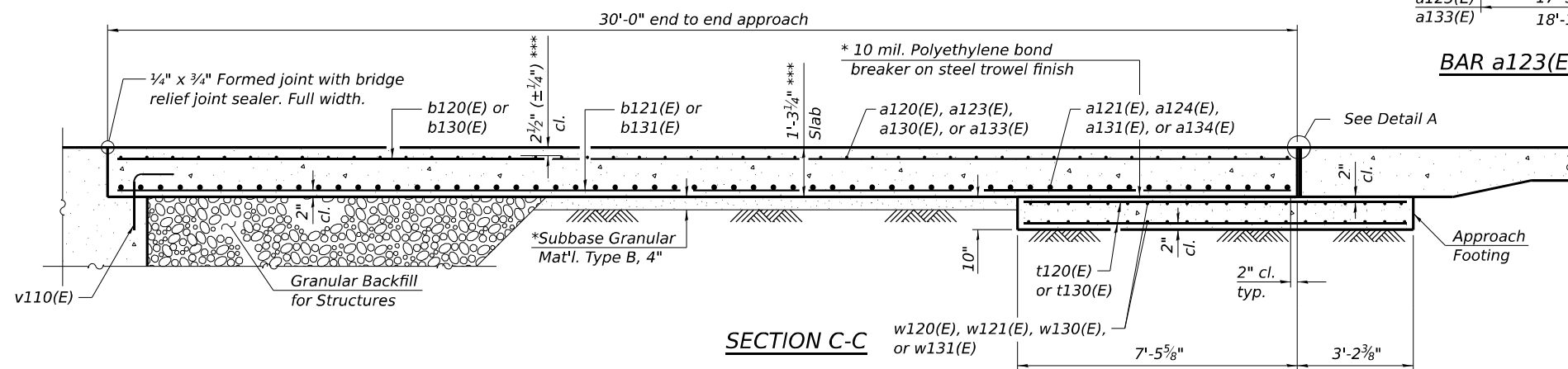
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	PLOT DATE = 8/5/2024					TO STA.		ILLINOIS		FED. AID PROJECT		



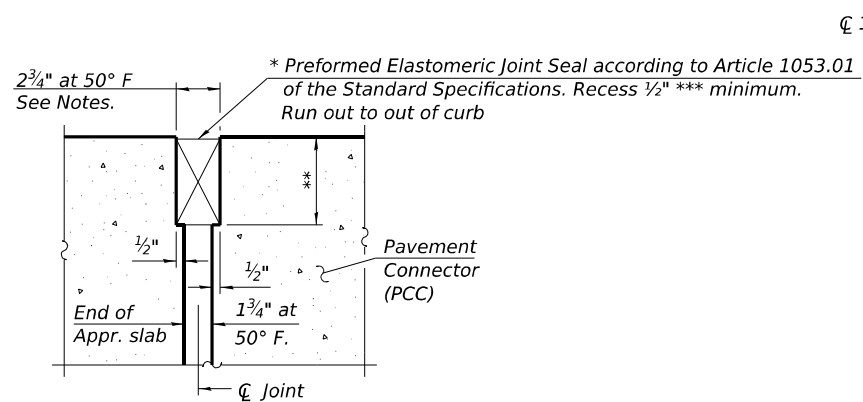
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INSIDE ELEVATION OF PARAPET AND CURB



SECTION C-C

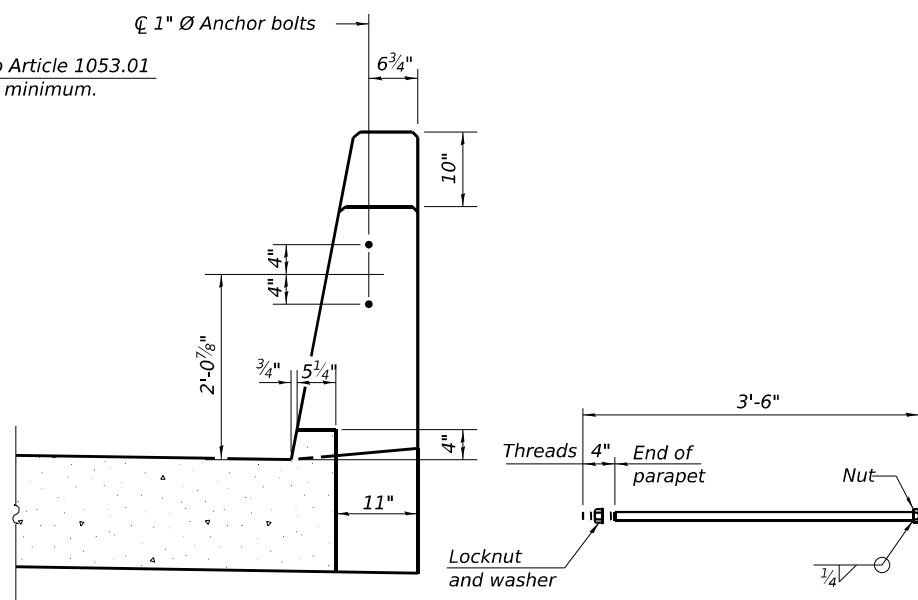


DETAIL A
(at Rt. L's)

* Cost included with Concrete Superstructure (Approach Slab).

** Per manufacturer recommendations

*** Prior to grinding



VIEW D-D

*** 1" Ø ANCHOR BOLT**
(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications)

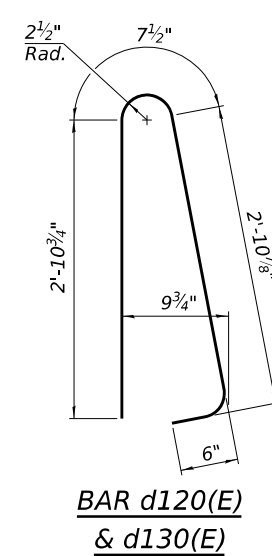
BAR a120(E) & a130(E)

BAR a122(E) & a132(E)

BAR a123(E) & a133(E)

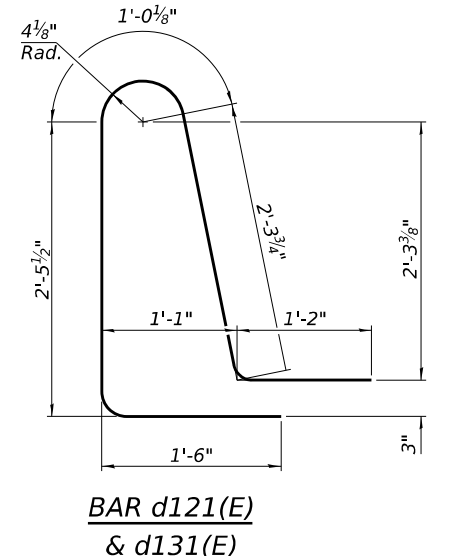
Notes:

1. The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
2. Parapet concrete shall be paid for as Concrete Superstructure.
3. Approach slab shall be paid for as Concrete Superstructure (Approach Slab).
4. Approach footing concrete shall be paid for as Concrete Structures.
5. The approach footing maximum applied service bearing pressure (Q_{max}) = 2.0 ksf.
6. Cost of excavation for approach footing included with Concrete Structures.
7. For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 46.



SN 025-0116
WEST APPROACH
BILL OF MATERIAL

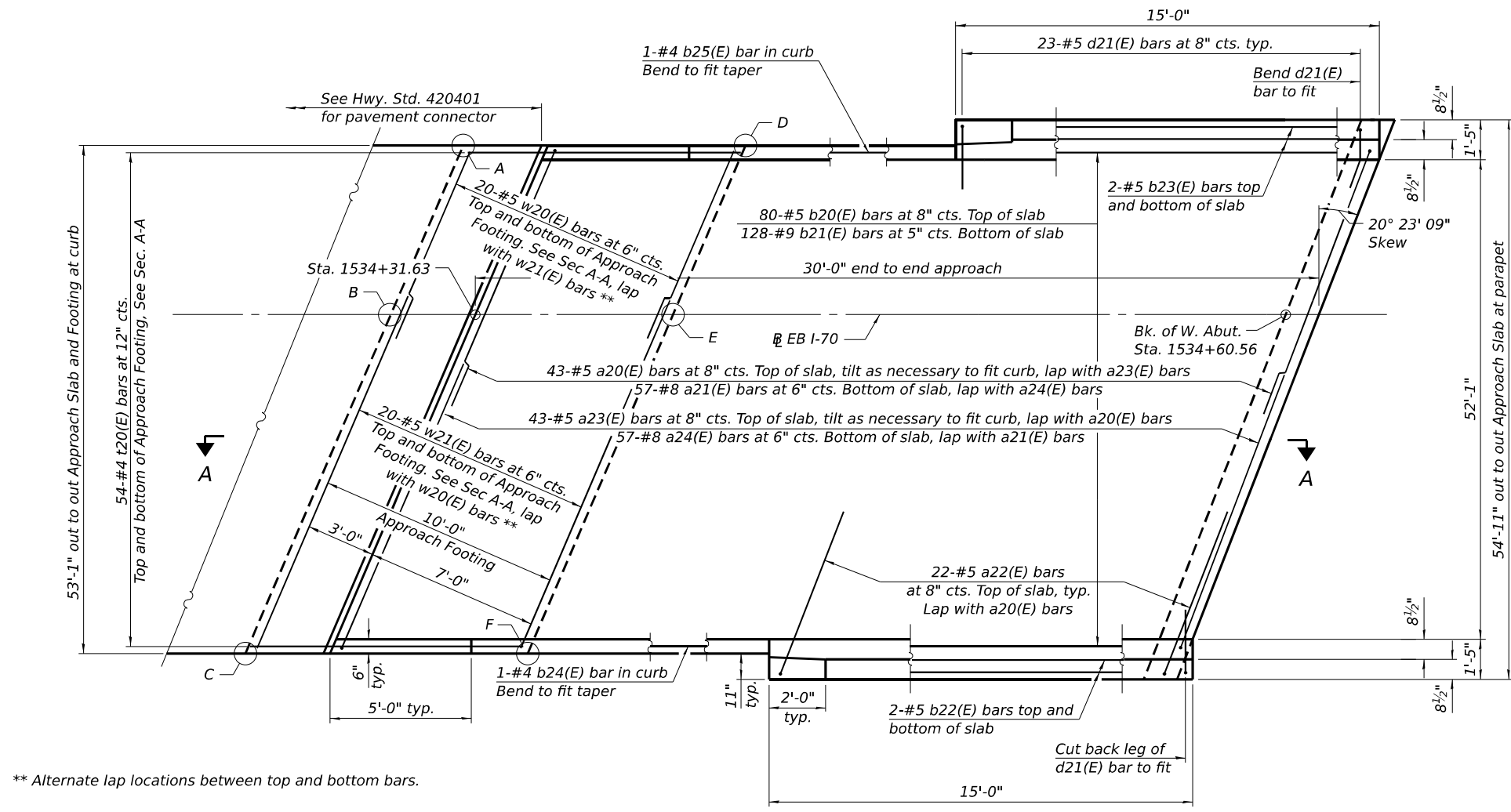
Bar	No.	Size	Length	Shape
a120(E)	43	#5	41'-11"	
a121(E)	57	#8	19'-2"	
a122(E)	44	#5	7'-4"	
a123(E)	43	#5	17'-9"	
a124(E)	57	#8	41'-4"	
b120(E)	79	#5	29'-8"	
b121(E)	127	#9	29'-8"	
b122(E)	4	#5	14'-2"	
b123(E)	4	#5	14'-11"	
b124(E)	1	#4	14'-8"	
b125(E)	1	#4	14'-7"	
d120(E)	46	#5	7'-0"	
d121(E)	46	#5	8'-6"	
e120(E)	24	#4	14'-8"	
t120(E)	106	#4	10'-4"	
w120(E)	40	#5	39'-11"	
w121(E)	40	#5	19'-2"	
Concrete Structures			Cu. Yd.	17.4
Concrete Superstructure			Cu. Yd.	4.3
Concrete Superstructure (Approach Slab)			Cu. Yd.	75.8
Reinforcement Bars, Epoxy Coated			Pound	31,800



SN 025-0116
EAST APPROACH
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a130(E)	43	#5	41'-11"	
a131(E)	57	#8	19'-2"	
a132(E)	44	#5	7'-4"	
a133(E)	43	#5	18'-7"	
a134(E)	57	#8	42'-0"	
b130(E)	80	#5	29'-10"	
b131(E)	128	#9	29'-10"	
b132(E)	4	#5	14'-11"	
b133(E)	4	#5	14'-2"	
b134(E)	1	#4	14'-6"	
b135(E)	1	#4	14'-11"	
d130(E)	46	#5	7'-0"	
d131(E)	46	#5	8'-6"	
e130(E)	24	#4	14'-8"	
t130(E)	108	#4	10'-4"	
w130(E)	40	#5	41'-0"	
w131(E)	40	#5	18'-9"	
Concrete Structures			Cu. Yd.	17.5
Concrete Superstructure			Cu. Yd.	4.3
Concrete Superstructure (Approach Slab)			Cu. Yd.	76.2
Reinforcement Bars, Epoxy Coated			Pound	32,190

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**TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING**

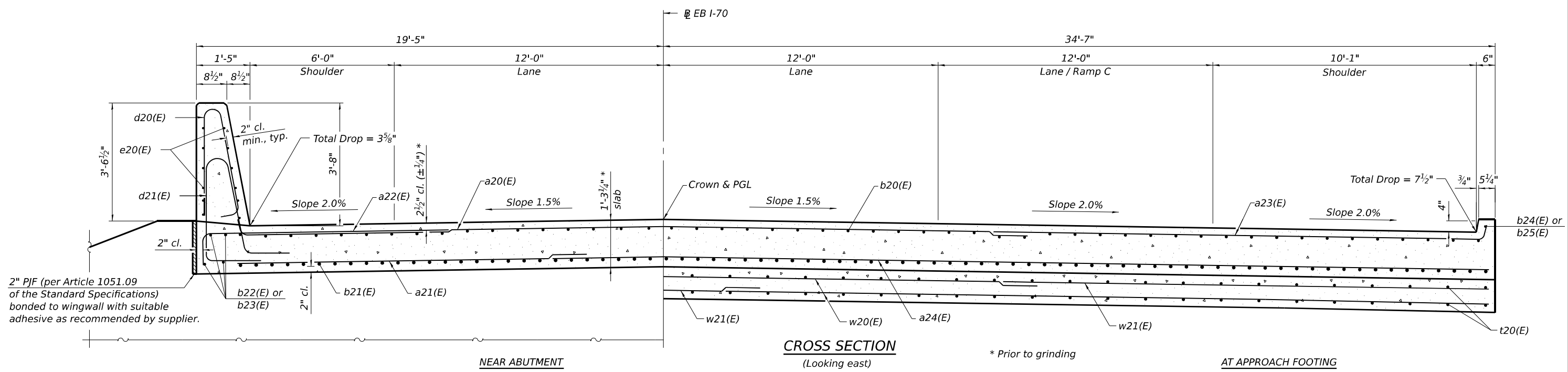
SN 025-0115 West Approach		
Point/ Location	Top	Bottom
A - NW Corner	631.75	630.92
B - W PGL	632.20	631.37
C - SW Corner	631.82	630.99
D - NE Corner	631.53	630.70
E - E PGL	631.98	631.15
F - SE Corner	631.61	630.78

MINIMUM BAR LAP

#5 bar = 3'-4"
#8 bar = 4'-9"

Note:
1. For Section A-A, see sheet 27 of 46.

** Alternate lap locations between top and bottom bars.



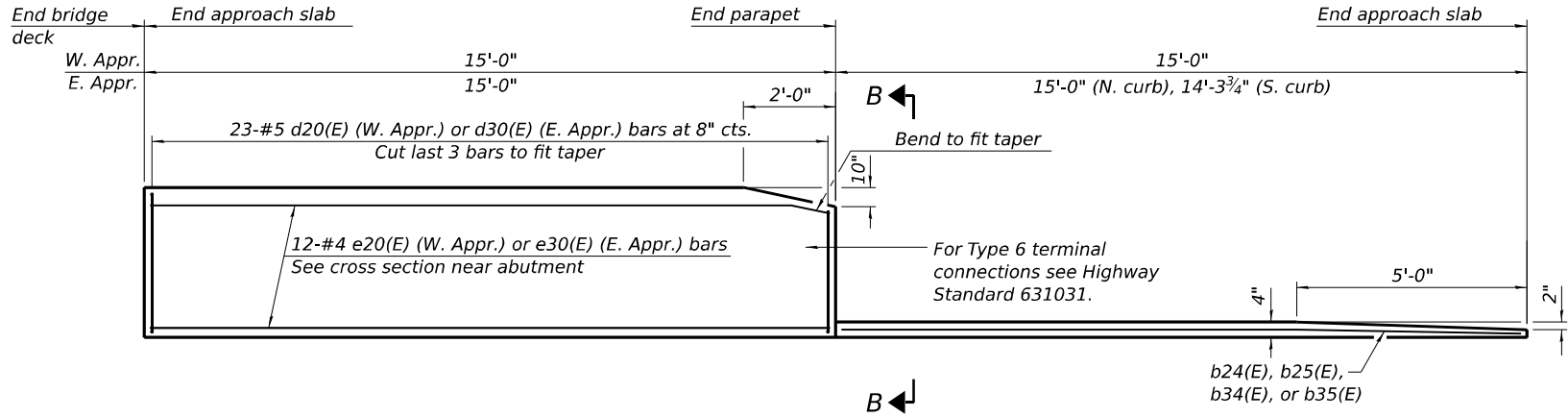
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PLOT DATE = 8/5/2024	DATE - 08/06/2024	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

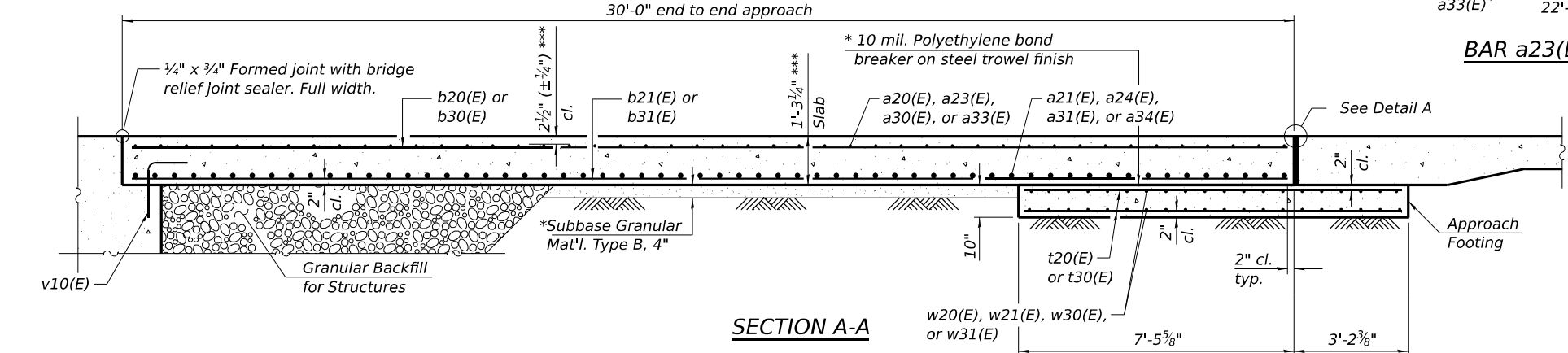
BRIDGE APPROACH SLAB DETAILS - EB - I
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

SCALE: SHEET 25 OF 46 SHEETS STA. TO STA.

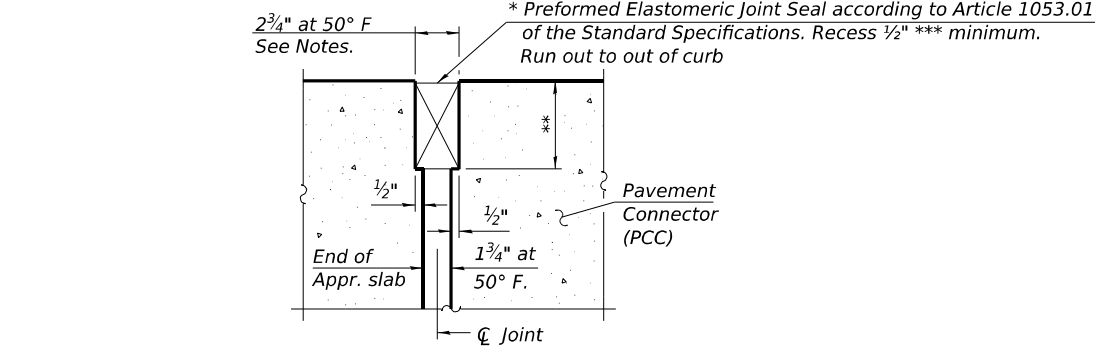
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	410
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				



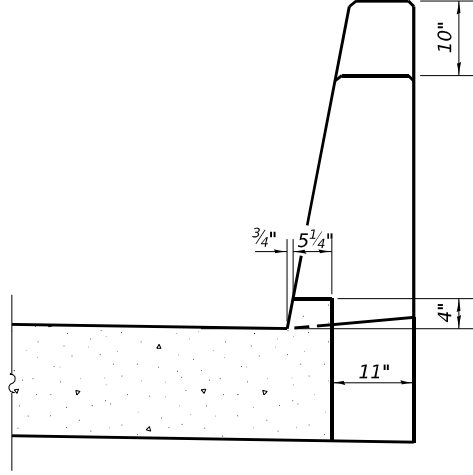
INSIDE ELEVATION OF PARAPET AND CURB



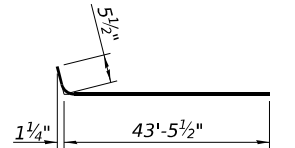
SECTION A-A



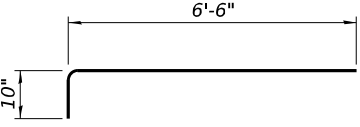
DETAIL A
(at Rt. L's)



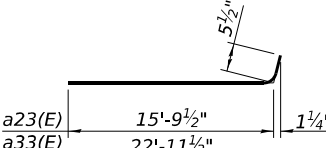
VIEW B-B



BAR a20(E) & a30(E)

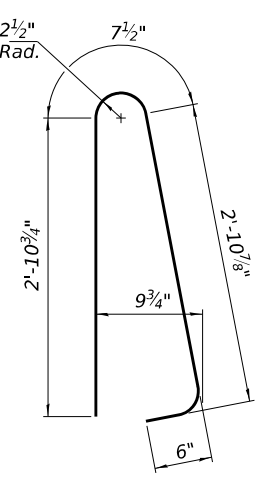


BAR a22(E) & a32(E)

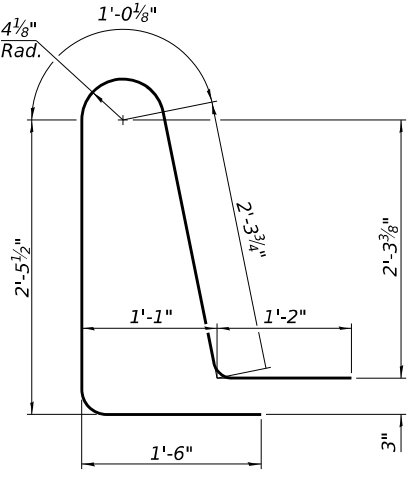


BAR a23(E) & a33(E)

- Notes:
1. The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
 2. Parapet concrete shall be paid for as Concrete Superstructure.
 3. Approach slab shall be paid for as Concrete Superstructure (Approach Slab).
 4. Approach footing concrete shall be paid for as Concrete Structures.
 5. The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
 6. Cost of excavation for approach footing included with Concrete Structures.
 7. For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 46.



BAR d20(E)
& d30(E)



BAR d21(E)
& d31(E)

SN 025-0115
WEST APPROACH
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a20(E)	43	#5	43'-11"	
a21(E)	57	#8	24'-10"	
a22(E)	44	#5	7'-4"	
a23(E)	43	#5	16'-3"	
a24(E)	57	#8	36'-3"	
b20(E)	80	#5	29'-8"	
b21(E)	128	#9	29'-8"	
b22(E)	4	#5	14'-2"	
b23(E)	4	#5	14'-11"	
b24(E)	1	#4	14'-9"	
b25(E)	1	#4	14'-7"	
d20(E)	46	#5	7'-0"	
d21(E)	46	#5	8'-6"	
e20(E)	24	#4	14'-8"	
t20(E)	108	#4	10'-4"	
w20(E)	40	#5	34'-10"	
w21(E)	40	#5	24'-10"	
Concrete Structures			Cu. Yd.	17.5
Concrete Superstructure			Cu. Yd.	4.3
Concrete Superstructure (Approach Slab)			Cu. Yd.	76.5
Reinforcement Bars, Epoxy Coated			Pound	32,080

SN 025-0115
EAST APPROACH
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a30(E)	43	#5	43'-11"	
a31(E)	57	#8	24'-10"	
a32(E)	44	#5	7'-4"	
a33(E)	43	#5	23'-5"	
a34(E)	57	#8	43'-1"	
b30(E)	90	#5	29'-8"	
b31(E)	144	#9	29'-8"	
b32(E)	4	#5	14'-10"	
b33(E)	4	#5	14'-2"	
b34(E)	1	#4	13'-10"	
b35(E)	1	#4	14'-8"	
d30(E)	46	#5	7'-0"	
d31(E)	46	#5	8'-6"	
e30(E)	24	#4	14'-8"	
t30(E)	122	#4	10'-4"	
w30(E)	40	#5	43'-1"	
w31(E)	40	#5	23'-8"	
Concrete Structures			Cu. Yd.	19.6
Concrete Superstructure			Cu. Yd.	4.3
Concrete Superstructure (Approach Slab)			Cu. Yd.	84.2
Reinforcement Bars, Epoxy Coated			Pound	35,750



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	DATE - 08/06/2024	REVISED -

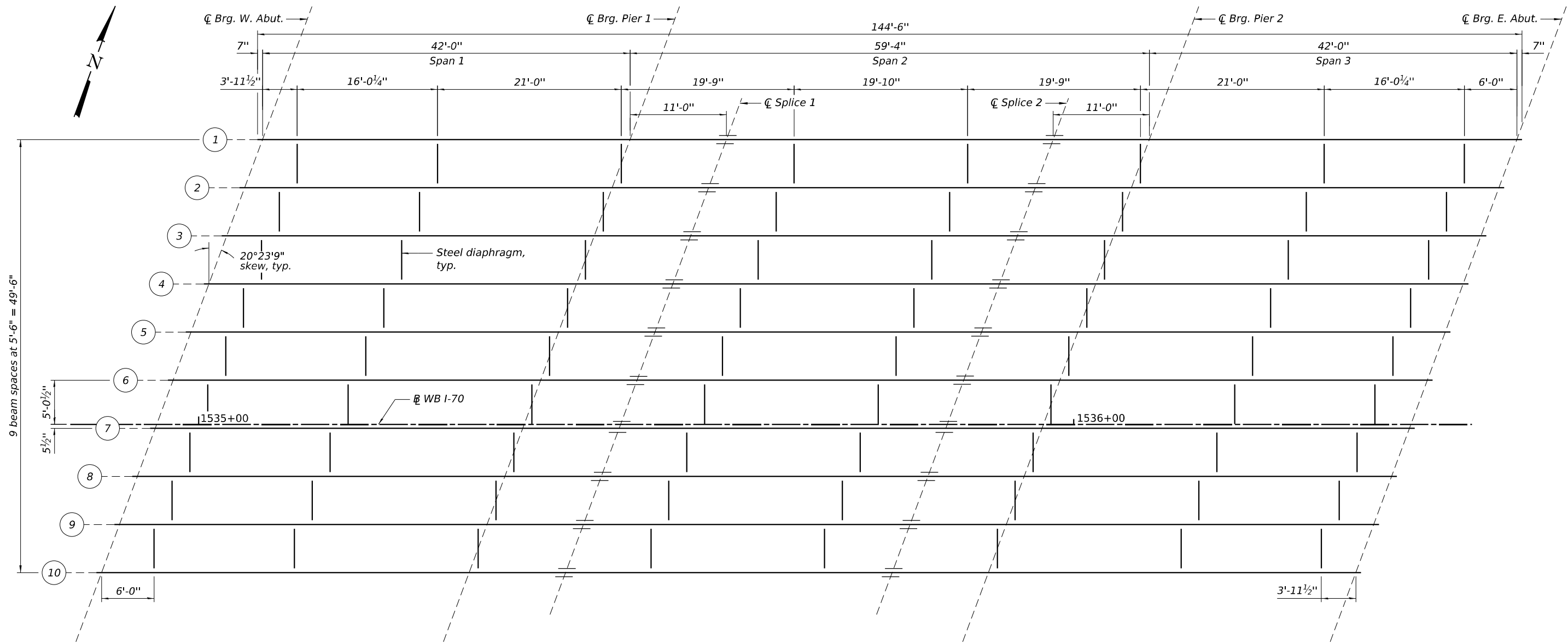
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DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB DETAILS - EB - III
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

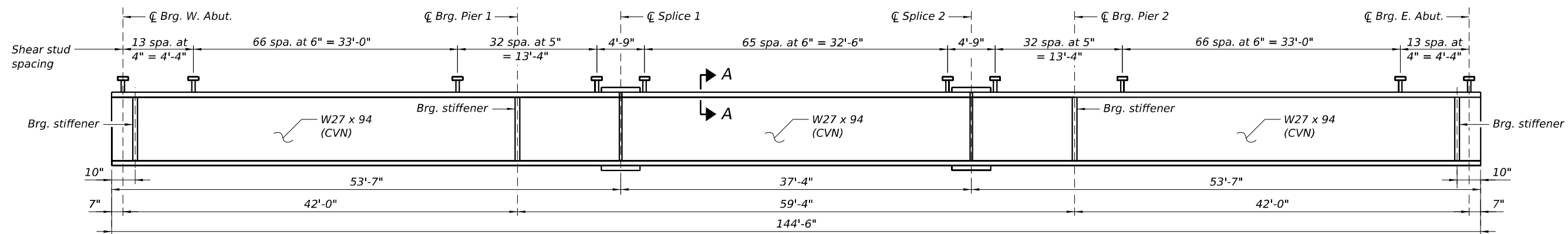
SCALE: SHEET 27 OF 46 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	412
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

FILE NAME: p:\cmt\engr\paw_bentley.com\cm-connect\projects\Documents\DOT22004197\20074893\ICADD Data\Bridg\ICADD Sheets\025-0115\0116-Framing Plan - WB.dgn



FRAMING PLAN



BEAM ELEVATION

- Notes:
1. All steel except bearing plates and diaphragms shall be AASHTO M270 Grade 50.
 2. Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
 3. All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.
 4. See sheet 31 of 46 for Section A-A.



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REVIS	-
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DATE	- 08/06/2024
REVIS	-

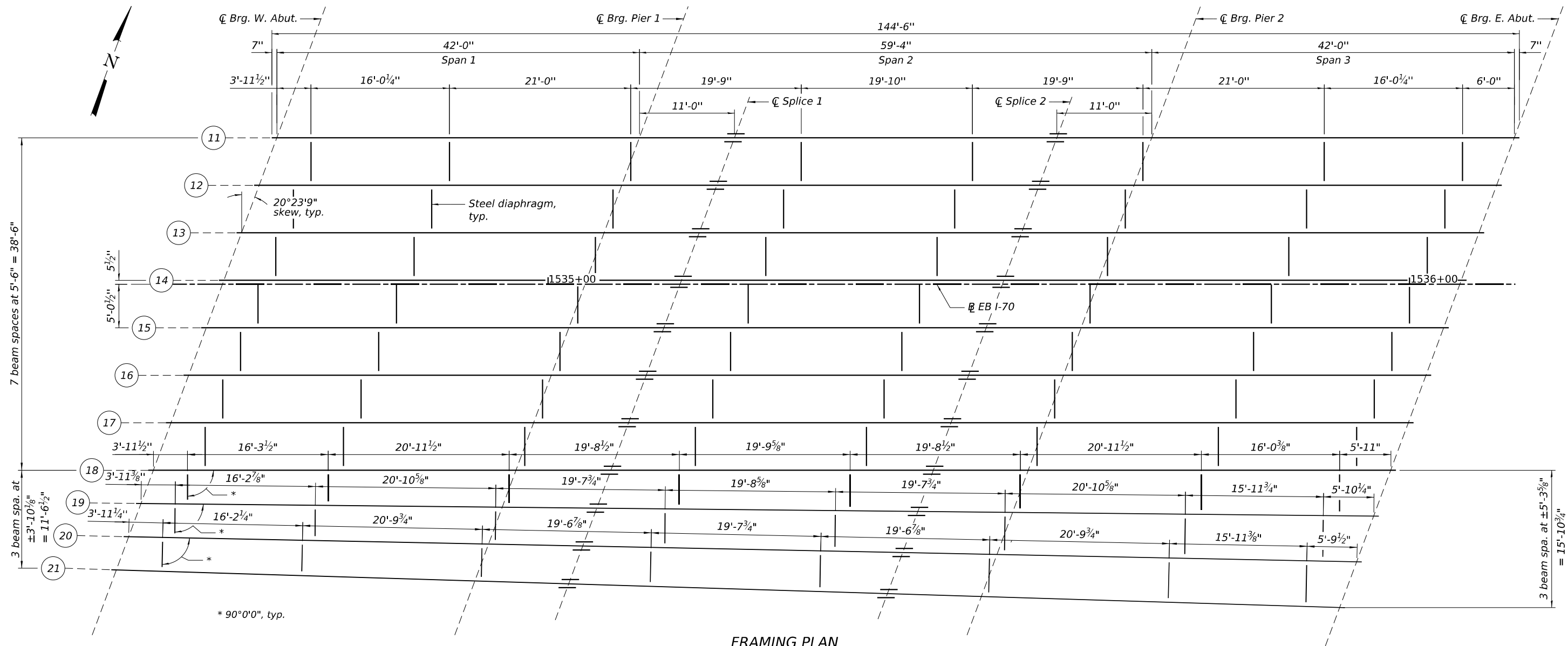
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FRAMING PLAN - WB
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

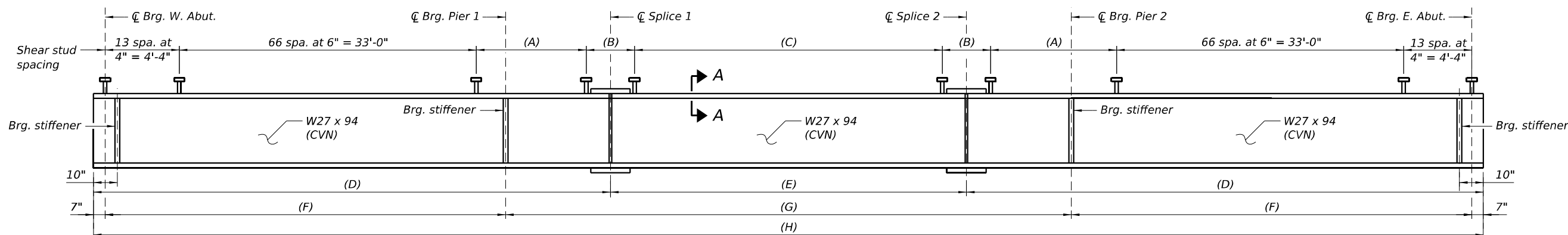
SCALE: SHEET 29 OF 46 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	414
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

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FRAMING PLAN



SHEAR STUDS SPACINGS

BEAM LENGTHS

BEAM ELEVATION

	Beams 11-18	Beam 19	Beam 20	Beam 21
A	32 spa. at 5" = 13'-4"	31 spa. at 5" = 12'-11"	31 spa. at 5" = 12'-11"	30 spa. at 5" = 12'-6"
B	4'-9"	4'-10 ¹³ / ₁₆ "	4'-10 ¹¹ / ₁₆ "	5'-0 ¹¹ / ₁₆ "
C	65 spa. at 6" = 32'-6"	65 spa. at 6" = 32'-6"	64 spa. at 6" = 32'-0"	64 spa. at 6" = 32'-0"

	Beams 11-18	Beam 19	Beam 20	Beam 21
D	53'-7"	53'-4 ⁵ / ₈ "	53'-2 ⁵ / ₁₆ "	53'-0 ¹ / ₈ "
E	37'-4"	37'-2 ³ / ₈ "	37'-0 ³ / ₄ "	36'-11 ¹ / ₈ "
F	42'-0"	41'-10 ¹ / ₂ "	41'-8 ⁵ / ₁₆ "	41'-6 ⁹ / ₁₆ "
G	59'-4"	59'-1 ³ / ₈ "	58'-10 ³ / ₄ "	58'-8 ¹ / ₄ "
H	144'-6"	143'-11 ⁵ / ₈ "	143'-5 ³ / ₈ "	142'-11 ³ / ₈ "

Notes:

- All steel except bearing plates and diaphragms shall be AASHTO M270 Grade 50.
- Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
- All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.
- See sheet 31 of 46 for Section A-A.



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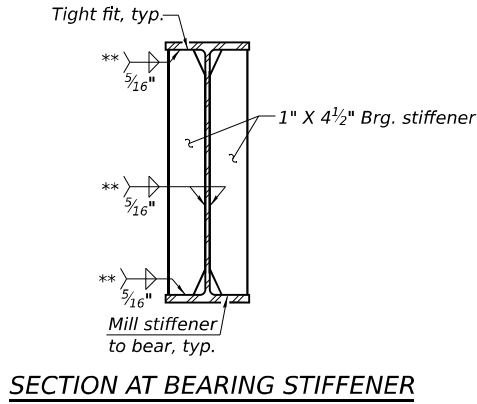
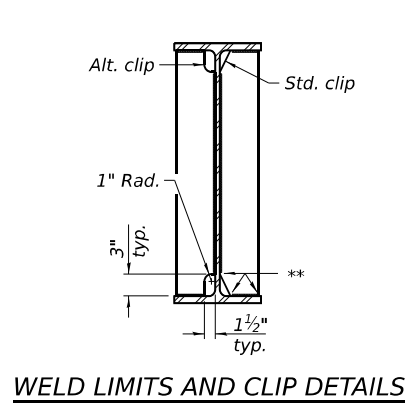
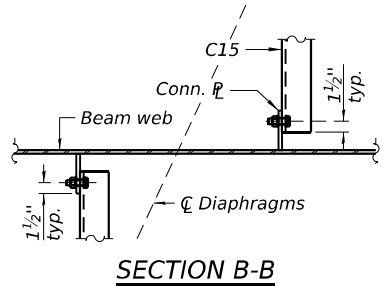
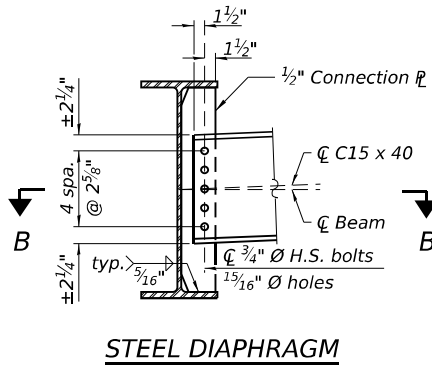
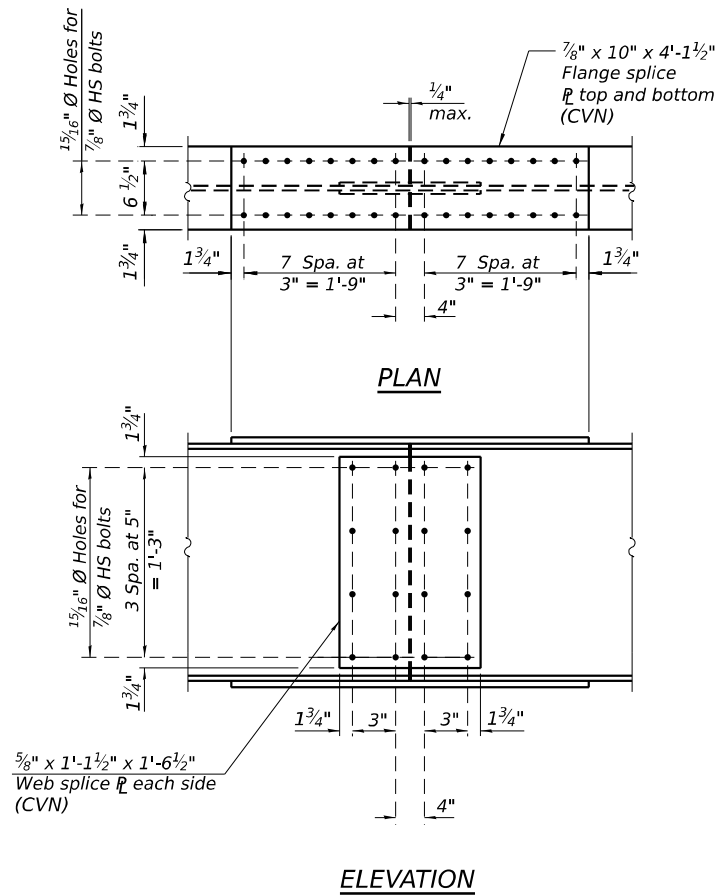
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FRAMING PLAN - EB
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

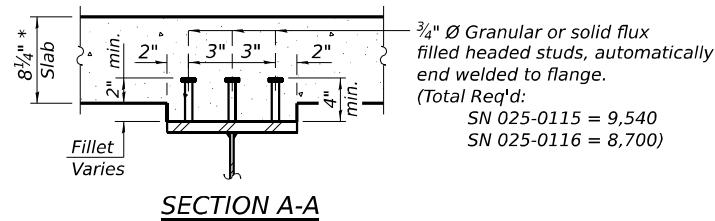
SCALE: SHEET 30 OF 46 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	415
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

FILE NAME: p:\cmt\engr\paw_bentley.com\cmt\connect\projects\Documents\DOT22004197\DOT22004197-0074893\ICADD Data\Bridge\ICADD Sheets\025-0115\0116-74893-431-Structural Steel Details - I



** Stop welds 1/4" ($\pm 1/8$ ") from edges as shown. Typical.



* Prior to grinding

SN 025-0116 TOP OF BEAM ELEVATIONS
(For fabrication only)

	Beam 1	Beam 2	Beam 3	Beam 4	Beam 5	Beam 6	Beam 7	Beam 8	Beam 9	Beam 10
⌀ Brg. W. Abut.	631.42	631.58	631.73	631.89	632.04	632.17	632.28	632.24	632.20	632.13
⌀ Brg. Pier 1	630.45	630.60	630.76	630.92	631.07	631.20	631.31	631.27	631.23	631.17
⌀ Splice 1	630.19	630.34	630.50	630.66	630.81	630.94	631.05	631.02	630.98	630.92
⌀ Splice 2	629.30	629.46	629.62	629.78	629.93	630.06	630.18	630.14	630.11	630.05
⌀ Brg. Pier 2	629.03	629.19	629.35	629.51	629.67	629.80	629.92	629.88	629.85	629.79
⌀ Brg. E. Abut.	628.01	628.17	628.33	628.50	628.66	628.80	628.92	628.89	628.86	628.79

SN 025-0115 TOP OF BEAM ELEVATIONS
(For fabrication only)

	Beam 11	Beam 12	Beam 13	Beam 14	Beam 15	Beam 16	Beam 17	Beam 18	Beam 19	Beam 20	Beam 21
⌀ Brg. W. Abut.	631.61	631.78	631.90	632.03	632.00	631.96	631.89	631.82	631.78	631.73	631.68
⌀ Brg. Pier 1	630.65	630.81	630.93	631.06	631.04	631.00	630.94	630.88	630.83	630.78	630.73
⌀ Splice 1	630.40	630.55	630.68	630.81	630.79	630.75	630.69	630.63	630.58	630.53	630.48
⌀ Splice 2	629.53	629.69	629.82	629.95	629.93	629.89	629.84	629.78	629.72	629.67	629.62
⌀ Brg. Pier 2	629.27	629.43	629.56	629.70	629.68	629.64	629.59	629.53	629.47	629.41	629.36
⌀ Brg. E. Abut.	628.28	628.45	628.58	628.72	628.70	628.67	628.61	628.55	628.49	628.43	628.38

- Notes:
- All steel except bearing plates and diaphragms shall be AASHTO M270 Grade 50.
 - Load carrying components designated "CVN" shall conform to the Charpy-V-Notch Impact Energy Requirement, Zone 2.
 - Two hardened washers required for each set of oversized holes.
 - Alternate channels of equal depth and larger weight are permitted to facilitate material acquisition. Alternate channels, if utilized, shall be provided at no additional cost to the Department.
 - All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.



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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL DETAILS - I
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

SCALE: SHEET 31 OF 46 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	416
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

FILE NAME: dw:\cmtengr\paw_bentley.com\cmt-connect\projects\Documents\DOT22004197\0074893\ICADD Data\Bridge\CADD_Sheets\025-0115\0116-74893-432-Structural Steel Details - II

Symmetrical 3 span				
INTERIOR GIRDER MOMENT TABLE				
		0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or Pier 2	0.5 Sp. 2
I_s	(in ⁴)	3,270	3,270	3,270
$I_c(n)$	(in ⁴)	9,572	--	9,572
$I_c(3n)$	(in ⁴)	7,193	--	7,193
$I_c(cr)$	(in ⁴)	--	4,648	--
S_s	(in ³)	243	243	243
$S_c(n)$	(in ³)	370	--	370
$S_c(3n)$	(in ³)	336	--	336
$S_c(cr)$	(in ³)	--	284	--
S_x	(in ³)	--	--	--
DC1	(k')	0.69	0.69	0.69
M _{DC1}	('k)	72	188	118
DC2	(k')	0.19	0.19	0.19
M _{DC2}	('k)	20	51	32
DW	(k')	0.26	0.26	0.26
M _{DW}	('k)	27	70	44
LLDF		0.56	0.55	0.53
M _{L + IM}	('k)	364	355	414
f_t (Strength I)	(ksi)	0.0	0.0	0.0
$M_u + \frac{1}{3} f_t S_x$	('k)	791	1,025	979
$\phi_t M_n$	('k)	1,889	--	1,889
$f_s DC1$	(ksi)	3.6	9.3	5.8
$f_s DC2$	(ksi)	0.7	2.2	1.2
$f_s DW$	(ksi)	1.0	2.9	1.6
$f_s (L+IM)$	(ksi)	11.8	15.0	13.4
f_t (Service II)	(ksi)	0.0	0.0	0.0
$f_s + \frac{1}{2} f_t$ (Service II)	(ksi)	20.6	33.9	26.0
Service II Resistance	(ksi)	47.5	47.5	47.5
$f_s + \frac{1}{3} f_t$ (Strength I)	(ksi)	27.4	45.0	34.6
$\phi_t F_n$	(ksi)	--	49.0	--
V_f	(k)	35.3	90.8	34.8

GIRDER REACTION TABLE		
	Abut.	Pier
LLDF	0.681	0.634
OCF	1.07	1.00
R _{DC1}	(k) 10.1	39.7
R _{DC2}	(k) 2.8	10.8
R _{DW}	(k) 3.8	14.7
R _L	(k) 44.6	67.6
R _{IM}	(k) 11.9	14.3
R _{Total} (Strength I)(Impact)	(k) 120.7	228.4
R _{Total} (Strength I)(No Impact)	(k) 99.8	203.4

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in.⁴ and in.³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

$I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.⁴ and in.³).

S_x : Section modulus about the major axis of a section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

LLDF: Live Load Distribution Factor for moment and shear computed according to Article 4.6.2.2 and further IDOT provisions.

M_{L + IM} : Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u : Strength I load combination of factored design moments (kip-ft.).
1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{L + IM}

f_t : Factored calculated flange lateral bending stress as calculated using Article 6.10.1.6 and as further simplified by IDOT provisions (ksi).

$\phi_t M_n$: Factored nominal flexural resistance of the section determined as specified in Article 6.10.7.1 or A6 as applicable (kip-ft.).

$f_s DC1$: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
M_{DC1} / S_s

$f_s DC2$: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
M_{DC2} / S_c (3n) or M_{DC2} / S_c(cr) as applicable.

$f_s DW$: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
M_{DW} / S_c(3n) or M_{DW} / S_c(cr) as applicable.

$f_s (L + IM)$: Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
M_{L + IM} / S_c(n) or M_{L + IM} / S_c(cr) as applicable.

$f_s + f_t/2$ (Service II): Sum of stresses as computed below (ksi).
 $f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (L + IM) + f_t/2$

Service II Resistance: Composite (0.95R_n F_{yf}) or noncomposite (0.80R_n F_{yf}) stress capacity according to Article 6.10.4.2 (ksi).

$f_s + f_t/3$ (Strength I): Sum of stresses as computed below on non-compact sections (ksi).
1.25 (f_s DC1 + f_s DC2) + 1.5 f_s DW + 1.75 f_s (L + IM) + f_t/3

$\phi_f F_n$: Factored nominal flexural resistance of the section as specified in Article 6.10.7.2 or 6.10.8 as applicable (ksi).

V_f : Maximum factored shear range in span computed according to Article 6.10.10.

OCF: Obtuse Correction Factor according to Article 4.6.2.2.3c or as further simplified by IDOT provisions.

R_{DC1} : Un-factored reaction due to non-composite dead load (kip).

R_{DC2} : Un-factored reaction due to long-term composite (superimposed excluding future wearing surface) dead load (kip).

R_{DW} : Un-factored reaction due to long-term composite (superimposed future wearing surface only) dead load (kip).

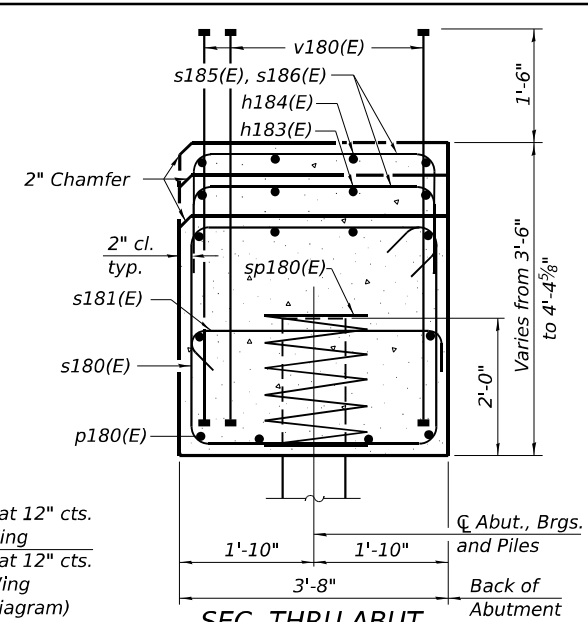
R_L : Un-factored live load reaction (kip).

R_{IM} : Un-factored dynamic load allowance (impact) (kip).

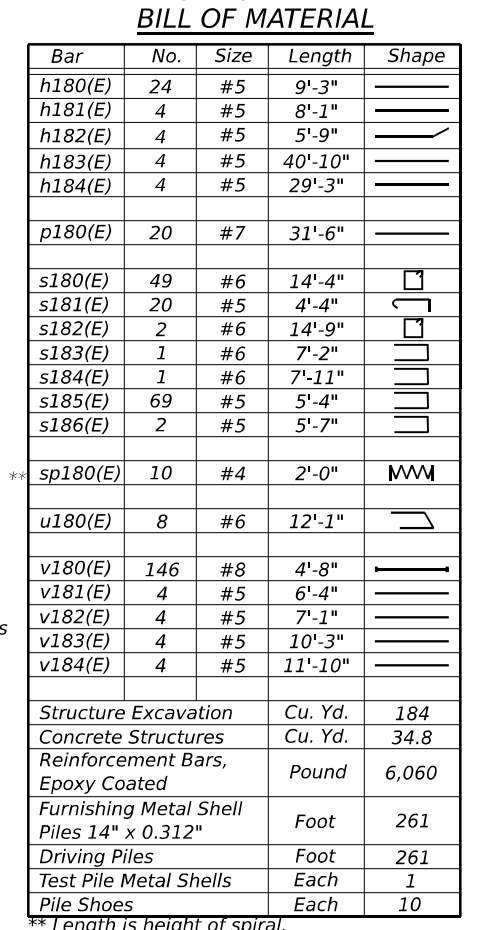
R_{Total} (Strength I)(Impact): Strength I load combination of factored design reactions (kip).
1.25 (R_{DC1} + R_{DC2}) + 1.5R_{DW} + 1.75 (R_L + R_{IM})

R_{Total} (Strength I)(No Impact): Strength I load combination of factored design reactions, not including dynamic load allowance (Impact) (kip).
1.25 (R_{DC1} + R_{DC2}) + 1.5R_{DW} + 1.75 (R_L)

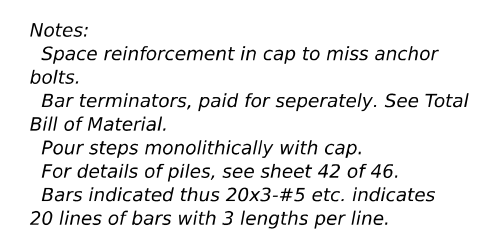
Notes:
1. M_L and R_L include the effects of centrifugal force and superelevation.
2. Demand on beams 18 through 21 of SN 025-0115 is less than that for interior beams shown due to reduced beam spacing.



SEC. THRU ABUT. Abu
Dimensions at right angles to abutment.



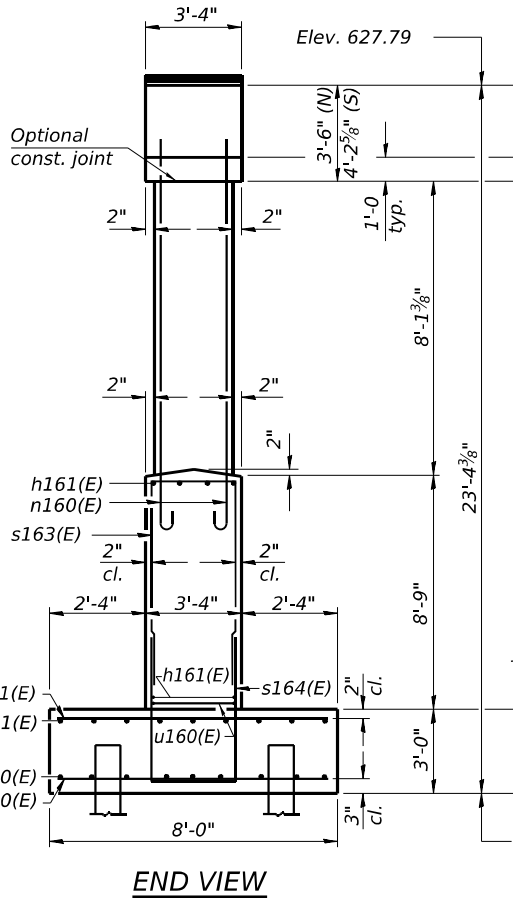
** Length is height of spiral.



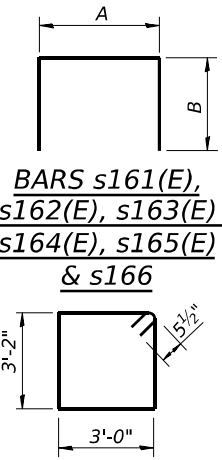
Notes:
Space reinforcement in cap to miss anchor bolts.
Pour steps monolithically with cap.
For details of piles, see sheet 42 of 46.
Bars indicated thus 20x3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
Concrete Sealer to be applied to all exposed areas of the proposed crash wall, columns and cap.

PILE DATA

Type: Metal Shell 14" x 0.312" w/ Pile Shoes
Nominal Required Bearing: 513 kips
Factored Resistance Available: 282 kips
Est. Length: 14 ft
No. Production Piles: 19
No. Test Piles: 1



END VIEW

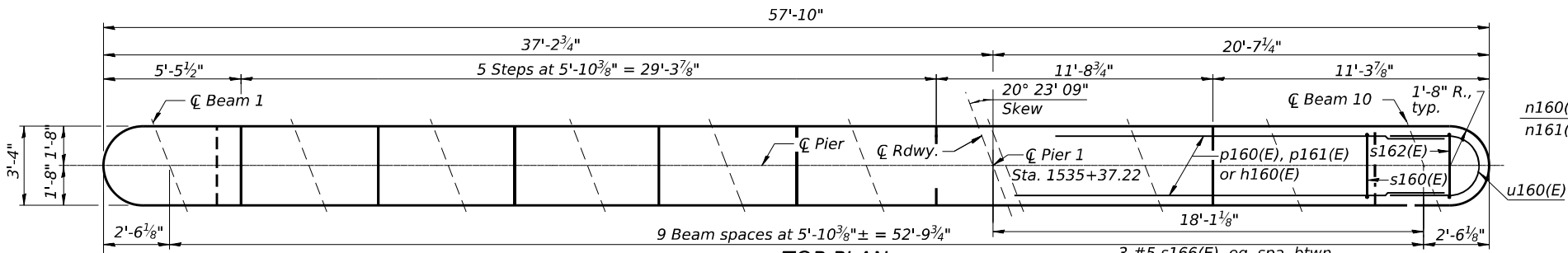


BAR s160(E)

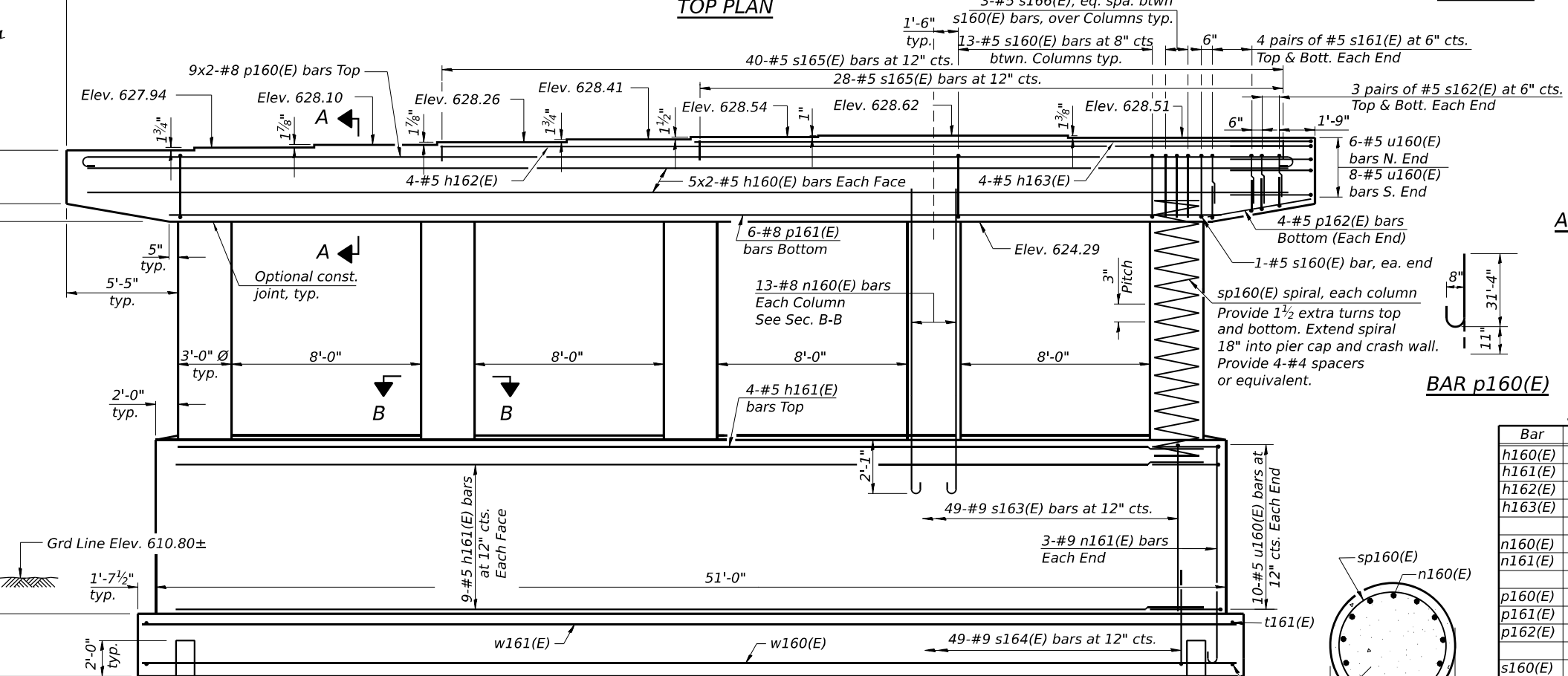
BAR u160(E)

A & B DIMENSIONS

Bar	A	B
s161(E)	3'-0"	2'-9"
s162(E)	3'-0"	2'-6"
s163(E)	3'-0"	8'-5"
s164(E)	3'-0"	7'-4"
s165(E)	3'-0"	1'-0"
s166(E)	3'-0"	3'-2"

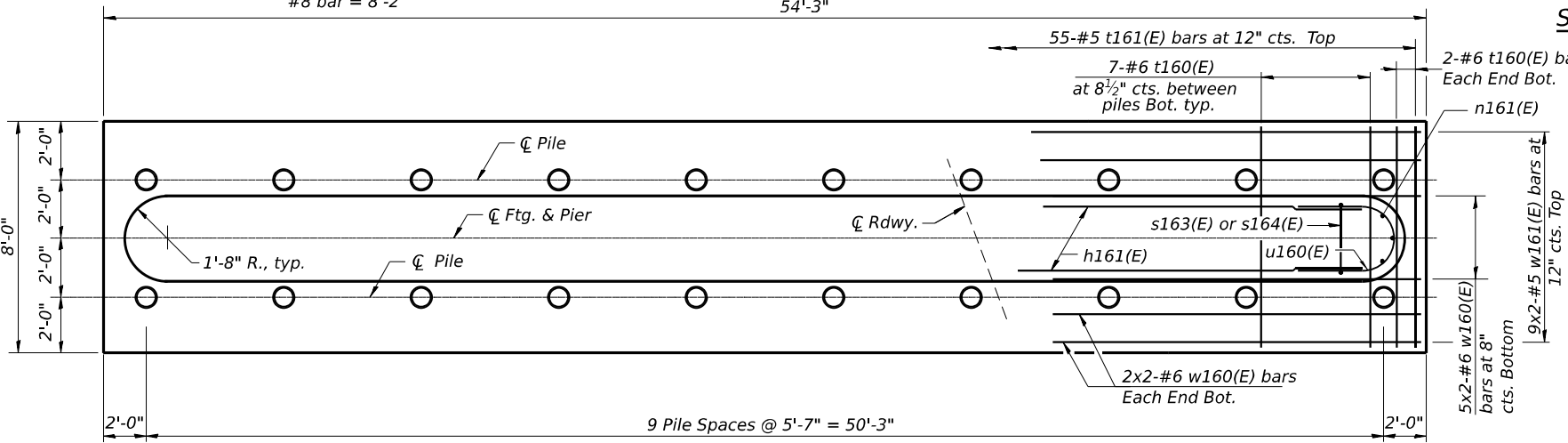


TOP PLAN



ELEVATION
(Looking East)

MINIMUM BAR LAP
#5 bar = 3'-7"
#6 bar = 3'-10"
#8 bar = 8'-2"



FOOTING PLAN

BAR n160(E)
& n161(E)

ANCHOR BOLT LAYOUT

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h160(E)	20	#5	29'-1"	—
h161(E)	22	#5	47'-8"	—
h162(E)	4	#5	38'-7"	—
h163(E)	4	#5	27'-0"	—
n160(E)	65	#8	14'-5"	U
n161(E)	6	#9	12'-7"	U
p160(E)	18	#8	32'-3"	U
p161(E)	6	#8	47'-10"	—
p162(E)	8	#5	5'-0"	—
s160(E)	54	#5	13'-3"	□
s161(E)	16	#5	8'-6"	□
s162(E)	12	#5	8'-0"	□
s163(E)	49	#9	19'-10"	□
s164(E)	49	#9	17'-8"	□
s165(E)	68	#5	5'-0"	□
s166(E)	15	#5	9'-4"	□
sp160(E)	5	#5	11'-2"	~
t160(E)	67	#6	7'-8"	—
t161(E)	55	#5	7'-8"	—
u160(E)	34	#5	11'-6"	U
w160(E)	18	#6	28'-10"	—
w161(E)	18	#5	28'-8"	—
Structure Excavation				Cu. Yd. 143
Concrete Structures				Cu. Yd. 141.6
Reinforcement Bars, Epoxy Coated				Pound 19,850
Furnishing Metal Shell Piles 14" x 0.312"				Foot 266
Driving Piles				Foot 266
Test Pile Metal Shells				Each 1
Pile Shoes				Each 20
Concrete Sealer				Sq. Ft. 1,156

** Length is height of spiral.

MODEL: WP_Pier_1 (Sheet)
FILE NAME: p:\projects\p14\pier1\pier1.dwg
PROJECT: 025-0115 EB & 025-0116 WB
SHEET: 70 OF 70
DATE: 08/06/2024
DRAWN: RBT
CHECKED: TJZ
DESIGNED: RBT
REVISIONS:
1. 08/06/2024 RBT
2. 08/06/2024 TJZ
3. 08/06/2024 RBT
4. 08/06/2024 TJZ
5. 08/06/2024 RBT
6. 08/06/2024 TJZ
7. 08/06/2024 RBT
8. 08/06/2024 TJZ
9. 08/06/2024 RBT
10. 08/06/2024 TJZ
11. 08/06/2024 RBT
12. 08/06/2024 TJZ
13. 08/06/2024 RBT
14. 08/06/2024 TJZ
15. 08/06/2024 RBT
16. 08/06/2024 TJZ
17. 08/06/2024 RBT
18. 08/06/2024 TJZ
19. 08/06/2024 RBT
20. 08/06/2024 TJZ
21. 08/06/2024 RBT
22. 08/06/2024 TJZ
23. 08/06/2024 RBT
24. 08/06/2024 TJZ
25. 08/06/2024 RBT
26. 08/06/2024 TJZ
27. 08/06/2024 RBT
28. 08/06/2024 TJZ
29. 08/06/2024 RBT
30. 08/06/2024 TJZ
31. 08/06/2024 RBT
32. 08/06/2024 TJZ
33. 08/06/2024 RBT
34. 08/06/2024 TJZ
35. 08/06/2024 RBT
36. 08/06/2024 TJZ
37. 08/06/2024 RBT
38. 08/06/2024 TJZ
39. 08/06/2024 RBT
40. 08/06/2024 TJZ
41. 08/06/2024 RBT
42. 08/06/2024 TJZ
43. 08/06/2024 RBT
44. 08/06/2024 TJZ
45. 08/06/2024 RBT
46. 08/06/2024 TJZ
47. 08/06/2024 RBT
48. 08/06/2024 TJZ
49. 08/06/2024 RBT
50. 08/06/2024 TJZ
51. 08/06/2024 RBT
52. 08/06/2024 TJZ
53. 08/06/2024 RBT
54. 08/06/2024 TJZ
55. 08/06/2024 RBT
56. 08/06/2024 TJZ
57. 08/06/2024 RBT
58. 08/06/2024 TJZ
59. 08/06/2024 RBT
60. 08/06/2024 TJZ
61. 08/06/2024 RBT
62. 08/06/2024 TJZ
63. 08/06/2024 RBT
64. 08/06/2024 TJZ
65. 08/06/2024 RBT
66. 08/06/2024 TJZ
67. 08/06/2024 RBT
68. 08/06/2024 TJZ
69. 08/06/2024 RBT
70. 08/06/2024 TJZ



USER NAME	= twendt
DESIGNED	- RBT
DRAWN	- RBT
CHECKED	- TJZ
DATE	- 08/06/2024
PLOT SCALE	= 0.16666633 1/ in.
PLOT DATE	= 8/5/2024

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIER 1 - WB
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

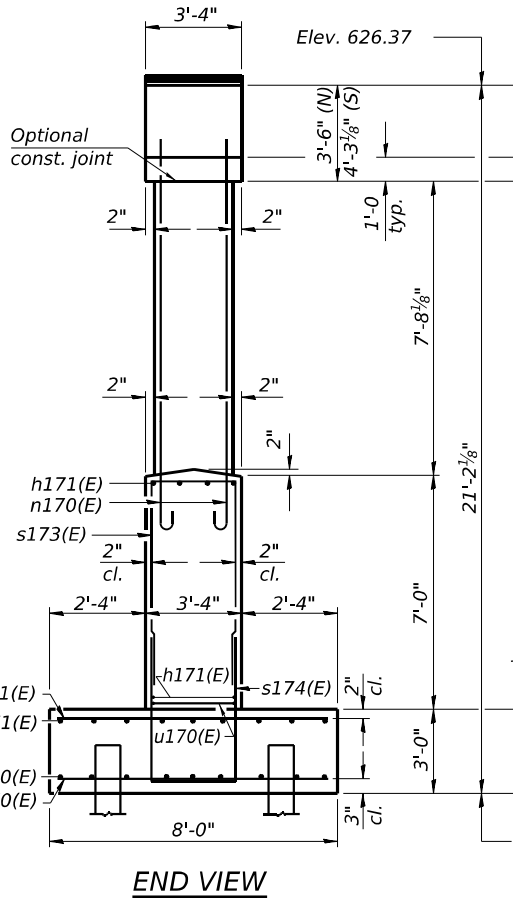
SCALE: SHEET 36 OF 46 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	421
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

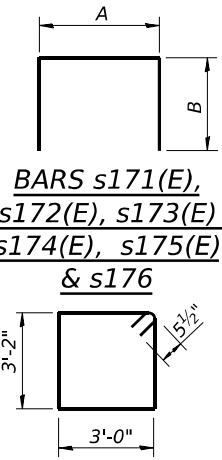
Notes:
Space reinforcement in cap to miss anchor bolts.
Pour steps monolithically with cap.
For details of piles, see sheet 42 of 46.
Bars indicated thus 20x3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
Concrete Sealer to be applied to all exposed areas of the proposed crash wall, columns and cap.

PILE DATA

Type: Metal Shell 14" x 0.312" w/ Pile Shoes
Nominal Required Bearing: 516 kips
Factored Resistance Available: 284 kips
Est. Length: 15 ft
No. Production Piles: 19
No. Test Piles: 1



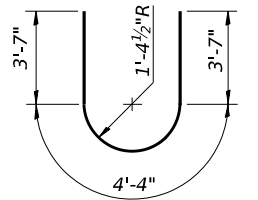
END VIEW



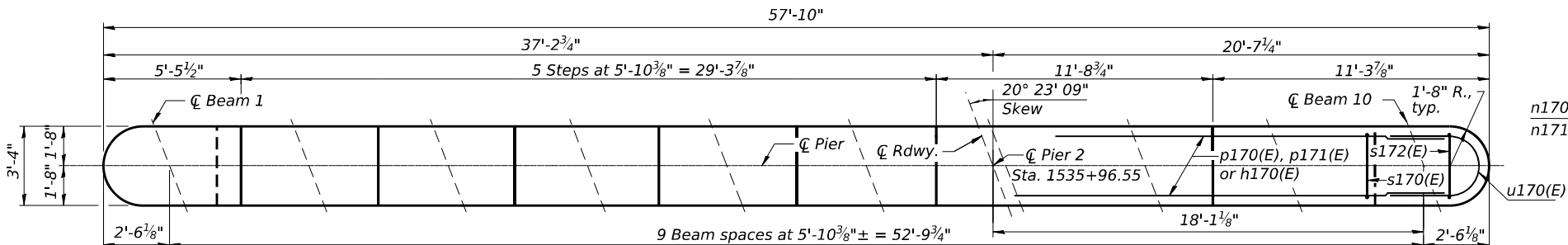
BAR s170(E)

A & B DIMENSIONS

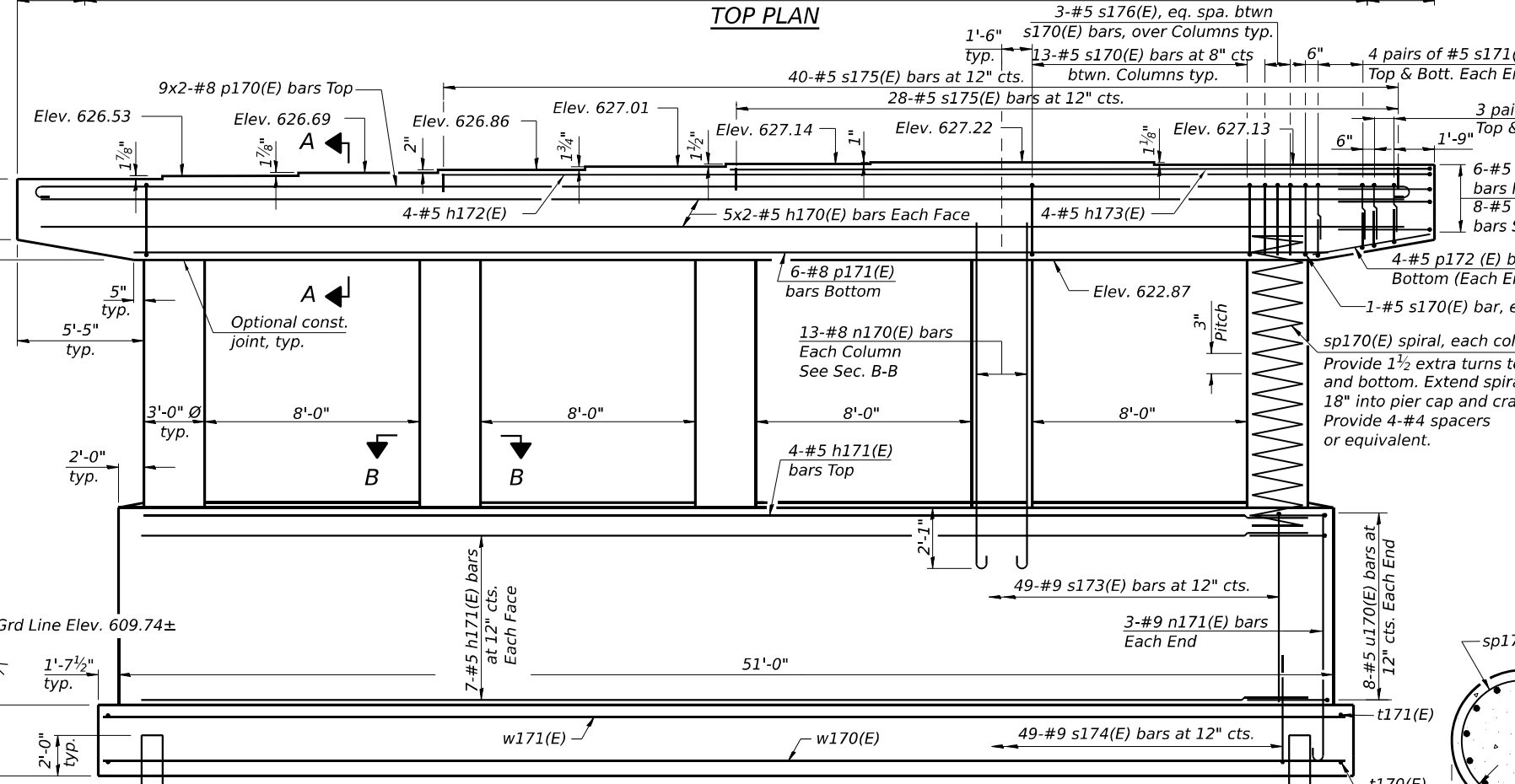
Bar	A	B
s171(E)	3'-0"	2'-9"
s172(E)	3'-0"	2'-6"
s173(E)	3'-0"	6'-8"
s174(E)	3'-0"	7'-4"
s175(E)	3'-0"	1'-0"
s176(E)	3'-0"	3'-2"



BAR u170(E)



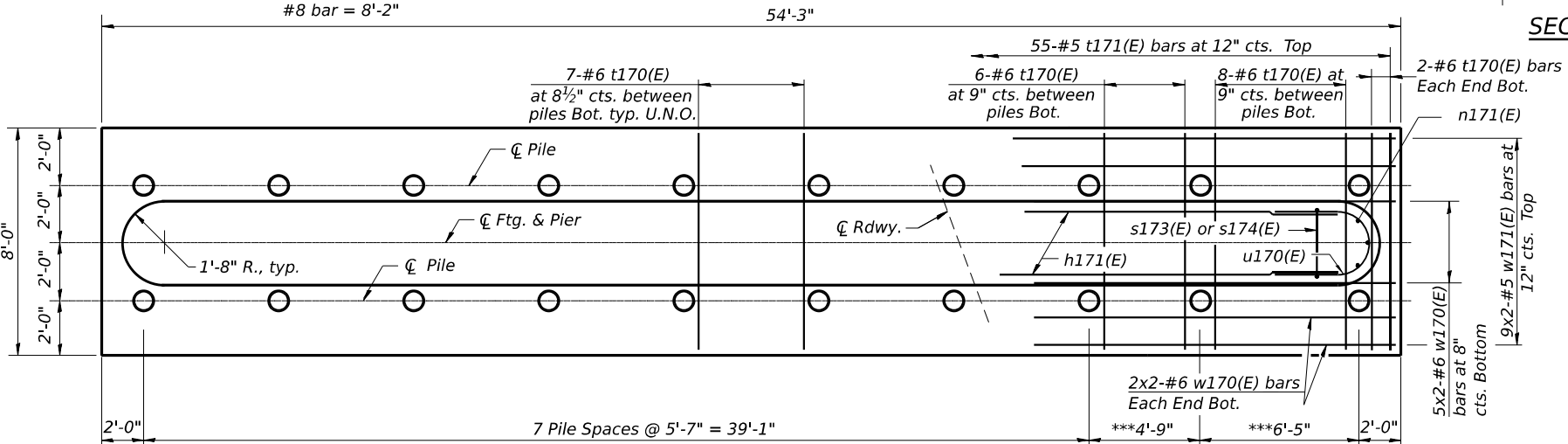
TOP PLAN



ELEVATION
(Looking East)

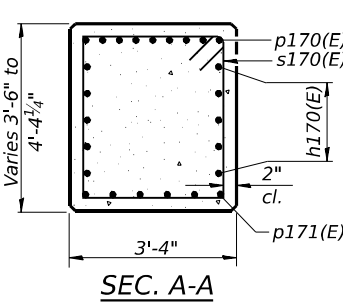
MINIMUM BAR LAP

#5 bar = 3'-7"
#6 bar = 3'-10"
#8 bar = 8'-2"



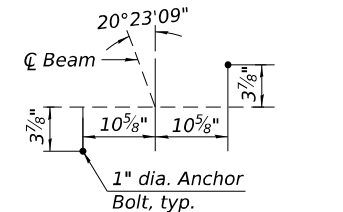
FOOTING PLAN

BAR n170(E)
& n171(E)



SEC. A-A

BAR p170(E)



ANCHOR BOLT LAYOUT

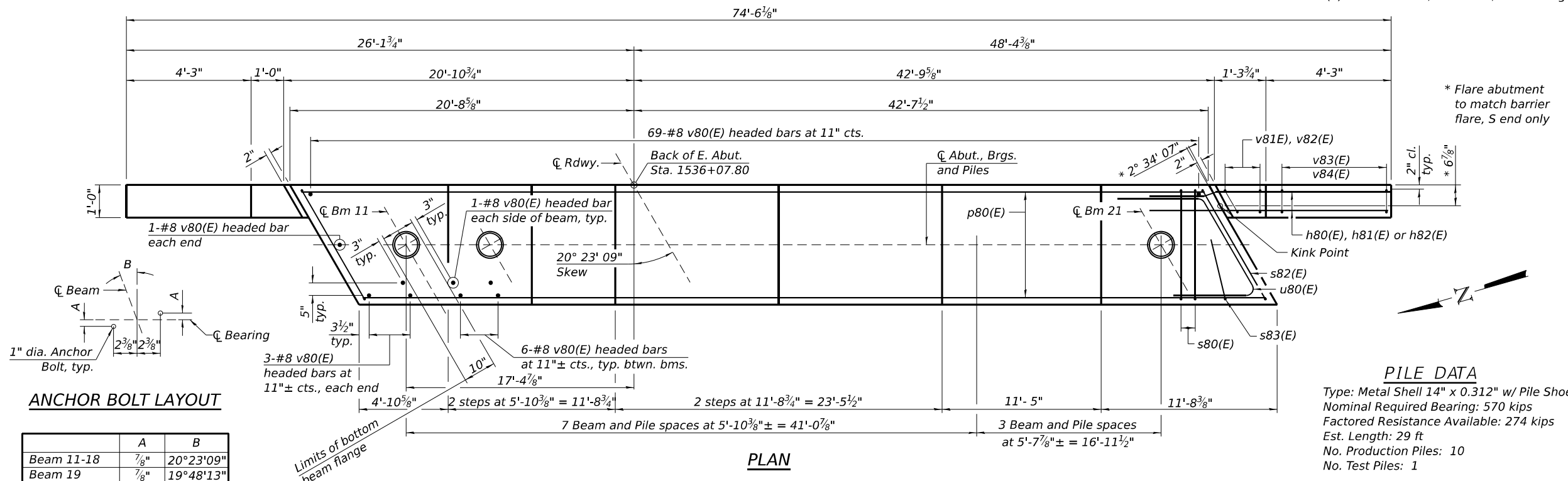
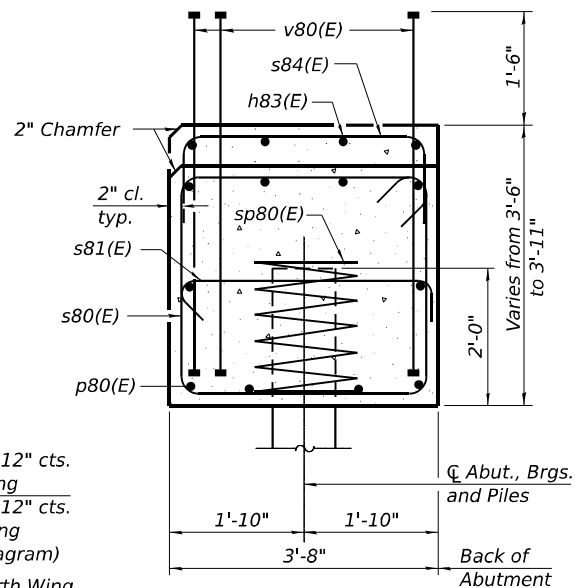
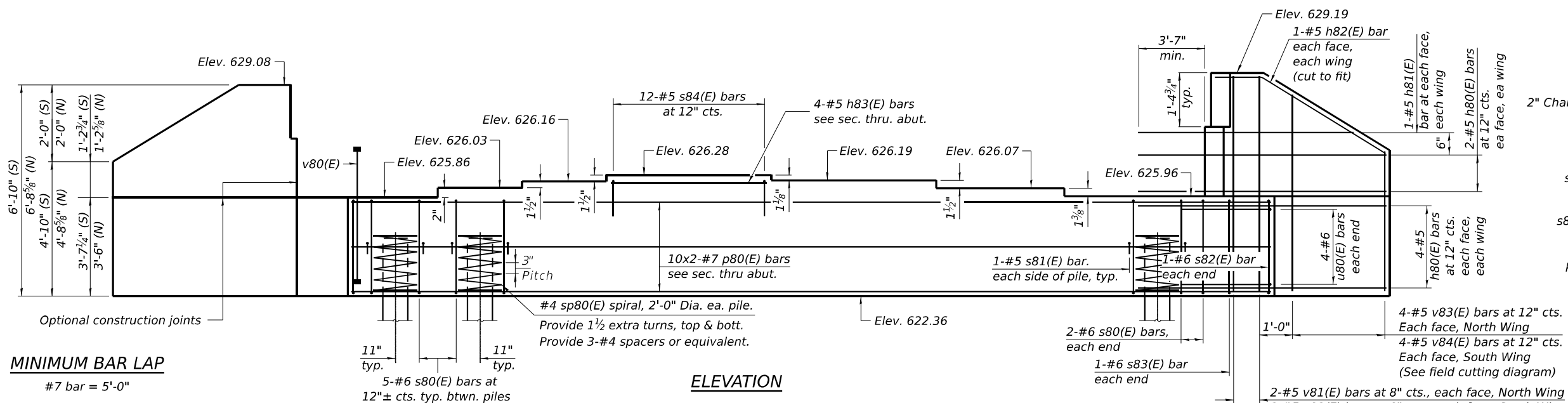
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h170(E)	20	#5	29'-1"	—
h171(E)	18	#5	47'-8"	—
h172(E)	4	#5	38'-7"	—
h173(E)	4	#5	27'-0"	—
n170(E)	65	#8	14'-0"	U
n171(E)	6	#9	10'-10"	U
p170(E)	18	#8	32'-3"	U
p171(E)	6	#8	47'-10"	—
p172(E)	8	#5	5'-0"	—
s170(E)	54	#5	13'-3"	□
s171(E)	16	#5	8'-6"	□
s172(E)	12	#5	8'-0"	□
s173(E)	49	#9	16'-4"	□
s174(E)	49	#9	17'-8"	□
s175(E)	68	#5	5'-0"	□
s176(E)	15	#5	9'-4"	□
sp170(E)	5	#5	10'-9"	~
t170(E)	67	#6	7'-8"	—
t171(E)	55	#5	7'-8"	—
u170(E)	30	#5	11'-6"	U
w170(E)	18	#6	28'-10"	—
w171(E)	18	#5	28'-8"	—
Structure Excavation		Cu. Yd.	92	
Concrete Structures		Cu. Yd.	130.3	
Reinforcement Bars, Epoxy Coated		Pound	18,840	
Furnishing Metal Shell Piles 14" x 0.312"		Foot	285	
Driving Piles		Foot	285	
Test Pile Metal Shells		Each	1	
Pile Shoes		Each	20	
Concrete Sealer		Sq. Ft.	1,151	

** Length is height of spiral.

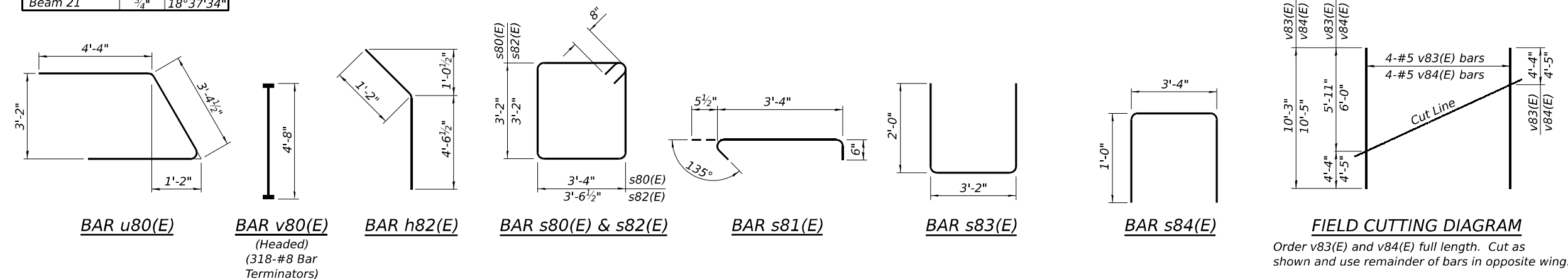
MODEL: WB Pier-2 (Sheet)
FILE NAME: p:\projects\paw_bentley.com\connect\projects\Documents\DOT22004197\20074893\CADD Data\Bri\paw_CADD_Sheets\025-0115\025-0116-24893-2-WB.dgn

MODEL: EB East Abutment2 (Sheet)
FILE NAME: p:\concrete\projects\025-0115\025-0115-EB-EB.dgn
CADD Data Bridge CADD Sheets: 025-0115-EB-EB.dgn



ANCHOR BOLT LAYOUT

	A	B
Beam 11-18	7/8"	20°23'09"
Beam 19	7/8"	19°48'13"
Beam 20	7/8"	19°13'00"
Beam 21	3/4"	18°37'34"



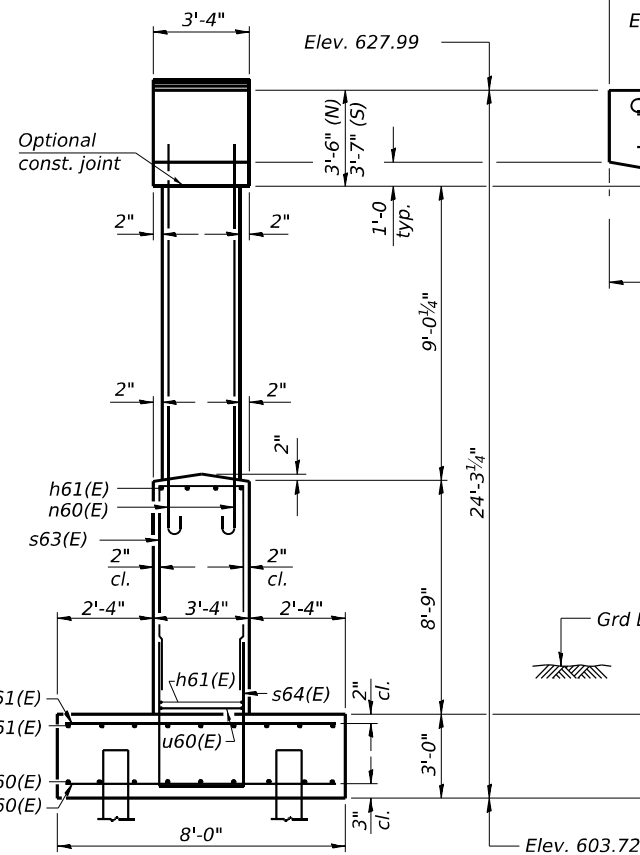
Bar	No.	Size	Length	Shape
h80(E)	24	#5	9'-3"	
h81(E)	4	#5	8'-1"	
h82(E)	4	#5	5'-9"	
h83(E)	4	#5	11'-2"	
p80(E)	20	#7	34'-1"	
s80(E)	54	#6	14'-4"	
s81(E)	22	#5	4'-4"	
s82(E)	2	#6	14'-9"	
s83(E)	2	#6	7'-2"	
s84(E)	12	#5	5'-4"	
sp80(E)	11	#4	2'-0"	
u80(E)	8	#6	12'-1"	
v80(E)	159	#8	4'-8"	
v81(E)	4	#5	6'-4"	
v82(E)	4	#5	6'-5"	
v83(E)	4	#5	10'-3"	
v84(E)	4	#5	10'-5"	
Structure Excavation		Cu. Yd.	184	
Concrete Structures		Cu. Yd.	34.7	
Reinforcement Bars, Epoxy Coated		Pound	5,900	
Furnishing Metal Shell Piles 14" x 0.312"		Foot	290	
Driving Piles		Foot	290	
Test Pile Metal Shells		Each	1	
Pile Shoes		Each	11	

** Length is height of spiral.

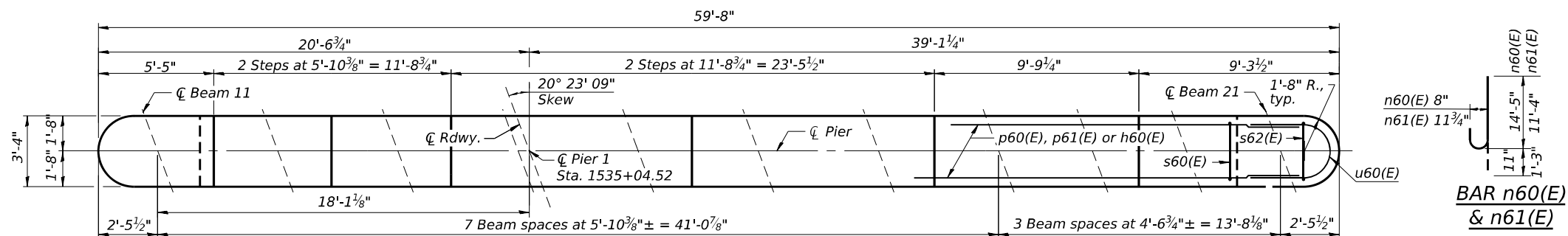
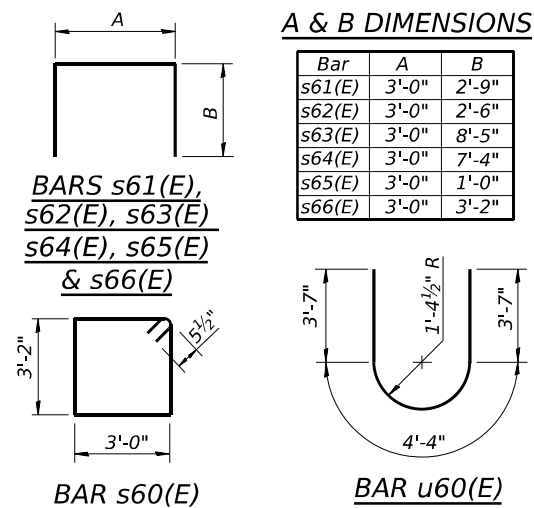
Notes:

- Space reinforcement in cap to miss anchor bolts.
- Bar terminators, paid for separately. See Total Bill of Material.
- Pour steps monolithically with cap.
- For details of piles, see sheet 42 of 46.
- Bars indicated thus 20x3-#5 etc. indicates 20 lines of bars with 3 lengths per line.

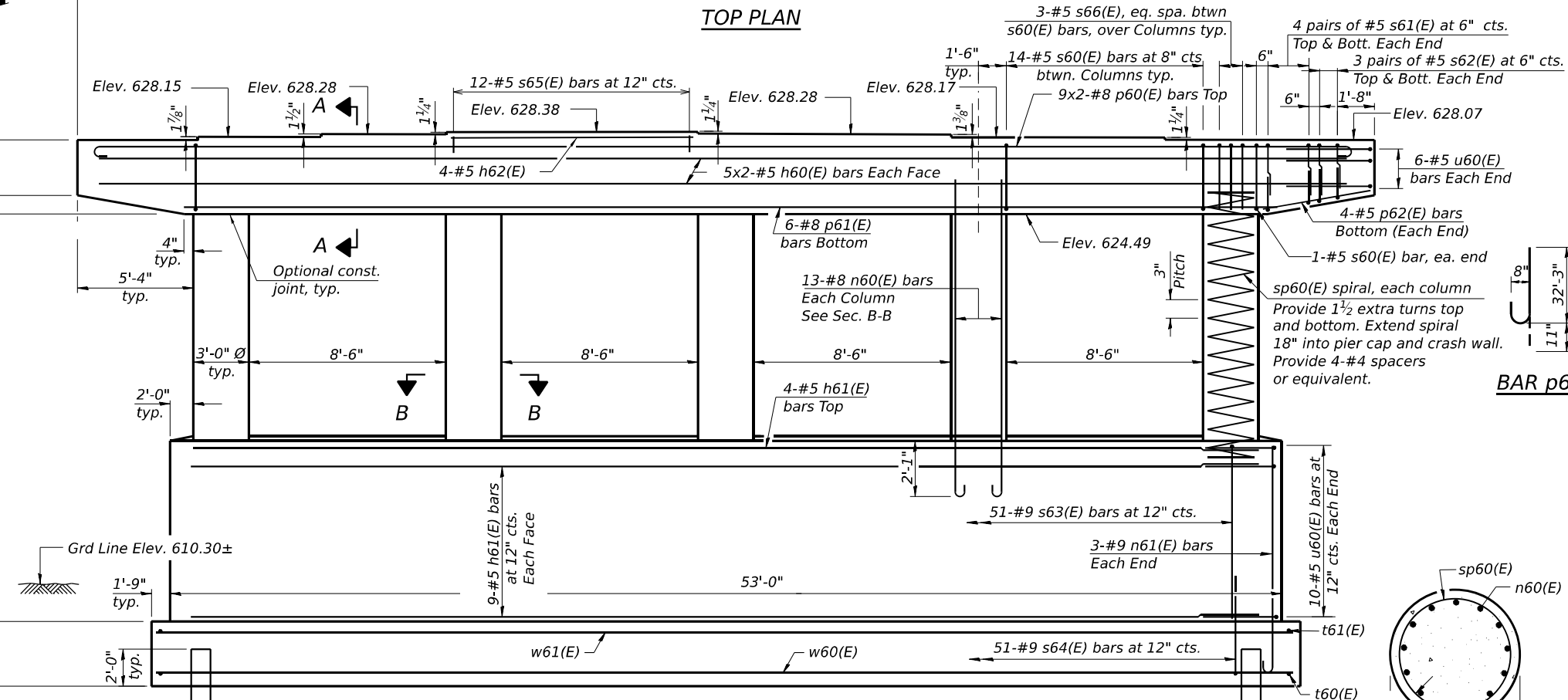
Type: Metal Shell 14" x 0.312" w/ Pile Shoes
Nominal Required Bearing: 510 kips
Factored Resistance Available: 281 kips
Est. Length: 13 ft
No. Production Piles: 19
No. Test Piles: 1



END VIEW



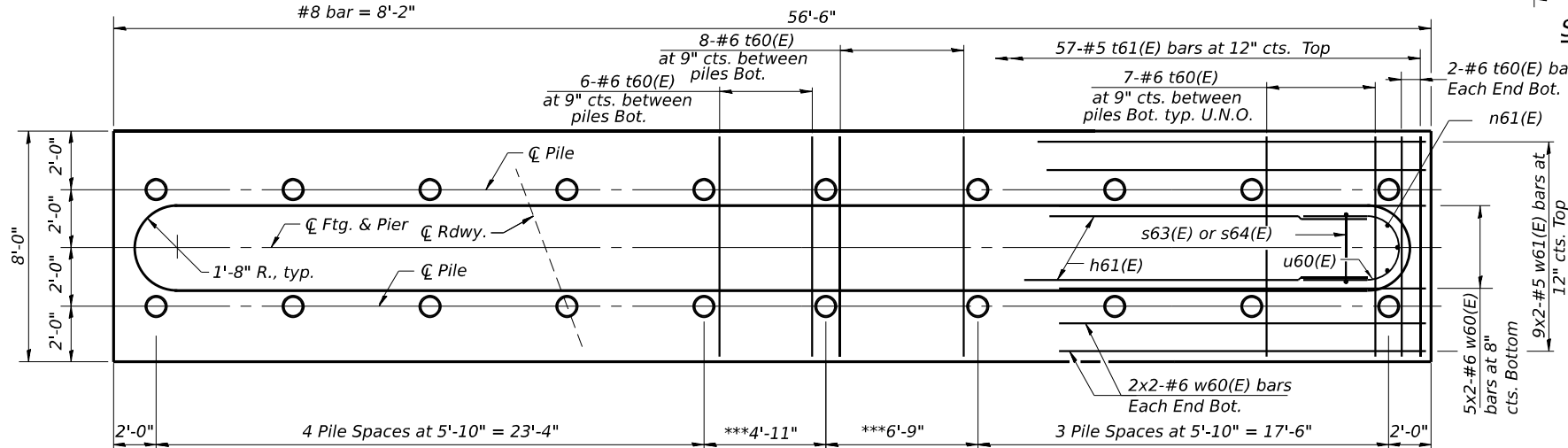
TOP PLAN



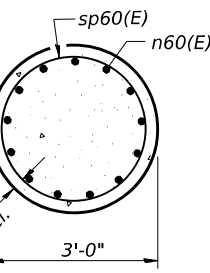
MINIMUM BAR LAP

#5 bar = 3'-7"
#6 bar = 3'-10"
#8 bar = 8'-2"

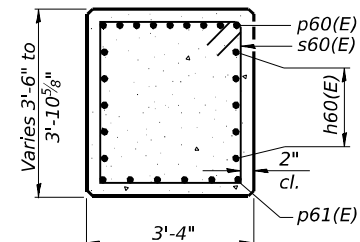
ELEVATION
(Looking East)



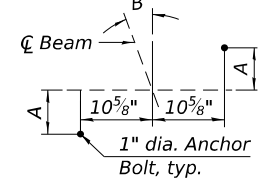
FOOTING PLAN



SEC. B-B



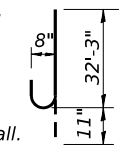
SEC. A-A



ANCHOR BOLT LAYOUT

	A	B
Beam 11-18	3 $\frac{7}{8}$ "	20°23'09"
Beam 19	3 $\frac{7}{8}$ "	19°48'13"
Beam 20	3 $\frac{3}{4}$ "	19°13'00"
Beam 21	3 $\frac{5}{8}$ "	18°37'34"

BAR p60(E)



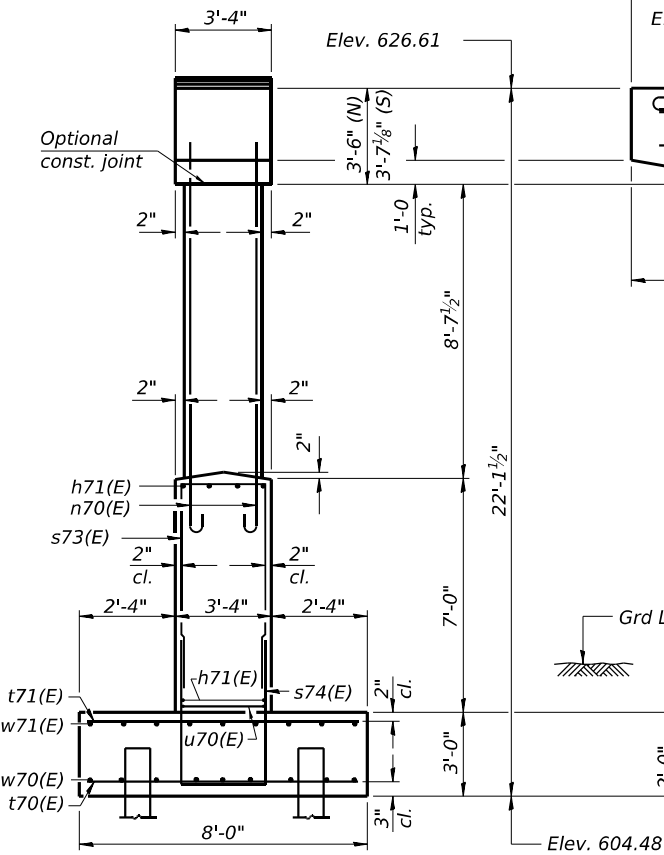
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h60(E)	20	#5	30'-0"	—
h61(E)	22	#5	49'-8"	—
h62(E)	4	#5	11'-2"	—
n60(E)	65	#8	15'-4"	⌋
n61(E)	6	#9	12'-7"	⌋
p60(E)	18	#8	33'-2"	⌋
p61(E)	6	#8	49'-8"	—
p62(E)	8	#5	5'-0"	—
s60(E)	58	#5	13'-3"	⌋
s61(E)	16	#5	8'-6"	⌋
s62(E)	12	#5	8'-0"	⌋
s63(E)	51	#9	19'-10"	⌋
s64(E)	51	#9	17'-8"	⌋
s65(E)	12	#5	5'-0"	⌋
s66(E)	15	#5	9'-4"	⌋
sp60(E)	5	#5	12'-1"	⌋
t60(E)	67	#6	7'-8"	—
t61(E)	57	#5	7'-8"	—
u60(E)	32	#5	11'-6"	⌋
w60(E)	18	#6	29'-11"	—
w61(E)	18	#5	29'-10"	—
Structure Excavation			Cu. Yd.	174
Concrete Structures			Cu. Yd.	145.5
Reinforcement Bars, Epoxy Coated			Pound	20,150
Furnishing Metal Shell Piles 14" x 0.312"			Foot	247
Driving Piles			Foot	247
Test Pile Metal Shells			Each	1
Pile Shoes			Each	20
Concrete Sealer			Sq. Ft.	1,203

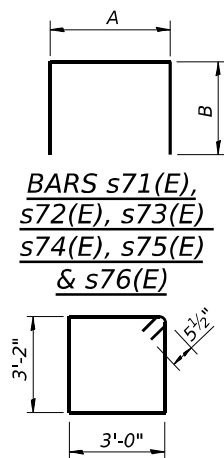
**

** Length is height of spiral.

Type: Metal Shell 14" x 0.312" w/ Pile Shoes
Nominal Required Bearing: 513 kips
Factored Resistance Available: 282 kips
Est. Length: 14 ft
No. Production Piles: 19
No. Test Piles: 1



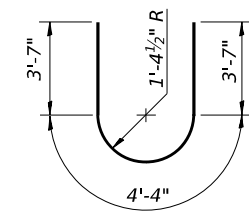
END VIEW



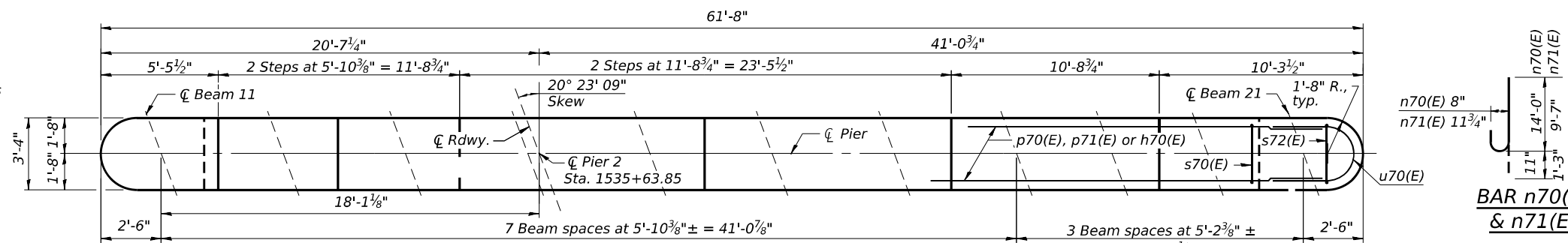
BAR s70(E)

A & B DIMENSIONS

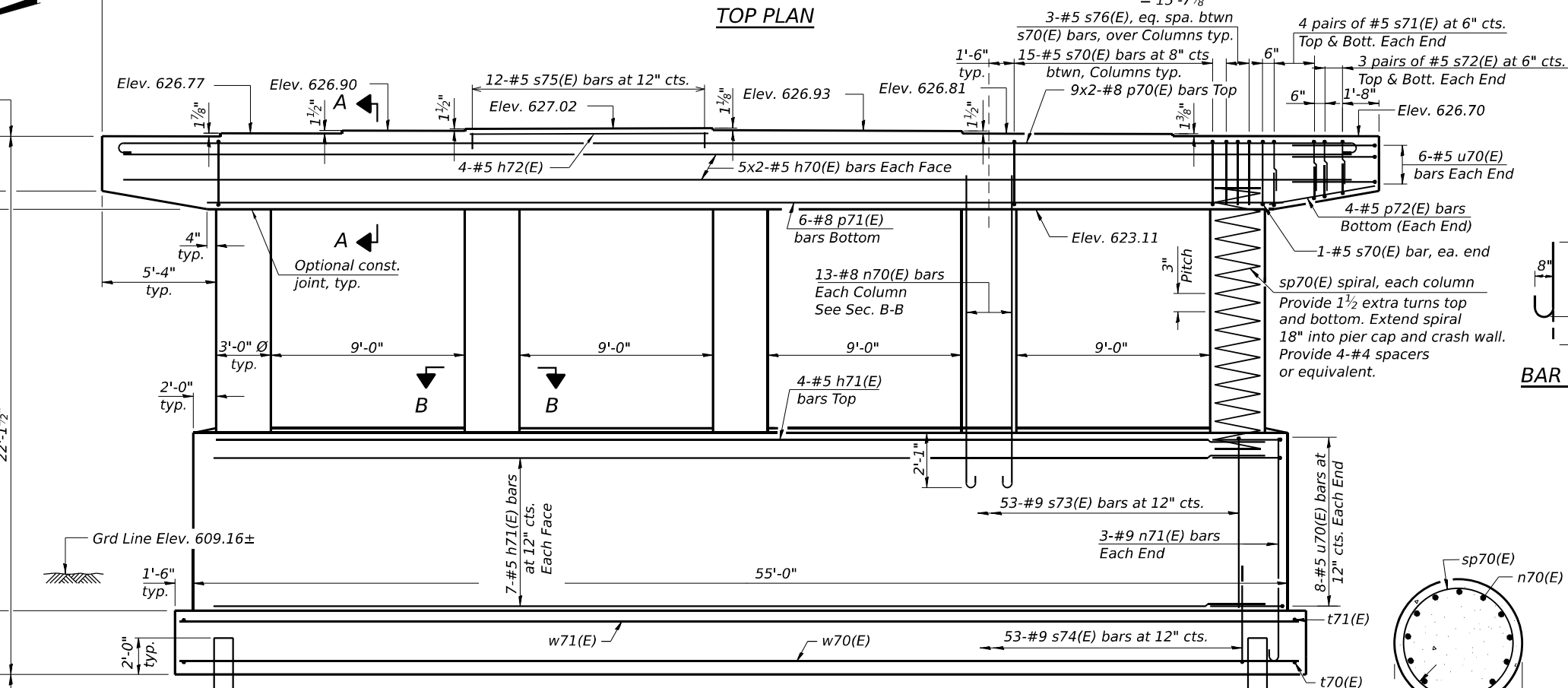
Bar	A	B
s71(E)	3'-0"	2'-9"
s72(E)	3'-0"	2'-6"
s73(E)	3'-0"	6'-8"
s74(E)	3'-0"	7'-4"
s75(E)	3'-0"	1'-0"
s76(E)	3'-0"	3'-2"



BAR u70(E)



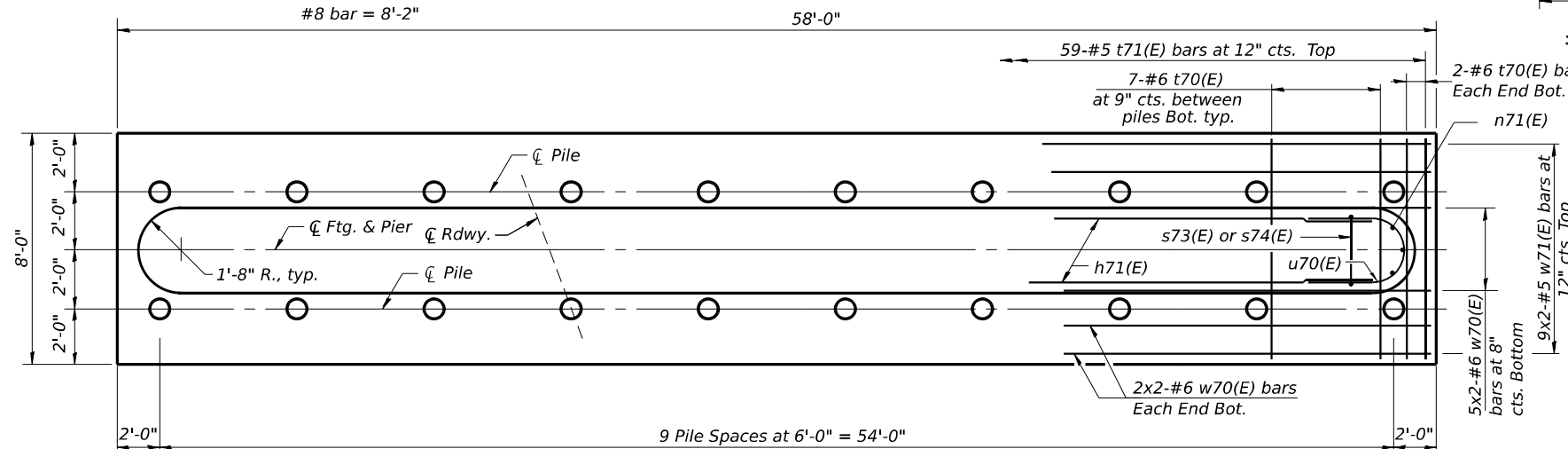
TOP PLAN



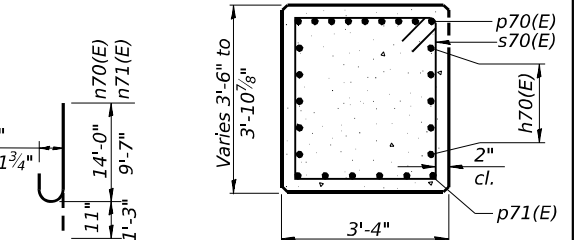
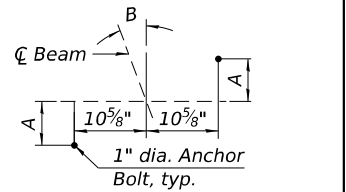
MINIMUM BAR LAP

#5 bar = 3'-7"
#6 bar = 3'-10"
#8 bar = 8'-2"

ELEVATION
(Looking East)

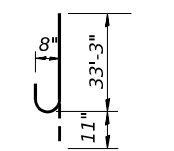


FOOTING PLAN


$$\frac{BAR\ n70(E)}{\&\ n71(E)}$$


ANCHOR BOLT LAYOUT

	A	B
Beam 11-18	3 ⁷ / ₈ "	20°23'09"
Beam 19	3 ⁷ / ₈ "	19°48'13"
Beam 20	3 ³ / ₄ "	19°13'00"
Beam 21	3 ⁵ / ₈ "	18°37'34"

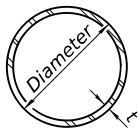


BAR p70(E)

BILL OF MATERIAL

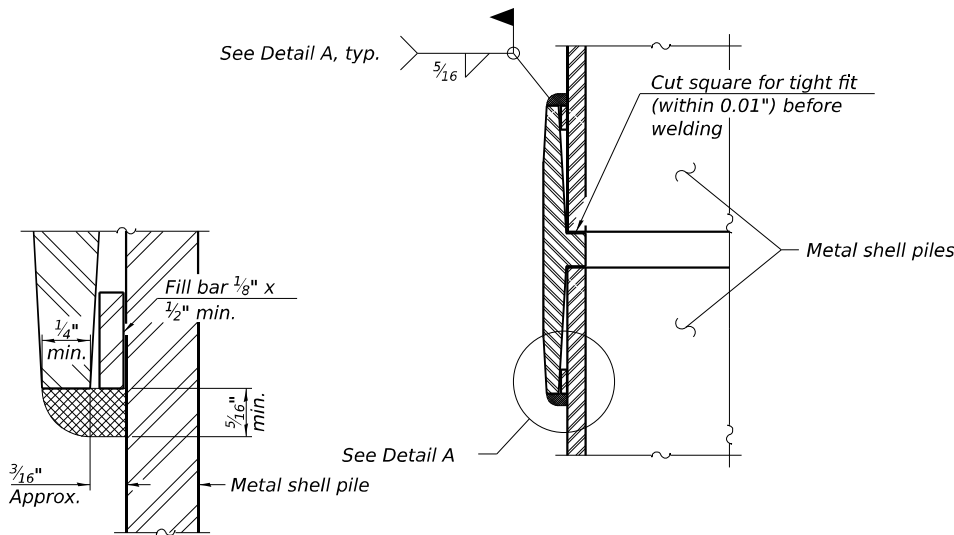
Bar	No.	Size	Length	Shape
h70(E)	20	#5	31'-0"	—
h71(E)	18	#5	51'-8"	—
h72(E)	4	#5	11'-2"	—
n70(E)	65	#8	14'-11"	⌋
n71(E)	6	#9	10'-10"	⌋
p70(E)	18	#8	34'-2"	⌋
p71(E)	6	#8	51'-8"	—
p72(E)	8	#5	5'-0"	—
s70(E)	62	#5	13'-3"	⌋
s71(E)	16	#5	8'-6"	⌋
s72(E)	12	#5	8'-0"	⌋
s73(E)	53	#9	16'-4"	⌋
s74(E)	53	#9	17'-8"	⌋
s75(E)	12	#5	5'-0"	⌋
s76(E)	15	#5	9'-4"	⌋
* sp70(E)	5	#5	11'-8"	⌋
t70(E)	67	#6	7'-8"	—
t71(E)	59	#5	7'-8"	—
u70(E)	28	#5	11'-6"	⌋
w70(E)	18	#6	30'-8"	—
w71(E)	18	#5	30'-7"	—
Structure Excavation			Cu. Yd.	114
Concrete Structures			Cu. Yd.	137.8
Reinforcement Bars, Epoxy Coated			Pound	19,580
Furnishing Metal Shell Piles 14" x 0.312"			Foot	266
Driving Piles			Foot	266
Test Pile Metal Shells			Each	1
Pile Shoes			Each	20
Concrete Sealer			Sq. Ft.	1,241

** Length is height of spiral.

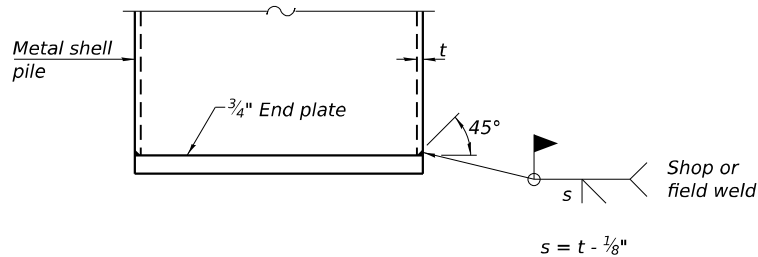


METAL SHELL PILE TABLE

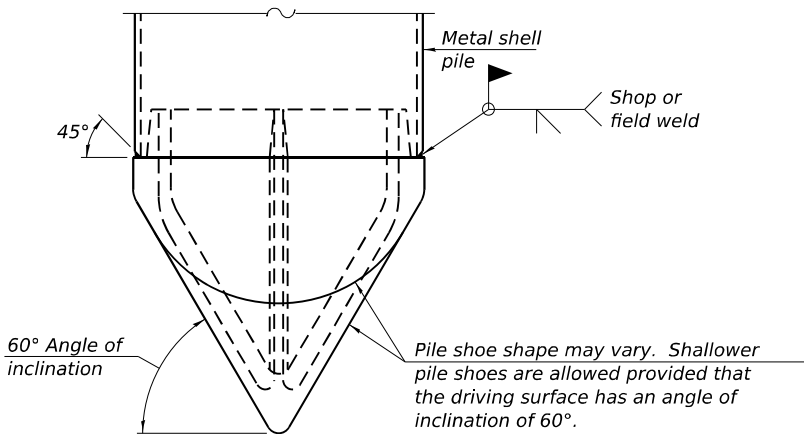
Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. ³ /ft.)
PP12	0.250"	31.40	0.0267
PP14	0.250"	36.75	0.0368
PP14	0.312"	45.65	0.0361
PP16	0.312"	52.32	0.0478
PP16	0.375"	62.64	0.0470



DETAIL A



END PLATE ATTACHMENT

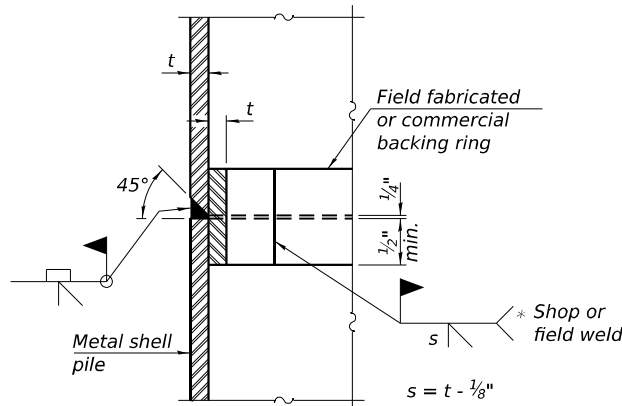


PILE SHOE ATTACHMENT

(When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A 148 Grade 80-50 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld).

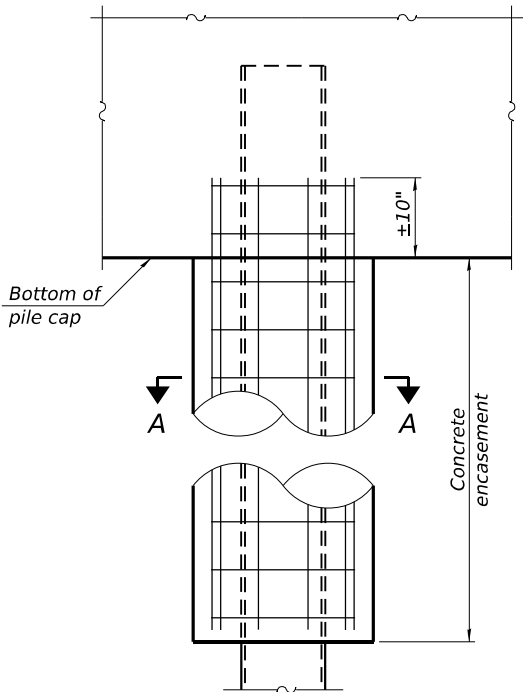
WELDED COMMERCIAL SPLICE

Notes:
The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them.
Pile segments shall be driven to solid contact with splicer before welding.

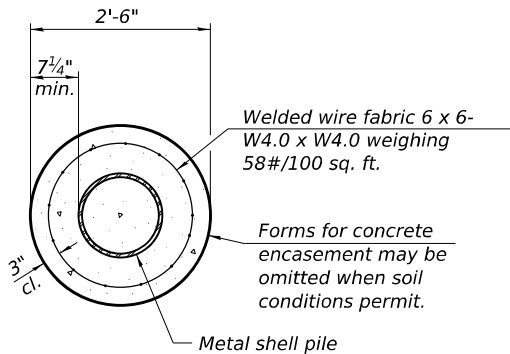


COMPLETE PENETRATION WELD SPLICE

* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.



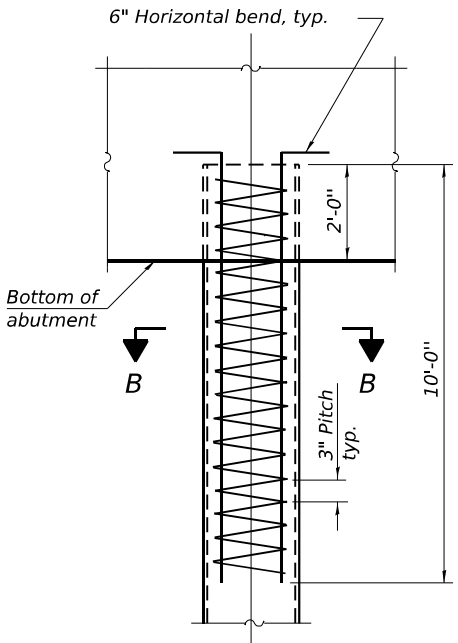
ELEVATION



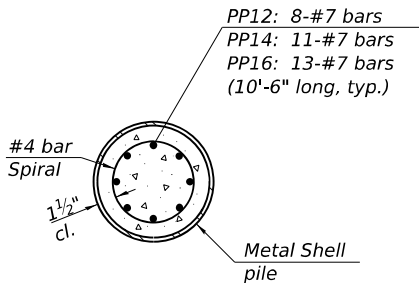
SECTION A-A

INDIVIDUAL PILE
CONCRETE ENCASEMENT

(When specified)



ELEVATION



SECTION B-B

REINFORCEMENT AT ABUTMENTS

(Omit when concrete encasement is specified)

Note:
The metal shell piles shall be according to Article 1006.05 of the Standard Specifications.

F-MS

5-15-2023



USER NAME	= twendt
PLOT SCALE	= 0.16666633 1/ in.
PLOT DATE	= 8/5/2024

DESIGNED	- TAW
DRAWN	- TAW
CHECKED	- TJZ
DATE	- 08/06/2024

REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

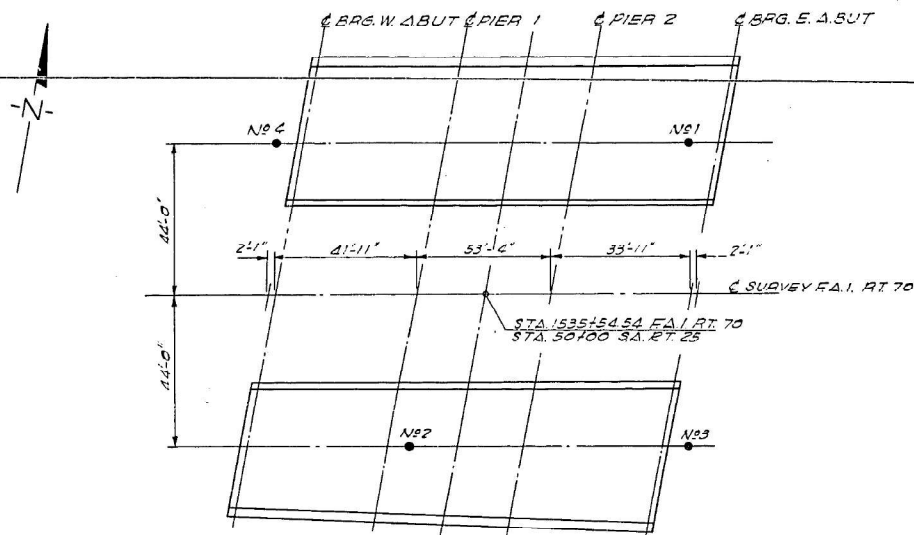
METAL SHELL PILE DETAILS
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

SCALE: SHEET 42 OF 46 SHEETS STA. TO STA.

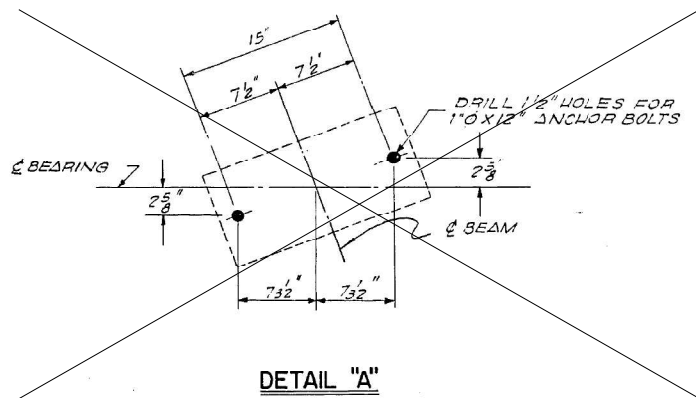
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	427
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

FAI ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-IHB-3	EFFINGHAM	30	9
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJ. NO.	

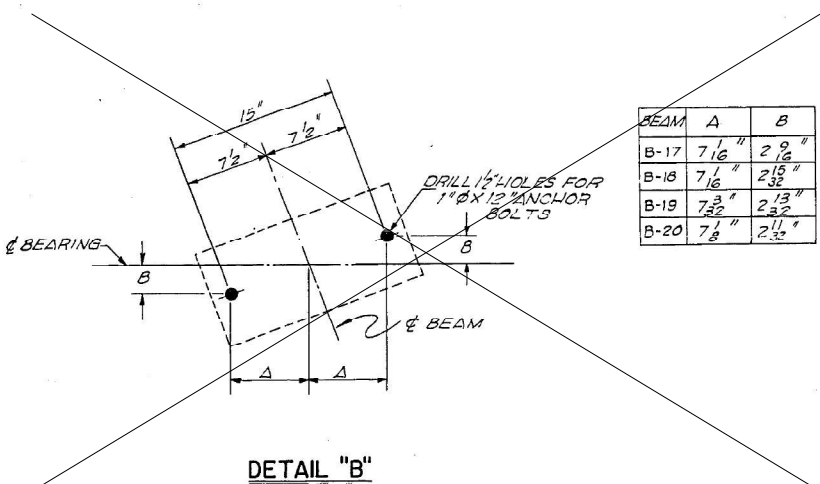
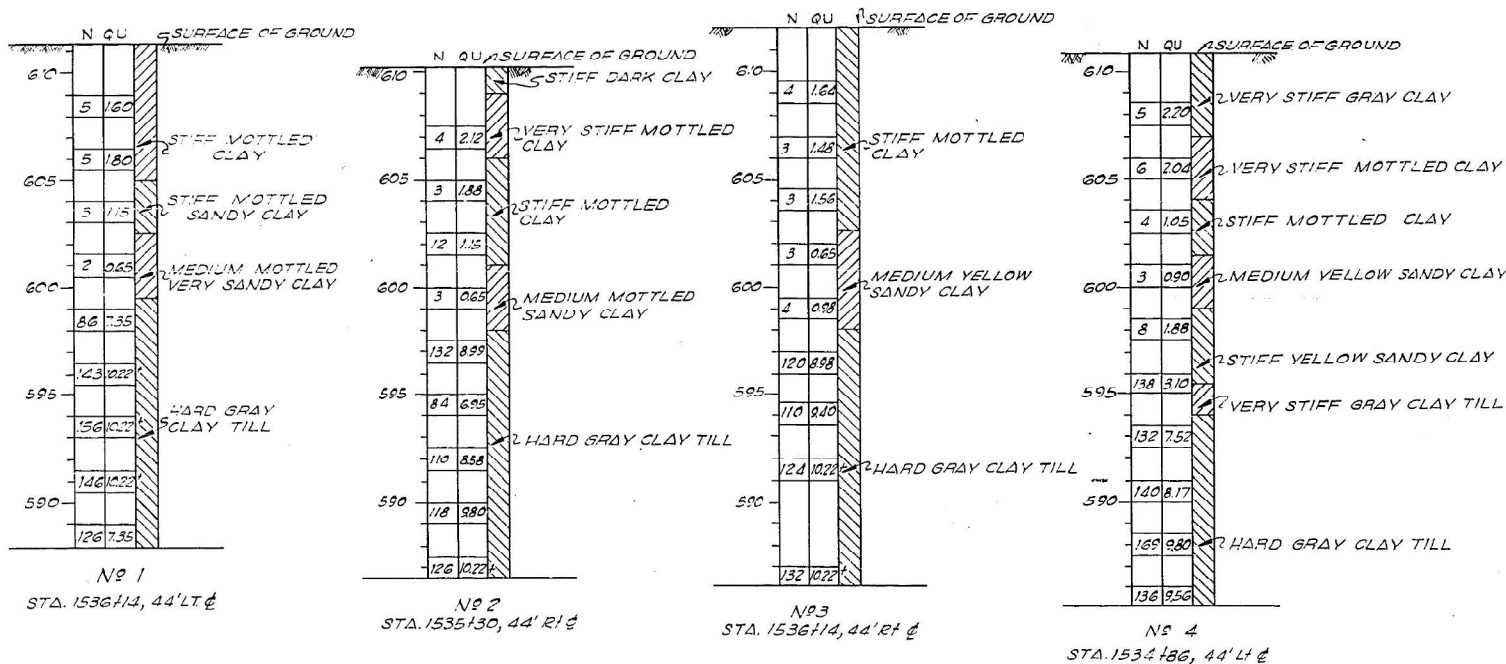
SHEET NO. 2
16 SHEETS



BORINGS LOCATION SKETCH



DETAIL "A"



DETAIL "B"

BEAM	A	B
B-17	7 1/8"	2 9/16"
B-18	7 1/8"	2 15/16"
B-19	7 3/8"	2 3/4"
B-20	7 3/8"	2 3/4"

BORINGS AND ANCHOR BOLTS DETAILS	
ROCHESTER AND GOODELL ENGINEERS INCORPORATED	
BOX 371	SALEM, ILLINOIS PH 350
BRIDGE OVER - ALTAMONT ROAD (S.A. RT. 25)	
STATION - 1535+54.54	
FAI. ROUTE - 70	
SECTION - 25-IHB-3	
EFFINGHAM COUNTY, ILLINOIS	
DESIGNED	E. G. PYLE
DRAWN	R. HEAVENER
CHECKED	D. WILLIAMS
APPROVED	E. MOLDOVAN
SCALE: NONE	
DATE: MAY 25, 1959	

FOR INFORMATION ONLY - FROM EXISTING PLANS



USER NAME	= mcdordova
PLOT SCALE	= N/A
PLOT DATE	= 8/5/2024

DESIGNED	- MAC
DRAWN	- IF
CHECKED	- JTH
DATE	- 08/06/2024

REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING - I
STRUCTURE NO. 025-0115 (EB) & 025-0116 (WB)

SCALE: SHEET 43 OF 46 SHEETS STA. TO STA.

FAI RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-IHB-3	EFFINGHAM	747	428
		CONTRACT NO. 74893		
		ILLINOIS FED. AID PROJECT		

Page 1 of 2

Date 9/9/19

SECTION (25-1)R-BR LOCATION NE 1/4, SEC. 21, TWP. 7N, RNG. 4E, 3rd PM,
Latitude N 39.044006, Longitude W 88.748377

STRUCT. NO. 025-0007 &
025-0008
Station 1535+54.54

D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)

D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)

SOIL BORING 025-0007, 0008 SOIL 2019.GPJ IL_DOT.GDT 10/3/19

BBS, form 137 (Rev. 8-99)



Page 2 of 2

Date 9/9/19

SECTION (25-1)R-BR LOCATION NE 1/4, SEC. 21, TWP. 7N, RNG. 4E, 3rd PM,
Latitude N 39.044006, Longitude W 88.748377

STRUCT. NO. 025-0007 &
025-0008
Station 1535+54.54

D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)

Surface Water Elev.	N/A	ft
Stream Bed Elev.	N/A	ft
Groundwater Elev.:		
First Encounter	Dry	ft
Upon Completion	Dry	ft
After 24 Hrs.	605.7	ft ▽

SOIL BORING 025-0007, 0008 SOIL 2019.GPJ IL_DOT.GDT 10/3/19

BBS, form 137 (Rev. 8-99)



SCALE:	SHEET 44 OF 46 SHEETS	STA.	TO STA.
--------	-----------------------	------	---------

ILLINOIS	FED. AID PROJECT
----------	------------------

S.M. Top R/W 1/2 mile in V.E. Quad of J. L. Mont. Road
& E. IV. Road N. E. Sta. 1535+54 (Elev. 613.09)

RT. Sta 1535+17.93 Westbound Rdwy.
Sta 1534+186.85 Eastbound Rdwy.

Note: Piles shall be prepared for concrete piles at abutments
in accordance with Article 60.9(c) of the Standard Specifications.

F.A.I. ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	30	8
FED. ROAD DIST. NO. 7				ILLINOIS
FED. AID PROJ. NO. I-70-3(2)				SHEET NO. 1 16 SHEETS

GENERAL NOTES

Class X Concrete shall be used throughout except in Handrail End Posts.
Handrail Concrete shall be used in the Handrail End Posts.
The Concrete Floor Slab shall be finished in accordance with Article 51.19 of the Standard Specifications, and shall be poured in one continuous operation between construction joints.
Piles 34" ϕ , Crown Holes 18" ϕ , unless noted.
Field Connections riveted, unless noted.
All Holes for Slices shall be punched 1/4" ϕ and reamed to proper size (18" ϕ in web and 18" ϕ in flange) with all stringers assembled in the shop in proper position. Leave assembled in shop for inspection.
All Rakers, Bolsters, Bearing plates, Fill plates, Lead plates, Pintles, and Anchor Bolts shall be fabricated and set in accordance with Article 51.15 of the Standard Specifications, and are included in the quantity of Structural Steel.
Anchor Bolts shall be set before riveting Diaphragms over Supports.
All surfaces of the expansion guards inaccessible after erection shall receive two (2) shop coats of Red Lead Paint except for the Anchor Studs. The Anchor Studs shall not be painted.
Except as otherwise provided, all Structural Steel shall receive one (1) shop coat of Red Lead Paint and two (2) field coats of Aluminum Paint. See Article 56.1 through 56.5 inclusive, of the Standard Specifications.
The Contractor shall drive one Concrete Test Pile at West Abutment of South Bridge and one Steel Test Pile at Pier 2 of North Bridge in permanent locations as directed by the Engineer before ordering the remainder of the piles.
Concrete Piles designed for maximum load of 30 ton per pile.
Steel Piles designed for a maximum load of 40 ton per pile.
All Piles shall be driven into the Hard Till Strata indicated on the Log of Borings and to a bearing value of not less than the above maximum design loads.
Holes for Concrete Piles shall be prepared at each Abutment in accordance with Article 60.9(c) of the Standard Specifications.
All paint shall be furnished and applied by the Contractor.



ITEM	SUPER	SUB	TOTAL
Class X Concrete	Cu Yds	338.7	366.3
Reinforcement Bars	Lbs	63,101	65,523
Hand-rail Concrete	Cu Yds		4.6
Furnishing and Erection Structural Steel	Lbs	310,113	340,113
Test Pile (Steel)	Each	1	1
Furnishing Concrete Piles	Lin Ft	1631	1631
Driving Concrete Piles	Lin Ft	1631	1631
Furnishing Steel Piles (105742)	Lin Ft	1049	1049
Driving Steel Piles	Lin Ft	1049	1049
Name Plate	Each	2	2
Class X Excavation for Structures	Cu Yds		320
Test Pile (Concrete)	Each	1	1
Slope Wall	Sq Yds		936
Furnishing and Erection Metal Handrail	Lin Ft		522
Sodding	Sq Yds	327	327
Supplemental Watering	Units	1	1

GENERAL PLAN AND ELEVATION

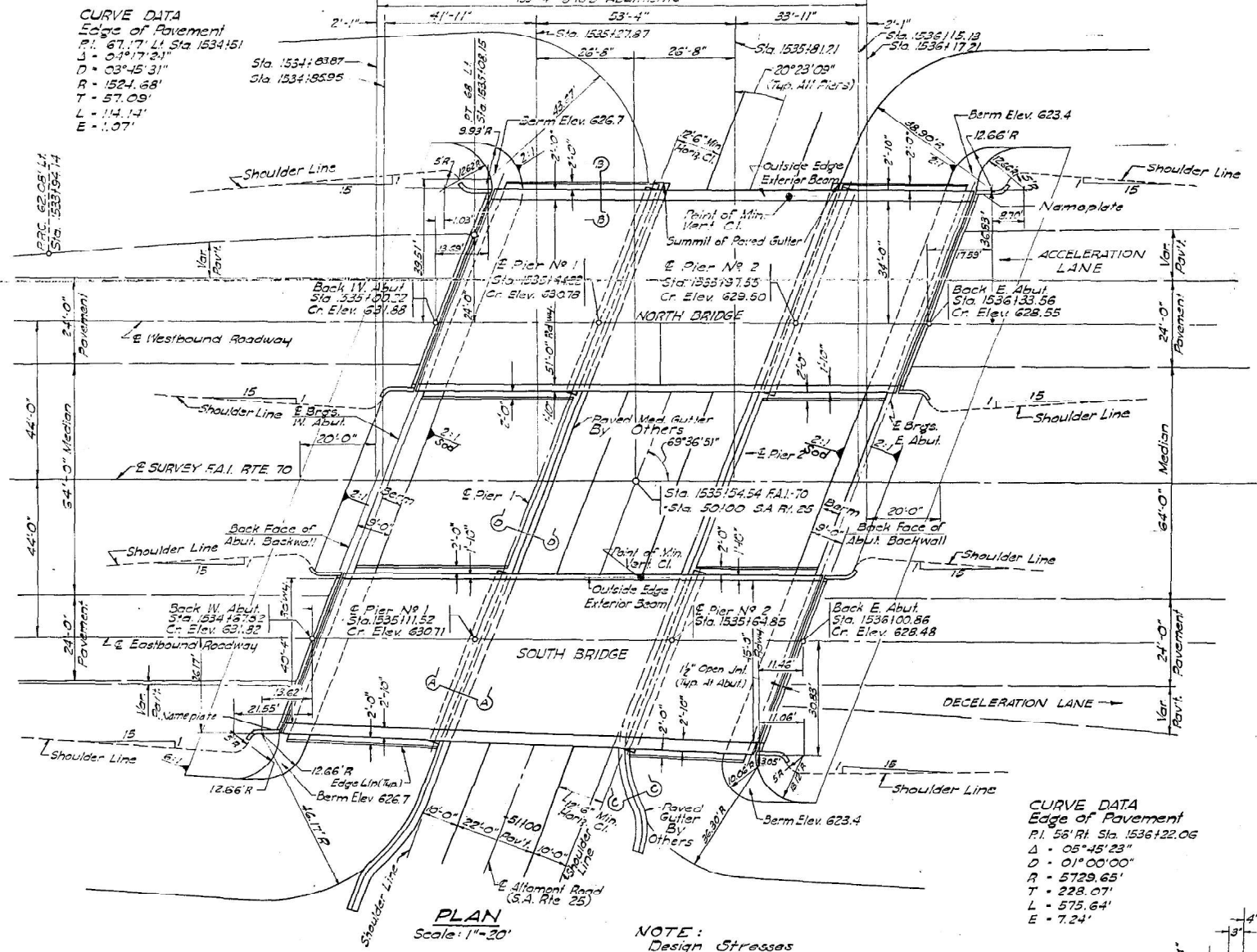
NAME	ROCHESTER AND GOODSELL ENGINEERS INCORPORATED		
BOX	371	SALEM, ILLINOIS	PH 350
BRIDGE OVER - ALTAMONT ROAD (S.A. RT 25)			
STATION	1535+54.54		
F.A.I. ROUTE	70		
SECTION	25-1HB-3		
EFFINGHAM COUNTY, ILLINOIS			
DESIGNED	E. G. PYLE		
DRAWN	E. HOLT		
CHECKED			
APPROVED	E. MOLDOVAN		
SCALE:	NONE		
DATE	MAY 25, 1959		

FOR INFORMATION ONLY

CURVE DATA
Edge of Pavement
P.I. 67.7' L Sta 1534+151
S = 0.17' 3.1"
D = 0.34' 5.31"
R = 1524.68'
T = 57.09'
L = 14.14'
E = 1.07'

ELEVATION

Normal to S. Altamont Rd. S.A. Route 25

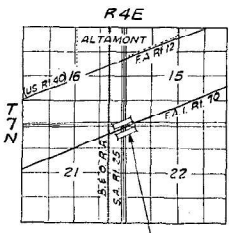


CURVE DATA
Edge of Pavement
P.I. 56.8' Rt. Sta. 1536+22.06
S = 0.5' 45.23"
D = 0.1' 00.00"
R = 5729.65'
T = 228.07'
L = 575.64'
E = 7.24'

NOTE:
Design Stresses
f_c = 1400 psi (Max.) Superstructure
f_c = 1000 psi Substructure in contact with Earth
f_c = 1400 psi (Max.) Substructure
f_s = 20,000 psi (Max.) Reinforcing
f_s = 18,000 psi (Normal) Structural Steel
n = 10
V₆ = 75 psi Footing
Loading
H20-S16-44 (Military and 18,000 LBS
(Future Wearing Surface))

SECTION B-B

SECTION C-C



EXAMINED	19
PASSED	
APPROVED	



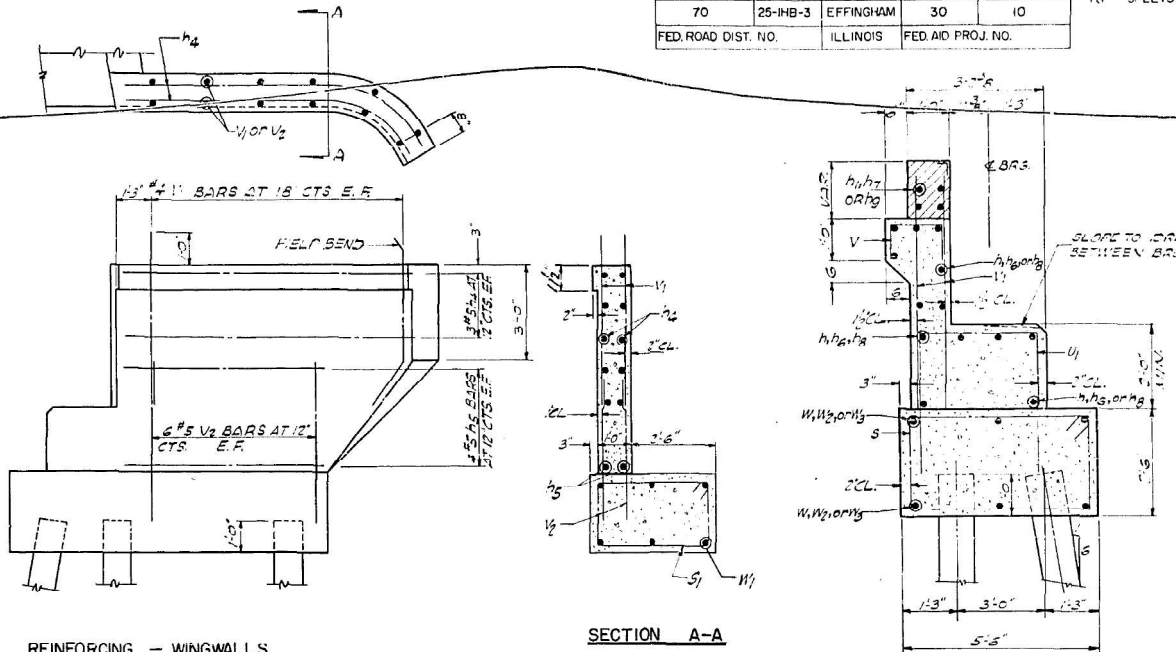
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DESIGNED	- MAC
DRAWN	- IF
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CHECKED	- JTH
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DATE	= 08/06/2024
REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING STRUCTURE PLANS - I
STRUCTURE NO. 025-0007 (EB) & 025-0008 (WB)

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	432
SCALE:				ILLINOIS
SHEET 1 OF 5 SHEETS				FED. AID PROJECT

F.A.I. ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 3 OF 3 SHEETS
70	25-1HB-3	EFFINGHAM	30	10	
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJ. NO.			



FOR INFORMATION ONLY

SECTION 1-1

BILL OF MATERIAL (4 ABUTMENTS)

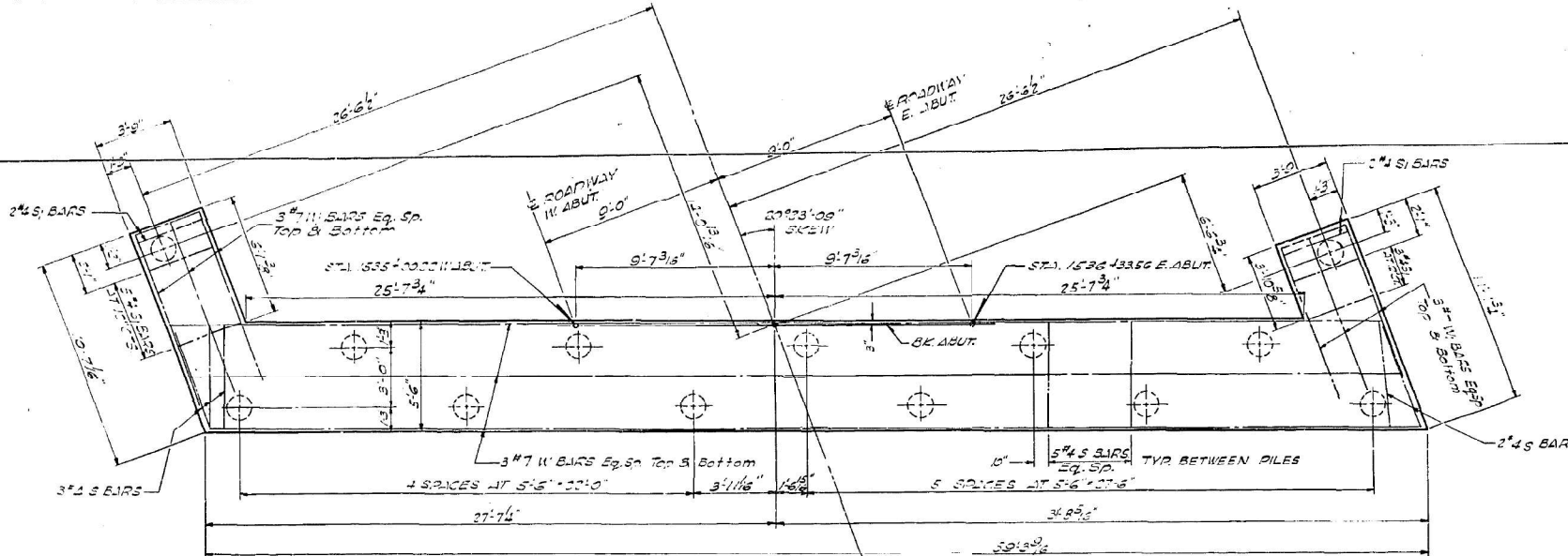
BAR	NO	SIZE	LENGTH	SLOPE
V	211	#4	2'-9"	
V	339	#3	5'-9"	
h	96	#5	3'-2"	
h	57	#4	28'-9"	
h	16	#4	28'-9"	
h2	32	#5	3'-10"	
h3	32	#5	3'-10"	
h4	48	#5	9'-6"	
h5	64	#5	8'-0"	
h6	26	#4	24'-3"	
h7	8	#4	21'-10"	
h8	26	#4	28'-9"	
h9	8	#4	23'-10"	
W	24	#7	30'-1"	
W1	48	#7	8'-3"	
W2	12	#7	24'-4"	
W3	12	#7	26'-10"	
U	24	#6	6'-8"	
U1	217	#4	15'-4"	
S	210	#4	15'-4"	
S1	48	#4	11'-10"	

Class "X" Concrete	Cu. Yds.	236.1
Reinforcing Bars	Lbs.	13,342
Concrete Piles	Lin. Ft.	163
Concrete Test Pile	Each	1

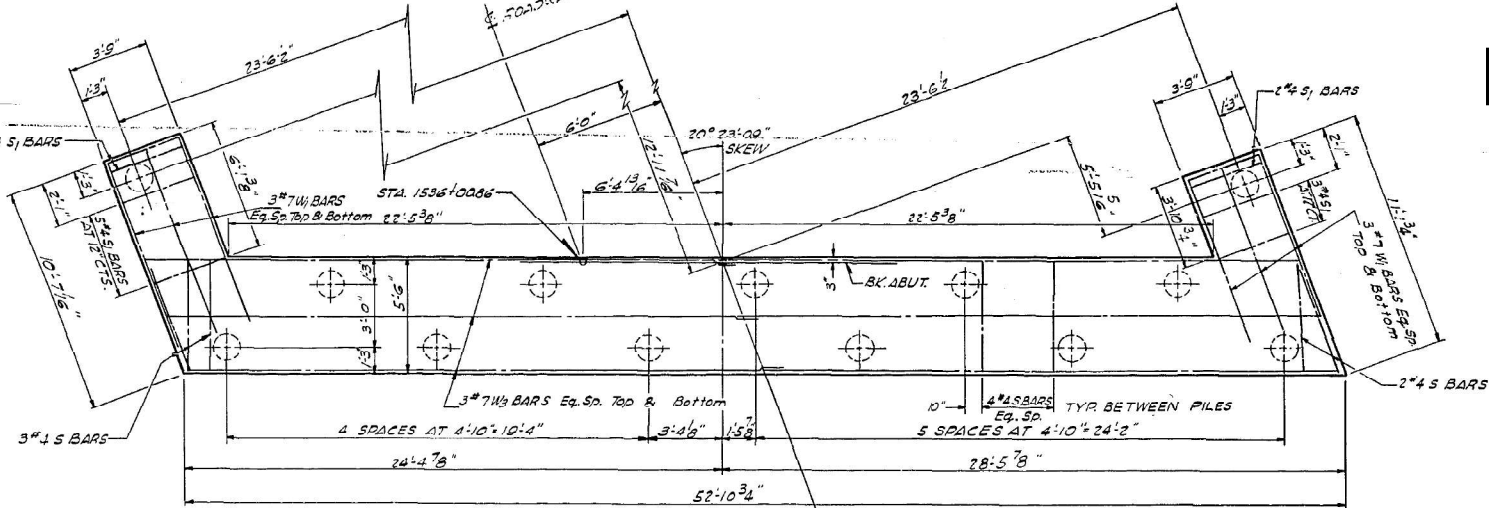
BAR	a	b	c
S	2'-1"	5'-2"	5'
S1	2'-1"	3'-5"	5'

PILE DATA

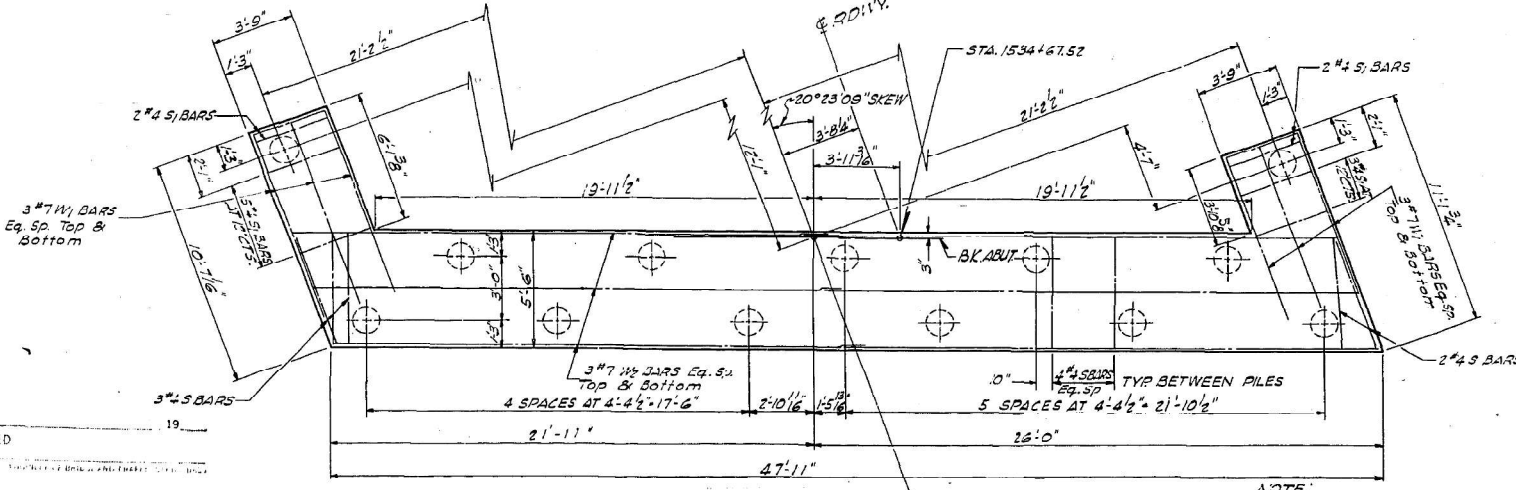
Capacity.....30 Ton
Est. Length:
E. Abuts. North & South Bridge...31 ft.
W. Abuts. North & South Bridge...33 ft.
No. Req'd. ... 52 Piles (4 Abuts.)
Batter All Piles Along Front Face of Abuts.



ABUTMENT FOOTING - NORTH BRIDGE



EAST ABUTMENT FOOTING - SOUTH BRIDGE



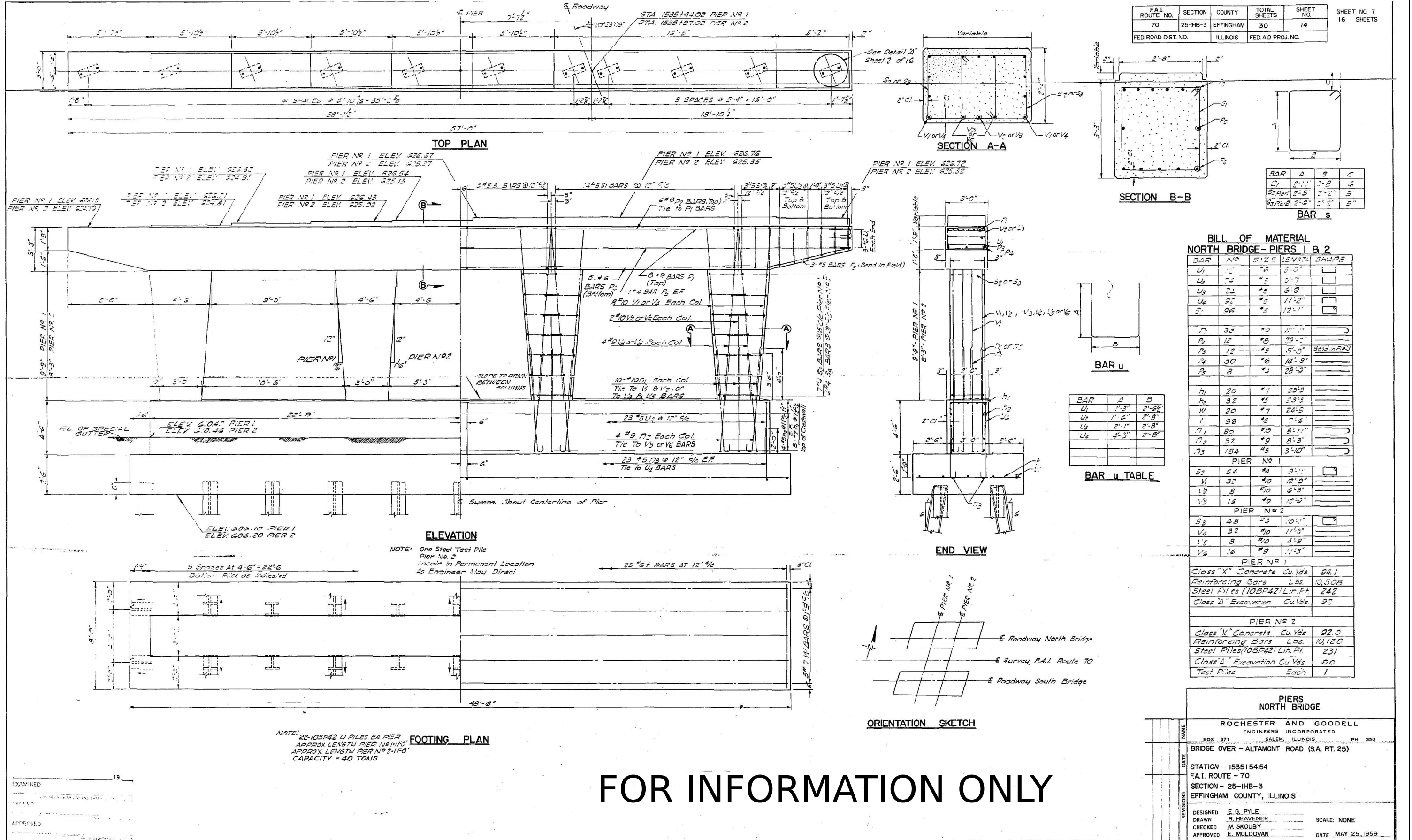
WEST ABUTMENT FOOTING - SOUTH BRIDGE

NOTE: LOCATE TEST PILE WEST ABUT SOUTH BRIDGE IN PERMANENT LOCATION AS DIRECTED BY THE ENGINEER

EXAMINED
PREPARED
APPROVED

REVISIONS	DATE	NAME	DESCRIPTION

FILE NAME: dw:\cmt\engr\proj\benl\cmt\connect\projects\Documents\DOT2200419740074893\CADD Data\Bridge\CADD Sheets\025-011560116-74893-3-Existing Structure Plans - WB Piers



FOR INFORMATION ONLY



USER NAME	= mcdordova	DESIGNED	- MAC	REVISED	-
DRAWN	- IF	REVISED	-		
PLOT SCALE	= N/A	CHECKED	- JTH	REVISED	-
PLOT DATE	= 8/5/2024	DATE	- 08/06/2024	REVISED	-

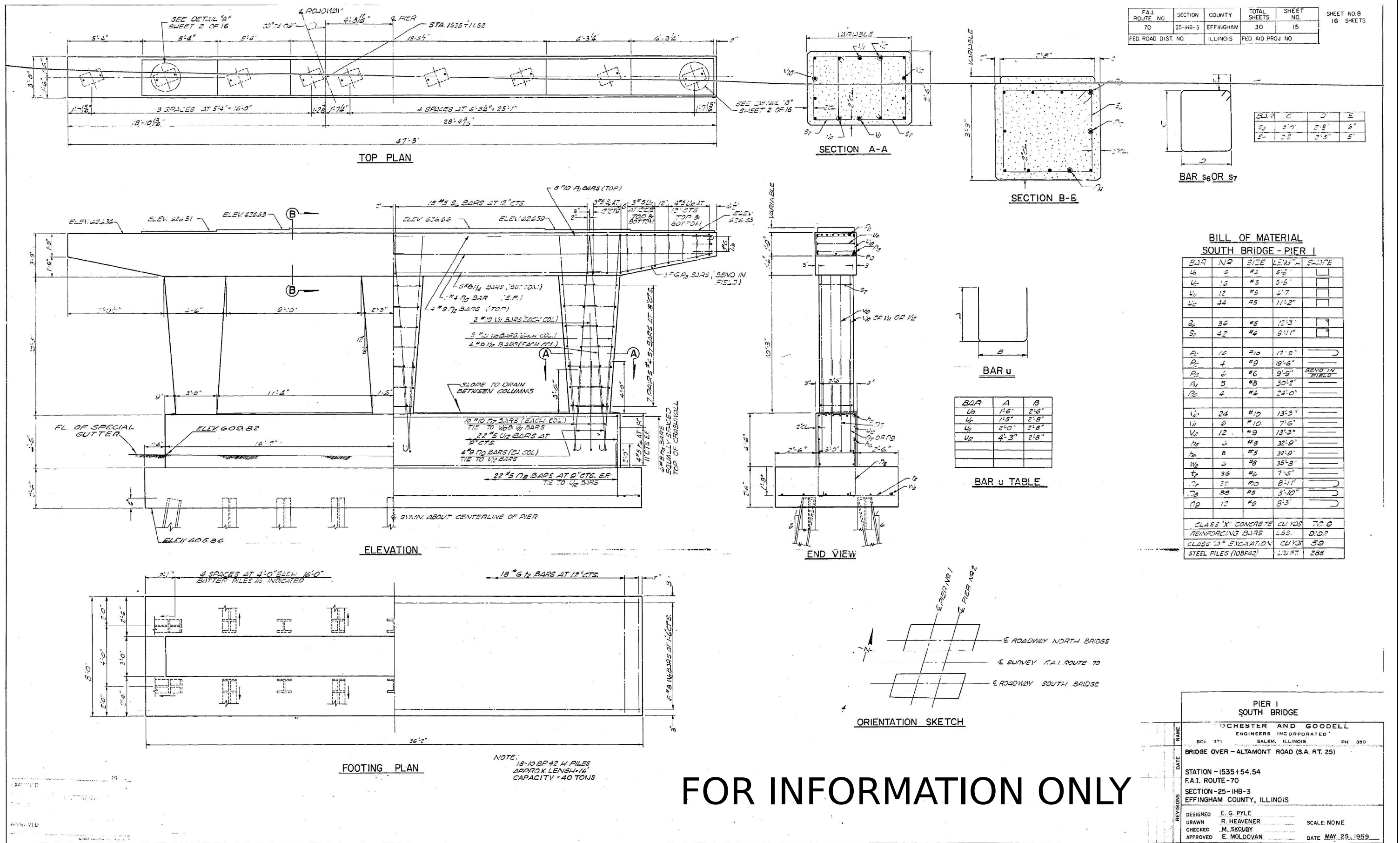
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING STRUCTURE PLANS - III
STRUCTURE NO. 025-0007 (EB) & 025-0008 (WB)

SCALE: SHEET 3 OF 5 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-IHB-3	EFFINGHAM	747	434
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

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FOR INFORMATION ONLY



License No. 184-000613 Copyright CMT, Inc.

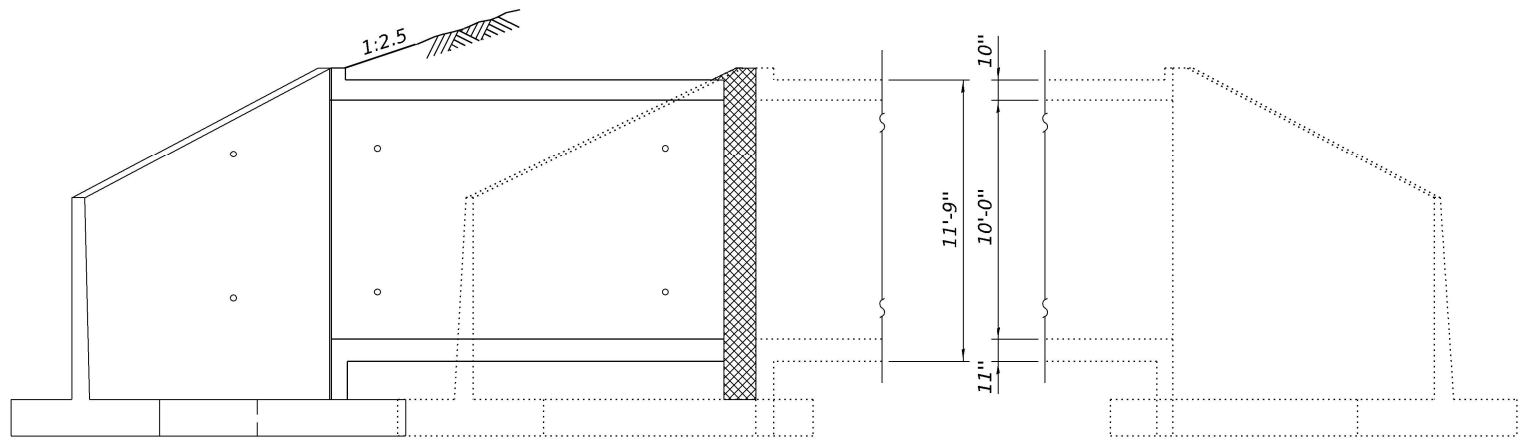
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PLOT SCALE = N/A	DRAWN - IF	REVISED -
PLOT DATE = 8/5/2024	CHECKED - JTH	REVISED -
	DATE - 08/06/2024	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING STRUCTURE PLANS - IV
STRUCTURE NO. 025-0007 (EB) & 025-0008 (WB)

SCALE: SHEET 4 OF 5 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	435
ILLINOIS				FED. AID PROJECT



LONGITUDINAL SECTION

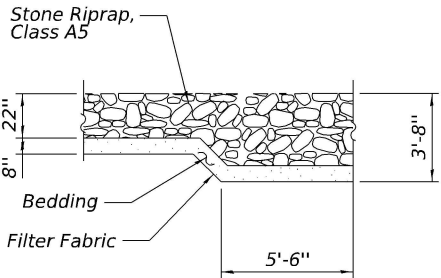
GENERAL NOTES

Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Expansion bolts shall consist of approved expansion anchors, providing minimum certified proof load = 4,080 lbs., and 3/4"Ø x 12" hooked bolts.

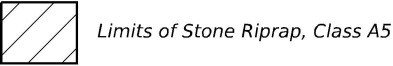
All exposed edges shall have standard 3/4" chamfers.

Reinforcement bars designated (E) shall be epoxy coated.



SECTION A-A

Cost of any excavation necessary for placement of riprap or bedding material shall be included with Stone Riprap, Class A5



12-11-25
Exp. Date 11-30-2026

DESIGN STRESSES

FIELD UNITS

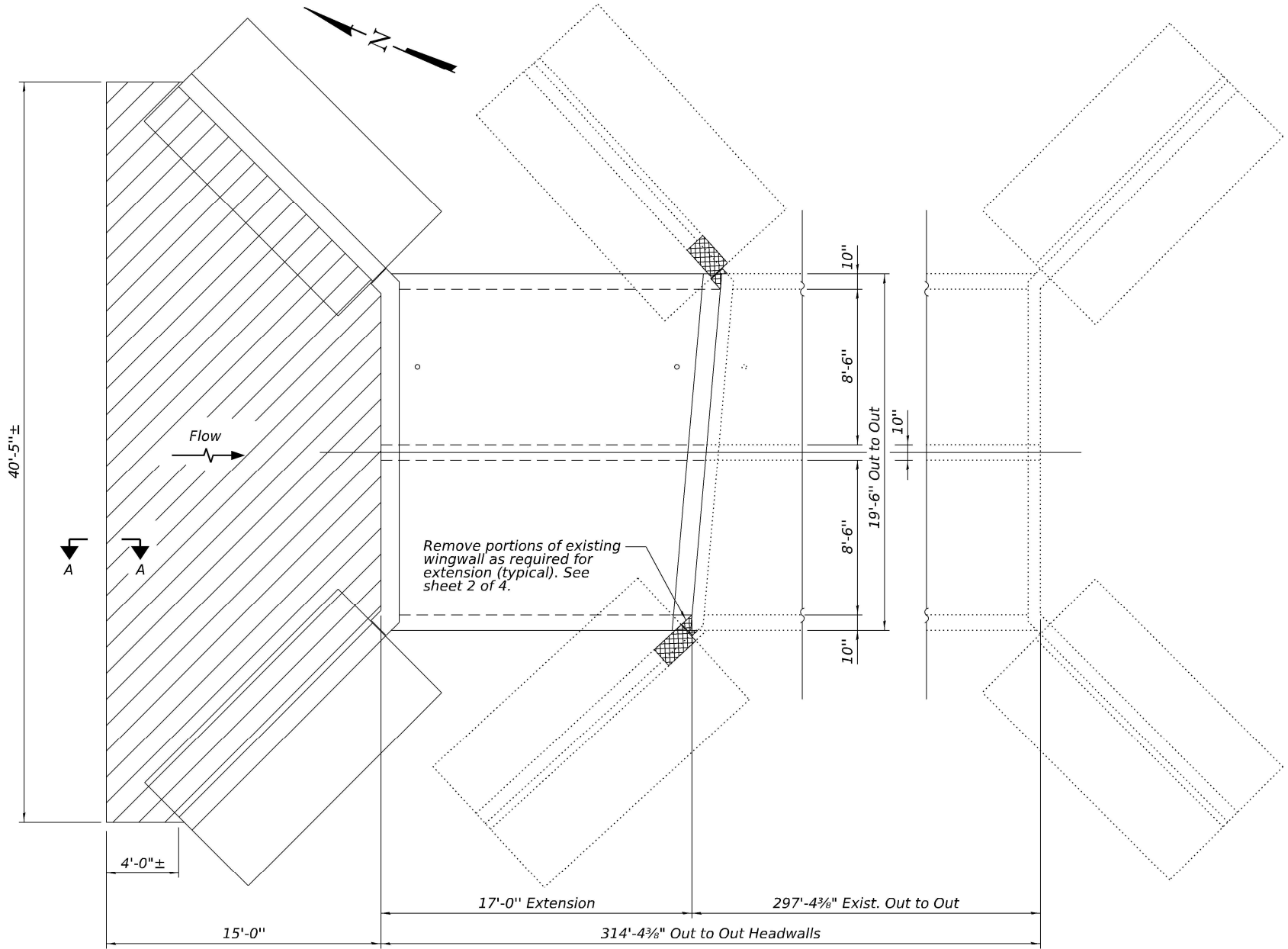
f_c = 3,500 psi
f_y = 60,000 psi (Reinforcement)

APPROVED
For Structural Adequacy Only
[Signature]
Engineer of Bridges & Structures

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Concrete Removal	Cu. Yd.	2.6
Concrete Box Culverts	Cu. Yd.	71.3
Reinforcement Bars	Pound	11,470
Reinforcement Bars, Epoxy Coated	Pound	830
Expansion Bolts 3/4"	Each	38
Structure Excavation	Cu. Yd.	130
Stone Riprap, Class A5	Sq. Yd.	55
Filter Fabric	Sq. Yd.	55
Rock Fill - Foundation	Ton	34
Removal and Disposal of Unsuitable Material for Structures	Cu. Yd.	30

The Contractor shall excavate to the specified depth for placement of the Rock Fill Foundation under the culvert extensions and the wingwall footings (if present). The limits and quantities of removal and replacement shown may be modified by the District Geotechnical and Field Engineers for variable subsurface conditions encountered in the field. See Special Provision for "Rock Fill - Foundation."



PLAN

MODEL: Untitled [Sheet]
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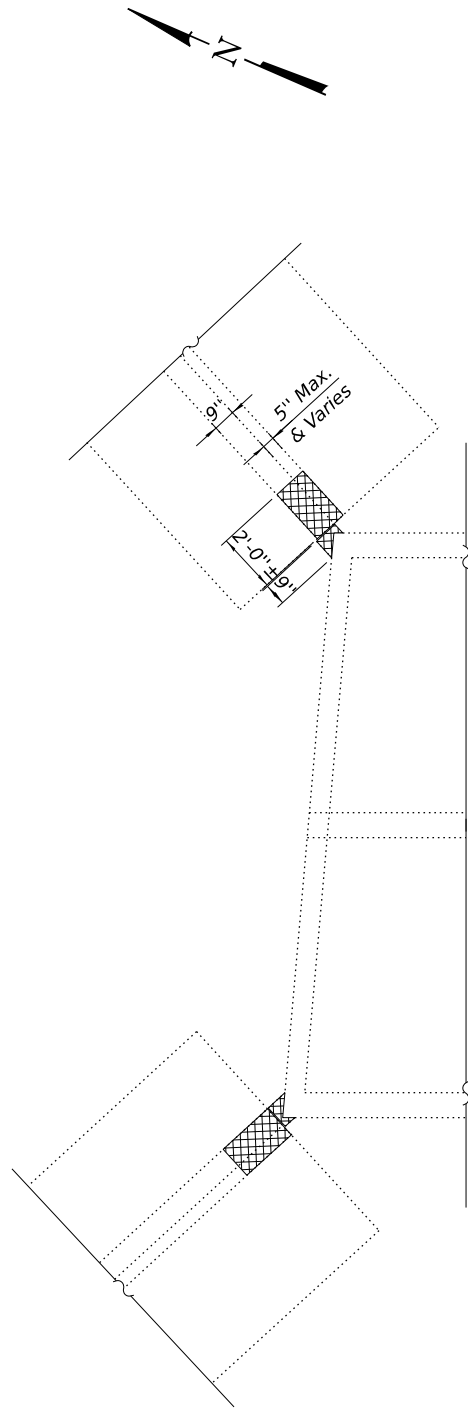
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

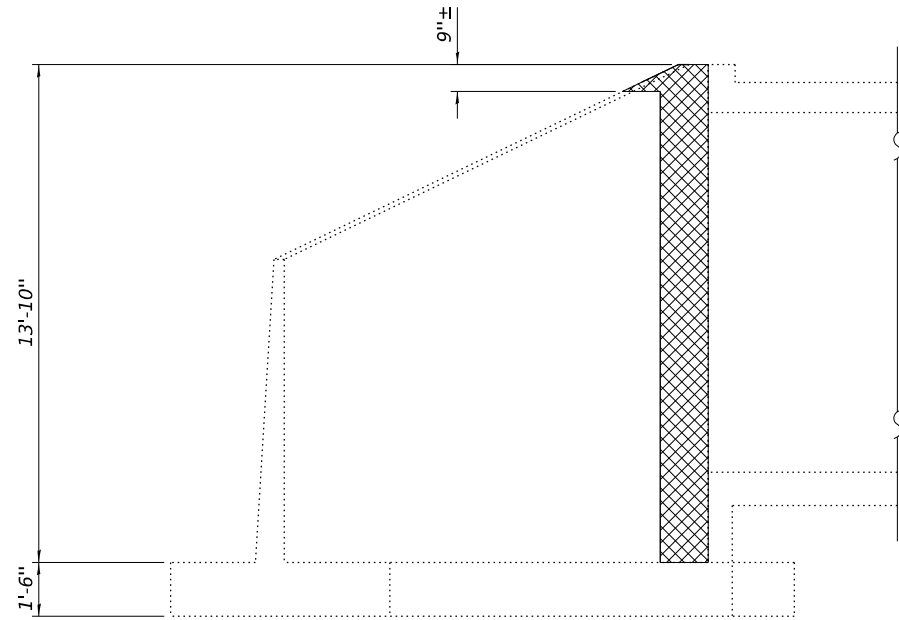
PLAN AND ELEVATION
S.N. 025-8302

SCALE: 1/4"=1'-0" SHEET 1 OF 4 SHEETS STA. ____+____ TO STA. ____+____

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	437
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				



WINGWALL REMOVAL DETAILS
Crosshatched areas indicate removal



ELEVATION

MODEL: Untitled [Sheet]
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CIVIL DESIGN, INC.

WBE | DBE

EFFINGHAM, IL

LICENSE # 184.003223

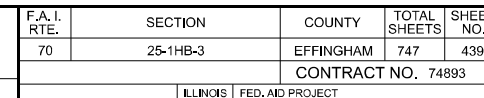
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PLOT DATE	= 10/14/2025	DATE	= 08/06/2024	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

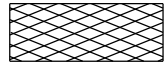
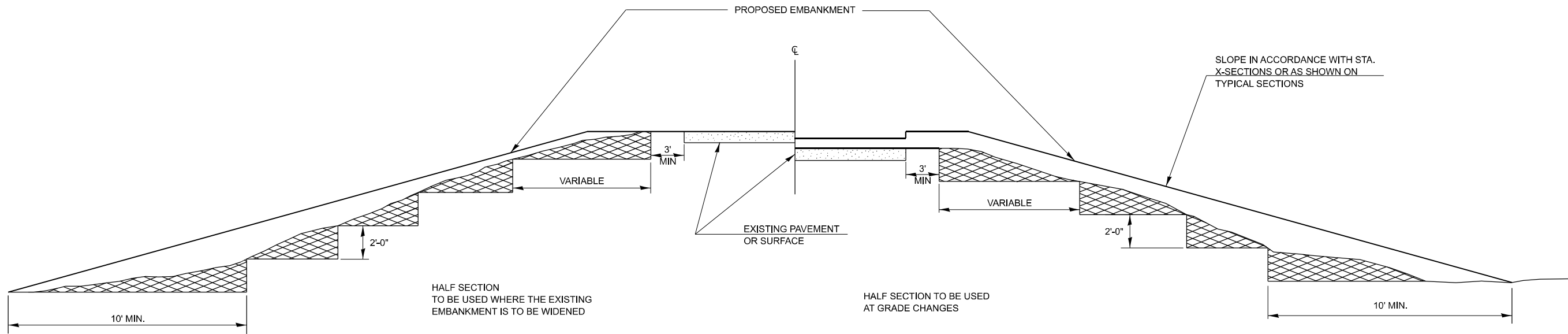
REMOVAL DETAILS
S.N. 025-8302

SCALE: 1/4"=1'-0" SHEET 2 OF 4 SHEETS STA. ____+____ TO STA. ____+____

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	25-1HB-3	EFFINGHAM	747	438
			CONTRACT NO. 74893	
		ILLINOIS	FED. AID PROJECT	



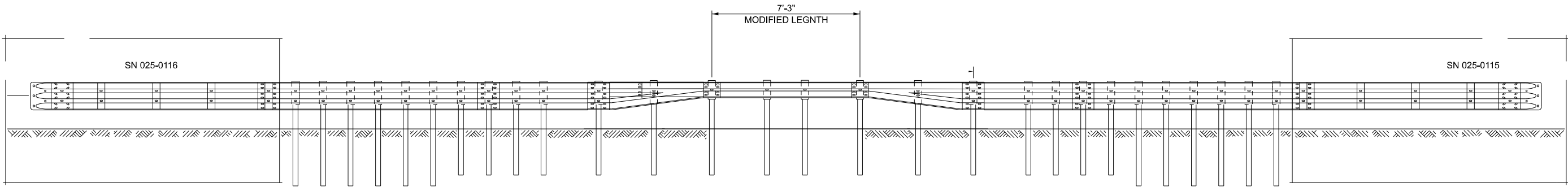
TYPICAL CROSS SECTION SHOWING
STEP CONSTRUCTION ON EXISTING FILL



STEPS SHALL BE CUT IN THE EXISTING SLOPE IN ACCORDANCE WITH ART. 205.03 EXCEPT THAT THE REQUIREMENT THAT STEPS SHALL BE CUT INTO THE EXISTING SLOPE BEFORE THE CONSTRUCTION OF THE EXISTING EMBANKMENT IS STARTED SHALL NOT APPLY. MATERIAL TO BE REMOVED AND REPLACED IN THE EMBANKMENT IN ACCORDANCE WITH ART. 205.04. COST TO BE INCLUDED IN THE VARIOUS ITEMS OF EXCAVATION AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED BECAUSE OF THIS WORK.

DRAWN	02-15-89
REVISED	05-07-08
REVISED	10-21-21
REVISED	05-27-22

STD. 9-25



TRAFFIC BARRIER TERMINAL, TYPE 6B (SPECIAL)
MODIFIED LENGTH DETAIL

MODEL: Detail_S8 [Sheet]
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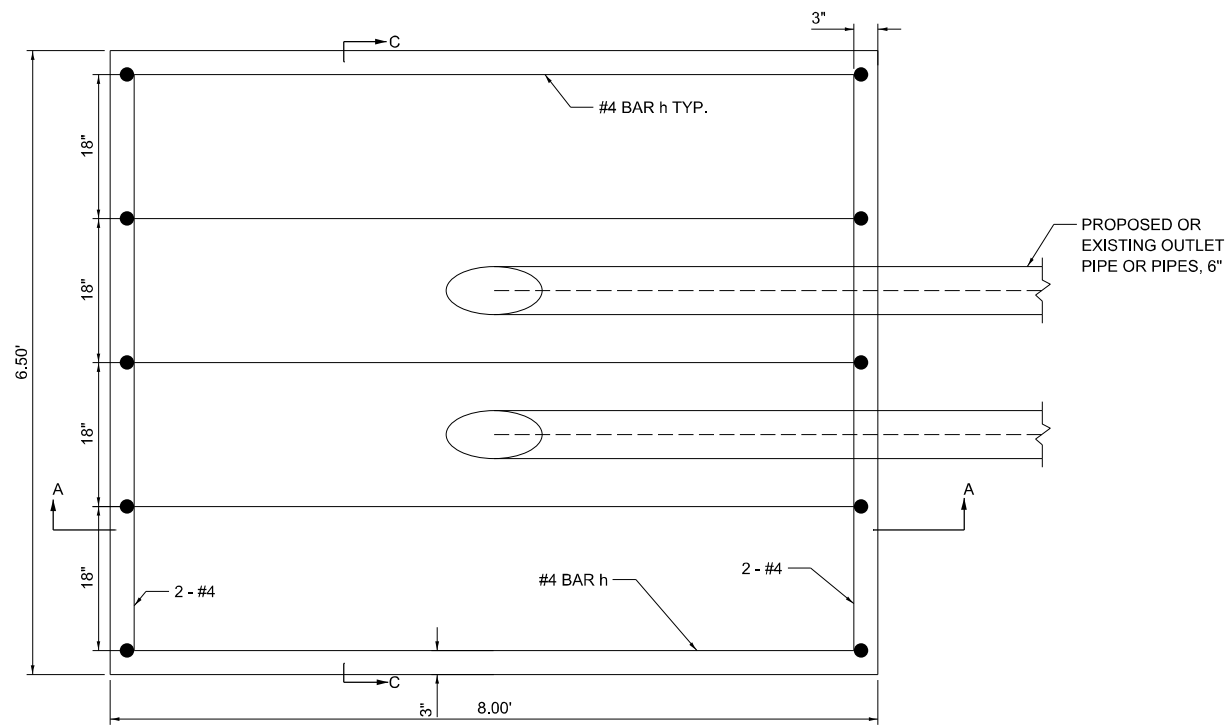
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PLOT DATE = 10/14/2025	DATE - 10/16/2025	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

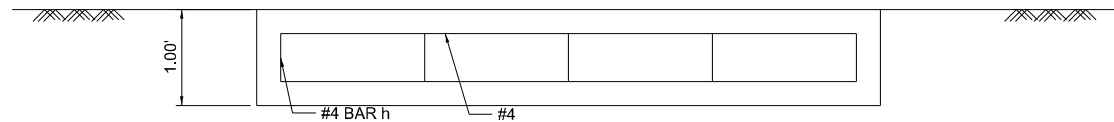
FAI ROUTE 70 - INTERSTATE 70
SLOPE BENCHING

SCALE: SHEET OF SHEETS STA. TO STA.

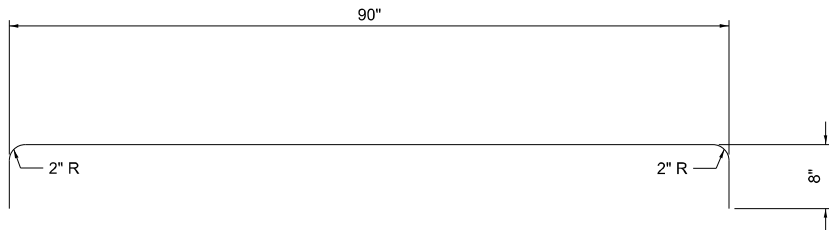
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	441
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				



PLAN VIEW



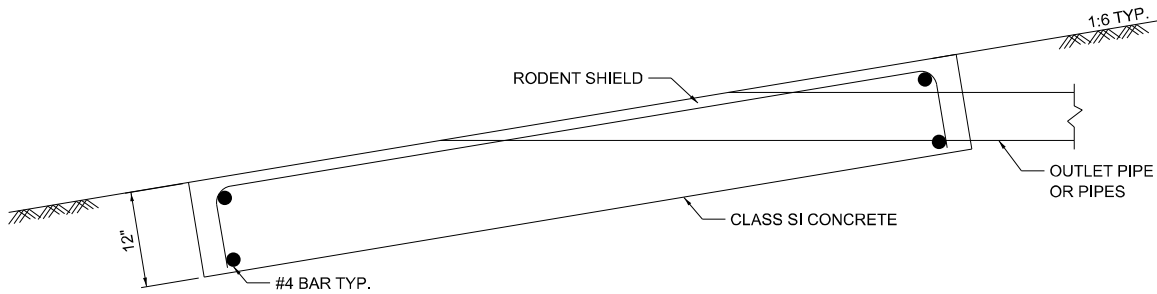
SECTION C-C



#4 h BAR

NOTES

- See Standard 601101 for details of rodent shields.
- The outlet pipes shall be located as close as possible to the center of the outlet protector.
- The last 10' of outlet pipe shall be schedule 40 PVC.
- The rebars may be cut or relocated to accomodate pipe.
- Cut outlet pipe on a bevel to match finished surface of surrounding PCC.
- Seeding Class 2 shall be considered included in the cost of the pay items.



SECTION A-A

APPROXIMATE OUTLET PROTECTOR QUANTITIES FOR EACH OUTLET PROTECTOR	
CONCRETE, CLASS SI	REINFORCING STEEL
CU YD	LB
2	45.5

MODEL: Outlet Protector Detail Plan2 (Sheet)
FILE NAME: p:\concrete\pcc\outlet protector.dgn
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DRAWN	- MLC
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CHECKED	- MJO
DATE	- 10/14/2025

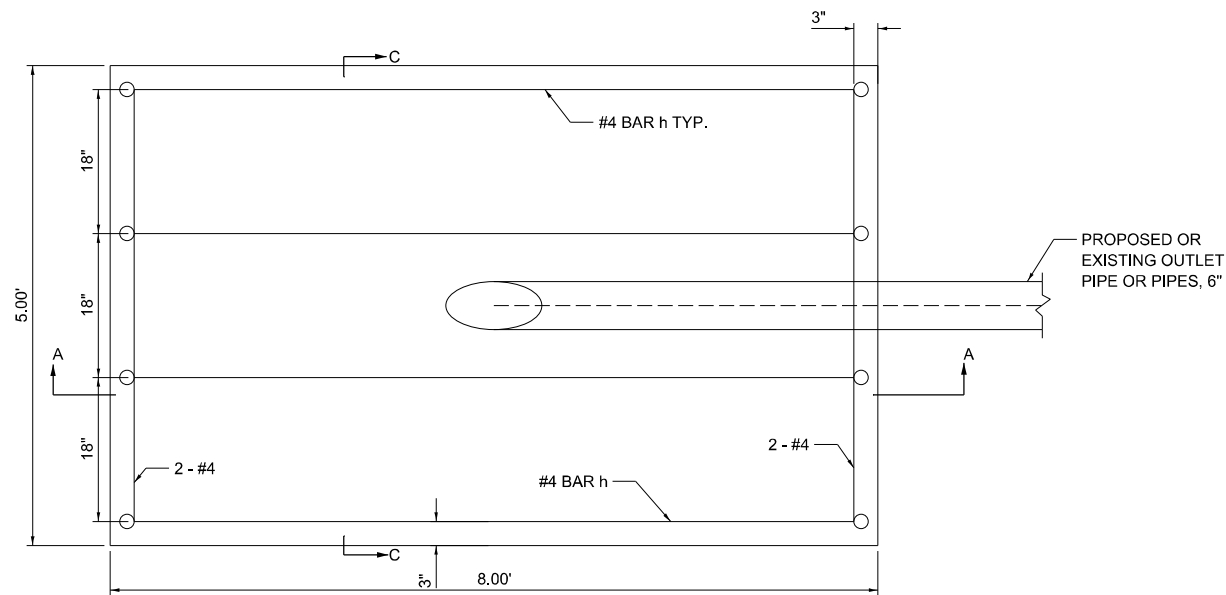
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

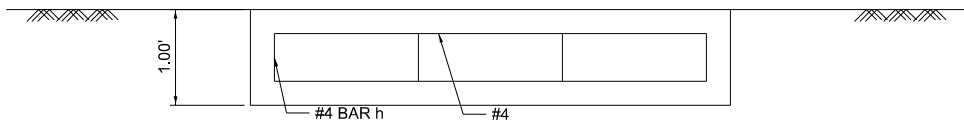
FAI ROUTE 70 - INTERSTATE 70
OUTLET PROTECTOR DETAIL

SCALE: 1"=20' SHEET 2 OF 2 SHEETS STA. TO STA.

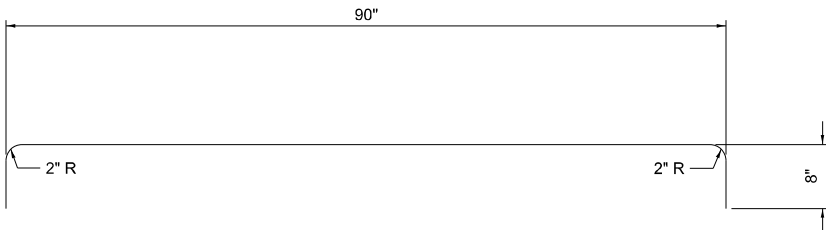
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70	(25-1)R-BR	EFFINGHAM	747	442
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				



PLAN VIEW



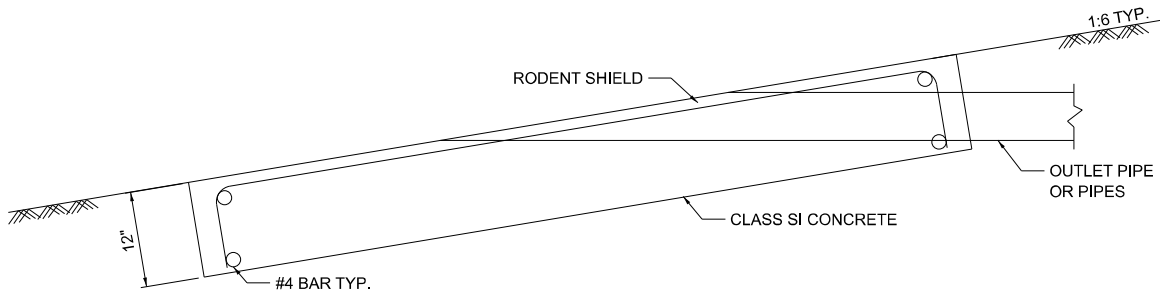
SECTION C-C



#4 h BAR

NOTES

- See Standard 601101 for details of rodent shields.
- The outlet pipe or pipes shall be located as close as possible to the center of the outlet protector.
- The last 10' of outlet pipe shall be schedule 40 PVC.
- The rebars may be cut or relocated to accomodate pipe.
- Cut outlet pipe on a bevel to match finished surface of surrounding PCC.
- Seeding Class 2 shall be considered included in the cost of the pay items.



SECTION A-A

APPROXIMATE OUTLET PROTECTOR QUANTITIES FOR EACH OUTLET PROTECTOR	
CONCRETE, CLASS SI	REINFORCING STEEL
CU YD	LB
1.5	35.6

MODEL: Outlet Protector Details Plan (Sheet)
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DRAWN - MLC	REVISIONS	
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PLOT DATE = 10/14/2025	DATE - 10/16/2025	REVISED -

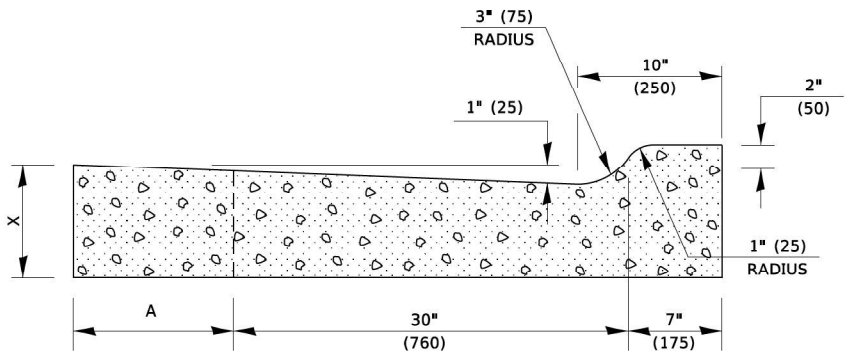
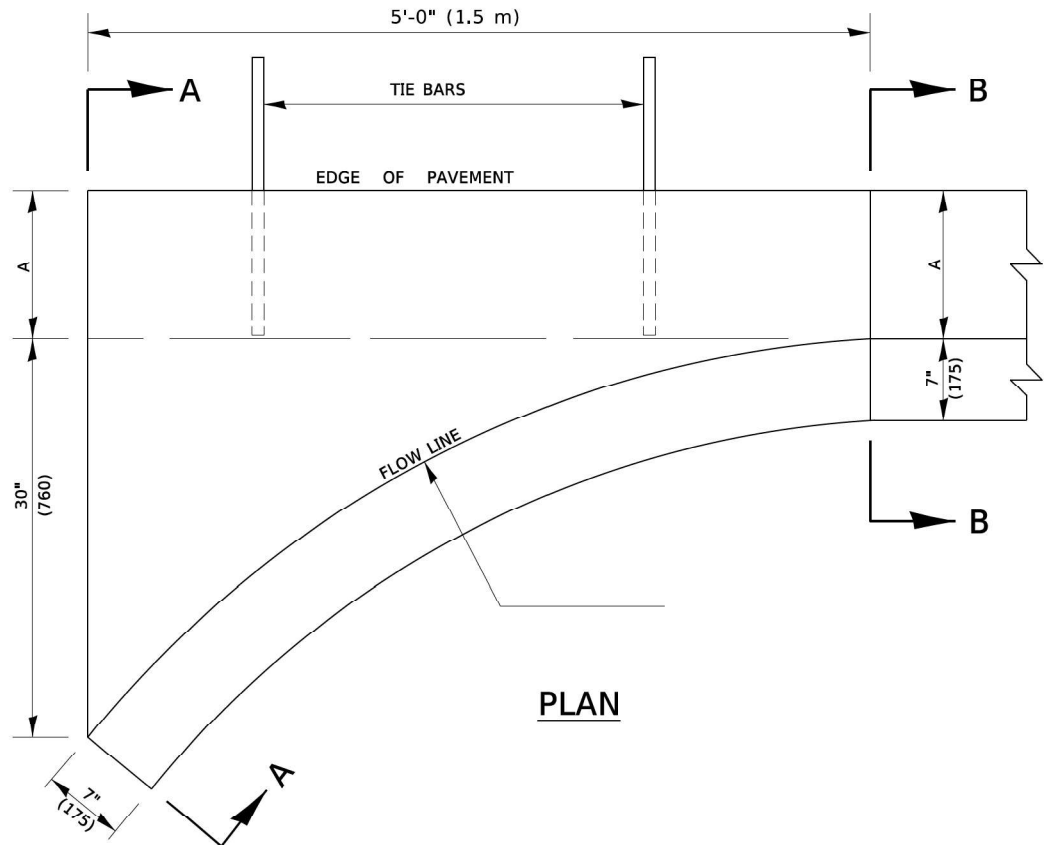
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 70 - INTERSTATE 70
OUTLET PROTECTOR DETAIL

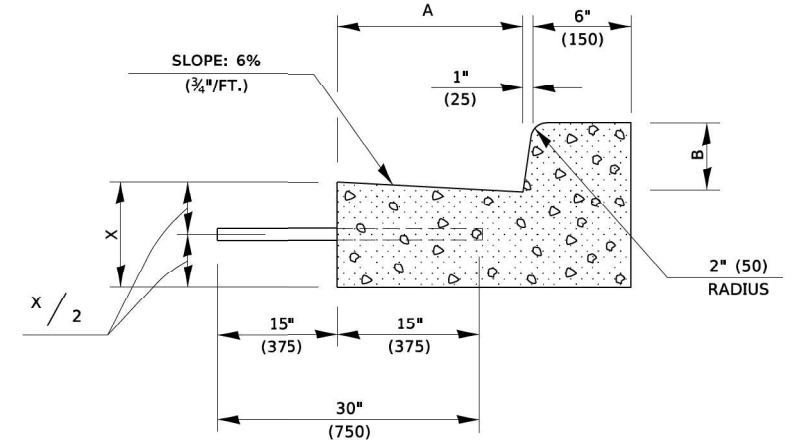
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F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	443
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				

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SECTION A-A



SECTION B-B

GENERAL NOTES

1. CLASS SI CONCRETE SHALL BE USED THROUGHOUT.
2. TIE BARS SHALL BE NO. 6 (NO. 20) AT 24" (600 mm) CENTERS UNLESS OTHERWISE SHOWN. OUTLETS SHALL BE TIED TO THE PAVEMENT IN ACCORDANCE WITH DETAILS FOR LONGITUDINAL CONSTRUCTION JOINT SHOWN ON STANDARD 420001.
3. WHEN SPECIAL OUTLET IS CONSTRUCTED ADJACENT TO FLEXIBLE PAVEMENT, THE TIE BARS SHALL BE OMITTED AND ALL CONSTRUCTION JOINTS SHALL BE PROVIDED WITH A DOWEL BAR CONFORMING TO ARTICLE 1006.11(b).

	B-6.12 (B15.30)		B-9.12 (B-22.30)		B-6.18 (B-15.45)		B-9.18 (B22.45)		B-6.24 (B15.60)		B-9.24 (B-22.60)	
A	12" (300)	12" (300)	12" (300)	12" (300)	18" (450)	18" (450)	16" (450)	18" (450)	24" (600)	24" (600)	24" (600)	24" (600)
B	6" (150)	6" (150)	9" (225)	9" (225)	6" (150)	6" (150)	9" (225)	9" (225)	6" (150)	6" (150)	9" (225)	9" (225)
X	9" (225)	10" (250)	9" (225)	10" (250)	9" (225)	10" (250)	9" (225)	10" (250)	9" (225)	10" (250)	9" (225)	10" (250)
CU. YD. (m ³) CLASS SI CONCRETE (OUTLET)	0.37 (0.28)	0.42 (0.32)	0.38 (0.29)	0.42 (0.32)	0.44 (0.34)	0.49 (0.37)	0.45 (0.34)	0.50 (0.38)	0.51 (0.39)	0.56 (0.43)	0.52 (0.40)	0.57 (0.44)

NOT TO SCALE
Note: All dimensions are in INCHES
(millimeters) unless otherwise shown.

DISTRICT 7 DETAIL NO. X6060097

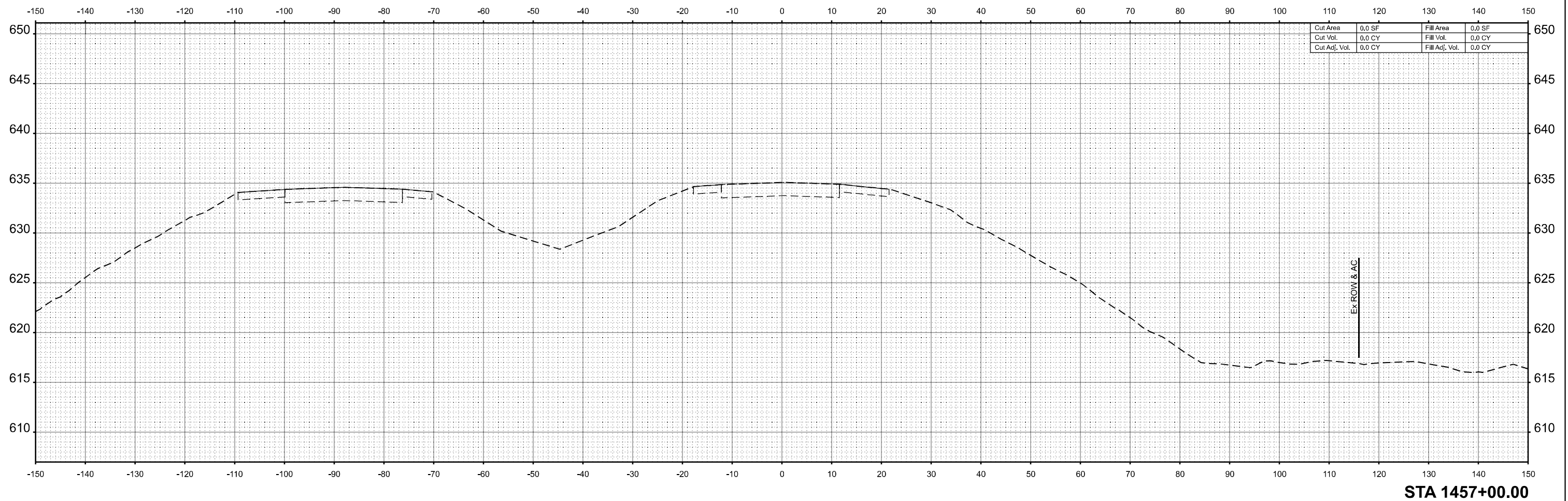


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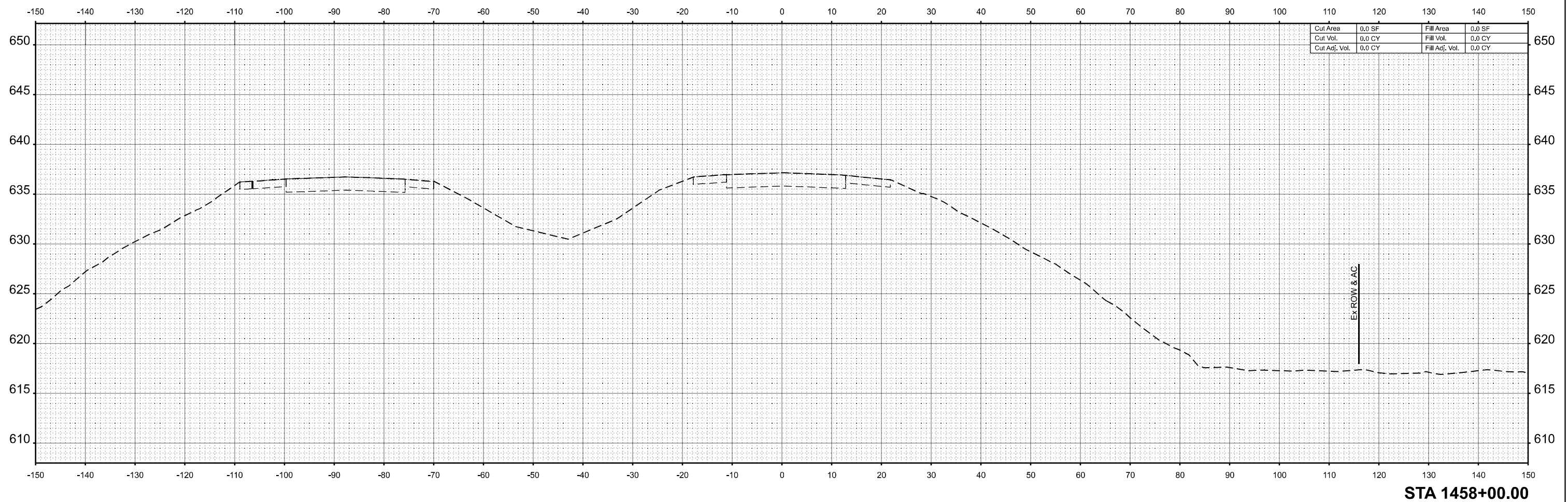
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 70 - INTERSTATE 70
CLASS SI CONCRETE (OUTLET) SPECIAL DETAILS
SCALE: 1"=20'

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	444
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				



STA 1457+00.00



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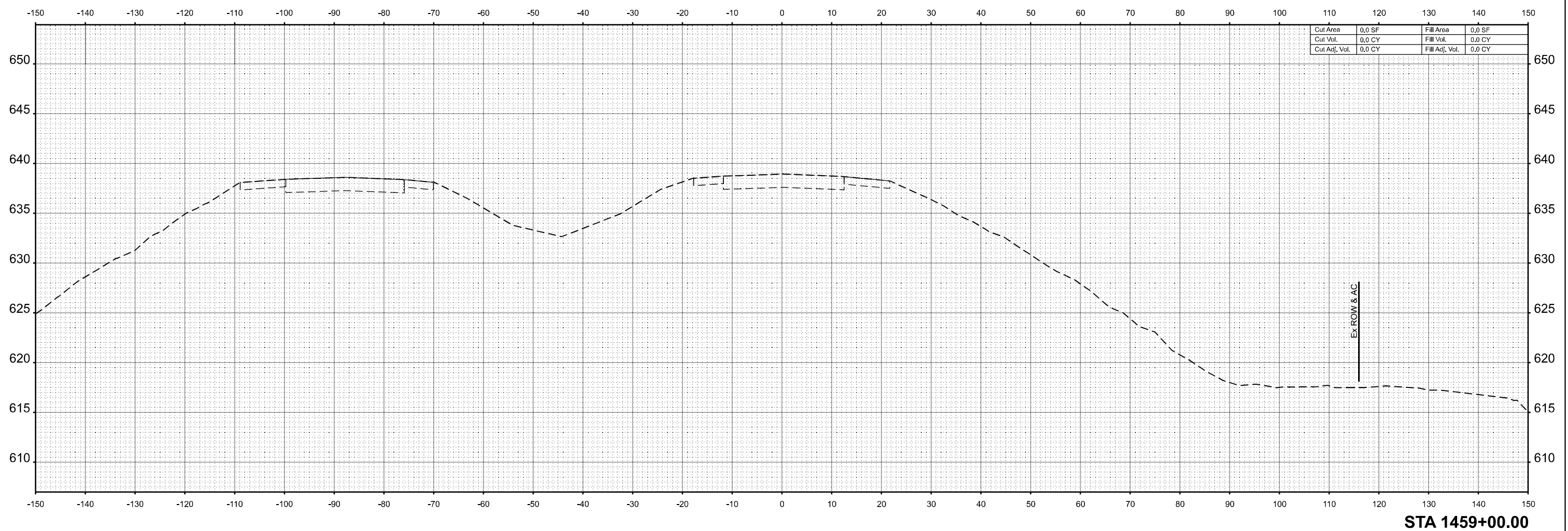
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10'	SHEET 5	OF 90	SHEETS	STA.	TO STA.
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F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	449
		CONTRACT NO. 74893		
ILLINOIS		FED. AID PROJECT		



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PLOT DATE	= 10/14/202

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CHECKED	-	MJO
DATE	-	10/16/2025

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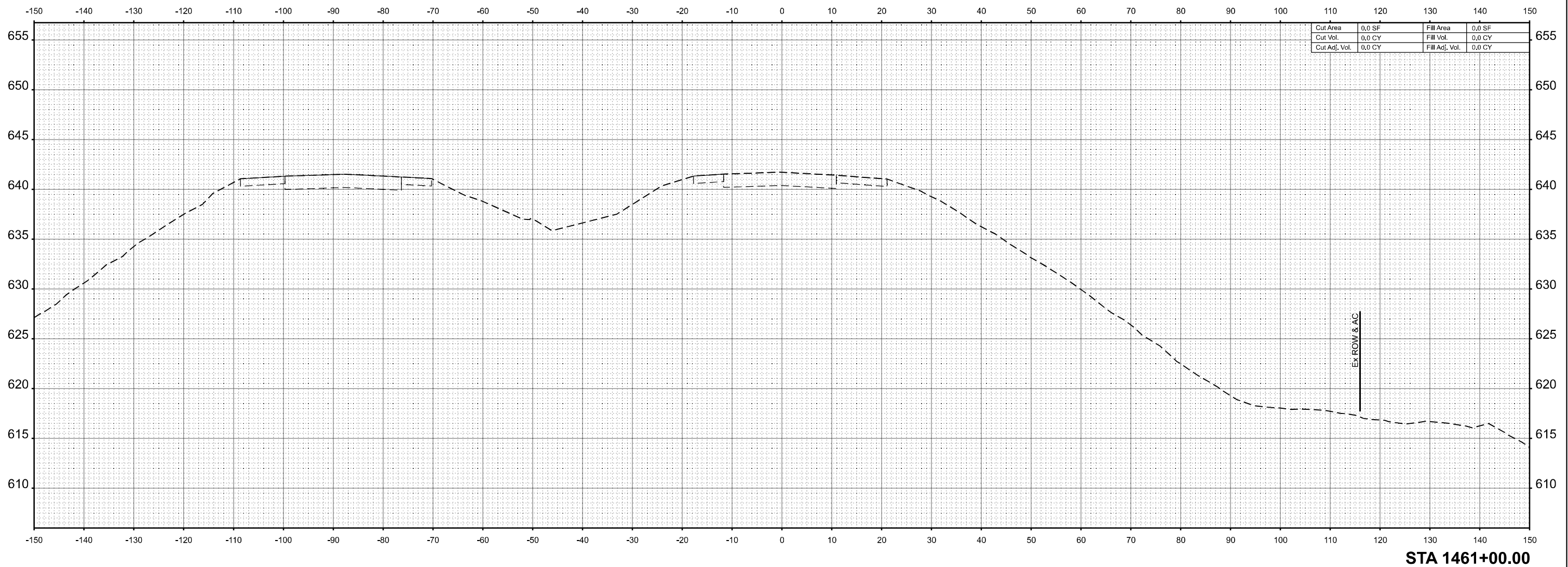
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10'	SHEET 7	OF 90	SHEETS	STA.	TO STA.
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F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	451
CONTRACT NO. 74893				
ILLINOIS		FED. AID PROJECT		

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PLOT DATE	= 10/14/2025

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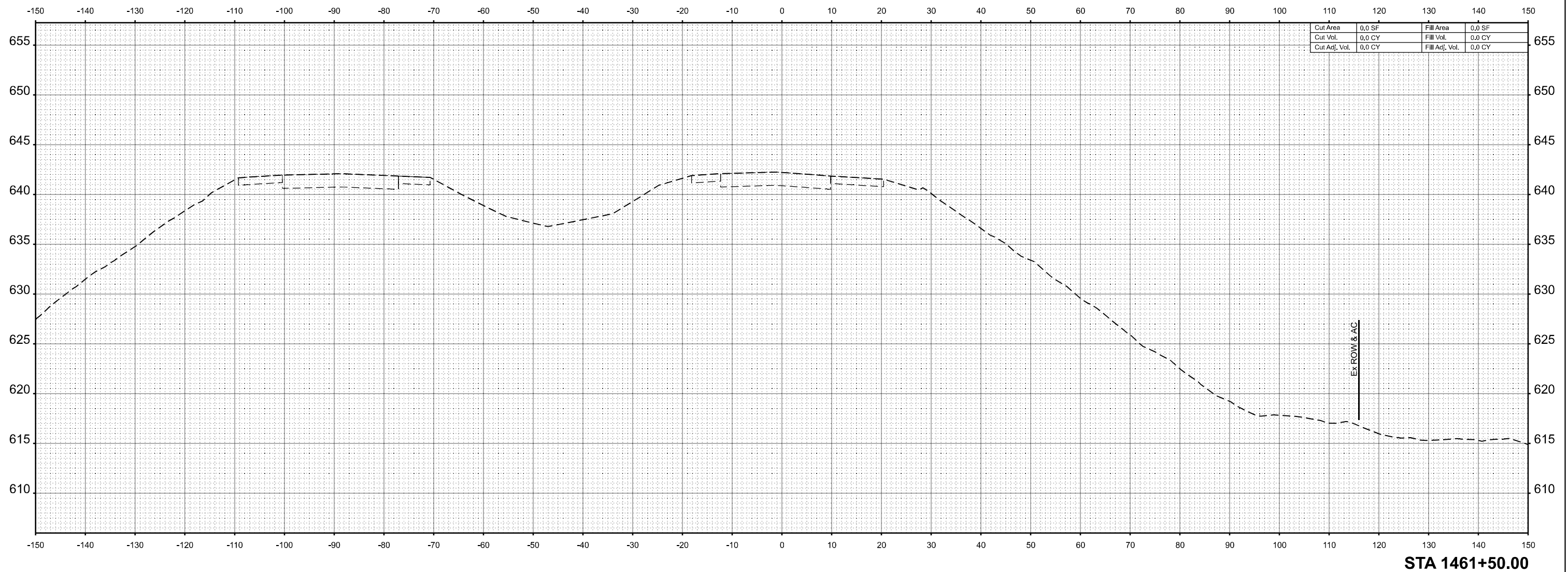
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

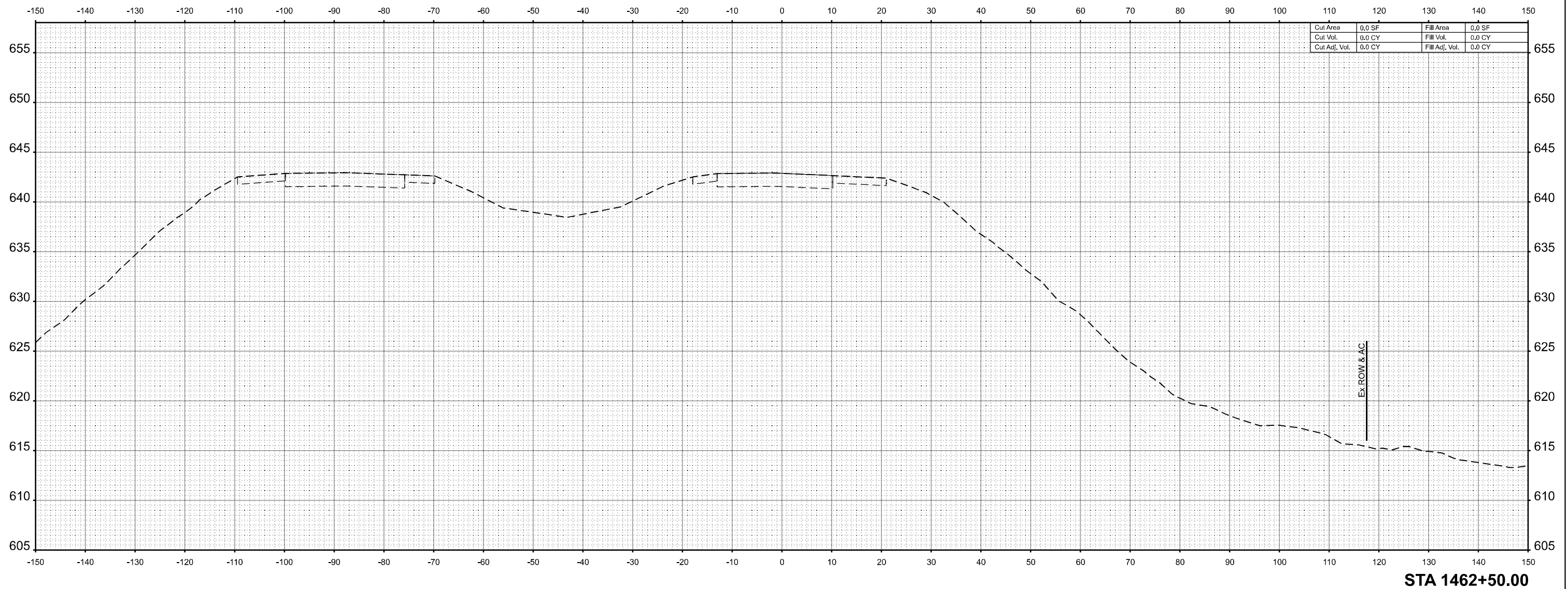
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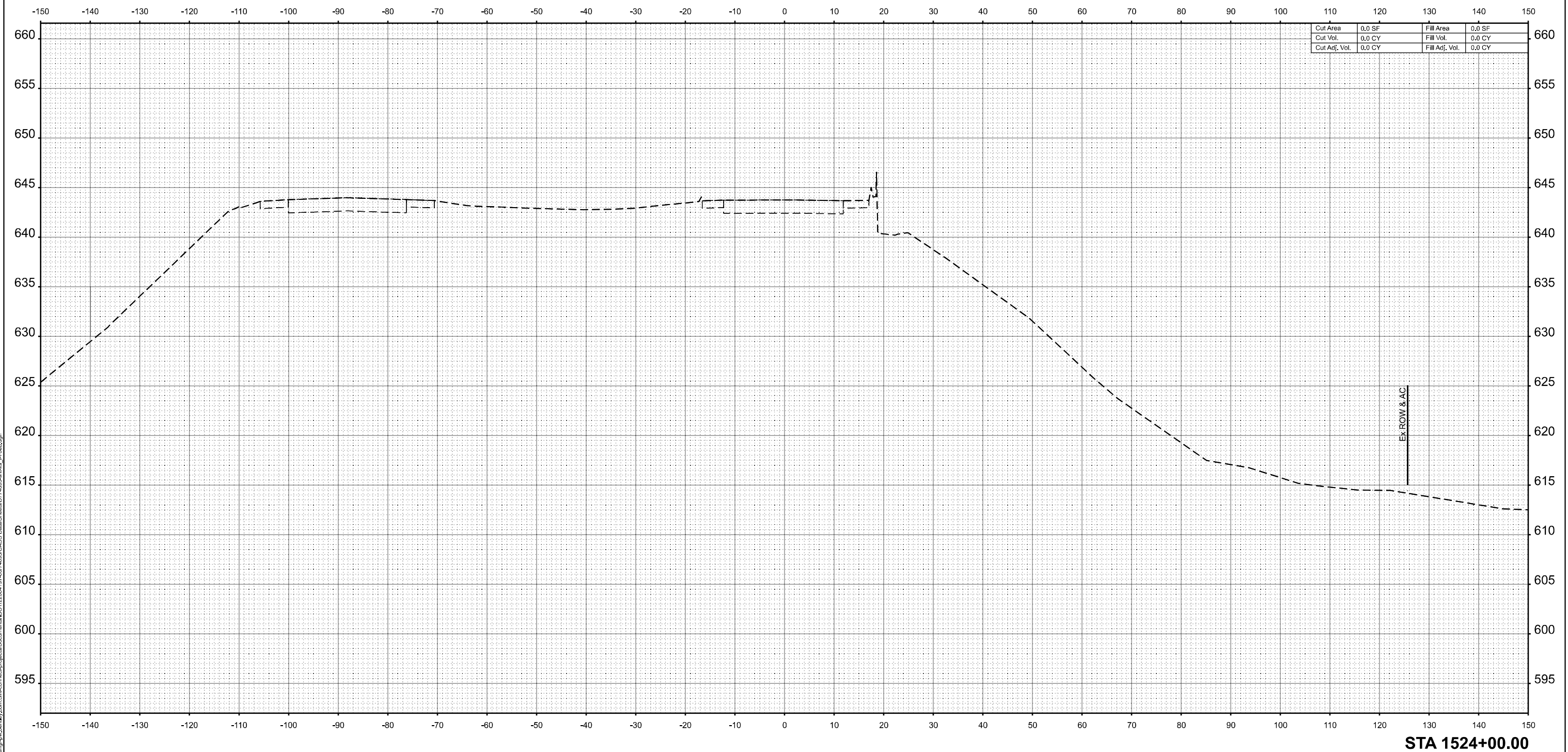
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70	(25-1)R-BR	EFFINGHAM	747	455
CONTRACT NO. 74893				
ILLINOIS			FED. AID PROJECT	

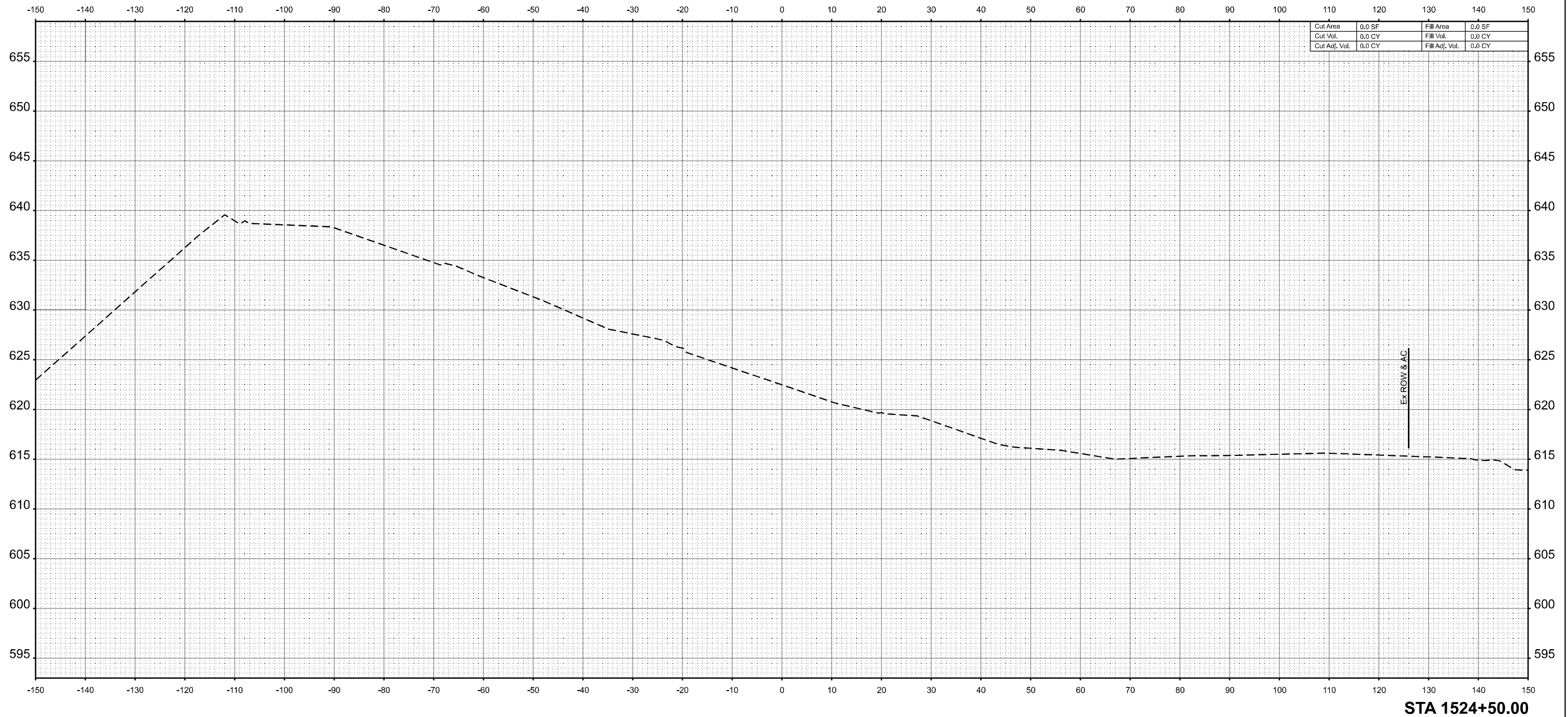


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Cut Vol.	0.0 CY	Fill Vol.	0.0 CY
Cut Adj. Vol.	0.0 CY	Fill Adj. Vol.	0.0 CY

**STA 1462+50.00**



STA 1524+00.00



STA 1524+50.00



USER NAME = moverbey

DESIGNED - AJ

DRAWN - AJ

PLOT SCALE = 0.166600

CHECKED - MJC

REVISÉ -

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REVISÉ -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10'

SHEET

OF 90

S	STA.
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F.A.I.

RTL.
70

SECTION

(25.4)B BB

COUNTY

FEELING

CONTRA

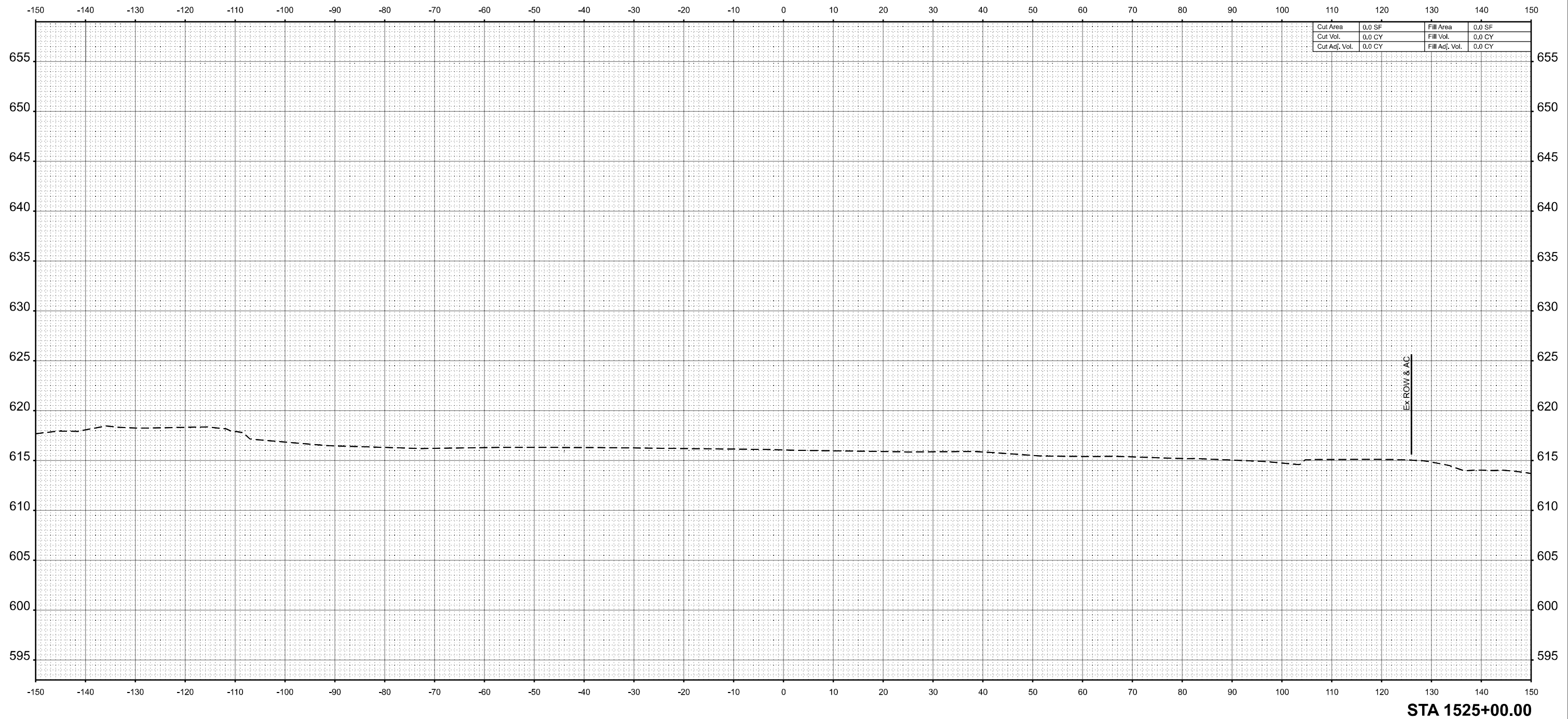
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CONTRA

ILLINOIS	FED. AID PROJECT
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DATE	-	10/16/2025

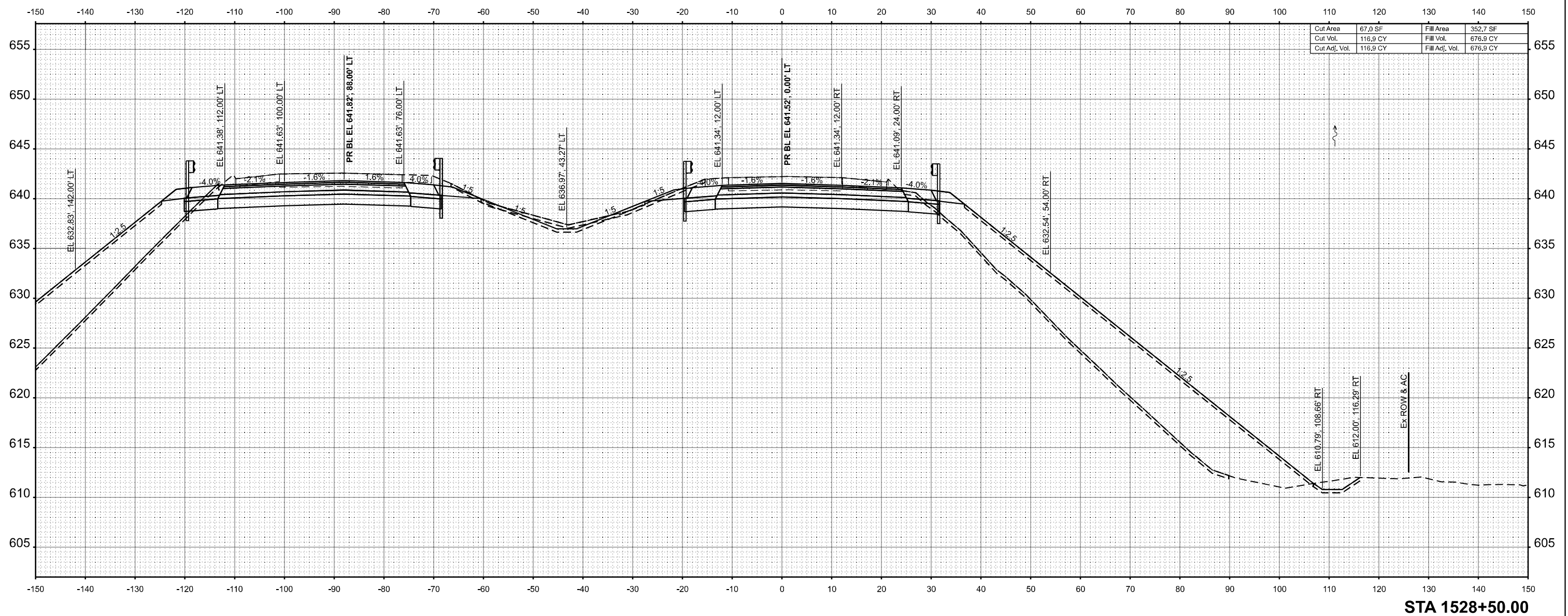
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

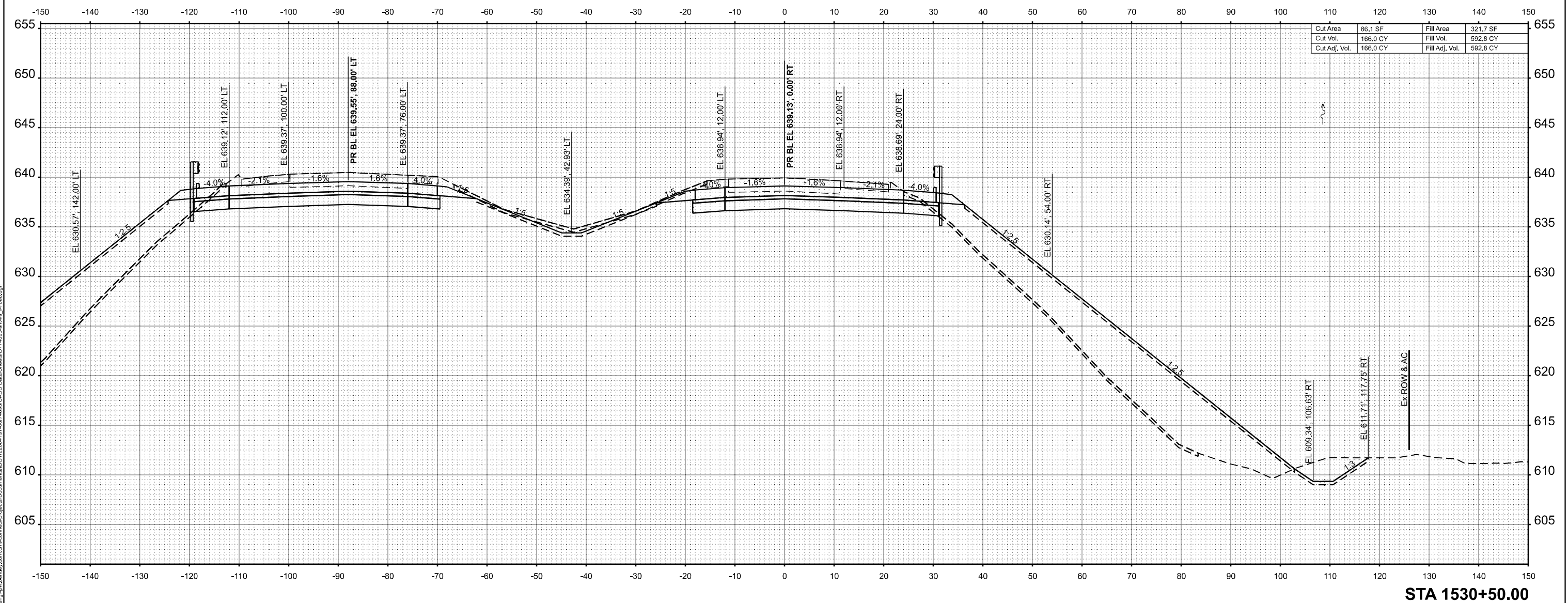
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CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10' SHEET 21 OF 90 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	465
CONTRACT NO. 74893				
		ILLINOIS	FED. AID PROJECT	



Cut Area	67.0 SF	Fill Area	352.7 SF
Cut Vol.	116.9 CY	Fill Vol.	676.9 CY
Cut Adj. Vol.	116.9 CY	Fill Adj. Vol.	676.9 CY



STA 1530+50.00

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DESIGNED -	AJ
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

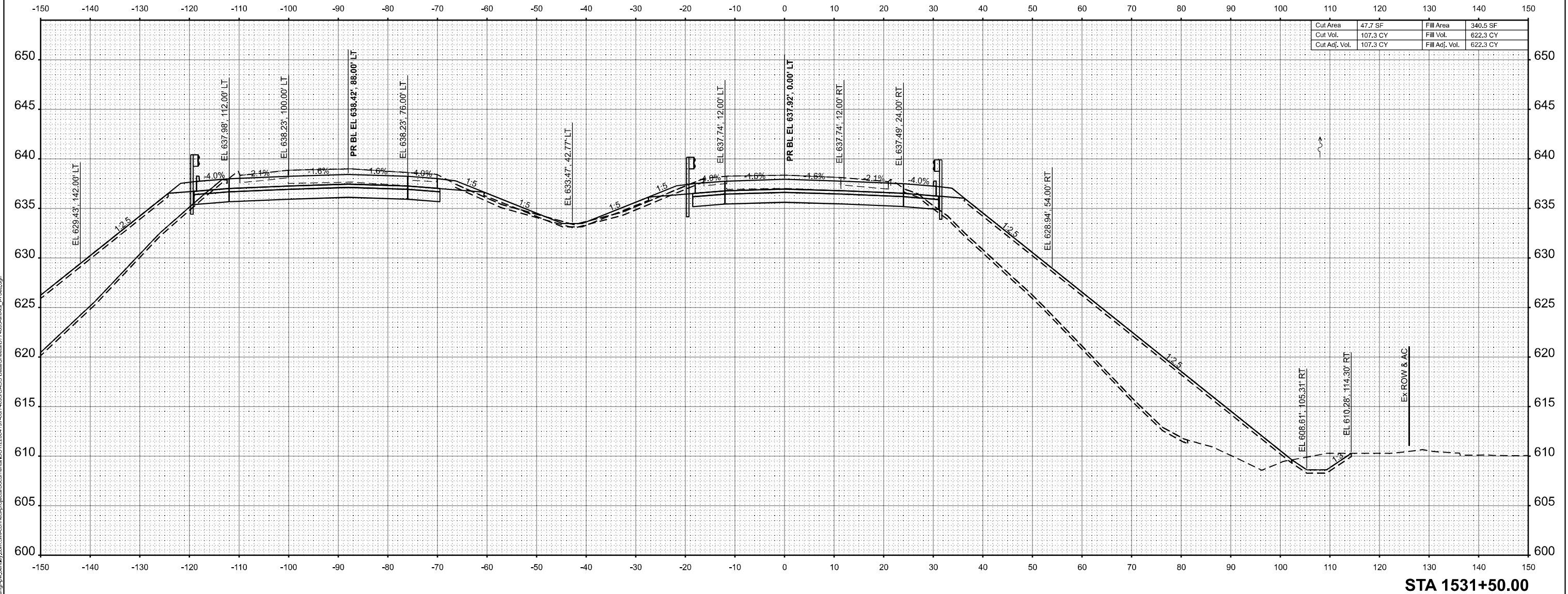
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SECTION

COUNTY

TOTAL	SHE
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7/1/81	EXPENSE	7/1/81	7/1/81
	CONTRACT NO. 74893		



Cut Area	47.7 SF	Fill Area	340.5 SF
Cut Vol.	107.3 CY	Fill Vol.	622.3 CY
Cut Adj. Vol.	107.3 CY	Fill Adj. Vol.	622.3 CY



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PLOT DATE = 10/14/2025	DATE - 10/16/2025

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DATE	-	10/16/2025

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

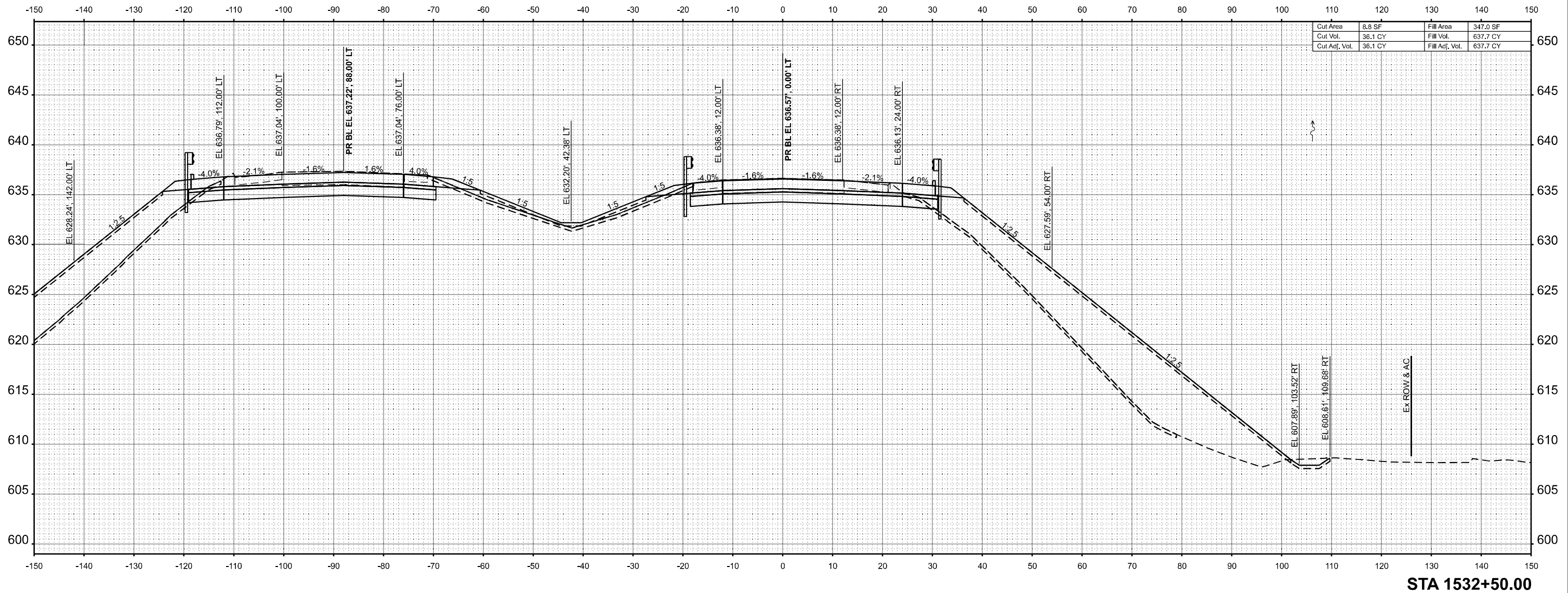
FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10'	SHEET 34	OF 90	SHEETS	STA.	TO STA.
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F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	478
CONTRACT NO. 74893				
ILLINOIS		FED. AID PROJECT		

STA 1531+50.00

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REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

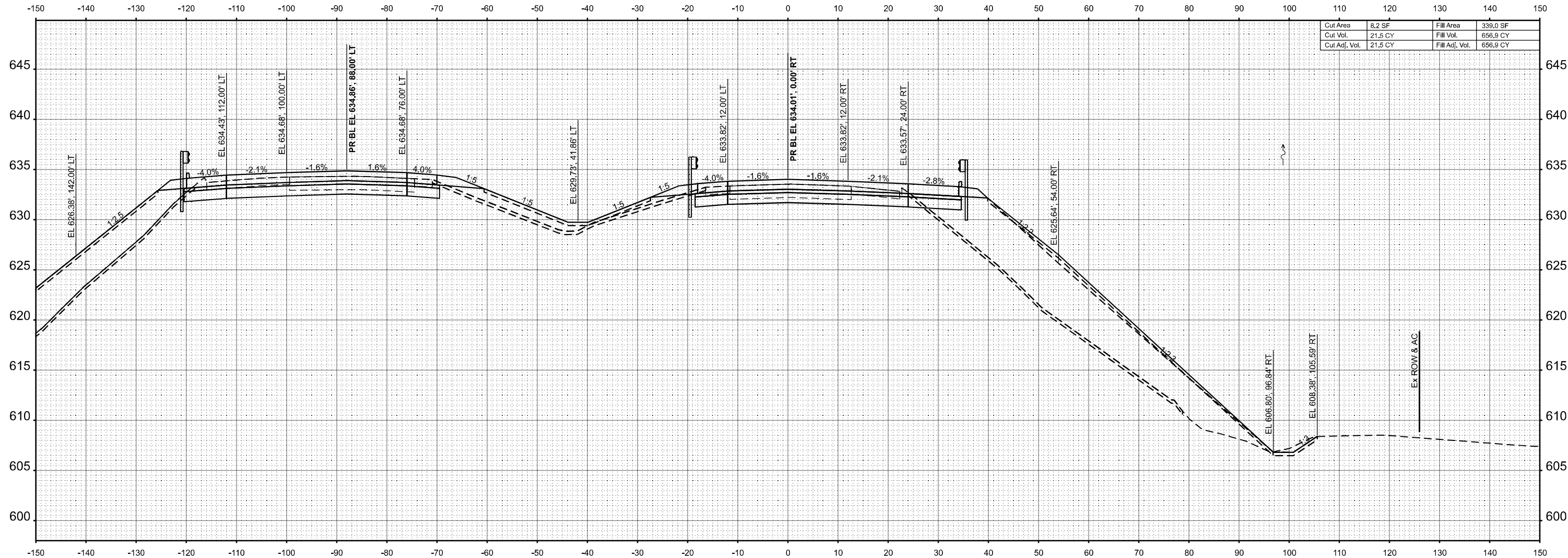
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CROSS SECTIONS - I-70 EASTBOUND

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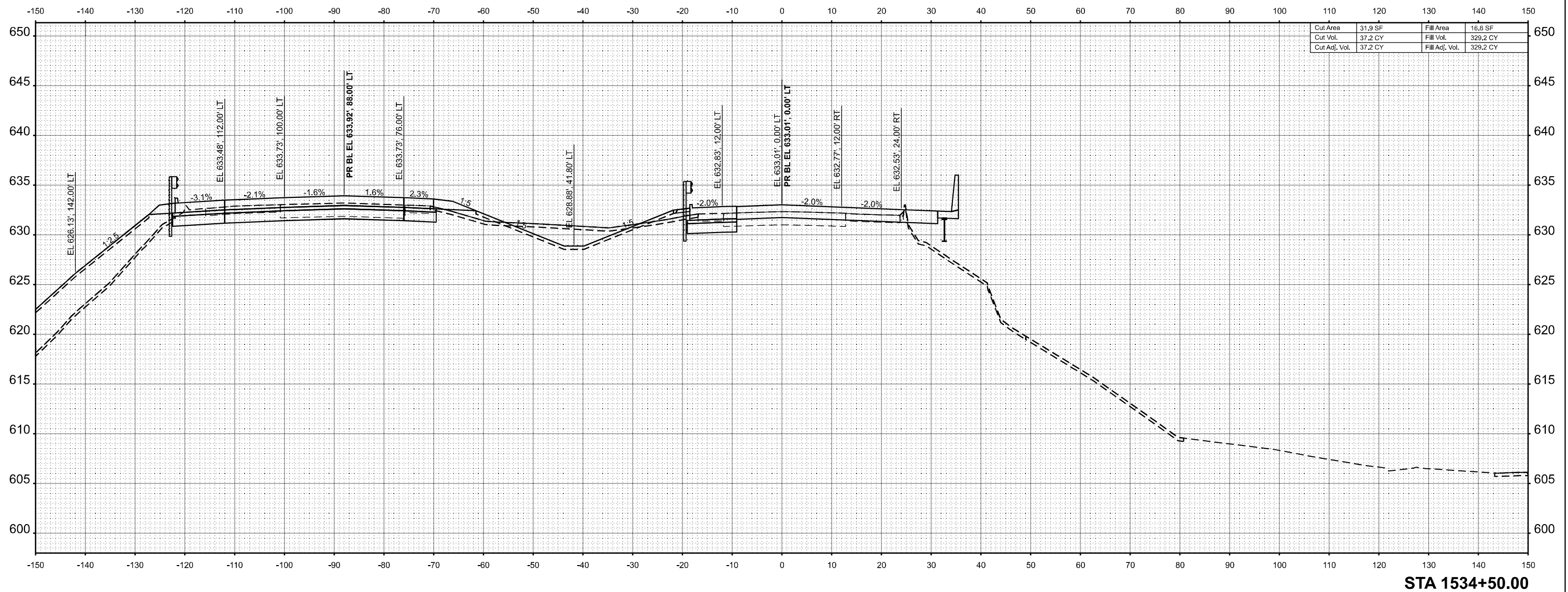
SHEET 36 OF 90 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	480
CONTRACT NO. 74893				
ILLINOIS		FED. AID PROJECT		

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STA 1534+00.00



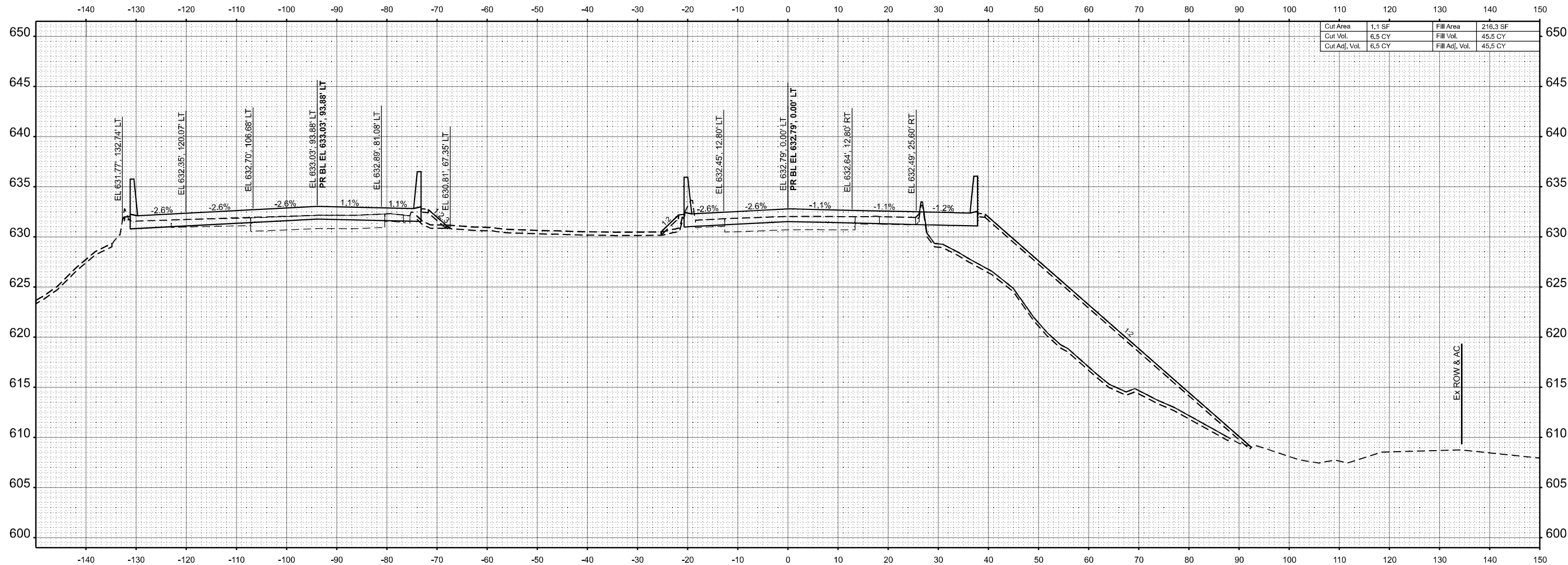
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Cut Area	1.1 SF	Fill Area	216.3 SF
Cut Vol.	6.5 CY	Fill Vol.	45.5 CY
Cut Adj. Vol.	6.5 CY	Fill Adj. Vol.	45.5 CY

STA 1534+60.56



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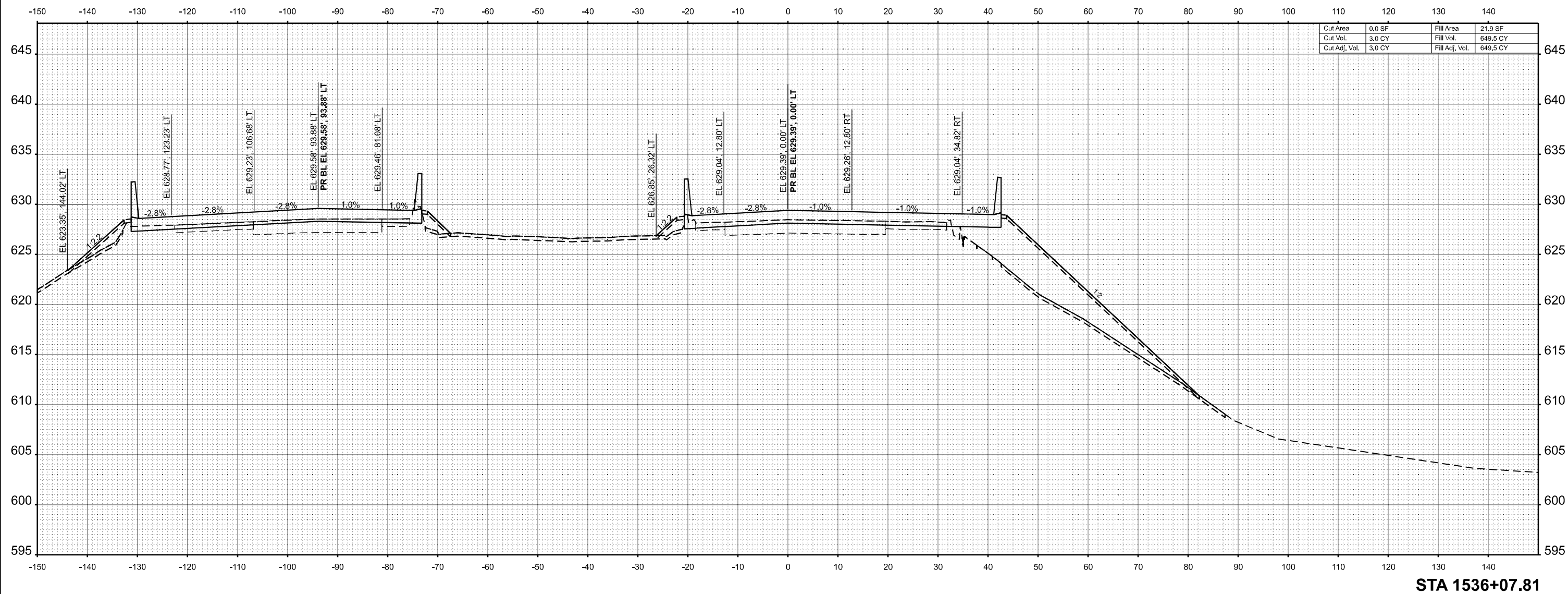
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PLOT DATE = 10/14/2025	DATE - 10/16/2025	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10' SHEET 41 OF 90 SHEETS STA. TO STA.

FAI RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	485
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				



STA 1536+07.81

License No. 184-000613

USER NAME	= moverbey
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DESIGNED -	AJ
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REVISÉ -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

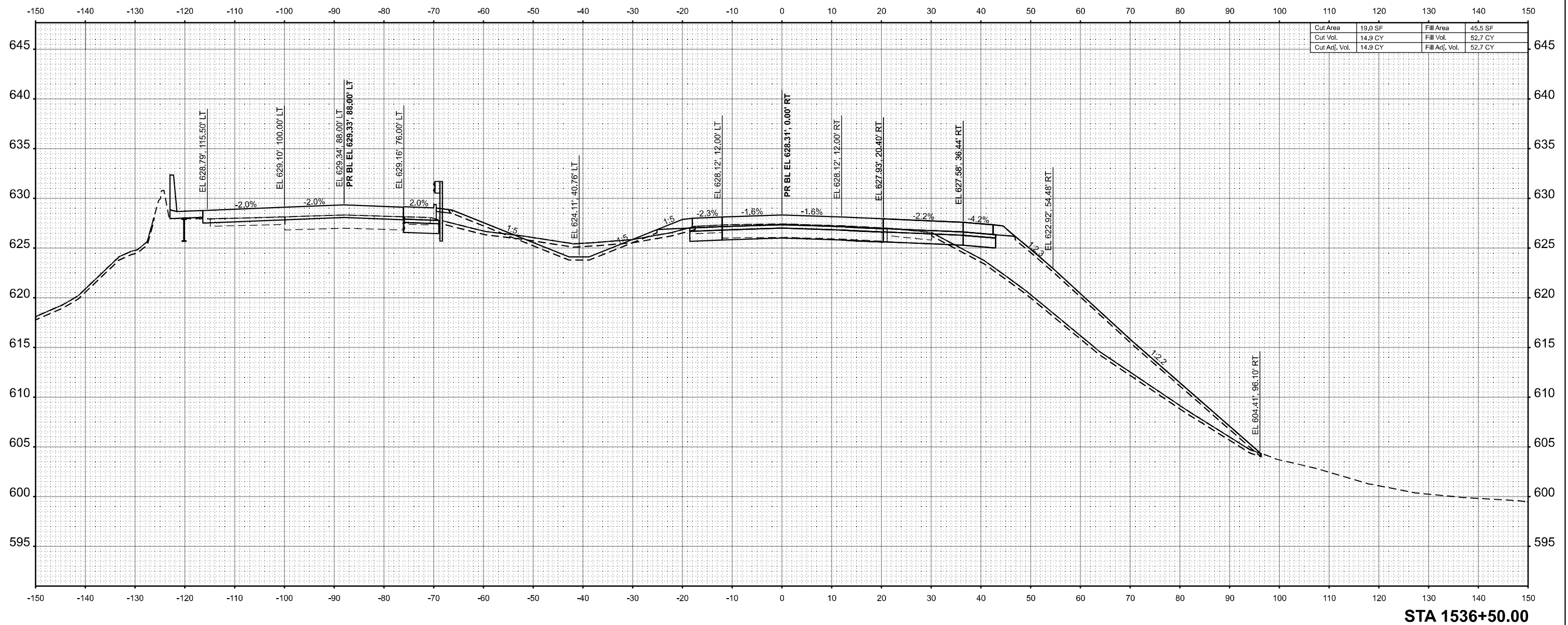
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SECTION

COUNTY

SHEE

CONTRACT NO. 74893	100	100	100
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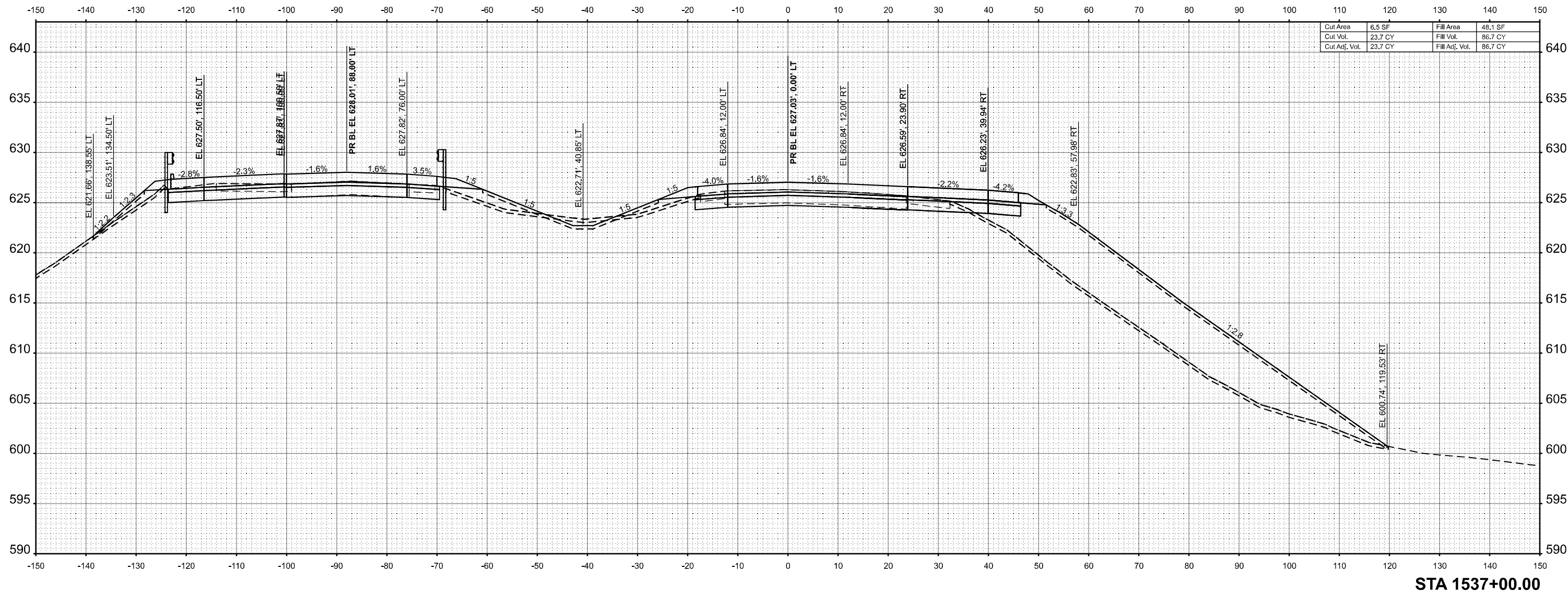


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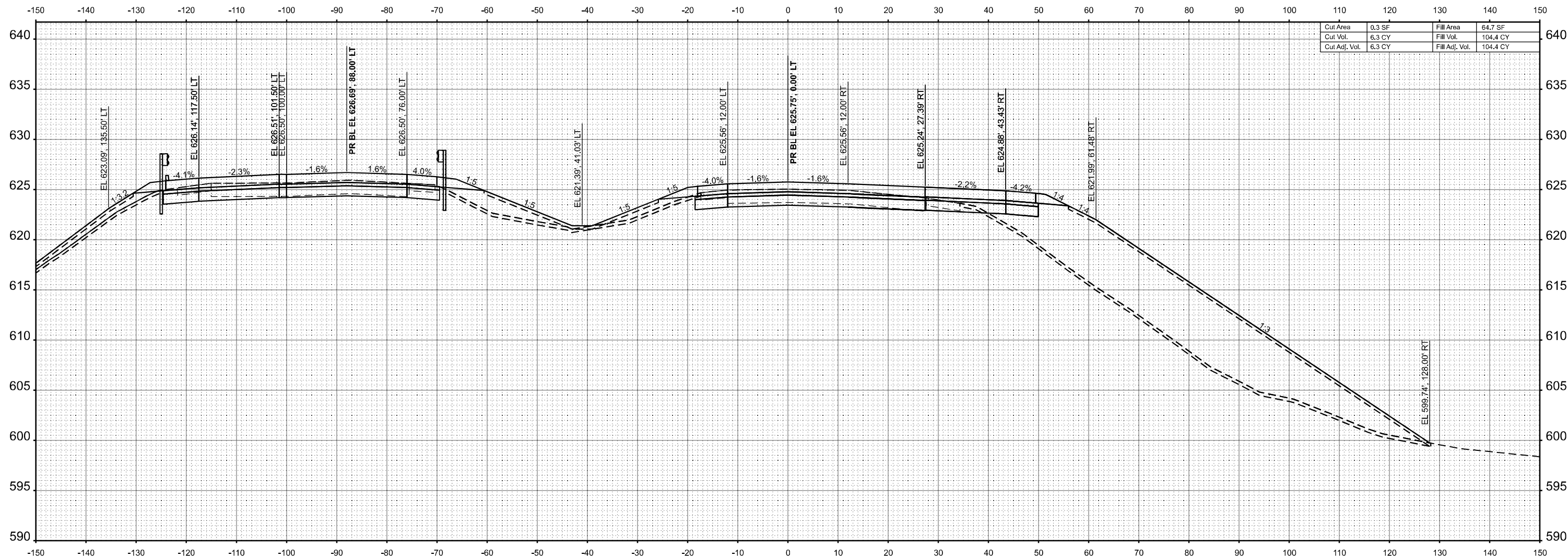
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Cut Area	0.3 SF	Fill Area	64.7 SF
Cut Vol.	6.3 CY	Fill Vol.	104.4 CY
Cut Adj. Vol.	6.3 CY	Fill Adj. Vol.	104.4 CY

STA 1537+50.00



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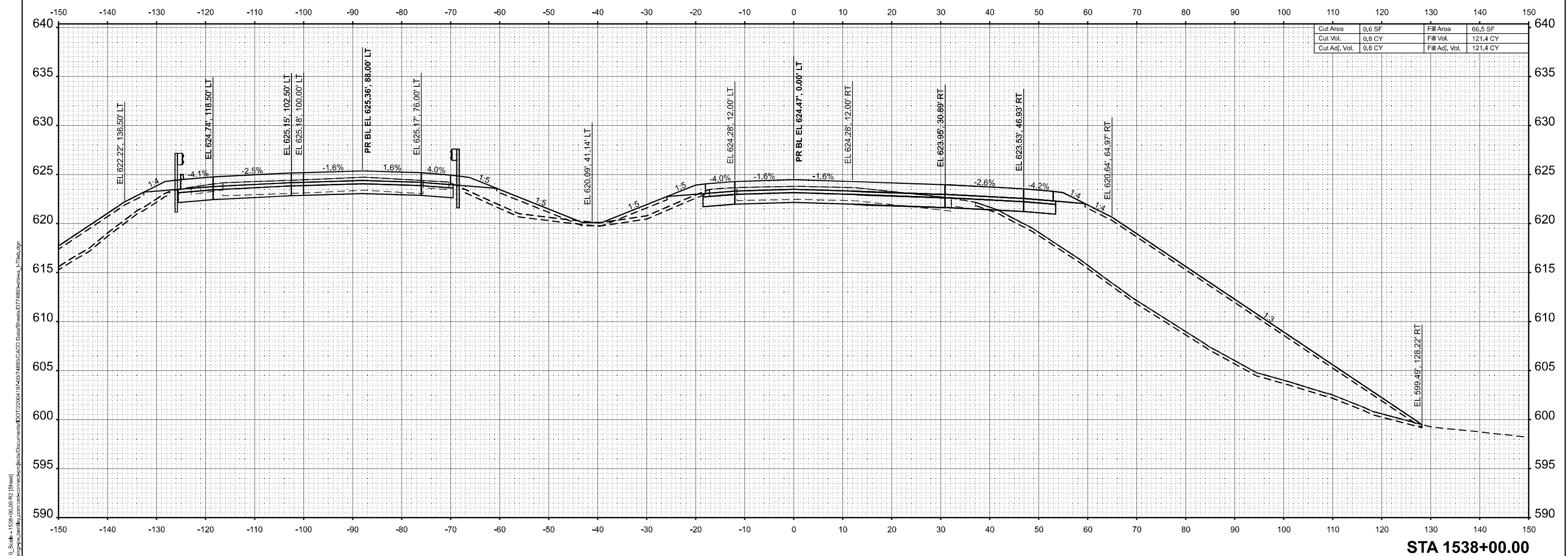
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PLOT DATE = 10/14/2025	DATE - 10/16/2025	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10' SHEET 45 OF 90 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	489
CONTRACT NO. 74893				
ILLINOIS FED. AID PROJECT				



STA 1538+00.00



USER NAME	= moverbey
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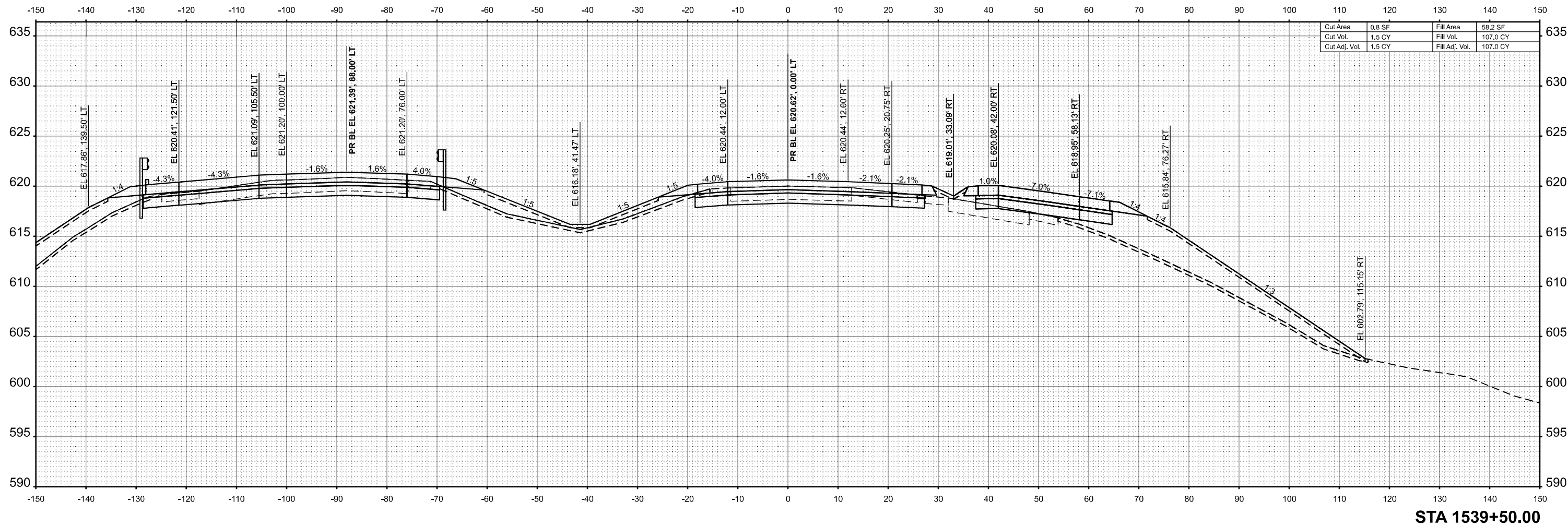
DESIGNED -	AJ
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REVISÉ -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	490
CONTRACT NO. 74893				
ILLINOIS		FED. AID PROJECT		

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USER NAME = moverbey
PLOT SCALE = 0.16660000' / in.
PLOT DATE = 10/14/2025

DESIGNED - AJ
DRAWN - AJ
CHECKED - MJO
DATE - 10/16/2025

REVISED -
REVISED -
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REVISED -

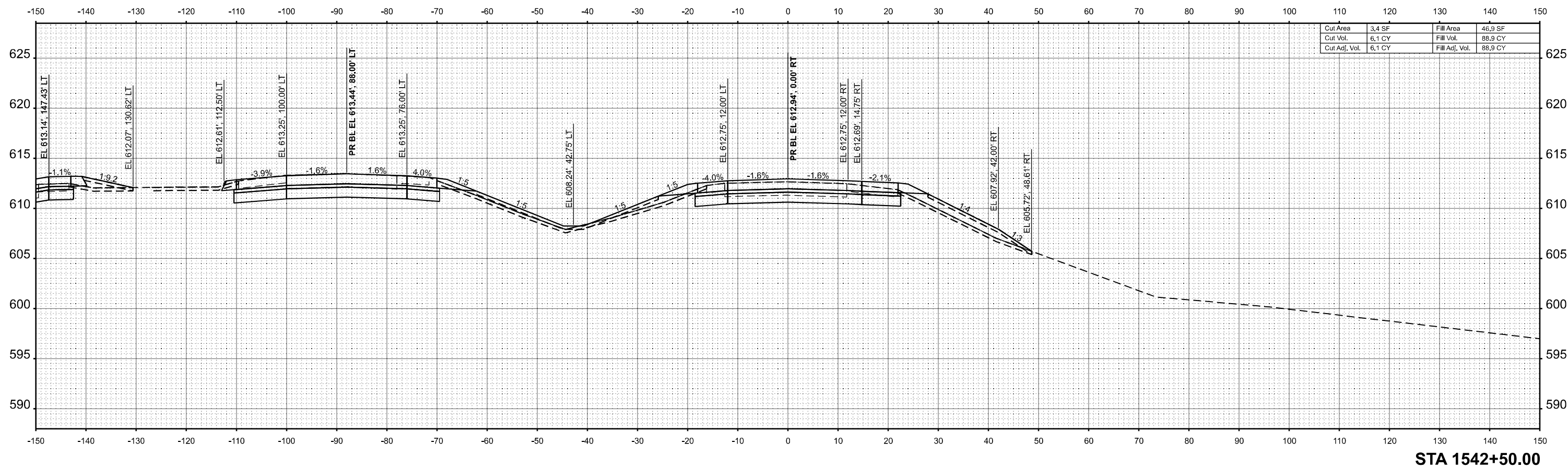
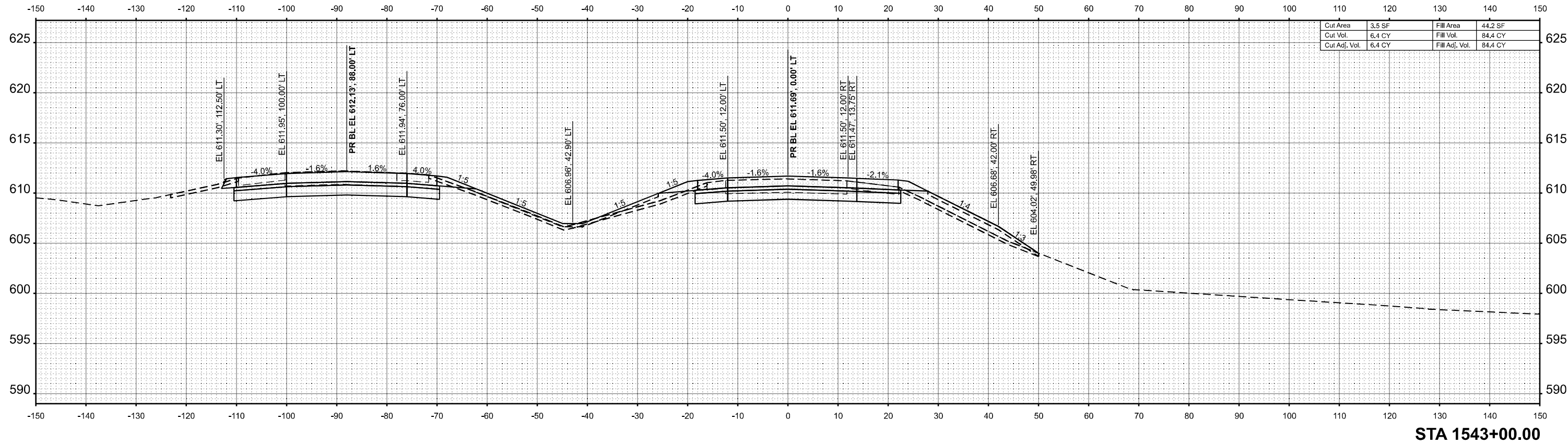
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10'

SHEET 49 OF 90 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	493
CONTRACT NO. 74893				
ILLINOIS		FED. AID PROJECT		



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PLOT DATE = 10/14/2025

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DRAWN - AJ
CHECKED - MJO
DATE - 10/16/2025

REVISED -
REVISED -
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REVISED -

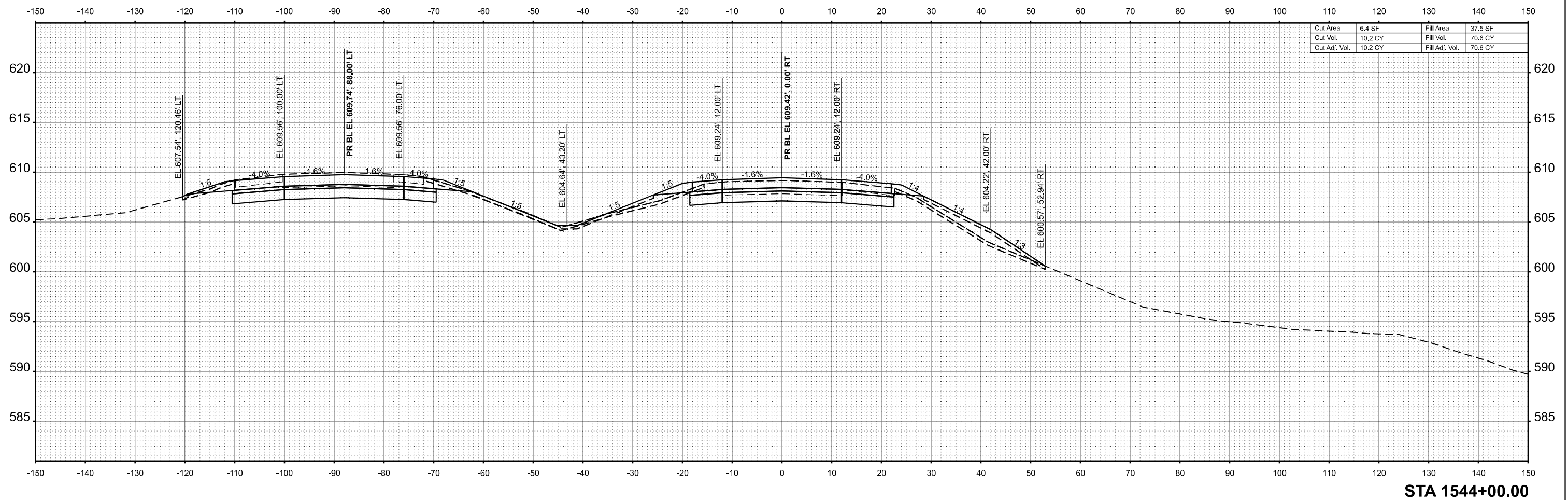
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FAI ROUTE 70 - INTERSTATE 70
CROSS SECTIONS - I-70 EASTBOUND

SCALE: 1"=10'

SHEET 53 OF 90 SHEETS STA. TO STA.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
70	(25-1)R-BR	EFFINGHAM	747	497
CONTRACT NO. 74893				
ILLINOIS		FED. AID PROJECT		



STA 1544+00.00



USER NAME	= moverbey
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