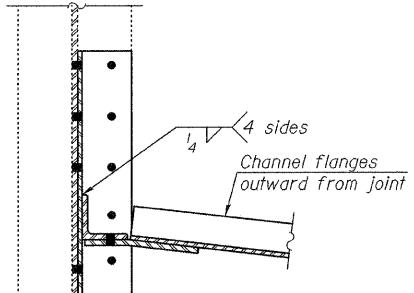
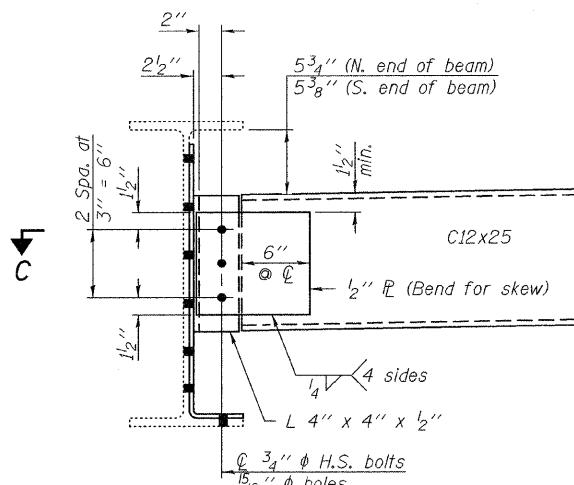


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

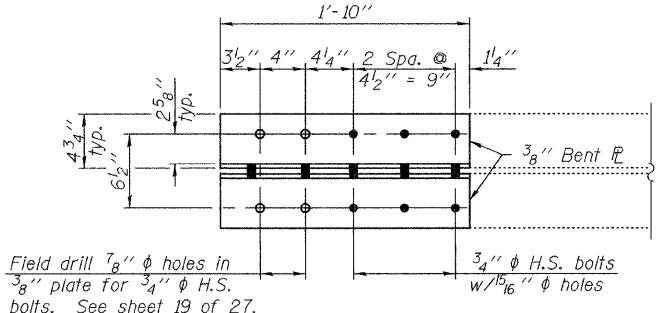


SECTION C-C

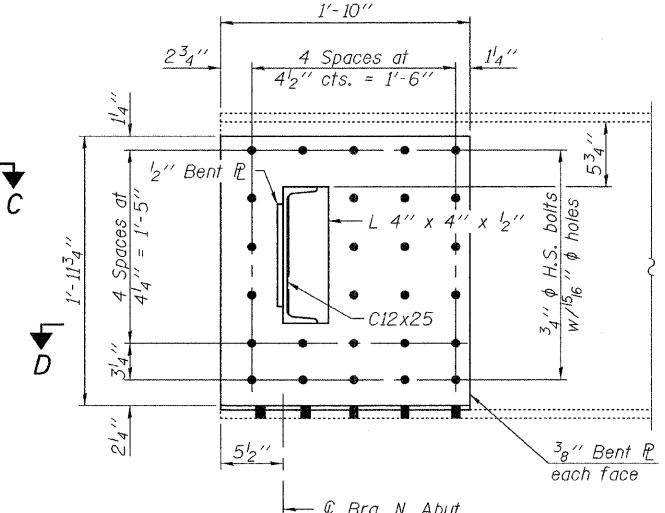


DIAPHRAGM D

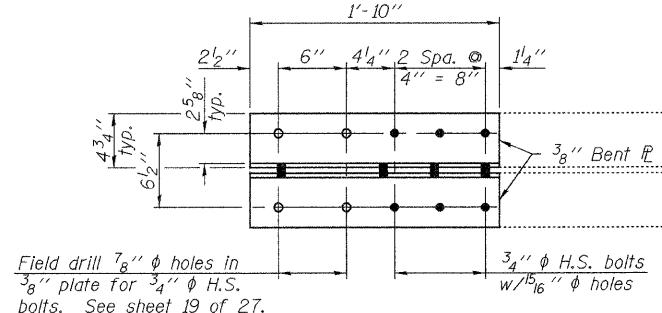
Note:  
Two hardened washers required for each set of oversized holes.



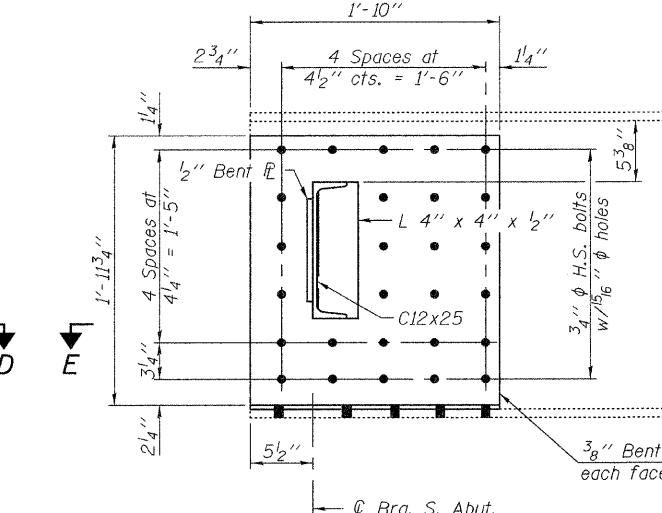
SECTION D-D



NORTH END OF BEAM ELEVATION



SECTION E-E



SOUTH END OF BEAM ELEVATION

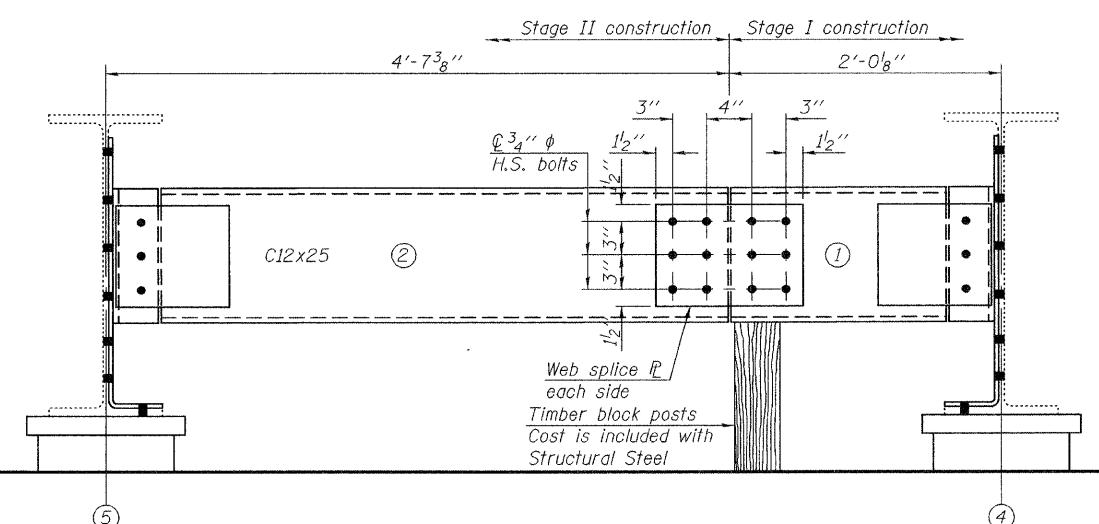
END DIAPHRAGM STAGE CONSTRUCTION SEQUENCE

- 1.) Order diaphragm in two sections.
- 2.) Attach section ① of diaphragm to beam (See Diaphragm D for connection).
- 3.) Place timber block posts between section ① of diaphragm and abutment bearing section.
- 4.) Attach section ② of diaphragm to both beam (See Diaphragm D for connection) and section ① of diaphragm during stage II construction with splice plates.
- 5.) Remove timber block posts.

DESIGNED Nicholas R. Barnett  
CHECKED Michael D. Rolope  
DRAWN Michael B. Mossman  
PASSED Ralph E. Anderson  
CHECKED N.R.B./M.D.R./G.R.A.

September 29, 2009

EXAMINED Thomas J. Domagalski  
ENGINEER OF BRIDGE DESIGN  
27 SHEETS



DIAPHRAGM D1

(North diaphragm D1 looking South, South diaphragm D1 similar)

	0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or Pier 2	0.5 Sp. 2
$I_s$	(in <sup>4</sup> )	3270	3270
$I_o(n)$	(in <sup>4</sup> )	10045	-
$I_o(3n)$	(in <sup>4</sup> )	7483	-
$S_s$	(in <sup>3</sup> )	243	243
$S_c(n)$	(in <sup>3</sup> )	382	-
$S_c(3n)$	(in <sup>3</sup> )	345	345
$Q$	(kip)	0.776	1.07
$M_Q$	(kip)	96.8	201.7
$S_Q$	(kip)	0.294	-
$M_{S_Q}$	(kip)	42.7	42.0
$M_L$	(kip)	244.4	255.5
$M_{Iu}$	(kip)	73.3	73.6
$S_3(M_L + M_I)$	(kip)	529.5	264.8
$M_a$	(kip)	869.8	606.6
$M_u$	(kip)	1021.4	1155.1
$f_s Q$ non-comp	(ksi)	4.8	10.0
$f_s Q$ (comp)	(ksi)	1.5	-
$f_s S_3(M_L + M_I)$	(ksi)	16.6	17.2
$f_s$ (Overload)	(ksi)	22.9	23.1
$f_s$ (Total)	(ksi)	-	30.0
VR	(kip)	38.0	-
			34.1

	0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or Pier 2	0.5 Sp. 2
$I_s$	(in <sup>4</sup> )	3270	3270
$I_o(n)$	(in <sup>4</sup> )	10045	-
$I_o(3n)$	(in <sup>4</sup> )	7483	-
$S_s$	(in <sup>3</sup> )	243	243
$S_c(n)$	(in <sup>3</sup> )	382	-
$S_c(3n)$	(in <sup>3</sup> )	345	345
$Q$	(kip)	0.776	0.776
$M_Q$	(kip)	96.8	70.9
$S_Q$	(kip)	0.294	-
$M_{S_Q}$	(kip)	42.7	42.0
$M_L$	(kip)	244.4	255.5
$M_{Iu}$	(kip)	73.3	73.6
$S_3(M_L + M_I)$	(kip)	529.5	548.5
$M_a$	(kip)	869.8	859.7
$M_u$	(kip)	1021.4	1155.1
$f_s Q$ non-comp	(ksi)	4.8	3.5
$f_s Q$ (comp)	(ksi)	1.5	1.5
$f_s S_3(M_L + M_I)$	(ksi)	16.6	17.2
$f_s$ (Overload)	(ksi)	22.9	22.2
$f_s$ (Total)	(ksi)	-	-
VR	(kip)	38.0	-
			34.1

	Abut.	Pier
$R_Q$	(kip)	17.3
$R_L$	(kip)	32.2
$R_I$	(kip)	9.7
$R_{Total}$	(kip)	59.2
		104.0

\* Compact section

\*\* Braced non-compact and partially braced section

$I_s$ ,  $S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$ (Total and Overload) due to non-composite dead loads ( $I_n^4$  and  $I_n^3$ ).

$I_o(n)$ ,  $S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, " $n$ ", used for computing  $f_s$ (Total and Overload) due to short-term composite live loads ( $I_n^4$  and  $I_n^3$ ).

$I_o(3n)$ ,  $S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, " $3n$ ", used for computing  $f_s$ (Total and Overload) due to long-term composite (superimposed) dead loads ( $I_n^4$  and  $I_n^3$ ).

$Q$ : Un-factored non-composite dead load (kips/ft.).

$M_Q$ : Un-factored moment due to non-composite dead load (kip-ft.).

$S_Q$ : Un-factored long-term composite (superimposed) dead load (kips/ft.).

$M_{S_Q}$ : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).

$M_L$ : Un-factored live load moment (kip-ft.).

$M_I$ : Un-factored moment due to impact (kip-ft.).

$M_a$ : Factored design moment (kip-ft.).

$I_3 [M_Q + M_{S_Q} + \frac{5}{3} (M_L + M_I)]$ : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).

$f_s$  (Overload): Sum of stresses as computed from the moments below (ksi).

$M_Q + M_{S_Q} + \frac{5}{3} (M_L + M_I)$ : Sum of stresses as computed from the moments below on non-compact section (ksi).

$I_3 [M_Q + M_{S_Q} + \frac{5}{3} (M_L + M_I)]$ : Maximum  $\frac{4}{3} +$  impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).

STRUCTURAL STEEL DETAILS  
STRUCTURE NO. 037-0017 (S.B.)

SHEET NO. 18	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
27 SHEETS	74	37-4HB-1	HENRY	148	84
					CONTRACT NO. 64264

ILLINOIS FED. AID PROJECT