STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS**

FAP ROUTE 865 (IL 152) SECTION 16B-2 **OVER PANTHER CREEK PERRY COUNTY**

PROPOSED BRIDGE OVER PANTHER CREEK STRUCTURE NO. 073-0037
3 SPAN STEEL W27 BRIDGE;

158'-0" BK TO BK ABUTMENTS; 25° SKEW \$\phi\$ STRUCTURE STA 875+28.00 EXISTING SN 073-0024

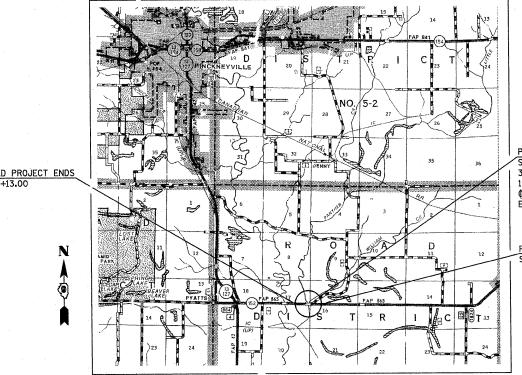
PROPOSED PROJECT BEGINS STA 873+49.00

PROPOSED HIGHWAY PLANS

STRUCTURE REPLACEMENT

C-99-050-08

PROJECT: ACBRF-0865(006)



ROADWAY LENGTH = 406 FT BRIDGE LENGTH = 158 FT GROSS LENGTH = 564 FT NET LENGTH = 564 FT

PROJECT ENGINEER T. WAYNE HALSTEAD PROJECT MANAGER DAVID SPENCER PICHE

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION SUBMITTED June 23 20 09 May C. Lamie
DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER

LOCATION OF SECTION INDICATED THUS: -

PRINTED BY THE AUTHORITY

OF THE STATE OF ILLINOIS 1 Rev. 10-20-09

TRAFFIC DATA 2008 ADT = 2480

10.6% TRUCKS POSTED SPEED 55 MPH

ROAD DISTRICT #06-2

ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

FOR INDEX OF SHEETS, SEE SHEET NO. 2

FOR SUMMARY OF QUANTITIES, SEE SHEETS NO. 3 AND 4

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JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123

CONTRACT NO. 78064

GENERAL NOTES

THE THICKNESS OF THE HOT MIX ASPHALT MIXTURE SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE HOT MIX ASPHALT MIXTURE IS PLACED.

FACTORS USED FOR QUANTITY CALCULATIONS ARE AS FOLLOWS:

ALL HOT MIX ASPHALT:
HOT MIX ASPHALT MATERIALS ON PAVEMENT:
AGGREGATE (PRIME COAT)
ALL AGGREGATE:
RIPRAP
2.016 TONS/CU. YD.
0.0015 TONS/CU. YD.
2.05 TONS/CU. YD.

PLAN DIMENSIONS AND DETAILS RELATIVE TO THE EXISTING STRUCTURE HAVE BEEN TAKEN FROM EXISTING PLANS AND ARE SUBJECT TO NOMINAL CONSTRUCTION VARIATIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY SUCH DIMENSIONS AND DETAILS IN THE FIELD AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OP ORDERING OF MATERIALS. SUCH VARIATIONS SHALL NOT BE A CAUSE FOR ADDITIONAL COMPENSATION FOR A CHANGE IN THE SCOPE OF THE WORK. THE CONTRACTOR, HOWEVER, WILL BE PAID FOR THE ACTUAL QUANTITY FURNISHED AT THE UNIT PRICE BID FOR THE WORK. CONSTRUCTION PLANS ARE AVAILABLE FOR REVIEW AT THE DISTRICT 9 OFFICE.

IN ADDITION TO THE REQUIREMENTS OF ARTICLE 107.16 THE CONTRACTOR SHALL PROTECT THE SURFACE OF ALL BRIDGE DECK AND BRIDGE APPROACH PAVEMENTS IN A MANNER SATISFACTORY TO THE ENGINEER BEFORE ANY EQUIPMENT IS ALLOWED TO CROSS THE STRUCTURE. PROTECTION SHALL BE PROVIDED FOR ALL EQUIPMENT AS DEFINED IN ARTICLE 101.17 REGARDLESS IF TRACK MOUNTED OR WHEELED.

AT ALL LOCATIONS WHERE HOT MIX ASPHALT OR CONCRETE PAVEMENT JOINS AN EXISTING HOT MIX ASPHALT OR CONCRETE PAVEMENT, A SAWED JOINT SHALL BE CONSTRUCTED. THE COST OF THIS JOINT SHALL BE INCLUDED IN THE TYPE OF PAVEMENT BEING CONSTRUCTED.

QUANTITIES SHOWN IN THE PLANS FOR BRIDGE DECK GROOVING AND PROTECTIVE COAT INCLUDE THE BRIDGE, THE BRIDGE APPROACH PAVEMENTS, AND THE BRIDGE APPROACH PAVEMENT CONNECTORS (PCC).

PROTECTIVE COAT SHALL BE APPLIED TO THE BRIDGE, THE BRIDGE APPROACH PAVEMENTS, AND THE BRIDGE APPROACH PAVEMENT CONNECTORS (PCC) IN ACCORDANCE WITH ARTICLE 503.19 OF THE STANDARD SPECIFICATIONS.

THE PROTECTIVE COAT SHALL BE APPLIED REGARDLESS OF THE CURING METHOD USED. THE RATE OF APPLICATION FOR EACH COAT ON SAW CUT GROOVED AREAS SHALL BE 25 SQUARE YARDS PER GALLON OF MIXTURE.

REMOVAL OF EXISTING 13" THICK BRIDGE APPROACH PAVEMENTS ARE INCLUDED IN THE QUANTITY FOR PAVEMENT REMOVAL - 116 SQ YD.

TREES SHALL BE PRESERVED THROUGHOUT THIS SECTION AS SHOWN ON THE PLANS AND AS DIRECTED BY THE ENGINEER. GENERALLY, TREES OUTSIDE THE CONSTRUCTION LIMITS, AND WHICH DO NOT INTERFERE WITH CONSTRUCTION, SHALL NOT BE DISTURBED.

IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO REMOVE ANY DEBRIS OR DIRT CAUSED BY CONSTRUCTION ACTIVITY THAT COVERS THE NEW RIPRAP. AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.

ALL TEMPORARY EROSION CONTROL MEASURES SHALL BE LEFT IN PLACE UNTIL REMOVAL IS REQUIRED TO CONSTRUCT FINAL GRADE LINES.

THE QUANTITY OF SHORT TERM PAVEMENT MARKING SHOWN IN THE PLANS IS BASED ON ONE APPLICATION EACH FOR THE BINDER COURSE AND THE SURFACE COURSE.

THE QUANTITY OF TEMPORARY PAVEMENT MARKING SHOWN IN THE PLANS IS BASED ON ONE APPLICATION FOR THE SURFACE COURSE.

PRIOR TO PLACEMENT OF THE FINAL PAVEMENT MARKINGS THE RESIDENT ENGINEER SHOULD CONTACT THE BUREAU OF OPERATIONS AND ARRANGE FOR INSPECTION AND APPROVAL OF THE PAVEMENT MARKING LAYOUT.

COST OF REMOVING HOT MIX ASPHALT BASE COURSE WIDENING, 10" USED FOR STAGE I TRAFFIC IS INCLUDED IN "PAVED SHOULDER REMOVAL-SQ YD."

THE ADVANCE DETECTOR LOOPS ARE TYPICALLY LOCATED 275 FEET IN ADVANCE OF THE STOP BAR. THE BUREAU OF OPERATIONS SHOULD APPROVE THE LOOP LOCATIONS PRIOR TO INSTALLATION.

THE CENTERLINE PAVEMENT MARKING SHOULD BE REMOVED FROM THE STOP BAR TO THE SAND ATTENUATORS OR DRUMS. EDGE LINE PAVEMENT MARKING SHOULD BE REMOVED IF A 10 FOOT LANE WIDTH CANNOT BE MAINTAINED. TEMPORARY EDGE LINES SHOULD BE INSTALLED WHEN THE EDGE LINES ARE REMOVED.

THE BARRIER WALL AND GUARDRAIL REFLECTORS AS SHOWN ON STANDARD 701321 SHALL 3E INSTALLED PRIOR TO OPENING TO TRAFFIC.

ANY TIME THE CONCRETE BARRIER IS NOT IN THE PROPER POSITION, FLAGGERS SHALL BE IN PLACE TO CONTROL TRAFFIC. THE TEMPORARY TRAFFIC SIGNALS SHALL BE SET TO FLASH ALL RED.

TRIM EDGES OF EXISTING HOT MIX ASPHALT SURFACE FLUSH WITH EXISTING FAVEMENT PRIOR TO CONSTRUCTING NEW BASE COURSE WIDENING.

THE HOT MIX ASPHALT BASE COURSE WIDENING, 10" CONSTRUCTED IN PRE-STAGE 1 MAY BE INCORPORATED INTO THE FINAL HOT MIX ASPHALT SHOULDERS, 8" DURING STAGE II CONSTRUCTION IF APPROVED BY THE ENGINEER. SUCH CHANGE WILL NOT BE A CAUSE FOR ADDITIONAL COMPENSATION, BUT THE CONTRACTOR WILL BE PAID FOR THE ACTUAL QUANTITY FURNISHED AT THE UNIT PRICE BID FOR THE WORK.

COMMITMENTS: NONE AS OF JUNE 26, 2009, REFER TO COMMITMENT FILE FOR ANY COMMITMENTS AFTER THIS DATE.

HIGHWAY STANDARDS

000001-05	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS	
280001-04	TEMPORARY EROSION CONTROL SYSTEMS	
420001-07	PAVEMENT JOINTS	
420401-07	BRIDGE APPROACH PAVEMENT	
421001-02	REINFORCEMENT FOR CONTINUOUSLY REINFORCED PCC PAVEMENT	
482006-03	HOT-MIX ASPHALT SHOULDER ADJACENT TO RIGID PAVEMENT	
515001-03	NAME PLATE FOR BRIDGES	
630001-08	STEEL PLATE BEAM GUARDRAIL	
630201-06	PCC/BITUMINOUS STABILIZATION AT STEEL PLATE BEAM GUARDRAIL	
630301-05	SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS	
631031-07	TRAFFIC BARRIER TERMINAL, TYPE 6	
635006-03	REFLECTOR AND TERMINAL MARKER PLACEMENT	
635011-02	REFLECTOR MARKER & MOUNTING DETAILS	
701006-03	OFF-ROAD OPERATIONS, 2L 2W, 4.5 m (15') TO 600 mm (24") AWAY, FOR SP	EEDS ≥ 45 MPH
701201-03	LANE CLOSURE, 2L 2W, DAY ONLY, ON-ROAD TO 600 mm (24") OFF-ROAD, FOR	SPEEDS ≥ 45 MPH
701301-03	LANE CLOSURE, 2L 2W, SHORT TIME OPERATIONS, FOR SPEEDS \geq 45 MPH	
701321-10	LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER	
701326-03	LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING, FOR SPEEDS > 45 MPH	
701961-01	TRAFFIC CONTROL DEVICES	
704001-05	TEMPORARY CONCRETE BARRIER	
780001-02	TYPICAL, PAVEMENT MARKINGS	
BLR21-8	TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR CONSTRUCTION ON RU	JRAL LOCAL ROADS
601101-01		

INDEX OF SHEETS

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9	ROW SHEET
10	STAGE CONSTRUCTION PLAN
11	DETOUR SIGNING AND ROAD CLOSURE
12	HOT-MIX ASPHALT SHOULDER AND GUARDRAIL PLAN
1.3	EROSION CONTROL PLAN
1 4	DETAILS- BUTT JOINT, SEEDING & MULCHING STEP CONSTRUCTION ON EXISTING FILL
15-16	DETAILS- BRIDGE APPROACH PAVEMENT
17-22	CROSS SECTIONS
DE *23-47	STRUCTURE PLANS

Prepared	Ву:	Denni Hillaham
F	D	DISTRICT STUDIES & PLANS ENGINEER
Examined	ву:	James Krains Emery
Examined	Ву:	DISTRICT LAND ACQUISITION ÉNGINEER Canai Nelson
Examined	Ву:	DISTRICT PROGRAM DEVELOPMENT ENGINEER
Examined	Ву:	DISTRICT OPERATIONS ENGINEER
Examined	Ву: .	DISTRICT CONSTRUCTION ENGINEER
Examined	Ву:	DISTRICT MATERIALS ENGINEER
Examined	Ву:	DISTRICT PROJECT INPLEMENTATION ENGINEER
Approved	Ву:	ASSISTANT REGIONAL ENGINEER Man C. Lamie
		DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER Jan 22 20 89

FILE NAME =	USER NAME = halsteadtw	DESIGNED -	REVISED -
c:\pw_work\PWIDOT\HALSTEADTW\dms49780\	978Ø64_sht_misc.dgn	DRAWN -	REVISED -
	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED -
78064/CADDSHEETS/D978064_SHT_MISC.DGN	PLOT DATE = 6/8/2009	DATE -	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL NOTES, HIGHWAY STANDARDS
AND INDEX OF SHEETS

SHEET NO. OF SHEETS STA. TO STA.

F.A.P. SECTION COUNTY TOTAL SHEETS NO.

865 16B-2 PERRY 47 2

CONTRACT NO. 78064

Bench Mark: Square cut in top of asphalt located at S.W. corner of Structure 073-0024 Elev. 399.036. STATE OF ILLINOIS Existing Structure: S.N. 073-0024 built 1927 as SBI-Route 152, Section 104 A, B, C at Station 875+10.0 as a 3 span reinforced concrete T-beam bridge. 160'-4'2'' Bk.-to-Bk. abutments supported on untreated timber piles. Superstructure replacement and widening to 33'-0" out to out in 1980 at Station 875+13.10 with DEPARTMENT OF TRANSPORTATION Hatched area indicates channel excavation. Simple Span PPC deck beams and bituminous wearing surface. Existing bridge to be removed and replaced. (See Roadway Plans) Traffic maintained utilizing stage construction. Traffic Barrier Terminal No salvage (typ. all four corners) -D.H.W. Elev. 394.9 W27 (comp)-_____ Elev. 392.48 Elev. 392.48 Elev. 388.21-Streambed -Elev. 388.2Î Borrow Excavation ·Steel Steel H-Pile | (See Roadway Plans) HTPSteel ₩ H-Pile Boring 2-S 💠 **ELEVATION** Boring 3-S◆ $B \blacktriangleleft 1$ © Pier 2 -Sta. 875+63.00 Elev. 399.37 _typ._ Bk. W. Abut. Bk. E. Abut. Pier Sta. 874+49.00\ Sta. 876+07.00 Sta. 874+93.00 Sta. 875+28.00 Elev. 399.37 Elev. 399.37 Elev. 399.37 Staae —Temp, sheet pilina. Const. Line tvp. Limits of Removal Line Existing Structure P 6" ♥ Floor 2 spa, at 4 spa. at 2 spa. at = 20'-0" Drain Spacing = 46'-0 44'-0" 158'-0'' Back to Back of Abutments PLAN DESIGN SCOUR ELEVATION TABLE Design Scour Elevation East Abut. Pier 1 Pier 2 West Abut. (feet) 392.48 367.56 367.56 392.48 0.00% WATERWAY INFORMATION Exist. Low Grade Elev. 397.3 ft. @ Sta. 597+50 Drainage Area = 288.2 sq. mi. Prop. Low Grade Elev. 397.3 ft. 🛭 Sta. 597+50 Opening Sq. Ft. Nat. Head - Ft. Headwater E Structure Number Exist. Prop. Exist. Prop. H.W.E. Exist. Prop. Exist. Prop. 073-0023 10,420 9,460 2,833 2,833 393.6 393.5 10 393.1 0.5 0.4 073-0024 3.860 4.820 823 1.051 3,656 3,884 3,825 3,825 PROFILE GRADE 073-0023 16,440 14,830 (along & Rdwy.) 50 993 1.284 394.9 0.6 0.5 395.5 395.4 Design 073-0024 5,180 6,790 Total 18,990 17,830 4,156 4.156

100

500

073-0023

073-0024 6,140 7,520 1,050 1,364

073-0024 5,610 8,100 1,142 1,568

24,690 5,206 5,520 27,200 24,710 4,928 4,928

6,070 6,496

10 Year Velocity through Existing Bridge = 4.7 fps

10 Year Velocity through Prop. Bridge = 5.6 fps

395.5

0.6

1.2

0.6

396.1

398.2

396.1

398.4

Base

Max. Calc.

EXPIRES 11-30-2010

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General Plan & Elevation General Data Stage Construction & Temporary Sheet Piling Details Temporary Concrete Barrier for Stage Construction Top of Slab Elevations Top of East Approach Slab Elevation Top of West Approach Slab Elevation Superstructure Superstructure Details 12 Diaphragm Details
13-15 (Intentionally Left Blank)
16 Structural Steel Bearing & Structural Steel Details East Abutment West Abutment 20 Piers Bar Splicer Assembly Details Steel HP Pile Details

STATION 875+28.00
BUILT 20 BY
STATE OF ILLINOIS
F.A.P. RTE. 865 SEC. 16B-2
LOADING HL-93
STRUCTURE NO. 073-0037

23-25 Soil Boring Logs

NAME PLATE See Std. 515001

LOADING HL-93
Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

2007 AASHTO LRFD Bridge Design Specifications with 2008 Interims

DESIGN STRESSES

f'c = 3,500 psi

fy = 60,000 psi (Reinforcement)

fy = 50,000 psi (M270 Grade 50) fy = 36,000 psi (M270 Grade 36)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 3
Design Spectral Acceleration at 1.0 sec. (SD1) = 0.319 g
Design Spectral Acceleration at 0.2 sec. (SDS) = 0.7429 g
Soil Site Classification = D



GENERAL PLAN & ELEVATION

IL. ROUTE 152 OVER PANTHER CREEK

F.A.P. ROUTE 865 - SECTION 16B-2

PERRY COUNTY

STATION 875+28.00

STRUCTURE NO. 073-0037

SHEET NO.1	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
3	865	16B-2	PERRY	47	23
25 SHEETS			CONTRACT	NO. 78	064
	FED. RO	AD DIST. NO ILLINOIS FED. AI	D PROJECT		

DESIGNED Stephen in Ryan

CHECKED Jay D Edward

CHECKED SMR/JOE

h.t. duona

EXAMINED

PASSED

GENERAL NOTES

Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts.

Bolts $^7g''$ ϕ , holes $^{15}_{16}('')$ ϕ , unless otherwise noted. Calculated weight of Structural Steel = 102,820 lbs. (AASHTO M270 Grade 50) = 12,180 lbs. (AASHTO M270 Grade 36)

No field welding is permitted except as specified in the contract documents. Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Bearing seat surfaces shall be constructed or adjusted to their designated elevations within a tolerance of l_8 inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be

used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Interstate Green, Munsell No. 7.5G 4/8. See Special Provision for "Cleaning and Painting New Metal Structures".

Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at substructures specified or approved by the Engineer before ordering the remainder of piles.

Slipforming of the parapets is not allowed.

The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.

The Contractor shall submit Structural Assessment Report(s) as required for the Contractor's Means and Methods of Contruction. See Special Provisions.

Current Ratings on File for Existing Structure Inventory: HS 17.2

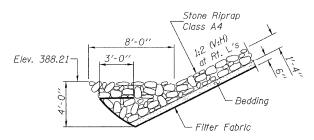
Operating: HS 29.9

Live Load Restriction: No

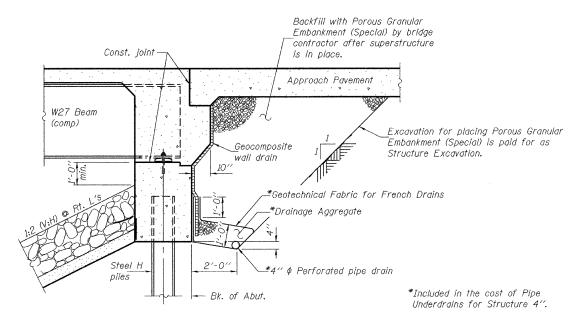
Inventory and Operating Ratings and Live Load Restrictions are provided for information only. Inventory and Operatings Ratings are based on HS loading and configurations. The Ratings and Live Load Restrictions are not necessarily

representative of capacities to support the Contractor's equipment.

The Contractor is advised that the existing structure contains members that are in a deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the existing structure when developing construction procedures for the complete or partial removal, or replacement of the structure. An Existing Structure Information Package is available upon request as noted in the special provisions.



SECTION A-A

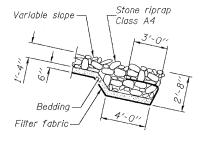


SECTION THRU INTEGRAL ABUTMENT

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL	
Porous Granular Embankment (Special)	Cu. Yd.		106	106	
Stone Riprap, Class A4	Sq. Yd.		538	538	
Filter Fabric	Sq. Yd.		538	538	
Removal of Existing Structures	Each			1	
Structure Excavation	Cu. Yd.		143	143	
Floor Drains	Each	22		22	
Concrete Structures	Cu. Yd.		140.4	140.4	
Concrete Superstructure	Cu. Yd.	200.5		200.5	├
Bridge Deck Grooving	Sq. Yd.	527		<i>527</i> I	
Concrete Encasement	Cu. Yd.		12.2	12,2	
Protective Coat	Sq. Yd.	702		702 -	1
Furnishing and Erecting Structural Steel	L. Sum	1		1	
Stud Shear Connectors	Each	3078		3078	
Reinforcement Bars, Epoxy Coated	Pound	47410	14990	62400:-	
Bar Splicers	Each	543	<i>88</i>	<u>631</u> .	21
Furnishing Steel Piles HP12x74	Foot		770	770	
Driving Piles	Foot		1380	1380	
Test Pile Steel HP12x74	Each		2	2	
Furnishing Steel Piles HP14x73	Foot		610	610	1
Test Pile Steel HP14x73	Each		2	2	
Temporary Sheet Piling	Sq. Ft.		575	575	İ
Name Plates	Each	1		1	
Anchor Bolts 1''	Each	48		48	
Geocomposite Wall Drain	Sq. Yd.		63.2	63.2	
Pipe Underdrains for Structures, 4''	Foot		156	156	
Underwater Structure Excavation Protection, Location 1	Each		1	1	
Underwater Structure Excavtion Protection, Location 2	Each		1	1	
Mechanical Splice	Each		72	72	
Asbestos Bearing Pad Removal	Each		22	22	



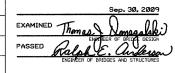
SECTION B-B S.W. Riprap quadrant only.

GENERAL DATA STRUCTURE NO. 073-0037

SHEET NO.2	F.A.P. RTE.		SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
011221 11012	865		16B	3-2		PERRY	47	24
25 SHEETS						CONTRACT	NO. 78	064
	FED. RC	AD DIST.	NO	ILLINOIS	FED. A	ID PROJECT		

A Revised 10/14/2009, SMR

DESIGNED	Stephen M. Ryan
CHECKED	Jay D. Edwards
DRAWN	h.t. duong
CHECKED	SMR/JDE



SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
East End E. Appr. Pav't.!	87426.46	- 16.00	399.10
A B	87436.46 87446.46	- 16.00 - 16.00	399.10 399.10
West End E. Appr. Pav't.	87456.46	- 16.00	399,10

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
East End E. Appr. Pav't.!	87424.60	-12.00 ·	399.18
$\bigwedge \qquad \bigwedge^A \qquad \stackrel{A}{B}$	87434.60 87444.60	-12.00 -12.00	399.18 399.18
West End E. Appr. Pav't.	8745 4. 60	-12.00	399.18

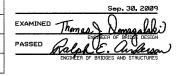
3 spa. at 10′-0″ = 30′-0″

<u>PLAN</u>

DESIGNED Stephen M. Ryan

CHECKED Jay D. Edwards
DRAWN h.t. duong

CHECKED SMR/JDE



STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations
East End E. Appr. Pav't.	87419.23	-0.50	399.36
A B	87429.23 87439.23	-0.50 -0.50	399.36 399.36
West End E. Appr. Pav't.	87449.23	-0.50	399.36

@ ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations
East End E. Appr. Pav't.	87419.00	0.00	399.37
A B	87429.00 87439.00	0.00 0.00	399.37 399.37
West End E. Appr. Pav't.	87449.00	0.00	399.37

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
East End E. Appr. Pav't.	87413.40	12.00	399.18
A B	87423.40 87433.40	12.00 12.00	399.18 399.18
West End E. Appr. Pav't.	87443.40	12.00	399.18

NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
East End E. Appr. Pav't.!	87411.54	16.00	399.10
A B	87421.54 87431.54	16.00 16.00	399.10 399.10
West End E. Appr. Pav't.	87441.54	16.00	399.10

TOP OF EAST APPROACH PAVEMENT ELEVATIONS STRUCTURE NO. 073-0037

SHEET NO. 8	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
J SHEET HOLD	865	16B-2	PERRY	47	30
25 SHEETS			CONTRACT	NO. 78	064
	FED. RO	DAD DIST. NO ILLINOIS FED. A	D PROJECT		

SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
East End W. Appr. Pav't.	87614.46	- 16.00	399.10
△ 0 P	87624.46 87634.46	- 16.00 - 16.00	399.10 399.10
West End W. Appr. Pav't.	87644.46	- 16.00	399.10

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
East End W. Appr. Pav't.	87612.60	-12.00	399.18
△ 0 P	87622.60 87632.60	-12.00 -12.00	399.18 399.18
West End W. Appr. Pav't.	87642.60	-12.00	399.18

@ ROADWAY & PROFILE GRADE

STAGE CONSTRUCTION JOINT

Location

East End W. Appr. Pav't.

West End W. Appr. Pav't.

Station

87607.23

87617.23 87627.23

87637.23

Theoretical Grade

Elevations

399.36

399.36 399.36

399.36

Offset

-0.50 -0.50

-0.50

Location	Station	Offset	Theoretical Grade Elevations
East End W. Appr. Pav't.	87607.00	0.00	399.37
△ 0 P	87617.00 87627 . 00	0.00 0.00	399.37 399.37
West End W. Appr. Pav't.	87637.00	0.00	399.37

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
East End W. Appr. Pav't.	87601.40	12.00	399.18
△ 0 P	87611.40 87621 . 40	12.00 12.00	399.18 399.18
West End W. Appr. Pav't.	87631.40	12.00	399.18

NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
East End W. Appr. Pav't.!	87599.54	16.00	399.10
0 P	87609.54 87619.54	16.00 16.00	399.10 399.10
West End W. Appr. Pav't.	87629.54	16.00	399.10

TOP OF WEST APPROACH PAVEMENT ELEVATIONS STRUCTURE NO. 073-0037

SHEET NO. 9	F.A.P. RTE.	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
J. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	865	168	3-2		PERRY	47	31
25 SHEETS					CONTRACT	NO. 78	064
	FED. RO	AD DIST. NO	ILLINOIS	FED. A	ID PROJECT		

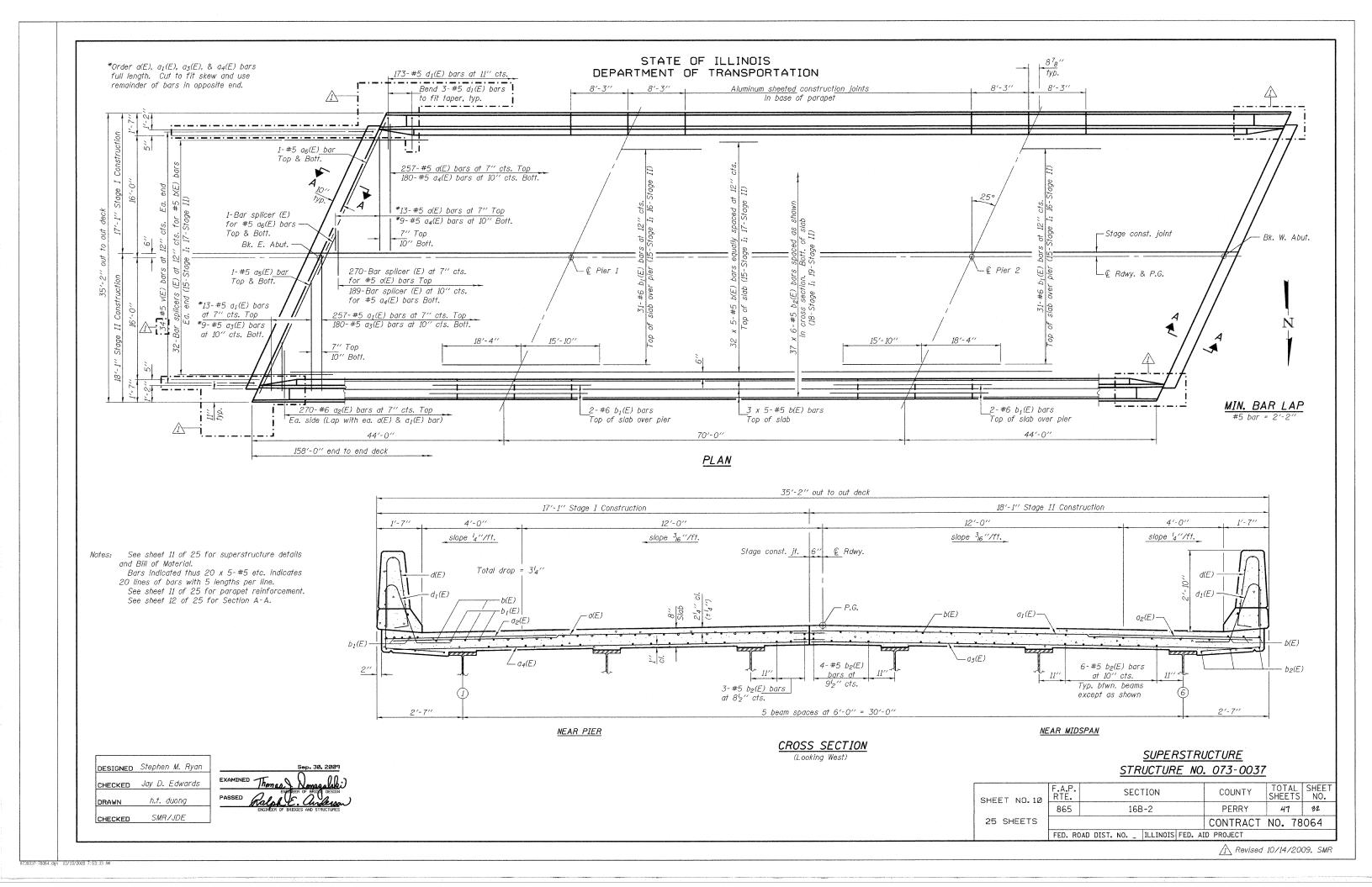
A Revised 10/14/2009, SMR

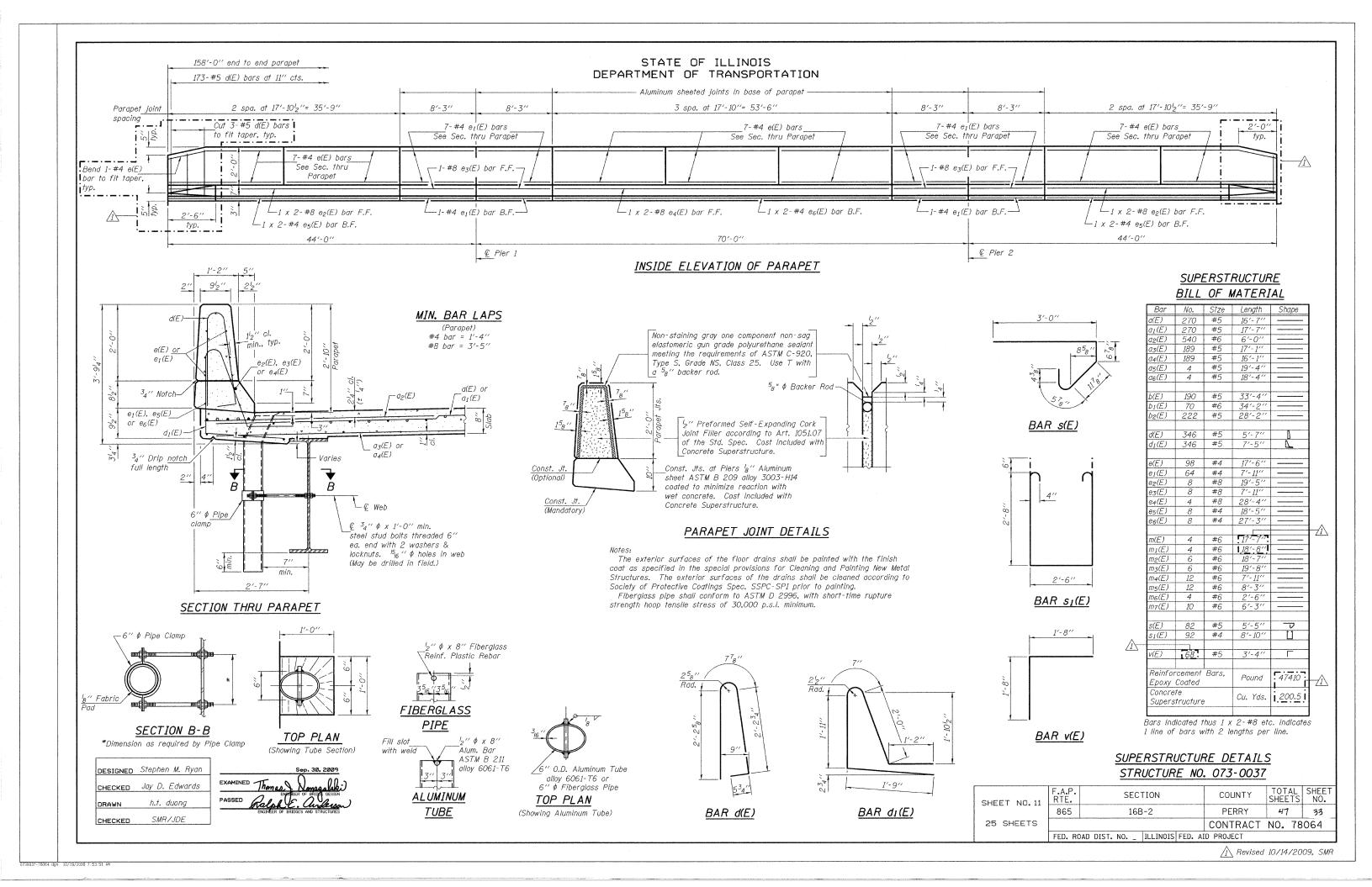
South Curb Line South Edge of Pav't. East end of West end of W. Appr. Pav't. Stage construction joint	16'-0''	
© Rdwy. & P.G.— North Edge of Pav1. North Curb Line— North Curb	,;0-,9[1
3 spa. at 10'-0'' = 30'-0'' PLAN	L	

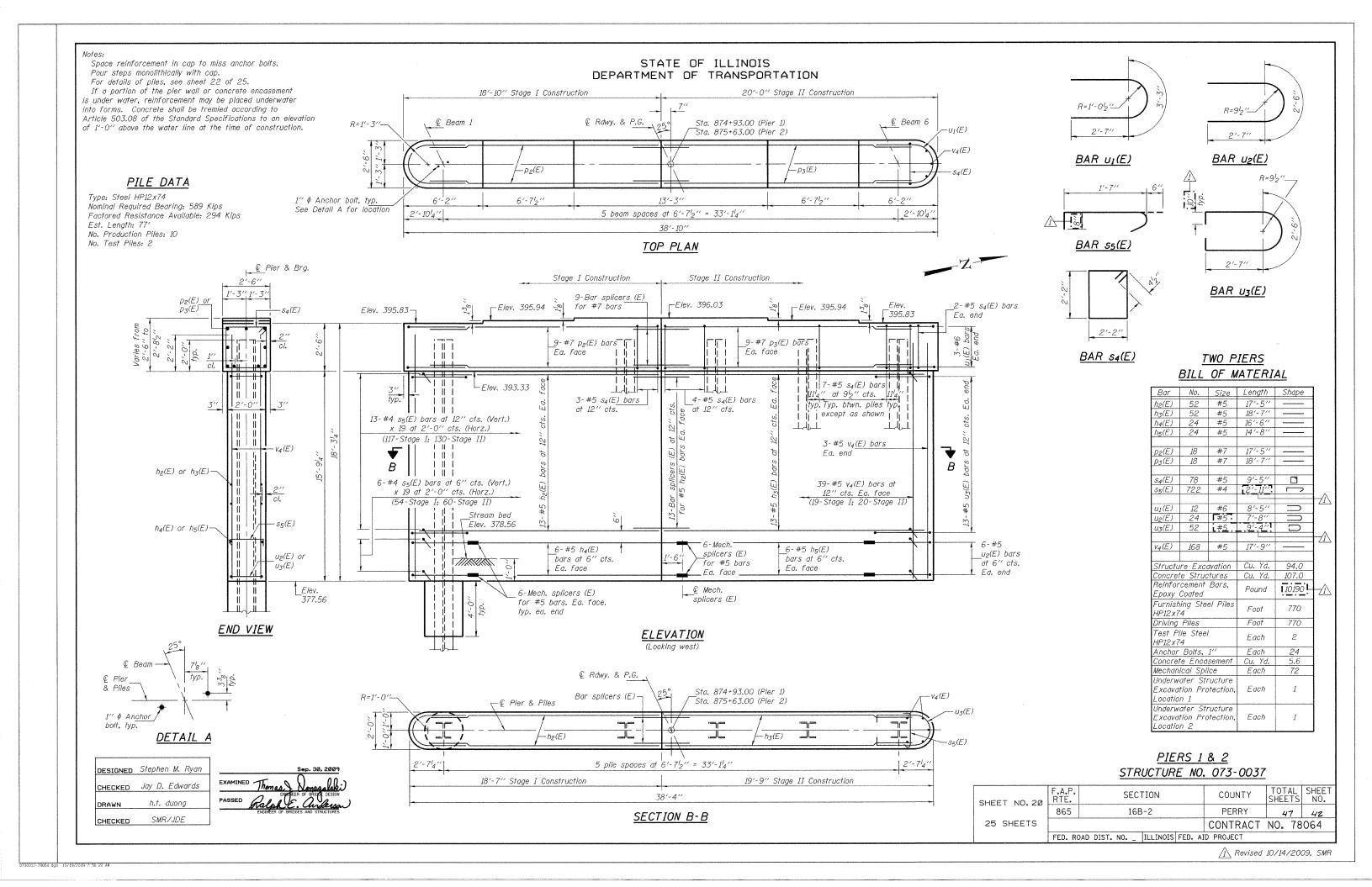
DESIGNED Stephen M. Ryan CHECKED Jay D. Edwards

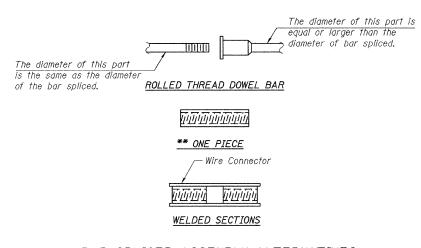
h.t. duong CHECKED SMR/JDE











BAR SPLICER ASSEMBLY ALTERNATIVES

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

Bridge Deck

4'-0"

Threaded or Coil

Loop Couplers (E)

-Stage Construction Line <u>"A"</u> Threaded or Coil Splicer Rods (E) Forms--Foam Plugs Washer Face <u>"B"</u>

INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.

Approach slab

FOR STUB

ABUTMENTS

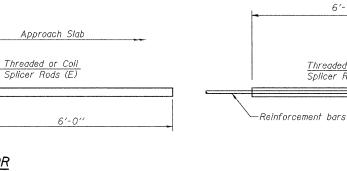
Abutment hatch block

Threaded or Coil

Loop Couplers (E)

6'-0"

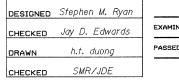
Threaded or Coil Splicer Rods (E)



Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required = 64

FOR INTEGRAL OR

SEMI-INTEGRAL ABUTMENTS



<u>Reinforcement</u>

PASSED

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required =

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

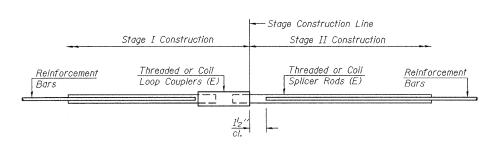
Minimum Capacity (Tension in kips) = $1.25 \times fy \times A_t$

Minimum *Pull-out Strength = $0.66 \times fy \times A_t$ (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

	BAR SPLIC	ER ASSEMBLI	ES
		Strengt	h Requirements
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length		Min. Pull-Out Strength kips - tension
#4	1'-8''	14.7	7.9
#5	2'-2"	23.0	12.3
#6	2'-7''	33.1	17.4
#7	3′-5″	45.1	23.8
#8	4'-6''	58.9	31.3
#9	5′-9′′	75.0	39.6
#10	7′-3′′	95.0	50.3
#11	9'-0''	117.4	61.8



STANDARD

Bar Size	No. Assemblies Required	Location
#5	463	Deck
#6	16	Diaphragm
#7	18	Abutments
#7	18	Piers
#5	52	Piers Sub.
		

BAR SPLICER ASSEMBLY DETAILS STRUCTURE NO. 073-0037

	SHEET NO. 21	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		865	16B-2	PERRY	47	43
	25 SHEETS	25 SHEETS		CONTRACT	NO. 78	064
		FED. ROAD DIST. NO ILLINOIS FED. AID PROJECT				

/1\ Revised 10/14/2009, SMR

BSD-1

10-1-08