INDEX OF SHEETS

DESCRIPTION

TITLE SHEET

TYPICAL SECTIONS

EARTHWORK SCHEDULE

GENERAL NOTES & IDOT STANDARDS SUMMARY OF QUANTITIES

SHEET NO.

PROJECT BEGINS

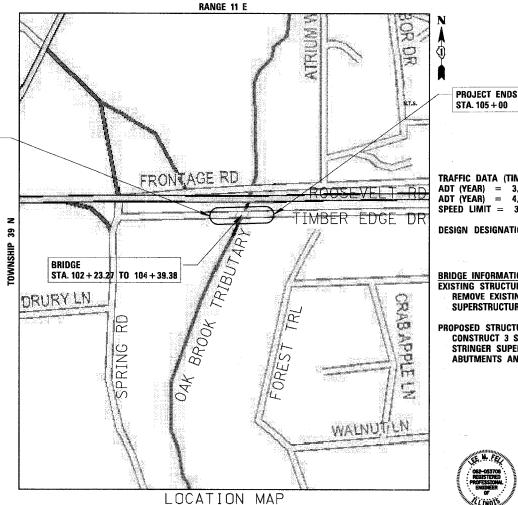
STA. 101 + 00

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS**

PLANS FOR PROPOSED FEDERAL AID HIGHWAY

TIMBER EDGE DRIVE FROM STA. 101 + 00 TO STA. 105 + 00 **PAVEMENT RECONSTRUCTION AND** BRIDGE SUB AND SUPERSTRUCTUTE RECONSTRUCTION **OVER OAK BROOK TRIBUTARY** L.A. SECTION No. 03-00019-00-BR PROJECT No. BHOS - 0043 (018)

> CITY OF OAKBROOK TERRACE **DUPAGE COUNTY** C-91-184-03



GROSS LENGTH OF PROJECT = 400 ft. (0.08 mi.) NET LENGTH OF PROJECT = 400 ft. (0.08 mi.)

TRAFFIC DATA (TIMBER EDGE DRIVE)

ADT (YEAR) = 3,000 (2005) ADT (YEAR) = 4,000 (2030) SPEED LIMIT = 30 MPH

DESIGN DESIGNATION: LOCAL

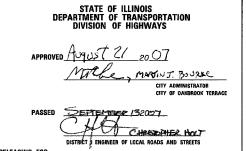
EXISTING STRUCTURE No. 022-0027 REMOVE EXISTING 3 SPAN STEEL GIRDER SUPERSTRUCTURE AND SUBSTRUCTURE.

PROPOSED STRUCTURE No. 022-6000 CONSTRUCT 3 SPAN COMPOSITE STEEL STRINGER SUPERSTRUCTURE ON INTEGRAL ABUTMENTS AND SOLID WEB PIERS

SECTION COUNTY RTE. SHEETS NO. -- 03-00019-00-BR DUPAGE 35 FED. ROAD DIST. NO. 1 | ILLINOIS | FED. AID PROJECT

CONTRACT NO. 83965





BID BASED ON

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600

tosemont, Illinois 60018

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL UNDERGROUND OR SURFACE UTILITIES EVEN THOUGH THEY MAY NOT BE SHOWN ON THE PLANS. ANY UTILITY THAT IS DAMAGED DURING CONSTRUCTION SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE ENGINEER, THE VILLAGE AND THE UTILITY OWNER.

BEFORE STARTING ANY EXCAVATING, THE CONTRACTOR SHALL CALL "J.U.L.I.E." AT 800-892-0123 FOR FIELD LOCATIONS OF BURIED ELECTRIC. TELEPHONE. CABLE AND GAS FACILITIES AND THE VILLAGE OF OAKBROOK TERRACE PUBLIC WORKS DEPT. FOR FIELD LOCATIONS OF BURIED WATER AND STORM FACILITIES (48-HOUR ADVANCE NOTIFICATION IS REQUIRED).

STAKING

THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL SECTION OR SUBSECTION MONUMENTS OR PROPERTY OR REFERENCE MARKERS UNTIL THE VILLAGE. HIS AGENT OR AN AUTHORIZED SURVEYOR HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATIONS.

STORM SEWER

WHENEVER DURING CONSTRUCTION OPERATIONS ANY LOOSE MATERIAL IS DEPOSITED IN THE FLOW LINE OF DRAINAGE STRUCTURES SUCH THAT THE NATURAL FLOW OF WATER IS OBSTRUCTED. IT SHALL BE REMOVED AT THE CLOSE OF EACH WORK ING DAY. AT THE CONCLUSION OF CONSTRUCTION OPERATIONS, ALL UTILITY STRUCTURES SHALL BE FREE FROM DIRT AND DEBRIS. THE WORK SPECIFIED ABOVE WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF TRAFFIC CONTROL AND PROTECTION.

WHEN EXISTING DRAINAGE FACILITIES ARE DISTURBED. THE CONTRICATOR SHALL PROVIDE AND MAINTAIN TEMPORARY OUTLETS AND CONNECTIONS FOR ALL PRIVATE ON PUBLIC DRAINS, SEWERS OR CATCH BASINS. HE SHALL PROVIDE AND SEWERS OR CATCH BASINS. HE SHALL PROVIDE AND SEWERS OR CATCH BASINS. HE SHALL PROVIDE AND SEWERS AND ISCARRED STORM WATER WHICH WILL BE RECEIVED BY THESE DRAINS AND SEWERS AND ISCARRED THE SAME. HE SHALL PROVIDE AND MAINTAIN AN EFFICIENT PUMPING PLANT; F NECESSARY, AND A TEMPORARY OUTLET, AND BE PREPARED AT ALL TIMES TO DISPOSE OF THE WATER RECEIVED FROM THESE TEMPORARY CONNECTIONS UNTIL SUCH TIME AS THE PERMANENT CONNECTIONS WITH SEWERS ARE BUILT AND IN SERVICE. THIS WORK WILL NOT BE PAID FOR SEPARATELY, BUT SHALL BE INCLUDED IN THE COST OF TRAFFIC CONTROL AND PROTECTION.

WATER MAIN

THE CONTRACTOR SHALL NOT OPEN OR SHUT ANY WATER VALVES OR FIRE HYDRANTS WITHOUT FRIOR AUTHORIZATION FROM THE VILLAGE PUBLIC WORKS DEPARTMENT. UNAUTHORIZED USE SHALL SUBJECT THE OFFENDER TO ARREST AND PROSECUTION.

MISCELLANEOUS

DIMENSIONS: IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND CONDITIONS EXISTING IN THE FIELD PRIOR TO ORDERING MATERIALS AND BEGINNING CONSTRUCTION.

ALL SAWCUTTING SHALL BE INCIDENTAL TO REMOVAL ITEMS AND SHALL BE PERFORMED PRIOR TO BEGINNING REMOVAL. ANY ITEMS OF WORK REMOVED PRIOR TO SAWCUTTING WILL NOT BE MEASURED FOR PAYMENT.

RELOCATING EXISTING SIGNS: EXISTING SIGNS WHICH ARE IN CONFLICT WITH PROPOSED IMPROVEMENTS SHALL BE REMOVED AND REINSTALLED UPON COMPLETION OF CONFLICTING IMPROVEMENTS IN ACCORDANCE WITH THE ILLINDIS DEPARTMENT OF TRANSPORTATION "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES" AND THE "STANDARD SPECIFICATIONS FOR TRAFFIC CONTROL ITEMS" SHALL BE INCLUDED IN THE COST OF TRAFFIC CONTROL AND PROTECTION.

POLLUTION CONTROL: THE CONTRACTOR WILL BE REQUIRED TO COMPLY WITH STATE REGULATIONS REGARDING AIR, WATER AND NOISE POLLUTION, CONSTRUCTION OPERATIONS SHALL BE CONFINED TO THE PERIOD BEGINNING AT 7:00 A.M. AND ENDING AT 6:00 P.M. WEEKDAYS, B:00A.M. TO 4:00P.M. SATURDAY, THE PERIOD SECIONING AT 7:00 A.M. AND ENDING AT 6:00 P.M. WEEKDAYS, BOORAM. TO 4:00 AND NO WORK SHALL BE PERFORMED ON SUNDAYS OR HOLIDAYS, PER VILLAGE ORDINANCE. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION OPERATIONS TO INSURE TRAFFIC MAINTENANCE, SURFACE DRAIMAGE, ETC. THROUGHOUT THE DURATION OF THE CONSTRUCTION PERIOD IN ACCORDANCE WITH THE REQUIREMENTS OF THE VILLAGE OF OAKBROOK TERRACE, AND ANY OTHER GOVERNING AGENCIES. AND ANY UTHER GOVERNING AGENCIES.
THE CONTRACTOR SHALL TAKE ALL NECESSARY SAFETY PRECAUTIONS TO PROTECT AND PROVIDE ACCESS TO ABUTTING PROPERTY, UTILITIES, PEDESTRIANS AND VEHICULAR TRAFFIC.
NO BURNING OR INCINERATION OF RUBBISH WILL BE PERMITTED ON SITE.

DO NOT SCALE DRAWINGS IF COORDINATES AND DIMENSIONS ARE GIVEN. THE PROPOSED GRADING ELEVATIONS SHOWN ON THE PLANS ARE FINISHED GRADE, ALLOW FOR THE THICKNESS OF TOPSOIL AS SHOWN.
THE CONTRACTOR IS ADVISED THAT MUD AND DEBRIS MUST NOT BE DEPOSITED ON THE ADJACENT ROADWAYS. ANY DIRT AND DEBRIS ACCUMULATED ON THE PAVEMENT SHALL BE CLEANED BY THE CONTRACTOR WITHIN FOUR (4) HOURS OF THE INCIDENT OR HE WILL BE BACK CHARGED AT THE RATE OF \$500.00 PER INCIDENT PLUS THE COST OF THE VILLAGE'S FORCES TO COMPLETE THE WORK.
THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO STREETS OR ROADWAYS AND ASSOCIATED STRUCTURES AND SHALL MAKE REPAIRS AS NECESSARY TO CORRECT DAMAGE AT HIS OWN EXPENSE THE GRADING AND CONSTRUCTION OF THE PROPOSED IMPROVEMENTS SHALL NOT CAUSE PONDING OF STORM WATER.

 F.A RTE.	SECTION		COUNT	Υ	TOTAL SHEETS	SHEE'
	03-00019-0)-BR	DUPA	GE.	35	2
STA.	* *	1	O STA.			
FED. RO	AD DIST. NO. 1	ILLIN	OIS FED.	AID	PROJECT	

CONTRACT NO. 83965

PROJECT UTILITY CONTACTS

COMMONWEALTH EDISON 2 LINCOLN CENTER OAKBROOK TERRACE, IL 60181 ATTN: TOM STUTZMAN TEL: 630-437-2236 SENT: 5/18/07 **COMMENTS: NO CONFLICTS ANTICIPATED**

COMCAST

688 INDUSTRIAL DRIVE ELMHURST, IL 60126 ATTN: MARY STEFAN TEL: 630-600-6346 SENT: 5/18/07 COMMENTS: NO CONFLICTS ANTICIPATED

NICOR GAS 1844 FERRY ROAD NAPERVILLE, IL 60563 ATTN: CONSTANCE LANE TEL: 630-983-8676 (Ext. 2362) SENT: 5/18/07 COMMENTS: NO CONFLICTS ANTICIPATED

AT&T (SBC) 1000 COMMERCE DRIVE FLOOR 2 OAK BROOK, IL 60523 ATTN: TOM FOLLIN SENT: 5/18/07 COMMENTS:

AT&T 301 WHALEY ST. ROOM 403 LONGVIEW TX 75601 ATTN: LLOYD MAGOWN SENT: 5/18/07 COMMENTS:

XO ILLINOIS. INC. 7001 FRONTAGE ROAD BURR RIDGE, IL 60527 ATTN: GLEN LUEHRSEN SENT: 5/18/07 COMMENTS:

HIGHWAY STANDARDS

000001-04	STANDARD SYMBOLS, ABBREVIATIONS, AND PATTERNS
280001-03	TEMPORARY EROSION CONTROL SYSTEMS
630001-07	STEEL PLATE BEAM GUARDRAIL
701321-08	LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
70150103	URBAN LANE CLOSURE. 21. 2W UNDIVIDED
701701-04	URBAN LANE CLOSURE. MULTILANE INTERSECTION
~ 702001-06	TRAFFIC CONTROL DEVICES
780001-01	TYPICAL PAVEMENT MARKINGS
631031-CL	TRAFFIC BARRIER TERMINAL, TYPE L.

REVISIONS ILLINOIS DEPARTMENT OF TRANSPORTATION **GENERAL NOTES** SCALE: N.T.S. DRAWN BY BEH DATE 8/24/2007 CHECKED BY LMF

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CONT	RACT NO.	8396	5			
FED. RO	AD DIST, NO. 1	ILLIN	DIS FED.	AID	PROJECT	
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	03-00019-00	D-BR	DUPA	GE	35	3
F.A RTE.	SECTION		COUNT	Y	TOTAL SHEETS	SHEET NO.

SUMMARY OF QUANTITIES

			1000-2A	X071-2A	TOTAL
ltem	ltems	Unit	QUANTITY	QUANTITY	QUANTITY
į	EARTH EXCAVATION	CU YD	320	0	320
	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL	CU YD	146	0	146
	POROUS GRANULAR EMBANKMENT, SPECIAL	CU YD	0	100	100
-	POROUS GRANULAR EMBANKMENT, SUBGRADE	CU YD	68	0	68
2	TOPSOIL FURNISH AND PLACE, 4"	SQ YD	690	0	690
	SEEDING, CLASS 3	ACRE	0.32	0	0.32
	EROSION CONTROL BLANKET	SQ YD	1020	0	1020
	SUPPLEMENTAL WATERING	UNIT	45	0	45
	TEMPORARY EROSION CONTROL SEEDING	ACRE	0.32	0	0.32
	PERIMETER EROSION BARRIER	FOOT	900	0	900
	STONE DUMPED RIPRAP, CLASS A4	SQ YD	0	190	190
	SUB-BASE GRANULAR MATERIAL, TYPE B 4"	SQ YD	562	0	562
	BITUMINOUS MATERIALS (PRIME COAT)	GAL	57	0	57
40600300	AGGREGATE (PRIME COAT)	TON	1.5	0	1.5
·	HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	TON	114	0	114
1	HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N50	TON	49	0	49
42001165	BRIDGE APPROACH PAVEMENT	SQ YD	220	0	220
	PAVEMENT REMOVAL	SQ YD	597	0	597
8	APPROACH SLAB REMOVAL	SQ YD	200	0	200
S	PORTLAND CEMENT CONCRETE SHOULDERS 9"	SQ YD	230	0	230
<u> </u>	REMOVAL OF EXISTING STRUCTURES	EACH	0	1	1
50200100	STRUCTURE EXCAVATION	CU YD	0	320	320
50300225	CONCRETE STRUCTURES	CU YD	0	183	183
50300255	CONCRETE SUPERSTRUCTURE	CU YD	0	201	201
50300260	BRIDGE DECK GROOVING	SQ YD	0	520	520
50300300	PROTECTIVE COAT	SQ YD	0	710	710
	FURNISHING AND ERECTING STRUCTURAL STEEL	LSUM	0	1	1
	STUD SHEAR CONNECTORS	EACH	0	2595	2595
	REINFORCEMENT BARS, EPOXY COATED	POUND	0	57780	57780
63100085	TRAFFIC BARRIER TERMINAL, TYPE L	EACH	4	0	Ч

**************************************			1000-2A	X071-2A	TOTAL
ltem	Items	Unit	QUANTITY		QUANTITY
50800515	BAR SPLICERS	EACH	0	64	64
1	SLOPE WALL 4 INCH	SQ YD	0	210	210
1	FURNISHING STEEL PILES HP10X42	FOOT	0	575	575
51201600	FURNISHING STEEL PILES HP12X53	FOOT	0	535	535
51202305	DRIVING PILES	FOOT	0	1110	1110
51203400	TEST PILE STEEL HP10X42	EACH	0	2	2
51203600	TEST PILE STEEL HP12X53	EACH	0	2	2
51204650	PILE SHOES	EACH	0	20	20
51500100	NAME PLATES	EACH	0	1	1
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	0	72	72
60109580	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	0	128	128
* 63000000	STEEL PLATE BEAM GUARD RAIL, TYPE A	FOOT	137	0	137
63200310	GUARDRAIL REMOVAL	FOOT	47	0	47
67100100	MOBILIZATION	LSUM	1	0	1
70102620	TRAFFIC CONTROL AND PROTECTION, STANDARD 701501	L. SUM	1	0	1
70102635	TRAFFIC CONTROL AND PROTECTION, STANDARD 701701	L. SUM	1	0	1
70102640	TRAFFIC CONTROL AND PROTECTION, STANDARD 701801	L. SUM	1	0	1
70300220	TEMPORARY PAVEMENT MARKING - LINE 4"	FOOT	400	0	400
* 78000200	THERMOPLASTIC PAVEMENT MARKING - LINE 4"	FOOT	305	0	305
* 78005110	EPOXY PAVEMENT MARKING - LINE 4"	FOOT	435	0	435
	STABILIZED CONSTRUCTION ENTRANCE	SQ YD	385	Ū	385
XD323080	DRAINAGE SCUPPERS, DS-12	EACH	0	4	4
XXXXIIOT	SILT CURTAIN	FOOT	185	0 .	185
Z0001040	AGGREGATE SUBGRADE 8"	SQ YD	850	0	850
Z0013798	CONSTRUCTION LAYOUT	LSUM	1	0	1
△ Z0076600	TRAINEES	HOUR	500	0	500
XX00.6339	FENCE TO BE REMOVED AND REPLACED	FOOT	40	0	40
xx007093	STRAW BALE BARRIER PLAN	FOOT	200	0	200
	UNDERWATER STRUCTURE EXCAVATION PROTECTION, LOCATION 1	EACH	0	1	1
X5020502	UNDERWATER STRUCTURE EXCAVATION PROTECTION, LOCATION 2	EACH	0	1	1
* SPECIAL	TY ITEM				

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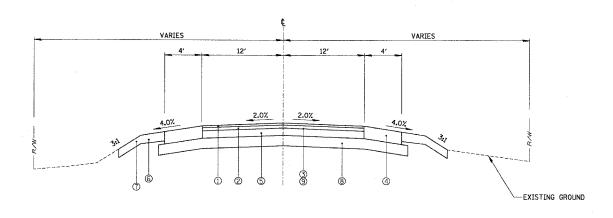
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NAME DAT	TE ILLINOIS DEPART	MENT OF TRANSPORTATION
		R EDGE DRIVE Y OF QUANTITIES
	SCALE: N.T.S.	DRAWN BY BEH
	DATE 8/24/2007	CHECKED BY LMF

CHRISTOPMER B. BURKE ENGINEERING LTD. PLAN 9575 West Higgins Road, Suite 600 BRosement, Illinois 60018 (847) 823-0500

CONTRACT NO. 83965

VARIES		VARIES	
	<u>v</u> 		
1.5'	VARIES VARIES		NG GROUND

EXISTING TYPICAL SECTION TIMBER EDGE DRIVE (STA. 101+00 - STA. 102+23.27) BRIDGE OMISSION (STA. 102+23.27 - STA. 104+39.39) TIMBER EDGE DRIVE (STA. 104+39.39 - STA. 105+00)



PROPOSED TYPICAL SECTION TIMBER EDGE DRIVE (STA, 101+00 - STA, 102+23,27)
BRIDGE OMISSION (STA, 102+23,27 - STA, 104+39.39)
TIMBER EDGE DRIVE (STA, 104+39.39 - STA, 105+00)

LEGEND:

EXISTING:

- A EXISTING PAVEMENT
- EXISTING SHOULDER
- © EXISTING BASE COURSE
- (E) PROPOSED POROUS GRANULAR EMBANKMENT, SUBGRADE UNDERCUTTING (PAID FOR AS REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL)

PROPOSED:

- ① PROPOSED HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N50, 2"
- PROPOSED HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50, 3"
- 3 PROPOSED BITUMINOUS MATERIALS (PRIME COAT)
- 4 PROPOSED PORTLAND CEMENT CONCRETE SHOULDER, 9"
- ⑤ PROPOSED SUB-BASE GRANULAR MATERIAL, TYPE B, 4"
- 6 TOPSOIL FURNISH AND PLACE, 4"
- SEEDING, CLASS 3
- AGGREGATE SUBGRADE, 8"
- 9 PROPOSED AGGREGATE PRIME COAT

HOT-MIX ASPHALT MIXTURE REQUIREMENTS	AC-TYPE	VOIDS
HOT-MIX ASPHALT SURFACE COURSE, MIX C, N50 - IL 9.5 mm	PG 64-22	4%@50GYR.
HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	PG 64-22*	4% e 50GYR.

- NOTE:

 1. THE UNIT WEIGHT USED TO CALCULATE ALL
 HOT-MIX ASPHALT SURFACE MIXTURE
 QUANTITIES IS 112 LBS/SY/IN.
- ALL WORK INCLUDING SOD MUST BE COMPLETED AND APPROVED BY ENGINEER PRIOR TO FINAL SURFACE LIFT OF ASPHALT.
- POROUS GRANULAR EMBANKMENT, SUBGRADE HAS BEEN PROVIDED TO BE UNSTABLE
 WHEN WET. THE ACTUAL NEED FOR REMOVAL AND REPLACEMENT WITH POES WILL BE DETERMINED IN THE FIELD AT THE
 TIME OF CONSTRUCTION BY THE ENGINEER. IF UNSUITABLE SOILS ARE ENCOUNTERED THE SOILS SHALL BE REMOVED
 AND REPLACED WITH POES. THE REMOVAL AND REPLACEMENT AREA SHALL EXTEND TO 12 INCHES BEYOND THE EDGE
 OF PAVEMENT AND COME UP AT A 1:1 SLOPE TO EXISTING GROUND SURFACE. THESE LIMITS MAY BE ALTERED BY THE
 ENGINEER IF FIELD CONDITIONS SO WARRANT. REMOVAL OF THESE UNSUITABLE SOILS SHALL BE PAID FOR AS "REMOVAL
 AND DISPOSAL OF UNSUITABLE MATERIAL."

	REVISIONS	5		
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			TEAR	ED EDAF DD0/F
			IIMB	ER EDGE DRIVE
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 F.A RTE.	SECTION	COUNTY	TOTAL	SHEE NO.
	03-00019-00-BR	DUPAGE	35	5
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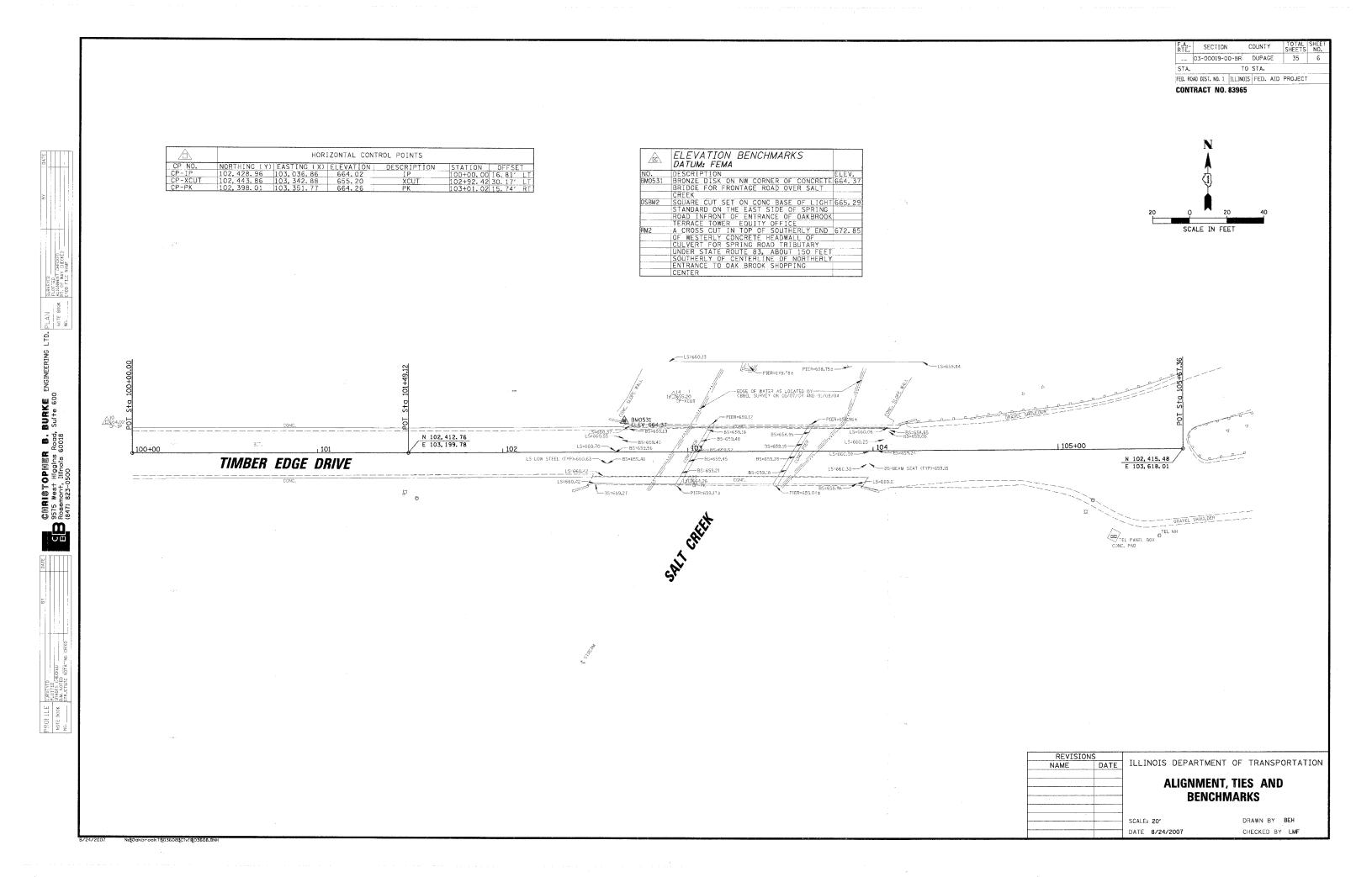
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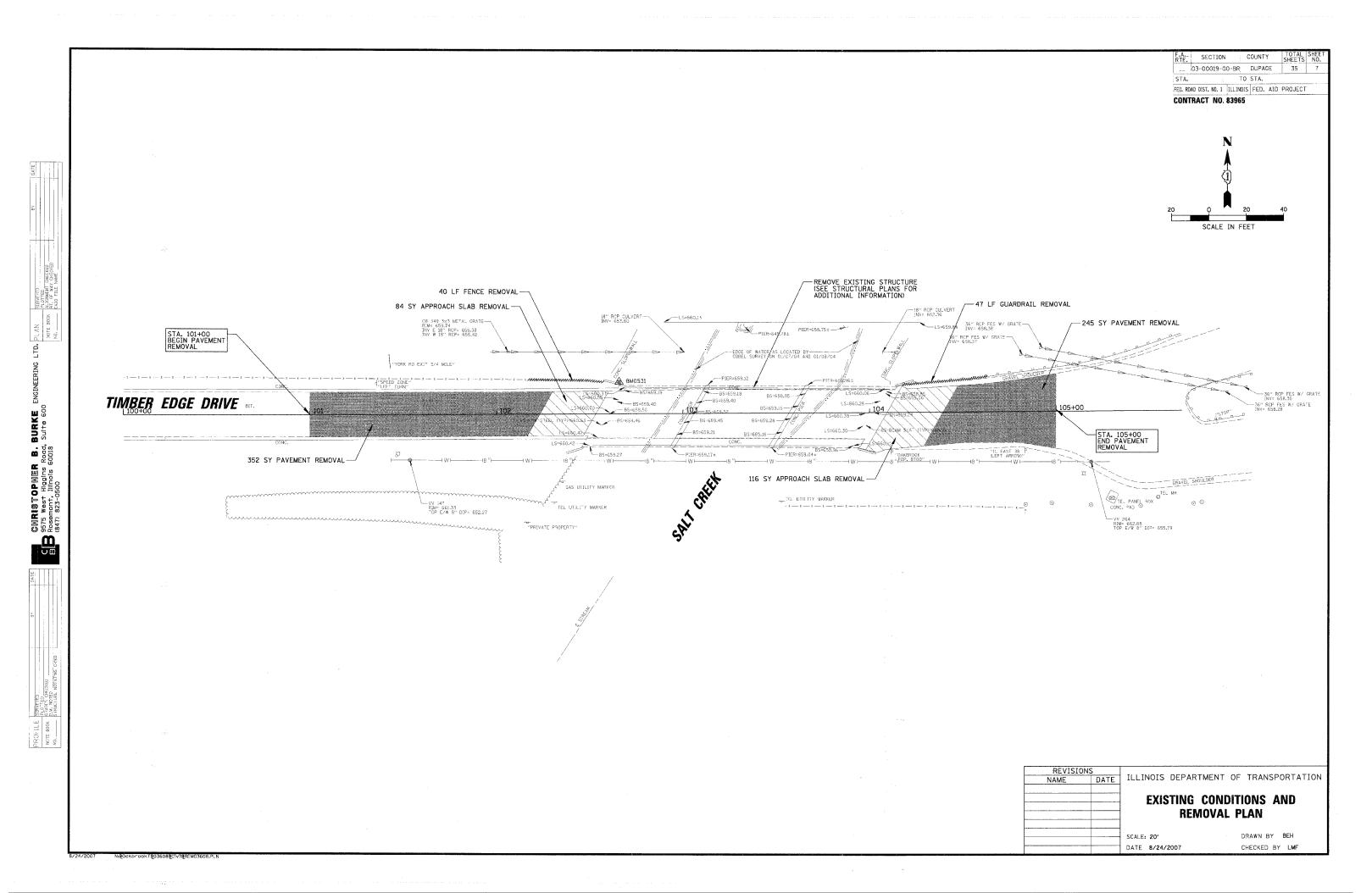
EARTHWORK SCHEDULE

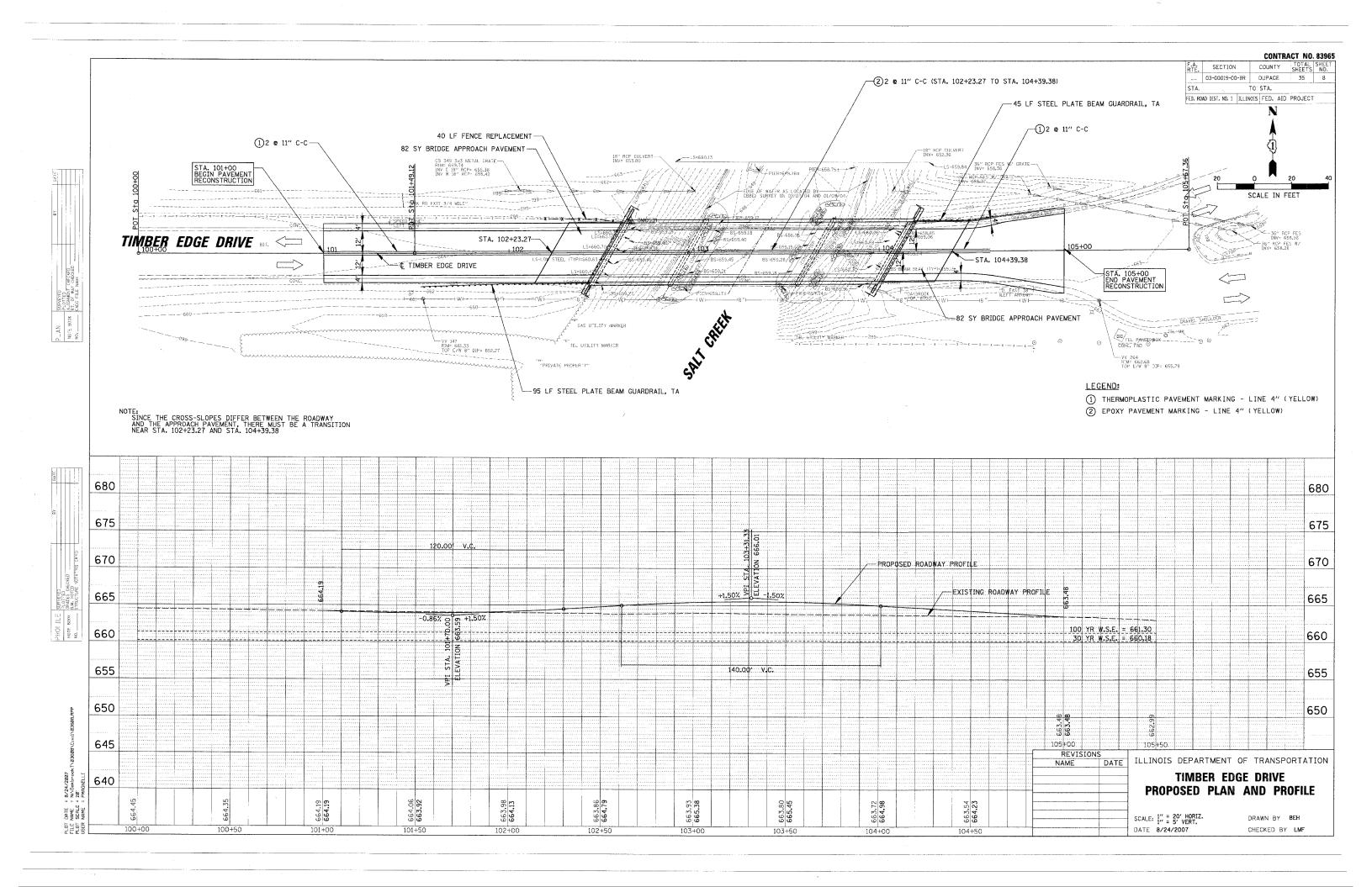
		EARTH EXCAVATION		EARTHWORK BALANCE
		ADJUSTED FOR		WASTE (+)
	EARTH EXCAVATION**	SHRINKAGE (15%)	EMBANKMENT	OR SHORTAGE (-)
STATION	(CY)	(CY)	(CY)	(CY)
WEST OF BRI	DGE			
101+00.00	0.0	0.0	0.0	0.0
101+50.00	96. 8	82. 2	6.8	75. 5
102+00.00	90. 9	77.3	4.3	73.0
102+23.27	33. 8	28.7	4.7	24. 0
SUBTOTAL	221.5	188. 2	15.8	172.5
EAST OF BRI	DGE			
104+39.38	0.0	0.0	0.0	0.0
104+50.00	8. 4	7.1	15.7	-8.6
105+00.00	87.9	74. 7	44. 2	30.5
SUBTOTAL	96. 2	81.8	59.9	21.9
TOTAL	317. 7	270.1	75.6	194.4

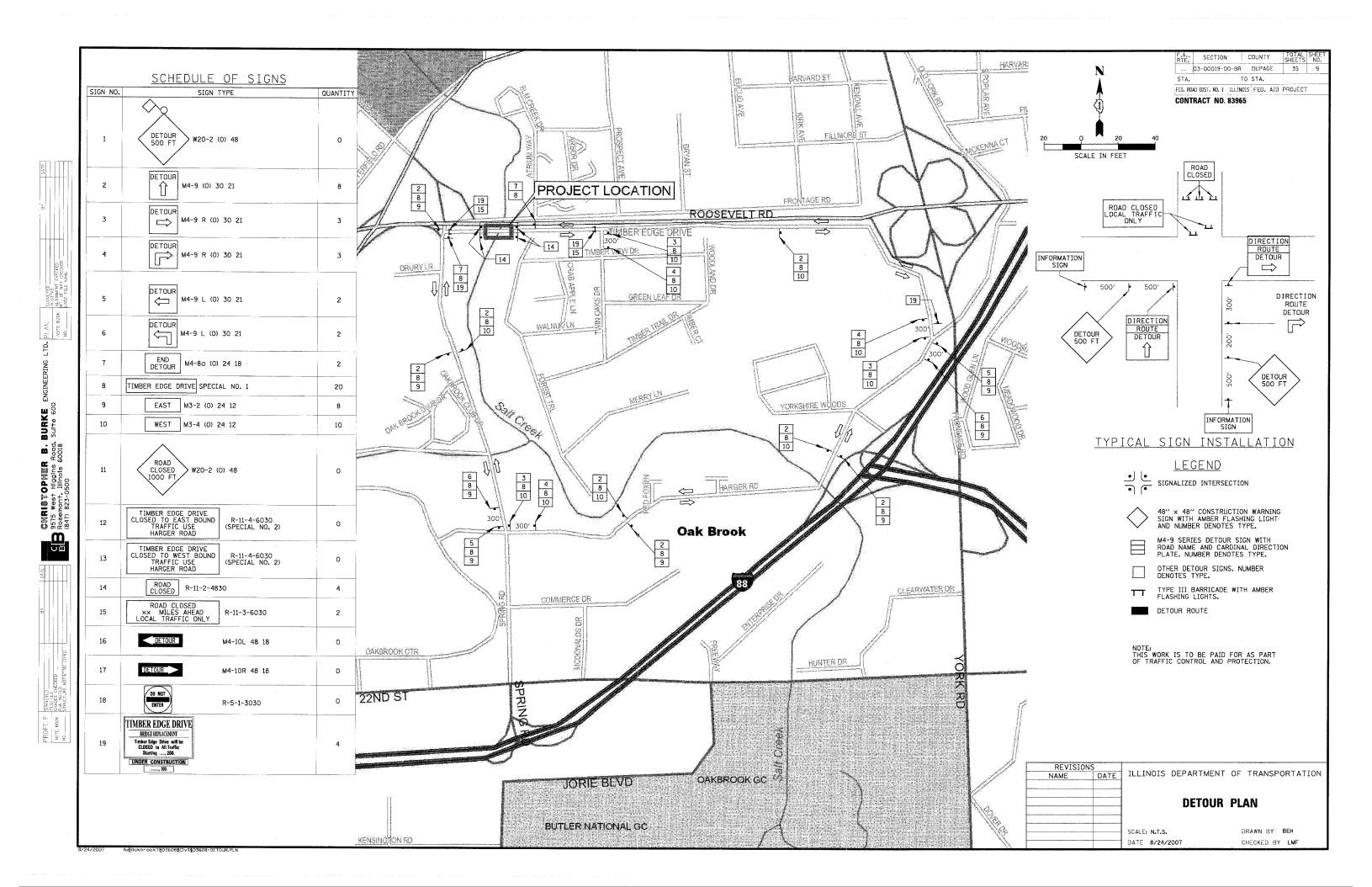
^{**} WILL BE PAID FOR AS EARTH EXCAVATION

REVISIONS						_
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1. SITE DESCRIPTION.

- A. THE FOLLOWING IS A DESCRIPTION OF THE CONSTRUCTION ACTIVITY FOLLOWING MASS GRADING WHICH IS THE SUBJECT OF THIS PLAN:
 THE PROPOSED DEVELOPMENT CONSISTS OF REPLACING THE EXISTING BRIDGE DECK AND PIERS. THE CONSTRUCTION ACTIVITIES FOR SITE IMPROVEMENTS WILL INCLUDE: EXCAVATION AND GRADING, DISPOSAL OF EXCAVATED MATERIAL AND SEEDING, EROSION BLANKET AND SOIL EROSION AND INSTALLATION OF THE BRIDGE AND PIERS.
- B. THE FOLLOWING IS A DESCRIPTION OF THE INTENDED SEQUENCE OF MAJOR ACTIVITIES WHICH WILL DISTURB SOILS FOR MAJOR PORTIONS OF THE CONSTRUCTION SITE, SUCH AS EXCAVATION, AND GRADING. SEQUENCE OF THE CONSTRUCTION ACTIVITIES MAY BE AS FOLLOWS: 1) INSTALL EROSION CONTROL 2) EXCAVATION, GRADING AND PLACEMENT OF PIERS AND BRIDGE 3) DISPOSAL OF EXCAVATED TERIALS 4) SEEDING AND EROSION BLANKET
- C. THE TOTAL AREA OF THE CONSTRUCTION SITES IS ESTIMATED TO BE 0.58 ACRES. THE TOTAL AREA OF THE SITE THAT IT IS ESTIMATED TO BE DISTURBED BY EXCAVATION, GRADING, OR OTHER ACTIVITIES, IS 0.58 ACRES.

2. CONTROLS.

THIS SECTION OF THE PLAN ADDRESSES THE VARIOUS CONTROLS THAT WILL BE IMPLEMENTED FOR EACH OF THE MAJOR CONSTRUCTION ACTIVITIES DESCRIBED IN 1.B ABOVE. FOR EACH MEASURE DISCUSSED, THE CONTRACTORS WILL BE RESPONSIBLE FOR ITS IMPLEMENTATION AS INDICATED. EACH SUCH CONTRACTOR HAS SIGNED THE REQUIRED CERTIFICATION ON FORMS WHICH ARE ATTACHED TO, AND ARE

- A. EROSION AND SEDIMENT CONTROLS.

 U. STABILIZATION PRACTICES. PROVIDED BELOW IS A DESCRIPTION OF INTERIM AND PERMANENT STABILIZATION PRACTICES. SITE PLANS WILL ENSURE THAT EXISTING VEGETATION OF THE IMPLEMENTATION OF THE PRACTICES. SITE PLANS WILL ENSURE THAT EXISTING VEGETATION IS PRESERVED WHERE ATTAINABLE AND DISTURBED PORTIONS OF THE SITE WILL BE STABILIZED. EXCEPT AS PROVIDED IN 2.A.(1).(A) AND 2.B., STABILIZATION MEASURES STABLIZE INTITIATED ON A DAILY BASIS WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, BUT IN NO CASE MORE THAN 14 DAYS AFTER THE CONSTRUCTION ACTIVITY IN THAT PORTION OF THE SITE HAS TEMPORARILY OR PERMANENTLY CEASED ON ALL DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY WILL NOT OCCUR FOR A PERIOD OF 21 OR MORE CALENDAR DAYS.

 (A). WHERE THE INITIATION OF STABILIZATION MEASURES BY THE 14TH DAY AFTER
 - CONSTRUCTION ACTIVITY TEMPORARILY OR PERMANENTLY CEASES IS PRECLIDED BY SNOW COVER, STABILIZATION MEASURES SHALL BY INITIATED AS SOON AS
 - (B). THE FOLLOWING INTERIM AND PERMANENT STABILIZATION PRACTICES, AS A MINIMUM, WILL BE IMPLEMENTED TO STABILIZE THE DISTURBED AREA OF THE SITE:
 - 1. TEMPORARY SEEDING 4. FILTER BARRIER
 - . PERMANENT SEEDING
- (II). STRUCTURAL PRACTICES. PROVIDED BELOW IS A DESCRIPTION OF STRUCTURAL PRACTICES THAT WILL BE IMPLEMENTED, TO THE DEGREE ATTAINABLE, TO DIVERT FLOWS FROM EXPOSED SOILS. STORE FLOWS OR OTHERWISE LIMIT RUNOFF AND THE DISCHARGE OF POLLUTANTS FROM EXPOSED AREAS OF THE SITE. THE INSTALLATION OF THESE DEVICES MAY BE SUBJECT TO SECTION 404 OF THE CLEAN WATER ACT.
 - 1. SLOPE WALL

B. STORMWATER MANAGEMENT.

(I). PROVIDED BELOW IS A DESCRIPTION OF MEASURES THAT WILL BE INSTALLED DURING THE CONSTRUCTION PROCESS TO CONTROL POLLUTANTS IN STORMWATER DISCHARGES THAT WILL OCCUR AFTER CONSTRUCTION OPERATIONS HAVE BEEN COMPLETED. THE INSTALLATION OF THESE DEVICES MAY BE SUBJECT TO SECTION 404 OF THE CLEAN WATER ACT. THE PRACTICES SELECTED FOR IMPLEMENTATION WERE DETERMINED ON THE BASIS OF THE TECHNICAL GUIDANCE CONTAINED IN 1EPA'S STANDARD SPECIFICATIONS FOR SOIL EROSION AND SEDIMENTATION CONTROL, AND OTHER ORDINANCES LISTED IN THE

THE STORMWATER POLLUTANT CONTROL MEASURES SHALL INCLUDE:

- 1. STRAW BALE BARRIER
 2. SILT CURTAIN
- (ID. VELOCITY DISSIPATION DEVICES WILL BE PLACED AT DISCHARGE LOCATIONS AND ALONG THE LENGTH OF ANY OUTFALL CHANNEL AS NECESSARY TO PROVIDE A NON-EROSIVE VELOCITY FLOW FROM THE STRUCTURE TO A WATER COURSE SO THAT THE NATURAL PHYSICAL AND BIOLOGICAL CHARACTERISTICS AND FUNCTIONS ARE MAINTAINED AND PROTECTED (E.G., MAINTENANCE OF HYDROLOGIC CONDITIONS, SUCH AS THE HYDROPERIOD AND HYDRODYNAMICS PRESENT PRIOR TO THE INITIATION OF CONSTRUCTION ACTIVITIES).
- STORMWATER MANAGEMENT CONTROL INCLUDES: 1. NOT APPLICABLE.

C. OTHER CONTROLS.

- (I). WASTE DISPOSAL. THE SOLID WASTE MATERIALS INCLUDING TRASH, CONSTRUCTION DEBRIS, EXCESS CONSTRUCTION MATERIALS, MACHINERY, TOOLS AND OTHER ITEMS WILL BE COLLECTED AND DISPOSED OFF-SITE BY THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE TO ACQUIRE ANY PERMIT REQUIRED FOR SUCH DISPOSAL. BURNING ON THE SITE WILL NOT BE PERMITTED. NO SOLID MATERIALS, INCLUDING BUILDING MATERIALS, SHALL BE DISCHARGED INTO WATERS OF THE STATE, EXCEPT AS AUTHORIZED BY A SECTION 404 PERMIT.
- (II). THE PROVISIONS OF THIS PLAN SHALL ENSURE AND DEMONSTRATE COMPLIANCE WITH APPLICABLE STATE AND/OR LOCAL WASTE DISPOSAL, SANITARY SEWER OR SEPTIC SYSTEM REGULATIONS.

D. APPROVED STATE OR LOCAL PLANS.
THE MANAGEMENT PRACTICES, CONTROLS, AND OTHER PROVISIONS CONTAINED IN THIS PLAN ARE AT LEAST AS PROTECTIVE AS THE REQUIREMENTS CONTAINED IN THE ILLINOIS ENVIRONMENTAL PROTECTION AGENCY'S STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL DATED COOBER 1987, ILLINOIS PROCEDURES AND STANDARDS FOR URBAN SOIL EROSION AND SEDIMENTATION PLAN, AND THE MUNICIPAL SUBDIVISION ORDINANCE, REQUIREMENTS SPECIFED IN SEDIMENT AND EROSION CONTROL SITE PLANS OR SITE PERMITS OR STORMWATER MANAGEMENT SITE PLANS OR SITE PERMITS APPROVED BY LOCAL OFFICIALS THAT ARE APPLICABLE TO PROTECTING SURFACE WATER RESOURCES ARE, UPON SUBMITTAL OF AN NOI TO BE AUTHORIZED TO DISCHARGE UNDER THIS PERMIT, INCORPORATED BY REFERENCE AND ARE ENFORCEABLE UNDER THIS PERMIT EVEN IF THEY ARE NOT SPECIFICALLY INCLUDED IN THE PLAN.

MAINTENANCE.

THE FOLLOWING IS A DESCRIPTION OF PROCEDURES THAT WILL BE USED TO MAINTAIN, IN GOOD AND EFFECTIVE OPERATING CONDITIONS, VEGETATION, EROSION AND SEDIMENT CONTROL MEASURES AND OTHER PROTECTIVE MEASURES IDENTIFIED IN THIS PLAN AND STANDARD

VEGETATIVE EROSION CONTROL MEASURES: THE VEGETATIVE GROWTH OF TEMPORARY AND PERMANENT SEEDING, SODDING, VEGETATIVE CHANNELS, VEGETATIVE FILTER, ETC. SHALL BE
MAINTAINED PERIODICALLY AND SUPPLY ADEQUATE WATERING AND FERTILIZER. THE VEGETATIVE ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES WILL BE COVER SHALL BE REMOYED AND RESEEDED AS NECESSARY. EROSION BLANKET SHALL BE PLACED

INSTALLED AT MINIMUM ACCORDING TO THE STANDARDS AND SPECIFICATIONS IN THE ILLINOIS

ON ALL SEEDED AREAS.

URBAN MANUAL, REVISED FEBRUARY 2002. A COPY OF THE APPROVED EROSION AND PERIMETER EROSION BARRIER WILL BE EXAMINED ON A DAILY BASIS AND REPAIRED

AS NECESSARY.

INSPECTIONS

- THE OWNER, OR OWNER'S REPRESENTATIVE SHALL PROVIDE QUALIFIED PERSONNEL TO INSPECT DISTURBED AREAS OF THE CONSTRUCTION SITE WHICH HAVE NOT BEEN FINALLY STABILIZED, STRUCTURAL CONTROL MEASURES, AND LOCATION WHERE VEHICLES ENTER OR EXIT THE SITE. SUCH INSPECTIONS SHALL BE CONDUCTED ON A WEEKLY BASIS AND WITHIN 24 HOURS OF THE END OF A STORM THAT IS 0.5 INCHES OR GREATER OR EQUIVALENT SNOWFALL
- A. DISTURBED AREAS AND AREAS USED FOR STORAGE OF MATERIALS THAT ARE EXPOSED A DISTURBED AREAS AND AREAS USED FOR STORAGE OF MATERIALS THAT ARE EXPOSED TO PRECEDITATION SHALL BE INSPECTED FOR EVIDENCE OF, OR THE POTENTIAL FOR, POLLUTANTS ENTERING THE DRAINAGE SYSTEM. EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN SHALL BE OBSERVED TO ENSURE THAT THEY ARE OPERATING CORRECTLY. WHERE DISCHARGE LOCATIONS OR POINTS ARE ACCESSIBLE, THEY SHALL BE INSPECTED TO ASCERTAIN WHETHER EROSION CONTROL MEASURES ARE EFFECTIVE IN PREVENTING SIGNIFICANT IMPACTS TO RECEIVING WATERS. LOCATIONS WHERE VEHICLE ELVIED ARE STITLE WHAT IN INSPECTED FOR EVIDENCE OF A STATEMENT OF THE PROPERTY OF OR EVIDENCE OF A STATEMENT OF THE PROPERTY OF OR EVIDENCE OF A STATEMENT OF THE PROPERTY OF OR EVIDENCE OF THE PROPERTY O WHERE VEHICLES ENTER OR EXIT THE SITE SHALL BE INSPECTED FOR EVIDENCE OF OFF SITE SEDIMENT TRACKING.
- B. BASED ON THE RESULTS OF THE INSPECTION, THE DESCRIPTION OF POTENTIAL POLLUTANT SOURCES IDENTIFIED IN SECTION 1 ABOVE AND POLLUTION PREVENTION MEASURES IDENTIFIED IN SECTION 2 ABOVE SHALL BE REVISED AS APPROPRIATE AS SOON AS PRACTICABLE AFTER SUCH INSPECTION. ANY CHANGES TO THIS PLAN RESULTING FROM THE REQUIRED INSPECTIONS SHALL BE IMPLEMENTED WITHIN 7 CALENDAR DAYS FOLLOWING THE INSPECTION.
- C. A REPORT SUMMARIZING THE SCOPE OF THE INSPECTION, NAME(S) AND QUALIFICATIONS OF PERSONNEL MAKING THE INSPECTION, THE DATE(S) OF THE INSPECTION, MAJOR OBSERVATIONS RELATING TO THE IMPLEMENTATION OF THIS STORMWATER POLLUTION PREVENTION PLAN, AND ACTIONS TAKEN IN ACCORDANCE WITH SCOTION ALS. SHALL BE MADE AND RETAINED AS PART OF THE PLAN FOR AT LEAST THREE

 (3) YEARS AFTER THE DATE OF THE INSPECTION. THE REPORT SHALL BE SIGNED IN

 ACCORDANCE WITH PART VI.G OF THE GENERAL PERMIT.
- D. IF ANY VIOLATION OF THE PROVISIONS OF THIS PLAN IS IDENTIFIED DURING THE CONDUCT OF THE CONSTRUCTION WORK COVERED BY THIS PLAN, THE RESIDENT ENGINEER OR RESIDENT TECHNICIAN SHALL COMPLETE AND FILE AN "INCIDENCE OF NONCOMPLIANCE" (ION) REPORT FOR THE IDENTIFIED VIOLATION. THE RESIDENT REGISTER OR RESIDENT TECHNICIAN SHALL USE FORMS PROVIDED BY THE ILLINOIS ENVIRONMENTAL PROTECTION AGENCY AND SHALL INCLUDE SPECIFIC INFORMATION ON THE CAUSE OF NONCOMPLIANCE, ACTIONS WHICH WERE TAKEN TO PREVENT ANY FURTHER CAUSES OF NONCOMPLIANCE, AND A STATEMENT DETAILING ANY ENVIRONMENTAL IMPACT WHICH MAY HAVE RESULTED FROM THE NONCOMPLIANCE. ALL REPORTS OF NONCOMPLIANCE SHALL BE SIGNED BY A RESPONSIBLE AUTHORITY IN ACCORDANCE WITH PART VI.G OF THE GENERAL PERMIT. THE REPORT OF NONCOMPLIANCE SHALL BE MAILED TO THE FOLLOWING ADDRESS: ILLINOIS ENVIRONMENTAL PROTECTION AGENCY

DIVISION OF WATER POLLUTION CONTROL ATTN: COMPLIANCE ASSURANCE SECTION 2200 CHURCHTLI ROAD POST OFFICE BOX 19276 SPRINGFIELD, IL 62794-9276

5. NON-STORMWATER DISCHARGES.

- EXCEPT FOR FLOWS FROM FIRE FIGHTING ACTIVITIES, SOURCES OF NON-STORMWATER THAT MAY BE COMBINED WITH STORMWATER DISCHARGES ASSOCIATED WITH THE INDUSTRIAL ACTIVITY ADDRESSED IN THIS PLAN, ARE DESCRIBED BELOW:
- 2. IRRIGATION DRAINAGE FOR VEGETATIVE GROWTH FOR SEEDING, ETC... THE POLLUTION PREVENTION MEASURES, AS DESCRIBED BELOW, WILL BE IMPLEMENTED FOR NON-STORMWATER COMPONENTS OF THE DISCHARGE:
- THE EROSION DUE TO IRRIGATION OF SEEDING SHALL BE CONSIDERED MINOR.

UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES WILL BE CONSTRUCTED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS IN THE ILLINOIS URBAN MANUAL REVISED JANUARY 1999

SOIL EROSION AND SEDIMENT CONTROL FEATURES SHALL BE CONSTRUCTED PRIOR TO THE COMMENCEMENT OF UPLAND DISTURBANCE, SOIL DISTURBANCE SHALL BE CONSTRUCTED IN SUCH A MANNER AS TO MINIMIZE EROSION, SOIL STABILIZATION MEASURES SHALL CONSIDER THE TIME OF YEAR, SITE CONDITIONS AND THE USE OF TEMPORARY OR PERMANENT DISTURBANCE OF AREAS NOT INCLUDED IN THE DESIGN SHALL REQUIR NOTIFICATION OF THE KOSCWO IN ACCORDANCE WITH THE 404 PERMIT SPECIAL CONDITIONS

SEDIMENT CONTROL PLAN SHALL BE MAINTAINED ON THE SITE AT ALL TIMES.

THE CONTRACTOR SHALL CONTACT THE KANE DUPAGE SOIL AND WATER CONSERVATION DISTRICT (KDSWCD). ONE WEEK PRIOR TO PRE-CONSTRUCTION CONFERENCE. ONE WEEK PRIOR TO LAND DISRIBUTING ACTIVITY. AS SOON AS THE INITIAL PROSION CONTROL ITEMS ARE INSTALLED AND ONE WEEK PRIOR TO FINAL INSPECTION.

2315 DEAN STREET, SUITE 100

THE EROSION CONTROL SHOWN ON THE PLANS ARE THE MINIMUM REQUIREMENTS. ADDITIONAL MEASURES MAY BE REQUIRED AS DIRECTED BY THE ENGINEER OR NCCSWCD.
ALL MEASURES SHALL BE IN PLACE WITHIN 3 DAYS OF DISTURBANCE.

THE CONTRACTOR SHALL MAINTAIN AND PRESERVE ANY EXISTING SUB-SURFACE DRAINAGE SYSTEMS (I.E. FIELD TILES) ACCORDING TO SECTION 611 OF THE IDOT STANDARD

ALL TEMPORARY FROSTON CONTROL MEASURES MUST BE MAINTAINED AND IMMEDIATELY REPLACED AS NEEDED AND AS DIRECTED BY THE ENGINEER. THE CONTRACTOR WILL BE RESPONSIBLE FOR ALL INSPECTION, MAINTENANCE AND REPAIR. THE CONTRACTOR SHALL INSPECT AND COMPLETE MAINTENANCE OF ALL ITEMS A MINIMUM OF EVERY 7 DAYS AND WITHIN 24 HOURS OF A 0.5-INCH RAIN, ALL TEMPORARY FROSION AND SEDIMENT CONTROL ITEMS, INCLUDING PERIMETER EROSION BARRIER, MUST BE REMOVED WITHIN 30 DAYS AFTER

PERIMETER EROSION BARRIER SHALL BE INSTALLED AT LOCATIONS SPECIFIED IN THE PLANS OR 1 FOOT INSIDE THE RIGHT-OF-WAY WHICH EVER IS CLOSER TO THE CENTER OF THE WORK, OR AS DIRECTED BY THE ENGINEER PRIOR TO THE START OF ANY EARTHWORK, CULVERT OR STORM SEWER CONSTRUCTION.

THE INSTALLATION, MAINTENANCE, REMOVAL AND RESTORATION OF THE AREA DISTURBED BY THE PLACEMENT OF THE PERIMETER EROSION BARRIER ARE INCLUDED IN THE CONTRACT UNIT PRICE FOR PERIMETER FROSION BARRIER, AFTER ALL PERIMETER FROSION BARRIER IS REMOVED THE AREAS DAMAGED BY THE PERIMETER EROSION BARRIER SHALL BE RESTORED.

THE CONTRACTOR SHALL CLEAN UP AND GRADE THE WORK AREA AS THE PROJECT PROGRESSES TO ELIMINATE THE CONCENTRATION OF RUNOFF, OR SHALL INSTALL APPROPRIATE SEDIMENT CONTROL DEVICES TO TRAP SEDIMENT, PAVEMENT SHALL BE CLEANED DAILY TO REMOVE EARTH MATERIAL TO THE SATISFACTION OF THE ENGINEER.

CONTRACTOR SHALL MAINTAIN OIL ARSORBENT BOOM DOWNSTREAM OF FOLIDMENT IN THE CHANNEL AT ALL TIMES. THE CONTRACTOR SHALL REPLACE THE OIL ABSORBENT BOOM WHEN IT

TEMPORARY SEEDING AND EROSION BLANKET SHALL BE COMPLETED IF FINAL SEEDING WILL

PRIOR TO COMMENCING LAND-DISTURBING ACTIVITIES OTHER THAN THOSE INDICATED ON THE PLANS (INCLUDING BUT NOT LIMITED TO ADDITIONAL PHASES OF THE DEVELOPMENT AND OFF-SITE BORROW OR WASTE AREAS) A SUPPLEMENTAL EROSION CONTROL PLAN SHALL BE SUBMITTED TO THE OWNER FOR REVIEW BY KDCSWD.

THE CONTRACTOR IS RESPONSIBLE FOR INSTALLATION OF ANY ADDITIONAL EROSION CONTROL MEASURES NECESSARY TO PREVENT EROSION AND SEDIMENTATION AS DETERMINED BY THE ENGINEER.

DURING DEWATERING OPERATIONS, WATER WILL BE PUMPED INTO SEDIMENT BASINS

ALL STORM SEWER INLETS SHALL BE PROTECTED WITH STORM SEWER INLET FILTERS PER

A COPY OF THE APPROVED EROSION AND SEDIMENT CONTROL PLAN SHALL BE MAINTAINED ON THE SITE AT ALL TIMES. STOCKPILES SHALL NOT BE LOCATED IN SPECIAL MANAGEMENT AREAS,

WHICH INCLUDE THE 100-YEAR FLOODPLAIN, WETLANDS AND WATERS OF THE US. EROSION CONTROL MEASURES MUST BE PROVIDED FOR ALL STOCKPILES THAT ARE IN PLACE FOR MORE THAN 3 DAYS.

FOR "SOIL PROTECTION SCHEDULE" AND "INSPECTION AND MAINTENANCE SCHEDULE" SEE "SEDIMENT AND EROSION CONTROL NOTES AND DETAILS" SHEET.

COUNTY SECTION SHEETS 03-00019-00-BR DUPAGE TO STA. STA. FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT CONTRACT NO. 83965

CONTROL MEASURE GROUP	CONTROL MEASURE	APPL.	KEY	CONTROL MEASURE CHARACTERISTICS					
	TEMPORARY SEEDING	X	(13)	PROVIDES QUICK TEMPORARY COVER TO CONTROL EROSION WHEN PERMANENT SEEDING IS NOT DESIRED OR TIME OF YEAR IS INAPPROPRIATE.	х				
VEGETATIVE SOIL	PERMANENT SEEDING	X	®	PROVIDES PERMANENT VEGETATIVE COVER TO CONTROL EROSION, FILTERS SEDIMENT FROM WATER. MAY BE PART OF FINAL LANDSCAPE PLAN.		×			
COVER	DORMANT SEEDING		6 3	SAME AS PERMANENT SEEDING EXCEPT IS DONE DURING DORMANT SEASON. HIGHER RATES OF SEED APPLICATION ARE REQUIRED.	х	х			
	SODDING		<u>s</u>	OUICK PERMANENT COVER TO CONTROL EROSION, OUICK WAY TO ESTABLISH VEGETATION FILTER STRIP. CAN BE USED ON STEEP SLOPES OR IN DRAINAGEMAYS WHERE SEEDING MAY BE DIFFICULT.	х	x			
4 J.	PLANTS, TREES & SHRUBS	X	@	PROVIDES CROUND COVER, SHRUBS AND TREES IN ADDITION TO PERMANENT VEGETATION. MAY BE USED AS PART OF A FINAL LANDSCAPE PLAN ALONG WITH SHRUBS AND TREES.		x			
NON VEGETATIVE SOIL	MULCHING		(4)	ADDED INSURANCE OF A SUCCESSFUL TEMPORARY OR PERMANENT SEEDING. CONTROLS UNMANTED VEGETATION AND PRESERVES MOISTURE. PROVIDES COVER WHERE VEGETATION CANNOT BE ESTABLISHED.					
COVER	EROSION BLANKET	\times	₿	PROTECTS THE SOIL SURFACE FROM RAINDROP IMPACTS AND OVERLAND FLOW DURING THE ESTABLISHMENT OF VEGETATION. REDUCES SOIL MOISTURE LOSS DUE TO EVAPORATION.	x	x			
	AGGREGATE COVER		Æ	PROVIDES SOIL COVER ON ROADS AND PARKING LOTS AND AREAS WHERE VEGETATION CANNOT BE ESTABLISHED. PREVENTS MUD FROM BEING PICKED UP AND TRANSPORTED OFF-SITE.	x	x			
	PAVING		P	PROVIDES PERMANENT COVER ON PARKING LOTS AND ROADS OR OTHER AREAS WHERE VEGETATION CANNOT BE ESTABLISHED.		x			
	RIDGE DIVERSION		œ	TYPICALLY USED ABOVE SLOPES. USED WHERE AN EXCESS OF SOIL IS AVAILABL	E.X	x			
	CHANNEL DIVERSION		@	TYPICALLY USED AT TOP OR BASE OF SLOPES. USED WHEN EXCESS SOIL IS NO AVAILABLE.	х	х			
DIVERSIONS	COMBINATION DIVERSION		600	TYPICALLY USED ANYWHERE ON A SLOPE. SOIL TAKEN OUT OF CHANNEL IS USED TO BUILD THE RIDGE.	x	х			
	CURB AND GUTTER		8	SPECIAL CASE OF DIVERSION USED IN CONJUNCTION WITH A STREET TO DIVERT WATER FROM AN AREA NEEDING PROTECTION.		х			
	BENCHES		B	SPECIAL CASE OF DIVERSION CONSTRUCTED WHEN WORKING ON CUT SLOPES TO SHORTEN LENGTH OF SLOPE AND ADD SLOPE STABILITY.	x	х			
	BARE CHANNEL		®	PROVIDES MEANS OF CONVEYING RUNOFF TO DESIRED LOCATION. MAY BE USED TO DRAIN DEPRESSIONAL AREAS. ONLY APPLICABLE WHEN VELOCITY OF FLOW IS VERY LOW.	x				
	STRUCTURAL STREAMBANK STABILIZATION		(83)	PROTECTS STREAMBANKS FROM EROSIVE FORCE OF FLOWING WATER		x			
WATERWAYS	VEGETATIVE CHANNEL		(8)	PROVIDED ADDED STABILITY TO CHANNEL. USED WHEN VELOCITY OF FLOW IS NOT EXTREMELY FAST.	x	Х			
	VEGETATIVE STREAMBANK STABILIZATION		(8)	PROTECTS STREAMBANKS FROM THE EROSIVE FORCE OF FLOWING WATER AND PROVIDES NATURAL, PLEASING APPEARANCE		х			
	LINED CHANNEL		(2)	USED WHEN VEGETATION WILL NOT PROTECT THE CHANNEL AGAINST HIGH VELOCITIES OF FLOW OR WHERE VEGETATION CANNOT BE ESTABLISHED.	х	х			
ENCLOSED	STORM SEWER		(3)	CAN BE USED TO CONVEY SEDIMENT LADEN WATER TO SEDIMENT BASIN OR IN CONJUNCTION WITH A WATERWAY.		х			
DRAINAGE	UNDERDRAIN		(9)	USED TO LOWER WATER TABLE AND INTERCEPT GROUNDWATER FOR BETTER VEGETATION GROWTH AND SLOPE STABILITY. USED TO CARRY BASE FLOW IN WATERWAYS AND TO DEWATER SEDIMENT BASINS.	х	x			
	STRAIGHT PIPE SPILLWAY		(33)	USED FOR RELATIVELY SMALL VERTICAL DROPS AND SMALL FLOWS OF WATER.		×			
	DROP INLET PIPE SPILLWAY		(13)	SAME AS PIPE SPILLWAY EXCEPT LARGER FLOWS AND LARGE VERTICAL DROPS CAN BE ACCOMMODATED.		х			
SPILLWAYS	WEIR SPILLWAY		®	USED FOR RELATIVELY SMALL VERTICAL DROPS AND FLOWS MUCH GREATER THAN PIPE STRUCTURES.	х	х			
	BOX INLET WEIR SPILLWAY		BS)	SAME AS WEIR SPILLWAY EXCEPT LARGER FLOWS CAN BE ACCOMMODATED BECAUSE OF LOWER WEIR LENGTH.	х	х			
OUTLETS	LINED APRON		(A)	PROTECTS DOWNSTREAM CHANNEL FROM HIGH VELOCITY OF FLOW DISCHARGING FROM STRUCTURES.	х	х			
	EMBANKMENT SEDIMENT BASIN		(ES)	USED WHERE TOPOGRAPHY LENDS ITSELF TO CONSTRUCTING A DAM AND EARTH FILL IS AVAILABLE.	х	х			
SEDIMENT BASINS	EXCAVATED SEDIMENT BASIN		(8)	USED WHERE EMBANKMENT COULD CAUSE A HAZARD DOWNSTREAM IN CASE OF FAILURE AND WHEN EXCESS EARTH FILL IS NOT AVAILABLE.	х	х			
	COMBINATION SEDIMENT BASIN		©	USED WHEN TOPOGRAPHY IS SUITABLE BUT ADDITIONAL CAPACITY IS NEEDED.	х	х			
SEDIMENT	BARRIER FILTER		⊕	USED FOR SINGLE LOTS OR DRAINAGE AREAS LESS THAN 1/2 ACRE TO FILTER SEDIMENT FROM RUNOFF.	х				
FILTERS	VEGETATIVE FILTER		ℯ	USED ALONG DRAINAGEWAYS OR PROPERTY LINES TO FILTER SEDIMENT FROM RUNOFF. SIZE MUST BE INCREASED IN PROPORTION TO DRAINAGE AREA.	х	х			
	FILTER FABRIC	X	(FF)	USED FOR ROADWAY CURB INLETS.	X	х			
MUD AND	STABILIZED CONST. ENTRANCE	X	(SE)	PREVENT MUD FROM BEING PICKED UP AND CARRIED OFF-SITE.	х	х			
DUST CONTROL	DUST AND TRAFFIC CONTROL		(10)	PREVENTS DUST FROM LEAVING CONSTRUCTION SITE.	x	х			
CONTRACTOR C	CONTRACTOR CERTIFICATION SUB-CONTRACTOR RESPONSIBLE FOR:								

'I CERTIFY UNDER PENALTY OF LAW THAT I UNDERSTAND THE TERMS AND CONDITIONS OF THE GENERAL NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) PERMIT (ILRIO) THAT AUTHORIZES THE STORM WATER DISCHARGES ASSOCIATED WITH INDUSTRIAL ACTIVITY FROM THE CONSTRUCTION SITE IDENTIFIED AS PART OF THIS CERTIFICATION"	SIGNATURE TITLE DATE			
GENERAL CONTRACTOR	WITNESSED BY OWNER			
STGNATURE TITLE DATE	SIGNATURE TITLE DATE			
COMPANY	COMPANY			

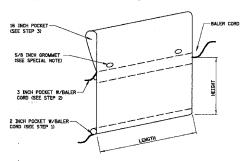
REVISIONS	
NAME DATE	ILLINOIS DEPARTMENT OF TRANSPORTATION
	TIMBER EDGE DRIVE
	SEDIMENT AND EROSION CONTRO
	NOTES
	SCALE: N.T.S. DRAWN BY BEH
	DATE 8/24/2007 CHECKED BY LMF

<u>m</u>

To assemble on-site you need three laborers with chest-high rubber waders or hip boots, 9/16 wrench, 3 utility knives, steel tee-posts or galvanized sign posts, sledge or a post driver and a roll of plastic electrical tape.

Lay out the sections of the curtain on the shore where it is to be installed. Open bags of chains and cables with clamps.

- A. Tie the black cord that runs through the bottom pocket to the end of the correct length chain and pull the chain into place in the bottom pocket. Cut a slit in the pocket and curtain at the ends and tie the chain in place with a short piece of black cord.
- B. Tie the black card in the small top pocket to the correct length cable (5-10 ft longer than the curtain section), cover the knot and cable with tape so it will slide through the pocket. Pull the cable in place with the black cord.
- C. Push the foam float blocks through the top pocket, one person installing the blocks and two more people pushing them along through the pocket.
- D. Take a short piece of black cord and tie the ends of the block pocket to keep the blocks from floating loose.
- E. Proceed with the next section of curtain, (repeat steps A through D) and lash the 1st section of curtain to the 2nd section with a piece of yellow rope through the grommets at the end of each section. Fasten the cables from each section together with cable clamps.
- F. When the whole curtain for the location is assembled, take 3-5 ft pieces of black cord and bundle the curtain, folding the heavy fabric accordion-fashion and tie the whole thing, around the blocks, cable, curtain & chain into long "sausage links". Place the block cords 4 to 8 ft. apart.
- G. Pull the whole "sausage links" into the water and float into the desired position. This is where the waders come into use.
- H. Drive the tee-post on shore at the ends of the curtain and wrap the cable around the post and fasten with clamps.



SILT CURTAIN

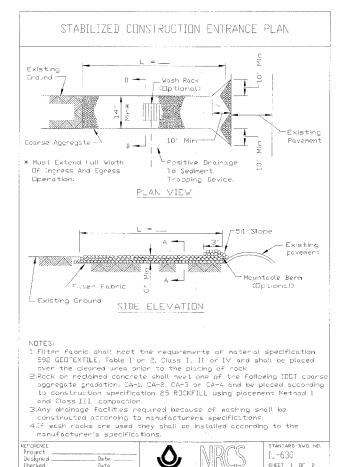
STABILIZATION TYPE	SEPT.	ост.	NOV.	DEC.	JAN.	FEB.	MAR.	APR.
PERMANENT SEEDING								+A
DORMANT SEEDING			+ <u>B</u>					
TEMPORARY SEEDING N/A								
SODDING N/A								
EROSION BLANKET								+ <u>B</u>

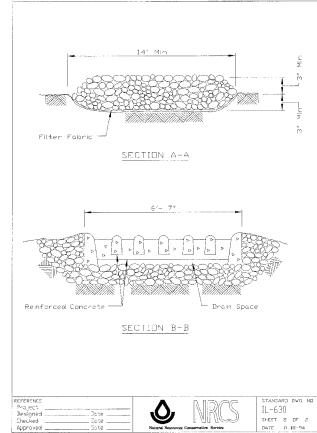
-Straw Bale BEDDING DETAIL Angle First Stake Toward Previously Laid Bale Re-Bars, Steel Pickets Or 2"x 2" Stakes Driven 1,5 Ft. To 2 Ft. Into Snound, Drive Stakes Flush With Bales. ANCHORING DETAIL 1. Bales shall be placed at the top of slope or on the contour and in a row with ends tightly abutting the adjacent bales. 2. Each bale shall be embedded in the soil a minimum of 4", and placed so that bindings are horizontal.

3 Bales shall be securely anchored in place by either two stakes on re-bars driven through the bale. The first stake in each bale shall be driven toward the previously lad bale at an angle to fonce the bales together. Stakes shall be on-ven flush with the bale.

4. Inspection shall be inequent and repair replacement shall be made promptly as needed. 5. Baies shall be removed when they have served their usefulness so as not to block or impede storm flow or drainage IL-635 SHEET 1 OF

STRAW BALE BARRIER PLAN





SECTION COUNTY
03-00019-00-BR DUPAGE

CONTRACT NO. 83965

STA.

STABILIZED CONSTRUCTION ENTRANCE PLAN

TO STA.

FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT

EROSION CONTROL BLANKET

EROSION CONTROL BLANKET SHALL BE 100% BIODEGRADABLE.

SITE CONDITION

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE VERIFICATION THAT ALL EROSION CONTROL MEASURES ARE FUNCTIONING PROPERLY AT ALL TIMES. THE CONTRACTOR SHALL REMOVE ALL EQUIPMENT, MATERIALS AND DEBRIS FROM THE CREEK AT THE END OF EACH DAY.

EMERGENCY SPILL CONTAINMENT

THE CONTRACTOR SHALL CONDUCT A DAILY INSPECTION OF ANY HYDRAULIC EQUIPMENT THAT WILL BE WORKING IN OR ADJACENT TO THE CREEK. THE INSPECTION REPORT WILL BE FILED WITH THE OWNER OR ITS ENGINEER. EQUIPMENT SHOWING SIGNS OF ANY FLUID LOSS WILL NOT BE PERMITTED TO WORK IN OR ADJACENT TO THE CREEK.

THE CONTRACTOR MUST ALSO HAVE AN EMERGENCY SPILL CONTAINMENT KIT AT THE SITE IN CLOSE PROXIMITY TO ANY HYDRAULICALLY OPERATED EQUIPMENT IN OR ADJACENT TO THE CREEK. THE KIT SHALL HAVE CONTAINMENT CAPACITY EQUAL TO OR GREATER THAN THE FULL VOLUME OF HYDRAULIC FLUID IN THE LARGEST PIECE OF EQUIPMENT BEING OPERATED IN OR ADJACENT TO THE CREEK.

REVISIONS		OF TO 110000T1TTO1
NAME DA	TE ILLINOIS DEPARTMENT	OF TRANSPORTATION
	SEDIMENT AND E	DGE DRIVE ROSION CONTROL ID DETAILS
	MOILS AN	D DETAILS
	SCALE: 1'	DRAWN BY BEH
	DATE 8/24/2007	CHECKED BY LMF

INSPECTION AND MAINTENANCE SCHEDULE

TROUTE TOTAL MINITUTE TRANSCE SETTEMBLE						
RESPONSIBLE PARTY	DURATION					
CONTRACTOR	NOVEMBER 2007 TO APRIL 2008					
ENGINEER	WEEKLY AND AFTER EACH					
	SIGNIFICANT RAINFALL EVENT					
CONTRACTOR	1 YEAR FROM COMPLETION					
VILLAGE OF OAKBROOK	ONGOING FROM CONSTRUCTION					
TERRACE	COMPLETION					
	RESPONSIBLE PARTY CONTRACTOR ENGINEER CONTRACTOR VILLAGE OF OAKBROOK					

PROPOSED SCHEDULE

IMPLEMENT EROSION	STREAMBANK	
CONTROL	STABILIZATION	RESTORATION
PHASE I		ı
NOVEMBER 2007		APRIL 2008

/2007 N:BOakbrookTB03608BCIV

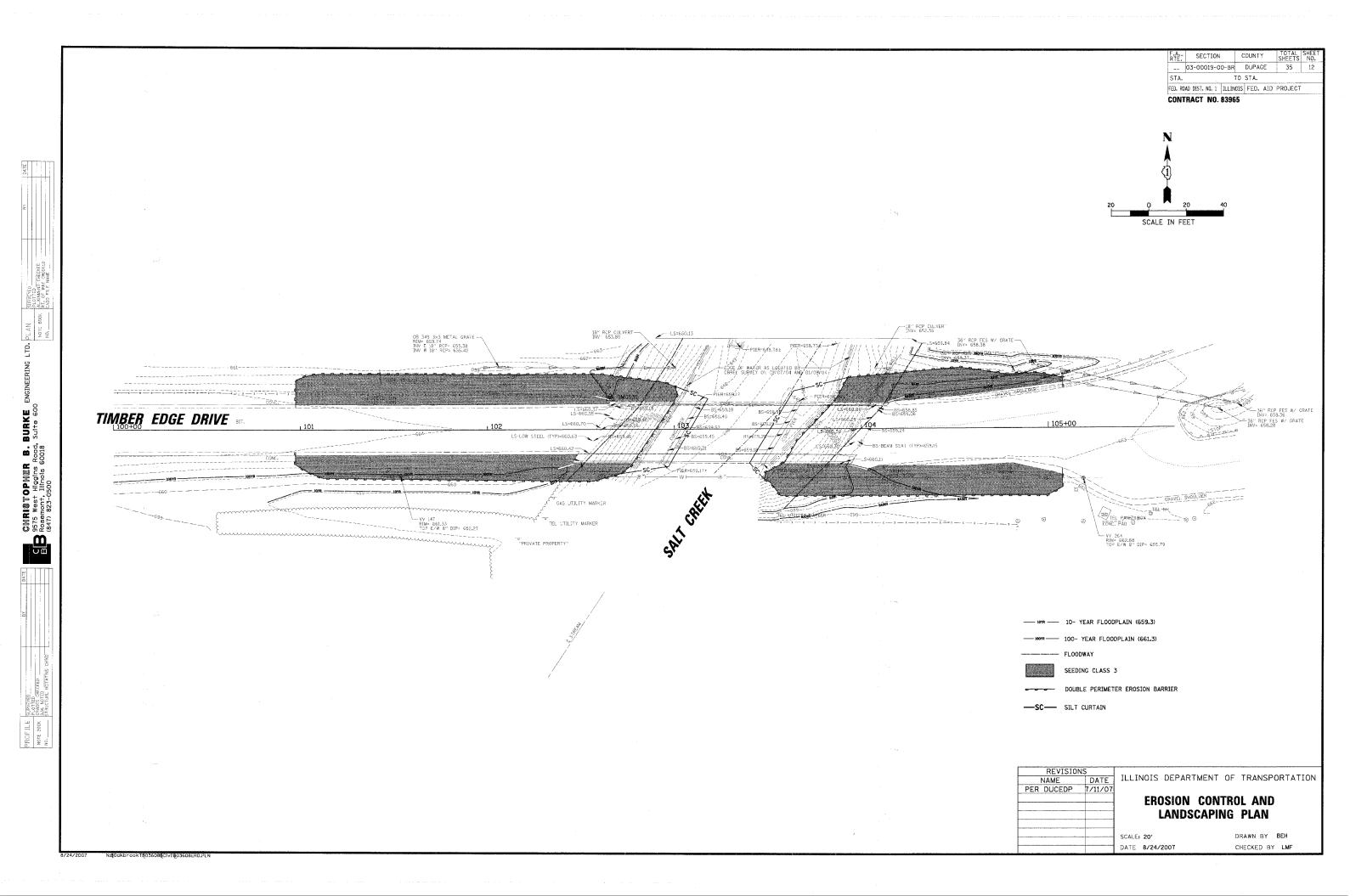
TOPHER B. BURKE ENGTH Higgins Road, Suite 600 1, Illinois 60018

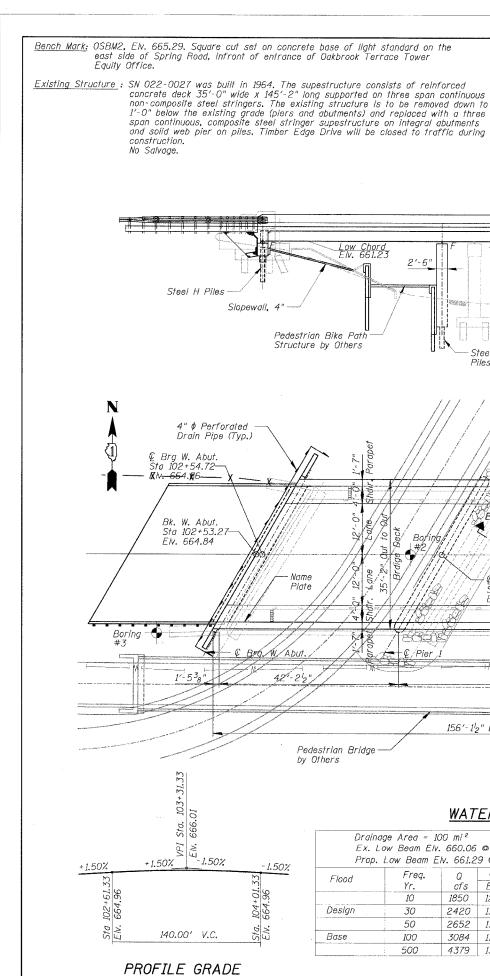
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| I.L. | SURVEYED | BY | CARON CARON

PROFIL NOTE BO

8/24/2007





N:BOakbrookT803608BStruct803608-S1/PLN

Structure by Others

Pedestrian Bridge

Flood

Design

Base

Drainage Area = 100 mi 2

Frea.

10

30

50

100

500

Ex. Low Beam Elv. 660.06 @ 104+08

Prop. Low Beam Elv. 661.29 @ 102+44

cfs

1850

2420

2652

3084

4379

by Others

-1.50%

VPI - 1.50%

SHEET NO. 13 1016L SHEET SHEETS NO. 13-00019-00-BR * 35 DHPAGE 35 SHEETS Contract #83965

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. .	0 (8) (5)
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S-5	Approach Slab Elevations
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S-12	East Abutment Plan and Elevation

S-13 Pier 1 - Detail S-14 Pier 2 - Detail

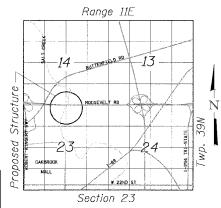
S-15 Drainage Scupper DS-12 S-16 Bar Splicer Assembly Details

S-17 Anchor Bolts Details

S-18 Boring Logs Boring Logs S-19

> SALT CREEK BUILT BY CITY OF OAKBROOK TERRACE SEC. 03-00019-00-BR STA. 103+31.33 STR. NO. 022-6000 LOADING HS20

NAME PLATE



LOCATION SKETCH

GENERAL PLAN AND ELEVATION

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+51.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

DRAWING NUMBER DATE ISAMI BY: PDR 103T5 8/24/2027 S-1

No Salvage. Traffic Barrier terminal Std. 631031, Type 6, Typ. W30x108 (COMPOSITE) 100-yr WSEL 661.30 30-yr WSEL 660.18 At Rt 2'-6" Appr. B/Channel Elv. 645.08 NWL Elev. 649.00 Steel H Piles Steel H Piles Slopewall, 4 Slopewall, 41 Pedestrian Bike Path

ELEVATION

Underwater Structure -Excavation Location 2 4" \(Perforated \) Drainage Scupper Drain Pipe (Typ.) ACOTYP.) Underwater Structure Excavation Location 1 Bk. E. Abut.— Sta 104+09.39 € Brg W. Abut. ¯(Тур**.**) Sta 102+54.72-Elv. 664.84 XIV.-664.186-Boring #6 ∕© Brg E. Abut. Sta 104+07.95 Bk. W. Abut. Boring #101 & PGL Timber, Sta 102+53.27-Elv. 664.84 Edge Drive Elv. 664.86 Name Plate Boring /#102 & 1024 / Bridge Approach Pavement (Std. 420401) Typ. Ea. Eft Sta 102+96.92 Sta 103+65.73 Elv. 665.36 -Existing Watermain Boring 🗣 🗜 Brg. E. Abut. 68'-934 1'-538" 42'-22 156'-1'2" Bk to Bk of Abutments

PLAN

Ex. Low Grade Elv. 663.48 @ 105+00

Created Head-ft

Exist.

0.01

0.00

0.01

0.02

0.05

Prop. Low Grade Elv. 663.48 @ 105+00

Prop.

0.11

0.05

0.09

0.02

0.00

Headwater El.

659.26

Exist. Prop.

660.18 660.23

660.66 660.74

661.32 661.32

664.09 664.04

659**.**16

WATERWAY INFORMATION TABLE

Nat.

H.W.F.

659.15

660.18

660,65

661.30

1539 | 664.04

DESIGN SPECIFICATIONS

Bridges 17th Edition.

Certify That To The Best Of My Knowledge, Information And Belief,

This Bridge Design Is Structurally Adequate For The Design Loading Shown On The Plans. The Design Is An Economical One For The Syle

Standard Specification For Highway And Bridges".

Of Structure And Complies With Requirements Of The Current "AASHTO

SEISMIC DATA

Seismic Performance Category (SPC) = A Bedrock Acceleration Coefficient = 0.037g

Site Coefficient = 1.0

OF ILLING

WOBASS!

081-005058

STRUCTURE

MADID MOBASSERI

EXPIRATION DATE: II/30/08

ILLINOIS REGISTRATION No. 081-005058

LOADING HS20-44

Allow 50 #/sq. ft. for future wearing surface

DESIGN STRESSES

2002 AASHTO Standard Specifications for Highway

FIELD UNITS

 $f'_{0} = 3,500 \text{ psi}$

- Steel H Piles

= 50,000 psi (structural steel) (M270 Grade 50) = 60,000 psi (Reinf.)

Opening ft2

Prop.

1256

1387

1450

1539

Exist.

1256

1369

1369

1369

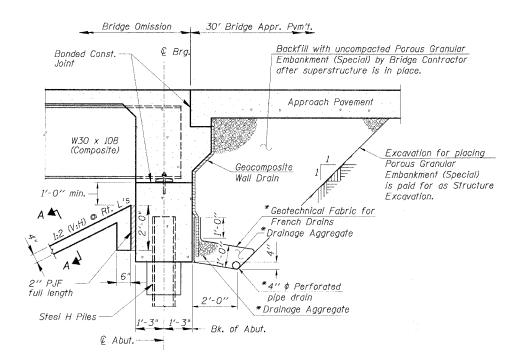
1369

-Steel H

Piles

GENERAL NOTES

- 1. Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts (in painted areas and M164 Type 3 in unpainted areas). Bolts '8 in. \$\phi\$, holes \$^{16}_{16}\$ in. \$\phi\$,
- 2. Calculated weight of Structural Steel = 89620 Lb. Grade 50 = 6080 Lb. Grade 36
- 3. No field welding is permitted except as specified in the contract documents.
- 4. Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60 (IL Modified). See Special Provisions
- 5. Reinforcement bars designated (E) shall be epoxy coated.
- 6. Bearing set surfaces shall be constructed or adjusted to the designated elevations within a tolerance of $l_8^{\,\prime\prime}$ in, (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- 7. The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 8. The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Interstate Green, Munsell No. 7.5G 4/8. See Special Provision for "Cleaning and Painting New Metal Structures".
- Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.
- 10. The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.
- 11. The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at substructures specified or approved by the Engineer before ordering the remainder of piles.
- 12. Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.
- 13. Two 'g in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.



*Included in the cost of Pipe Underdrains for Structures.

All drainage components shall extend inside face to inside face of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

> Edge of deck --5'-0"

SECTION THRU INTEGRAL ABUTMENT

(Horiz. dim. @ Rt. L's)

Stone Dumped Riprap, Class A4

SECTION A-A

SECTION B-B

TOTAL SHEET'S NO. SHEET NO. 14 * 03-00019-00-BR 35 SHEETS DUPAGE 35

Contract #83965

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment (Special)	Cu. Yd.		100	100
Stone Dumped Riprap, Class A4	Sq. Yd.			190
Removal of Existing Structures	Each	1		1
Structure Excavation	Cu. Yd.		320	320
Concrete Structures	Cu. Yd.		182.2	182.2
Concrete Superstructure	Cu. Yd.	200.6		200.6
Bridge Deck Grooving	Sq. Yd.	520		520
Protective Coat	Sq. Yd.	710		710
Furnishing & Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	2595		2595
Reinforcement Bars, Epoxy Coated	Pound.	46360	11420	57780
Slope Walls 4 Inch	Sq. Yd.		210	210
Furnishing Steel Piles HP10x42	Foot		575	575
Furnishing Steel Piles HP12x53	Foot		535	535
Driving Piles	Foot		1110	1110
Test Pile Steel HP10x42	Each		2	2
Test Pile Steel HP12x53	Each		2	2
Name Plates	Each	1		1
Geocomposite Wall Drain	Sq. Yd.		72	72
Pipe Underdrain for Structures, 4"	Foot		128	128
Underwater Structure Excavation	Each		1	1
Protection Location 1				
Underwater Structure Excavation	Each		1	1
Protection Location 2				
Drainage Scupper DS-12	Each	4		4
Pile Shoes	Each		20	20
Bar Splicers	Each	64		64

GENERAL NOTES AND DETAILS

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY
F.A. ROUTE 7. SEC. 03-00019-00-BR
STATION 103+31.33 STRUCTURE NO. 022-6000

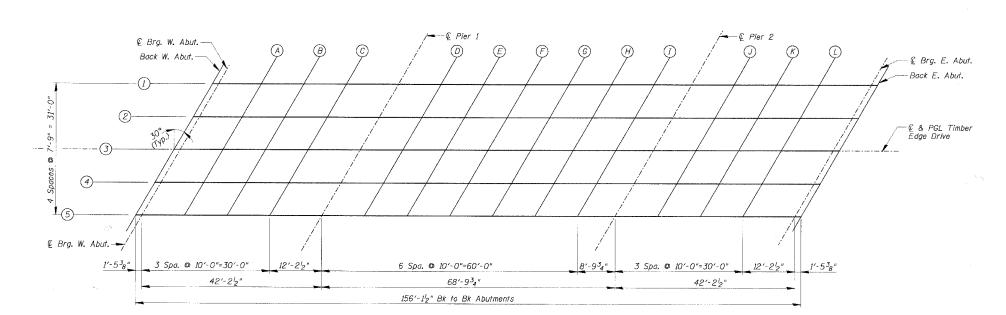
CHRISTOPHER B. BURKE ENGINEERING LTD.
9575 West Higgins Road, Suite 600
Rosemont, Illinois 60018

REVISIONS DATE	MOTER DIMENSIONAL DATA IS NOT TO BE OBTAINED BY SCALING ANY PORTION OF THIS DRAWING.	DRAWING NUMBER
	PRINCET NO. PRINCET NO.	S-2

N:段OakbrookT段03608段Struct段03608-S2.pln

SHEET NO. 15 # 03-00019 DUPAGE 35 35 SHEETS

Contract #83965



PLAN

BEAM 1

	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
	Bk. W. Abutment	102+62.222	15.50	664.723	664.723
	CL Brg W. Abut.	102+63.665	<i>1</i> 5.50	664.744	664.744
	Α	102+73.665	15.50	664.879	664.885
	В	102+83.665	<i>1</i> 5 . 50	664.992	664.996
	С	102+93.665	15.50	665.083	665.079
	CL. Pier 1	103+05.874	15.50	665 . 166	665.166
	D	103+15,874	15.50	665.209	665.246
	E	103+25,874	15.50	665.232	665.311
	F	103+35.874	15.50	665.233	665.337
	G	103+45.874	15.50	665.212	665.315
	Н	103+55.874	15.50	665.170	665.245
i	I	103+65.874	15.50	665.107	665.138
	CL. Pier 2	103+74.686	15.50	665.034	665.034
	J	103+84.686	15.50	664.930	664.925
	K	103+94.686	15.50	664 . 805	664.808
	L	104+04.686	15.50	664.660	664.667
	CL Brg E. Abut.	104+16.894	15.50	664.477	664.477
	Bk. E. Abutment	104+18.338	15.50	664.455	664.455

BEAM 2

	Γ	1		1
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abutment	102+57.747	7.75	664.790	664.790
CL Brg W. Abut.	102+59.191	7.75	664.812	664.812
А	102+69.191	7.75	664.955	664.962
В	102+79.191	7.75	665.077	665.082
С	102+89.191	7.75	665.178	665.175
CL. Pler 1	103+01.399	7.75	665.273	665.273
D	103+11.399	7.75	665.326	665.362
E	103+21,399	7.75	665.358	665.437
F	103+31.399	7.75	665.369	665.473
G	103+41.399	7.75	665.358	665.461
Н	103+51.399	7.75	665.326	665.400
I	103+61.399	7.75	665,272	665.303
CL. Pier 2	103+70.212	7.75	665,207	665.207
J	103+80.212	7.75	665.113	665.108
К	103+90.212	7.75	664.997	665.000
L	104+00.212	7,75	664.860	664.867
CL Brg E. Abut.	104+12.420	7.75	664.677	664.677
Bk. E. Abutment	104+13.863	7.75	664.656	664.656

BEAM 3, @ ROADWAY & P.G.L.

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abutment	102+53.273	0.00	664.839	664.839
CL Brg W. Abut.	102+54.716	0.00	664.861	664.861
Α	102+64.716	0.00	665.010	665.0 <i>1</i> 6
В	102+74.716	0.00	665.142	665.146
<i>C</i>	102+84.716	0.00	665.252	665,249
CL. Pler 1	102+96.925	0.00	665.358	665.358
D	103+06.925	0.00	665 . 421	665.457
E	103+16.925	0.00	665.463	665.542
F	103+26.925	0.00	665.483	665.588
G	103+36.925	0.00	665.482	665 . 585
Н	103+46.925	0.00	665.459	665 . 533
I	103+56.925	0.00	665 . 415	665 . 446
CL. Pier 2	103+65.737	0.00	665.358	665.358
J	103+75.737	0.00	665.274	665.269
K	103+85.737	0.00	665.168	665.171
L	103+95.737	0.00	665,041	665.048
CL Brg E. Abut.	104+07.946	0.00	664,861	664,861
Bk. E. Abutment	104+09.389	0.00	664.839	664.839

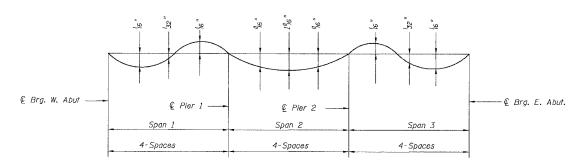
TOP OF DECK ELEVATIONS I

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-0019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

REVISIONS NAME DATE	NOTE: DIMENSIONAL DATA IS NOT TO BE DETAINED BY SCALING NOT PORTION OF THIS DRAWING.	DRAWING NUMBER
	CELISED N1	S-3

N:@OakbrookT@03608&Struct@03608-S3.PLN

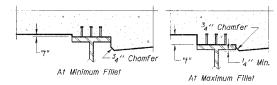


DEAD LOAD DEFLECTION DIAGRAM

(includes weight of concrete deck only)

Note:

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below and on S-3.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below and on sheet S-3. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below and on sheet S-3, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS

<u>BEAM 4</u>

				~~~~
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abutment	102+48.799	- 7.75	664,656	664.656
ÇL Brg W. Abut.	102+50.242	-7.75	664.677	664.677
A	102+60.242	-7.75	664.827	664.834
В	102+70.242	-7.75	664.969	664.974
С	102+80.242	- 7.75	665.089	665.086
CL. Pier 1	102+92.450	-7.75	665.207	665,207
D	103+02.450	- 7.75	665.279	665.316
Ε	103+12.450	- 7.75	665.331	665.409
F	103+22.450	-7.75	665.360	665.465
G	103+32.450	- 7.75	665.369	665.471
Н	103+42.450	- 7.75	665.356	665.430
I	103+52.450	- 7.75	665.321	665.352
CL. Pier 2	103+61,263	- 7.75	665.273	665.273
<u>J</u>	103+71.263	- 7.75	665.198	665.193
K	103+81.263	-7.75	665.102	665.105
L	103+91.263	- 7.75	664.984	664.991
CL Brg E. Abut.	104+03.471	-7.75	664.812	664.812
Bk. E. Abutment	104+04.914	- 7.75	664.790	664.790
			1	

#### BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abutment	102+44.324	- 15.50	664.455	664.455
CL Brg W. Abut.	102+45.767	-15.50	664.477	664.477
A	102+55.767	- 15.50	664.627	664.633
В	102+65.767	- 15.50	664.774	664.779
С	102+75.767	- 15.50	664.904	664.901
CL. Pier 1	102+87.976	- 15.50	665.034	665.034
D	102+97.976	- 15.50	665.116	665.152
E	103+07.976	-15.50	665,177	665.255
F	103+17.976	- <i>1</i> 5.50	665.216	665.321
G	103+27.976	- 15.50	665.234	665.337
H	103+37.976	- 15.50	665.230	665.305
I	103+47.976	-15.50	665.205	665.236
CL. Pier 2	103+56.788	-15.50	665.166	665.166
J	103+66.788	-15.50	665,100	665.095
K	103+76.788	- 15.50	665.014	665.017
L	103+86.788	-15.50	664.905	664.912
CL Brg E. Abut.	103+98.997	- 15.50	564.744	664.744
Bk. E. Abutment	104+00.440	- 15.50	664.723	664.723

Contract #83965

TOP OF DECK ELEVATIONS II

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103-31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinots 60018

REVISIONS DATE	WHITE DIMENSIONAL DATA IS NOT TO BE DETAINED BY SCALING DAY PORTION OF THIS DRAWING.	DRAWING NUMBER
	0550860 PT	S-4

N:#0akbrookT#03608#Struct#03608-S4.PLN

#### NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
End W. Appr. Pav't.	102+32.51 102+42.51	16.00′ LT	664.27 664.42
В	102+42.51	16.00° LT	664.57
Bk. W. Abutment	102+62.51	16.00′ LT	664.72

#### NORTH EDGE OF PAVEMENT

Location 4,5	Station	Offset	Theoretical Grade Elevations
End W. Appr. Pav't.	102+30,20	12.00′ LT	664.31
Α	102+40,20	12.00' LT	664.46
В	102+50.20	12.00′ LT	664.61
Bk. W. Abutment	102+60,20	12.00′ LT	664.76

## @ ROADWAY & P.G.L.

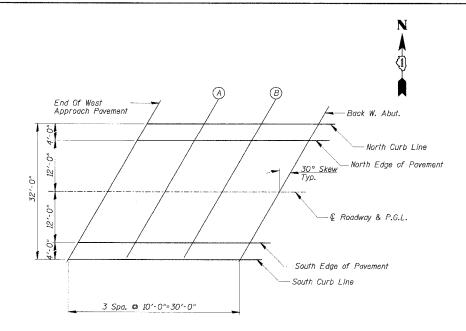
Location	Station	Offset	Theoretical Grade Elevations
End W. Appr. Pav't.	102+23,27	0.00′	664.39
A	102+33.27	0.00′	664.54
В	102+43.27	0.00'	664.69
Bk. W. Abutment	102+53.27	0.00′	664.84

#### SOUTH CURB LINE

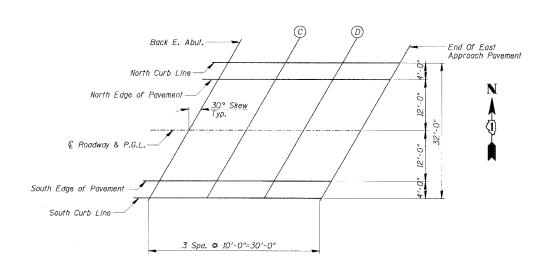
Location	Station	Offset	Theoretical Grade Elevations
End W. Appr. Pav't.  A  B  BK. W. Abutment	102+14.03 102+24.03 102+34.03 102+44.03	16.00' RT 16.00' RT 16.00' RT 16.00' RT	663.93 664.06 664.21 664.36

#### SOUTH EDGE OF PAVEMENT

	·		
Location	Station	Offset	Theoretical Grade Elevations
End W. Appr. Pav't.	102+16.34	12.00' RT	664.12
A B	102+26.34 102+36.34	12.00' RT	664.26 664.40
Bk. W. Abutment	102+46.34	12.00' RT	664.55



#### WEST APPROACH PAVEMENT



EAST APPROACH PAVEMENT

#### NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment C D End E. Appr. Pav't.	104+18.62 104+28.62 104+38.62 104+48.62	16.00' LT 16.00' LT 16.00' LT	664.44 664.29 664.14 663.99

ROUTE NO.	SECTION	co	por r	TOTAL SAFETS	SHEET NO.	SHEET NO. $17$
*	03-00019- 00-BR	DUP.	AGE	35	DOWNEY	<i>3</i> 5 sheets
PER MONO C	IST. NO. :	(LINON	FEO. ALL PR	D./KC7-		

## NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	104+16.31	12.00′ LT	664.56
C	104+26.31	12.00′ LT	664.41
D	104+36,31	12.00′ LT	664.26
End E. Appr. Pav't.	104+46.31	12.00′ LT	664.11

#### ROADWAY & P.G.L.

Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment C D End E. Appr. Pay't.	104+09.38 104+19.38 104+29.38 104+39.38	0.00' 0.00' 0.00'	664.84 664.69 664.54 664.39

#### SOUTH CURB LINE

Location	Station	Offset	Theoretica Grade Elevations
Bk. E. Abutment C D End E. Appr. Pav't.	104+00.15	16.00' RT	664.72
	104+10.15	16.00' RT	664.57
	104+20.15	16.00' RT	664.42
	104+30.15	16.00' RT	664.27

#### SOUTH EDGE OF PAVEMENT

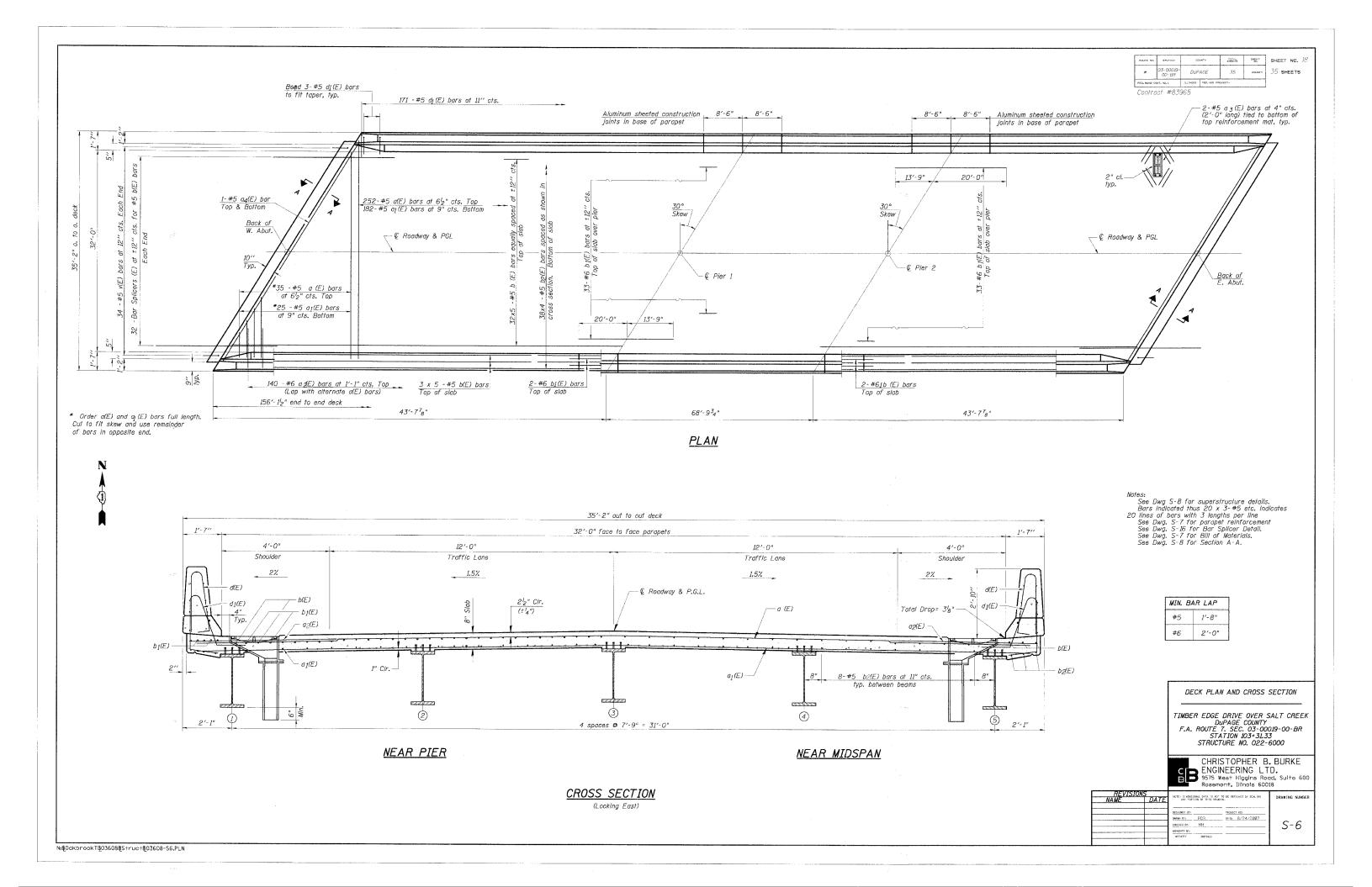
Location	Station	Offset	Theoretical Grade Elevations
Bk. E. Abutment	104+02,46	12.00′ RT	664.76
	104+12.46	12.00′ RT	664.61
D	104+22.46	12.00' RT	664.46
End E. Appr. Pav't.	104+32.46	12.00' RT	664.31

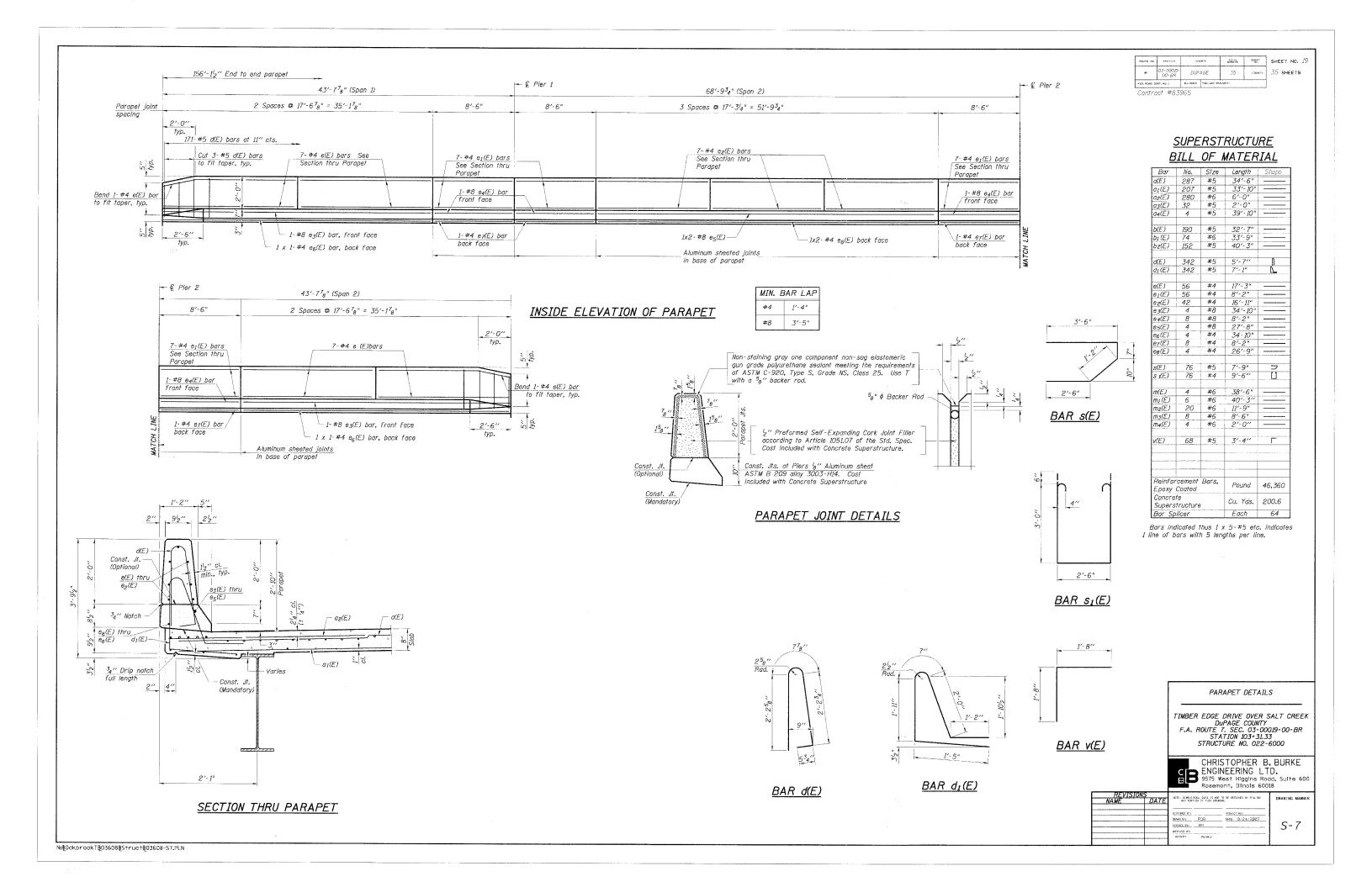
APPROACH SLAB ELEVATIONS

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

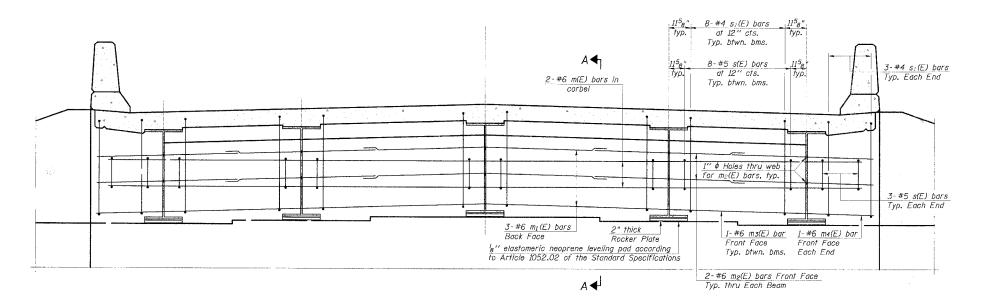
REVISIONS DATE	NOTE: DIMENSIONAL DATA IS NOT TO REPORTAINED BY SOM INC. ANY PORTION OF THIS DRAWING.	DRAWING NUMBER
	PAC-FET NOT	S-5







Contract #83965



#### DIAPHRAGM ELEVATION AT ABUTMENT

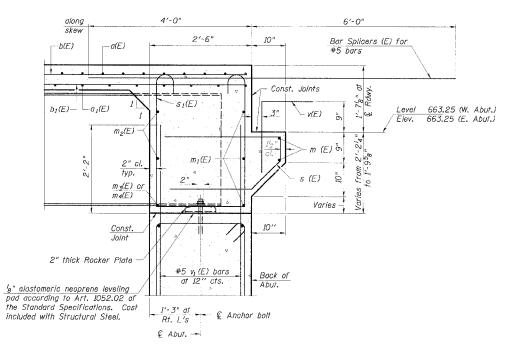
Notes: Reinforcement bars in diaphragm are billed with superstructure on sheet S-7.

Concrete in diaphragm is included with Concrete
Superstructure on sheet S-7.
For details of bars s(E) & s(E) see sheet S-7.
The s(E) and s1 (E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.

The s(E) and s1(E) bars shall be placed along skew.

#### MIN. BAR LAP

#6 bar = 2'-9"



#### SECTION A-A

Dimensions at right angles to abutment, except as shown.

SUPERSTRUCTURE DETAILS

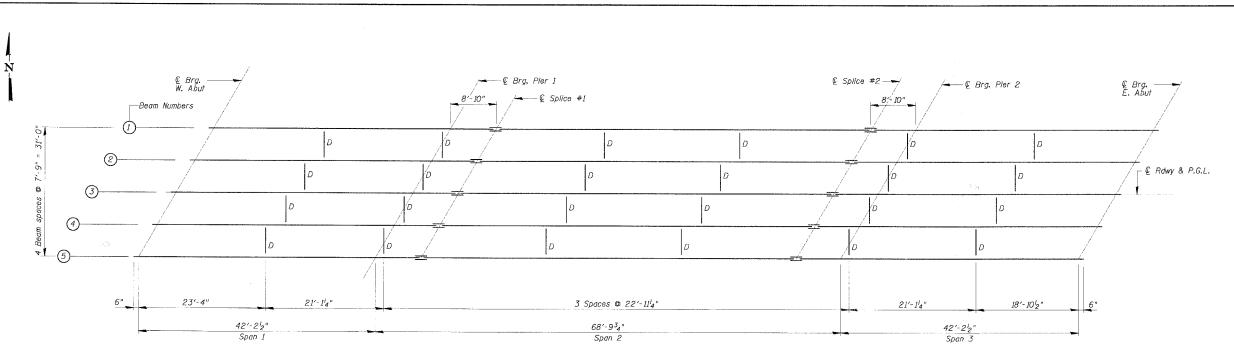
TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000



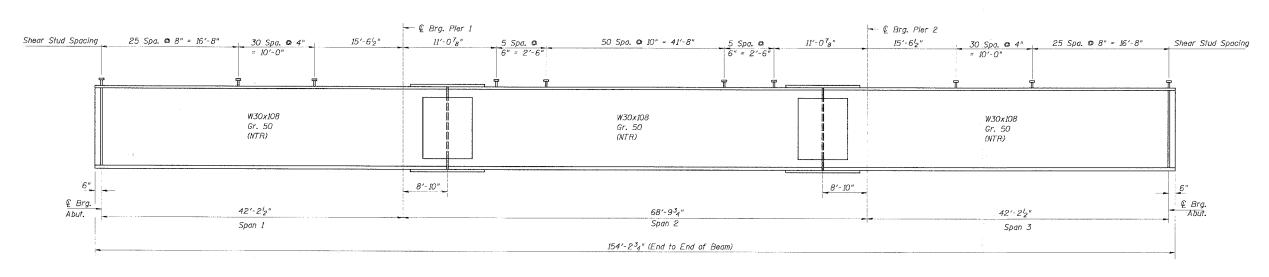
CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

	Roselloni, Illinois 600	10
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	Page   Page	S-8

N:#0akbrookT#03608#Struct#03608-S8.PLN



# FRAMING PLAN



#### **GIRDER ELEVATION**

#### TOP OF BEAM ELEVATIONS-BEFORE DEFLECTION (For Fabrication use only)

LOCATION	BEAM 1	BEAM 2	BEAM 3	BEAM 4	BEAM 5
Brg. W. Abut.	663.994	664.062	664.111	663.928	663.727
© Brg. Pier 1	664.323	664.431	664.517	664.367	664.196
	664.425	664.541	664.635	664.492	664,327
	664.327	664.492	664.635	664.541	664.425
Brg. Pier 2	664.196	664.367	664.517	664.431	664.323
© Brg. E. Abut.	663.727	663,928	664.111	664.062	663.994

FOUTE 404	SECTION	DOUNTY	TETAL SHEETS	SHEET 40.	SHEET NO. 2
*	03-00019- 00-BR	DUPAGE	35	COUNTY	35 sheets
PED. 8040 31	57. NO. 1	BLUNDES FED. 410 PS	олест-		

Contract #83965

- N.T.R. designates members subject to the supplemental requirements for notch toughness (Zone 2).
- All structural steel for W30x108 beams and splice plates shall be AASHTO M270 Grade 50.
- Fasteners shall be high strength bolts, conforming to AASHTO M-164 Specification (ASTM A 325). Bolts ⁷₈ "\$, open holes ¹⁵₁₆ "\$, unless noted otherwise.

FRAMING PLAN AND DETAILS

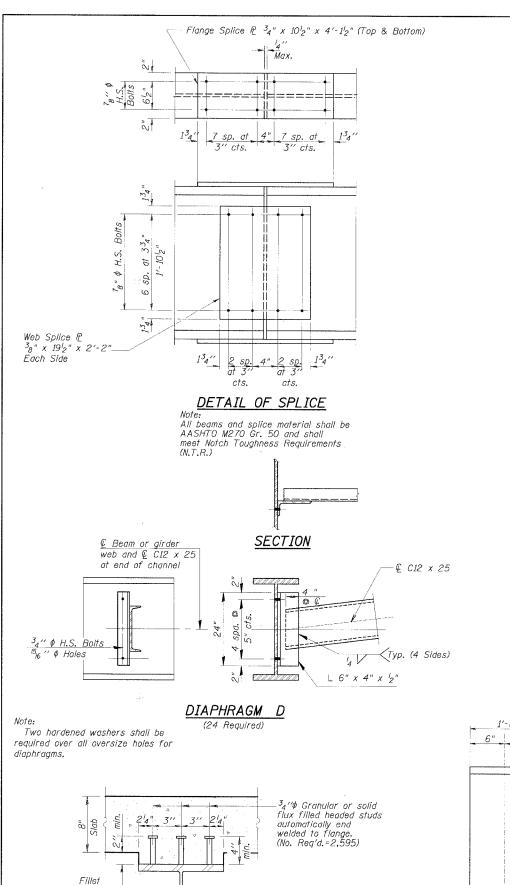
TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000

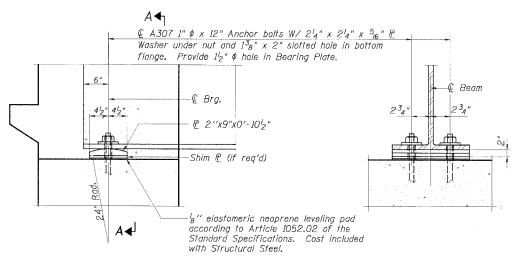


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#### **ELEVATION**

#### SECTION A-A

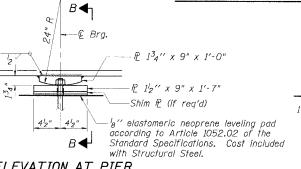
1³8"¢ Holes-1" deep in top ₽

 $1_2'' \phi$  Holes in bottom  $\mathbb{R}$ .

- $\[ \]$  A307 1"  $\phi$  x 12" Anchor bolts with  $2^{1}_{4}$ " x  $2^{1}_{4}$ " x  $5_{16}$ " P washer under nut

for 14" o pintles. Thread or press fit in bottom P.

#### FIXED BEARING AT NORTH & SOUTH ABUTMENT



#### ELEVATION AT PIER

## FIXED BEARING AT PIERS 1 & 2



#### PINTLE

#### SHIM PLATES AT ABUTMENTS

SECTION B-B

Ε.	Abutment	Beam	2	¹³ 16 "	Х	9"	Х	10½"
W.	Abutment	Beam	4	¹³ 16 "	х	9"	Х	10½"



SHEET NO. 22

35 SHEETS

Contract #83965

	INTERIOR	GIRDER MO	MENT TABLE	-
		0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or Pier 2	0.5 Sp. 2
	Is (in4)	4470	4470	4470
	Ic (n) (in4)	13105		13105
	Ic (3n) (in4)	9879		9879
	Ss (in ³ )	300	300	300
	Sc (n) (in ³ )	459		459
	Sc (3n) (in ³ )	417		417
	Z (in 3) Q (k/ft.)	l		
	₽ (k/ft.)	0.910	1.42	0.910
	M2 ('k)	69	458	226
	s₽ (k/ft.)	0.510		0.510
	Ms@ ('k)	52		159
	MŁ ('k)	291	218	490
	M ( $Imp$ ) ('k)	87	61	126
	53[M½+M(Imp)] ('k)	630	463	1027
	Ma ('k)	977	1197	1836
*	mu (11)	1932	1249	1963
	fs⊉ non-comp (k.s.i.)		18,3	9.0
	fs⊉(comp) (k.s.i.)			4.6
	$fs5_3(4+Imp)$ (k.s.i.)		18.6	26.9
	fs (Overload) (k.s.i.)		36.9	40.5
*	7.0 (1.0.0)	27.0	47.9	52.7
	VR (k)	61		65
w				

* Non-Compact Section

INTERIOR	GIRL	DER REACT.	ION TABLE
		Abut.	Pier
R <b>Q</b>	(k)	19.2	89.9
R4	(k)	42.8	52.7
Imp.	(k)	12.8	14.7
R (Total)	(k)	74.8	157.3

Is. Ss: Non-composite moment of inertia and section modulus of the steel section used for computing f (Total and Overload) due to non-composite dead loads (in., and in., s).  $I_c(n)$ ,  $S_c(n)$ : Composite moment of inertia and section modulus of the steel

and deck based upon the modular ratio, "n", used for computing f (Total and Overload) due to short-term composite live loads (th.4 and in.3).

Io(3n), So(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for

computing f (Total and Overload) due to long-term composite (superimposed) dead loads (in.4 and in.3).

Z: Plastic Section Modulus of the steel section in non-composite

Un-factored non-composite dead load (kips/ft.).

M§: Un-factored moment due to non-composite dead load kep-1 s§: Un-factored long-term composite (superimposed) dead load Un-factored moment due to non-composite dead load (kip-ft.). (kips/ft.)

Ms 2: Un-factored moment due to long-term composite (superimposed)

dead load (kip-ft.).
Mt: Un-factored live load moment (kip-ft.).

Mimp: Un-factored moment due to impact (kip-ft.).

Ma: Factored design moment (kip-ft.). 1.3 [ M $\frac{9}{2}$  +  $\frac{5}{3}$  (M $\frac{4}{2}$  + M_{Imp})]

Mu: Compact composite moment capacity according to AASHTO LFD 10.50.1.1. or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).

f_s (Overload): Sum of stresses as computed from the moments below (ksi).  $M \varrho + M_s \varrho + \frac{5}{3} (M_{\xi} + M_{Imp})$  f_s (Total): Sum of stresses as computed from the moments below on

non-compact section (ksi).

1.3 [Mê + Msê + 5 (M£ +  $M_{Imp}$ )]

VR: Maximum½ + impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).

#### STEEL DETAILS

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY
F.A. ROUTE 7. SEC. 03-00019-00-BR
STATION 103+31.33 STRUCTURE NO. 022-6000



CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

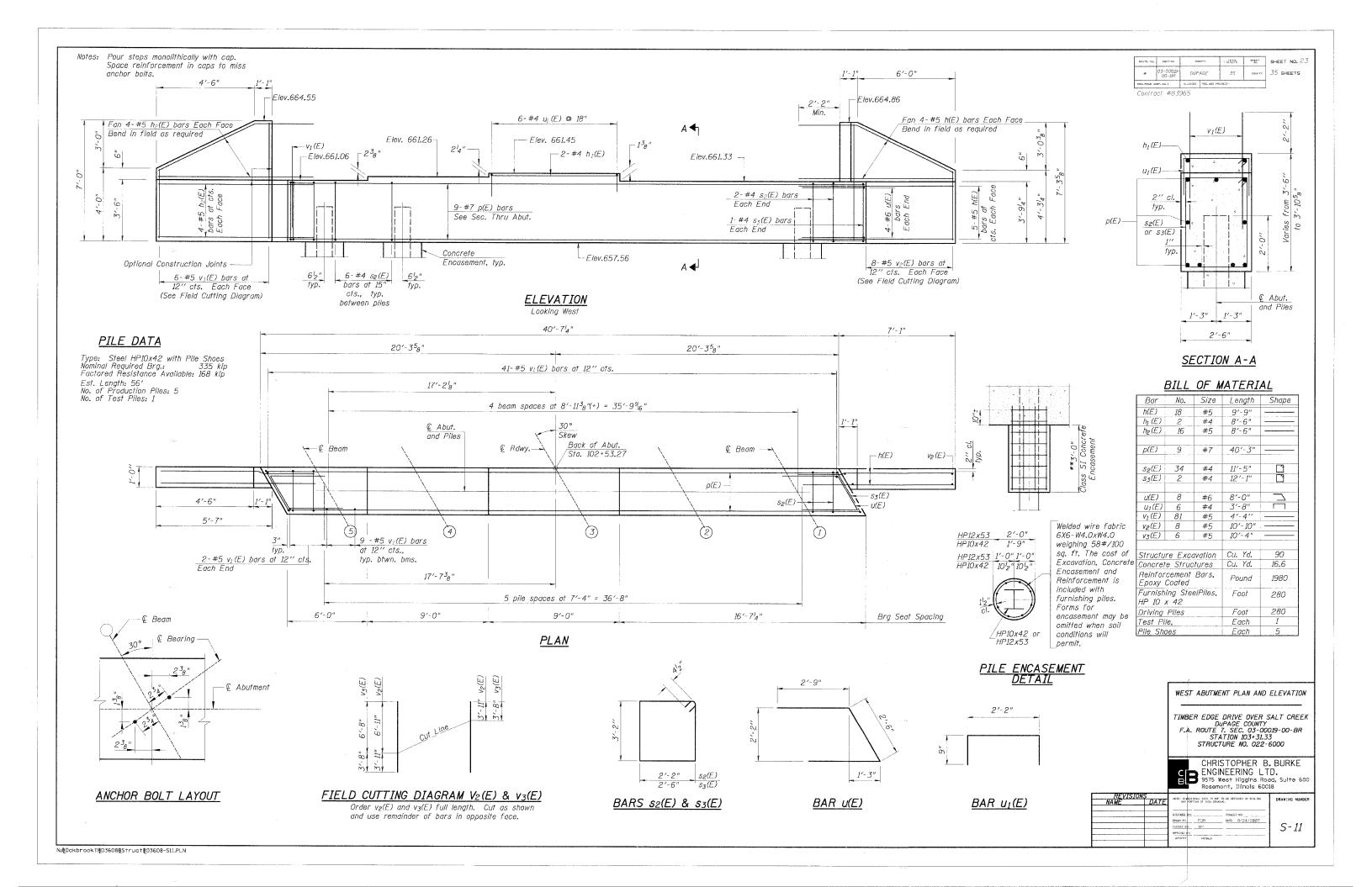
REVISIONS DATE	NOTE: DIMENSIONAL DATA IS NOT TO BE CETAINED BY SCALING ANY PORTION OF THIS DRAWING.	DRAWING NUMBER
	OCCUPATO BY:   PROJECT NO.	S-10

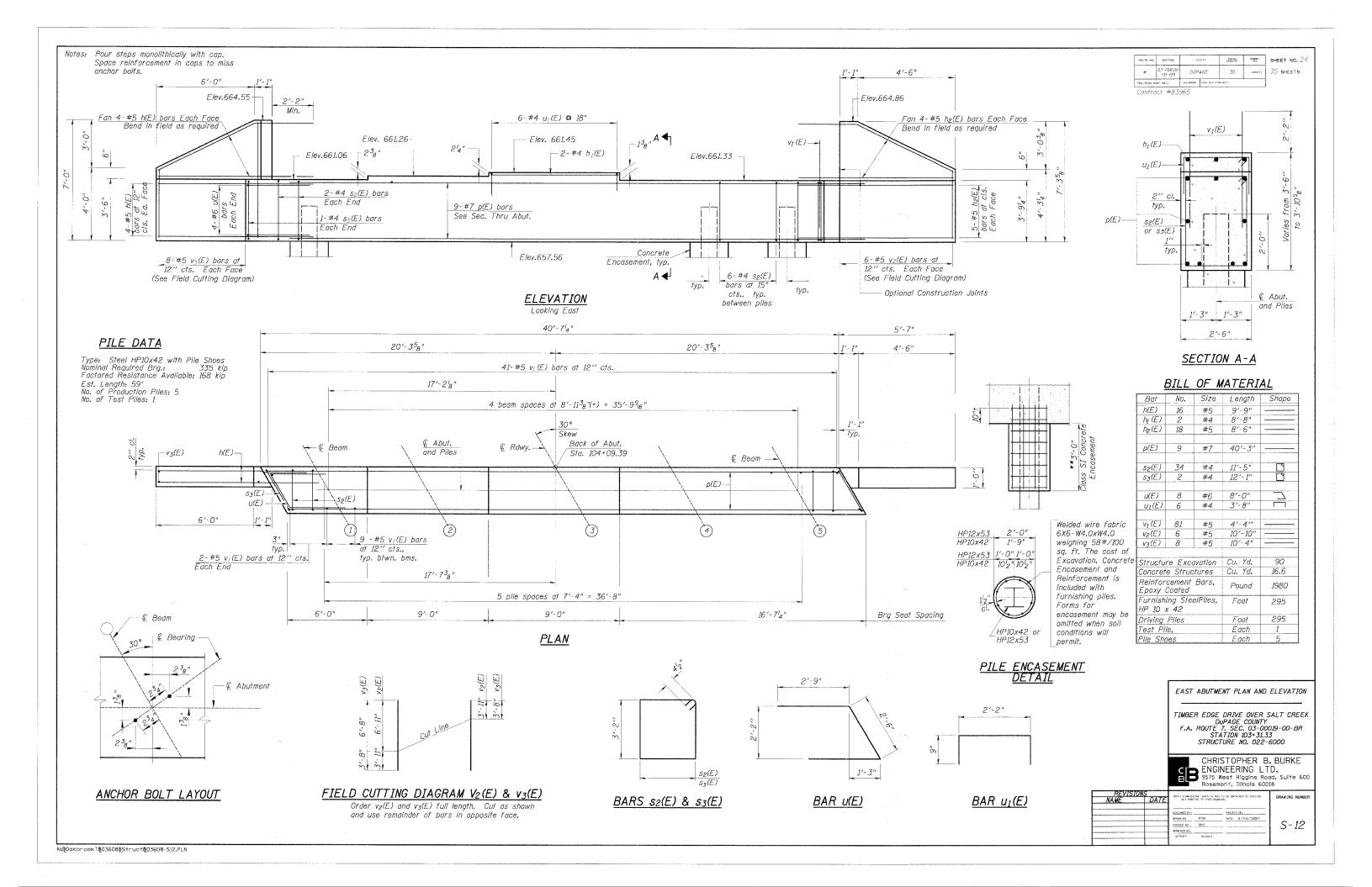
1'-8" 6" ! 1'-2" 2 1" Holes in beam for m2(E) bars. See Dwg. S-8. Cost of drilling holes is included in contract unit price for Furnishing and Erecting Structural Steel.

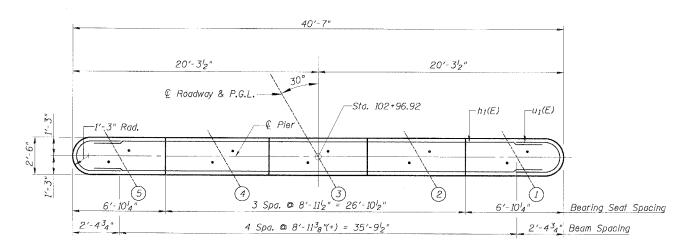
TYPICAL END OF BEAM ELEVATION

Varies

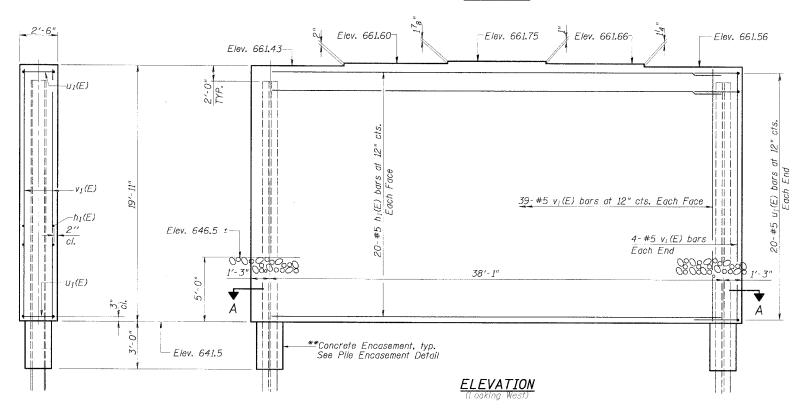
SECTION C-C







#### TOP PLAN



#### **END VIEW**

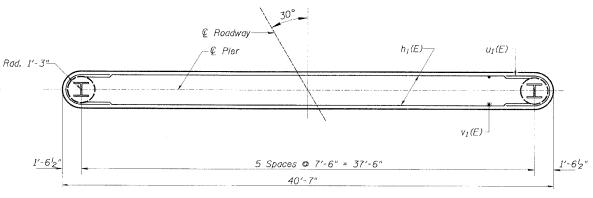
#### PILE DATA

Notes: Space reinforcement in cap to miss

Pour steps monolithically with cap.

anchor bolts.

Type: Steel HP12x53 with Pile Shoes Nominal Required Brg.: 419 k Factored Resistance Available: 209 k Est. Length: 54' No. of Production Piles: 5 No. of Test Piles: 1



SECTION A-A

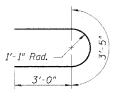
TOTAL SMEET NO. sheet no.25 * 03-00019-00-BR 35 35 SHEETS DUPAGE COUNTY

Contract #83965

#### BILL OF MATERIAL

Bar	No.	Size	Length	Shape
$h_1(E)$	40	#5	38'-0"	
			07.5"	
$u_1(E)$	40	#5	9′-5"	-
V1(E)	86	#5	19'-7"	
*1(_/	20	77.5	13 1	
Concrete	Struc	tures	Cu. Yd.	74.5
Reinforc		Bars,	Pound	3,730
Epoxy Co				
Structure			Cu. Yd.	70
Furnishir		Foot	270	
Piles HP		Foot	270	
Driving F		F 001	2/0	
Test Pile Steel HP12x53			Each	1
Underwater Structure			A P. C. and A	
Excavation Protection			Each	1
Location	1			
Pile Shoe	S		Each	5

Reinforcement Bars designated (E) shall be epoxy coated.



BAR U1(E)

#### HP12x53 HP10x42 1'-9" HP12x53 1'-0" 1'-0" HP10x42 10¹2" 10¹2"



-Welded wire fabric 6X6-W4.0xW4.0 weighing 58#/100 sq. ft. The cost of Excavation, Concrete Encasement and Reinforcement is included with furnishing piles. Forms for encasement may be omitted when soil conditions will permit.

# PILE ENCASEMENT DETAIL

-⊈ Beam

ANCHOR BOLT LAYOUT

∣ @ Bearing

**Forms shall be placed below Elevation 641.50 after excavation for Pier walls. Reinforcement and Concrete Encasement shall be poured underwater into forms. The cost of Concrete Encasement, Reinforcement, form excavation, and furnishing and placing forms is included with furnishing piles. If a portion of the pier wall is under water, concrete shall be trimied under water into forms according to Article 503.08 of the Standard Specifications. Concrete shall be trimied to an Elevation 1'-0" above the water level at the time of Construction.

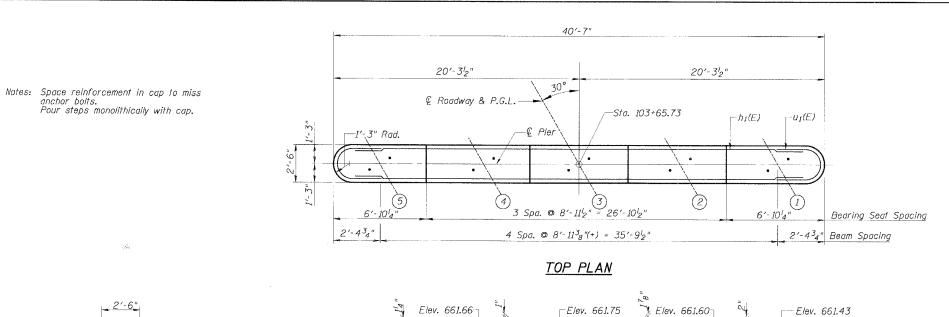
PIER 1 - DETAIL

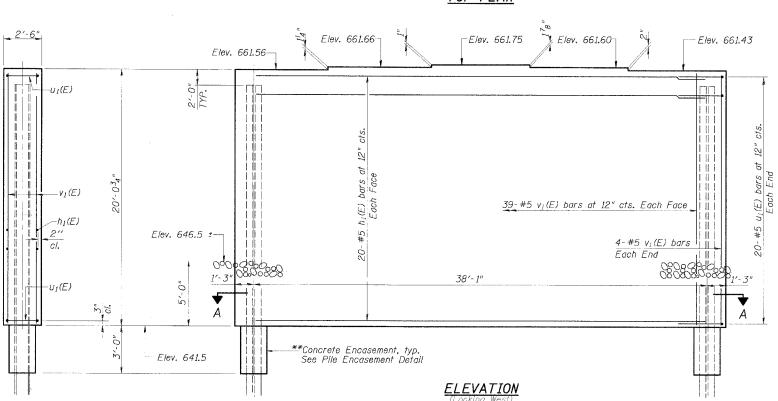
TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103-31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

REVISION:	S   DATE	NETT: DIMENSIONAL DATA IS NOT TO ANY PORTION OF THIS DEADLINE	BE CREATINED BY SCALING	DRAWING NUMBER
		26518460 871  086491 971  086491 971  0862060 871  MM  46779070 871  871794.8	PROJECT NO: DATE: 8/24/2007	S-13

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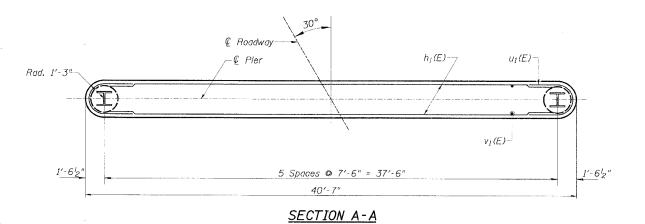


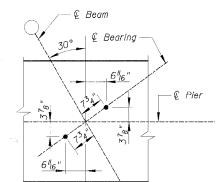


#### **END VIEW**

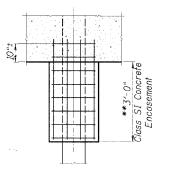
#### PILE DATA

Type: Steel HP12x53 with Pile Shoes Nominal Required Brg.: 419 k Factored Resistance Available: 209 k Est. Length: 53' No. of Production Piles: 5 No. of Test Piles: 1





#### ANCHOR BOLT LAYOUT



Contract #83968

<u>BI</u>	LL C	F M.	ATERI	TAL
Bar	No.	Size	Length	Shape
$h_I(E)$	40	#5	38'-0"	
u ₁ (E)	40	#5	9'-5"	$\Rightarrow$
v1(E)	00	#5	19'- 7"	
VICE	86	#5	19 - 7	
Concrete			Cu. Yd.	74.5
Reinford Epoxy Co		Pound	3,730	
Structure	е Ехсо	Cu. Yd.	70	
Furnishir Piles HP		Foot	265	
Driving F		Foot	265	
Test Pile HP12x53	Steel			1
I'M IL NOO				

DUPAGE

SHEET NO.26 35 SHEETS

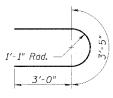
chosty

35

Reinforcement Bars designated (E) shall be epoxy coated.

Underwater Structure Excavation Protection Each

Location 2



BAR u1(E)

HP12x53 HP10x42 HP12x53 1'-0" 1'-0" HP10x42 10¹2" 10¹2"

HP10x42 or HP12x53

-Welded wire fabric 6X6-W4.0xW4.0 weighing 58#/100 sq. ft. The cost of Excavation, Concrete Encasement and Reinforcement is included with furnishing piles. Forms for encasement may be omitted when soil conditions will permit.

# PILE ENCASEMENT DETAIL

**Forms shall be placed below Elevation 641.50 after excavation for Pier walls. Reinforcement and Concrete Encasement shall be poured underwater into forms. The cost of Concrete Encasement, Reinforcement, form excavation, and furnishing and placing forms is included with furnishing piles. If a portion of the pier wall is under water, concrete shall be trimled under water into forms according to Article 503.08 of the Standard Specifications. Concrete shall be trimied to an Elevation 1'-0" above the water level at the time of Construction.

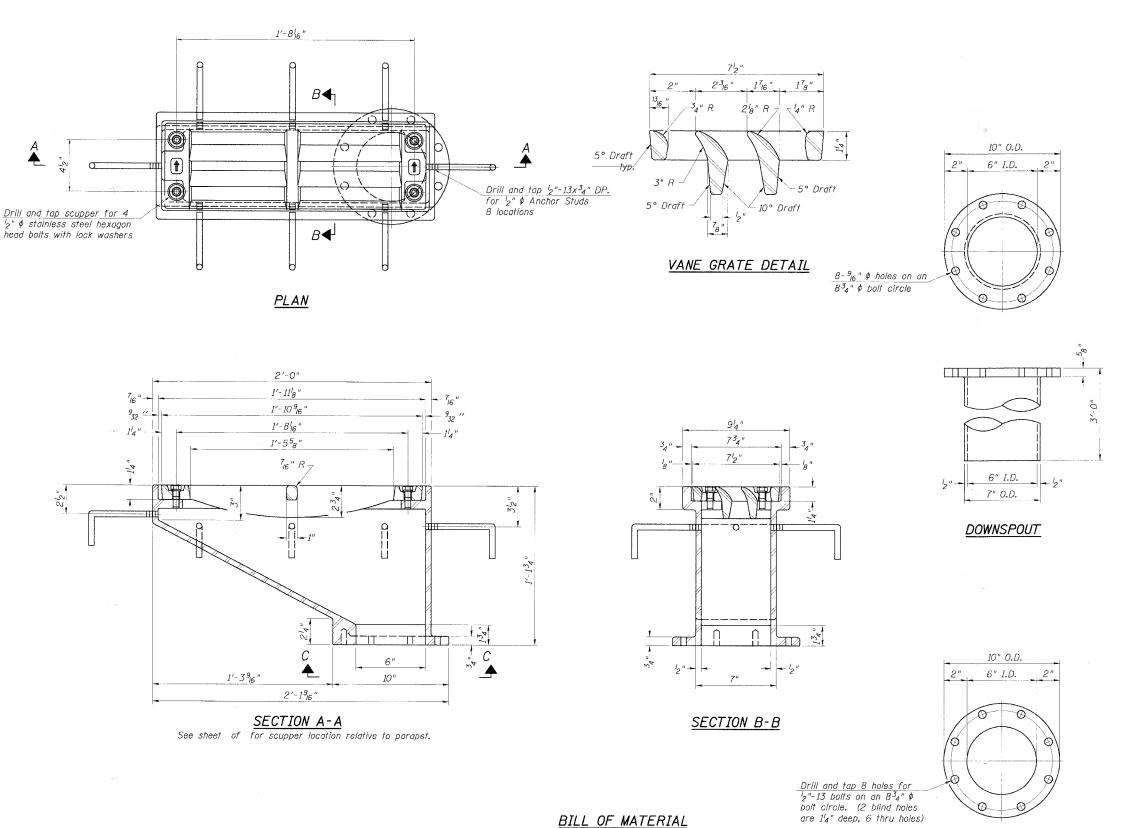
PIER 2 - DETAIL

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 osemont, Illinois 60018

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		DESIGNED BY1  URAWAY URY CHECKED BY2  APPROVED BY4  ACTIVITY  DIGITALS	PROJECT 431 DATE: 8/24/2007	S-	14

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ROUTE NO.	86 CTUBN	ca	INTY	TOTAL SHEETS	SHECT NO.	SHEET NO. 27
*	03 · 00019 · 00 · BR	DUP.	AGE	35	COLNTY	35 снеетс
FED. ROAD CO	ST. NO. 1	ELL INCES	PEG. NJC PR	OUECT~		

Notes:

Contract #83965

All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.

Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.

Downspouts located on the exterior side of a painted steel fascia beam shall be painted with the finish coat specified for the exterior side of the fascia beam.

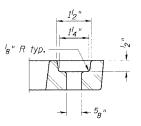
As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.

Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M111.

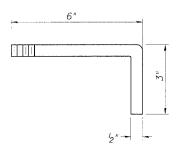
The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.

Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-12.

Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel eauivalent.



#### BOLT HOLE DETAIL



#### ANCHOR STUD DETAIL

DRAINAGE SCUPPER, DS-12

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

REVISIONS NAME DATE	NATE: DIMENSIONAL DATA IS NOT TO SE DETAINED BY SCALING ANY PORTION OF THIS DRAWLING.	DRAWING NUMBER
		S-15

UNIT QUANTIT Drainage Scupper, DS-12

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VIEW C-C



Contract #83965

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

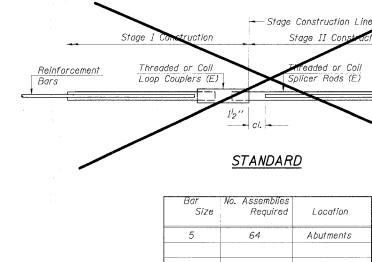
Minimum Capacity (Tension in kips) =  $1.25 \times fy \times A_t$ 

Minimum *Pull-out Strength =  $0.66 \times fy \times A_t$ (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi. A_t = Tensile stress area of lapped reinforcement bars.

* = 28 day concrete

	BAR SPLIC	ER ASSEMBLI	ES
Bar Size to	Splicer Rod or	Strengt	h Requirements
be Splice	d Dowel Bar Lengi	Min. Capacity kips - tens.	Min. Pull-Out Strength ion kips - tension
		<i>'</i>	,
#4	1'-8''	14.7	7.9
#5	2'-0"	23.0	12.3
#6	2'-7''	33.1	17.4
#7	3′-5″	45.1	23.8
#8	4'-6''	58.9	31.3
#9	5′-9′′	75.0	39.6
#10	7′-3′′	95.0	50.3
#11	9'-0''	117.4	61.8



	Bar Size	No. Assemblies Required	Location
1	5	64	Abutments

BAR SPLICER ASSEMBLY DETAILS

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+51.33 STRUCTURE NO. 022-6000



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Reinforcement

Bars

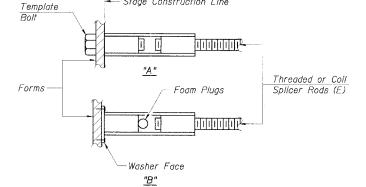


ROLLED THREAD DOWEL BAR

** ONE PIECE -Wire Connector WELDED SECTIONS

#### BAR SPLICER ASSEMBLY ALTERNATIVES

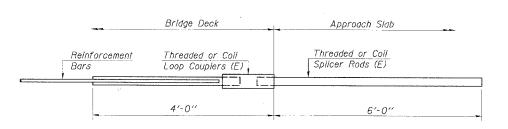
** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



Stage Construction Line

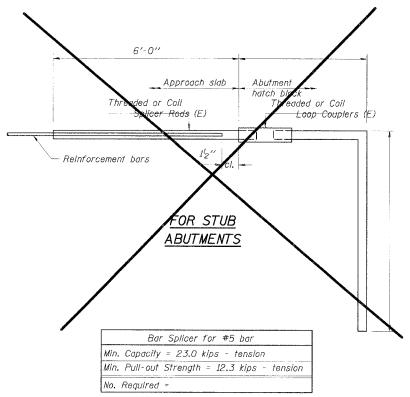
#### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.



#### FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

	Bar	Splicer	foi	r #5	bar		
Min.	Capacity	= 23.0	kip	s - :	tensic	n	
Min.	Pull-out	Strength	=	12.3	kips	-	tension
	Required		**********				



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**** SHEET NO.29 FOTOL CHETTES 35 35 SHEETS DUPAGE

Contract #83965

#### MATERIALS FOR ILLINOIS COIL-LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold drawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 519, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coll wire shall be made of any suitable soft steel wire.

The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed. The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type I, Grade 1 and of a Class suitable for the temperature at installation.

## INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

1. With the coil wire in place, the bolt shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete bearina seat.

2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

#### ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

1. A threaded rod stud with nut and washer of the type specified.

A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical.

Location	Туре
W. Abut.	-
Pier 1	1" φ
Pier 2	1" φ
E. Abut.	-

ASTM F 1554 Grade 105, ASTM A 449 and AASHTO M 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above,

#### GENERAL NOTES

Holes in the masonry for anchor bolts shall be drilled through the base plates to the diameter and depth shown or according to the manufacturer's recommendation after beams or girders have been erected and adjusted. Prior to setting the bolts, the holes shall be dry and all dust and loose particles shall be removed by the use of compressed air or vacuuming.

The anchor bolts, furnished and installed and including the epoxy grout or capsules shall not be paid for separately but shall be included in the unit bid price for Furnishing and Erecting Structural Steel.

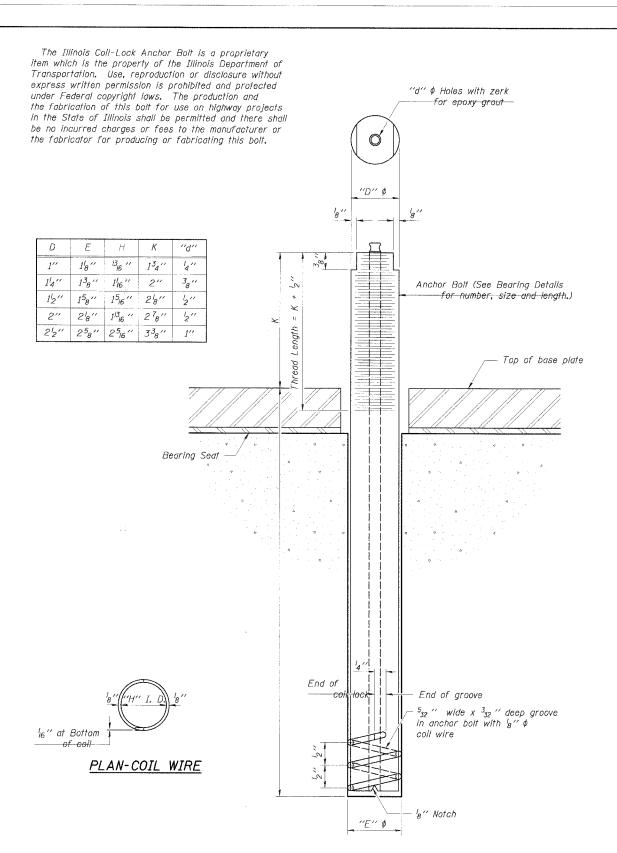
## ANCHOR BOLT DETAILS

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000



CHRISTOPHER B. BURKE ENGINEERING LTD. 3575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

REVISIONS DATE	NOTE: DIVENSIONAL DATA IS NOT TO BE OBTAINED BY SCALING ANY PORTION OF THIS THANKING.	DRAWING NUMBER
		S-17



ILLINOIS COIL-LOCK ANCHOR BOLT

N:80akbrookT8036088S+ruc+803608-S17.PLN

#### BORING B-101

Page 1 of 2 Date Started 1/9/07

Date Completed ____1/9/07___

Testing Service Corporation

STRUCTURE BORING LOG

 COUNTY
 DuPage
 LOCATION
 N. End West Pier
 S.
 23
 , TWP.
 39 N. RNG.
 11 E

STRUCT. NO. 0222-6000 DRILLED BY TSC L-67.897

635.74

clayey SAND and GRAVEL 638,24

Med. dense brown and gray fine SAND with silt layers,

Med. dense brownish-gray to gray SILT, very moist 4-4

Dense gray clayey SILT, moist A-4

Hard to very stiff gray SILTY LGAM, moist A-4/A-6

ROUTE F.A. Roule 7 DESCRIPTION Timber Edge Drive over Sult Creek

Surface Fley 663,74 (4

Air Below Deck Casing extended to stream bed

Very soft gray and dark gray
SILTY CLAY LOAM,
cocasional sand seams,
trace organic, very moist
A-4

BORING B-102

ROUTE NO.	SECTION	500	NTY	TOTOTA	SEET SC.	SHEET NO. 30
*	03-00019- 00-88	DUP	4CE	35	COUNTY	35 sheets
PKU, 8040 Q	ńst. NO. 1	0.0.04018	≤Up.Ajn PR	31607-		

Contract #83965

Page 2 pf 2 Date Started/2/627_ Date Completed/2/07_ STRUCTURE NO022=5000	
ROUTE F.A. Route 7.	SECTSTRUCY. NO. 022-6000 DRILLED BY
COUNTY DUPOGS	COUNTY DuPage LOCATION S. End East Pier S. 23. TWP. 39 N., RNG. 11.E
Boring No. 9=101 D B Slotlon 1192+16 E L Offset 8.8997 i.7 P O U W Elevation 513.74 ft H S faf X	Borling No.   B-102   D   B   Surface Wafer Flev.   D   B   Station   103±50   E   L   Groundwater Elev.   E   L   E   L   Surface Elev.   E   L   Surface Elev.   Sab275   H   S   Ist   X   Surface Elev.   656275   H   S   Ist   X   Surface Elev.   Surface Elev.   Surface Elev.   Surface Elev.   Surface Elev.   Surface Ele
Very stiff to bard gray CLAY LOAM, fiftle sand and gravel, molst	3.5° Asphalric Centrele 7.25° C. Concrele Bridge 162.82  Desk.  Med. dense Gravel with sond, occasional Cobbles, sorturated A-1-a 157.85 5 6.9 10 16.8
409.74 33 5 13.5 49 5.3 49 5.3 45 7% 10.4 Very danse groy Cobbes	Ved. dense groy fine silty
and Boulers (probable weathered and broken rook), troce to little clay	
66	Casing extended to stream   550.75   Loose gray fine sandy SiLI,   3   19.0   4   19.0   4   4   4   4   4   4   537.25     4   537.25     4   537.25   1   5   5   5   5   5   5   5   5
DOLOWITC, very light gray, light to moderate tracturing, org/licensus, hick bedded, dense, numerous withis and gray nodules (>2") spaced of 5" intervals  Core Run from 58 to 68 ft.  Recovery = 100% Recovery = 100% Recovery = 100%	- B S 1.9 16.9 - 10 1.3 16.9 - 13 11%
Core Run from 58 to 68 ft. —— Recovery = 100%	SLT. molest 9107 clopes 0 10 17.7
9574	Very solf don't brown and block CLAY (DAM (no sumpling)   12 9 17.8   12 9 17.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   14.9   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8   15.8
End of Core at 68.0'  CME-75 Truck Rig (#256) -79	LQAM, occasional site
CME Automatic Hammer	sampling) ——25 ——45 ——45 ——618.25
Reform Wesh Drill  Reck Core with NX Core  Bornol  Bornol	1
SPT. (N) = Sum of last two blaw values in sample. (Qu) B=Bulge S=Shear P=Penetration Test	Start, dentise grory clayey 7 5 ASAD and GRAVCI, wet 11 12.0 13 5 ASAD 2nd GRAVCI, wet 11 12.0 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18

			Comple		1/8/07
STRUCTURE NO.		Jule	-		
Boring No. 8-102 Station 193±60 Offset 8.00ff RT  Elevation 613.75 ff	D E F F F F F F F F F F F F F F F F F F	B L O ₩	Qu	w	
Very stiff gray SILTY CLAY LOAM, accasional sill seams, moist A-6		3	tst		
Hard gray CLAY with gravel, moist A=6	09.75	18 50	P 4.25		
Very dense gray Cobbles and Boulders (weathered and broken rock)	-55 	50	4.25	13.0	
Auger Refusal at 57.0'	06.75				
CME-75 Truck Rig (#256) CME Automatic Hammer Rotary Wash Drill	60				
	-65				
software the					
astast space extension					
$\frac{8}{6}$ SPT. (N) = Sum of last two blow Stations, Depths, Offset, and Eleve	{ 75 values in utions are i	sample. n Feel	(Qu)	B=B	ulge S=Shear P=Penetration Test

## BORING B-102A

3 B 4 9.3 23.4 LDAM, occasional silf seams, moist A-6

- 9		12770			
	Te	•	ce Corporation		Page 1 of
		STRUCTURE	BORING LOG		Date Started1/30/07_
ROUTE F.A. Rou <u>te 7</u> DESCR		Timber Edge Dr	ve over Salt Creek		Date Completed 1/30/07
SECT	STRUCT, N	022-600	)	DRILLED BY	TSC L-67,897
COUNTY DuPage LOC	M_ MOITA	ddle of East Pi	E !	323	, TWP. 39_N., RNG. 11
Boring No.         B-102A           Station         103 H60           Offset         8.00ff, RT           Surface Elev.         650.25 H	D B L P O W H S	Qu W	Surface Water Elev. Groundwater Elev.: when drilling at Completion after H	644.8	
Very soft dark brown and black CLAY LOAM, Bitle organic, very maist A-7-6	HA HA	<0.25 50.4 P <0.25 32.6			
Gray clayey SAND and GRAVEL, wet A-2-4	HA	12.9			
Very loose gray layers of Silt and Clay, moist A-4	HA	14.4			
End of Boring at 10.0'					
Hand Auger			lge S≡Shear P≡Pend		

<u>BOF</u>	RING	<i>B-2</i>
Testing	Service	Corporation

		Test	ing S	ervi	ce Corporation			p.	nna 1	of 1
			STRUCT	TURE	BORING LOG			Started	1/	8/07
ROUTE <u>F.A. Route 7</u>	DESCRIPTION	. IIn	nber Edd	ge Ori	ve over Salt Creek	Date	в Со	mpleted	1/3	3/07
SECT	STR	JCT. NO.	022	-6000	ORILLED BY	_₩	fight	& Comp	any	
COUNTY <u>BuPage</u>	LOCATION	Cent	er of W.	Pier	s. 23	T	WP.	39 N	RNG.	<u> 11 E</u>
Boring No.         B-2           Station         102±89           Offset         0.00ft RT           Surface Elev.         654.25	D E P T H	B L O W	Qu tsf	w	Surface Water Elev. Groundwater Elev.: when drilling at Completion after Hrs.	_	D E P T H	B L O W	Qu tsf	w %
	-		<b>.</b>			27.45	_	21		_
WET DARK CLAY, RIVER SETTLING		ļ								
		6	0.24					29		
	649.25									
	992:47. ~	4			WET GRAY SILTY SAND	-	~50	30		
WET GRAY SAND		j			THE TOTAL SELECTION					
	<u>846.75</u>	<del> </del>								
	_	6		-			-35	30	0.59	-
							-35			
		10						33		i
							=			
	_	<u> </u>					ne.			
	_	17					_	40		
	-1				6	14.25 _	-40			
WET GRAY SAND AND GRAVEL	_	18					Ξ	34	0.11	
					WET GRAY CLAY AND					
		18			GRAVEL			39	0.66	
		<u> </u>		-				39	0.00	
					6	09.25_	-45			
This boring performed by others in 1964. Log has been relyped by TSC for								109		
been relyped by TSC for bridge reconstruction.					WET HARD PAN					
bridge reconstruction.		19				06.75	_	115		
					End of Boring at 48.5'	95.75	-			
SPT. (N) = Sum of last two Stations, Depths, Ottset, and	2 blow values i Elevations are	s cample	(Qu)	B≖B	i µlge S≔Shear P=Penetration Test	-	-50			li

BORING LOGS

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

REVISIONS DATE	NOTE: DIVERSIONAL DATA IS NOT TO BE OBTAINED BY SCALEMS ANY PORTIN OF THIS DRAWING.	DRAWING NUMBER
	#856#\$2.70.	S-18

N:80akbrookT80360885+ruc+803608-S18.PLN

Γ	ROUTE NO.	SACTION	SIQUITY		содиту		TOTA. SHEETS	SHEET NC.	SHEET	NO.	31
I	*	03-00019- 00-BR	DUPAGE		35	SOUNTY	35 sı	HEETS	6		
	FED. ROAD O	ST, 10. :	DLLINOS	PED. AID PR	DJECT*						

sheet no. 31

Contract #83965

Page 2 of 2
Date Started ______1/8/07

Date Completed ______1/8/07

#### BORING B-3

Testing Service Corporation STRUCTURE BORING LOG ROUTE F.A. Route 7 DESCRIPTION Timber Edge Drive over Soft Creek Date Completed ____1/8/07__ SECT. STRUCT. NO. 022-6000 DRILLED BY Wight & Company Surface Elev. 655.53 ft DAMP DARK CLAY 27 0.25 28 1.28 WET GRAY SILTY SAND 27 0.64 31 2,37 WET GRAY SILTY SAND 30 0.66 36 0.87 WET GRAY SILTY CLAY S WET GRAY SAND AND GRAVEL 100 4.44  $\frac{8}{8} {\rm SPT.} \ (N) = {\rm Sum} \ {\rm of} \ {\rm last} \ {\rm two} \ {\rm blow} \ {\rm volues} \ {\rm in} \ {\rm sample}. \ ({\rm Qu}) \ {\rm B=Bulge} \ {\rm S=Shear} \ {\rm P=Penetration} \ {\rm Test} \ {\rm Stations}, \ {\rm Depths}, \ {\rm Offset}, \ {\rm and} \ {\rm Elevations} \ {\rm ore} \ {\rm in} \ {\rm Feet}$ 

STRUCTURE NO. 022-6	000			Duic	Completed1/8/07
SECTION	***			_	_
Boring No.         B=3           Station         102+34           Offset         15.50ff RT           Elevation         605.53         ff	D E P T H	B L O W	Qu tsf	w	
WET HARD PAN	604.33	110	6.09		
End of Boring at 51.2'					
	58				
	_				
	_				
	-50				
	=				
	-65				
	65				
an los les	_				
n forty interior particular	$\equiv$				
	_				

			STRUC	CTURE	BORING LOG				Started		8/0
ROUTE F.A. Route 7	DESCRIPTION		limber E	dge Dri	ve over Solt Creel	<b></b>		Date Co	mpleted	1/	8/0
SECT.	STRE	ICT. NO	. 02	2-6000	L	_ DRIL	TED BA	Wight	& Com	pany	
COUNTY	LOCATION	<u>N.</u>	End Eas	l Abult	nent	. s	23	., TWP.	39 N	, RNG.	1
Borling No.   B-6	D E P T H	B L O W S	Qu tsf	w %	Surface Water El Groundwater Elev when drilling at Completion ofter	ur		- D E P T H	B L O W	Qu tsf	7
DAMP BROWN CLAY		14	0.11		GRAY SAMD AND GRAVEL, WET		636		17		
		24			This boring perfi others in 1964, been relyped by	ormed b		-52	15	1.40	
GRAY CLAY, WET	652.62 -16	15	0.23		bridge réconstru	tion.		-36	19	0.75	
	-1	10			GRAY SILTY SAND	, WET			41	0.64	
	-	19							44	1.21	_
GRAY SAND, WET		21						-45	35	0.63	
		26							38	0.52	
	25	ļ						-50	37	1.17	

BORING B-6

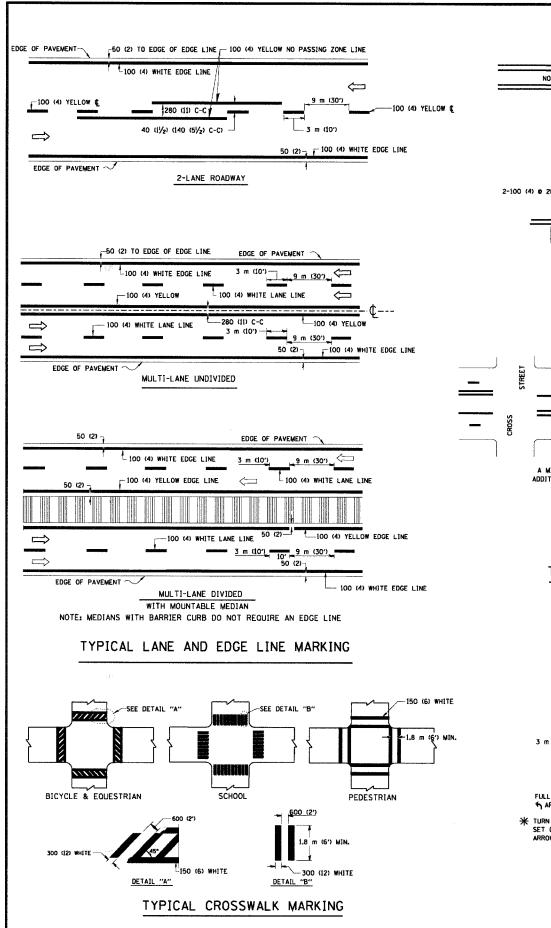
STRUCTURE NO. 022-6000
ROUTE F.A. Route 7
SECTION
COUNTY DuPage 49 2.55 HARD PAN End of Boring at 60.8" 

BORING LOGS

TIMBER EDGE DRIVE OVER SALT CREEK DUPAGE COUNTY F.A. ROUTE 7. SEC. 03-00019-00-BR STATION 103+31.33 STRUCTURE NO. 022-6000

CHRISTOPHER B. BURKE ENGINEERING LTD. 9575 West Higgins Road, Suite 600 Rosemont, Illinois 60018

REVISIONS NAME DATE	NETER DISCHESSIONAL DATA IS NOT TO DE COTAZINOD DY SCALING ANY PORTION OF THIS DRAWING.	DRAWING NUMBER
	983/940 971 P994831 Y35 24846 871 PDR DATE 8/24/2087 CHEROL BY MM 4F5/9049 881	S-19



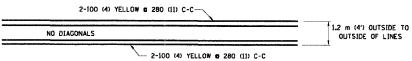
CHECKED Y CHECKED NAME

ENGINEERING

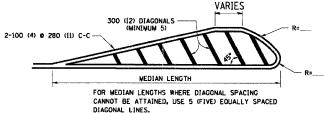
CHRISTOPHER B. BURKE E 9575 West Highs Road, Suite 600 (847) 823-0500

**B** 

N:#0akbrookT#03608#CIvII#03068_s+d.s+d

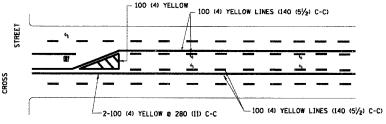


#### 1.2 m (4') WIDE MEDIANS ONLY



DIAGONAL LINE SPACING: 15 m (50') C-C (LESS THAN 50 km/h (30 MPH))
25 m (75') C-C (50 km/h (30 MPH) T0 70 km/h (45 MPH)) 45 m (150") C-C (MORE THAN 70 km/h (45 MPH))

#### MEDIANS OVER 1.2 m (4') WIDE

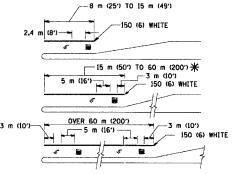


A MINIMUM OF TWO PAIRS OF TURN ARROWS SHALL BE USED, WHITE IN COLOR, ADDITIONAL PAIRS SHALL BE PLACED AT 60 m (200°) TO 90 m (300°) INTERVALS.



MEDIAN WITH TWO-WAY LEFT TURN LANE

#### TYPICAL PAINTED MEDIAN MARKING



FULL SIZE LETTERS 2.4 m (8') AND ARROWS SHALL BE USED.  $\P$  AREA = 1.5 m² (15.6 SO. FT.)  $\P$  AREA = 1.9 m² (20.8 SQ. FT.)

* TURN LANES IN EXCESS OF 120 m (400') IN LENGTH MAY HAVE AN ADDITIONAL SET OF ARROW - "ONLY" INSTALLED MIDWAY BETWEEN THE OTHER TWO SETS OF ARROW - "ONLY".

TYPICAL LEFT (OR RIGHT) TURN LANE

#### TYPICAL TURN LANE MARKING



ISLAND OFFSET FROM PAVEMENT EDGE - 50 (2) 200 (8) WHITE -RAISED ISLAND ISLAND AT PAVEMENT EDGE

200 (8) WHITE --

300 (12) WHITE DIAGONALS 3 m (10') OR LESS SPACING

#### TYPICAL ISLAND MARKING

TYPE OF MARKING	WIDTH OF LINE	PATTERN	COLOR	SPACING / REMARKS
CENTERLINE ON 2 LANE PAVEMENT	100 (4)	SKIP-DASH	YELLOW	3 m (10') LINE WITH 9 m (30') SPACE
CENTERLINE ON MULTI-LANE UNDIVEDED PAVEMENT	2 8 100 (4)	SOLID	YELLOW	280 (11) C-C
NO PASSING ZONE LINES: FOR ONE DIRECTION FOR BOTH DIRECTIONS	100 (4) 2 <b>0</b> 100 (4)	SOLID SOLID	YELLOW YELLOW	140 (5½) C-C FROM SKIP-DASH CENTERLINE 280 (11) C-C OMIT SKIP-DASH CENTERLINE BETWEEN
LANE LINES	100 (4) 125 (5) ON FREEWAYS	SKIP-DASH SKIP-DASH	WHITE WHITE	3 m (10') LINE WITH 9 m (30') SPACE
DOTTED LINES (EXTENSIONS OF CENTER, LANE OR TURN LANE MARKINGS)	SAME AS LINE BEING EXTENDED	SKIP-DASH	SAME AS LINE BEING EXTENDED	600 (2') LINE WITH 1.8 m (6') SPACE
EDGE LINES	100 (4)	SOLID	YELLOW-LEFT WHITE-RIGHT	OUTLINE MOUNTABLE MEDIANS IN YELLOW: EDGE LINES ARE NOT USED NEXT TO BARRIER CURB
TURN LANE MARKINGS	150 (6) LINE; FULL SIZE LETTERS & SYMBOLS (2.4 m (8'))	SOLID	WHITE	SEE TYPICAL TURN LANE MARKING DETAIL
TWO WAY LEFT TURN MARKING	2 @ 100 (4) EACH DIRECTION	SKIP-DASH AND SOLID	YELLOW	3 m (10') LINE WITH 9 m (30') SPACE FOR SKIP-DASH; 140 (51/2) C-C BETWEEN SOLID LINE AND SKIP-DASH LINE
	2.4 m (8') LEFT ARROW	IN PAIRS	WHITE	SEE TYPICAL TWO-WAY LEFT TURN MARKING DETAIL
CROSSWALK LINES (PEDESTRIAN) A. DIAGONALS (BIKE & EQUESTRIAN) B. LONGITUDINAL BARS (SCHOOL)	2 <b>a</b> 150 (6) 300 (12) <b>a</b> 45° 300 (12) <b>a</b> 90°	SOLID SOLID SOLID	WHITE WHITE WHITE	NOT LESS THAN 1.8 m (6') APART 600 (2') APART 600 (2') APART SEE TYPICAL CROSSWALK MARKING DETAILS.
STOP LINES	600 (24)	SOLID	WHITE	PLACE 1.2 m (4") IN ADVANCE OF AND PARALLEL TO CROSSWALK, IF PRESENT OTHERWISE, PLACE AT DESIRED STOPPING POINT. PARALLEL TO CROSSROAD CENTERLINE, WHERE POSSIBLE
PAINTED MEDIANS	2 @ 100 (4) WITH 300 (12) DIAGONALS @ 45° NO DIAGONALS USED FOR 1.2 m (4') WIDE MEDIANS	SOLID	YELLOW: TWO WAY TRAFFIC WHITE: ONE WAY TRAFFIC	280 (1) C-C FOR THE DOUBLE LINE SEE TYPICAL PAINTED MEDIAN MARKING.
GORE MARKING AND CHANNELIZING LINES	200 (8) WITH 300 (12) DIAGONALS @ 45°	SOLID	WHITE	DIAGONALS: 4.5 m (15') C-C (LESS THAN 50 km/h (30 MPH)) 6 m (20') C-C (50 km/h (30 MPH) TO 70 km/h (45 MPH 9 m (30') C-C (OVER 70 km/h (45 MPH))
RAILROAD CROSSING	600 (24) TRANSVERSE LINES; "RR" IS 1,8 m (6') LETTERS; 400 (16) LINE FOR "X"	SOLID	WHITE	SEE STATE STANDARD 780001 AREA OF: "R":0.33m2 (3.6 SQ. FT.) EACH "X"=5.0 m2 (54.0 SQ. FT.)
SHOULDER DIAGONALS	300 (12) <b>c</b> 45°	SOLID	WHITE - RIGHT YELLOW - LEFT	15 m (50") C-C (LESS THAN 50 km/h (30 MPH)) 25 m (75") C-C (50 km/h (30 MPH) TO 70 km/h (45 MPH 45 m (150") C-C (OVER 70 km/h (45 MPH))

FOR FURTHER DETAILS ON PAVEMENT MARKING REFER TO STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND STATE STANDARD 780001.

All dimensions are in millimeters (inches) unless otherwise shown,

REVISIONS		ILLINOIS DEPARTMENT OF TRANSPORTATION			
NAME	DATE	ILLINOIS DEFARINE	MI OF TRANSFORTATION		
EVERS	03-19-90				
T. RAMMACHER	10-27-94	DISTRICT ONE			
ALEX HOUSEH	10-09-96				
ALEX HOUSEH	10-17-96	TYPICAL	PAVEMENT		
T. RAMMACHER	01-06-00	MAC	MARKINGS		
		MARKINGS			
		SCALE: NONE	DRAWN BY CADD		
		DATE: 2/15/2006	CHECKED BY		
			TC-13		

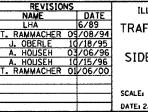
REVISION DATE: 01/06/00

COUNTY SECTION __ 03-00019-00-BR DUPAGE STA. TO STA. FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT CONTRACT NO. 83965 TYPE III BARRICADES WITH TWO FLASHING AMBER LIGHTS ON EACH. TYPE I OR TYPE II BARRICADES WITH ONE FLASHING AMBER LIGHT ON EACH, OR TYPE III BARRICADES WITH TWO FLASHING 60 m± (200'±)-AMBER LIGHTS ON EACH. DRIVEWAY STREET, SPEED 40 MPH OR LESS 150 m± (500'±) 60 m± (200'±) COLLECTOR LIMIT>60 km/h (40 N W20-1(0) ROAD ONSTRUCTION M6-4(0)-2115 M6-1(0)-2115 TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS

#### NOTES:

- A. FOR NO LANE RESTRICTION ON THE SIDE ROAD OR DRIVEWAYS
- 1. SIDE ROAD WITH A SPEED LIMIT OF 60 km/h (40 MPH) OR LESS AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEERS
- Q) ONE ROAD CONSTRUCTION AHEAD SIGN 900x900 (36x36) WITH A FLASHER AND FLAG MOUNTED ON IT APPROXIMATELY 60 m (200") IN ADVANCE OF THE MAIN ROUTE.
- b) THE CLOSED PORTION OF THE MAIN ROUTE SHALL BE PROTECTED BY BLOCKING WITH TYPE I, TYPE II OR TYPE III BARRICADES, 1/3 OF THE CROSS SECTION OF THE CLOSED PORTION.
- 2. SIDE ROAD WITH A SPEED LIMIT GREATER THAN 60 km/h (40 MPH)
  AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEER:
- O) ONE ROAD CONSTRUCTION AHEAD SIGN 1.2 m x 1.2 m (48x48) WITH A FLASHER MOUNTED ON IT APPROXIMATELY 150 m (500°) IN ADVANCE OF THE MAIN ROUTE.
- b) THE CLOSED PORTION OF THE MAIN ROUTE SHALL BE PROTECTED BY BLOCKING WITH TYPE II] BARRICADES, 1/2 OF THE CROSS SECTION OF THE CLOSED PORTION.
- 3. WHEN THE SIDE ROAD LIES BETWEEN THE BEGINNING OF THE MAINLINE SIGNING AND THE WORK ZONE, A SINGLE HEADED ARROW (MG-1) SHALL BE USED IN LIEU OF THE DOUBLE HEADED ARROW (MG-4).

- B. FOR A LANE CLOSURE ON A SIDE ROAD OR DRIVEWAY:
- USE APPLICABLE PORTIONS OF THE TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES (STD. 701501, STD. 701606 OR THE APPROPRIATE STANDARD). THE SPACING OF SIGNS AND BARRICADES SHALL BE ADJUSTED FOR FIELD CONDITIONS AS DIRECTED BY THE ENGINEER. THE DIRECTIONAL ARROW SHALL BE COVERED OR REMOVED WHEN NO LONGER CONSISTENT WITH THE SIDE ROAD LANE CLOSURE.
- C. ADVANCE WARNING SIGNS ARE TO BE OMITTED ON DRIVEWAY UNLESS OTHERWISE NOTED.
- D. THE TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS SHALL BE INCIDENTAL TO THE COST OF SPECIFIED TRAFFIC CONTROL STANDARDS OR ITEMS.



ILLINOIS DEPARTMENT OF TRANSPORTATION
TRAFFIC CONTROL AND PROTECTION
FOR

SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS

DATE: 2/15/2006

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