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Structural Geotechnical Report

IL Rte. 145 over Bay Creek (FAP Rte 132)

Section 103 (B-2)

Pope County, Illinois

PTB190-035

IDOT Job Number D-99-028-19

Existing Structure 076-0024

Proposed Structure 076-0033

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Millennia Project Number MG19034.09

August 17, 2021

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50 D D D D R MM D	10
51 Dr d d	10
52 Dr d d	14
53 r d d d r	21
60 R R R R R R	25
61 r r d R	25
62 r d	25
63 r d r r	25
64 Dr d r	25
65 Dr d r	26
66 r d d	26
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**Structure Geotechnical Report
IL Route 145 over Bay Creek
FAP Route 132, Section 103(B-2)
Existing Structure 076-0024
Proposed Structure 076-0033
Pope County, Illinois**

1.0 Project Description and Proposed Structure Information

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1.1 Introduction

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This report provides information on the geotechnical investigation of IL Route 145 over Bay Creek. The bridge spans approximately 23 feet across the creek. The existing bridge was built in 1979 and has a total length of 325 feet. The proposed bridge will have a total length of 325 feet and will be located approximately 10 feet upstream from the existing bridge. The investigation was conducted in March 2021.

1.2 Project Description

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The project involves the replacement of the existing bridge (076-0024) with a new bridge (076-0033). The new bridge will be a concrete girder bridge with a total length of 325 feet. The new bridge will be located approximately 10 feet upstream from the existing bridge. The investigation was conducted in March 2021.

1.3 Proposed Structure Information

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The proposed bridge (076-0033) will have a total length of 325 feet. The bridge will be a concrete girder bridge with a total length of 325 feet. The bridge will be located approximately 10 feet upstream from the existing bridge. The investigation was conducted in March 2021.

2.0 Subsurface Exploration and Laboratory Testing

2.1 Subsurface Exploration

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2.2 Laboratory Testing

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3.0 Subsurface Conditions

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3.1 Geology

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M d r r r r r r r r r r r r r r r r r r
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3.2 Generalized Subsurface Profile

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2021 Study

2021 Study: Mr. M. 20157, R. 0128

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□□□r□□□□□328□□329□□d□□□□□□15□□185□□□□□r□□□□□320□□5□□324□□□□□
□□□□□□□□r□□□□□□□□r□□0□□M□□r□□□□□□r□□□□r□□35□□132□□r□□□□R□□□□
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dr r d d r 109 r
229 50 r 3 r r 75 50 r 0
r

2019 Study:

24 42 R d r M 0
5 R d r 01
15 r r

1979 Study:

1979 d 332 d 2021 d 2019 d

3.3 Groundwater

4.0 Geotechnical Evaluations

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4.1 Earthwork and Slope Stability

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Soil parameters used for earthwork and slope stability analysis were determined from laboratory tests and field observations. Millennia's experience in the area was used to develop the soil parameters. The results of the laboratory tests are shown in Table 7.8. The results of the field tests are shown in Table 7.9.

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Soil parameters used for earthwork and slope stability analysis were determined from laboratory investigations, along with Millennia's experience in the area, and are shown on the following pages.

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Soil parameters used for earthwork and slope stability analysis were determined from laboratory tests and field observations. The results of the laboratory tests are shown in Table 7.8. The results of the field tests are shown in Table 7.9.

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Summary of Global Stability Results

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Analysis Location Section	Assumed Conditions	Minimum Computed Factor of Safety		
		Short Term	Long Term	Seismic
15' SLOPE	3:1 SLOPES	2.10	1.60	1.00
15' SLOPE	3:1 SLOPES	2.10	1.70	1.00
15' SLOPE	1:1 SLOPES	1.90	1.50	1.00
15' SLOPE	1:1 SLOPES	2.10	1.50	1.20

□

The following table summarizes the minimum computed factor of safety for the various sections analyzed. The factors of safety are based on the short term analysis. The long term analysis factors of safety are also provided for comparison. The seismic factor of safety is also provided for comparison.

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The following table summarizes the minimum computed factor of safety for the various sections analyzed. The factors of safety are based on the short term analysis. The long term analysis factors of safety are also provided for comparison. The seismic factor of safety is also provided for comparison.

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4.2 Settlement

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7 8 1980 The original structure's TS&L
7 M M M

4.3 Mining Activity

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4.4 Seismicity

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Seismic Bridge Design has been performed for the site. The IDOT Spreadsheet "Seismic Site Determination" was used to determine a Soil Site Class and a Seismic Category. The results are summarized below:

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3010 d 270 400 3120 299.5 r 3 d 4 r
d d 2019 r d r d r
33.5 48.5 318.9 3014 d 31 36 319.1 314.1 r 1
d 2 r d r d r d r d r 58.5
291.6 r 2 d r r 60 2019 d 2021
r r d r d r d r d r D r d r d r
r d r d r d r d r d r
d d d d d r d r
r r r r r d r d 1979 r d d d
r d r d r r d r d r r 326 d
308 r 1 1979 r r r r r d r
d r d r d r r r r d r
d r r r r d r r r 310.0 304.0
r 2 1979 M d r r r r d r r d r
319 294.5 r 3 1979

4.5 Scour

Summary of Design Scour Elevations

Event/Limit State	Design Scour Elevations (ft.)					Item 113
	□□□□□	□□r1□	□□r2□	□□r3□	□□□□□	
□100□	3432□	3316□	3199□	3289□	3432□	
□200□	3432□	3313□	3194□	3286□	3432□	
D□□□□□	3432□	3316□	3199□	3289□	3432□	
□□□□□	3432□	3313□	3194□	3286□	3432□	5□

5.0 Foundation Evaluations and Design Recommendations

5.1 Drilled Shaft Foundations

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R□d□□□□□□□□r□□□□□□□□d□□d□□r□□□□□□□□d□□□□□□□□d□□r□□□□□□□□□□□□□□□□□□□□□□□□
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Unfactored Side Resistance and Tip Resistance Values for Boring 1-S (2019) North Abutment Station 898+26.00

Elevation (ft)	Material Classification	Nominal Unit Side Resistance (psf)	Nominal Tip* Resistance (psf)	Nominal Liquefaction Downdrag Unit Values (psf)**
350 □ 341 □	□ □ □ □ □ □ □	□ □	□ □	230 □
341 □ 336 □ 5 □	□ □ □ □ □ □ □	385 □	□ □	360 □
336 □ 5 □ 334 □	□ □ □ □	55 □	□ □	55 □
334 □ 331 □ 5 □	□ □ □ □ □ □ □ r □ □ □ □ □	500 □	□ □	400 □
331 □ 5 □ 326 □ 5 □	□ □ □ □ □ □ □ r □ □ □ □	220 □	□ □	400 □
326 □ 5 □ 324 □	□ □ □ □	660 □	□ □	1200 □
324 □ 321 □ 5 □	□ □ □ □ □ □ □ r □ □ □ □	385 □	□ □	700 □
321 □ 5 □ 314 □	□ □ □ □ □ □ □ □ □ □ □ □ □ □ □	190 □	□ □	350 □
314 □ 296 □ 5 □	□ □ d □ □ □ □ r □ □ □	1310 □	□ □	200 □
296 □ 5 □ 276 □ 5 □	□ □ d □ □ □ □ r □ □ □	2000 □	36000 □	□ □
276 □ 5 □ 255 □	□ □ d □ □ □ □ r □ □ □	1970 □	36000 □	□ □

5 **d** **r** **r** **r** **r** **r** **r**

r **d** **r** **d** **r** **d**

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Unfactored Side Resistance and Tip Resistance Values for Boring 2-S (2019) South Abutment Station 895+06.00

Elevation (ft)	Material Classification	Nominal Unit Side Resistance (psf)	Nominal Tip* Resistance (psf)	Nominal Liquefaction Downdrag Unit Values (psf)**
350 344	□□□□	□□	□□	130□
344 336 5	□□□□□□□□□□	275□	□□	445□
336 5 334	□□□□	110□	□□	200□
334 324	□□□□□□□□r□□□□□□	430□	□□	670□
324 321 5	□□□□	385□	□□	690□
321 5 316 5	□□□□	220□	□□	400□
316 5 311 5	□□d□	730□	□□	200□
311 5 298	□□d□□□□□□□	2025□	36000□	□□
298 291 5	□□d□□□□□□□	1600□	28800□	□□
291 5 260	□□d□	1925□	28800□	□□

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Unfactored Side Resistance and Tip Resistance Values for Boring 4-S (2021) Pier 1 Station 895+76.00 □

Elevation (ft)	Material Classification	Nominal Unit Side Resistance (psf)	Nominal Tip* Resistance (psf)	Nominal Liquefaction Downdrag Unit Values (psf)**
339 329 □	□□□□□□□□□□□□□	275 □	□□	350 □
329 324 □	□□□□□□□□□r□□□□□	140 □	□□	350 □
324 312 □	□□□□□□□□□□□□□□□	165 □	□□	480 □
312 300 □	□□d □	1400 □	□□	300 □
312 249 5 □	□□d □	1900 □	24000 □	□□

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Unfactored Side Resistance and Tip Resistance Values for Boring 3-S (2021) Pier 2 Station 896+66.00 and Pier 3 Station 897+56.00

Elevation (ft)	Material Classification	Nominal Unit Side Resistance (psf)	Nominal Tip* Resistance (psf)	Nominal Liquefaction Downdrag Unit Values (psf)**
338 330 □	□ □□□□□ □□□□□□□ □	550 □	□□	310 □
330 320 □	□ □□□□□□□□ r □□□□□□□	140 □	□□	320 □
320 309 □	□ □□□□□□□□ □□□□□□□□□□□d □□□□□	165 □	□□	320 □
309 301 □	□□□d □	1050 □	□□	100 □
301 278 □	□□□d □□	2000 □	□□	□
278 229 5 □	□□□d □□	1830 □	24000 □	□□

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Resistance Factors

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5.2 Driven Pile Foundations

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spreadsheet “Pile Capacity and Length Estimates.” Steel H
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r d d d d d d r r r r r r
r r d d d d d r r r r r d
rr d d d d d r r r r D r
d r d d r r r r r dr r r d

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Estimated Pile Length Table – North Abutment (Boring 1-S 2019)
(Estimated Pile Cutoff Elevation: 345.18)

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls	201□	110□	49□
	375□□	206□	90□□
MS 14" w/0.25" walls	254□	139□	49□
	451□□	248□	90□□
MS 14" w/0.312" walls	252□	139□	49□
	451□□	248□	90□□
MS 16" w/0.312" walls	312□	171□	49□
	531□□	292□	90□□
MS 16" w/0.375" walls	312□	171□	49□
	531□□	292□	90□□
8 36□	92□□	50□	90□□
10 42□	116□□	64□	90□□
10 57□	119□□	66□	90□□
12 53□	146□□	80□	90□□
12 63□	147□□	81□	90□□
12 74□	149□□	82□	90□□
12 84□	152□□	83□	90□□
14 73□	180□□	99□	90□□
14 89□	183□□	100□	90□□
14 102□	185□□	102□	90□□
14 117□	187□□	103□	90□□

*= maximum nominal bearing and maximum estimated pile length below existing boring depth

Estimated Pile Length Table – South Abutment (Boring 2-S 2019)
(Estimated Pile Cutoff Elevation: 345.18)

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls □	213□	117□	49□
	392□	213□	82□
MS 14" w/0.25" walls □	259□	143□	49□
	459□	251□	80□
MS 14" w/0.312" walls □	259□	143□	49□
	479□□	263□	84□□
MS 16" w/0.312" walls □	308□	169□	49□
	564□	310□	84□□
MS 16" w/0.375" walls □	308□	170□	49□
	564□□	310□	84□□
□□8□36□	93□□	51□	84□□
□□10□42□	119□□	66□	84□□
□□10□57□	122□□	67□	84□□
□□12□53□	150□□	82□	84□□
□□12□63□	152□□	83□	84□□
□□12□74□	154□□	84□	84□□
□□12□84□	156□□	86□	84□□
□□14□73□	186□□	102□	84□□
□□14□89□	189□□	104□	84□□
□□14□102□	191□□	105□	84□□
□□14□117□	194□□	107□	84□□

*= maximum nominal bearing and maximum estimated pile length below existing boring depth

Estimated Pile Length Table – Pier 1 (Boring 4-S 2021)
(Pile Cutoff Elevation: 336.3)

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls □	230 □	125 □	47 □
	363 □□	198 □	86 □□
MS 14" w/0.25" walls □	289 □	157 □	47 □
	435 □□	237 □	91 □□
MS 14" w/0.312" walls □	289 □	157 □	47 □
	445 □□	237 □	86 □□
MS 16" w/0.312" walls □	353 □	192 □	47 □
	510 □□	279 □	86 □□
MS 16" w/0.375" walls □	353 □	192 □	47 □
	510 □□	279 □	86 □□
□□8□36□	75 □□	40 □	86 □□
□□10□42□	96 □□	52 □	86 □□
□□10□57□	98 □□	53 □	86 □□
□□12□53□	120 □□	65 □	86 □□
□□12□63□	121 □□	65 □	86 □□
□□12□74□	123 □□	66 □	86 □□
□□12□84□	125 □□	67 □	86 □□
□□14□73□	148 □□	80 □□	86 □□
□□14□89□	150 □□	81 □□	86 □□
□□14□102□	153 □□	82 □	86 □□
□□14□117□	155 □□	84 □	86 □□

*= maximum nominal bearing and maximum estimated pile length below existing boring depth

Estimated Pile Length Table – Pier 2 (Boring 3-S 2021)
(Pile Cutoff Elevation: 327.0)

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls □	267□	143□	50□
	392□	211□	79□
MS 14" w/0.25" walls □	325□	174□	50□
	459□	246□	76□
MS 14" w/0.312" walls □	325□	174□	50□
	570□	275□	89□
MS 16" w/0.312" walls □	387□	207□	50□
	654□	323□	89□
MS 16" w/0.375" walls □	387□	207□	50□
	782□	394□	95□
□□8□36□	270□□	146□	102□□
□□10□42□	335□	181□	102□
□□10□57□	454□□	250□	103□□
□□12□53□	418□□	227□	102□
□□12□63□	497□□	273□	103□□
□□12□74□	589□□	324□	103□□
□□12□84□	664□□	365□	103□□
□□14□73□	578□□	318□	103□□
□□14□89□	705□□	388□	103□□
□□14□102□	810□□	445□	103□□
□□14□117□	929□□	511□	103□□

*= maximum nominal bearing and maximum estimated pile length below existing boring depth

Estimated Pile Length Table – Pier 3 (Boring 3-S 2021)
(Pile Cutoff Elevation: 333.3)

Pile Type and Size	Nominal Required Bearing (kips)	Factored Resistance Available (kips)	Estimated Pile Length (ft.)
MS 12" w/0.25" walls	320□	174□	50□
	392□	208□	80□
MS 14" w/0.25" walls	406□	220□	50□
	459□	231□	79□
MS 14" w/0.312" walls	406□	220□	50□
	570□	286□	94□
MS 16" w/0.312" walls	500□	272□	50□
	654□	333□	94□
M 16" w/0.375" walls	500□	272□	50□
	782□	409□	101□
8 36□	286□	156□	107□
10 42□	335□	183□	106□
10 57□	454□□	250□	108□□
12 53□	418□	228□	106□
12 63□	497□	271□	107□
12 74□	589□□	324□	108□□
12 84□	664□□	365□	108□□
14 73□	578□	316□	107□
14 89□	705□□	388□	108□□
14 102□	810□□	445□	108□□
14 117□	929□□	511□	108□□

*= maximum nominal bearing and maximum estimated pile length below existing boring depth

5.3 Lateral Load Capacity Considerations

Recommended Design Values for Deep Foundations

Parameters for Use in LPILE Analysis at Boring 1-S (2019)

North Abutment at Station 898+26.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
350	□□□□□□□□□□ □r□□□□□r□	125□ □	1000□	0.009□	□□□	350□
341	□□□□□□□□□ □r□□□□□r□	58□	700□	0.010□	□□□	100□
336.5	□□□□□□ M□□□□□□□	58□	100□	0.020□	□□□	30□
334	□□□□□□□□□ □r□□□□□r□	58□	900□	0.009□	□□□	350□
331.5	□□□□□□ M□□□□□□□	53□	400□	0.020□	□□□	30□
326.5	□□□□□□□□□ □r□□□□□r□	53□	750□	0.010□	□□□	100□
321.5	□□□□□□ M□□□□□□□	53□	350□	0.020□	□□□	30□
314	□□d□R□□□□	58□□	□□□□	□□□□	31□	41□
296.5	□□d□R□□□□	58□□	□□□□	□□□□	34□	72□
276.5	□□d□R□□□□	58□□	□□□□	□□□□	33□	60□

Parameters for Use in LPILE Analysis at Boring 2-S (2019) South Abutment at Station 895+06.00 □

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
350 344 0	□□□□□□□□□□□□ □r□□□□□□r□	125 □ □	900 □	0.009 □	□□□	350 □
344 336 5 0	□□□□□□□□□□ □r□□□□□□r□	58 □	600 □	0.010 □	□□□	100 □
336 5 334 0	□□□□□□□□ M□□□□□□□□	58 □	200 □	0.020 □	□□□	30 □
334 324 0	□□□□□□□□□ □r□□□□□□r□	58 □	780 □	0.010 □	□□□	100 □
324 321 5 0	□□□□□□□□□ □r□□□□□□r□	58 □	700 □	0.010 □	□□□	100 □
321 5 316 5 0	□□□□□□□□ M□□□□□□□□	53 □	400 □	0.020 □	□□□	30 □
316 5 311 5 0	□□□d□R□□□□	58 □□	□□□	□□□	28 □	13 □
311 5 298 0	□□□d□R□□□□	58 □□	□□□	□□□	36 □	97 □
298 291 5 0	□□□d□R□□□□	58 □□	□□□	□□□	32 □	51 □
291 5 260 0 0	□□□d□R□□□□	58 □□	□□□	□□□	33 □	60 □

Parameters for Use in LPILE Analysis at Boring 4-S (2021)

Pier 1 at Station 895+76.00 □

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
339 329 □	□□□□□□□□ M□□□□□□□	58 □□	500 □□	0.020 □□	□□□□	30 □□
329 324 □	□□□□□□□□ M□□□□□□□	58 □□	400 □□	0.020 □□	□□□□	30 □□
324 312 □	□□□□□□□□ M□□□□□□□	58 □□	300 □□	0.012 □□	□□□□	75 □□
312 300 □	□□d□R□□□□□	58 □□	□□□□	□□□□	30 □□	13 □□
300 249 5 □	□□d□R□□□□□	58 □□	□□□□	□□□□	33 □□	60 □□

dr dr dr

Parameters for Use in LPILE Analysis at Boring 3-S (2021)

Pier 2 Station 896+66.00 and Pier 3 at Station 897+56.00

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
338@330□	□□□□□□□□□□ □r□□□□□r□	58□□ □	1000□	0.009□	□□□□	350□
330@320□	□□□□□□□□ □M□□□□□□□	58□□	250□	0.020□	□□□□	30□
320@309□	□□□□□□□□□□ □r□□□□□r□	58□□	300□	0.010□	□□□□	100□
309@301□	□□d□R□□□□□	58□□	□□□□	□□□□	31□	41□
301@278□	□□d□R□□□□□	58□□	□□□□	□□□□	34□	72□
278@229.5□	□□d□R□□□□□	58□□	□□□□	□□□□	33□	60□
229.5@224.5□	□□□□□□□□□□ □r□□□□□r□	73□□	5000□	0.004□	□□□□	1500□

□□□□□ d □ r □□□□□□□
□□□□□□□ d □ r □□□□□□□

1

1

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 1-S (2019)

North Abutment at Station 898+26.00 □

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
318.9 301.4	□□□□□□□□□ M□□□□□□□□	58□□	240□□	0.02□□	□□□□	30□□

1

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 2-S (2019) South Abutment at Station 895+06.00 □

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
319□ 314□ □	□□□□□□□□□ M□□□□□□□□	58□□	160□□	0.02□□	□□□□	30□□

1

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 4-S (2021) Pier 1 at Station 895+76.00 □

Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
312 299.5 0	□□□□□□□□□□ M□□□□□□□□	58 □□	300 □□	0.02 □□	□□□□	30 □□

Parameters for Use in LPILE Analysis of Liquefiable Layers at Boring 3-S (2021)

Pier 2 Station 896+66.00 and Pier 3 at Station 897+56.00 □

Pile 2 Station 301 (left) and Pile 3 at Station 301 (right)						
Elevation (ft)	LPILE Soil Type	Effective Unit Weight (pcf)	Undrained Cohesion (psf)	Strain at 50% Maximum Stress	Angle of Internal Friction (degrees)	p-y Soil Modulus K _{static} (pci)
316@301@	□□□□□□□□□ M□□□□□□□	58□□	100□□	0@2□□	□□□□	30□□

1

6.0 Construction Considerations

□

6.1 Temporary Sheeting and Soil Retention

□

Temporary sheeting will be installed around the perimeter of the excavation. Sheet piles will be driven to a minimum depth of 15' 10" below grade. The sheet piles will be driven in pairs, staggered vertically, and connected by horizontal tie rods. The sheet piles will be driven to a minimum depth of 15' 10" below grade. The sheet piles will be driven in pairs, staggered vertically, and connected by horizontal tie rods.

□

Excavation 145 will be retained using a combination of sheet piles and soil nails. The sheet piles will be driven to a minimum depth of 31' 6" below grade. The sheet piles will be driven in pairs, staggered vertically, and connected by horizontal tie rods.

6.2 Cofferdam

A cofferdam will be constructed around Excavation 3370. The cofferdam will be built using sheet piles and soil nails. The sheet piles will be driven to a minimum depth of 28' 1" below grade. The sheet piles will be driven in pairs, staggered vertically, and connected by horizontal tie rods.

6.3 Subgrade Water Protection

□

Drilled wells will be installed around the perimeter of the excavation. The drilled wells will be installed at a minimum depth of 22' below grade. The drilled wells will be installed in pairs, staggered vertically, and connected by horizontal tie rods.

6.4 Drilled Shaft Construction

□

Drilled shafts will be installed around the perimeter of the excavation. The drilled shafts will be installed at a minimum depth of 516'. The drilled shafts will be installed in pairs, staggered vertically, and connected by horizontal tie rods. The drilled shafts will be installed at a minimum depth of 333' 3" below grade. The drilled shafts will be installed in pairs, staggered vertically, and connected by horizontal tie rods.

rr
d
d
Tr rr
rr rr rr rr rr rr rr rr rr rr rr rr rr rr rr
rr rr rr rr rr rr rr rr rr rr rr rr rr rr rr
r r r r r r

□

d
d
d
d d

□

rr
rr rr rr rr rr rr rr rr rr rr rr rr rr rr rr
rr rr rr rr rr rr rr rr rr rr rr rr rr rr rr
rr rr rr rr rr rr rr rr rr rr rr rr rr rr rr
rr rr rr rr rr rr rr rr rr rr rr rr rr rr rr

6.5 Driven Pile Installation

dr d d d d r r r r r r r d d
d rd r r 512 D d rd d
2012 M r d dr r r r
rd 512 15 R r d d d d d
d d d d r d r r r r r r r r r r
dr

6.6 Subgrade, Fill, and Backfill

7.0 Closing

□

D

□

M

□

M

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M

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Millennia Professional Services of Illinois, Ltd.

□

□

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□

□

□

□



MILLENNIA PROFESSIONAL SERVICES

6439 Plymouth Avenue, Suite W-129, St. Louis, Missouri 63133 314-531-3981

□

Appendix A:

Figure 1: Vicinity Map

Figure 2: TS&L Plan

Figure 3: Subsurface Profile

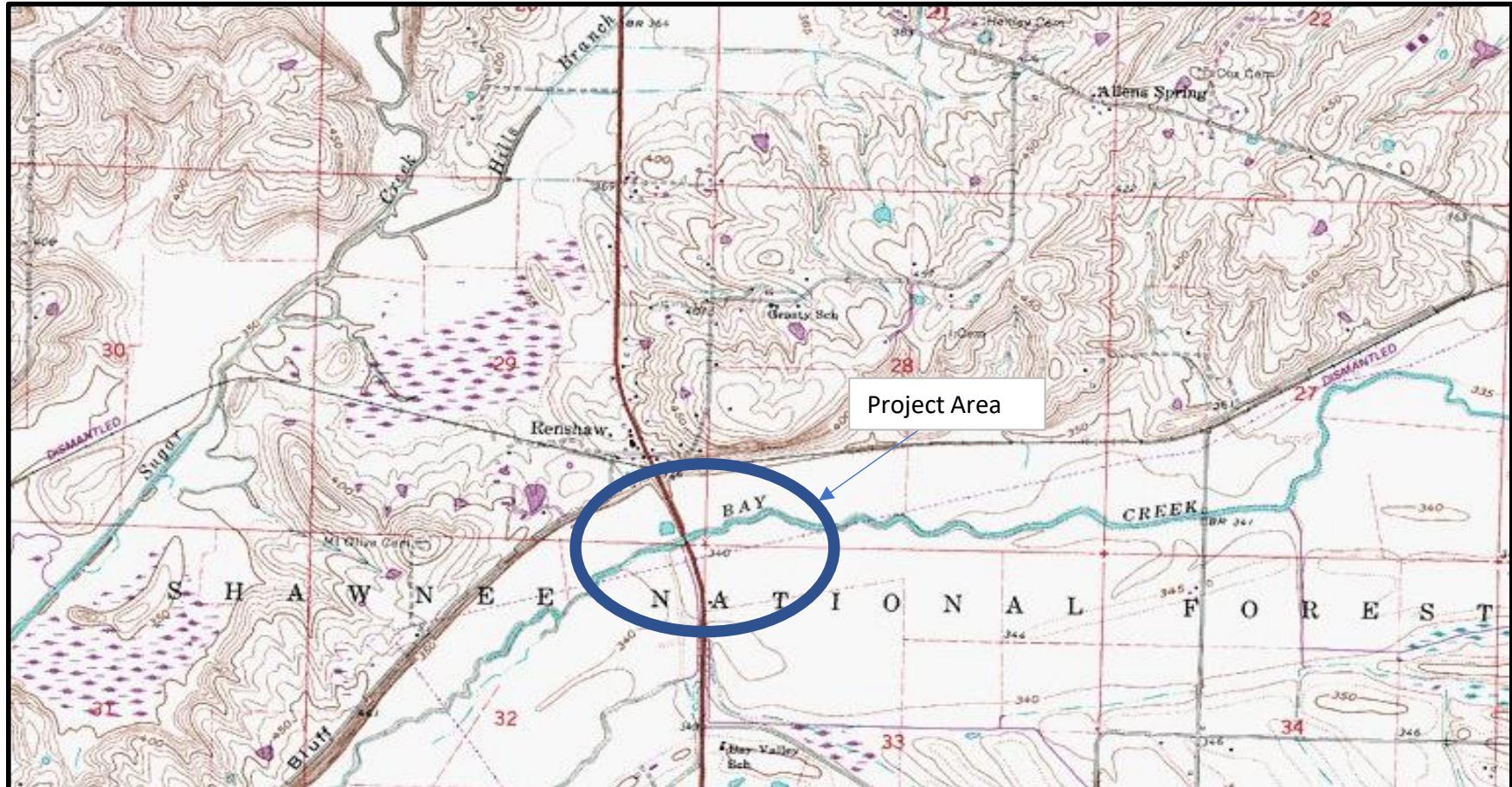
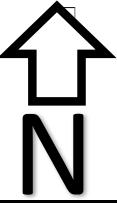
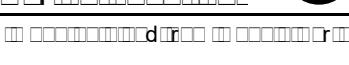
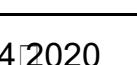
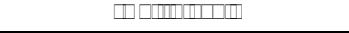
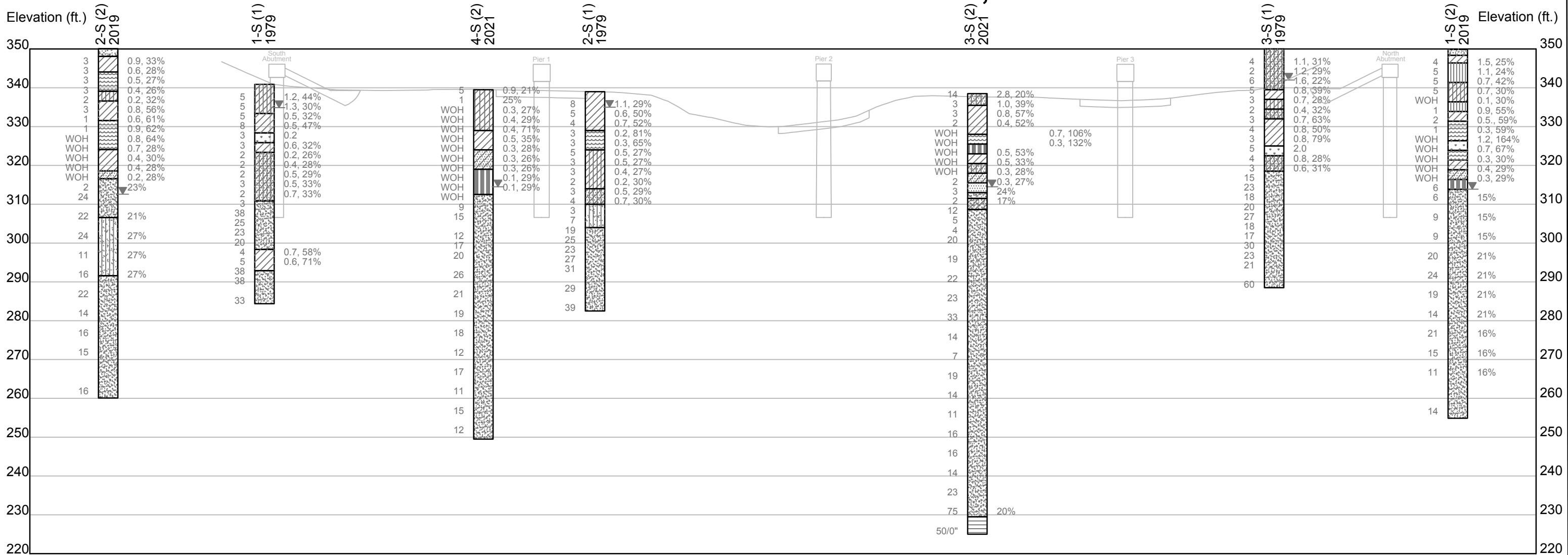


FIGURE 1: VICINITY MAP

 N	145 r r r r 			
 	Drill rig			
 	Drill rig			
	Drill rig	M 19034 09	D 000	8 14 2020

STRUCTURE NO. 076-0033, -0024



Note: Elevations are approximate. Actual conditions between borings are unknown, and are subject to change. Drilled shaft depths shown are only for reference.

STRATIGRAPHY

	CLAY		LOAM		SHALE
	CLAY LOAM		PEAT		SILT
	CLAY WITH ORGANICS		SAND		SILTY CLAY
	CONCRETE		SANDY CLAY		SILTY CLAY LOAM
	SANDY LOAM		SILTY LOAM		

BORING DATA KEY:

Boring ID	Rimac Value (tsf), Moisture Content (%)
	▼ Groundwater level encountered at boring

Vertical Scale: Approx. 1" = 25'
Horizontal Scale: IGNORE

FIGURE 3:
SUBSURFACE PROFILE

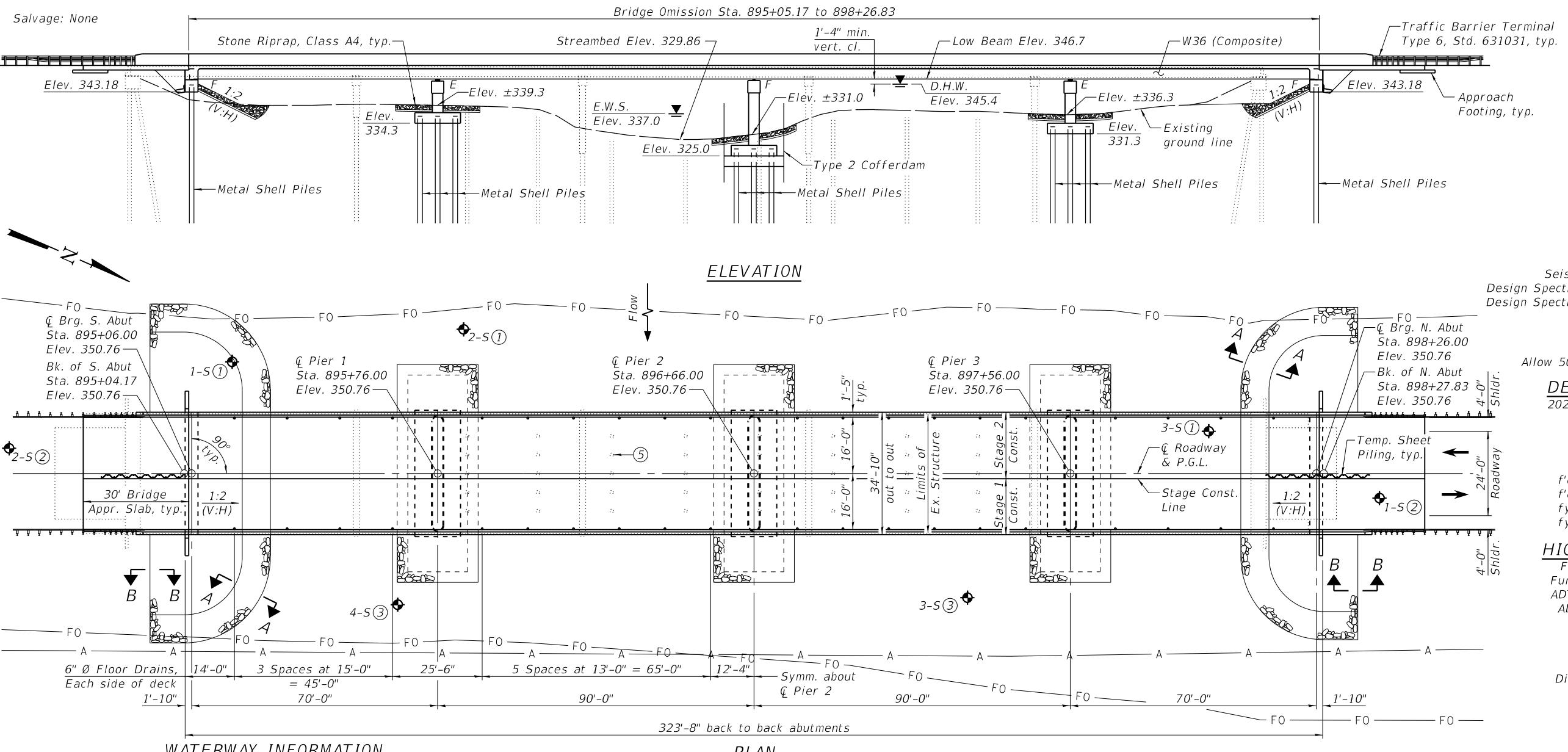
PROJECT NAME:	PROJECT No.:	DRAWN BY:	CHECKED BY:	
Work Order No. 9 IL 145 over Bay Creek	MG19034.09	B. FISHER	J. SCHAEFFER	
		08/05/2021	08/05/2021	

Bench Mark: BM 804 - Chiseled square on northeast wingwall, Sta. 898+11, 17' RT., Elev. 350.05.

Existing Structure: S.N. 076-0024 was originally built in 1983 as F.A. Route 132, Section 103A-B. The back to back abutment length is 324'-7" and the out to out deck width is 34'-0". The existing structure consists of a five span 27" PPC deck beam superstructure supported by concrete stub abutments founded on concrete piles and concrete pile bent piers founded on concrete piles. Structure is to be removed and replaced.

Traffic Control: One lane of traffic will be maintained by utilizing staged construction.

Salvage: None



0.00%
PVT Sta. 894+70.00
Ellev. 350.76
PVT Sta. 894+35.00
Ellev. 350.76

PROFILE GRADE (along Q roadway)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 4
Design Spectral Acceleration at 1.0 sec. (SD1) = 0.701g
Design Spectral Acceleration at 0.2 sec. (SDS) = 0.918g
Soil Site Class = E

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design
Specifications, 9th Edition

DESIGN STRESSES

FIELD UNITS

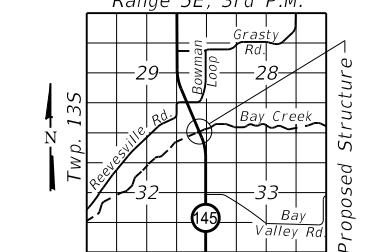
f'_c = 3,500 psi
 f'_c = 4,000 psi (Superstructure concrete)
 f_y = 60,000 psi (Reinforcement)
 f_y = 50,000 psi (M270 Grade 50)

HIGHWAY CLASSIFICATION

F.A.P. Rte. 132 - IL Rte. 145
Functional Class: Minor Arterial
ADT: 2,050 (2018); 2,500 (2038)
ADTT: 185 (2018); 225 (2038)
DHV: 250

Design Speed: 55 m.p.h.
Posted Speed: 55 m.p.h.
Two-Way Traffic
Directional Distribution: 50:50

Range 5E, 3rd P.M.



GENERAL PLAN & ELEVATION

IL RTE. 145 OVER BAY CREEK

F.A.P. RTE. 132 - SEC. 103B-2

POPE COUNTY

STA. 896+66.00

STRUCTURE NO. 076-0033

PRELIMINARY PLANS

NOT FOR REVIEW

DESIGN SCOUR ELEVATION TABLE

Event / Limit	Design Scour Elevation (ft.)				
	S. Abut.	Pier 1	Pier 2	Pier 3	N. Abut.
Q100	343.2	331.6	319.9	328.9	343.2
Q200	343.2	331.3	319.4	328.6	343.2
Design	343.2	331.6	319.9	328.9	343.2
Check	343.2	331.3	319.4	328.6	343.2

5

Notes:

- ① 1979 Boring.
- ② 2019 Boring.
- ③ 2021 Boring.

④ For Sections A-A and B-B, and Riprap Protection at Piers, see sheet 2 of 2.

⑤ Timber piles from 1959 bridge construction, typical.



OATES
ASSOCIATES

www.oatesassociates.com

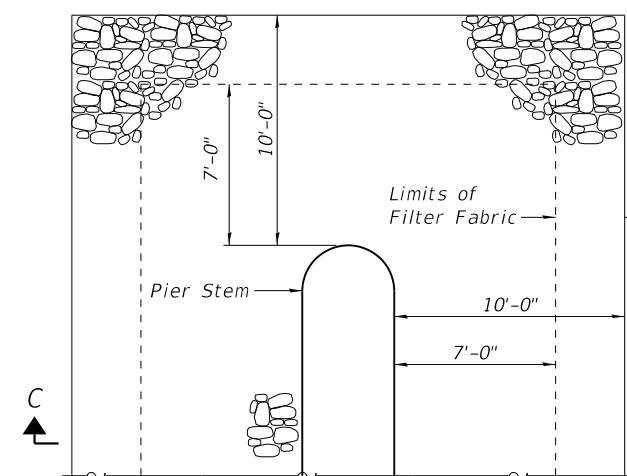
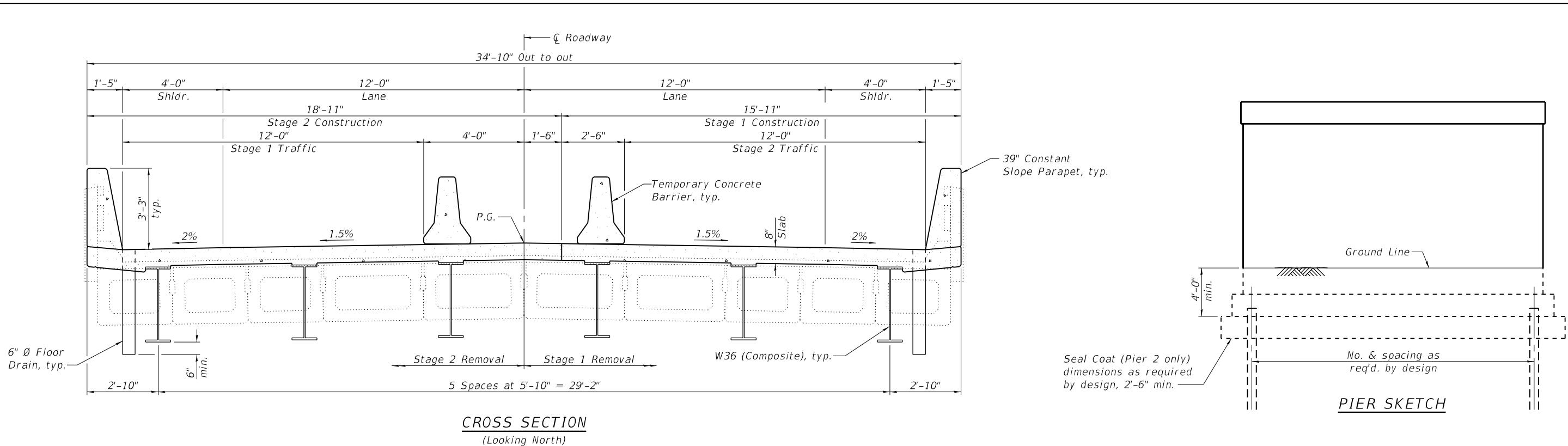
USER NAME = DESIGNED - JAD
CHECKED - ETH
PLOT SCALE = DRAWN - JAD
PLOT DATE = 8/16/2021
CHECKED - ETH
REVISED -

REVISED -
REVISED -
REVISED -
REVISED -

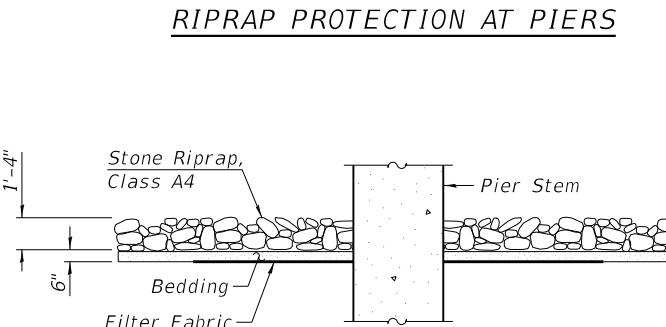
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SHEET 1 OF 2 SHEETS

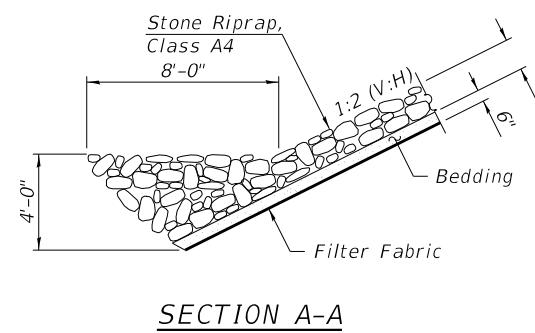
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
132	103B-2	POPE	1	CONTRACT NO. 78719



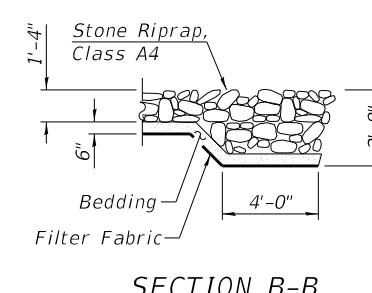
RIPRAP PROTECTION AT PIERs



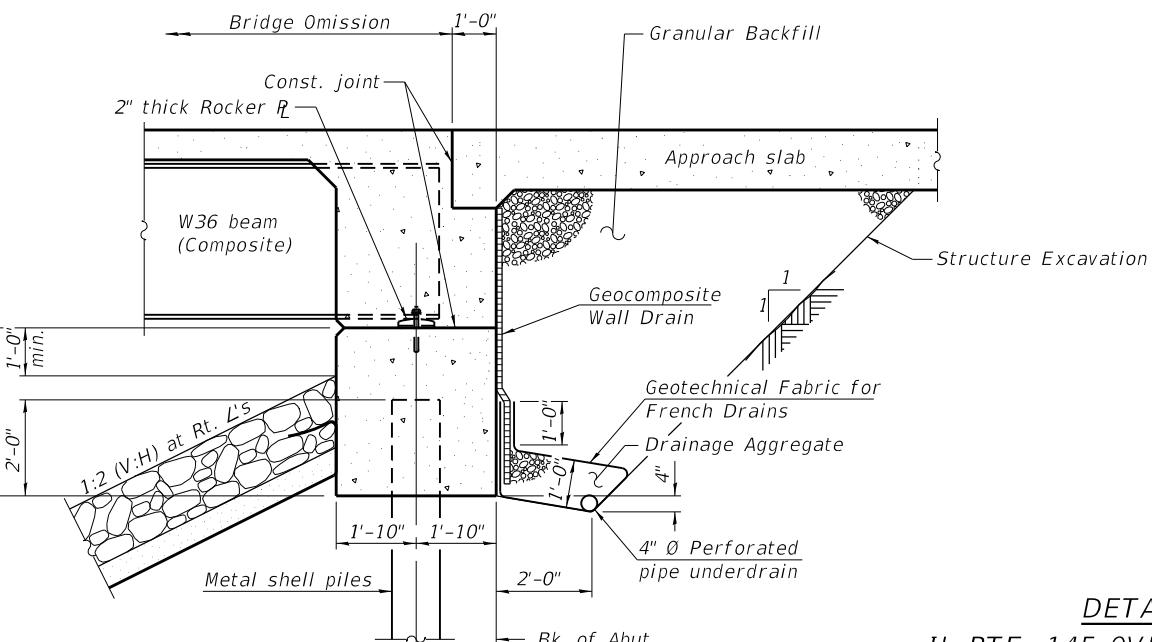
SECTION C-C



SECTION A-



SECTION B-B



SECTION THRU INTEGRAL ABUTMENT

DETAILS

POPE COUNTY

ATA. 896+66.00

STRUCTURE NO. 076-0033



MILLENNIA PROFESSIONAL SERVICES Boring Log

11 Executive Drive, Suite 12

Fairview Heights, Illinois 62208

618-624-8610

Appendix B: **Boring Logs**



3-S
SOIL BORING LOG

Sheet 1 of 3

COUNTY	Pope
SECTION	103(B-2)
ROUTE	132
MPS PROJECT NO.	076-0024
DATE	6/30/2021

DESCRIPTION	IL-145 over Bay Creek	DISTRICT	9								
LOCATION	2.3 miles South of IL 146 (North Abut.)	CONSULTANT	Millennia Professional Services								
DRILLED BY	Geotechnology	LOGGED BY	P. Adhikari								
DRILLING METHOD	Hollow Stem Auger	HAMMER TYPE	Automatic								
BORING NO.	3-S	RIG TYPE	CME 550X								
Station	897+25	EFFICIENCY	75%								
Offset	35 ft RT										
Northing											
Easting											
Ground Surface Elev.	338.5 ft										
LITHOLOGY	(ft)	(ft)	(/6")	(tsf)	(%)	LITHOLOGY	(ft)	(ft)	(/6")	(tsf)	(%)
Topsoil (2.0")	338.30					Gray, soft, moist, SILTY CLAY LOAM (continued)	318.00				
FILL: Brown, very stiff, SILTY CLAY LOAM		2				Gray, soft, moist, CLAY LOAM		WOH			
		7	2.8	20				WOH			
	335.50	7	S					WOH	0.3	28	
Gray, soft to medium-stiff, moist, CLAY		WOH				Gray, soft, moist, LOAM - Atterberg Limits: LL = 23, PI = 7	315.50				
- trace organics below 6.5 ft. - Atterberg Limits: LL = 95, PI = 61		1	1.0	39				WOH	1	0.3	27
		-5	S					1	1	B	
		2									
		WOH				Gray, loose, SAND, fine- to medium-grained	313.00				
		1	0.8	57				2			
		-10	S								
		1				Gray, soft, SANDY CLAY, trace organics	311.50				
		1	0.4	52				1			
		-10	S			- Atterberg Limits: LL = 23, PI = 8		WOH			
		1						2			
		1	0.7	106							
Black, medium-stiff, moist, ORGANIC CLAY	328.00	WOH	S			Gray, loose to medium-dense, SAND, medium- to coarse-grained, with fine gravel - 4.1% passing No. 200 sieve, 15.0% retained on No. 4 sieve	308.70				
		WOH						5			
		1						5			
		1	0.3	132				7			
		-15	W								
Brownish gray, soft, moist, ORGANIC SILTY LOAM	325.50	WOH	P								
		WOH									
		1									
		1	0.5	53							
Gray, soft, moist, CLAY, trace organics	323.00	WOH	B								
		WOH									
		1									
		2									
Gray, soft, moist, SILTY CLAY LOAM	320.50	WOH	B								
		WOH									
		4									
- Atterberg Limits: LL = 41, PI = 20		WOH	0.5	33				8			
		WOH	B					12			
		-20									

The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.



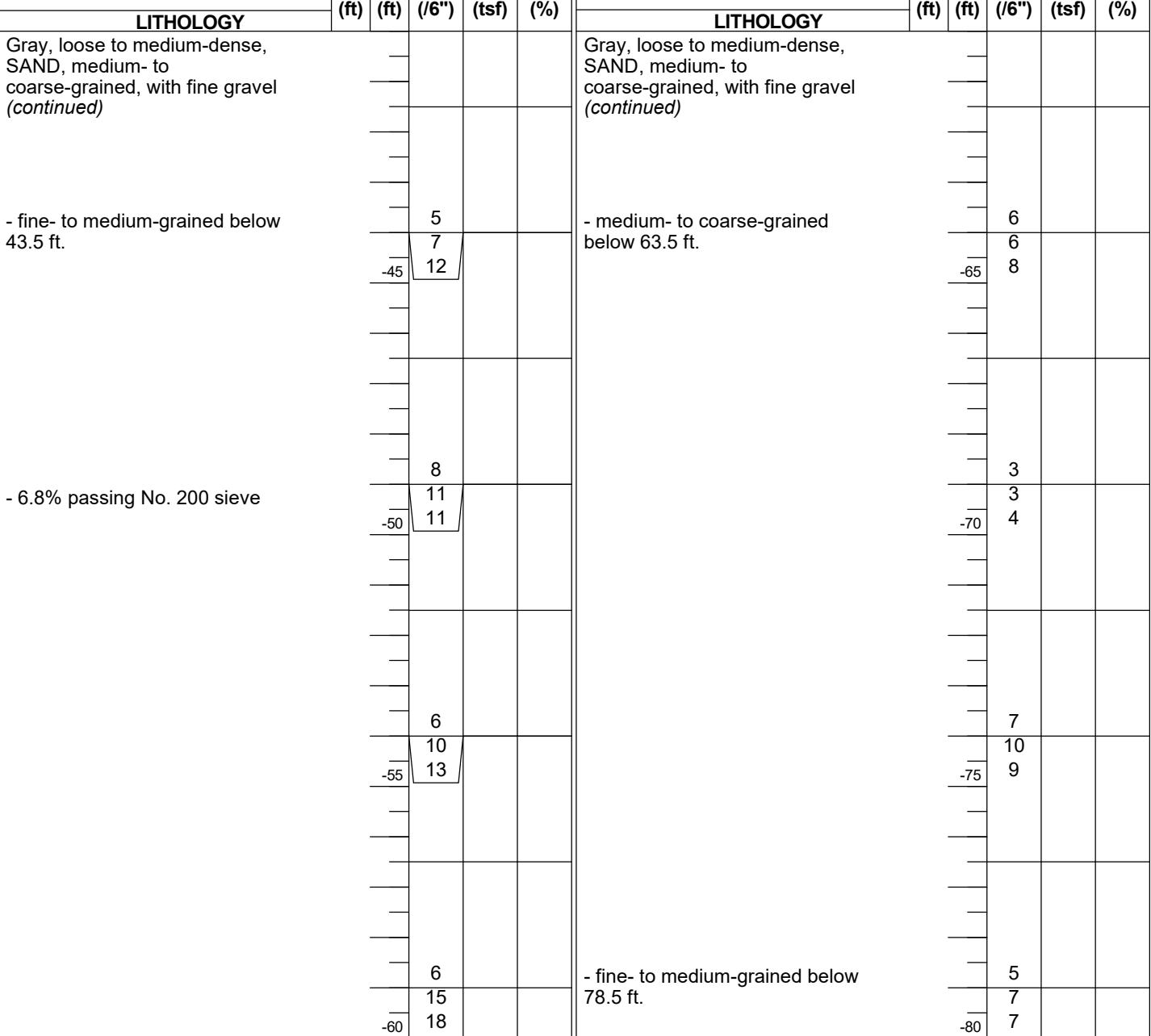
3-S
SOIL BORING LOG

Sheet 2 of 3

COUNTY	Pope
SECTION	103(B-2)
ROUTE	132
MPS PROJECT NO.	076-0024
DATE	6/30/2021

DESCRIPTION	IL-145 over Bay Creek	DISTRICT	9
LOCATION	2.3 miles South of IL 146 (North Abut.)	CONSULTANT	Millennia Professional Services
DRILLED BY	Geotechnology	LOGGED BY	P. Adhikari
DRILLING METHOD	Hollow Stem Auger	HAMMER TYPE	Automatic
		RIG TYPE	CME 550X
		EFFICIENCY	75%

BORING NO.	3-S	E	D	B	U	M	Surface Water Elev.	ft	E	D	B	U	M
Station	897+25	L	E	L	C	O	Stream Bed Elev.	ft	L	E	L	C	O
Offset	35 ft RT	E	P	O	S	I	Groundwater Elev.:	ft	E	P	O	S	I
Northing		V	T	W	Qu	S	First Encounter	314.5 ft	V	T	W	Qu	S
Easting		H	S				Upon Completion	ft	H	S			
Ground Surface Elev.	338.5 ft	(ft)	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(ft)	(/6")	(tsf)	(%)



The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.



3-S
SOIL BORING LOG

Sheet 3 of 3

COUNTY	Pope
SECTION	103(B-2)
ROUTE	132
MPS PROJECT NO.	076-0024
DATE	6/30/2021

DESCRIPTION	IL-145 over Bay Creek	DISTRICT	9
LOCATION	2.3 miles South of IL 146 (North Abut.)	CONSULTANT	Millennia Professional Services
DRILLED BY	Geotechnology	LOGGED BY	P. Adhikari
DRILLING METHOD	Hollow Stem Auger	HAMMER TYPE	Automatic

BORING NO.	3-S	E	D	B	U	M	Surface Water Elev.	ft	E	D	B	U	M
Station	897+25	L	E	L	C	O	Stream Bed Elev.	ft	L	E	L	C	O
Offset	35 ft	E	P	O	S	I	Groundwater Elev.:	ft	E	P	O	S	I
Northing		V	T	W	S	T	First Encounter	314.5 ft	V	T	W	S	I
Easting		H	S	Qu			Upon Completion	ft	H	S	Qu		
Ground Surface Elev.	338.5 ft	(ft)	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(ft)	(/6")	(tsf)	(%)

LITHOLOGY							LITHOLOGY						
Gray, loose to medium-dense, SAND, medium- to coarse-grained, with fine gravel (continued)							Gray, loose to medium-dense, SAND, medium- to coarse-grained, with fine gravel (continued)						
- medium- to coarse-grained below 83.5 ft.			6						4				
			6						9				
			-85	5					-105	14			
- with gravel below 88.5 ft.							Gray SHALE, weathered						
			7					229.50	32				
			7						25				
			9						50				
			-90										
			11					225.00					
			9						50/0"				
			7										
			-95										
			6										
			7										
			-100										

The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.



4-S
SOIL BORING LOG

Sheet 1 of 3

COUNTY	Pope
SECTION	103(B-2)
ROUTE	132
MPS PROJECT NO.	076-0024
DATE	6/29/2021

DESCRIPTION	IL-145 over Bay Creek	DISTRICT	9
LOCATION	2.3 miles South of IL 146 (South Abut.)	CONSULTANT	Millennia Professional Services
DRILLED BY	Geotechnology	LOGGED BY	P. Adhikari
DRILLING METHOD	Hollow Stem Auger	HAMMER TYPE	Automatic
BORING NO.	4-S	RIG TYPE	CME 550X
Station	895+65	EFFICIENCY	75%
Offset	37 ft RT		
Northing			
Easting			
Ground Surface Elev.	339.5 ft		
LITHOLOGY		LITHOLOGY	
Topsoil (2.0")	339.25	Gray, very soft to soft, moist, SILT LOAM	319.00
Brown, soft to medium-stiff, SILTY CLAY	2	WOH	WOH
	3	WOH	WOH
	2	S	0.3
- gray, soft, moist below 3.5 ft.			26
	WOH		WOH
	WOH		WOH
	1	25	B
	WOH		WOH
	WOH		WOH
	0.3	27	0.1
- trace organics below 8.5 ft.			29
	WOH		WOH
	WOH		WOH
	1	25	B
	WOH		WOH
	WOH		WOH
	0.3	27	0.1
	WOH		WOH
	WOH		WOH
	0.3	27	B
Gray, soft, moist, CLAY, trace organics	329.00	Gray, very loose to medium-dense, SAND, fine- to medium-grained, trace gravel	312.50
	WOH	WOH	WOH
	WOH		WOH
	0.4	29	0.1
	WOH		WOH
	WOH		WOH
	0.4	29	B
- with gravel below 31.0 ft.			
- 4.4% passing No. 200 sieve,			
- 24.7% retained on No. 4 sieve			
- trace gravel below 33.5 ft.			
	WOH		WOH
	WOH		WOH
	0.4	71	3
	WOH		4
	WOH		5
	WOH		3
	WOH		6
	0.5	35	9
	WOH		
	WOH		
	0.5	35	
Gray, soft, moist, CLAY LOAM, trace organics	324.00	- fine- to coarse-grained below 38.5 ft.	6
	WOH		6
	WOH		6
	0.3	26	
	WOH		
	WOH		
	0.3	26	

The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.



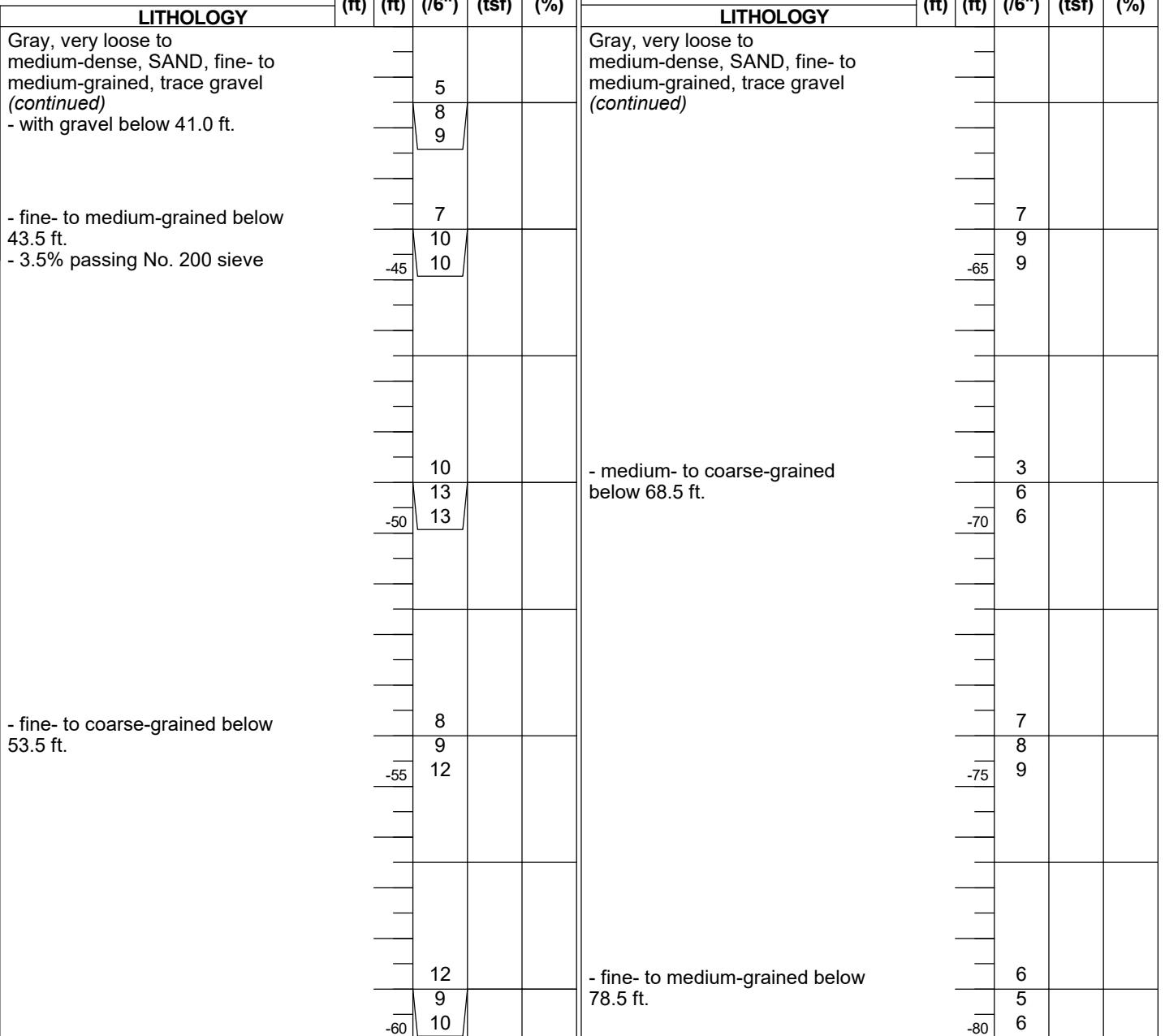
4-S
SOIL BORING LOG

Sheet 2 of 3

COUNTY	Pope
SECTION	103(B-2)
ROUTE	132
MPS PROJECT NO.	076-0024
DATE	6/29/2021

DESCRIPTION	IL-145 over Bay Creek	DISTRICT	9
LOCATION	2.3 miles South of IL 146 (South Abut.)	CONSULTANT	Millennia Professional Services
DRILLED BY	Geotechnology	LOGGED BY	P. Adhikari
DRILLING METHOD	Hollow Stem Auger	HAMMER TYPE	Automatic
		RIG TYPE	CME 550X
		EFFICIENCY	75%

BORING NO.	4-S	E	D	B	U	M	Surface Water Elev.	ft	E	D	B	U	M
Station	895+65	L	E	L	C	O	Stream Bed Elev.	ft	L	E	L	C	O
Offset	37 ft RT	E	P	O	S	I	Groundwater Elev.:	ft	E	P	O	S	I
Northing		V	T	W	Qu	T	First Encounter	314.5 ft	V	T	W	Qu	T
Easting		H	S				Upon Completion	ft	H	S			
Ground Surface Elev.	339.5 ft	(ft)	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(ft)	(/6")	(tsf)	(%)



The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.



MILLENNIA
PROFESSIONAL SERVICES
www.millennia-pro.com

4-S SOIL BORING LOG

Sheet 3 of 3

COUNTY	Pope
SECTION	103(B-2)
ROUTE	132
MPS PROJECT NO.	076-0024
DATE	6/29/2021

DESCRIPTION	IL-145 over Bay Creek	DISTRICT	9
LOCATION	2.3 miles South of IL 146 (South Abut.)	CONSULTANT	Millenia Professional Services
DRILLED BY	Geotechnology	LOGGED BY	P. Adhikari
DRILLING METHOD	Hollow Stem Auger	HAMMER TYPE	Automatic
		RIG TYPE	CME 550X
		EFFICIENCY	75%

BORING NO.	4-S	E	D	B	U	M	Surface Water Elev.	ft	E	D	B	U	M
Station	895+65	L	E	L	C	O	Stream Bed Elev.	ft	L	E	L	C	O
Offset	37 ft RT	E	P	O	S	I	Groundwater Elev.:	ft	E	P	O	S	I
Northing		V	T	W	S	S	First Encounter	314.5 ft	V	T	W	S	I
Easting		H	S	Qu	T		Upon Completion	ft	H	S	Qu	T	ST
Ground Surface Elev.	339.5 ft						After Hrs.	ft					

The Unconfined Compressive Strength (UCS) Qu column represents either the IDOT Rimac or AASHTO T 208 Test Procedure. The Qu failure mode is indicated by B for Bulge or S for Shear. P is a Pocket Penetrometer test. The Standard Penetration Test (SPT) N value is the sum of the second and third Blows /6 in. values in each sample using AASHTO T 206.



Illinois Department of Transportation

Memorandum

To: Carrie Nelsen Attn: Dave Piche
From: Keith Roberts By: Aaron Hayes
Subject: Boring Logs
Date: November 22, 2019

**IL 145 over Bay Creek
Structure 076-0024 (Ex.)
Section 103(B-2)
Pope County**

Foundation boring logs have been obtained for the above listed structure and are attached. Please note the borings were completed on two separate occasions.

Rock was not encountered in the two borings completed for this project. The borings were drilled to a depth that will achieve a minimum nominal driven bearing capacity for a 14 in. diameter metal shell pile of 435 kips at Boring 1-S and 500 kips at Boring 2-S. Boring 1-S, near the north abutment, shows a layer of potentially liquefiable soils at a depth of 33.5 to 53.5 feet. Boring 2-S, near the south abutment, shows layers of potentially liquefiable soils at depths of 33.5 to 43.5 feet and 53.5 to 58.5 feet. A liquefaction analysis should be completed once the proposed structure's final dimensions are determined.

Borings completed in 1979 before the existing structure was constructed have been attached for additional subsurface information. Also attached is a General Plan and Elevation sheet from the 1979 plans showing the old boring locations.

Attachments
AWH:ah

cc: Materials Geotechnical Unit\Boring Logs using gINT\State Structures\Pope



Illinois Department of Transportation

Division of Highways
District 9

SOIL BORING LOG

Page 2 of 3

Date 5/30/19

ROUTE IL 145 **DESCRIPTION** Structure over Bay Creek **LOGGED BY** L. Estel

SECTION 103(B-2) **LOCATION** 2.3 miles South of IL 146 (North Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope **DRILLING METHOD** Hollow stem auger (8" O.D., 3.25" I.D.) **HAMMER TYPE** Auto SPT 140 lb

STRUCT. NO. 076-0024
Station 896+49

D E P T H	B L O W S	U C S W S	M O I S T	Surface Water Elev. 333.6 ft Stream Bed Elev. 330.1 ft Groundwater Elev.: <input checked="" type="checkbox"/> First Encounter 313.9 ft <input checked="" type="checkbox"/> Upon Completion _____ ft <input checked="" type="checkbox"/> After Hrs. _____ ft	D E P T H	B L O W S	U C S W S	M O I S T
(ft)	(tsf)	(%)			(ft)		(tsf)	(%)

BORING NO. 1-S
Station 898+44
Offset 7.0ft Rt
Ground Surface Elev. 349.9

Loose Grey, Moist m. and c.
SAND with f. GRAVEL
(non-plastic)
40% f. GRAVEL and c. SAND,
58% m. SAND, 1% SILT, 1%
CLAY (Lab 43) (*continued*)

Soil Profile Log (continued)

Depth (m)	Lab 43 Description	Lab 43 Properties	Lab 44 Description	Lab 44 Properties
296.40	M. Dense Grey, Moist m. and c. SAND with f. GRAVEL (non-plastic) 14% c. SAND and f. GRAVEL, 82% m. SAND, 3% SILT, 1% CLAY (Lab 43) (<i>continued</i>)		M. Dense Grey, Moist m. and c. SAND with f. GRAVEL (non-plastic) 14% c. SAND and f. GRAVEL, 82% m. SAND, 3% SILT, 1% CLAY (Lab 44) (<i>continued</i>)	
294.40				
292.40				
290.40				
288.40				
286.40				
284.40				
282.40				
280.40				
278.40				
276.40				
274.40				
272.40				
270.40				
268.40				
266.40				
264.40				
262.40				
260.40				
258.40				
256.40				
254.40				
252.40				
250.40				
248.40				
246.40				
244.40				
242.40				
240.40				
238.40				
236.40				
234.40				
232.40				
230.40				
228.40				
226.40				
224.40				
222.40				
220.40				
218.40				
216.40				
214.40				
212.40				
210.40				
208.40				
206.40				
204.40				
202.40				
200.40				
198.40				
196.40				
194.40				
192.40				
190.40				
188.40				
186.40				
184.40				
182.40				
180.40				
178.40				
176.40				
174.40				
172.40				
170.40				
168.40				
166.40				
164.40				
162.40				
160.40				
158.40				
156.40				
154.40				
152.40				
150.40				
148.40				
146.40				
144.40				
142.40				
140.40				
138.40				
136.40				
134.40				
132.40				
130.40				
128.40				
126.40				
124.40				
122.40				
120.40				
118.40				
116.40				
114.40				
112.40				
110.40				
108.40				
106.40				
104.40				
102.40				
100.40				
98.40				
96.40				
94.40				
92.40				
90.40				
88.40				
86.40				
84.40				
82.40				
80.40				
78.40				
76.40				
74.40				
72.40				
70.40				
68.40				
66.40				
64.40				
62.40				
60.40				
58.40				
56.40				
54.40				
52.40				
50.40				
48.40				
46.40				
44.40				
42.40				
40.40				
38.40				
36.40				
34.40				
32.40				
30.40				
28.40				
26.40				
24.40				
22.40				
20.40				
18.40				
16.40				
14.40				
12.40				
10.40				
8.40				
6.40				
4.40				
2.40				
0.40				
296.40	M. Dense Grey, Moist m. and c. SAND with f. GRAVEL (non-plastic) 14% c. SAND and f. GRAVEL, 82% m. SAND, 3% SILT, 1% CLAY (Lab 43) (<i>continued</i>)	4	21	
276.40	M. Dense Grey, Moist m. and c. SAND with f. GRAVEL (non-plastic) 29% c. SAND and f. GRAVEL, 69% m. SAND, 1% SILT, 1% CLAY (Lab 45)	5	16	
296.40		8	21	
276.40		9	16	
296.40		12	16	
276.40		11	16	
296.40		13	21	
276.40		11	16	
296.40		7	21	
276.40		4	16	
296.40		11	16	
276.40		8	16	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



**Illinois Department
of Transportation**

Division of Highways
District 9

SOIL BORING LOG

Page 3 of 3

Date 5/30/19

ROUTE IL 145 DESCRIPTION Structure over Bay Creek LOGGED BY L. Estel

SECTION 103(B-2) LOCATION 2.3 miles South of IL 146 (North Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope DRILLING METHOD Hollow stem auger (8" O.D., 3.25" I.D.) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. 076-0024
Station 896+49

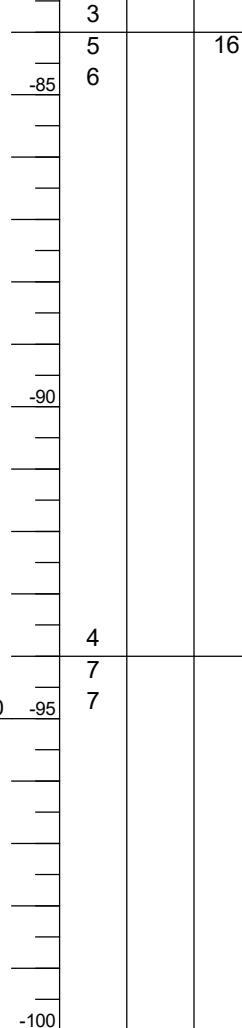
BORING NO. 1-S
Station 898+44
Offset 7.0ft Rt
Ground Surface Elev. 349.9

D	B	U	M
E	L	C	O
P	O	S	I
T	W	Qu	S
H	S		T
		(tsf)	(%)

Surface Water Elev. 333.6 ft
Stream Bed Elev. 330.1 ft

Groundwater Elev.:
 First Encounter 313.9 ft
 Upon Completion ft
 After ft Hrs.

M. Dense Grey, Moist m. and c.
SAND with f. GRAVEL
(non-plastic)
29% c. SAND and f. GRAVEL,
69% m. SAND, 1% SILT, 1%
CLAY (Lab 45) (continued)



To convert "N" values to "N60", multiply by 1.5

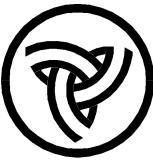
(end of boring)

Bottom of hole @ 95 ft

N.L. = Not Liquefiable

Elevation referenced to BM 804,
Chiseled Square on the NE
wingwall of SN 076-0024; EL.
350.05

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated)
Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating
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**Illinois Department
of Transportation**
Division of Highways
District 9

SOIL BORING LOG

Page 1 of 3

Date 10/18/19

ROUTE IL 145 DESCRIPTION Structure over Bay Creek LOGGED BY L. Estel

SECTION 103(B-2) LOCATION 2.3 miles South of IL 146 (South Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope DRILLING METHOD Hollow stem auger (8" O.D., 3.25" I.D.) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. 076-0024
Station 896+49

BORING NO. 2-S
Station 894+54
Offset 7.0ft Lt
Ground Surface Elev. 350.1 ft

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. <u>333.3</u> ft Stream Bed Elev. <u>330.3</u> ft	D E P T H	B L O W S	U C S Qu	M O I S T
				Groundwater Elev.: First Encounter <u>312.6</u> ft Upon Completion <u>ft</u> After <u>ft</u> Hrs.				
				N.L. PI > 11 (Est.)				
				Dark Grey				
				WOH				
				WOH <u>0.9</u>				<u>62</u>
				1 <u>B</u>				
				WOH				
				WOH <u>0.8</u>				<u>64</u>
				-25 WOH <u>B</u>				
				324.10				
				M. Stiff Grey, Moist CLAY				
				N.L. PI > 11 (Est.)				
				WOH				
				WOH <u>0.7</u>				<u>28</u>
				WOH <u>B</u>				
				321.60				
				Soft Grey, Moist CLAY				
				N.L. PI > 11 (Est.)				
				WOH				
				WOH <u>0.4</u>				<u>30</u>
				-30 WOH <u>B</u>				
				318.60				
				WOH				
				V. Soft Grey, Moist CLAY LOAM				
				N.L. PI > 11 (Est.)				
				WOH <u>0.4</u>				<u>28</u>
				WOH <u>B</u>				
				316.60				
				V. Loose Grey, Moist well graded f. SAND				
				WOH				
				WOH <u>0.2</u>				<u>28</u>
				-35 WOH <u>B</u>				
				Non-plastic, 18% c. SAND, 62% f. SAND, 16% SILT, 4% CLAY (Lab 59)				
				WOH				
				WOH <u>2</u>				<u>23</u>
				311.60				
				M. Dense (No sample)				
				(washed) <u>8</u>				
				12				
				12				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated)
 Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

Page 2 of 3

Date 10/18/19

ROUTE IL 145 DESCRIPTION Structure over Bay Creek LOGGED BY L. Estel

SECTION 103(B-2) LOCATION 2.3 miles South of IL 146 (South Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope DRILLING METHOD Hollow stem auger (8" O.D., 3.25" I.D.) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. 076-0024
Station 896+49

BORING NO. 2-S
Station 894+54
Offset 7.0ft Lt
Ground Surface Elev. 350.1 ft

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. <u>333.3</u> ft Stream Bed Elev. <u>330.3</u> ft	D E P T H	B L O W S	U C S Qu	M O I S T
				Groundwater Elev.: First Encounter <u>312.6</u> ft Upon Completion <u>ft</u> After <u>ft</u> Hrs.				

M. Dense (No sample)

(washed) (continued)

306.60
M. Dense Grey, Moist c. SANDY
LOAM with rounded f. GRAVEL
Non-plastic, 19% f. GRAVEL to c.
SAND, 19% m. SAND, 25% f.
SAND, 30% SILT, 7% CLAY (Lab 60)
(washed)

301.60
M. Dense Grey, Wet SANDY
LOAM A-6(1)
LL = 28, PI = 14, 21% c. SAND to
f. GRAVEL, 24% m. SAND, 27%
f. SAND, 16% SILT, 12% CLAY
(Lab 61)
(washed)

(washed)

291.60

5				M. Dense Grey, moist f. and m. SAND (washed) (continued)	17			
10			21		8			
-45	12				-65	14		
6				M. Dense Grey, moist m. and c. SAND with rounded f. GRAVEL and 1" COAL seam	10			
12			27		7			
-50	12				-70	7		
4				M. Dense Grey, moist m. and c. SAND with rounded GRAVEL Non-plastic, 29% c. SAND and f. GRAVEL, 69% m. SAND, 1% SILT, 1% CLAY (based on visual ID and Lab 45) (washed)	4			
8			27		8			
-55	3				-75	8		
5					5			
8			27		6			
-60	8				-80	9		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated)
Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



**Illinois Department
of Transportation**

Division of Highways
District 9

SOIL BORING LOG

Page 3 of 3

Date 10/18/19

ROUTE IL 145 DESCRIPTION Structure over Bay Creek LOGGED BY L. Estel

SECTION 103(B-2) LOCATION 2.3 miles South of IL 146 (South Abut.), SEC. 29, TWP. 13S, RNG. 5E, PM

COUNTY Pope DRILLING METHOD Hollow stem auger (8" O.D., 3.25" I.D.) HAMMER TYPE Auto SPT 140 lb

STRUCT. NO. 076-0024
Station 896+49

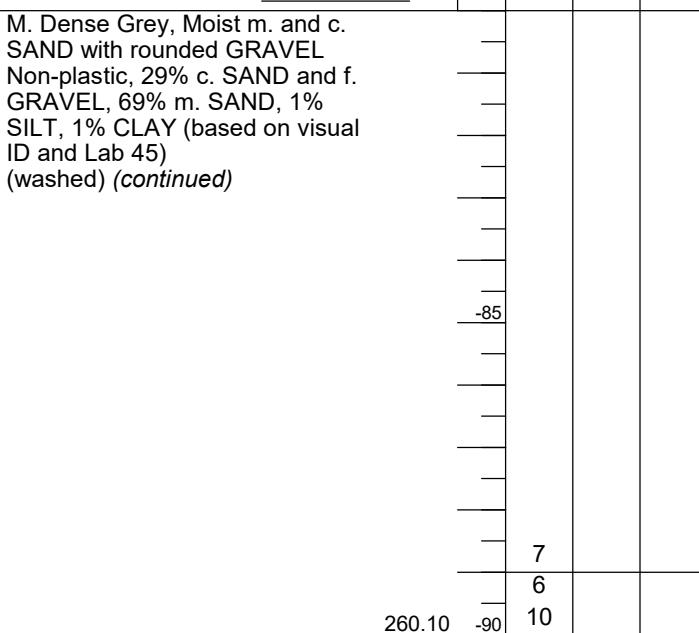
BORING NO. 2-S
Station 894+54
Offset 7.0ft Lt
Ground Surface Elev. 350.1 ft

D	B	U	M
E	L	C	O
P	O	S	I
T	W	Qu	S
H	S		T
(ft)		(tsf)	(%)

Surface Water Elev. 333.3 ft
Stream Bed Elev. 330.3 ft

Groundwater Elev.:
 First Encounter 312.6 ft
 Upon Completion ft
 After Hrs. ft

M. Dense Grey, Moist m. and c.
SAND with rounded GRAVEL
Non-plastic, 29% c. SAND and f.
GRAVEL, 69% m. SAND, 1%
SILT, 1% CLAY (based on visual
ID and Lab 45)
(washed) (continued)



Bottom of hole @ 90.0 ft

N.L. = Not Liquefiable

Elevation referenced to BM 804,
Chiseled Square on the NE
wingwall of SN 076-0024; EL.
350.05

To convert "N" values to "N60",
multiply by 1.5

B.M. "□" Cut in N end of East hubguard 10'-6" Rt Sta. 897 +37
Elev. 351.36

Existing Structure: #076-0017 Built 1934 at Sta. 896+56, Section 103 A on S. B. L Rte. 145 Seven I-bm spans with 4" Timber planks superstructure and timber pile bents substructure. Timber planks replaced with 6" concrete slab in 1959. Existing structure is 147'-3" long by 22'-0" wide Stage Construction shall be utilized so as to maintain one way traffic during reconstruction.

No Salvage

ROUTE	SECTION	COUNTY	TOTAL SITES	SHEET NO.
SA 132	103A-B	POPE	25	13
SA 1		TO STA.		
SA 132, RR NO. 4	ILLINOIS	FED AID PROJECT		

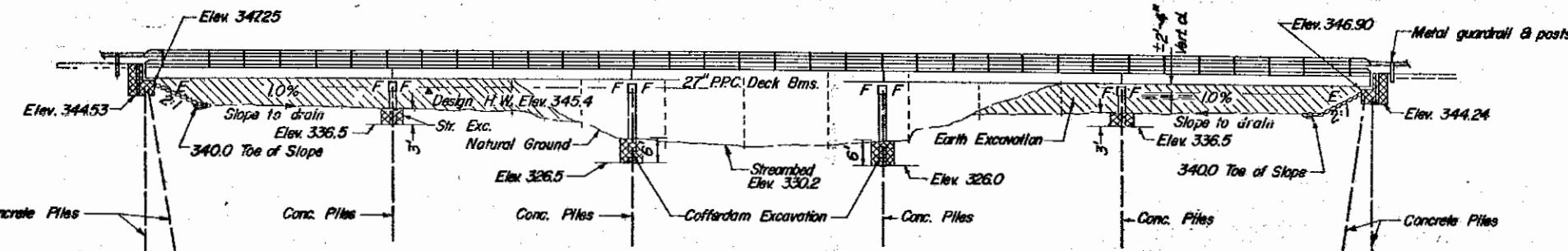
GENERAL NOTES

See Proposal for Boring Data.

The top surface of the beams shall be finished in accordance with Article 505.06 of the Standard Specification except that the surface shall not be roughened by brooming. The finished surface shall be free of depressions or high spots with sharp corners.

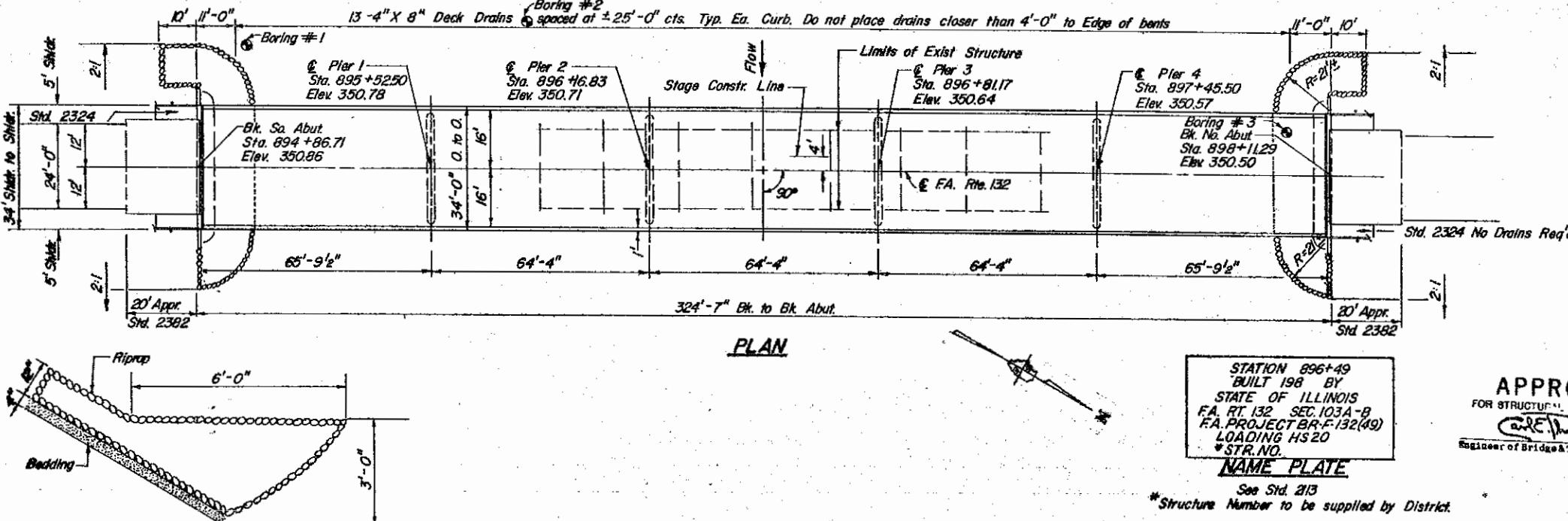
Protective Coat shall not be applied to surfaces to which **Waterproofing Membrane System** is applied.
Reinforcement bars shall conform to the requirements of AASHTO M-31.

The Contractor shall drive one precast concrete test pile in a permanent location at Pier 2 and at Pier 4 as directed by the Engineer.



ELEVATION

NOTE: Hatched Area shows Earth Excavation which is included in the Earth work balances in the road plans.



RIPRAP ANCHOR DETAIL

WATERWAY INFORMATION

Drainage Area 16466 ^a mi. Low Grade Ele ^b 349.16' at Sta. 890+00							
Flood	Freq. Yr	Q. C.F.S.	Opening Exist.	Sq. Ft. Prop.	Nat. H.W.E.	Head - Ft. Exist.	Headwater Prop.
Design	50	9900	1398	2360	345.4	2.0	0.47
Base	100	11700	1478	2528	346.0	2.5	0.60
Overtopping	—	—	—	—	—	—	—
Max. Calc.	500	15515	1627	2842	347.12	3.0	0.75

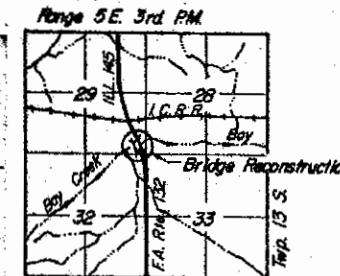
PROFILE GRADE
@ ROADWAY

<u>FIELD UNITS</u>	<u>PRECAST PRESTRESSED UNITS</u>
$f_c = 3,500 \text{ psi}$	$f_c = 5,000 \text{ psi}$
$f_y = 60,000 \text{ psi}$	$f_y = 4,000 \text{ psi}$
Reinforcement	$f_u = 270,000 \text{ psi } (\frac{1}{2}^{\text{nd}} \text{ A Shor})$
	$f_g = 189,000 \text{ psi } (\frac{1}{2}^{\text{nd}} \text{ A Shor})$

*Design Specifications 1977 AASHTO & 1978,
1979 & 1980 Interims as applicable*

Allow for 25 #/sq.ft. Future Wearing Surface

LOADING HS20-44



LOCATION SKETCH

GENERAL PLAN AND ELEVATION

A. Rte. 132 (ILL. 145) Over BAY CREEK
 F.A. Rte. 132 SECTION 103A-B
 POPE COUNTY Sta. 896+49.00

GREENE & ELLIOTT, Ltd.

CONSULTING ENGINEERS

19 STEVENSON DR. • 217/529-6681 • SPRINGFIELD, ILL.

OWN L/R CHECKED R.B.J.	REVISED	DATE OCT, 1980	PROJECT
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BRIDGE FOUNDATION BORING LOG

BRIDGE STRUCTURE CARRYING F.A.
ROUTE 132 OVER BAY CREEK

Date OCTOBER, 1979

JOHN R. HAYDEN

Checked By GARY L. PULLEY

PROJECT _____

ROUTE F.A. 132

SEC

103A-B

NTY POPE
Boring No. 1-S
Station 895+05
Offset 34' LT. CL

N - Standard Penetration Test -
Blows per foot to drive 2"

O.D. Split Spoon Sampler 12" with

140# hammer falling 30".

• 100 • WASHINGTON STATE

Qu - Unconfined Compressive Strength - t/sf

mean Water Content in percentage

$w = \text{Water Content} - \text{percentage}$
 $\text{of oven dry weight} - \%$

Type failure:

B - Bulge Failure

S - Shear Failure

E – Estimated Value

D **D**emand **D**ecreases

BRIDGE FOUNDATION BORING LOG

STRUCTURE CARRYING F.A. ROUTE 132 OVER BAY CREEK
 SEC. 103A-B
 POPE COUNTY
 BORING NO. 1-S

Elevation
 N
 Qu t/s.
 (%)

Elevation
 N
 Qu t/s.
 (%)

SEE PREVIOUS SHEET

-45
 5 0.6S 71

292.9

DENSE WET GREY COARSE GRAINED
 SAND & SOME SMALL GRAVEL

38 -

-50
 38 - -

284.4

33 - -

BOTTOM OF HOLE = 56.5'.

DURING DRILLING OPERATIONS
 IT APPEARED THAT FREE WATER
 WAS ENCOUNTERED AT 30.0'.

DURING DRILLING OPERATIONS
 WASH OUT PROCEDURES USED FROM
 29.0' TO 56.5'.

STRUCTURE LOCATION:

SE 1/4, SE 1/4, SECTION 29,
 T13S, R5E, 3RD P.M., POPE
 COUNTY, ILLINOIS.

-60
 -65
 -70

-75
 -80
 -85
 -90
 -95

BRIDGE FOUNDATION BORING LOG

PROJECT F.A. 132
 ROUTE 103A-B
 SEC. POPE
 COUNTY

Boring No. 2-S
 Station 895+83
 Offset 41' LT. CL

BRIDGE STRUCTURE CARRYING F.A.
 ROUTE 132 OVER BAY CREEK

STA. 896+46

Date OCTOBER, 1979

Bored By JOEL CONGIARDO

Checked By GARY L. PULLEY

Elevation	N	Qu/t/s.t.	(%)	Surface Water El.	NONE	Elevation	N	Qu/t/s.t.	(%)
Ground Surface	339.0	0							
STIFF MOIST TO VERY MOIST BROWN CLAY A-7-6							2	0.2B	30
						314.0	-25		
	8	1.1B	29	MEDIUM VERY MOIST GREY TO DARK GREY SILTY CLAY TO CLAY LOAM A-6 WITH SAND SEAMS		3	0.5B	29	
334.0	-5								
MEDIUM VERY MOIST GREY CLAY A-6 TO A-7-6	5	0.6B	50		310.0	4	0.7B	30	
				LOOSE TO VERY LOOSE WET GREY FINE TO COARSE GRAINED SAND WITH SILTY CLAY SEAMS	-30				
	4	0.7B	52			3	-	-	
329.0	-10								
SOFT TO VERY SOFT VERY MOIST GREY CLAY A-6 WITH ROTTEN WOOD MIXED	3	0.2B	81			7	-	-	
	3	0.3B	65	304.0	-35				
				MEDIUM WET GREY COARSE GRAINED SAND & SOME SMALL GRAVEL		19	-	-	
324.0	-15								
MEDIUM VERY MOIST GREY SILTY CLAY TO CLAY A-6 TO A-7-6	5	0.5B	27			25	-	-	
	3	0.5B	27	-40					
319.0	-20					23	-	-	
SOFT TO VERY SOFT VERY MOIST GREY SILTY CLAY A-6	3	0.4B	27						
SEE NEXT COLUMN				294.0	-45				
				SEE NEXT SHEET					

N - Standard Penetration Test -

Blows per foot to drive 2"

O.D. Split Spoon Sampler 12" with
140# hammer falling 30".Qu - Unconfined Compressive
Strength - t/sfw - Water Content - percentage
of oven dry weight - %.

Type failure:

B - Bulge Failure

S - Shear Failure

E - Estimated Value

P - Penetrometer

BRIDGE FOUNDATION BORING LOG

STRUCTURE CARRYING F.A. ROUTE SECTION 103A-B POPE COUNTY BORING NO. 2-S	138	OVER	BAY CREEK
	Elevation	N	Quat/sq. mi. (%)

BRIDGE FOUNDATION BORING LOG

PROJECT _____
ROUTE F.A. 132
SEC. 103A-B
COUNTY POPE

BRIDGE STRUCTURE CARRYING F.A.
ROUTE 132 OVER BAY CREEK
STA 896+46
Date OCTOBER, 1979
Bored By JOHN R. HAYDEN
Checked By GARY L. PULLEY

SEE NEXT COLUMN

N - Standard Penetration Test

Blows per foot to drive 2"
O. D. Split Spoon Sampler 12" with
140# hammer falling 30".

Qu - Unconfined Compressive Strength - t/sf

w - Water Content - percentage
of oven dry weight - %.

Type failure:
B - Bulge Failure
S - Shear Failure
E - Estimated Value
P - Penetrometer

BRIDGE FOUNDATION BORING LOG

STRUCTURE CARRYING F.A. ROUTE 132 SECTION 103A-B POPE COUNTY BORING NO. 3-S	Elevation	M	Qu t/s. ft	(%)	Elevation	M	Qu t/s. ft	(%)
SEE PREVIOUS SHEET	-45							
	18							
	17							
	-50							
	30							
	23							
	-55							
	21							
	-60							
	289.5							
VERY DENSE WET GREY COARSE GRAINED SAND	288.5	60						
BOTTOM OF HOLE = 61.5'.								
DURING DRILLING OPERATIONS IT APPEARED THAT FREE WATER WAS ENCOUNTERED AT 31.5'.	-65							
DURING DRILLING OPERATIONS WASH OUT PROCEDURE USED FROM 31.5' TO 60.0'.	-70							



MILLENNIA Professional Services Ltd

11 Executive Drive, Suite 12

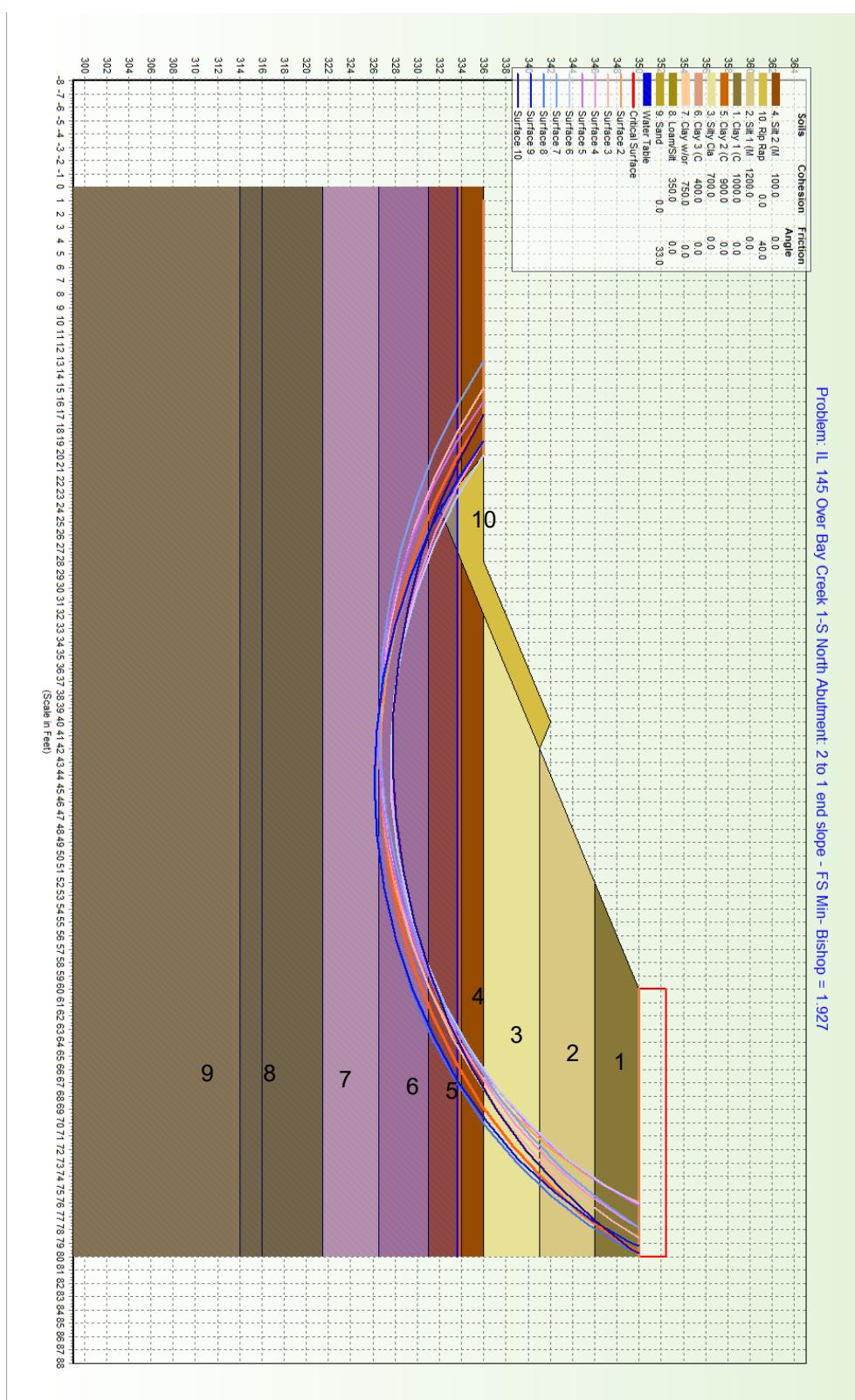
Fairview Heights, Illinois 62208

618-624-8610

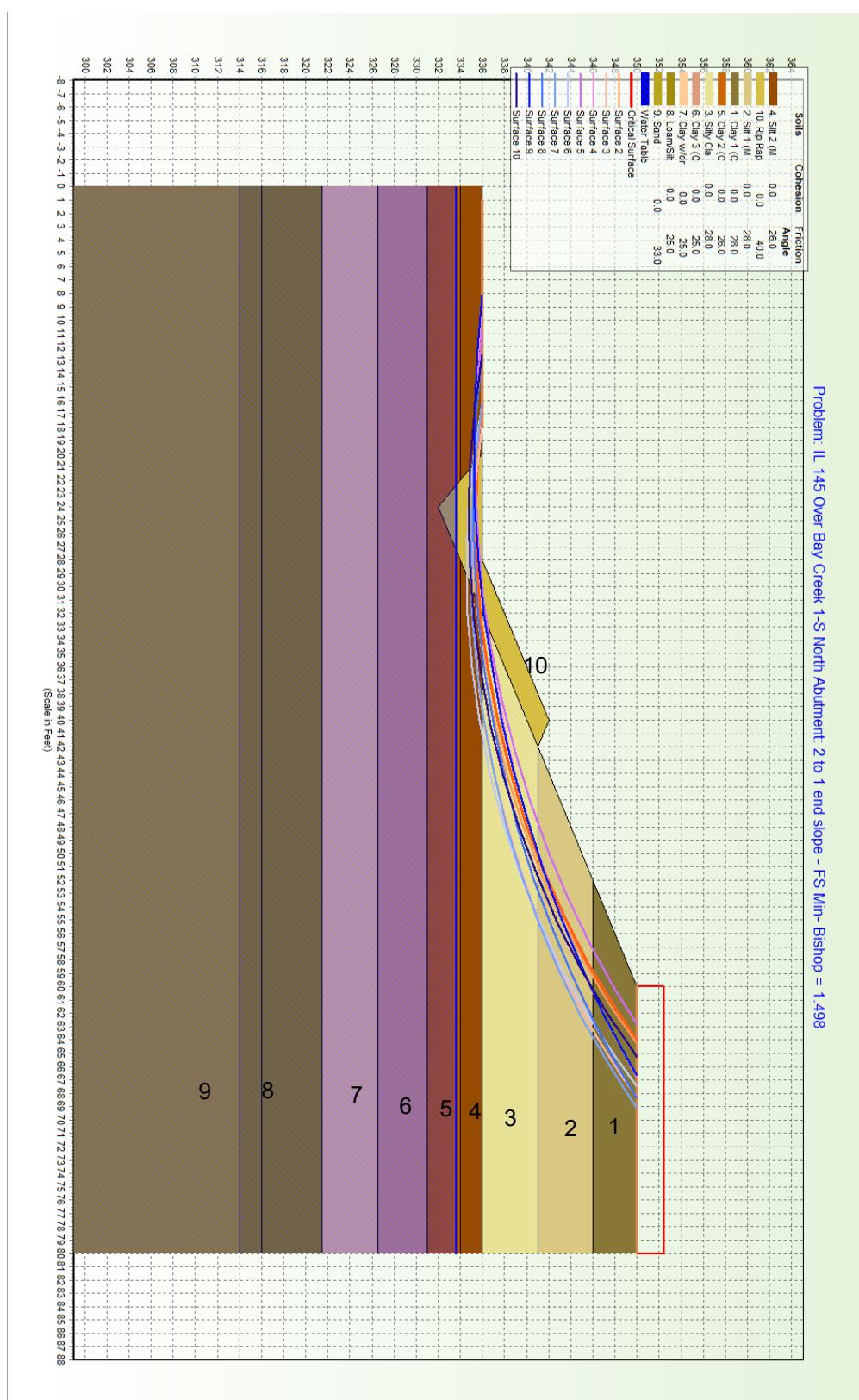
Appendix C:

Summary Stability Profiles

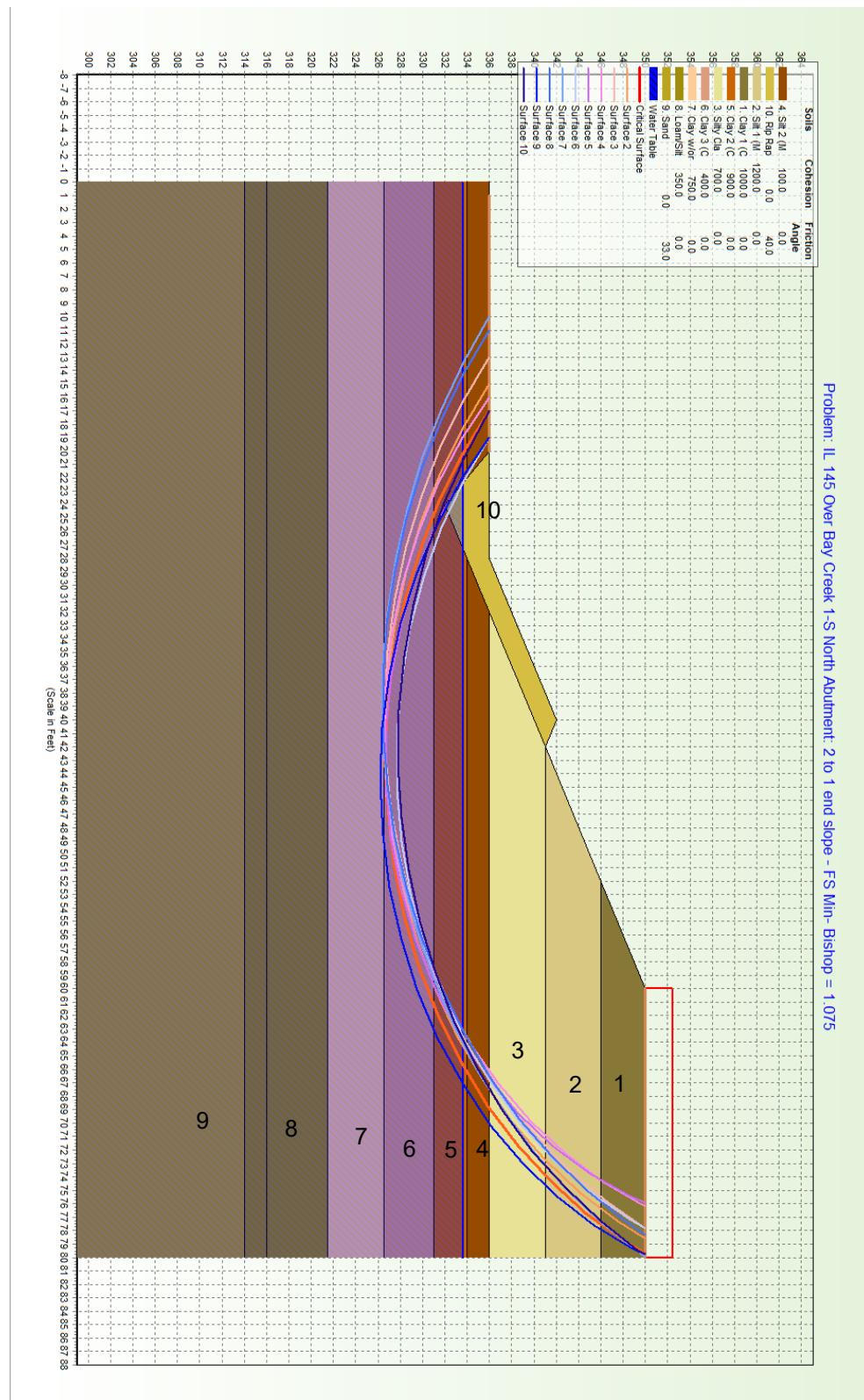
IL 145 over Bay Creek 1-S North Abutment- End Slope (undrained condition)



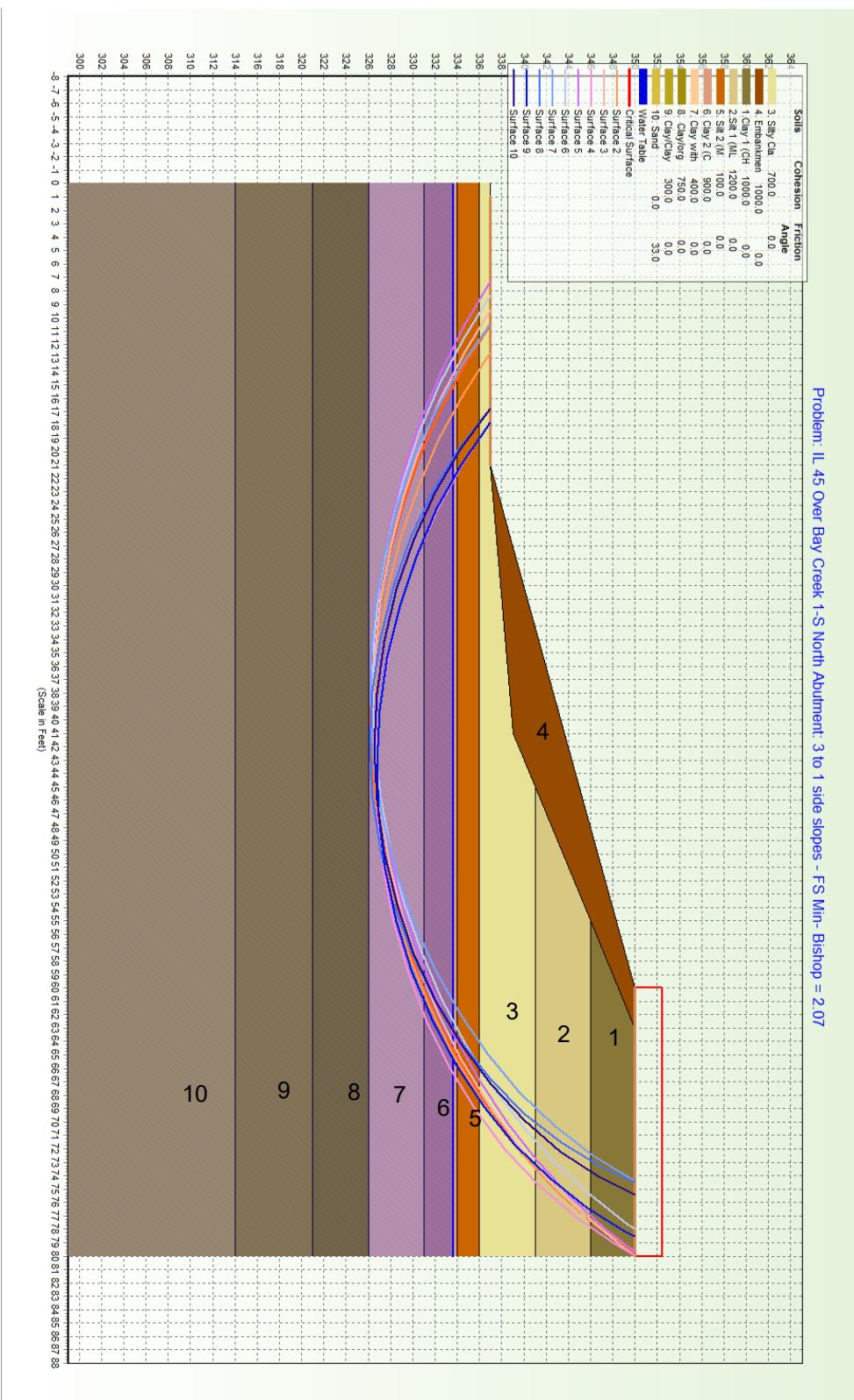
IL 145 over Bay Creek 1-S North Abutment- End Slope (drained condition)



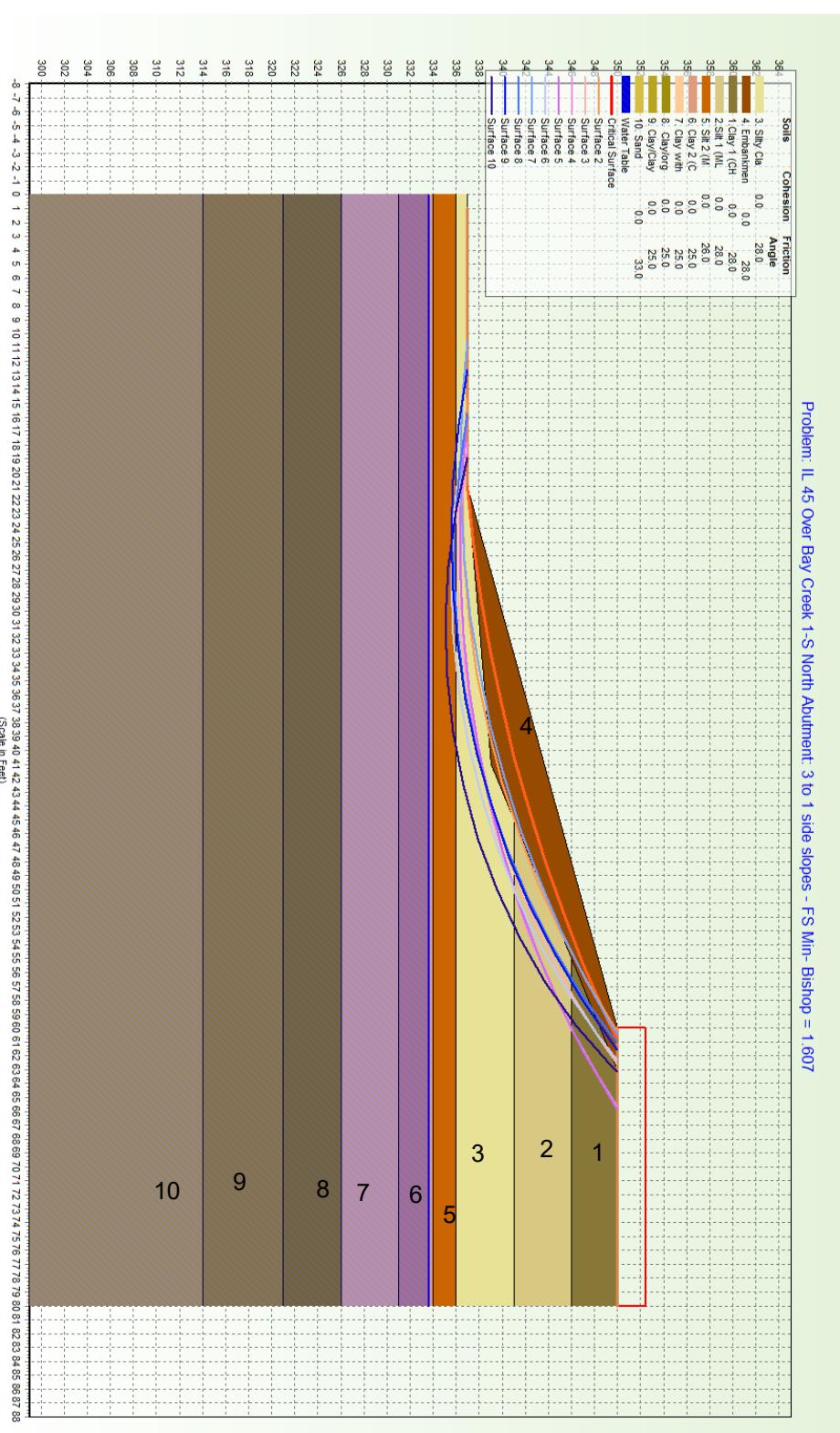
IL 145 over Bay Creek 1-S North Abutment- End Slope (seismic condition)



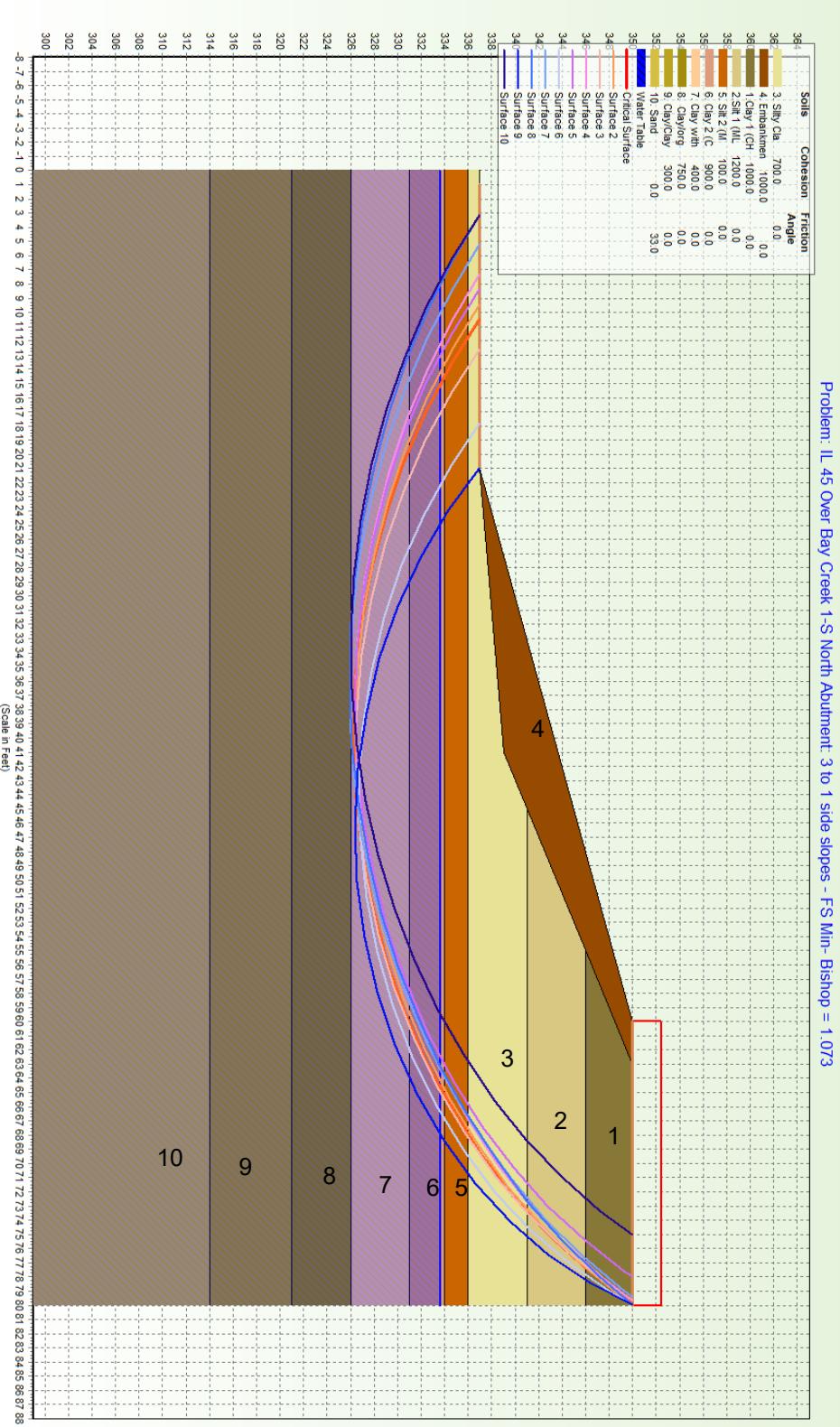
IL 145 over Bay Creek 1-S North Abutment – 3:1 Side Slope (undrained condition)



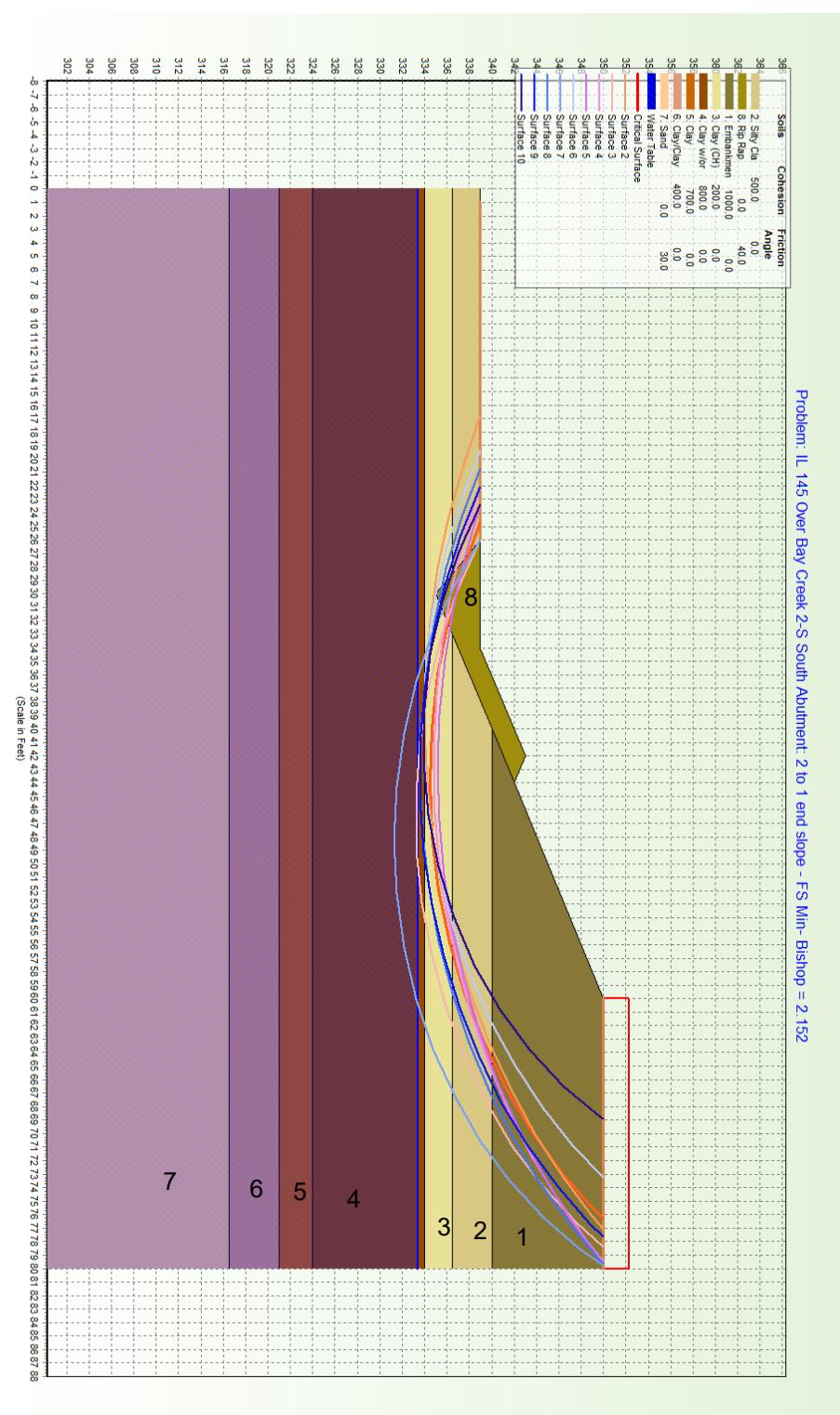
IL 145 over Bay Creek 1-S North Abutment – 3:1 Side Slope (drained condition)



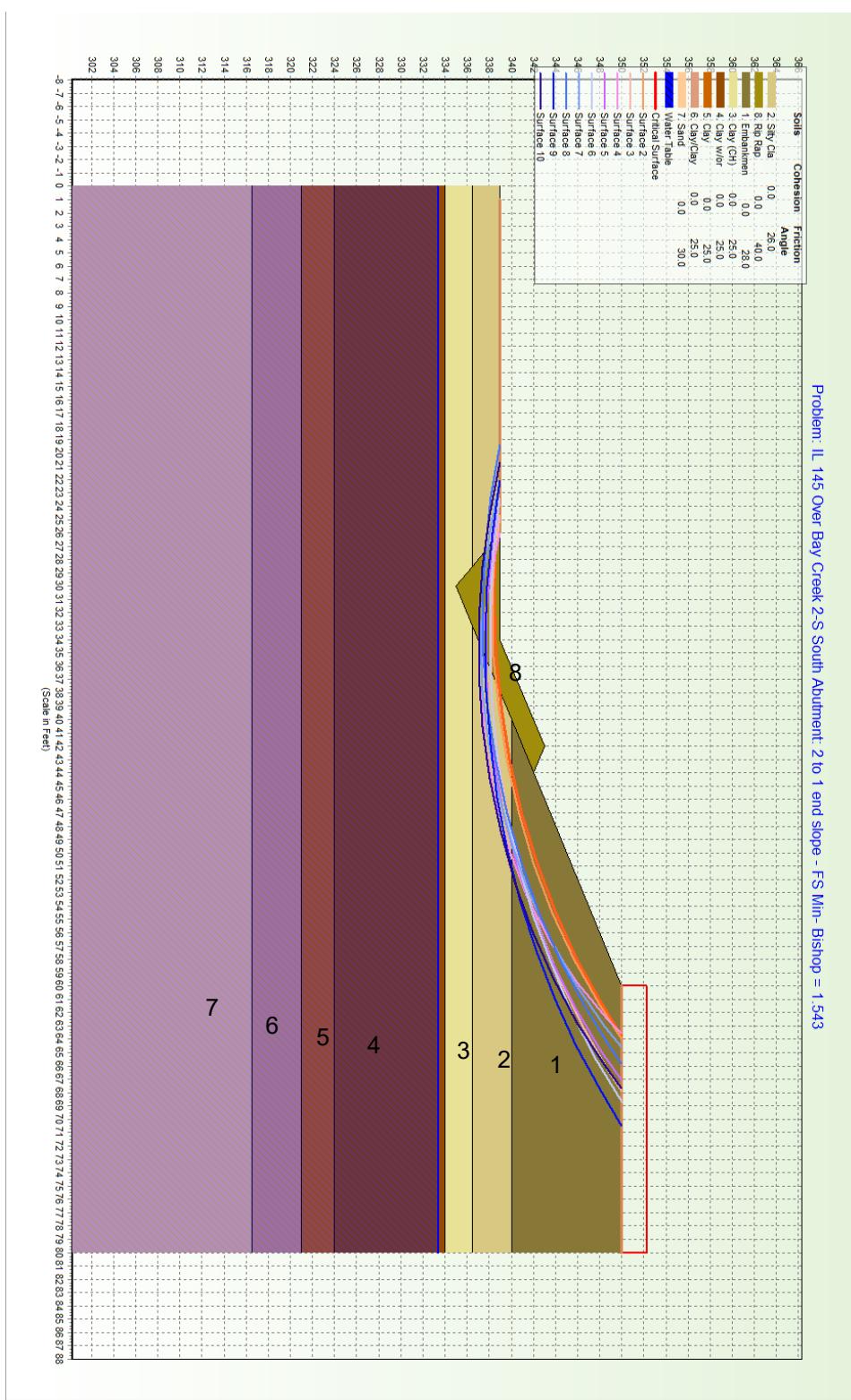
IL 145 over Bay Creek 1-S North Abutment – 3:1 Side Slope (seismic condition)



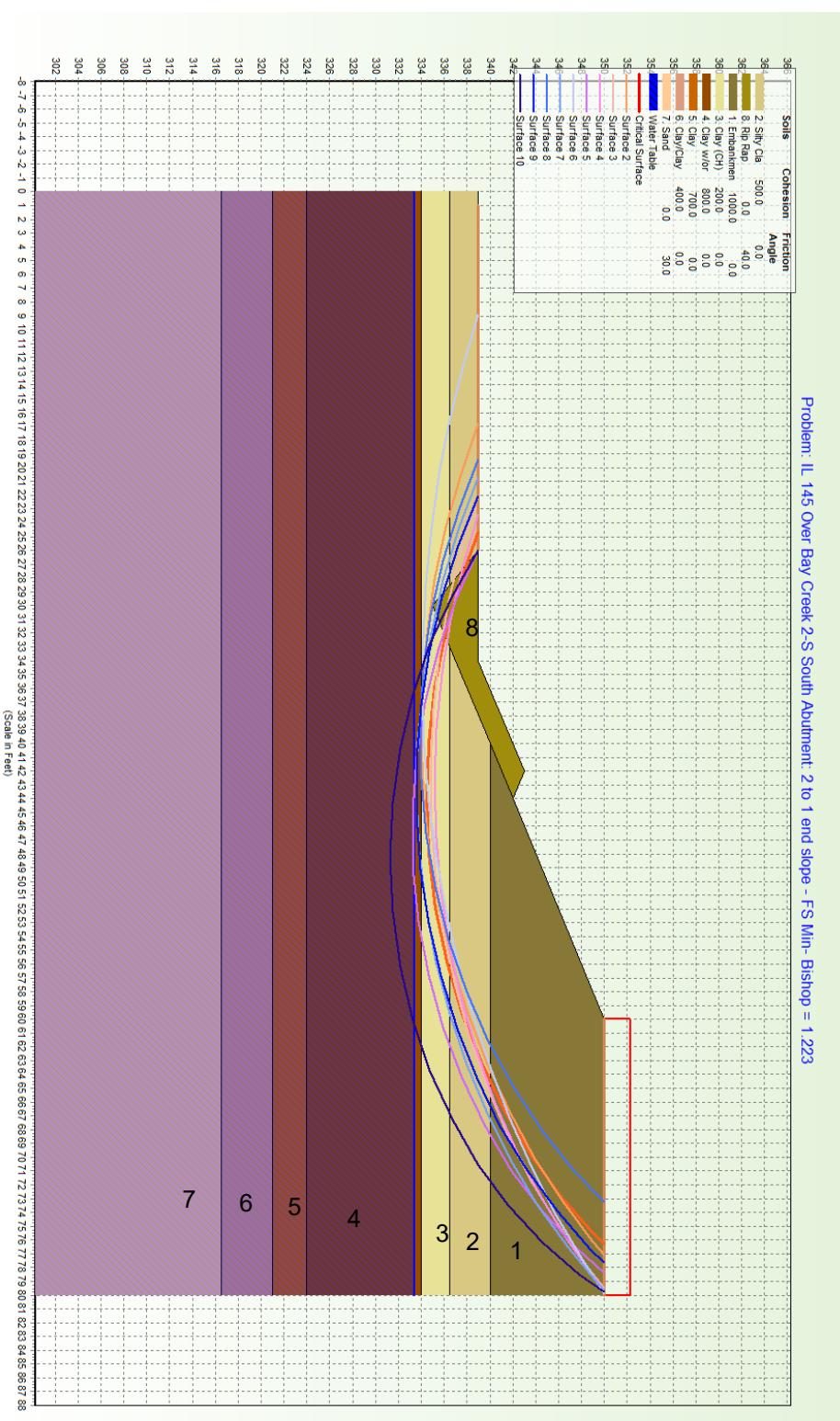
IL 145 over Bay Creek 2-S South Abutment- End Slope (undrained condition)



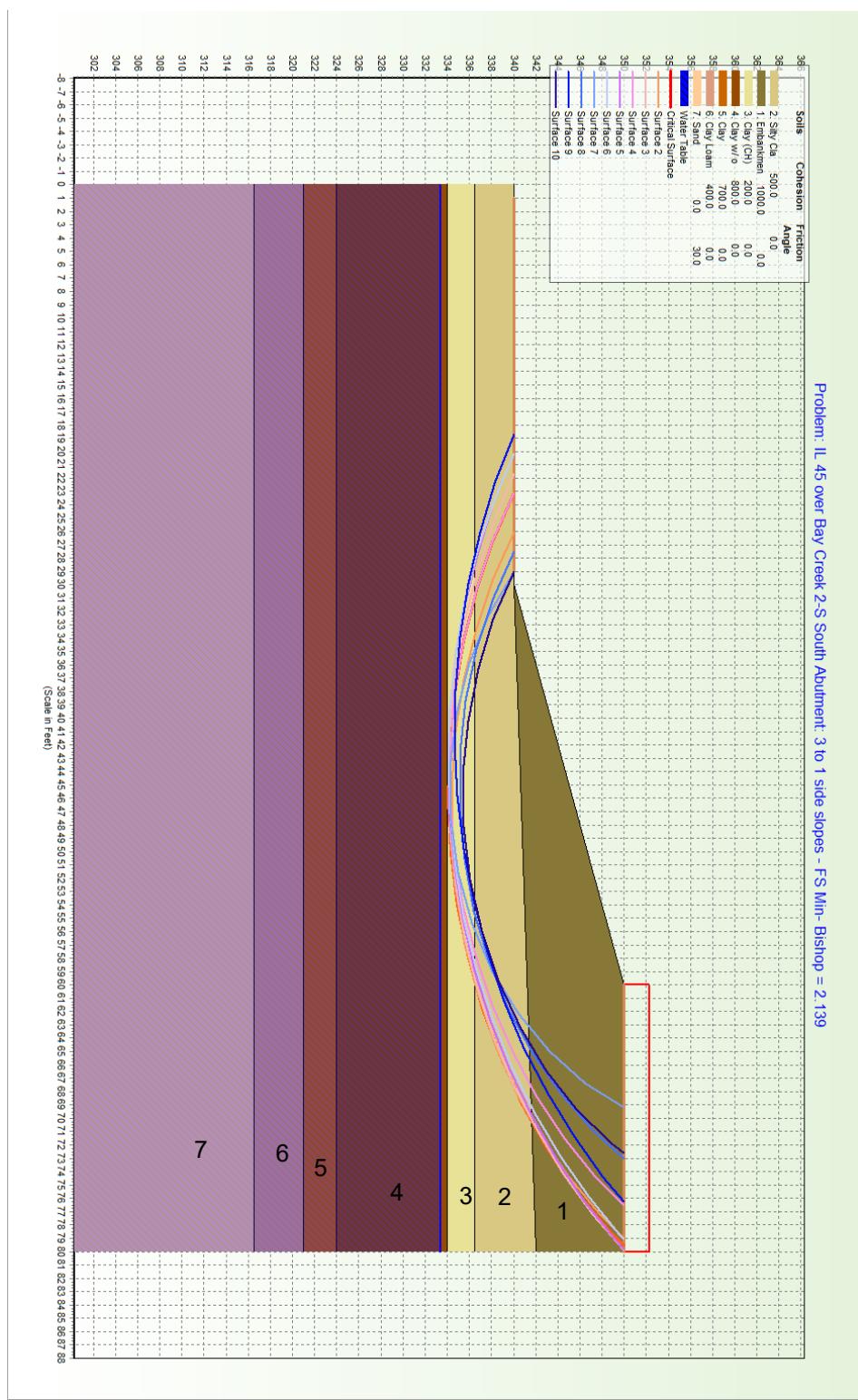
IL 145 over Bay Creek 2-S South Abutment- End Slope (drained condition)



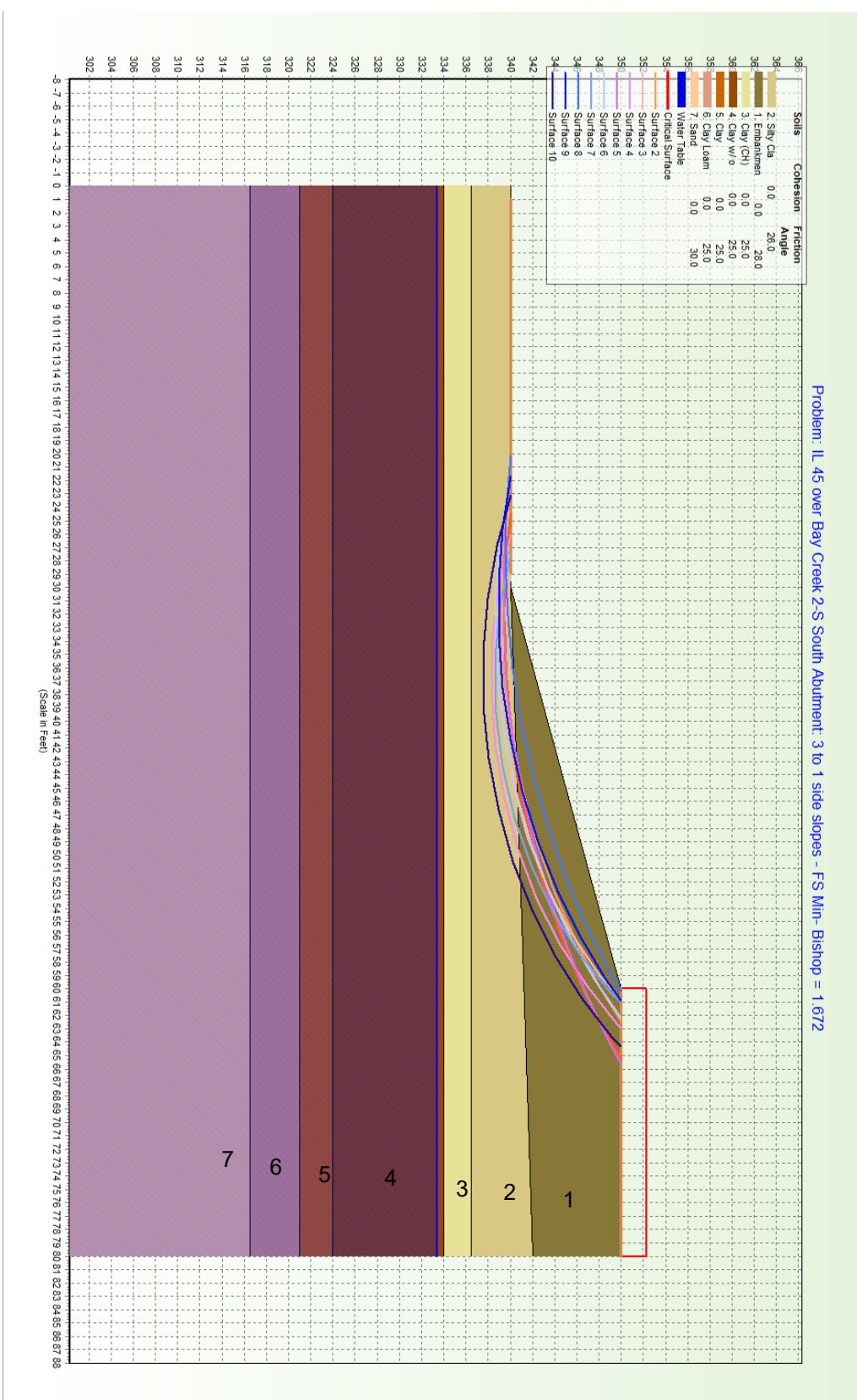
IL 145 over Bay Creek 2-S South Abutment- End Slope (seismic condition)



IL 145 over Bay Creek 2-S South Abutment 3:1 Side Slope (undrained condition)

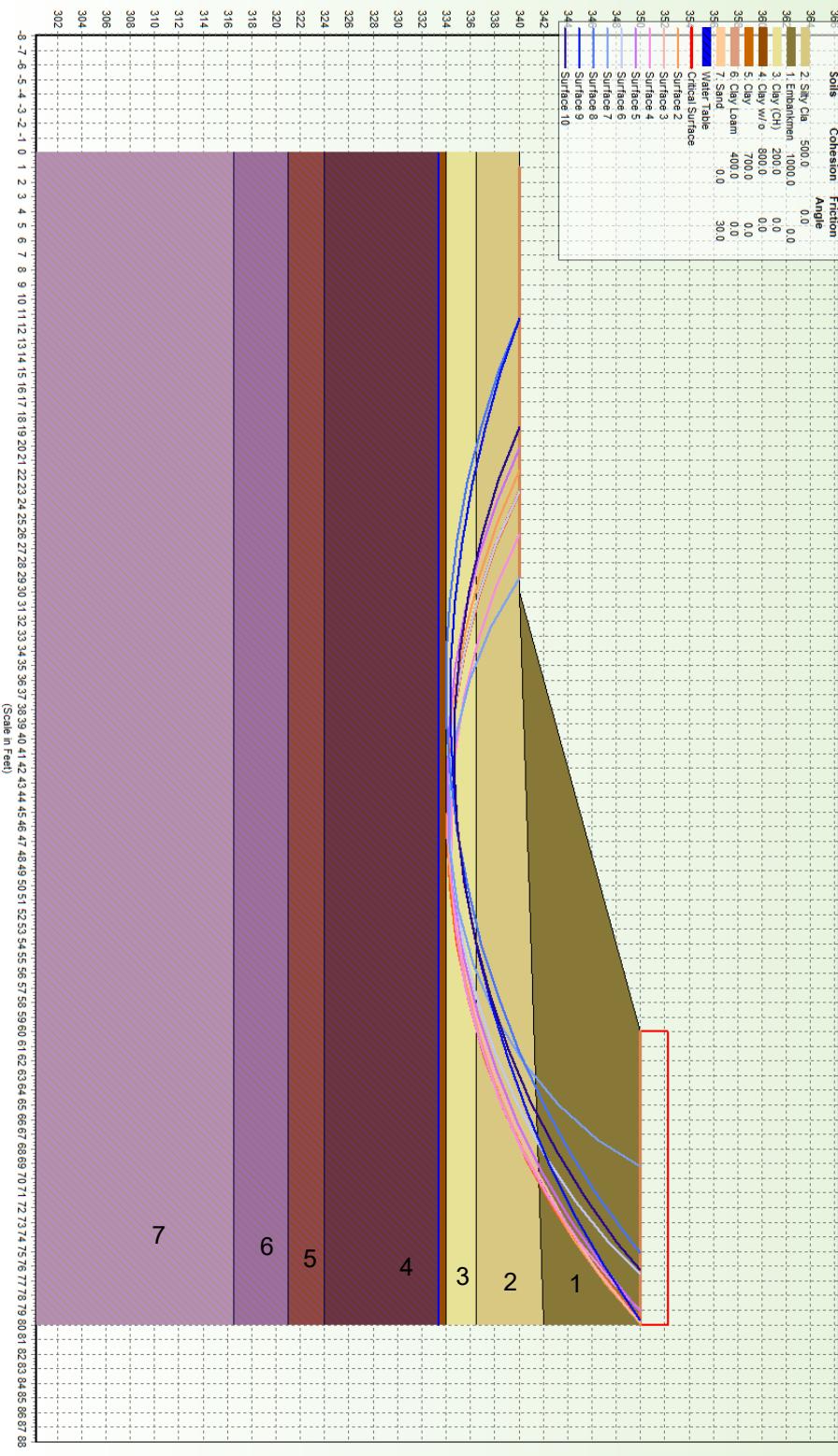


IL 145 over Bay Creek 2-S South Abutment 3:1 Side Slope (drained condition)



IL 145 over Bay Creek 2-S South Abutment 3:1 Side Slope (seismic condition)

Problem: IL 45 over Bay Creek 2-S South Abutment: 3 to 1 side slopes - FS Min- Bishop = 1.102





MILLENNIA Professional Services Appendix D

11 Executive Drive, Suite 12

Fairview Heights, Illinois 62208

618-624-8610

Appendix D: Seismic Site Class and Liquefaction Spreadsheets

SEISMIC SITE CLASS DETERMINATION

D 145 D 342.88 D 342.88 D 336.88 D 336.88

M d 12.10.10

DR 145 r 342.88

Substructure 1			
Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick.	Layer Description
		Thickness (tsf)	Boundary
		342.88	
		12	
		349.9	
		336.88	

Individual Site Class Definition:

4 13 0.33

Substructure 2			
Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick.	Layer Description
		Thickness (tsf)	Boundary
		342.88	
		12	
		2	
		350.1	
		336.88	

Individual Site Class Definition:

4 16 0.38

Substructure 3			
Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick.	Layer Description
		Thickness (tsf)	Boundary
		336.3	
		12	
		3	
		338.5	
		330.3	

Individual Site Class Definition:

5 16 0.39

Substructure 4			
Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick.	Layer Description
		Thickness (tsf)	Boundary
		339.3	
		12	
		4	
		339.5	
		333.3	

Individual Site Class Definition:

4 15 0.22

Substructure 5			
Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick.	Layer Description
		Thickness (tsf)	Boundary

Individual Site Class Definition:

4 15 0.22

Global Site Class Definition: Substructures 1 through 4

4 15 0.33

Global Site Class Definition: Substructures 1 through 4			
Seismic Soil Column Depth (ft)	Bot. Of Sample Elevation (ft)	Sample Thick.	Layer Description
		Thickness (tsf)	Boundary
		4	
		15	
		0.33	

SEISMIC SITE CLASS DETERMINATION

Mid 12.10.10

OR 145 r

Substructure 1			
Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	N	Qu
		342.88	
		48	
		1	
		349.9	
		318.88	

Individual Site Class Definition:



Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	Layer Description
		Boundary
347.4	2.50	4 1.50
344.9	2.50	4 1.50
342.4	2.50	4 1.50
341.4	1.00	4 1.50
338.9	2.50	5 0.70
336.4	2.50	5 0.70
333.9	2.50	1 0.10
331.4	2.50	1 0.90
328.9	2.50	2 0.50
326.4	2.50	1 0.30
323.9	2.50	1 1.20
321.4	2.50	1 0.70
318.9	2.50	1 0.30
316.4	2.50	1 0.40
313.9	2.50	1 0.30
311.4	2.50	6
308.9	2.50	9
306.4	2.50	9
303.9	2.50	9
301.4	2.50	9
298.9	2.50	9
296.4	2.50	9
293.9	2.50	20
291.4	2.50	20
288.9	2.50	24
286.4	2.50	24
283.9	2.50	19
281.4	2.50	19
278.9	2.50	14
276.4	2.50	14
273.9	2.50	21
271.4	2.50	21
268.9	2.50	15
266.4	2.50	15
263.9	2.50	11
261.4	2.50	11
258.9	2.50	11
256.4	2.50	11
253.9	2.50	11
251.4	2.50	14
248.9	2.50	14
246.4	2.50	14
243.9	2.50	14
241.4	2.50	16
238.9	2.50	16
236.4	2.50	16
233.9	2.50	16
231.4	2.50	16
228.9	2.50	16
226.4	2.50	16
223.9	2.50	16
221.4	2.50	16
218.9	2.50	16
216.4	2.50	16
213.9	2.50	16
211.4	2.50	16
208.9	2.50	16
206.4	2.50	16
203.9	2.50	16
201.4	2.50	16
198.9	2.50	16
196.4	2.50	16
193.9	2.50	16
191.4	2.50	16
188.9	2.50	16
186.4	2.50	16
183.9	2.50	16
181.4	2.50	16
178.9	2.50	16
176.4	2.50	16
173.9	2.50	16
171.4	2.50	16
168.9	2.50	16
166.4	2.50	16
163.9	2.50	16
161.4	2.50	16
158.9	2.50	16
156.4	2.50	16
153.9	2.50	16
151.4	2.50	16
148.9	2.50	16
146.4	2.50	16
143.9	2.50	16
141.4	2.50	16
138.9	2.50	16
136.4	2.50	16
133.9	2.50	16
131.4	2.50	16
128.9	2.50	16
126.4	2.50	16
123.9	2.50	16
121.4	2.50	16
118.9	2.50	16
116.4	2.50	16
113.9	2.50	16
111.4	2.50	16
108.9	2.50	16
106.4	2.50	16
103.9	2.50	16
101.4	2.50	16
98.9	2.50	16
96.4	2.50	16
93.9	2.50	16
91.4	2.50	16
88.9	2.50	16
86.4	2.50	16
83.9	2.50	16
81.4	2.50	16
78.9	2.50	16
76.4	2.50	16
73.9	2.50	16
71.4	2.50	16
68.9	2.50	16
66.4	2.50	16
63.9	2.50	16
61.4	2.50	16
58.9	2.50	16
56.4	2.50	16
53.9	2.50	16
51.4	2.50	16
48.9	2.50	16
46.4	2.50	16
43.9	2.50	16
41.4	2.50	16
38.9	2.50	16
36.4	2.50	16
33.9	2.50	16
31.4	2.50	16
28.9	2.50	16
26.4	2.50	16
23.9	2.50	16
21.4	2.50	16
18.9	2.50	16
16.4	2.50	16
13.9	2.50	16
11.4	2.50	16
8.9	2.50	16
6.4	2.50	16
3.9	2.50	16
1.4	2.50	16
-0.9	2.50	16
0.0	2.50	16

Substructure 2

Substructure 2			
Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	N	Qu
		342.88	
		48	
		2	
		350.1	
		318.88	

Individual Site Class Definition:



Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	Layer Description
		Boundary
347.6	2.50	3 0.90
345.1	2.50	3 0.90
344.1	1.00	3 0.90
341.6	2.50	3 0.80
339.1	2.50	3 0.50
336.6	2.50	3 0.40 R
334.1	2.50	2 0.20
331.6	2.50	3 0.80
329.1	2.50	1 0.60
326.6	2.50	1 0.90
324.1	2.50	1 0.80
321.6	2.50	1 0.70
319.1	2.50	1 0.40
316.6	2.50	1 0.40
314.1	2.50	1 0.20
73	2.50	2
98	2.50	24
123	2.50	24
148	2.50	22
173	2.50	22
198	2.50	24
223	2.50	24
248	2.50	11
273	2.50	11
298	2.50	16
323	2.50	16
348	2.50	22
373	2.50	22
398	2.50	14
423	2.50	14
448	2.50	16
473	2.50	16
498	2.50	15
523	2.50	15
548	2.50	15
573	2.50	15
598	2.50	15
623	2.50	15
648	2.50	16
100.0	218.9	35.20 16

Substructure 3

Substructure 3			
Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	N	Qu
		336.3	
		48	
		3	
		338.5	
		312.3	

Individual Site Class Definition:



Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	Layer Description
		Boundary
335.5	3.00	14 2.80
333.0	2.50	3 1.00
330.5	2.50	3 0.80
328.0	2.50	2 0.40
325.5	2.50	1 0.70
323.0	2.50	1 0.30
320.5	2.50	1 0.50
318.0	2.50	1 0.50
315.5	2.50	1 0.30
313.0	2.50	2 0.30
1.8	310.5	2.50 3
4.3	308.0	2.50 2
6.8	305.5	2.50 12
9.3	303.0	2.50 5
11.8	300.5	2.50 4
14.3	298.0	2.50 20
16.8	295.5	2.50 19
19.3	293.0	2.50 19
21.8	290.5	2.50 22
24.3	288.0	2.50 22
26.8	285.5	2.50 23
29.3	283.0	2.50 23
31.8	280.5	2.50 33
34.3	278.0	2.50 33
36.8	275.5	2.50 14
39.3	273.0	2.50 14
41.8	270.5	2.50 7
44.3	268.0	2.50 7
46.8	265.5	2.50 19
49.3	263.0	2.50 19
51.8	260.5	2.50 14
54.3	258.0	2.50 14
56.8	255.5	2.50 11
59.3	253.0	2.50 11
61.8	250.5	2.50 16
64.3	248.0	2.50 16
66.8	245.5	2.50 16
69.3	243.0	2.50 16
71.8	240.5	2.50 14
74.3	238.0	2.50 14
100.0	212.3	25.70 23

Substructure 4

Substructure 4			
Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	N	Qu
		339.3	
		48	
		3	
		339.5	
		315.3	

Individual Site Class Definition:



Seismic Soil Column Depth (ft)	Bot. Of Sample Thickness (ft)	Layer Description
		Boundary
336.5	3.00	5 0.90
334.0	2.50	1 0.30
331.5	2.50	1 0.30
329.0	2.50	1 0.40
326.5	2.50	1 0.40
324.0	2.50	1 0.50
321.5	2.50	1 0.30
319.0	2.50	1 0.30
316.		

1	349.90
2	36.00
3	16.30
4	0.481
5	7.5
6	0.00
7	89
8	8
9	M

EQ MAGNITUDE SCALING FACTOR

M 1.000

AVG. SHEAR WAVE VELOCITY (top 40')

V40' = 231

PGA CALCULATOR

 M = 7.5
 D = 30
 R = 30
 PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA						CONDITIONS DURING DRILLING						CONDITIONS DURING EARTHQUAKE							
	BORING DEPTH (FT.)	SPT VALUE (BLOWS)	UNCONF. COMPR. STR., Q _u (TSF.)	% FINES < #200	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	CORR. SPT N VALUE (N ₁) ₆₀	EQUIV. CLN. N VALUE (N ₁) _{60s}	CRR RESIST. MAG 7.5 CRR _{7.5}	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	TOTAL STRESS (KS)	OVER- BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR _{7.5} CRR	SOIL MASS PART. FACT. (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR
344.9	5	4	1.50				25	0.126	0.630	8.263	8.263	0.098	0.126	0.630	0.630	1.307	0.128	0.828	0.259	N.L. (1)
343.9	6	5	1.10				24	0.123	0.753	9.943	9.943	0.113	0.123	0.753	0.753	1.270	0.143	0.796	0.249	N.L. (1)
341.4	8.5	5	0.70				42	0.117	1.046	9.510	9.510	0.109	0.117	1.046	1.046	1.175	0.128	0.718	0.224	N.L. (1)
338.9	11	5	0.70				30	0.117	1.338	9.397	9.397	0.108	0.117	1.338	1.338	1.111	0.120	0.648	0.202	N.L. (1)
336.4	13.5	1	0.10	10	32	30	0.098	1.583	1.852	1.852	0.052	0.098	1.583	1.583	1.060	0.055	0.586	0.183	N.L. (1)	
333.9	16	1	0.90	12	35	55	0.120	1.883	1.784	1.784	0.052	0.120	1.883	1.883	1.024	0.053	0.532	0.166	N.L. (1)	
331.4	18.5	2	0.50	12	35	59	0.114	2.168	3.432	3.432	0.061	0.052	2.013	2.150	1.010	0.062	0.487	0.163	N.L. (2)	
328.9	21	1	0.30	12	35	59	0.108	2.438	1.650	1.650	0.051	0.046	2.128	2.421	0.999	0.051	0.450	0.160	N.L. (2)	
326.4	23.5	1	1.20	12	35	164	0.124	2.748	1.571	1.571	0.051	0.062	2.283	2.732	0.985	0.050	0.419	0.157	N.L. (2)	
323.9	26	1	0.70	12	35	67	0.117	3.041	1.501	1.501	0.051	0.055	2.421	3.026	0.974	0.049	0.395	0.154	N.L. (2)	
321.4	28.5	1	0.30	12	35	30	0.108	3.311	1.440	1.440	0.051	0.046	2.536	3.297	0.965	0.049	0.375	0.153	N.L. (2)	
318.9	31	1	0.40	12	35	29	0.111	3.588	1.382	1.382	0.050	0.049	2.658	3.575	0.956	0.048	0.360	0.151	N.L. (2)	
316.4	33.5	1	0.30	8	27	29	0.108	3.858	1.329	1.329	0.050	0.046	2.773	3.846	0.948	0.048	0.347	0.151	0.318 (C)	
313.9	36	6	2				0.113	4.141	7.661	7.661	0.093	0.175	3.211	4.440	0.914	0.085	0.338	0.146	0.582 (C)	
311.4	38.5	6	2				15	0.057	4.283	7.525	7.525	0.092	0.057	3.353	4.738	0.906	0.083	0.330	0.146	0.568 (C)
306.4	43.5	9	2				15	0.060	4.583	10.875	10.875	0.121	0.060	3.653	5.350	0.879	0.106	0.320	0.146	0.726 (C)
301.4	48.5	9	2				15	0.060	4.883	10.488	10.488	0.117	0.060	3.953	5.962	0.864	0.101	0.314	0.148	0.682 (C)
296.4	53.5	20	4				21	0.067	5.218	22.799	22.799	0.254	0.067	4.288	6.609	0.803	0.204	0.310	0.149	1.369 (D)
291.4	58.5	24	4				21	0.069	5.563	26.676	26.676	0.330	0.069	4.633	7.266	0.769	0.254	0.308	0.151	N.L. (3)
286.4	63.5	19	4				21	0.067	5.898	19.710	19.710	0.212	0.067	4.968	7.913	0.780	0.165	0.306	0.153	1.078 (D)
281.4	68.5	14	4				21	0.064	6.218	13.932	13.932	0.150	0.064	5.288	8.545	0.791	0.118	0.299	0.151	0.781 (D)
276.4	73.5	21	2				16	0.068	6.558	20.264	20.264	0.219	0.068	5.628	9.197	0.749	0.164	0.292	0.149	1.101 (D)
271.4	78.5	15	2				16	0.065	6.883	13.898	13.898	0.149	0.065	5.953	9.834	0.768	0.115	0.285	0.147	0.782 (D)
266.4	83.5	11	2				16	0.062	7.193	9.876	9.876	0.112	0.062	6.263	10.456	0.779	0.087	0.278	0.145	0.600 (C)
256.4	93.5	14	2					0.064	7.833	11.794	11.794	0.129	0.064	6.903	11.720	0.751	0.097	0.264	0.140	0.693 (C)

 R = 1
 R = 2
 R = 3
 R = D
 D = 12
 D = 25
 D = 0.85


LIQUEFACTION ANALYSIS
EQ MAGNITUDE SCALING FACTOR
1.000

2 10	
350 10	
37 50	
16 80	
0 481	
	7.5
	0.00
	89
	8
	50

AVG. SHEAR WAVE VELOCITY (top 40')
230
PGA CALCULATOR

M	7.5
D	30
M	0.535

PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA						CONDITIONS DURING DRILLING						CONDITIONS DURING EARTHQUAKE						CORR. RESIST. CRR _{7.5} / CRR	SOIL MASS PART. FACTOR (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR
	BORING DEPTH (FT.)	SPT N VALUE (BLOWS)	UNCONF. STR., Q _u (TSF.)	% FINES < #200	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. STRESS (KCF.)	CORR. SPT N VALUE (N ₁) ₆₀	EQUIV. CLN. SAND SPT VALUE (N ₁) _{60s}	CRR RESIST. MAG 7.5 (KCF.)	EFFECTIVE UNIT WT. STRESS (KSF.)	TOTAL VERT. STRESS (KSF.)	OVER-BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR _{7.5} / CRR	SOIL MASS PART. FACTOR (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR				
346.6	3.5	3	0.90				33	0.120	0.420	6.636	6.636	0.085	0.120	0.420	0.420	1.006	0.119	0.878	0.275	N.L. (1)		
344.1	6	3	0.60				28	0.116	0.710	6.045	6.045	0.080	0.116	0.710	0.710	1.254	0.100	0.795	0.248	N.L. (1)		
341.6	8.5	3	0.50				27	0.114	0.995	5.787	5.787	0.078	0.114	0.995	0.995	1.168	0.091	0.717	0.224	N.L. (1)		
339.1	11	3	0.40				26	0.111	1.273	5.735	5.735	0.078	0.111	1.273	1.273	1.110	0.086	0.646	0.202	N.L. (1)		
336.6	13.5	2	0.20	12	35	32	0.104	1.533	3.750	3.750	0.063	0.104	1.533	1.533	1.067	0.067	0.584	0.183	N.L. (1)			
334.1	16	3	0.80	12	35	56	0.119	1.830	5.418	5.418	0.075	0.119	1.830	1.830	1.030	0.077	0.531	0.166	N.L. (1)			
331.6	18.5	1	0.60	12	35	61	0.126	2.120	1.733	1.733	0.052	0.054	1.965	2.071	1.015	0.052	0.486	0.160	N.L. (2)			
329.1	21	1	0.90	12	35	62	0.120	2.420	1.655	1.655	0.051	0.058	2.110	2.372	1.001	0.051	0.449	0.158	N.L. (2)			
326.6	23.5	1	0.80	12	35	64	0.119	2.718	1.580	1.580	0.051	0.057	2.253	2.671	0.988	0.050	0.418	0.155	N.L. (2)			
324.1	26	1	0.70	12	35	28	0.117	3.010	1.509	1.509	0.051	0.055	2.390	2.964	0.976	0.050	0.394	0.153	N.L. (2)			
321.6	28.5	1	0.40	12	35	30	0.111	3.288	1.446	1.446	0.051	0.049	2.513	3.243	0.967	0.049	0.374	0.151	N.L. (2)			
319.1	31	1	0.40	12	35	28	0.111	3.565	1.387	1.387	0.050	0.049	2.635	3.521	0.957	0.048	0.359	0.150	N.L. (2)			
316.6	33.5	1	0.20	0	0	28	0.104	3.825	1.335	1.335	0.050	0.042	2.740	3.782	0.950	0.048	0.346	0.150	0.320 (C)			
314.1	36	2				23	0.101	4.078	2.578	2.578	0.056	0.039	2.838	4.036	0.943	0.053	0.337	0.150	0.353 (C)			
311.6	38.5	24				21	0.069	4.250	32.025	32.025	0.739	0.069	3.010	4.364	0.879	0.649	0.329	0.149	N.L. (3)			
306.6	43.5	22	37			27	0.068	4.590	27.615	38.137	0.036	0.068	3.350	5.016	0.833	0.030	0.319	0.149	N.L. (3)			
301.6	48.5	24	28			27	0.069	4.935	29.028	37.601	0.004	0.069	3.695	5.673	0.801	-0.003	0.313	0.150	N.L. (3)			
296.6	53.5	11	28			27	0.062	5.245	12.271	18.528	0.198	0.062	4.005	6.295	0.834	0.165	0.309	0.152	1.086 (C)			
291.6	58.5	16					0.065	5.570	17.174	17.174	0.183	0.065	4.330	6.932	0.821	0.150	0.307	0.154	0.974 (D)			
286.6	63.5	22					0.068	5.910	23.128	23.128	0.259	0.068	4.670	7.584	0.781	0.202	0.306	0.155	1.303 (D)			
281.6	68.5	14					0.064	6.230	13.913	13.913	0.149	0.064	4.990	8.216	0.803	0.120	0.299	0.154	0.779 (D)			
276.6	73.5	16	2				0.065	6.555	15.338	15.338	0.163	0.065	5.315	8.853	0.784	0.128	0.292	0.152	0.842 (D)			
271.6	78.5	15	2				0.065	6.880	13.902	13.902	0.149	0.065	5.640	9.490	0.779	0.116	0.285	0.150	0.773 (D)			
261.6	88.5	16	2				0.065	7.530	13.884	13.884	0.149	0.065	6.290	10.764	0.757	0.113	0.271	0.145	0.779 (C)			

R D R D R D R D R D R D
 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25
 0.85
 25



LIQUEFACTION ANALYSIS

3	338.50
23.50	
5.00	
0.481	
7.5	
0.00	
75	
8	

EQ MAGNITUDE SCALING FACTOR
M 1.000

AVG. SHEAR WAVE VELOCITY (top 40')
V_s 262

PGA CALCULATOR

M	M	M	7.5
D	D	IR	30
R	R	R	M

PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA						CONDITIONS DURING DRILLING						CONDITIONS DURING EARTHQUAKE						CORR. RESIST. PART. CRR _{7.5} CRR	SOIL MASS FACTOR (r _d)	FACTOR OF SAFETY * CRR/CSR
	BORENG DEPTH (FT.)	SPT N VALUE (BLOWS)	UNCONF. STR., Q _u (TSF.)	% FINES < #200 (%)	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. STRESS (KCF.)	CORR. SPT N VALUE (N ₁) ₆₀	EQUIV. CLN. SAND SPT N VALUE (N ₁) _{60s}	CRR RESIST. MAG 7.5	UNIT WT. STRESS (KSF.)	TOTAL VERT. STRESS (KSF.)	OVER-BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR _{7.5} CRR						
336	2.5	14	2.8				20	0.134	0.335	29.998	29.998	0.467	0.134	0.335	0.335	1.500	0.701	0.925	0.289	N.L. (1)	
333.5	5	3	1				39	0.122	0.640	5.212	5.212	0.074	0.122	0.640	0.640	1.273	0.094	0.849	0.266	N.L. (1)	
331	7.5	3	0.8		12	35	57	0.119	0.938	4.797	4.797	0.071	0.057	0.783	0.939	1.221	0.086	0.775	0.291	N.L. (2)	
328.5	10	2	0.4		12	35	52	0.111	1.215	3.198	3.198	0.060	0.049	0.905	1.217	1.186	0.071	0.706	0.297	N.L. (2)	
326	12.5	1	0.7		12	35	106	0.117	1.508	1.565	1.565	0.051	0.055	1.043	1.511	1.153	0.059	0.642	0.291	N.L. (2)	
323.5	15	1	0.3		12	35	132	0.108	1.778	1.524	1.524	0.051	0.046	1.158	1.782	1.129	0.057	0.585	0.281	N.L. (2)	
321	17.5	1	0.5		20	41	53	0.114	2.063	1.468	1.468	0.051	0.052	1.288	2.068	1.105	0.056	0.535	0.269	N.L. (2)	
318.5	20	1	0.5		20	41	33	0.114	2.348	1.409	1.409	0.050	0.052	1.418	2.354	1.084	0.055	0.494	0.256	N.L. (2)	
316	22.5	1	0.3		12	30	28	0.108	2.618	1.353	1.353	0.050	0.046	1.533	2.625	1.067	0.054	0.459	0.246	N.L. (2)	
313.5	25	2	0.3		7	23	27	0.046	2.733	2.672	2.672	0.056	0.046	1.648	2.896	1.052	0.059	0.430	0.236	0.250 (C)	
311	27.5	3						0.051	2.860	3.941	3.941	0.064	0.051	1.775	3.179	1.036	0.067	0.407	0.228	0.294 (C)	
308.5	30	3	0.3		8	23	17	0.046	2.975	3.879	3.879	0.064	0.046	1.890	3.450	1.023	0.066	0.388	0.222	N.L. (2)	
306	32.5	12						0.063	3.133	15.147	15.147	0.162	0.063	2.048	3.764	1.009	0.163	0.373	0.215	0.758 (D)	
303.5	35	5						0.055	3.270	6.184	6.184	0.081	0.055	2.185	4.057	0.994	0.081	0.362	0.210	0.386 (C)	
301	37.5	4						0.053	3.403	4.853	4.853	0.071	0.053	2.318	4.346	0.982	0.070	0.353	0.207	0.338 (C)	
298.5	40	20						0.067	3.570	24.802	24.802	0.288	0.067	2.485	4.669	0.950	0.274	0.346	0.203	1.350 (D)	
293.5	45	19						0.067	3.905	22.168	22.168	0.244	0.067	2.820	5.316	0.916	0.224	0.336	0.198	1.131 (D)	
288.5	50	22						0.068	4.245	24.794	24.794	0.288	0.068	3.160	5.968	0.879	0.253	0.330	0.195	1.297 (D)	
283.5	55	23						0.068	4.585	24.756	24.756	0.287	0.068	3.500	6.620	0.850	0.244	0.326	0.193	1.264 (D)	
278.5	60	33						0.072	4.945	35.498	35.498	0.409	0.072	3.860	7.292	0.791	-0.324	0.324	0.191	N.L. (3)	
273.5	65	14						0.064	5.265	13.187	13.187	0.142	0.064	4.180	7.924	0.843	0.120	0.321	0.190	0.632 (D)	
268.5	70	7						0.058	5.555	6.358	6.358	0.083	0.058	4.470	8.526	0.856	0.071	0.314	0.187	0.380 (C)	
263.5	75	19						0.067	5.890	16.658	16.658	0.177	0.067	4.805	9.173	0.800	0.142	0.307	0.183	0.776 (D)	
258.5	80	14						0.064	6.210	11.792	11.792	0.129	0.064	5.125	9.805	0.807	0.104	0.300	0.179	0.581 (C)	
253.5	85	11						0.062	6.520	8.950	8.950	0.104	0.062	5.435	10.427	0.809	0.084	0.293	0.176	0.477 (C)	
248.5	90	16						0.065	6.845	12.568	12.568	0.136	0.065	5.760	11.064	0.781	0.107	0.286	0.172	0.622 (C)	
243.5	95	16						0.065	7.170	12.147	12.147	0.133	0.065	6.085	11.701	0.772	0.102	0.279	0.168	0.607 (C)	
238.5	100	14						0.064	7.490	10.086	10.086	0.114	0.064	6.405	12.333	0.774	0.088	0.272	0.164	0.537 (C)	
233.5	105	23						0.068	7.830	15.237	15.237	0.162	0.068	6.745	12.985	0.737	0.120	0.265	0.159	0.755 (D)	
228.5	110	75	5		20			0.079	8.225	49.954	49.954	0.303	0.079	7.140	13.692	0.615	0.186	0.258	0.154	N.L. (3)	
225	113.5	50	5					0.079	8.502	27.568	27.568	0.355	0.079	7.417	14.187	0.652	0.232	0.253	0.151	N.L. (3)	

R	D	R	D
1	2	3	1
25	16	12	25
0.85			



EQ MAGNITUDE SCALING FACTOR

M 1.000

AVG. SHEAR WAVE VELOCITY (top 40')

215

Page 1

PGA CALCULATOR					
<input type="checkbox"/> r	<input type="checkbox"/> d	M	<input type="checkbox"/> M	<input type="checkbox"/> d	7.5
<input type="checkbox"/> r	<input type="checkbox"/> d	<input type="checkbox"/> D	<input type="checkbox"/> R	<input type="checkbox"/> d	30
<input type="checkbox"/> r	<input type="checkbox"/> d	M	<input type="checkbox"/> r	<input type="checkbox"/> d	<input type="checkbox"/> M
PGA = 0.535					

PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA							CONDITIONS DURING DRILLING							CONDITIONS DURING EARTHQUAKE						
	BORING SAMPLE	SPT N	UNCONF. COMPR.	% FINES	PLAST. INDEX	LIQUID LIMIT	MOIST. CONTENT	EFFECTIVE UNIT (KCF.)	CORR. VERT. (KSF.)	EQUIV. CLN. SPT N VALUE (N ₁) ₆₀	CRR RESIST. MAG 7.5 CRR _{7.5}	EFFECTIVE UNIT (KCF.)	TOTAL VERT. STRESS (KSF.)	OVER- BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR _{7.5} CRR	SOIL MASS PART. EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR				
	DEPTH (FT.)	VALUE (BLOWS)	STR., Q _u (TSF.)	< #200 (%)	PI	LL	w _c (%)	WT. (KCF.)	STRESS (KSF.)	N VALUE (N ₁) _{60cs}		WT. (KCF.)	STRESS (KSF.)								
337	2.5	5	0.9				21	0.120	0.300	9.724	9.724	0.111	0.120	0.300	0.300	1500	0.166	0.907	0.284	N.L. (1)	
334.5	5	1	0.1				25	0.098	0.545	1.791	1.791	0.052	0.098	0.545	0.545	1312	0.068	0.818	0.256	N.L. (1)	
332	7.5	1	0.3		12	35	27	0.108	0.815	1.857	1.857	0.051	0.046	0.660	0.816	1263	0.065	0.735	0.284	N.L. (2)	
329.5	10	1	0.4		12	35	29	0.111	1.093	1.653	1.653	0.051	0.049	0.783	1.095	1221	0.063	0.659	0.288	N.L. (2)	
327	12.5	1	0.4		20	40	71	0.111	1.370	1.620	1.620	0.051	0.049	0.905	1.373	1186	0.061	0.593	0.281	N.L. (2)	
324.5	15	1	0.5		20	40	35	0.114	1.655	1.569	1.569	0.051	0.052	1.035	1.659	1154	0.059	0.536	0.269	N.L. (2)	
322	17.5	1	0.3		12	35	28	0.108	1.925	1.514	1.514	0.051	0.046	1.150	1.930	1130	0.057	0.489	0.256	N.L. (2)	
319.5	20	1	0.3		12	31	26	0.108	2.195	1.454	1.454	0.051	0.046	1.265	2.201	1109	0.056	0.449	0.244	N.L. (2)	
317	22.5	1	0.3		12	31	26	0.108	2.465	1.394	1.394	0.050	0.046	1.380	2.472	1090	0.055	0.417	0.234	N.L. (2)	
314.5	25	1	0.1		12	31	29	0.098	2.710	1.342	1.342	0.050	0.160	1.780	3.028	1036	0.052	0.391	0.208	N.L. (2)	
312	27.5	1	0.1		12	31	29	0.035	2.798	1.329	1.329	0.050	0.035	1.868	3.272	1026	0.051	0.370	0.203	N.L. (2)	
309.5	30	1					0.043	2.905	1.309	1.309	0.050	0.043	1.975	3.535	1014	0.051	0.354	0.198	0.258 (C)		
307	32.5	9					0.060	3.055	11.518	11.518	0.127	0.060	2.125	3.841	0999	0.127	0.341	0.193	0.658 (D)		
304.5	35	15					0.065	3.218	19.086	19.086	0.204	0.065	2.288	4.160	0978	0.200	0.331	0.188	1.064 (D)		
299.5	40	12					0.063	3.533	14.290	14.290	0.153	0.063	2.603	4.787	0948	0.145	0.317	0.182	0.797 (D)		
297	42.5	17					0.066	3.698	20.252	20.252	0.219	0.066	2.768	5.108	0924	0.202	0.312	0.180	1.122 (D)		
294.5	45	20					0.067	3.865	23.648	23.648	0.267	0.067	2.935	5.431	0902	0.241	0.308	0.178	1.354 (D)		
289.5	50	26					0.069	4.210	30.181	30.181	0.481	0.069	3.280	6.088	0855	0.411	0.303	0.176	N.L. (3)		
284.5	55	21					0.068	4.550	22.470	22.470	0.249	0.068	3.620	6.740	0847	0.211	0.300	0.175	1.206 (D)		
279.5	60	19					0.067	4.885	19.130	19.130	0.205	0.067	3.955	7.387	0835	0.171	0.298	0.174	0.983 (D)		
274.5	65	18					0.066	5.215	17.191	17.191	0.183	0.066	4.285	8.029	0823	0.151	0.296	0.173	0.873 (D)		
269.5	70	12					0.063	5.530	10.934	10.934	0.121	0.063	4.600	8.656	0832	0.101	0.289	0.170	0.594 (C)		
264.5	75	17					0.066	5.860	14.890	14.890	0.159	0.066	4.930	9.298	0802	0.127	0.282	0.166	0.765 (D)		
259.5	80	11					0.062	6.170	9.308	9.308	0.107	0.062	5.240	9.920	0814	0.087	0.275	0.162	0.537 (C)		
254.5	85	15					0.065	6.495	12.239	12.239	0.133	0.065	5.565	10.557	0789	0.105	0.268	0.159	0.660 (C)		
249.5	90	12					0.063	6.810	9.461	9.461	0.108	0.063	5.880	11.184	0792	0.086	0.261	0.155	0.555 (C)		

Diagram illustrating the sequence of R and D units in a polymer chain:

- Block 1: 12 R units
- Block 2: 0.85 D units
- Block 3: 25 units containing 160 R units and 1 D unit
- Final block: R units

Total length: 100 units



R	R	M	R	1979
R	R	D	R	339.00
D	R	D	R	6.00
D	R	D	R	6.00
R	R	D	R	0.481
R	M	M	D	7.5
D	R	R	M	0.00
M	R	R	R	60
R	D	M	R	8
M	M	D	R	

EQ MAGNITUDE SCALING FACTOR

M 1.000

AVG. SHEAR WAVE VELOCITY (top 40')

30

[PGA CALCULATOR](#)

□r□□□□M□□□□M□□□□d□□ 75
□□r□□□□□D□□□□R□□□□ 30
□□d□M□□□□r□d□□□□□□□□ 50

ordMord

$$PGA = 0.535$$

ELEV. OF SAMPLE (FT.)	BORING DATA						CONDITIONS DURING DRILLING						CONDITIONS DURING EARTHQUAKE						FACTOR OF SAFETY *	
	BORING SAMPLE	SPT N	UNCONF. COMPR.	% FINEs	PLAST. INDEX	LIQUID LIMIT	MOIST. CONTENT	EFFECTIVE UNIT	VERT. WT.	CORR. SPT N	EQUIV. CLN. SAND SPT	CRR RESIST.	EFFECTIVE UNIT	VERT. WT.	TOTAL STRESS	OVER- BURDEN	CORR. RESIST.	SOIL MASS PART.	EQ INDUCED	FACTOR OF SAFETY *
	DEPTH (FT.)	VALUE (BLOWS)	STR., Q _u (TSF.)	< #200 (%)	PI	LL	w _c (%)	KCF.)	(KSF.)	(N ₁) ₆₀	N VALUE (N ₁) _{60s}	MAG 7.5 CRR _{7.5}	(KCF.)	(KSF.)	(KSF.)	corr. fact. (k _s)	CRR _{7.5}	FACTOR CSR	FACTOR OF SAFETY *	
336	3	5	120		15	35	44	0.124	0.372	7.587	7.587	0.092	0.124	0.372	0.372	1.457	0.135	0.949	0.297	N.L. (1)
333.5	5.5	5	130		15	35	30	0.125	0.685	6.853	6.853	0.086	0.125	0.685	0.685	1.271	0.110	0.901	0.282	N.L. (1)
331	8	5	0.50		15	40	32	0.051	0.812	6.753	6.753	0.086	0.051	0.812	0.937	1.225	0.105	0.850	0.307	N.L. (2)
328.5	10.5	8	0.50		15	40	47	0.051	0.940	11.252	11.252	0.124	0.051	0.940	1.220	1.215	0.151	0.797	0.324	N.L. (2)
326	13	3	0.20		8	32	32	0.042	1.045	4.278	4.278	0.067	0.042	1.045	1.481	1.152	0.077	0.743	0.329	0.234 (C)
323.5	15.5	3	0.60		15	40	32	0.053	1.177	4.274	4.274	0.067	0.053	1.177	1.770	1.125	0.075	0.691	0.325	N.L. (2)
321	18	2	0.20		15	35	26	0.042	1.282	2.841	2.841	0.057	0.042	1.282	2.031	1.106	0.063	0.642	0.318	N.L. (2)
318.5	20.5	2	0.40		15	35	28	0.049	1.405	2.801	2.801	0.057	0.049	1.405	2.309	1.086	0.062	0.597	0.307	N.L. (2)
316	23	2	0.50		15	35	29	0.051	1.532	2.747	2.747	0.057	0.051	1.532	2.593	1.067	0.061	0.558	0.295	N.L. (2)
313.5	25.5	3	0.50		15	35	33	0.051	1.660	4.032	4.032	0.065	0.051	1.660	2.876	1.050	0.068	0.523	0.284	N.L. (2)
311	28	2	0.70		15	35	33	0.055	1.797	2.619	2.619	0.056	0.055	1.797	3.170	1.034	0.058	0.494	0.273	N.L. (2)
308.5	30.5	3						0.051	1.925	3.836	3.836	0.064	0.051	1.925	3.453	1.020	0.065	0.470	0.264	0.246 (C)
306	33	38						0.073	2.107	55.390	55.390	0.359	0.073	2.107	3.792	1.002	0.360	0.450	0.253	N.L. (3)
303.5	35.5	25						0.069	2.280	33.680	33.680	3.368	0.069	2.280	4.120	0.973	3.277	0.434	0.245	N.L. (3)
301	38	23						0.068	2.450	29.375	29.375	0.429	0.068	2.450	4.446	0.950	0.408	0.422	0.239	N.L. (3)
298.5	40.5	20						0.067	2.617	24.094	24.094	0.275	0.067	2.617	4.770	0.935	0.257	0.411	0.234	1.098 (D)
296	43	4	0.70		15	50	58	0.055	2.755	4.385	4.385	0.068	0.055	2.755	5.063	0.949	0.064	0.403	0.232	N.L. (2)
293.5	45.5	5	0.6		15	50	71	0.053	2.887	5.362	5.362	0.075	0.053	2.887	5.352	0.939	0.070	0.397	0.230	N.L. (2)
291	48	38						0.073	3.070	46.712	46.712	0.263	0.073	3.070	5.690	0.862	0.226	0.392	0.227	N.L. (3)
288.5	50.5	38						0.073	3.252	45.340	45.340	0.243	0.073	3.252	6.029	0.843	0.205	0.388	0.225	N.L. (3)
283.5	55.5	33						0.072	3.612	35.764	35.764	0.307	0.072	3.612	6.701	0.811	-0.249	0.383	0.222	N.L. (3)



LIQUEFACTION ANALYSIS

R 1979
 R 339.00
 R 4.00
 R 4.00
 R 0.481
 R 7.5
 R 0.00
 R 60
 R 8
 M

EQ MAGNITUDE SCALING FACTOR
 M 1.00

AVG. SHEAR WAVE VELOCITY (top 40')
 V_s 369

PGA CALCULATOR

M 7.5
 D 30
 M PGA = 0.53

ELEV. OF SAMPLE (FT.)	BORING DATA						CONDITIONS DURING DRILLING						CONDITIONS DURING EARTHQUAKE						N.L. (1) N.L. (2) N.L. (2)	
	BORING DEPTH (FT.)	SPT N VALUE (BLOWS)	UNCONF. STR., Q _u (TSF.)	% FINES < #200	PLAST. INDEX PI	LIQUID LIMIT LL	MOIST. CONTENT w _c (%)	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	CORR. SPT N VALUE (N ₁) ₆₀	EQUIV. CLN. SAND SPT N VALUE (N ₁) _{60cs}	CRR RESIST. MAG 7.5 CRR _{7.5}	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	TOTAL OVER- BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR _{7.5} CRR	SOIL MASS PART. FACTOR (r _d)	EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR	
335.5	3.5	8	1:10				29	0.123	0.431	12:176	12:176	0.133	0.123	0.431	0.431	1.478	0.196	0.943	0.295	N.L. (1)
333	6	5	0.60		15	41	50	0.053	0.563	7:121	7:121	0.089	0.053	0.563	0.688	1.327	0.118	0.897	0.343	N.L. (2)
330.5	8.5	4	0.70		15	40	52	0.055	0.701	5:681	5:681	0.077	0.055	0.701	0.981	1.254	0.097	0.848	0.371	N.L. (2)
328	11	3	0.20		15	35	81	0.042	0.806	4:410	4:410	0.068	0.042	0.806	1.242	1.214	0.082	0.796	0.384	N.L. (2)
325.5	13.5	3	0.30		15	35	65	0.046	0.921	4:468	4:468	0.068	0.046	0.921	1.513	1.182	0.081	0.744	0.382	N.L. (2)
323	16	5	0.50		11	40	27	0.051	1:048	7:422	7:422	0.091	0.051	1:048	1.797	1:64	0.106	0.693	0.371	N.L. (2)
320.5	18.5	3	0.50		11	40	27	0.051	1:176	4:401	4:401	0.068	0.051	1:176	2.080	1:25	0.076	0.645	0.357	N.L. (2)
318	21	3	0.40		12	35	27	0.049	1:298	4:331	4:331	0.067	0.049	1:298	2.359	1:103	0.074	0.601	0.341	N.L. (2)
315.5	23.5	2	0.20		12	35	30	0.042	1:403	2:843	2:843	0.057	0.042	1:403	2.620	1:086	0.062	0.562	0.328	N.L. (2)
313	26	3	0.50		12	35	29	0.051	1:531	4:165	4:165	0.066	0.051	1:531	2.903	1:067	0.070	0.528	0.313	N.L. (2)
310.5	28.5	4	0.70		12	35	30	0.055	1:668	5:405	5:405	0.075	0.055	1:668	3:197	1:050	0.079	0.499	0.299	N.L. (2)
308	31	3					0.051	1:796	3:953	3:953	0.065	0.051	1:796	3:480	1:034	0.067	0.475	0.288	0.233 (C)	
305.5	33.5	7					0.058	1:941	8:961	8:961	0.104	0.058	1:941	3:781	1:020	0.106	0.456	0.278	0.381 (C)	
303	36	19					0.067	2:108	25:462	25:462	0.301	0.067	2:108	4:105	1:002	0.302	0.440	0.268	N.L. (3)	
300.5	38.5	25					0.069	2:281	33:800	33:800	5:241	0.069	2:281	4:433	0:973	5.098	0:427	0.260	N.L. (3)	
298	41	23					0.068	2:451	29:474	29:474	0:435	0.068	2:451	4:759	0:950	0.413	0:417	0.253	N.L. (3)	
295.5	43.5	27					0.070	2:626	34:319	34:319	2:887	0.070	2:626	5:090	0:921	-2.660	0:409	0.248	N.L. (3)	
293	46	31					0.071	2:803	39:019	39:019	0:085	0.071	2:803	5:424	0:894	0.076	0:403	0.244	N.L. (3)	
289	50	29					0.071	3:087	33:963	33:963	27:189	0.071	3:087	5:957	0:867	23.560	0:395	0.239	N.L. (3)	
284	55	39					0.073	3:452	45:117	45:117	0:239	0.073	3:452	6:634	0:823	0.197	0:390	0.234	N.L. (3)	

R D R
 1 12 0.85
 2 25
 3 1:60 25
 D D



LIQUEFACTION ANALYSIS

3 1979
350.00
8.00
8.00
0.481
7.5
0.00
60
8
M

EQ MAGNITUDE SCALING FACTOR
M 1.00

Avg. Shear Wave Velocity (top 40')
V_s = 366

PGA CALCULATOR

M 7.5
D 30
R M
PGA = 0.535

ELEV. OF SAMPLE (FT.)	BORING DATA						CONDITIONS DURING DRILLING						CONDITIONS DURING EARTHQUAKE						
	BORING DEPTH (FT.)	SPT VALUE (BLOWS)	UNCONF. STR., Q _u (TSF.)	% FINEs < #200	PLAST. INDEX (PI)	LIQUID LIMIT (LL)	MOIST. CONTENT. (%)	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	CORR. SPT N VALUE (N ₁) ₆₀	EQUIV. CLN. SAND SPT N VALUE (N ₁) _{60cs}	CRR RESIST. CRR _{7.5} CRR _{7.5}	EFFECTIVE UNIT WT. (KCF.)	VERT. STRESS (KSF.)	TOTAL OVER- BURDEN CORR. FACT. (Ks)	CORR. RESIST. CRR _{7.5} CRR	SOIL MASS PART. EQ INDUCED CSR	FACTOR OF SAFETY * CRR/CSR	
346.5	3.5	4	1:10	8	32	31	0.123	0.431	5.951	5.951	0.079	0.123	0.431	0.431	1.389	0.110	0.942	0.294	N.L. (1)
344	6	2	1:20	8	32	29	0.124	0.741	2.694	2.694	0.057	0.124	0.741	0.741	1.234	0.070	0.895	0.280	N.L. (1)
341.5	8.5	6	1:60	12	35	22	0.065	0.903	8.020	8.020	0.096	0.065	0.903	0.934	1.206	0.116	0.844	0.273	N.L. (2)
339	11	5	0.80	12	35	39	0.057	1.046	6.859	6.859	0.087	0.057	1.046	1.233	1.162	0.101	0.792	0.292	N.L. (2)
336.5	13.5	3	0.70	12	35	28	0.055	1.183	4:153	4:153	0.066	0.055	1:183	1:526	1:124	0.074	0.739	0.298	N.L. (2)
334	16	2	0.40	8	38	32	0.049	1:306	2.770	2.770	0.057	0.049	1:306	1:805	1:102	0.063	0.688	0.297	N.L. (2)
331.5	18.5	3	0.70	30	50	63	0.055	1:443	4:106	4:106	0.066	0.055	1:443	2.098	1:080	0.071	0.640	0.291	N.L. (2)
329	21	4	0.80	30	50	50	0.057	1:586	5:372	5:372	0.075	0.057	1:586	2:397	1:061	0.079	0.596	0.282	N.L. (2)
326.5	23.5	3	0.80	30	50	79	0.057	1:728	3:940	3:940	0.064	0.057	1:728	2:695	1:042	0.067	0.557	0.272	N.L. (2)
324	26	5	2.00	12	32	32	0.067	1:896	6:371	6:371	0.083	0.067	1:896	3:019	1:024	0.085	0.524	0.261	N.L. (2)
321.5	28.5	4	0.80	12	35	28	0.057	2:038	4.969	4.969	0.072	0.057	2:038	3:317	1:008	0.072	0.495	0.252	N.L. (2)
319	31	3	0.60	12	35	31	0.053	2:171	3:639	3:639	0.062	0.053	2:171	3:606	0:995	0.062	0.472	0.245	N.L. (2)
316.5	33.5	15					0.065	2:333	18:354	18:354	0:196	0.065	2:333	3:924	0:973	0.191	0:452	0.238	0.803 (D)
314	36	23					0.068	2:503	28:936	28:936	0:407	0.068	2:503	4:250	0:944	0:384	0:437	0.232	N.L. (3)
311.5	38.5	18					0.066	2:668	21:064	21:064	0:229	0.066	2:668	4:571	0:933	0:214	0:424	0.227	0.943 (D)
309	41	20					0.067	2:836	22:985	22:985	0:257	0.067	2:836	4:895	0:913	0:234	0:414	0.224	1.045 (D)
306.5	43.5	27					0.070	3:011	31:506	31:506	0:630	0:070	3:011	5:226	0:880	0:554	0:406	0.220	N.L. (3)
304	46	18					0:066	3:176	19:120	19:120	0:205	0:066	3:176	5:547	0:890	0:182	0:400	0.218	0.835 (D)
301.5	48.5	17					0:066	3:341	17:409	17:409	0:185	0:066	3:341	5:868	0:881	0:163	0:395	0.217	0.751 (D)
299	51	30					0:071	3:518	32:415	32:415	0:866	0:071	3:518	6:201	0:828	0:718	0:392	0.216	N.L. (3)
297	53	23					0:068	3:654	23:199	23:199	0:260	0:068	3:654	6:462	0:843	0:219	0:389	0.215	1.019 (D)
294.5	55.5	21					0:068	3:824	20:308	20:308	0:219	0:068	3:824	6:788	0:840	0:184	0:387	0.215	0.856 (D)
289.5	60.5	60					0:077	4:209	61:756	61:756	0:417	0:077	4:209	7:485	0:760	0:317	0:383	0.213	N.L. (3)

R D R
1 12 R 0.85
2 16 R 25
3 20 R
D 10



MILLENNIA PROFESSIONAL SERVICES

6439 Plymouth Avenue, Suite W-129, St. Louis, Missouri 63133 314-531-3981

□

Appendix E:

Estimated Pile Length Spreadsheets



MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses			
M_____	M_____	M_____	M_____
R_____	R_____	R_____	Dr_____
654 _____	564 _____	310 _____	*** _____

Metal Shell 16"Φ w/.312" walls

LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL						□ NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	□ FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
				SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)								
4.08	0.50	3		10.4		16.6				17	0	0	9	6
2.50	0.40	3		5.2	6.1	18.7				19	0	0	10	9
2.50	0.20	2		2.7	3.1	30.6				31	0	0	17	11
2.50	0.80	3		9.7	12.3	37.2				37	0	0	20	14
2.50	0.60	1		7.5	9.2	49.3				49	0	0	27	16
2.50	0.90	1		10.7	13.8	58.5				58	0	0	32	19
2.50	0.80	0		9.7	12.3	66.6				67	0	0	37	21
2.50	0.70	0		8.6	10.7	70.7				71	0	0	39	24
2.50	0.40	0		5.2	6.1	75.9				76	0	0	42	26
2.50	0.40	0		5.2	6.1	74.9				75	0	0	41	29
2.50	0	□□□□□d		0.0	0.0	92.5				93	0	0	51	31
2.50	2	□□□□□d		1.8	17.6	287.7				288	0	0	158	34
5.00	24	M□□□□d		46.3	210.9	316.4				316	0	0	174	39
5.00	22	M□□□□d		42.4	193.4	376.4				376	0	0	207	44
5.00	24	M□□□□d		46.3	210.9	308.4				308	0	0	170	49
5.00	11	M□□□□d		21.2	96.7	373.6				374	0	0	205	54
1.00	16	M□□□□d		6.2	140.6	379.7				380	0	0	209	55
4.00	16	M□□□□d		24.7	140.6	457.2				457	0	0	251	59
5.00	22	M□□□□d		42.4	193.4	429.3				429	0	0	236	64
5.00	14	M□□□□d		27.0	123.1	473.8				474	0	0	261	69
1.00	16	M□□□□d		6.2	140.6	480.0				480	0	0	264	70
1.00	16	M□□□□d		6.2	140.6	486.2				486	0	0	267	71
1.00	16	M□□□□d		6.2	140.6	492.3				492	0	0	271	72
1.00	16	M□□□□d		6.2	140.6	498.5				499	0	0	274	73
1.00	16	M□□□□d		6.2	140.6	495.9				496	0	0	273	74
1.00	15	M□□□□d		5.8	131.8	501.7				502	0	0	276	75
1.00	15	M□□□□d		5.8	131.8	507.5				507	0	0	279	76
1.00	15	M□□□□d		5.8	131.8	513.2				513	0	0	282	77
1.00	15	M□□□□d		5.8	131.8	519.0				519	0	0	285	78
1.00	15	M□□□□d		5.8	131.8	533.6				534	0	0	293	79
1.00	16	M□□□□d		6.2	140.6	539.8				540	0	0	297	80
1.00	16	M□□□□d		6.2	140.6	545.9				546	0	0	300	81
1.00	16	M□□□□d		6.2	140.6	552.1				552	0	0	304	82
1.00	16	M□□□□d		6.2	140.6	558.3				558	0	0	307	83
1.00	16	M□□□□d		6.2	140.6	564.5				564	0	0	310	84

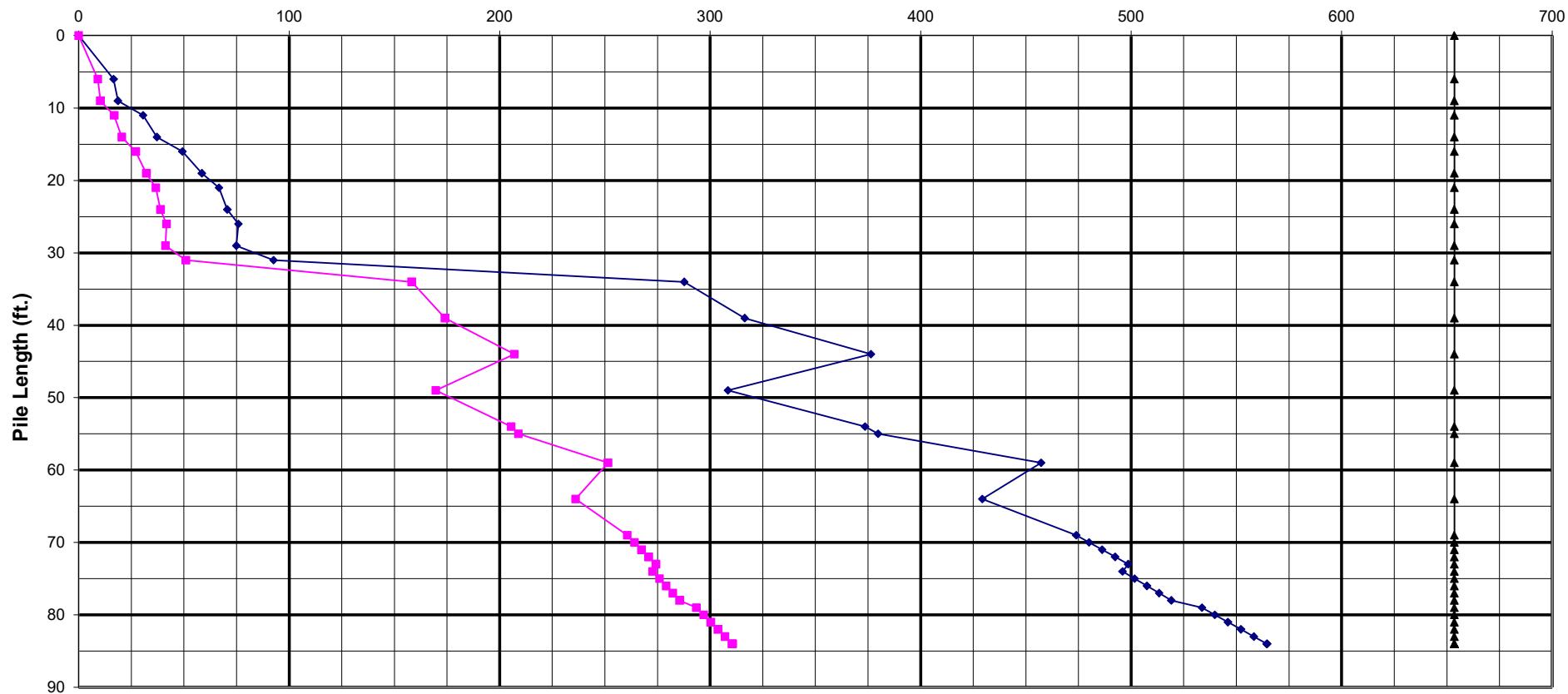
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

NOMINAL REQ'D BEARING

FACTORED RESISTANCE AVAILABLE

Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



Pile Design Table for South Abutment utilizing Boring #2-S 201



<u>MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses</u>			
M_____	M_____	M_____	M_____
Ro_____d	Ro_____d	Ro_____d	Dr_____r
654 _____	564 _____	413 _____	*** _____

Metal Shell 16"Φ w/.312" walls

LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	ULTIMATE						NOMINAL REQ'D BEARING (KIPS)	NOMINAL GEOTECH. LOSS FROM LIQUEF. & DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	SEISMIC RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
				SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)								
4.08	0.50	3		10.4		16.6				17	10	11	.5	6
2.50	0.40	3		5.2	6.1	18.7				19	16	17	14	9
2.50	0.20	2		2.7	3.1	30.6				31	18	20	8	11
2.50	0.80	3		9.7	12.3	37.2				37	28	31	22	14
2.50	0.60	1		7.5	9.2	49.3				49	36	39	25	16
2.50	0.90	1		10.7	13.8	58.5				58	46	51	39	19
2.50	0.80	0		9.7	12.3	66.6				67	56	62	51	21
2.50	0.70	0		8.6	10.7	70.7				71	65	71	65	24
2.50	0.40	0		5.2	6.1	75.9				76	70	77	71	26
2.50	0.40	0		5.2	6.1	74.9				75	75	77	77	29
2.50	0	□□□□□d		0.0	0.0	92.5				93	75	77	59	31
2.50	2	□□□□□d		1.8	17.6	287.7				288	75	77	136	34
5.00	24	M□□□□d		46.3	210.9	316.4				316	75	77	165	39
5.00	22	M□□□□d		42.4	193.4	376.4				376	75	77	225	44
5.00	24	M□□□□d		46.3	210.9	308.4				308	75	77	157	49
5.00	11	M□□□□d		21.2	96.7	373.6				374	75	77	222	54
1.00	16	M□□□□d		6.2	140.6	379.7				380	75	77	228	55
4.00	16	M□□□□d		24.7	140.6	457.2				457	75	77	305	59
5.00	22	M□□□□d		42.4	193.4	429.3				429	75	77	277	64
5.00	14	M□□□□d		27.0	123.1	473.8				474	75	77	322	69
1.00	16	M□□□□d		6.2	140.6	480.0				480	75	77	328	70
1.00	16	M□□□□d		6.2	140.6	486.2				486	75	77	334	71
1.00	16	M□□□□d		6.2	140.6	492.3				492	75	77	341	72
1.00	16	M□□□□d		6.2	140.6	498.5				499	75	77	347	73
1.00	16	M□□□□d		6.2	140.6	495.9				496	75	77	344	74
1.00	15	M□□□□d		5.8	131.8	501.7				502	75	77	350	75
1.00	15	M□□□□d		5.8	131.8	507.5				507	75	77	356	76
1.00	15	M□□□□d		5.8	131.8	513.2				513	75	77	361	77
1.00	15	M□□□□d		5.8	131.8	519.0				519	75	77	367	78
1.00	15	M□□□□d		5.8	131.8	533.6				534	75	77	382	79
1.00	16	M□□□□d		6.2	140.6	539.8				540	75	77	388	80
1.00	16	M□□□□d		6.2	140.6	545.9				546	75	77	394	81
1.00	16	M□□□□d		6.2	140.6	552.1				552	75	77	400	82
1.00	16	M□□□□d		6.2	140.6	558.3				558	75	77	406	83
1.00	16	M□□□□d		6.2	140.6	564.5				564	75	77	413	84



IDOT STATIC METHOD OF ESTIMATING PILE LENGTH

4/17/2021	R.D.	336.30	334.30	331.30

R.D. 336.30	R.D. 334.30	R.D. 331.30

Metal Shell 16"Φ w/.312" walls	4189	1396

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M	M	M	M
Rod	Rod	Rod	Dr.
654	510	279	***

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)	
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)						
332.00	2.30	0.30			3.7		9.8					3	4
329.50	2.50	0.40			5.2	6.1	15.0					6	7
327.00	2.50	0.40			5.2	6.1	21.7					10	9
324.50	2.50	0.50			6.4	7.7	25.0					12	12
322.00	2.50	0.30			4.0	4.6	29.0					14	14
319.50	2.50	0.30			4.0	4.6	33.0					16	17
317.00	2.50	0.30			4.0	4.6	33.9					17	19
314.50	2.50	0.10			1.4	1.5	35.3					17	22
312.00	2.50	0.10			1.4	1.5	43.9					22	24
309.50	2.50	1		Mud	1.0	8.8	115.2					61	27
307.00	2.50	9		Mud	8.7	79.1	176.6					95	29
304.50	2.50	15		Mud	14.5	131.8	164.7					89	32
299.50	5.00	12		Mud	23.1	105.5	231.8					125	37
297.00	2.50	17		Mud	16.4	149.4	274.5					149	39
294.50	2.50	20		Mud	19.3	175.8	346.5					189	42
289.50	5.00	26		Mud	50.2	228.5	352.8					192	47
284.50	5.00	21		Mud	40.5	184.6	375.7					205	52
279.50	5.00	19		Mud	36.6	167.0	403.6					220	57
274.50	5.00	18		Mud	34.7	158.2	385.6					210	62
269.50	5.00	12		Mud	23.1	105.5	452.6					247	67
264.50	5.00	17		Mud	32.8	149.4	432.7					236	72
259.50	5.00	11		Mud	21.2	96.7	489.1					267	77
254.50	5.00	15		Mud	28.9	131.8	491.6					268	82
250.50	4.00	12		Mud	18.5	105.5	510.1					279	86
249.50	1.00	12		Mud		105.5							

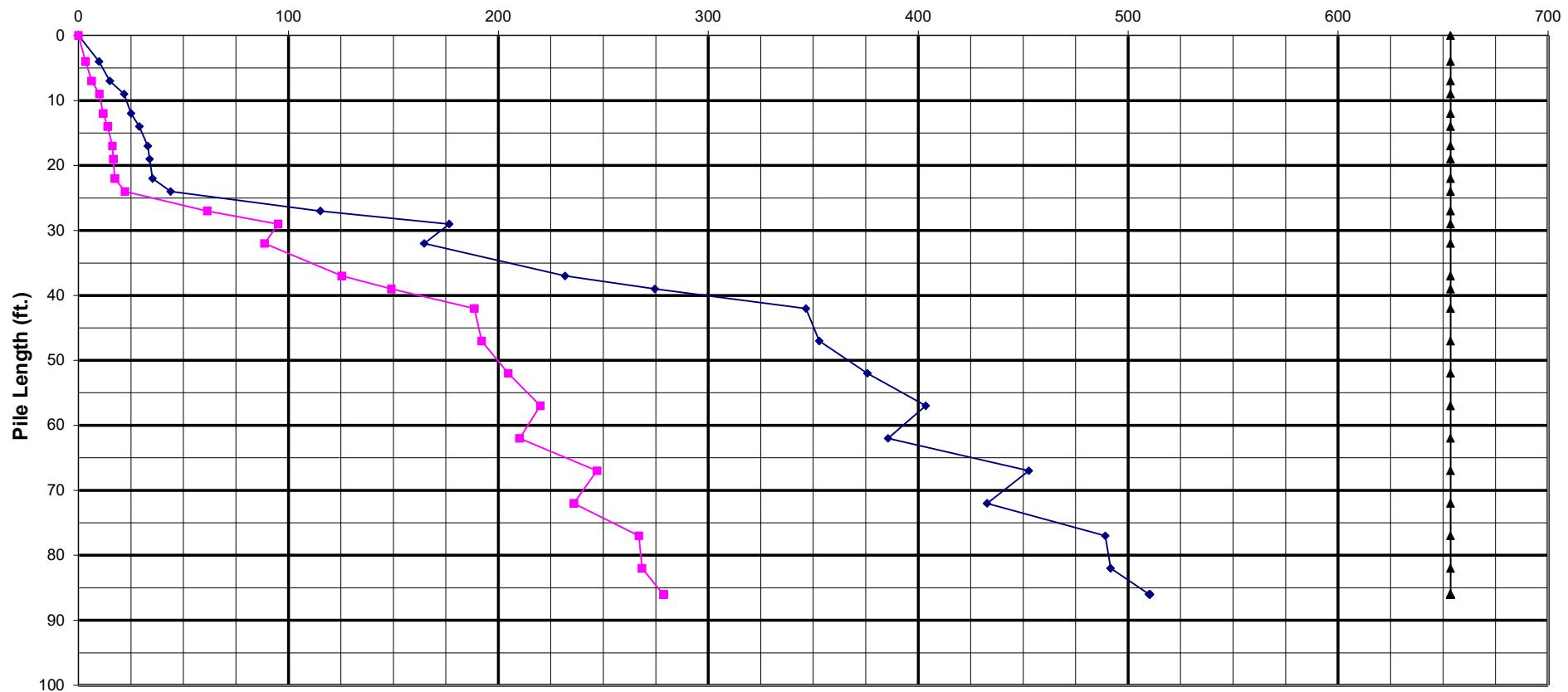
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

NOMINAL REQ'D BEARING

FACTORED RESISTANCE AVAILABLE

Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



Pile Design Table for Pier 1 utilizing Boring #4-S 2021

	R R R	R R R	R R R	Steel HP 10 X 42	Steel HP 12 X 84		R R R	R R R	R R R	Steel HP 14 X 73	Steel HP 14 X 89		R R R	R R R	R R R	Steel HP 12 X 63	Steel HP 14 X 102	Steel HP 14 X 117	Steel HP 14 X 117		
Metal Shell 12"Ø w.25" walls	6	2	4	3	1	4	4	1	4	4	1	4									
	10	4	7	7	3	7	9	4	7	9	4	7									
	15	7	9	11	5	9	14	6	9	17	5	12									
	18	8	12	12	13	6	20	10	14	22	11	19									
	21	10	14	15	7	14	20	10	14	22	11	19									
	24	12	17	17	8	17	22	11	19	23	11	22									
	26	12	19	17	8	19	23	11	22	25	12	24									
	28	13	22	18	9	22	26	12	24	26	12	24									
	31	16	24	19	10	24	28	18	27	28	18	27									
	72	38	27	28	14	27	41	21	29	44	23	32									
	104	56	32	31	16	29	52	28	37	58	31	39									
	146	79	37	34	18	32	66	35	42												
	173	94	39	40	21	37															
	217	118	42	45	24	39															
	230	125	47	51	27	42	78	42	47												
	250	136	52	61	33	47	89	47	52												
	269	147	62	70	37	52	98	53	57												
	305	167	72	76	41	62	101	54	62												
	342	187	77	83	45	72	105	59	72												
	349	190	82	82	50	62	121	65	82												
	363	198	86	86	52	86	125	67	86												
Metal Shell 14"Ø w.25" walls	8	3	4	3	1	4	5	1	4	5	1	4									
	12	5	7	7	3	7	10	4	7	10	4	7									
	18	8	9	11	5	9	16	7	9	16	7	9									
	21	10	12	13	6	12	20	10	12	23	11	14									
	25	12	14	16	7	14	25	12	19	25	12	19									
	28	14	17	18	9	17	26	13	22	26	13	22									
	29	14	19	18	9	19	26	13	22	29	14	24									
	31	15	22	20	10	24	41	21	32	41	21	32									
	37	19	24	34	17	27	42	22	37	42	22	37									
	92	49	27	38	20	29	48	25	29	48	25	29									
	133	71	32	41	21	32	51	27	32	51	27	32									
	186	101	37	49	25	37	61	32	37	61	32	37									
	221	120	39	42	22	37	66	35	39	66	35	39									
	278	151	42	47	25	39	74	39	42	74	39	42									
	289	157	47	53	28	42	89	48	47	89	48	47									
	311	169	52	63	34	47	102	54	52	102	54	52									
	326	177	62	72	38	52	113	61	57	113	61	57									
	368	201	72	78	42	62	121	65	62	121	65	62									
	414	226	77	85	46	72	130	70	72	130	70	72									
	419	228	82	95	51	82	144	78	82	144	78	82									
	435	237	86	98	53	86	148	80	86	148	80	86									
Metal Shell 14"Ø w.312" walls	8	3	4	4	1	4	5	1	4	5	1	4									
	12	5	7	8	3	7	11	4	7	11	4	7									
	18	8	9	9	6	9	16	8	9	16	8	9									
	21	10	12	17	8	12	21	10	12	21	10	12									
	25	12	14	19	9	14	24	12	14	24	12	14									
	28	14	17	21	11	22	25	13	19	25	13	19									
	29	14	19	22	11	22	26	13	22	26	13	22									
	31	15	22	24	12	24	29	14	24	29	14	24									
	37	19	24	34	17	27	42	22	37	42	22	37									
	92	49	27	38	20	29	48	25	29	48	25	29									
	133	71	32	41	21	32	51	27	32	51	27	32									
	186	101	37	49	25	37	51	27	37	51	27	37									
	221	120	39	42	22	37	56	35	39	56	35	39									
	278	151	42	47	25	39	74	39	42	74	39	42									
	289	157	47	53	28	42	89	48	47	89	48	47									
	311	169	52	63	34	47	102	54	52	102	54	52									
	326	177	62	72	38	52	113	61	57	113	61	57									
	368	201	72	78	42	62	121	65	62	121	65	62									
	414	226	77	85	46	72	130	70	72	130	70	72									
	419	228	82	95	51	82	144	78	82	144	78	82									
	435	237	86	98	53	86	148	80	86	148	80	86									
Metal Shell 16"Ø w.375" walls	10	3	4	4	1	4	5	1	4	5	1	4									
	15	6	7	9	4	7	11	4	7	11	4	7									
	22	10	9	14	6	9	17	8	9	17	8	9									
	25	12	12	17	8	12	21	10	12	21	10	12									
	29	14	14	19	9	14	24	12	14	24	12	14									
	33	16	17	21	11	19	26	13	19	26	13	19									
	34	17	19	22	11	22	27	13	22	27	13	22									
	35	17	22	24	12	24	30	15	24	30	15	24									
	44	22	24	35	18	27	44	23	27	44	23	27									
	115	61	27	40	21	29	50	26	29	50	26	29									
	165	89	32	43	22	32	53	28	32	53	28	32									
	232	125	37	51	27	37	64	34	37	64	34	37									
	275	149	39	57	36	39	71	38	39	71	38	39									
	347	189	42	65	34	42	81	43	42	81	43	42									
	353	192	47	77	41	47	95	51	47	95	51	47									
	376	205	52	87	47	52	108	58	52	108	58	52									
	388	210	62	97	52	57	120	64	57	120	64	57									
	433	236	72	99	53	62	126	68	62	126	68	62									
	469	267	77	107	56	72	135	73	72	135	73	72									
	462	268	82	119	64	82	150	81	82	150	81	82									
	510	279	86	123	66	86	155	84	86	155											

4/17/2021
336.30
334.30
299.50
312.00

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M _{req'd} (kips)	M _{req'd} (kips)	M _{req'd} (kips)	M _{req'd} (kips)
654	510	389	***

M_{req'd} = Required resistance
 R_{req'd} = Required bearing
 M_{loss} = Maximum loss from liquefaction & DD
 Drilled = Drilled pile
 DD = Design Depth
 SPT = Standard Penetration Test
 N = SPT value
 TSF = Unconfined Compressive Strength
 Thick = Thickness
 Elev. = Elevation
 Bot. of Layer = Bottom of layer
 Geotech. = Geotechnical
 Seismic = Seismic
 Factor = Factor of safety
 Available = Available resistance
 Estimated = Estimated pile length

Metal Shell 16"Φ w/.312" walls

4189 1396

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	ULTIMATE			NOMINAL REQ'D BEARING (KIPS)	NOMINAL GEOTECH. LOSS FROM LIQUEF. & DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	SEISMIC RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
332.00	2.30	0.30			3.7		9.8	10	4	4	2	4
329.50	2.50	0.40			5.2	6.1	15.0	15	9	10	4	7
327.00	2.50	0.40			5.2	6.1	21.7	22	14	15	8	9
324.50	2.50	0.50			6.4	7.7	25.0	25	20	23	18	12
322.00	2.50	0.30			4.0	4.6	29.0	29	24	27	22	14
319.50	2.50	0.30			4.0	4.6	33.0	33	28	31	27	17
317.00	2.50	0.30			4.0	4.6	33.9	34	32	36	34	19
314.50	2.50	0.10			1.4	1.5	35.3	35	34	37	36	22
312.00	2.50	0.10			1.4	1.5	43.9	44	35	39	30	24
309.50	2.50	1		M _{req'd} 115.2	1.0	8.8	115.2	115	36	39	40	27
307.00	2.50	9		M _{req'd} 176.6	8.7	79.1	176.6	177	45	39	93	29
304.50	2.50	15		M _{req'd} 164.7	14.5	131.8	164.7	165	59	39	67	32
299.50	5.00	12		M _{req'd} 231.8	23.1	105.5	231.8	232	82	39	111	37
297.00	2.50	17		M _{req'd} 274.5	16.4	149.4	274.5	275	82	39	153	39
294.50	2.50	20		M _{req'd} 346.5	19.3	175.8	346.5	347	82	39	226	42
289.50	5.00	26		M _{req'd} 352.8	50.2	228.5	352.8	353	82	39	232	47
284.50	5.00	21		M _{req'd} 375.7	40.5	184.6	375.7	376	82	39	255	52
279.50	5.00	19		M _{req'd} 403.6	36.6	167.0	403.6	404	82	39	283	57
274.50	5.00	18		M _{req'd} 385.6	34.7	158.2	385.6	386	82	39	265	62
269.50	5.00	12		M _{req'd} 452.6	23.1	105.5	452.6	453	82	39	332	67
264.50	5.00	17		M _{req'd} 432.7	32.8	149.4	432.7	433	82	39	312	72
259.50	5.00	11		M _{req'd} 489.1	21.2	96.7	489.1	489	82	39	368	77
254.50	5.00	15		M _{req'd} 491.6	28.9	131.8	491.6	492	82	39	371	82
250.50	4.00	12		M _{req'd} 510.1	18.5	105.5	510.1	510	82	39	389	86
249.50	1.00	12				105.5						

Drill 2	
3/20/2021	
R.D.	327.00
D.R.	325.00
M.D.	319.40

R.D. = Drill Depth
 D.R. = Drill Recovery
 M.D. = Material Displaced

Metal Shell 16"Φ w/.312" walls

4189	000
1396	000 000

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M <small>aximum</small> R <small>equired</small> B <small>earing</small> (kips)	M <small>aximum</small> R <small>equired</small> R <small>esistance</small> (kips)	M <small>aximum</small> R <small>equired</small> S <small>oil</small> L <small>oss</small> (kips)	M <small>aximum</small> D <small>riven</small> S <small>oil</small> L <small>oss</small> (kips)
654	598	323	89

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
322.50	2.50	0.30			4.0	7.7	11.6	12	2	0	4	5
320.00	2.50	0.50			6.4	18.0		18	6	0	4	7
317.50	2.50	0.50			6.4	21.3		21	6	0	6	10
315.00	2.50	0.30			4.0	4.6	25.3	25	6	0	8	12
312.50	2.50	0.30			4.0	4.6	51.1	51	6	0	22	15
310.00	2.50		3	□□□□d	2.7	26.4	32.0	32	6	0	12	17
307.50	2.50	0.30			4.0	4.6	136.9	137	6	0	70	20
305.00	2.50		12	M□d□□□d	11.6	105.5	86.9	87	6	0	42	22
302.50	2.50		5	M□d□□□d	4.8	43.9	82.9	83	6	0	40	25
300.00	2.50		4	M□d□□□d	3.9	35.2	227.4	227	6	0	119	27
297.50	2.50		20	M□d□□□d	19.3	175.8	237.9	238	6	0	125	30
292.50	5.00		19	M□d□□□d	36.6	167.0	300.9	301	6	0	160	35
287.50	5.00		22	M□d□□□d	42.4	193.4	352.1	352	6	0	188	40
282.50	5.00		23	M□d□□□d	44.3	202.2	484.4	484	6	0	261	45
277.50	5.00		33	M□d□□□d	69.7	290.1	387.1	387	6	0	207	50
272.50	5.00		14	M□d□□□d	27.0	123.1	352.5	353	6	0	188	55
267.50	5.00		7	M□d□□□d	13.5	61.5	471.5	472	6	0	254	60
262.50	5.00		19	M□d□□□d	36.6	167.0	464.2	464	6	0	250	65
257.50	5.00		14	M□d□□□d	27.0	123.1	464.8	465	6	0	250	70
254.50	3.00		11	M□d□□□d	12.7	96.7	477.5	478	6	0	257	73
253.50	1.00		11	M□d□□□d	4.2	96.7	525.7	526	6	0	283	74
252.50	1.00		16	M□d□□□d	6.2	140.6	531.9	532	6	0	287	75
251.50	1.00		16	M□d□□□d	6.2	140.6	538.1	538	6	0	290	76
250.50	1.00		16	M□d□□□d	6.2	140.6	544.2	544	6	0	294	77
249.50	1.00		16	M□d□□□d	6.2	140.6	550.4	550	6	0	297	78
248.50	1.00		16	M□d□□□d	6.2	140.6	556.6	557	6	0	300	79
247.50	1.00		16	M□d□□□d	6.2	140.6	562.8	563	6	0	304	80
242.50	5.00		16	M□d□□□d	30.9	140.6	576.0	576	6	0	311	85
238.50	4.00		14	M□d□□□d	21.6	123.1	597.6	598	6	0	323	89
237.50	1.00		14	M□d□□□d	5.4	123.1	682.1	682	6	0	369	99
235.50	2.00		23	M□d□□□d	17.7	202.2	699.9	700	6	0	379	92
232.50	3.00		23	M□d□□□d	26.6	202.2	726.5	726	6	0	394	95
229.00	3.50		23	M□d□□□d	31.0	202.2	994.8	995	6	0	541	98
228.00	1.00				263.7	439.5	1258.5	1259	6	0	686	99
227.00	1.00				263.7	439.5	1522.2	1522	6	0	832	100
226.00	1.00				263.7	439.5	1785.9	1786	6	0	977	101
225.00	1.00				263.7	439.5	2049.6	2050	6	0	1122	102
224.00	1.00					439.5						

Pile Design Table for Pier 2 utilizing Boring #3-S 2021

R e f e r e n c e s													
Metal Shell 12"Ø w/.25" walls													
7		5			1	5							
12	2	7	8	2	7								
15	4	10	11	3	10								
18	6	12	13	4	12								
23	8	17	16	6	17								
56	26	25	24	10	25								
138	71	27	31	14	27								
147	77	30	35	16	30								
160	100	35	44	21	35								
226	120	40	54	27	40								
263	135	55	66	34	45								
322	173	60	67	34	55								
325	175	65	86	44	70								
330	177	70	88	46	73								
340	183	73	97	51	74								
366	198	74	98	51	75								
373	201	75	98	52	76								
377	203	76	100	52	77								
382	208	77	101	53	78								
386	208	78	102	53	79								
391	211	79	103	54	80								
Metal Shell 14"Ø w/.25" walls													
9	3	5	108	56	89								
15	3	7	124	65	90								
18	5	10	127	67	92								
22	7	12	131	69	95								
26	10	17	151	81	98								
69	33	25	335	181	102								
180	94	27											
190	99	30	4	1	5								
242	128	35	9	2	7								
286	152	40	12	3	10								
302	161	55	14	5	12								
393	211	65	16	6	17								
396	215	70	25	11	25								
407	219	73	52	15	27								
445	240	74	37	17	30								
450	243	75	46	22	35								
455	246	76	56	28	40								
Metal Shell 14"Ø w/.312" walls													
9	3	5	88	46	70								
15	3	7	90	47	73								
18	5	10	100	52	74								
22	7	12	101	52	75								
26	10	17	102	53	76								
69	33	25	103	53	77								
180	94	27	104	54	78								
190	99	30	104	55	79								
242	128	35	105	55	80								
286	152	40	107	56	85								
302	161	55	110	58	89								
393	211	65	127	67	90								
396	215	70	128	67	90								
407	219	73	134	71	95								
445	240	74	157	84	98								
450	243	75	454	250	103								
455	246	76											
Steel HP 12 X 53													
461	248	77	4	1	5								
466	251	78	10	2	7								
472	254	79	14	4	10								
477	257	80	17	6	12								
489	265	85	50	7	17								
508	275	89	29	13	20								
Metal Shell 16"Ø w/.312" walls													
12		5	31	13	25								
18	4	7	37	17	27								
21	6	10	42	20	30								
25	8	12	53	26	35								
32	12	17	65	32	40								
83	40	25	80	40	45								
227	119	27	109	55	60								
238	125	30	110	57	73								
301	160	35	31	14	25								
352	198	40	40	20	34								
353	188	55	55	27	35								
484	250	65	67	33	40								
485	250	70	82	42	45								
478	257	73	84	43	55								
528	283	74	108	56	60								
532	287	75	109	56	70								
538	290	76	111	58	73								
544	294	77	126	66	78								
550	297	78	135	71	89								
557	300	79	137	66	76								
563	304	80	128	67	77								
576	311	85	129	68	78								
598	323	89	131	68	79								
682	369	90	132	69	80								
700	379	92	133	69	85								
726	394	95	137	72	89								
Steel HP 8 X 36													
1	5	103	98	50	54								
7	1	7	168	89	95								
9	2	10	188	100	98								
10	3	12	497	273	103								
12	4	17	17	6	12								
19	8	25	20	7	17								
25	11	27	10	2	7								
28	13	30	15	5	10								
36	17	35	17	6	12								
44	22	40	53	31	44								
53	27	55	32	14	25								
67	35	65	40	18	27								
67	35	70	40	18	27								
69	36	73	45	21	30								
75	39	74	56	27	35								
76	39	75	68	34	40								
77	40	76	85	43	45								
78	40	77	86	44	55								
78	41	78	110	57	60								
79	41	79	111	57	70								
80	42	80	113	59	73								
82	43	85	127	66	74								
84	44	89	128	67	75								
95	50	90	129	68	76								
98	51	92	130	68	77								
101	53	95	131	69	78								
123	55	98	133	69	79								
286	157	103	134	70	80								
			135	71	85								
			139	73	89								
			160	85	90								
			165	87	92								
			171	91	95								
			175	92</									

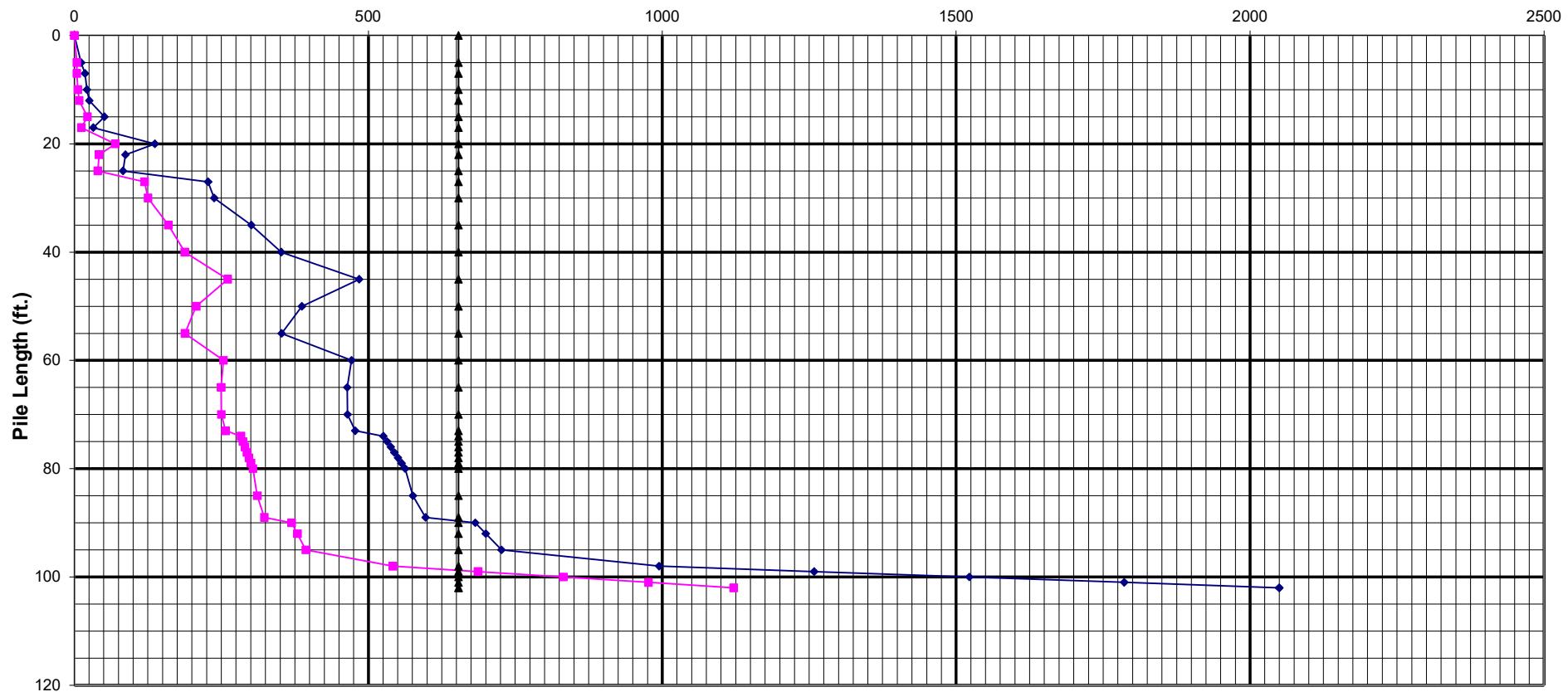
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

NOMINAL REQ'D BEARING

FACTORED RESISTANCE AVAILABLE

Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile





MAX. REQUIRED BEARING & RESISTANCE for Selected Pipe, Soil Profile, & Losses			
M ₁₀₀₀ 100 100 100	M ₁₀₀₀ 100 100 100	M ₁₀₀₀ 100 100 100	M ₁₀₀₀ 100 100
R ₁₀₀₀ 100 100 100	R ₁₀₀₀ 100 100 100	R ₁₀₀₀ 100 100 100	Dr ₁₀₀₀ 100 100
654 100	598 100	531 100	89 100

Metal Shell 16"Φ w/.312" walls

4189 000
1396 00000



IDOT STATIC METHOD OF ESTIMATING PILE LENGTH

333:30
331:30
328:60
328:60

4189
1396

Metal Shell 16"Φ w/.312" walls
4189
1396

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M <u>u</u> = 0.40	M <u>u</u> = 0.45	M <u>u</u> = 0.50	M <u>u</u> = 0.60
Rod 654	Rod 611	Rod 333	Rod 94

BOT. OF LAYER ELEV. (FT.)	LAYER THICK.	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
328:60	2.70	0.40			5.6		16.3	16	3	0	6	5
326:00	2.60	0.70			9.0	10.7	19.2	19	3	0	7	7
323:00	3.00	0.30			4.8	4.6	27.0	27	3	0	12	10
318:00	5.00	0.50			12.8	7.7	36.7	37	3	0	17	15
313:00	5.00	0.30			7.9	4.6	66.4	66	3	0	33	20
310:50	2.50		3	□□□□d	2.7	26.4	47.4	47	3	0	23	23
308:00	2.50	0.30			4.0	4.6	152.2	152	3	0	81	25
305:50	2.50		12	Mod. □□d	11.6	105.5	102.3	102	3	0	53	28
303:00	2.50		5	Mod. □□d	4.8	43.9	98.3	98	3	0	51	30
300:50	2.50		4	Mod. □□d	3.9	35.2	242.8	243	3	0	130	33
298:00	2.50		20	Mod. □□d	19.3	175.8	253.3	253	3	0	136	35
293:00	5.00		19	Mod. □□d	36.6	167.0	316.3	316	3	0	171	40
288:00	5.00		22	Mod. □□d	42.4	193.4	367.5	368	3	0	199	45
283:00	5.00		23	Mod. □□d	44.3	202.2	499.8	500	3	0	272	50
278:00	5.00		33	Mod. □□d	69.7	290.1	402.5	402	3	0	218	55
273:00	5.00		14	Mod. □□d	27.0	123.1	367.9	368	3	0	199	60
268:00	5.00		7	Mod. □□d	13.5	61.5	486.9	487	3	0	265	65
263:00	5.00		19	Mod. □□d	36.6	167.0	479.6	480	3	0	261	70
258:00	5.00		14	Mod. □□d	27.0	123.1	480.2	480	3	0	261	75
255:00	3.00		11	Mod. □□d	12.7	96.7	492.9	493	3	0	268	78
254:00	1.00		11	Mod. □□d	4.2	96.7	497.2	497	3	0	270	79
253:00	1.00		11	Mod. □□d	4.2	96.7	545.4	545	3	0	297	80
248:00	5.00		16	Mod. □□d	30.9	140.6	576.2	576	3	0	314	85
243:00	5.00		16	Mod. □□d	30.9	140.6	589.5	589	3	0	321	90
239:00	4.00		14	Mod. □□d	21.6	123.1	611.1	611	3	0	333	94
238:00	1.00		14	Mod. □□d	5.4	123.1	695.6	696	3	0	379	96
233:00	5.00		23	Mod. □□d	44.3	202.2	739.9	740	3	0	404	100
232:00	1.00		23	Mod. □□d	8.9	202.2	748.8	749	3	0	409	101
231:00	1.00		23	Mod. □□d	8.9	202.2	995.0	996	3	0	544	102
230:00	1.00			□□□	263.7	439.5	1258.7	1259	3	0	689	103.3
229:00	1.00			□□□	263.7	439.5	1522.4	1522	3	0	834	104.3
228:00	1.00			□□□	263.7	439.5	1786.0	1786	3	0	979	105.3
227:00	1.00			□□□	263.7	439.5	2049.7	2050	3	0	1124	106.3
226:00	1.00			□□□	263.7	439.5	2313.4	2343	3	0	1269	107.3
225:00	1.00			□□□		439.5						

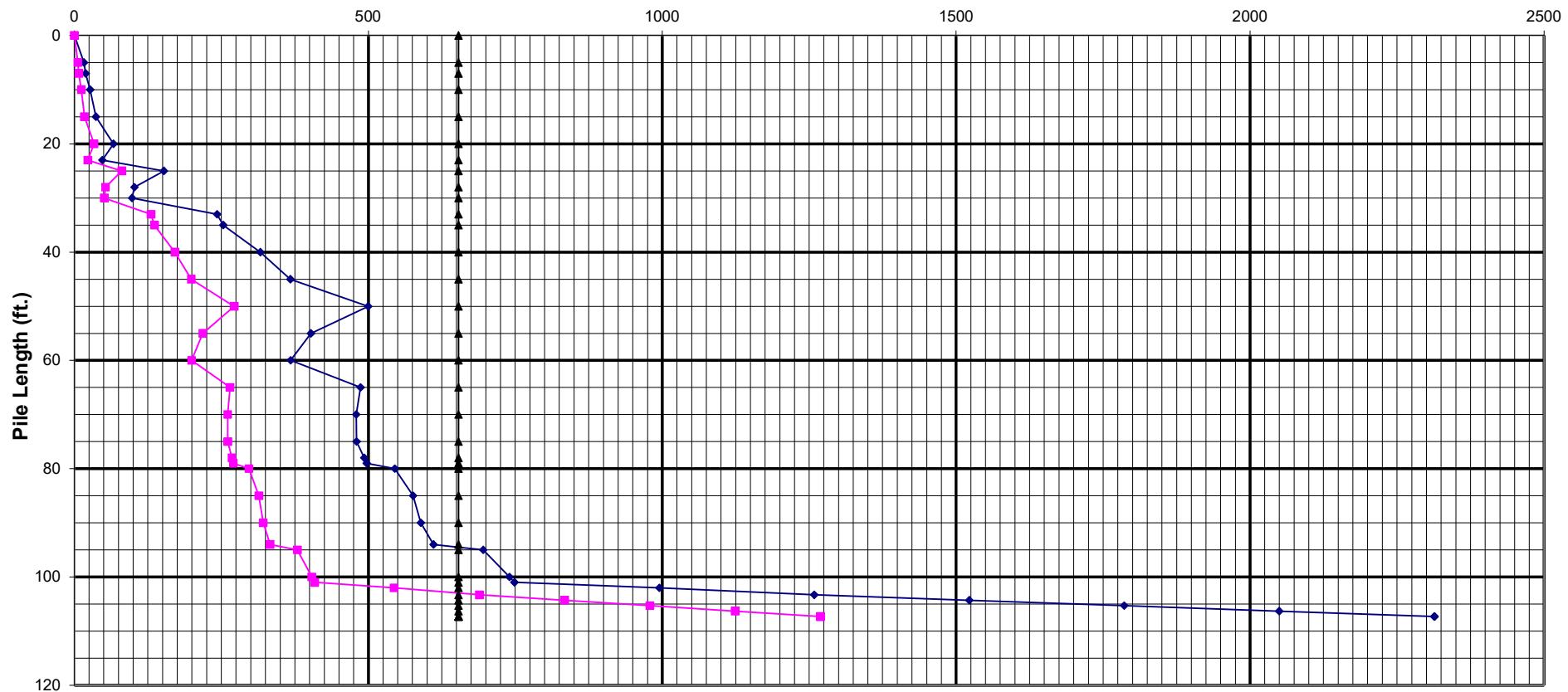
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

NOMINAL REQ'D BEARING

FACTORED RESISTANCE AVAILABLE

Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



Pile Design Table for Pier 3 utilizing Boring #3-S 2021

Steel HP 10 X 42			Steel HP 12 X 84		
10	3	5	5	1	5
14	5	7	10	4	7
19	8	10	15	6	10
27	12	15	19	9	15
35	17	23	23	11	23
67	35	30	32	16	30
149	80	23	42	22	33
159	85	35	46	24	35
201	108	40	55	29	40
238	128	45	65	34	45
266	143	60	75	40	60
334	181	65	94	50	75
337	183	70	96	51	78
342	186	75	96	51	79
352	191	78	106	56	80
355	193	79	110	56	85
383	208	80	112	60	90
Steel HP 10 X 57			Steel HP 14 X 73		
13	5	5	15	7	5
16	6	7	18	7	10
23	10	10	140	75	101
32	15	15	161	87	102
41	20	23	335	183	106
82	42	30	Steel HP 10 X 57		
193	104	33	5	1	5
203	109	35	10	4	7
256	138	40	15	7	10
299	162	45	19	9	15
315	171	60	24	12	23
406	221	70	33	16	30
410	223	75	44	23	33
421	229	78	48	25	35
424	231	79	57	30	40
Metal Shell 14"Ø w.25" walls			Steel HP 14 X 73		
15	5	5	7	2	5
16	6	7	18	7	10
23	10	10	88	52	78
32	15	15	99	53	79
41	20	23	108	58	80
82	42	30	113	61	85
193	104	33	114	61	90
203	109	35	118	63	94
256	138	40	135	72	95
299	162	45	142	76	100
315	171	60	143	77	101
406	221	70	165	89	102
410	223	75	454	250	108
421	229	78	Steel HP 12 X 53		
424	231	79	57	30	40
462	251	80	13	5	7
489	266	85	18	8	10
502	274	90	24	11	15
521	284	94	29	14	23
Metal Shell 16"Ø w.312" walls			Steel HP 12 X 53		
16	6	5	40	20	30
19	7	7	51	26	33
27	12	10	66	35	40
37	17	15	78	41	45
47	23	23	93	49	60
98	51	30	117	63	75
243	130	33	120	64	78
253	136	35	130	64	79
316	171	40	131	72	80
368	199	45	139	75	85
368	199	60	140	75	90
480	261	70	144	77	94
480	261	75	167	90	95
493	268	78	176	95	100
497	270	79	177	96	101
545	297	80	193	104	102
576	314	85	418	228	106
589	321	90	Steel HP 12 X 63		
611	333	94	6	2	5
Metal Shell 16"Ø w.375" walls			13	5	7
16	6	5	18	8	10
19	7	7	24	11	15
27	12	10	29	14	23
37	17	15	40	20	30
47	23	23	52	27	33
98	51	30	57	30	35
243	130	33	68	36	40
253	136	35	81	42	45
316	171	40	94	50	60
368	199	45	118	63	75
368	199	60	121	65	78
480	261	70	122	65	79
480	261	75	135	72	80
493	268	78	141	76	85
497	270	79	142	76	90
545	297	80	146	78	94
576	314	85	169	91	95
589	321	90	177	96	100
611	333	94	179	97	101
666	379	95	186	107	102
740	404	100	407	271	107
749	409	101	Steel HP 12 X 74		
4	1	5	7	2	5
8	3	7	13	5	7
11	5	10	19	8	10
15	7	15	24	11	15
19	9	23	30	14	23
25	13	30	54	28	33
34	17	33	59	30	35
37	19	35	70	37	40
45	23	40	82	43	45
53	26	45	95	50	60
59	31	60	120	64	75
74	39	70	123	66	78
74	39	75	124	66	79
75	40	78	137	73	80
76	40	79	143	77	85
82	44	80	144	77	90
86	46	85	148	79	94
88	47	90	172	93	95
90	48	94	180	97	100
101	54	95	182	98	101
107	58	100	201	109	102
108	58	101	589	324	108
129	70	102	Steel HP 14 X 117		
286	156	107	8	2	5
Steel HP 8 X 36			17	7	7
4	1	5	23	10	10
8	3	7	30	14	15
11	5	10	36	18	23
15	7	15	52	26	30
19	9	23	67	35	33
25	13	30	87	46	40
34	17	33	102	54	45
37	19	35	118	63	60
45	23	40	151	81	75
53	26	45	154	82	76
59	31	60	155	83	79
74	39	70	174	93	80
74	39	75	181	97	90
75	40	78	186	100	94
76	40	79	213	115	95
82	44	80	227	123	100
86	46	85	230	124	101
88	47	90	250	135	102
90	48	94	929	511	108

301.30	301.30	301.00	316.00
331.30	331.30	301.00	316.00
333.30	333.30	301.00	316.00
333.30	333.30	301.00	316.00
333.30	333.30	301.00	316.00

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M <small>aximum</small> R <small>equired</small> B <small>earing</small> <small>(KIPS)</small>	M <small>aximum</small> R <small>equired</small> R <small>esistance</small> <small>(KIPS)</small>	M <small>aximum</small> R <small>equired</small> L <small>oss</small> <small>(KIPS)</small>	M <small>aximum</small> D <small>riven</small> L <small>oss</small> <small>(KIPS)</small>
654	611	512	94

Metal Shell 16"Φ w/.312" walls

4189 1396

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. (FT.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	ULTIMATE			NOMINAL REQ'D BEARING (KIPS)	NOMINAL GEOTECH. LOSS FROM LIQUEF. & DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	SEISMIC RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. (KIPS)	TOTAL RESIST. (KIPS)					
328.00	2.70	0.40			5.6	16.3		16	6	6	5	5
326.00	2.60	0.70			9.0	19.2		19	15	16	11	7
323.00	3.00	0.30			4.8	4.6	27.0	27	19	21	14	10
318.00	5.00	0.50			12.8	7.7	36.7	37	32	35	31	15
313.00	5.00	0.30			7.9	4.6	66.4	66	40	35	9	20
310.50	2.50		3	□□□□□d	2.7	26.4	47.4	47	43	35	31	23
308.00	2.50	0.30			4.0	4.6	152.2	152	47	35	70	25
305.50	2.50		12	□□□□□d	11.6	105.5	102.3	102	58	35	9	28
303.00	2.50		5	□□□□□d	4.8	43.9	98.3	98	63	35	0	30
300.50	2.50		4	□□□□□d	3.9	35.2	242.8	243	63	35	144	33
298.00	2.50		20	□□□□□d	19.3	175.8	253.3	253	63	35	155	35
293.00	5.00		19	□□□□□d	36.6	167.0	316.3	316	63	35	218	40
288.00	5.00		22	□□□□□d	42.4	193.4	367.5	368	63	35	269	45
283.00	5.00		23	□□□□□d	44.3	202.2	499.8	500	63	35	401	50
278.00	5.00		33	□□□□□d	69.7	290.1	402.5	402	63	35	304	55
273.00	5.00		14	□□□□□d	27.0	123.1	367.9	368	63	35	269	60
268.00	5.00		7	□□□□□d	13.5	61.5	486.9	487	63	35	388	65
263.00	5.00		19	□□□□□d	36.6	167.0	479.6	480	63	35	381	70
258.00	5.00		14	□□□□□d	27.0	123.1	480.2	480	63	35	382	75
255.00	3.00		11	□□□□□d	12.7	96.7	492.9	493	63	35	394	78
254.00	1.00		11	□□□□□d	4.2	96.7	497.2	497	63	35	399	79
253.00	1.00		11	□□□□□d	4.2	96.7	545.4	545	63	35	447	80
248.00	5.00		16	□□□□□d	30.9	140.6	576.2	576	63	35	478	85
243.00	5.00		16	□□□□□d	30.9	140.6	589.5	589	63	35	491	90
239.00	4.00		14	□□□□□d	21.6	123.1	611.1	611	63	35	512	94
238.00	1.00		14	□□□□□d	5.4	123.1	695.6	696	63	35	597	96
233.00	5.00		23	□□□□□d	44.3	202.2	739.9	740	63	35	641	100
232.00	1.00		23	□□□□□d	8.9	202.2	748.8	749	63	35	650	101
231.00	1.00		23	□□□□□d	8.9	202.2	995.0	996	63	35	896	102
230.00	1.00			□□□□□	263.7	439.5	1258.7	1259	63	35	1160	103.3
229.00	1.00			□□□□□	263.7	439.5	1522.4	1522	63	35	1424	104.3
228.00	1.00			□□□□□	263.7	439.5	1786.0	1786	63	35	1687	105.3
227.00	1.00			□□□□□	263.7	439.5	2049.7	2050	63	35	1951	106.3
226.00	1.00			□□□□□	263.7	439.5	2313.4	2343	63	35	2245	107.3
225.00	1.00			□□□□□		439.5						



MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses			
M_____	M_____	M_____	M_____
Re_____	Re_____	Re_____	Dr_____
654	531	292	***

Metal Shell 16"Φ w/.312" walls

□□□□□r□□r□□□□□□□□□□□□□□□□
□□□□□d□□r□□r□□□□□□□□□□□□□□

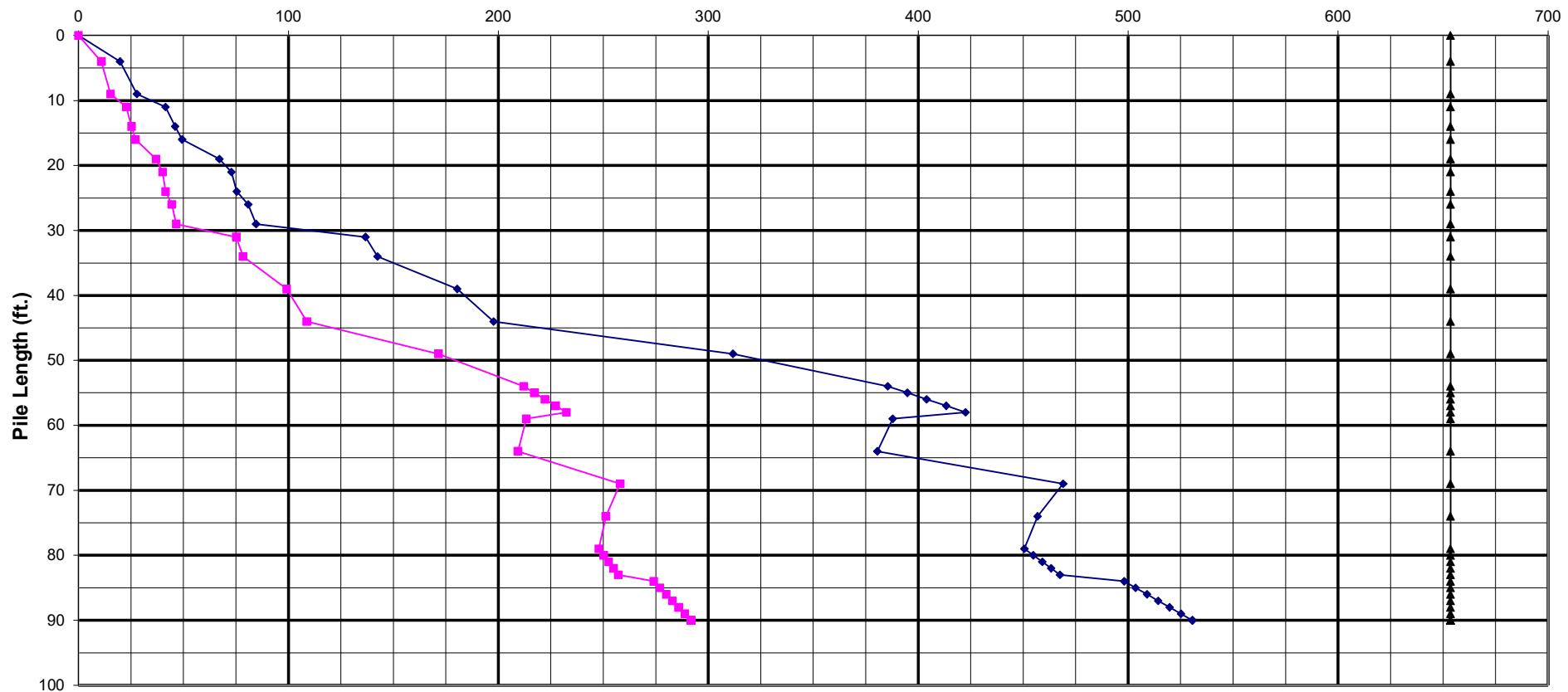
Pile Bearing vs. Estimated Length

Bearing Resistance (kips)

NOMINAL REQ'D BEARING

FACTORED RESISTANCE AVAILABLE

Maximum Bearing For Metal Shell 16"Φ w/.312" walls Pile



Pile Design Table for North Abutment utilizing Boring #1-S 2019

Steel HP 10 X 42				Steel HP 12 X 84			
R	H	D	T	R	H	D	T
Metal Shell 12"Ø w.25" walls							
4	4	4	4	4	4	5	4
21	11	9	9	14	8	9	10
29	16	14	11	22	12	11	15
33	18	14	11	24	13	14	17
35	20	16	13	25	14	15	19
47	26	19	16	36	20	19	47
53	29	21	16	38	21	21	49
56	34	24	16	38	21	24	53
63	33	26	16	41	23	26	54
63	34	26	16	43	24	29	58
93	51	31	16	52	29	31	68
97	51	34	16	53	29	34	69
120	66	39	16	60	33	39	83
133	73	44	16	63	35	44	99
201	110	49	16	78	43	49	112
250	134	54	16	88	54	54	143
257	141	55	16	90	50	55	117
260	143	59	16	92	51	56	119
324	184	64	16	93	51	64	126
317	174	66	16	103	57	79	123
318	175	74	16	104	57	80	123
320	176	79	16	104	57	81	134
323	179	80	16	105	59	82	137
326	179	81	16	106	58	83	135
329	181	82	16	111	61	84	136
333	183	83	16	112	62	85	137
351	195	84	16	113	62	85	148
355	195	85	16	114	63	87	147
359	197	86	16	115	63	88	148
363	198	86	16	116	64	89	149
367	202	88	16	116	64	90	150
371	204	89	16	116	64	90	151
375	206	90	16	116	64	90	152
Metal Shell 14"Ø w.25" walls							
4	4	4	4	8	4	4	8
16	9	9	9	14	9	9	14
24	13	9	9	22	12	11	31
35	19	11	9	25	14	14	31
39	23	14	14	26	14	16	31
43	23	16	14	31	20	19	37
57	31	19	19	39	21	21	54
63	34	21	19	39	22	24	54
65	36	24	19	42	23	26	57
70	39	26	19	44	24	29	57
73	40	26	19	54	30	31	53
73	40	29	19	55	30	34	53
114	51	31	19	56	30	34	51
119	65	34	19	56	34	39	52
149	82	39	19	80	44	49	99
164	90	44	19	90	50	54	114
204	139	49	19	93	51	55	130
314	173	54	19	95	52	56	131
319	176	64	19	95	52	64	134
324	181	70	19	100	58	79	137
387	213	80	19	106	58	80	140
391	215	81	19	107	59	81	141
395	217	82	19	107	59	82	147
399	220	83	19	108	59	83	147
422	232	84	19	114	63	84	159
427	235	85	19	115	63	85	160
432	238	86	19	116	64	86	161
437	239	87	19	117	64	87	162
441	243	88	19	117	65	88	173
446	245	89	19	118	65	89	174
451	248	90	19	118	66	90	179
Metal Shell 14"Ø w.312" walls							
16	9	4	4	9	5	4	8
24	13	9	9	17	10	9	17
35	20	11	9	26	14	11	19
39	22	14	14	30	17	14	21
43	23	16	16	31	17	16	21
57	31	19	19	48	25	19	31
63	36	21	19	47	26	24	38
65	36	24	19	51	28	26	21
70	39	26	19	53	29	29	38
75	40	26	19	85	36	31	55
114	43	31	19	57	37	34	57
119	65	34	19	76	42	39	62
149	82	39	19	79	44	44	64
164	94	44	19	84	45	49	83
254	139	49	19	105	58	54	83
314	173	54	19	108	59	55	96
319	176	64	19	111	61	66	100
324	181	70	19	111	62	67	114
387	213	80	19	116	64	58	131
391	215	81	19	117	64	59	134
395	219	82	19	117	65	64	137
399	219	83	19	128	71	79	140
422	232	84	19	129	71	80	143
427	235	85	19	130	71	81	144
432	238	86	19	131	72	82	147
437	240	87	19	131	72	83	160
441	243	88	19	131	72	84	161
446	245	89	19	141	77	85	162
451	248	90	19	143	78	87	164
Metal Shell 16"Ø w.312" walls							
20	11	4	4	9	5	4	8
28	15	9	9	17	10	9	18
41	23	11	9	26	14	11	20
46	25	14	14	32	16	14	21
49	27	16	16	45	20	16	32
67	37	19	16	53	36	31	44
73	40	21	16	67	37	34	58
75	41	24	16	77	42	39	63
91	44	26	16	80	44	44	85
455	250	80	16	98	53	49	83
459	252	81	16	108	59	54	94
463	255	82	16	111	61	55	100
468	258	83	16	111	62	56	102
498	274	84	16	116	64	57	118
504	277	85	16	118	65	58	133
509	280	86	16	118	65	58	134
514	283	87	16	119	66	60	139
518	283	87	16	130	71	79	139
520	286	88	16	130	72	80	142
525	289	89	16	131	72	81	145
529	291	90	16	132	72	82	148
Metal Shell 16"Ø w.375" walls							
20	11	4	4	9	5	4	8
28	15	9	9	17	10	9	18
41	23	11	11	27	15	11	21
46	25	14	14	32	16	14	22
49	27	16	16	45	20	16	32
67	37	19	16	145	80	88	180
73	40	21	16	147	81	90	180
75	41	24	16	177	82	90	180
91	44	26	16	180	84	92	180
458	253	83	16	191	85	93	181
468	274	84	16	198	84	93	182
504	277	85	16	110	60	54	84
509	280	86	16	113	62	55	85
514	283	87	16	114	63	56	86
518	286	88	16	118	65	57	103
520	288	88	16	120	66	58	120
525	289	89	16	121	66	58	121
529	292	90	16	132	72	79	139
533	293	91	16	133	73	80	142
541	293	91	16	134	74	82	143
546	294	92	16	135	74	83	148
551	295	93	16	136	74	83	148
556	296	94	16	136	74	83	148
561	297	95	16	137	74	84	148
566	298	95	16	146	81	87	166
571	39	54	16	147	81	88	167
576	40	64	16	148	82	89	167
581	41	64	16	149	82	90	168
586	42	34	16	150	82	90	168
591	42	34	16	151	82	90	168
596	42	34	16	152	82	90	168
601	42	34	16	153	82	90	168
606	42	34	16	154	82	90	168
611	42	34	16	155	82	90	168
616	42	34	16	156	82	90	168
621	42	34	16	157	82	90	168
626	26	39	16	158	82	90	168
631	35	49	16	159	82	90	168
636	35	49	16	160	82	90	168
641	39	54	16	161	82	90	168
646	43	84	16	162	82	90	168
651	48	84	16	163	82	90	168
656	48	85	16	164	82	90	168
661	48	85	16	165	82	90	168
666	48	85	16	166	82	90	168
671	49	87	16	167	82	90	168
676	50	88	16	168	82	90	168
681	50	88	16	169	82	90	168
686	50	89	16	170	82	90	168
691	50	89	16	171	82	90	168
696	50	90	16	172	82	90	168
701	51	90	16	173	82	90	168
706	51	90	16	174	82	90	168
711	51	90	16	175	82	90	168
716	51						



MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses

M	□□□□□	M	□□□□□	M	□□□□□	M	□□□□□
R	□□□□□	R	□□□□□	R	□□□□□	R	□□□□□
654	□□□□	531	□□□□	351	□□□□	Dr	□□□□□

The diagram illustrates the sequence of units in a neural network layer. It starts with a group of **M** units, followed by a group of **R** units, then a group of **D** units, and finally another group of **R** units. The units are represented by small squares, and they are grouped into larger rectangles. The first group of **R** units contains 8 units, and the second group contains 3 units.

Metal Shell 16"Φ w/.312" walls



MILLENNIA PROFESSIONAL SERVICES • Laboratory Test Results • Appendix F

6439 Plymouth Avenue, Suite W-129, St. Louis, Missouri 63133 314-531-3981

□

Appendix F:

Laboratory Test Results (2021 Borings)



Sieve Analysis Data Sheet

MID42263

Project Name: M1903409
Location: 145m
Boring No: 3
Sample Depth: 31325

Tested By: [Signature]
Checked By: [Signature]
Sample ID: 13

Date: 7/7/2021
Date: 7/8/2021

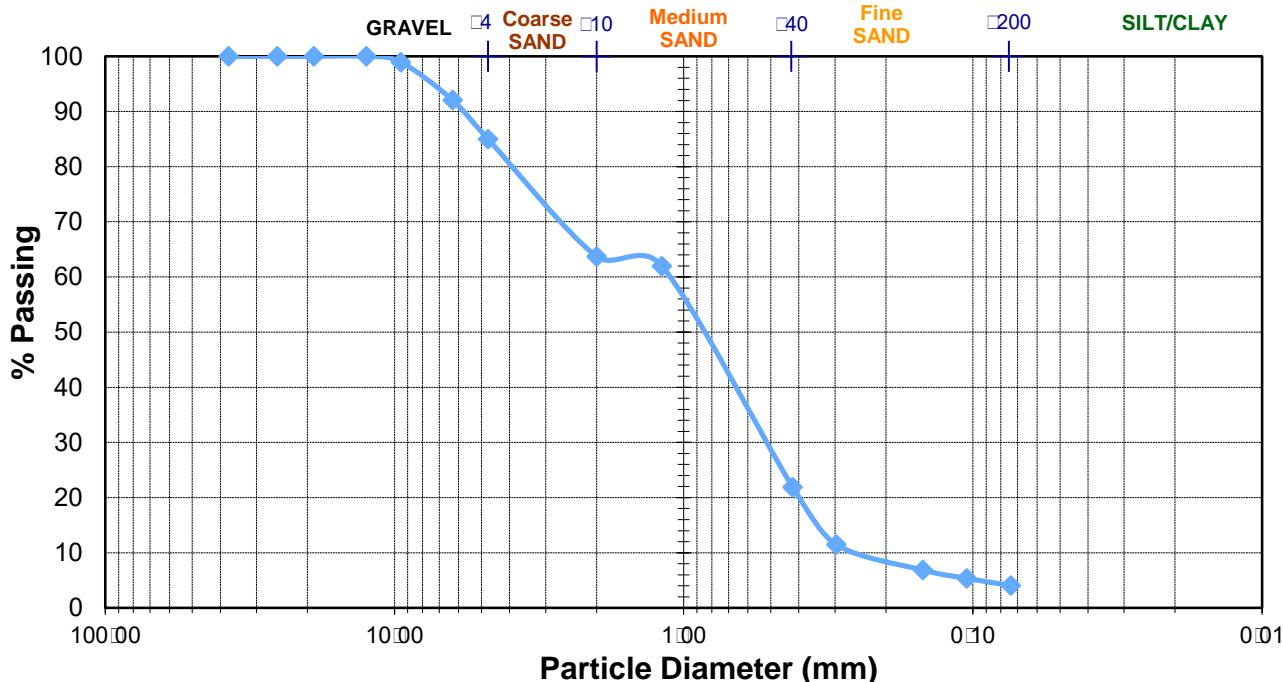
IDH Soil Classification: Sand with Gravel

Weight of Container (g): 152.3

Weight of Container & Soil (g): 349.5

Weight of Dry Sample (g): 196.8

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15	37.50			0.0	0.0	100.0
10	25.40			0.0	0.0	100.0
34	19.00			0.0	0.0	100.0
12	12.50			0.0	0.0	100.0
3.8	9.50	527.90	530	2.1	1.1	98.9
1.4	6.30	513.00	526.5	13.5	6.9	92.1
4	4.75	523.70	537.6	13.9	7.1	85.0
10	2.00	678.10	720	41.9	21.3	63.7
16	1.19	421.20	424.6	3.4	1.7	62.0
40	0.420	564.400	643.3	78.9	40.1	21.9
50	0.297	497.800	518.3	20.5	10.4	11.5
100	0.149	518.300	527.4	9.1	4.6	6.9
140	0.105	333.000	336	3.0	1.5	5.3
200	0.074	326.100	328.6	2.5	1.3	4.1
Pan		371.1	378.8	7.7	3.9	0.2
		TOTAL:	196.5	99.8		





Sieve Analysis Data Sheet

MID42263

Project Name: M1903409
Location: 145m from corner
Boring No: 3
Sample Depth: 36.375m

Tested By: [Signature]
Checked By: [Signature]
Sample ID: 15

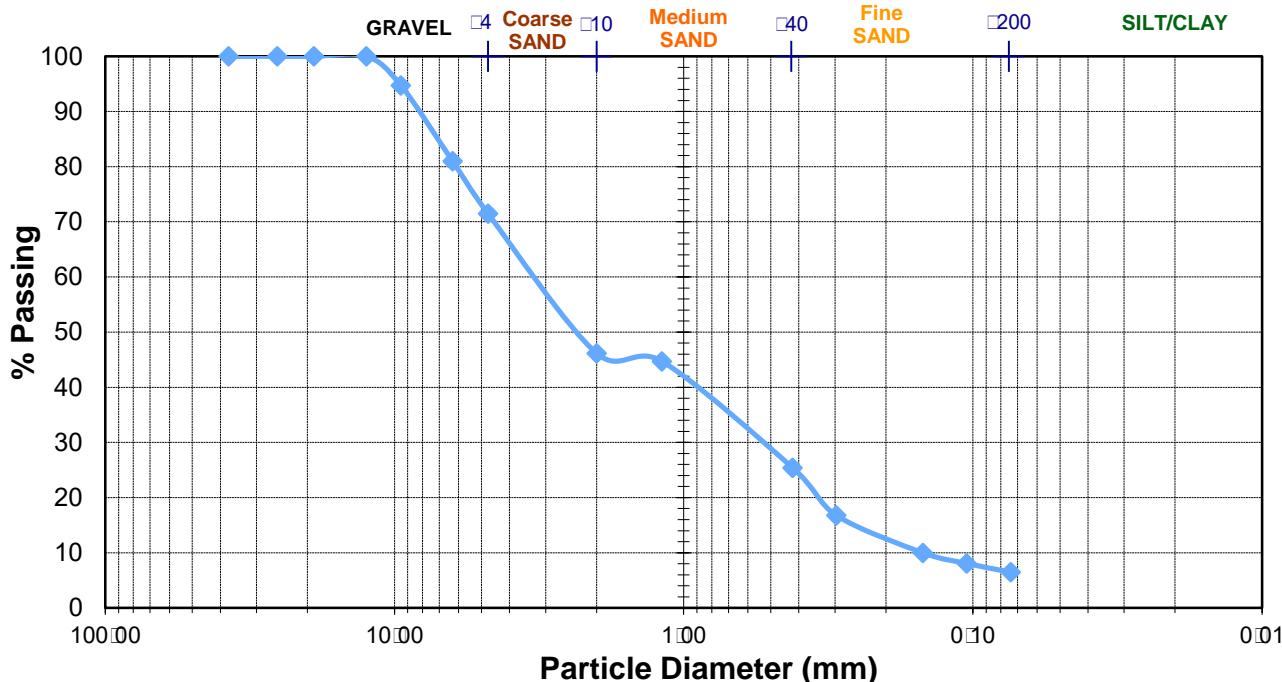
Date: 7/7/2021
Date: 7/8/2021

IDH Soil Classification: Sand with Gravel

Weight of Container (g): 151.2
Weight of Dry Sample (g): 159.0

Weight of Container & Soil (g): 310.2

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15	37.50			0.0	0.0	100.0
10	25.40			0.0	0.0	100.0
34	19.00			0.0	0.0	100.0
12	12.50			0.0	0.0	100.0
38	9.50	527.80	536.2	8.4	5.3	94.7
14	6.30	513.00	534.8	21.8	13.7	81.0
4	4.75	523.60	538.7	15.1	9.5	71.5
10	2.00	676.80	717.1	40.3	25.3	46.2
16	1.19	420.80	423.1	2.3	1.4	44.7
40	0.420	573.000	603.7	30.7	19.3	25.4
50	0.297	497.700	511.4	13.7	8.6	16.8
100	0.149	518.400	529.2	10.8	6.8	10.0
140	0.105	333.100	336.2	3.1	1.9	8.1
200	0.074	326.300	328.8	2.5	1.6	6.5
Pan		480.2	485.7	5.5	3.5	3.0
		TOTAL:		154.2	97.0	





Sieve Analysis Data Sheet

MID42263

Project Name: M1903409
Location: 145m from corner
Boring No: 3
Sample Depth: 48.5 to 50.0

Tested By: [Signature]
Checked By: [Signature]
Sample ID: 18

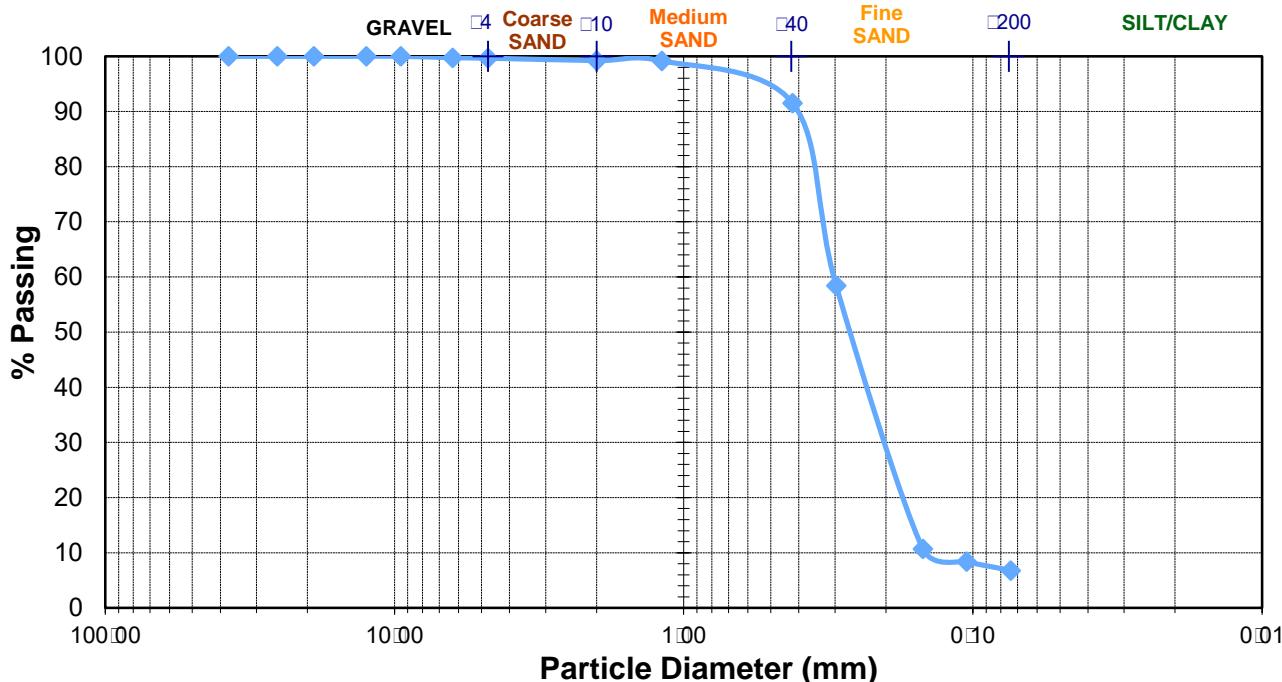
Date: 7/7/2021
Date: 7/8/2021

IDH Soil Classification: Fine Sand

Weight of Container (g): 151.2
Weight of Dry Sample (g): 206.2

Weight of Container & Soil (g): 357.4

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15	37.50			0.0	0.0	100.0
10	25.40			0.0	0.0	100.0
3.4	19.00			0.0	0.0	100.0
1.2	12.50			0.0	0.0	100.0
3.8	9.50			0.0	0.0	100.0
1.4	6.30	513.00	513.5	0.5	0.2	99.8
4	4.75	523.60	523.8	0.2	0.1	99.7
10	2.00	676.80	677.7	0.9	0.4	99.2
16	1.19	420.80	420.9	0.1	0.0	99.2
40	0.420	573.000	588.8	15.8	7.7	91.5
50	0.297	497.700	565.9	68.2	33.1	58.4
100	0.149	518.400	616.8	98.4	47.7	10.7
140	0.105	333.100	337.9	4.8	2.3	8.4
200	0.074	326.300	329.6	3.3	1.6	6.8
Pan		480.2	494	13.8	6.7	0.1
		TOTAL:		206	99.9	





Sieve Analysis Data Sheet

MID42263

Project Name: M1903409
Location: 145m
Boring No: 4
Sample Depth: 45m

Tested By:
Checked By:
Sample ID: 17

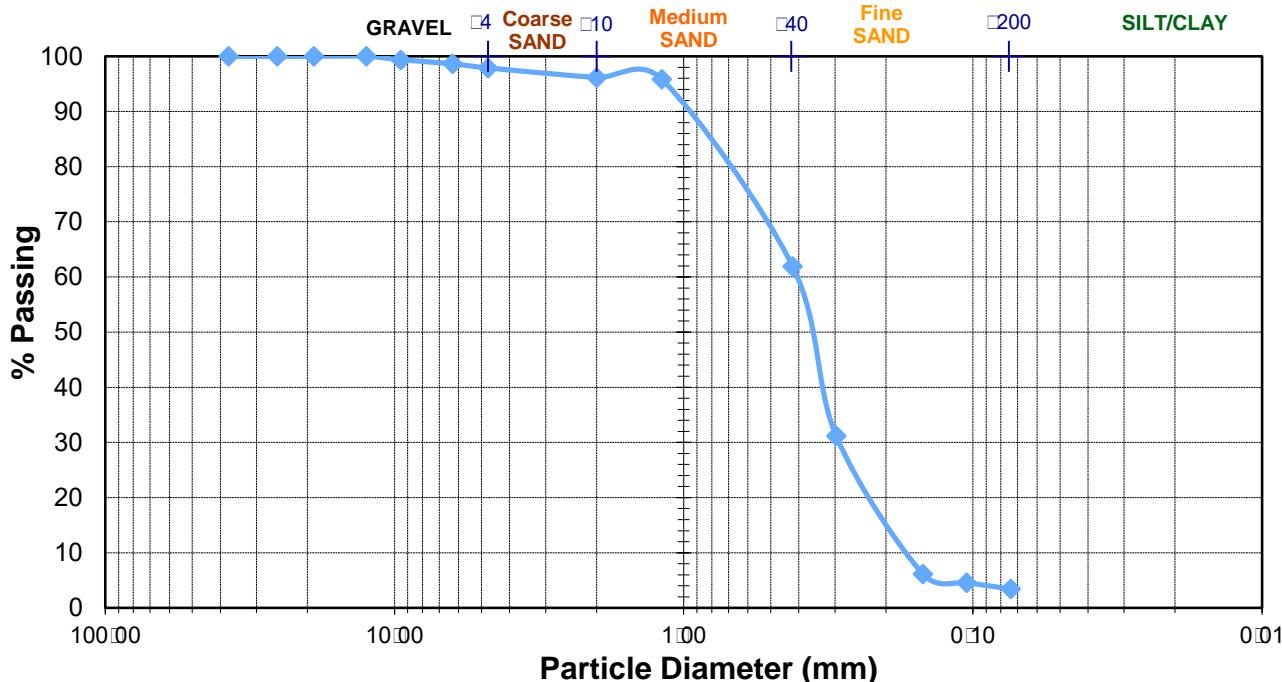
Date: 7/7/2021
Date: 7/8/2021

IDH Soil Classification: Fine SAND trace gravel

Weight of Container (g): 151.2
Weight of Dry Sample (g): 237.3

Weight of Container & Soil (g): 388.5

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15	37.50			0.0	0.0	100.0
10	25.40			0.0	0.0	100.0
34	19.00			0.0	0.0	100.0
12	12.50			0.0	0.0	100.0
38	9.50	527.80	529.3	1.5	0.6	99.4
14	6.30	513.00	514.7	1.7	0.7	98.7
4	4.75	523.60	525.4	1.8	0.8	97.9
10	2.00	676.80	680.9	4.1	1.7	96.2
16	1.19	420.80	421.6	0.8	0.3	95.8
40	0.420	573.000	653.5	80.5	33.9	61.9
50	0.297	497.700	570.6	72.9	30.7	31.2
100	0.149	518.400	577.8	59.4	25.0	6.2
140	0.105	333.100	336.8	3.7	1.6	4.6
200	0.074	326.300	329	2.7	1.1	3.5
Pan		480.2	487.7	7.5	3.2	0.3
		TOTAL:		236.6	99.7	





Sieve Analysis Data Sheet

MID42263

Project Name: M1903409
Location: 145m
Boring No: 4
Sample Depth: 31325

Tested By:
Checked By:
Sample ID: 13

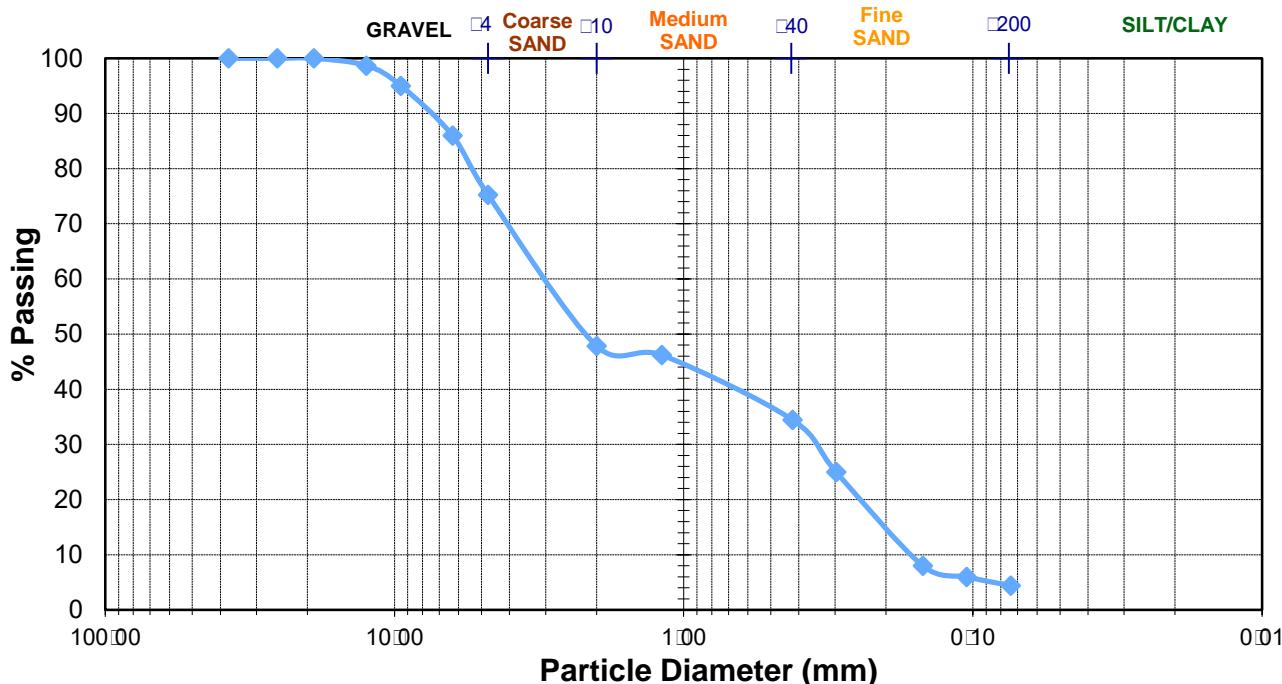
Date: 7/7/2021
Date: 7/8/2021

IDH Soil Classification: Fine to coarse SAND trace gravel

Weight of Container (g): 151.2
Weight of Dry Sample (g): 206.5

Weight of Container & Soil (g): 357.7

Sieve Number	Diameter (mm)	Mass of Sieve (g)	Mass of Sieve & Soil (g)	Soil Retained (g)	Soil Retained (%)	Soil Passing (%)
15	37.50			0.0	0.0	100.0
10	25.40			0.0	0.0	100.0
34	19.00			0.0	0.0	100.0
12	12.50	551.90	554.6	2.7	1.3	98.7
38	9.50	527.80	535.4	7.6	3.7	95.0
14	6.30	513.00	531.6	18.6	9.0	86.0
4	4.75	523.60	545.8	22.2	10.8	75.3
10	2.00	676.80	733.4	56.6	27.4	47.8
16	1.19	420.80	424.2	3.4	1.6	46.2
40	0.420	573.000	597.3	24.3	11.8	34.4
50	0.297	497.700	517.2	19.5	9.4	25.0
100	0.149	518.400	553.5	35.1	17.0	8.0
140	0.105	333.100	337.3	4.2	2.0	6.0
200	0.074	326.300	329.5	3.2	1.5	4.4
Pan		480.2	488.9	8.7	4.2	0.2
		TOTAL:		206.1	99.8	



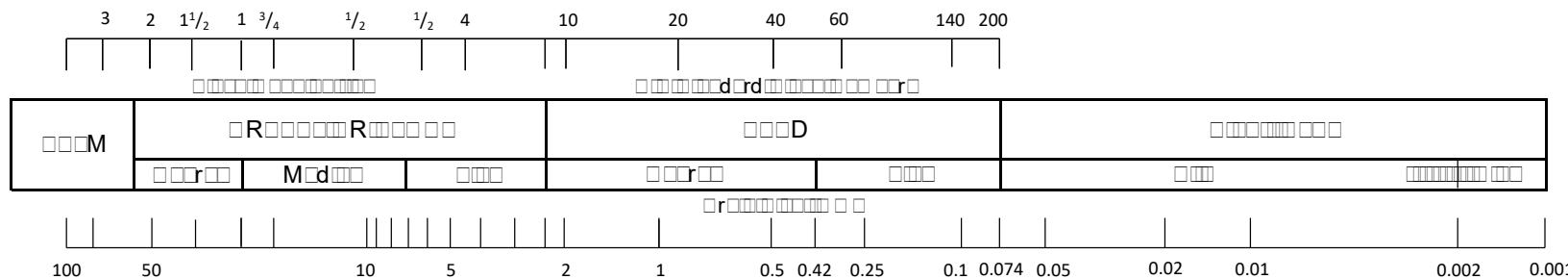
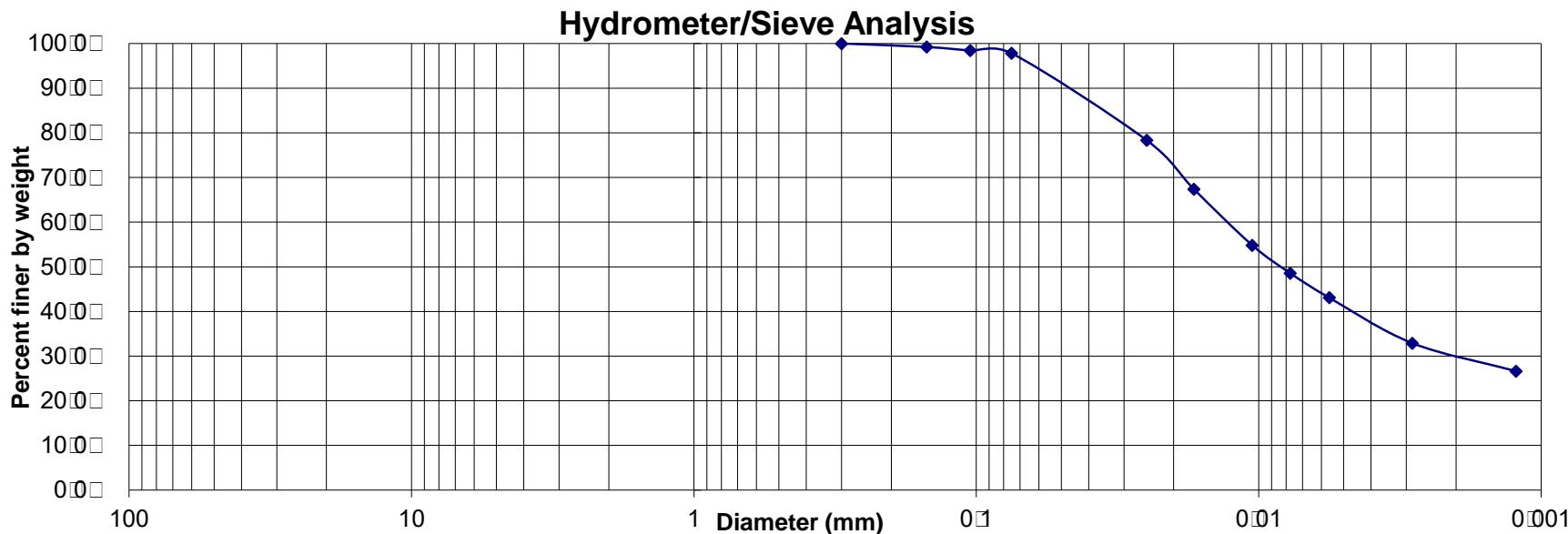
Sieve / Hydrometer Analysis (ASTM D 422)



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Gravel:	0.00
Sand:	2.20
Silt:	68.20
Clay:	29.60

USCS Classification:
□ □ □ □ □ □ □



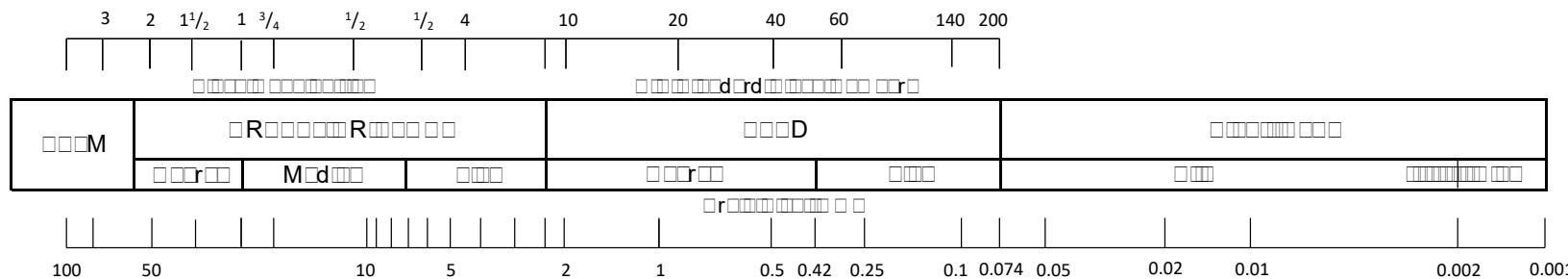
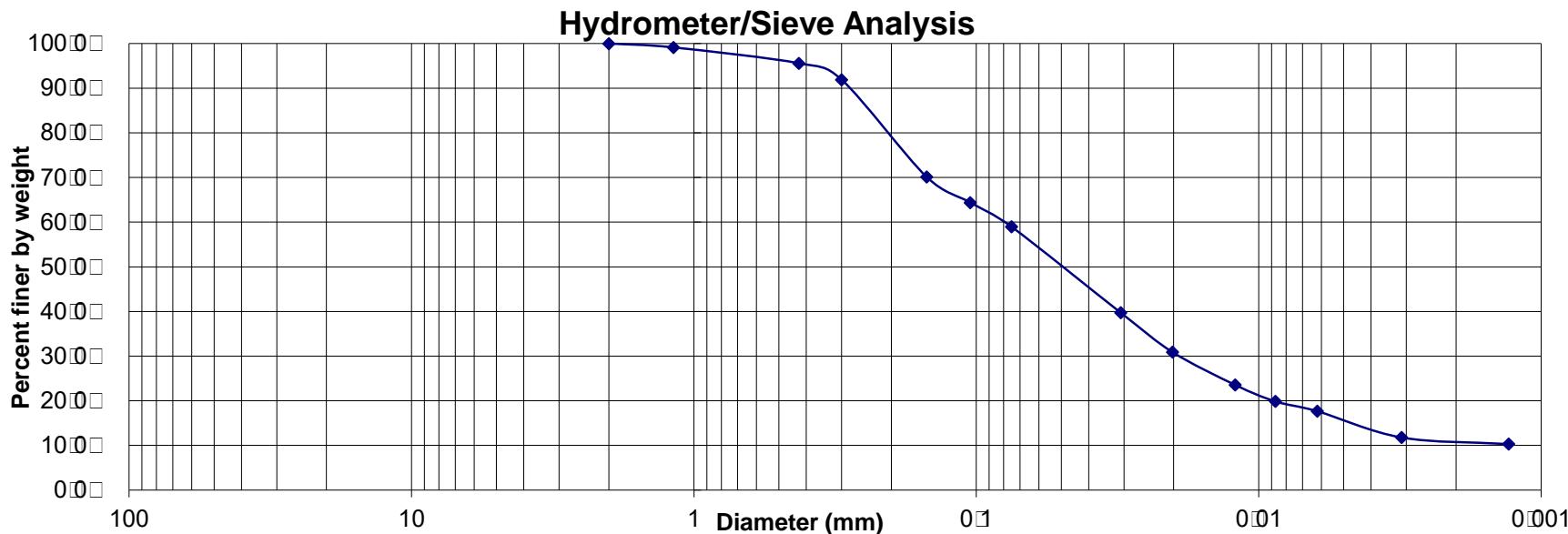
Sieve / Hydrometer Analysis (ASTM D 422)



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Gravel:	0.0□
Sand:	41.0□
Silt:	48.□□
Clay:	10.9□

USCS Classification:
□□□



Sieve / Hydrometer Analysis (ASTM D 422)



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Gravel:	0.00
Sand:	6.60
Silt:	74.70
Clay:	18.70

USCS Classification:
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