
**ROADWAY GEOTECHNICAL REPORT
INTERSTATE 80 IMPROVEMENTS
HOUBOLT ROAD TO WEST OF CENTER STREET
STATION 410+00 TO STATION 518+00
CONTRACT 62R27 - WEST MAINLINE
WILL COUNTY, ILLINOIS**

**For
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**Submitted by
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**Original Report: July 22, 2022
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| 11. Abstract The proposed improvements include roadway reconstruction and widening along I-80 from Station 410+00 to Station 518+00. A major widening is proposed over the existing interstate median, which is currently a ditch, along both directions. The proposed grade will mostly remain the same. At the surface, the borings encountered 1 to 51 inches of silty clay to sandy loam topsoil. The recommended topsoil thickness to be stripped is 7 inches. The existing shoulder pavements are made of either asphalt or concrete with average thickness of 11 inches over aggregate base. The mainline pavements are made of either asphalt over concrete or concrete with average thickness of 13 inches over aggregate base. The existing subgrade consists of variety soil types, including medium dense to dense sand to sandy gravel, medium dense to very dense sandy loam fill, very stiff to hard silty clay to clay loam fill, medium dense to dense RAP aggregate base, or stiff to hard silty clay to clay loam natural ground. With only 17% of the borings encountered groundwater, perched groundwater was observed between 1 and 11 feet below ground surface. The groundwater is mainly deep seated. The subgrade soils will generally provide a stable working platform for the placement of fill and pavement construction. We recommend subgrade treatment of 12 inches undercut for several sections. We recommend placing geofabric at the base of undercut areas. For a mechanistic pavement design, the pavement sections should be designed using an SSR of POOR. For an AASHTO pavement design, the pavement sections should be designed using an IBR of 2. We estimate the embankment will have adequate factors of safety against slope instability and foundation soil settlement will be 1 inch or less. A shrinkage factor of 15% should be used to measure borrowed and furnished excavation quantities. | | |
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1.0 INTRODUCTION

This report presents the results of our subsurface investigation, laboratory testing, and geotechnical evaluations and recommendations in support of the roadway improvements proposed along Interstate 80 (I-80) from Houbolt Road to west of Center Street in Will County, Illinois. A *Site Location Map* is presented as Exhibit 1.

Wang Engineering, Inc. (Wang) understands the proposed improvements include I-80 roadway widening over the median and the outer shoulders between Station 410+00 and Station 518+00. This section of the I-80 is included in Contract 62R27, as west mainline.

The purpose of our investigation was to characterize the pavement, subgrade, and groundwater conditions; perform geotechnical engineering analyses; and provide geotechnical recommendations for the design and construction of the proposed roadway mainline widening. The results of geotechnical investigation, laboratory testing, and geotechnical evaluations and recommendations for east mainline (contract 62R89), Larkin Interchange (contract 62R25), and Wheeler Road (contract 62R30) are addressed in separate Roadway Geotechnical Reports (RGRs).

2.0 GEOLOGICAL SETTING

The project area extends through western Will County, Illinois. On the USGS *Channahon and Plainfield 7.5 Minute Series Quadrangle* maps, the project runs from west to east along the limit between NW ¼ of Section 26, SE ¼ of Section 23 and NW ¼ of Section 24, Tier 35N, Range 9E of the Troy Township of the Third Principal Meridian.

The following review of published geologic data, with emphasis on factors that might influence the design and construction of the proposed engineering works, is meant to place the project area within a geological framework and confirm the dependability and consistency of the subsurface investigation results. For the study of the regional geologic framework, Wang considered northeastern Illinois in general and Will County in particular.

2.1 Physiography

The project area is located within the northern part of the lowland Kankakee Plain physiographic subsection of the Till Plains Section (Leighton et al. 1948). This intermorainic area, once occupied by Glacial Lake Wauponsee, is characterized by flat to gently undulatory topography, with low morainic islands, glacial terraces, torrent bars, and sand dunes. The surface along the project alignment slopes east to west, within the eastern extent of the intermorainal area between Minooka and Rockdale Moraines. The surface elevation along the project alignment ranges from 580 feet at the west end to 605 feet from the center of the section to the east end of the section.

2.2 Pedological Features

After the Wisconsin glaciation, several types of soils developed through weathering of glacigenic sediments. In Will County, the soil types were surveyed by the USDA (2021). A summary of the USDA soil types present within the project area, including their relevant geotechnical index properties and suitability as subgrade and road fill are shown in Exhibits 2-1 to 2-3. The soil information provided by USDA is meant to be used as a general reference in the absence of a site-specific investigation. In this instance, our findings regarding soil features affecting suitability for highway and street construction are not necessarily in agreement with the information presented in the exhibits.

2.3 Surficial Cover

The surficial cover is the result of Wisconsinan-age glacial activity. The glacigenic deposits were emplaced during pulsating advances and retreats of an ice-sheet lobe responsible for the formation of end moraines and associated low-relief till and lake plains (Hansel and Johnson 1996). Along the project area, the drift thickness varies from about 5 feet to 50 feet. Predominantly the drift is dominated by silty clay diamicton of the Yorkville Member of the Lemont Formation. In the project area, discontinuous patches of lacustrine deposits of the Equality Formation and alluvium of the Cahokia Formation resting over sand and gravel outwash of the Henry Formation may be encountered in sag areas or channels carved by meltwater into silty clayey diamicton of the Yorkville Member of the Lemont Formation (Hansel and Johnson 1996, Willman et al. 1971). Occasionally, beneath the

Lemont Formation diamicton, sand and gravel outwash of the Henry Formation may be found filling bedrock valleys. Exhibit 3 illustrates the *Site and Regional Geology*.

The Equality Formation, less than 10 feet thick, consists of brown to gray, bedded fine sand, silt, and clay lacustrine deposits (Caron 2017). The Henry Formation consists of stratified sand and gravel outwash with thicknesses of about 5 to 40 feet (Caron 2017). The Yorkville Member of the Lemont Formation, up to 70-foot thick, consists of yellowish brown to gray silty clay to silty clay loam diamicton that contains lenses of gravel, sand, silt, and clay (Hansel and Johnson 1996, Caron 2017).

From a geotechnical viewpoint, the Yorkville Member is characterized by low to moderate plasticity, high strength, and low to moderate moisture content (Bauer et al. 1991).

2.4 Bedrock

Within the project limits, the surficial cover rests unconformably on top of Silurian-age bedrock that dips eastward. The top of the bedrock lies at 5 to 50 feet below the ground surface (bgs). The bedrock is Silurian-age dolostone (Kolata 2005), slightly to highly weathered.

Structurally, the site is located on the eastern flank of the Wisconsin Arch. The northwest to southeast trending inactive Sandwich Fault Zone is about 2.5 miles southwest of the project.

2.5 Climatological Data

The subsurface investigation was performed in April of 2021 and from April to May of 2022. To assess the possible effects of temperature and precipitation on water table data and soil moisture, the climatic conditions for the investigation period and three months prior to the start of the investigation are summarized graphically in Figures 1 through 4. The precipitation and temperature data for the investigation period are compared against thirty-year monthly data (1991 to 2020) in box-and-whiskers format to show deviations from “normal” climate conditions during the current investigation. Local climate data were obtained from the O’Hare Station (NCDC 2022).

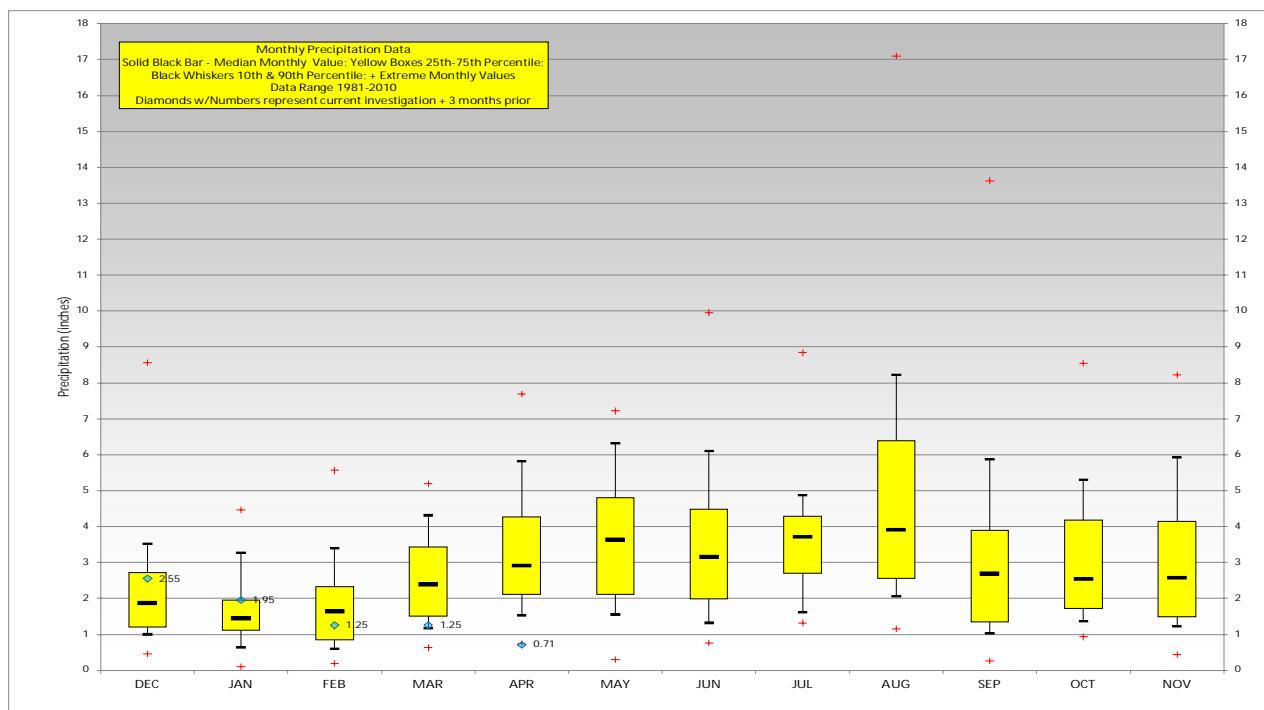


Figure 1: Monthly Precipitation Data for 2020 to 2021

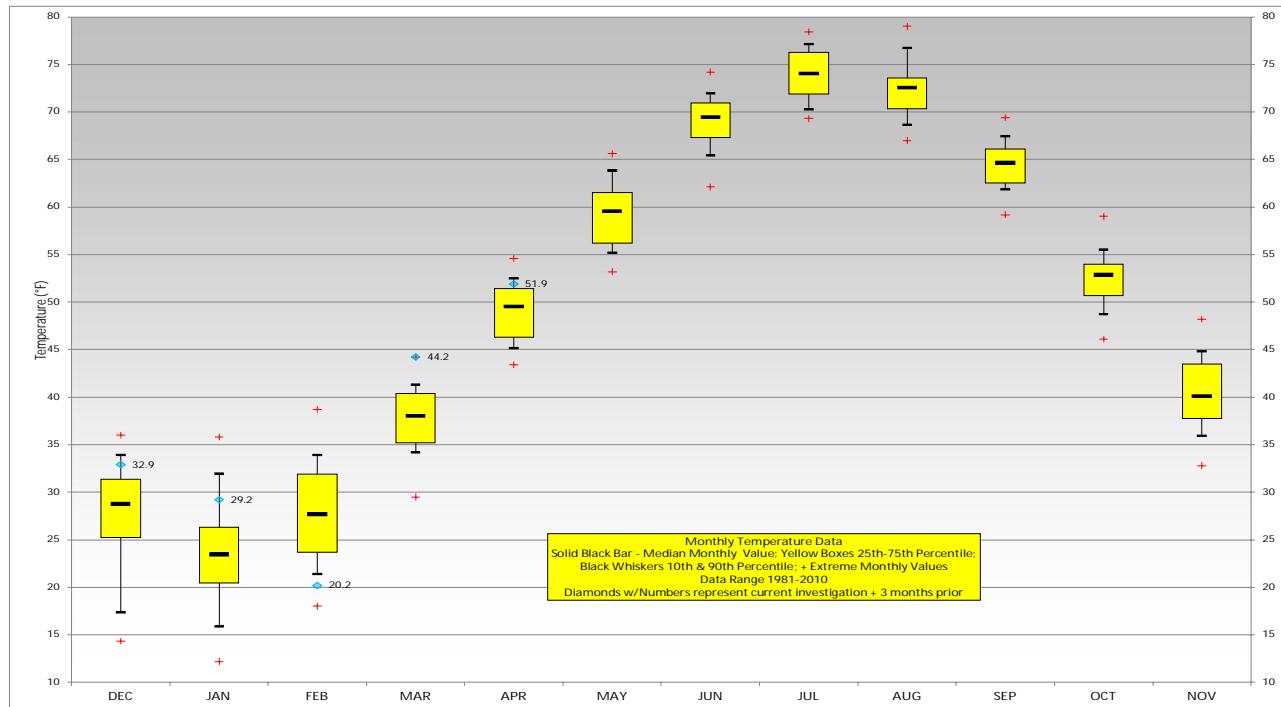


Figure 2: Monthly Temperature Data for 2020 to 2021

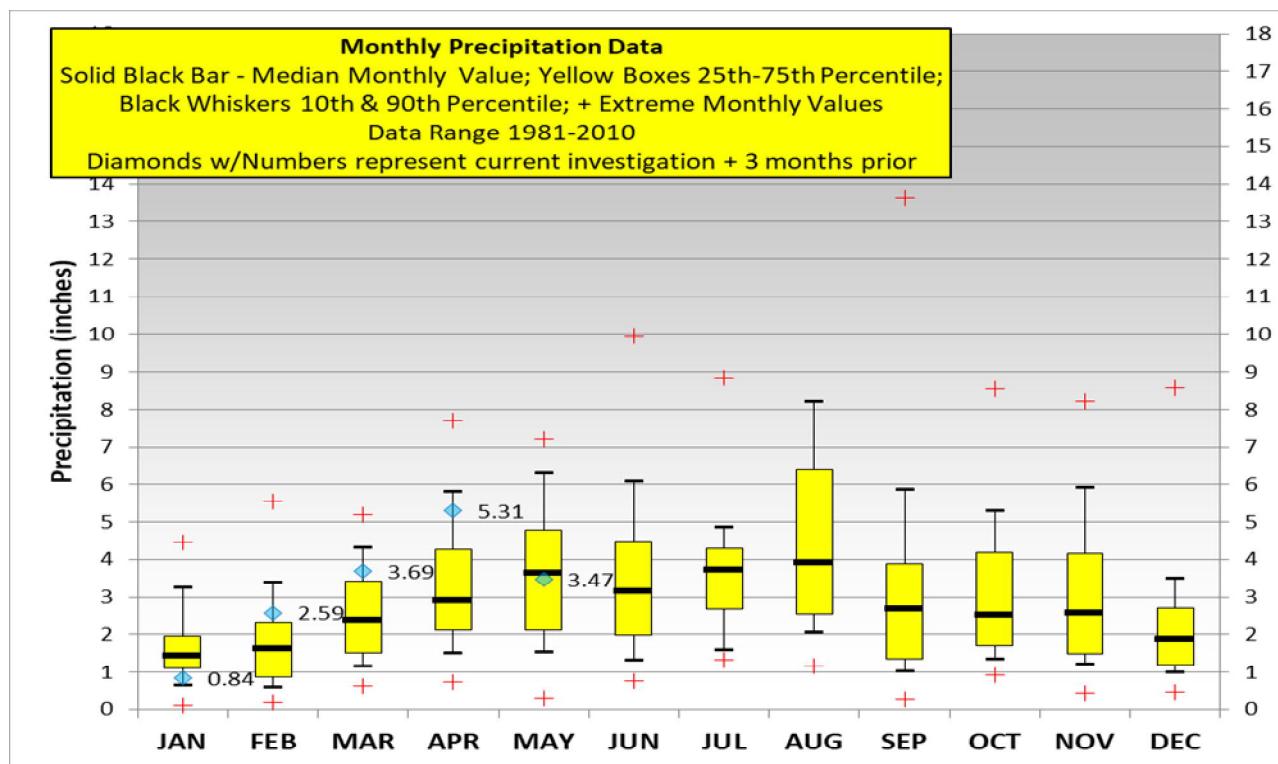


Figure 3: Monthly Precipitation Data for 2022

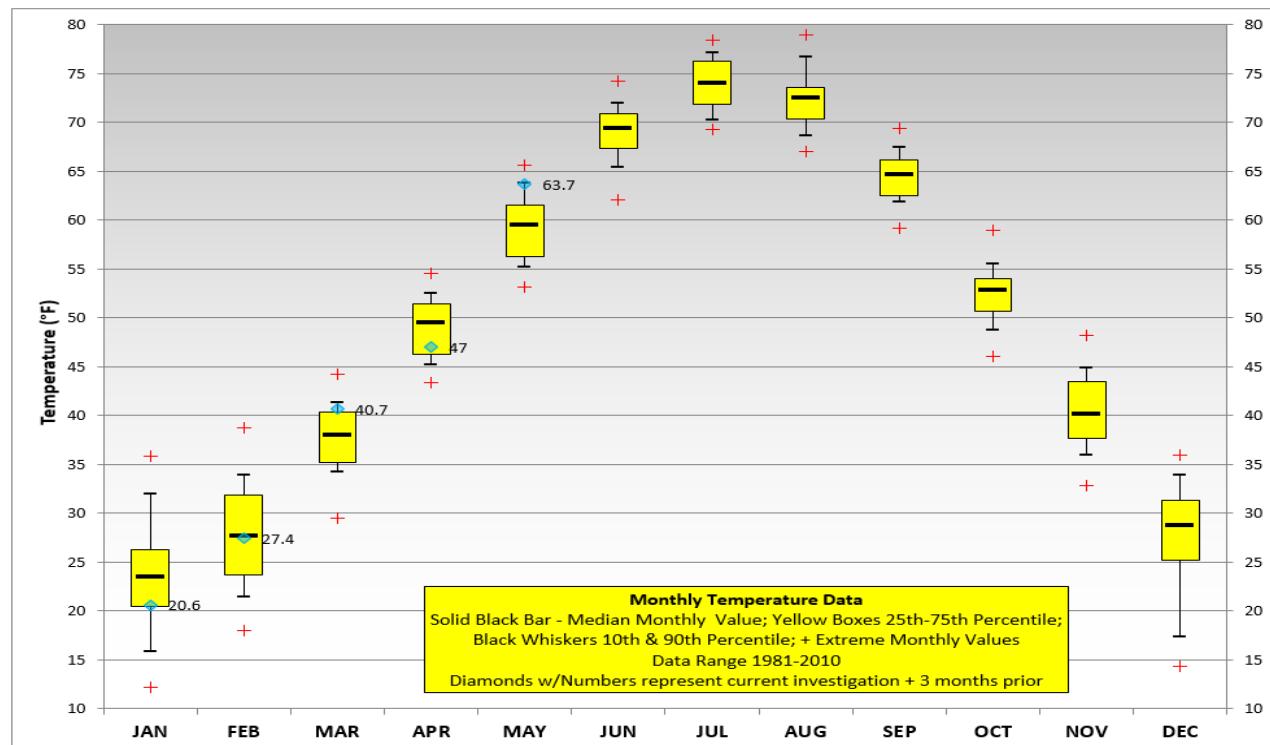


Figure 4: Monthly Temperature Data for 2022

The deviations from the historical 30-year climate data show the investigation period was characterized in general by average precipitations and average to high temperatures with the exception of record high temperatures and average to low precipitation in March 2021 and May 2022 and record low temperatures with average precipitation in February 2021. A record low precipitation with high temperature was recorded in April 2021. Observations of perched water within the granular fill may have been influenced by these climate factors.

3.0 METHODS OF INVESTIGATION

The following sections outline the subsurface and laboratory investigations performed by Wang.

3.1 Field Investigation

The subsurface investigation along the I-80 mainline west section, between Stations 410+00 and 518+00, consisted of subgrade borings (SGB) drilled along the I-80 eastbound (EB), westbound (WB) and centerline/median (CL). To supplement the subsurface data, we considered for our analysis soil borings performed for nearby bridge structure (BSB) boring. The roadway borings were drilled by Wang in April and May of 2022. The bridge boring was drilled in April 2021. The borings were drilled from surface elevations of 579.0 to 605.0 feet and were advanced to depths of 6.0 to 12.0 feet bgs. A summary of soil borings, ground surface elevations, and termination depths is provided in Table 1.

Table 1: Soil Boring Summary

| Roadway Alignment | Alignment Limits (Station to Station) | Location | Reference Borings IDs | Ground Surface Elevations (feet) | Termination Depths (feet) |
|-------------------|---------------------------------------|-------------|--|----------------------------------|---------------------------|
| I-80 | 410+00.00 to 518+00.00 | Eastbound | EB-SGB-01 through EB-SGB-17 | 584.4 to 604.5 | 10.5 to 12.0 |
| | | Center line | CL-SGB-01 through CL-SGB-16 | 579.0 to 601.8 | 6.0 to 10.0 |
| | | Westbound | WB-SGB-01 through WB-SGB-18, HR-BSB-01 | 580.1 to 605.0 | 7.0 to 11.0 |

The as-drilled northing and easting coordinates were surveyed by Wang with a mapping-grade GPS unit, whereas the stations, offsets, and elevations were provided by TranSystems. Boring location data are presented in the *Boring Logs* (Appendix A) and the as-drilled locations are shown in the *Boring Location Plans and Soil Profiles* (Appendix G).

Geoprobe, ATV-, and truck-mounted drilling rigs equipped with hollow stem augers were used to advance and maintain open boreholes. Soil sampling was performed according to AASHTO T206, "*Penetration Test and Split Barrel Sampling of Soils.*" The soil was sampled continuously to the boring termination depths. Soil samples collected from each sampling interval were placed in sealed jars and transported to the laboratory for further examination and laboratory testing.

Field boring logs, prepared and maintained by a Wang field engineer, included lithological descriptions, visual-manual soil classifications, results of Rimac and/or pocket penetrometer unconfined compressive strength tests, and results of Standard Penetration Tests (SPT) recorded as blows per 6 inches of penetration. The N-values shown in the *Boring Location Plans and Soil Profiles* (Appendix G) are the sum of the second and third set of blows per 6 inches of penetration.

Groundwater levels were measured while drilling and at the completion of each boring. For safety considerations each borehole was backfilled upon completion with soil cuttings and bentonite chips and the pavement surface was restored as close as possible to the original condition.

3.2 Laboratory Testing

The soil samples were tested in the laboratory for moisture content (T265). Atterberg limits (T89 and T90), particle size analysis (T88), and organic content by loss on ignition (T267) tests were performed on select samples. Field visual descriptions of the soil samples were verified in the laboratory and the soils were classified according to the IDH and AASHTO Soil Classification Systems. The laboratory test results are shown in the *Boring Logs* (Appendix A), in the *Laboratory Test Results* (Appendix B), in the *IDOT Forms* (Appendix C), and in the *Boring Location Plans and Soil Profile* (Appendix G).

4.0 INVESTIGATION RESULTS

Detailed descriptions of the soil conditions encountered during the subsurface investigation are presented in the attached *Boring Logs* (Appendix A) and in the *Boring Location Plans and Soil Profile* (Appendix G). Please note that the strata contact lines shown on the logs and profiles represent approximate boundaries between soil types. The actual transition between soil types in the field may be gradual in horizontal and vertical directions.

4.1 Surface Characterization

The proposed improvement will include reconstruction and widening within the median and outer shoulders. Most of eastbound and westbound borings were drilled through pavement, and all centerline borings were drilled through grassy area in the existing ditch. Topsoil measurements were performed off the paved areas, within the improvement right-of-way (ROW) to supplement the topsoil data obtained from borings. Topsoil thicknesses are summarized in Table 2.

Table 2: Summary of Topsoil Thickness

| Alignment | Number of Measurements | Topsoil Thickness Range (inches) | Average Thickness (inches) |
|-----------|------------------------|----------------------------------|----------------------------|
| EB | 17 | 3 to 9 | 5 |
| CL | 16 | 1 to 51 | 11 |
| WB | 16 | 4 to 8 | 6 |

The borings were mostly drilled through paved shoulders. The borings drilled on the shoulders show pavement structures consisting of either asphalt or concrete. The pavement thickness ranges from 3 to 16 inches with an average of 11 inches. The aggregate base consists either reclaimed asphalt grinds, gravel, gravelly sand or sandy gravel and its thickness ranges from 2 to 24 inches. Pavement structure thicknesses of gravel, sandy gravel, or reclaimed asphalt pavement (RAP) and its thickness ranges from 1 to 21 inches. Shoulder pavement thicknesses are summarized in Table 3.

Table 3: Summary of Shoulder Pavement Thickness and Composition

| Alignment | Total Number of Measurements (No) | Pavement Structure Thickness (inches) | | | Average Pavement Thickness (inches) |
|-----------|-----------------------------------|---------------------------------------|---------------------------------|---------------------------------------|-------------------------------------|
| | | Asphalt No ¹ /Range | Concrete No ¹ /Range | Total Pavement No ¹ /Range | |
| I-80 | EB | 16 | 11/8-10 | 5/10-12 | 16/8-12 |
| | WB | 18 | 11/3-16 | 7/10-16 | 18/3-16 |

¹No = number of measurements along the alignment

Additional pavement cores were obtained from I-80 roadway pavements. The cores obtained from the travel lanes show various pavement structures consisting of asphalt over concrete or concrete. The travel lanes pavement thickness ranges from 12 to 14 inches with an average of 13 inches. The aggregate base consists of either gravel or RAP. Roadway pavement structure thicknesses are summarized in Table 4. The breakdown of pavement composition for both shoulder and mainline are included in Appendix D.

Table 4: Summary of Roadway Lane Pavement Thickness and Composition

| Alignment | Total Number of Measurements (No) | Pavement Structure Thickness (inches) | | | Average Pavement Thickness (inches) |
|-----------|-----------------------------------|---------------------------------------|---------------------------------|---------------------------------------|-------------------------------------|
| | | Asphalt No ¹ /Range | Concrete No ¹ /Range | Total Pavement No ¹ /Range | |
| I-80 | EB | 4 | 2/4 | 4/8-13.5 | 4/12-14 |
| | WB | 3 | 2/4 | 3/8.25-12.75 | 3/12.25-12.75 |

¹No = number of measurements along the alignment

4.2 Subgrade Conditions

Beneath the surface, in descending order, the lithologic succession encountered includes: 1) man-made ground (fill); 2) soft to hard silty clay; 3) stiff to hard silty clay to silty clay loam; 4) loose to very dense sand, sandy loam, and sandy gravel; 5) dense to very dense silty loam; 6) very dense weathered bedrock; and 7) dolostone bedrock. The following section presents the subgrade conditions encountered within top 20 feet along the roadway alignment by our subsurface investigation. Thus, the top four units geotechnical properties are presented below.

1) Man-made ground (fill)

Beneath the surface, the borings encountered up to 16.3 feet of cohesive and granular fill along I-80. Granular fill consists of loose to very dense sandy gravel aggregate base, reclaimed asphalt grinds, sandy loam, sand, gravelly sand, to gravelly loam with N values of 9 blows per foot to greater than 50 blows per 3 inches, and moisture content values of 1 to 18% with an average of 8%. The cohesive fill generally consists of medium stiff to hard silty clay, clay loam, loam to silty clay loam with unconfined compressive strength (Q_u) values of less than 0.2 to 6.0 tsf with an average of 2.9 tsf, and moisture content values of 6 to 45% with an average of 17%. Laboratory index testing shows liquid limit (L_L) values of 26 to 42% and plastic limit (P_L) values of 12 to 18%. The soil belongs primarily to the A-6 and A-7-6 group in accordance with AASHTO.

Table 5: Summary of Unit 1 Properties

| Alignment | Q_u Min-Max/Avg. (tsf) | SPT N-values Min-Max/Avg. (blows per foot) | Moisture Content Min-Max/Avg (%) | Liquid Limit Min-Max (%) | Plastic Limit Min-Max (%) |
|-----------|--------------------------------|--|--|--------------------------------|---------------------------------|
| EB | <0.2-6.0/2.9 | 9- >50/5"/33 | 1-24/10 | 26-31 | 12-13 |
| CL | 0.7-5.3/2.7 | 10- >50/3"/53 | 6-45/18 | 40 | 18 |
| WB | 0.8-5.3/3.0 | 13-44/26 | 3-27/15 | 32-42 | 13-16 |

Buried topsoil was encountered below the fill in 13 borings along I-80. Buried topsoil thickness varies from 6 to 24 inches; it is a black and dark brown silty clay, silty clay loam to sandy loam characterized by Q_u values of less than 0.2 to 4.7 tsf, moisture content of 18 to 75%, L_L value of 50%, and plasticity index (PI) value of 24%.

2) Soft to hard silty clay lacustrine deposits

Beneath the fill, topsoil, or buried topsoil, at elevations of 585 to 598 feet (1.8 to 8.8 feet bgs), the borings encountered 1.5- to 7.5-foot thick, soft to hard silty clay lacustrine deposits, discontinuously present along the alignments. The unit is characterized by Q_u values of 0.3 to 5.5 tsf, averaging 2.5 tsf, SPT N-values of 3 to 26 blows per foot, averaging 14 blows per foot, moisture content of 15 to 42% and an average of 24%, L_L values of 48 to 60%, and P_L of 18 to 24%. The AASHTO soil classification show the soil belongs to A-7-6 group. Within this unit, lenses of sand and silt are discontinuously encountered. Lenses are 0.5- to 2.5-foot thick, moist to saturated, with N-values of 3 to 12 blows per foot, and moisture content values of 21 to 25%.

Table 6: Summary of Unit 2 Properties

| Alignment | Q_u | SPT N-values | Moisture Content | Liquid Limit | Plasticity Index |
|-----------|-----------------------|----------------------------------|--------------------|----------------|------------------|
| | Min-Max/Avg. (tsf) | Min-Max/Avg. (blows per foot) | Min-Max/Avg (%) | Min-Max (%) | Min-Max (%) |
| EB | 1.3-5.5/3.0 | 9-26/15 | 16-25/21 | 48-60 | 30-36 |
| CL | 0.8-4.4/2.4 | 3-20/12 | 15-42/27 | NA | NA |
| WB | 0.3-3.2/2.0 | 10-24/15 | 16-31/24 | NA | NA |

3) Stiff to hard silty clay to silty clay loam diamicton

Below the surface, fill, buried topsoil or Unit 2, at elevations of 574 to 600 feet (1 to 20.5 feet bgs), the borings advanced through stiff to hard silty clay to silty clay loam diamicton. Throughout this unit, occasional silt and sand lenses are encountered. The unit is characterized by Q_u values of 1.5 to 7.3 tsf averaging 3.6 tsf, SPT N-values of 8 blows per foot to spoon refusal averaging 19 blows per foot, and moisture content values of 10 to 26% averaging 18%.

Table 7: Summary of Unit 3 Properties

| Alignment | Q_u | SPT N-values | Moisture Content | Liquid Limit | Plasticity Index |
|-----------|-----------------------|----------------------------------|--------------------|----------------|------------------|
| | Min-Max/Avg. (tsf) | Min-Max/Avg. (blows per foot) | Min-Max/Avg (%) | Min-Max (%) | Min-Max (%) |
| EB | 1.5-4.2.2/2.8 | 10-28/16 | 15-24/18 | NA | NA |

| Alignment | Q _u | SPT N-values | Moisture Content | Liquid Limit | Plasticity Index |
|-----------|-----------------------|----------------------------------|--------------------|----------------|------------------|
| | Min-Max/Avg. (tsf) | Min-Max/Avg. (blows per foot) | Min-Max/Avg (%) | Min-Max (%) | Min-Max (%) |
| CL | 1.5-7.0/4.2 | 9- >50/1" /25 | 10-26/18 | NA | NA |
| WB | 1.6-7.3/3.9 | 8-30/16 | 16-22/18 | NA | NA |

4) Loose to very dense sand, sandy loam, and sandy gravel outwash

Below the surface, fill, or Units 2 and 3, at elevations of 573 to 604 feet (0.8 to 23 feet bgs), borings encountered 0.2 to 23 feet of loose to very dense sand, sandy loam, gravelly sand and sandy gravel outwash. The unit is characterized by SPT N-values of 6 blows per foot to spoon refusal averaging 34 blows per foot, and moisture content values of 2 to 19% averaging 7%.

Table 8: Summary of Unit 4 Properties

| Alignment | SPT N-values | Moisture Content |
|-----------|----------------------------------|--------------------|
| | Min-Max/Avg. (blows per foot) | Min-Max/Avg (%) |
| EB | 6-44/25 | 2-13/6 |
| CL | 11-94/34 | 4-19/11 |
| WB | 6- >50/6" /42 | 3-14/7 |

4.3 Groundwater Conditions

Groundwater was recorded during and upon completion of drilling. The groundwater was encountered in 17% of the roadway borings, perched within granular lenses, mainly along I-80 between Station 441+00 to Station 454+00 and between Station 483+50 to Station 510+00. However, it should be noted that groundwater levels might change with seasonal rainfall patterns or may be influenced by local site conditions. A groundwater data summary is presented in Table 9.

Table 9: Summary of Groundwater Measurements

| Roadway Alignment | Groundwater measurements No ¹ /out of ² | Groundwater while drilling | | Groundwater after drilling | |
|-------------------|--|----------------------------|-------------------|----------------------------|-------------------|
| | | (feet) | | (feet) | |
| | | Depth min-max | Elevation min-max | Depth min-max | Elevation min-max |
| EB | 2/17 | 1.0-10.5 | 588.8-601.6 | 11.0 | 588.3 |
| CL | 7/16 | 6.0-8.0 | 584.2-595.5 | 8.0-10.0 | 584.9-593.5 |
| WB | 1/19 | 28.5 | 576.4 | NA | NA |

¹ number of borings that encountered groundwater; ² total number of borings drilled along the alignment

5.0 ANALYSIS AND RECOMMENDATIONS

Cross-section drawings indicate the proposed grade will be slightly changed. Up to 5 feet raise in grade is proposed along the existing median ditch. Some of the proposed outer embankment grades will require two to three feet of fill or up to five feet of cut through side slopes along I-80. Major cuts are proposed between Stations 437+00 and 443+00 to accommodate the excavation of a detention basin. The side slope will be graded mainly at 1:4 to 1:6 (V:H).

5.1 Site Preparation

For the proposed pavement widening and reconstruction, it is recommended that any topsoil and existing pavement be stripped within the limits of the improvements. For estimating purposes, the topsoil thickness to be stripped is 7 inches. As per IDOT District One, a shrinkage factor of 15% should be used to measure borrowed and furnished excavation quantities.

After stripping, the stability of the exposed subgrade should be observed for the presence of any unsuitable and/or unstable soils to determine if remedial treatment is necessary. The prepared subgrade should be proofrolled to check for rutting and subgrade deformation. Using a static or dynamic cone penetrometer, any unstable and/or unsuitable soils revealed during proofrolling should be tested and evaluated according to the IDOT *Subgrade Stability Manual* (IDOT 2005). The side slopes along the right and left offsets should be benched to accommodate the new embankment fill. We recommend including the IDOT District One benching detail (Appendix H) in the contract plans.

5.2 Subgrade Treatment Recommendations

Based on the results of our investigation, the subgrade will consist of variety soil types, including medium dense to dense sand to sandy gravel, medium dense to very dense sandy loam fill, very stiff to hard silty clay to clay loam fill, medium dense to dense RAP aggregate base, or stiff to hard silty clay to clay loam natural ground. The proposed pavement structure will be supported on existing fill, natural ground, or new fill.

The soil borings indicate the proposed subgrade generally consists of soils with Q_u values greater than 1.0 tsf, moisture contents of less than 25%, and L_L values below 50%. Overall, the subgrade soils will provide a stable working platform for the construction of the new pavement structure and the aggregate base. However, a few borings revealed soil with moisture content values higher than 30% or Q_u values less than 1 tsf. At these boring locations we are recommending subgrade treatment as summarized in Table 10. The proposed treatment undercuts are below the 12 inches of aggregate subgrade improvement that is included in as part of the proposed pavement section.

The improved subgrade should be in accordance with the IDOT Bureau of Design and Environment (BDE) *Aggregate Subgrade Improvement* Special Provision (April 1, 2022). We recommend placing geotextile fabric at the base of undercut areas. Fabric should meet the requirements of Article 210, Fabric for Ground Stabilization of IDOT *Standard Specifications* (IDOT 2022).

Table 10: Summary of Subgrade Treatment Recommendations

| Limits Station to Station | Treatment Width | Treatment Type | Treatment Depth ⁽¹⁾ (inch) | Reference Boring, Subgrade Concerns |
|------------------------------|----------------------------|--------------------------------|---------------------------------------|---|
| I-80 EB 514+60 to 516+70 | EB pavement ⁽²⁾ | Aggregate Subgrade Improvement | 12 | EB-SGB-17 (Q_u = 0.74 tsf) |
| I-80 CL 422+50 to 424+50 | Within Existing Median | Aggregate Subgrade Improvement | 36 | CL-SGB-02 Topsoil (MC= 35%) |
| I-80 CL 500+70 to 502+70 | Within Existing Median | Aggregate Subgrade Improvement | 24 | CL-SGB-14 Buried Topsoil (LL=50%; MC=75%) |
| I-80 WB 412+00 to 416+50 | WB pavement ⁽²⁾ | Aggregate Subgrade Improvement | 12 | WB-SGB-01 (Q_u = 0.75 tsf; MC=27%) |

⁽¹⁾ The treatment depths are below 12 inches of aggregate improvement that is included in proposed pavement section.

⁽²⁾ The proposed treatment limits under EB or WB proposed pavement and shoulders should be from the outside edge of the outside shoulder to the inside median barrier.

The proposed treatment limits under EB or WB pavement should be from the outside edge of the shoulder to the inside median. Other than topsoil removal for site preparation, the existing median will not need to follow treatment recommendation under EB or WB pavement. Especially in areas where the proposed roadway is much higher than the existing median.

Any highly moist soils, if not otherwise unsuitable or unstable, encountered within the exposed roadway subgrade should be disked or tilled, dried, and compacted before placing the new pavement structure.

As per IDOT District One, *in addition to the undercuts recommended in Table 10, we recommend that a plan quantity of Aggregate Subgrade Improvement (CU YD) equal to 25% of the planned full depth pavement area assuming a thickness of 12 inches should be added for estimating purposes. This material should be used to replace any unsuitable soils below the bottom of the improved subgrade layer that are encountered in the field during construction. The actual need for removal and replacement with Aggregate Subgrade Improvement should be determined in the field at the time of construction by the Geotechnical Engineer or soils inspector. All potentially unstable soils should be tested with a cone penetrometer and treated in accordance with Article 301.04 of the SSRBC and the undercut guidelines in the IDOT Subgrade Stability Manual. Any material not needed for undercut replacement at the time of construction should be deleted from the contract with no extra compensation to the contractor.*

Based on the above recommendation, there will be a need for two separate Aggregate Subgrade Improvement line items in the Schedule of Quantities (SOQ) included in the design plans:

- **AGGREGATE SUBGRADE IMPROVEMENT 12" (SQ YD)** – *This will be used for the 12 inch aggregate subgrade improvement below new pavement sections and widening pavement sections.*
- **AGGREGATE SUBGRADE IMPROVEMENT (CU YD)** – *This will be used in locations where there are undercuts (below the 12 inch improved subgrade layer) where poor soils were removed.*

It should be noted that both above items refer to the IDOT Bureau of Design and Environment (BDE) Aggregate Subgrade Improvement Special Provision (April 1, 2022).

As per IDOT District One, we also recommend including a plan quantity of **geotechnical fabric for ground stabilization (SQ YD)** equal to at least 25% of the planned pavement area in addition to the areas in the Table 10. We recommend placing geotextile fabric at the base of undercut areas where low strength subgrade soils are encountered. The 12 inches of improved subgrade is not considered an undercut, and we do not recommend placing the fabric at the base of the proposed 12 inch improved subgrade layer unless it is determined to be necessary to achieve stability by the Geotechnical Engineer or soils inspector at the time of construction. Fabric should meet the requirements of Article 210, *Fabric for Ground Stabilization*, of the SSRBC. Any material not needed at time of construction should be deleted from the contract with no extra compensation to the contractor.

The frost depth for pavement design in northern Illinois could be expected to range from 45 to 60 inches (IDOT 2020). Within the frost susceptible depths, most of the samples tested in the laboratory had plasticity indices (PI) of 14 to 36%. In our opinion, the soils will exhibit low to moderate frost susceptibility. Adequate drainage will suffice to alleviate frost heave.

5.3 Pavement Design Recommendations

For a Mechanistic Pavement Design (MPD), IDOT rates the subgrade using the Subgrade Support Rating (SSR). Laboratory testing on representative samples of the subgrade soil shows SSR ratings of POOR to FAIR (Exhibit 4). Considering the worst subgrade conditions and unknown new fill type, we recommend an SSR of POOR be used for the purpose of pavement design. Pavement structure conforming to IDOT's MPD requires a minimum of 12 inches of improved subgrade below the design pavement structure to ensure stability during construction and long-term pavement performance (IDOT 2020).

For an AASHTO pavement design, the subgrade soil support is characterized using the Illinois Bearing Ratio (IBR). Based on soil tests and classifications of A-7-6 and A-6, we recommend that the pavement be designed based on an IBR value of 2 (IDOT 2020).

5.4 Embankment and cut sections

Based on the cross-sections drawings, the proposed I-80 embankment widenings will have minor cuts into existing slopes or fill placed on the existing embankment slope and centerline. The slope will be graded mainly at 1:4 to 1:6 (V: H). We have evaluated the potential long-term settlement and global slope stability at critical sections along the proposed improvements.

5.4.1 Settlement

In general, we do not anticipate excessive settlement. We performed settlement analysis at selected sections with the highest fill and lower soil strength. Settlement estimates have been made based on correlations to measured index properties obtained from the laboratory tests (Appendix B). We Settlement evaluations are summarized and presented in Table 11. Unless specify in Table 10, removal and replacement is not required at locations shown in table 11.

Table 11: Summary of Estimated Consolidation Settlements

| Alignment | Approximate Station | New Fill Height (feet) | Reference Boring(s) | Concern | Estimated Settlement (inches) |
|-----------|---------------------|------------------------|---------------------|------------------------------|-------------------------------|
| I-80 CL | 436+00 | 3.5 | CL-SGB-04 | MC= 25 to 30% below topsoil | 0.14 |
| I-80 CL | 452+00 | 3.5 | WB-SGB-07 | Soft soil Qu= 0.25 at 593 ft | 0.50 |
| I-80 CL | 466+00 | 2.5 | CL-SGB-09 | MC = 26% | 0.10 |
| I-80 CL | 472+00 | 3.0 | CL-SGB-10 | MC = 27 to 39% | 0.30 |
| I-80 CL | 484+00 | 3.5 | CL-SGB-12 | MC= 42% at 592 LL= 60% | 0.33 |
| I-80 CL | 502+00 | 2.5 | CL-SGB-14 | MC=25% | 0.20 |

5.4.2 Global Stability

The proposed embankment and cut side slopes will be graded mainly at 1:4 to 1:6 (V: H). The global stability at a critical section was analyzed at Station 443+00 based on the soil information from the nearest borings. In this area, a major cut is anticipated for the excavation of the regional detention basin. The analysis shows the factors of safety (FOS) of 5.3 and 3.3, higher than IDOT's minimum requirement of 1.7 for cuts. Slope stability analyses results are included in Appendix F.

5.5 Roadway Drainage

The proposed subgrade and pavement should have proper surface grading to prevent the pooling of water. The soils encountered beneath the proposed subgrade will exhibit poor to fair drainage characteristics. The fill material to be placed for rising the grade in the center line and in support of the widening will likely be cohesive and will also exhibit poor drainage characteristics. We

recommend installing longitudinal pipe underdrains under the edge of new pavement in widening areas, and transverse pipe underdrains at the low points in the profile, spaced 300 feet on either side of the low point, and at the base of any undercuts. For transverse underdrains at the low points where the distance to the previous or the following high point is greater than 600 feet, we recommend two transverse underdrains to be placed at 300 feet interval on either side. The pipe underdrains should be 4 inches in diameter and should be installed per Article 601 in the IDOT *Standard Specifications* (IDOT 2022) and consist of Type 2 underdrains (Adopted 1, 2022).

6.0 CONSTRUCTION CONSIDERATIONS

6.1 Excavation, Dewatering, and Utilities

Excavations should be performed in accordance with local, state, and federal regulations. The potential effect of ground movements upon nearby utilities should be considered during construction. Excavations should be sloped at no steeper than 1:2 (V: H) for cohesive soils and 1:2.5 (V:H) for granular soils.

We do not anticipate the need for special dewatering systems. However, during and immediately following periods of heavy precipitation, the excavations may encounter perched groundwater within any granular layers interbedded within the cohesive layers. Therefore, the Contractor should ensure proper surface grading to prevent pooling of water and run-off into open excavations. Any water allowed to enter excavations should immediately be removed via sump-pump.

6.2 Filling and Backfilling

Fill material used for replacement of any unstable and/or unsuitable soils encountered during construction should be pre-approved by the Engineer. The fill material should be free of organic matter and debris and should be placed in lifts compacted in accordance with Section 205, *Embankment* (IDOT 2022). For new fill to be placed on existing slopes, we recommend benching the slopes according to IDOT embankment construction details.

6.3 Reuse of Materials

Site soils may be reused as embankment fill if testing shows it conforms to the following criteria: a) L_L less than 50%; b) PI value of more than 12%; c) maximum dry density greater than 90 pcf according to AASHTO T99; and d) organic content less than 10%. The soils should be removed, brought to within $\pm 2\%$ of the optimum moisture content and recompacted according to Section 205,

Embankment (IDOT 2022).

6.4 Earthwork Operations

The required earthwork can be accomplished with conventional construction equipment. Moisture and traffic will cause deterioration of the exposed subgrade soils. Precautions should be taken by the Contractor to prevent water erosion of the exposed subgrade. A compacted subgrade will minimize water runoff erosion.

Earth moving operations should be scheduled to avoid excessive cold or wet weather (early spring, late fall or winter). Any soil allowed to freeze or soften due to the standing water should be removed. Wet weather can cause problems with subgrade compaction.

It is recommended that an experienced geotechnical engineer be retained to inspect the exposed subgrade, monitor earthwork operations, and provide material inspection services during the construction phase of this project.

7.0 QUALIFICATIONS

The analyses and recommendations in this report are based upon data obtained from the borings drilled at the locations shown on the *Boring Logs* (Appendix A) and in the *Boring Location Plans and Soil Profiles* (Appendix G). This report does not reflect any variations that may occur between the borings or elsewhere on the site, variations whose nature and extent may not become evident until the course of construction. In the event that any changes in the design and/or location of the proposed improvements are planned, we should be timely informed so that our recommendations can be adjusted accordingly.

It has been a pleasure to assist TranSystems and the Illinois Department of Transportation on this project. Please call if there are any questions, or if we can be of further service.

Respectfully Submitted,

WANG ENGINEERING, INC.

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Senior Engineer

Jessica Bensen, P.G.
Senior Staff Geologist

Corina T Farez, P.E., P.G.
QA/QC Reviewer

REFERENCES

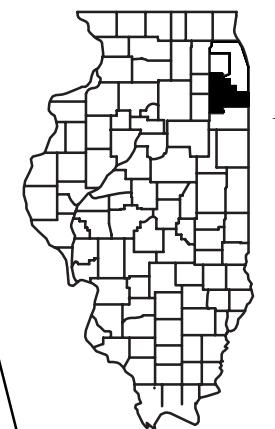
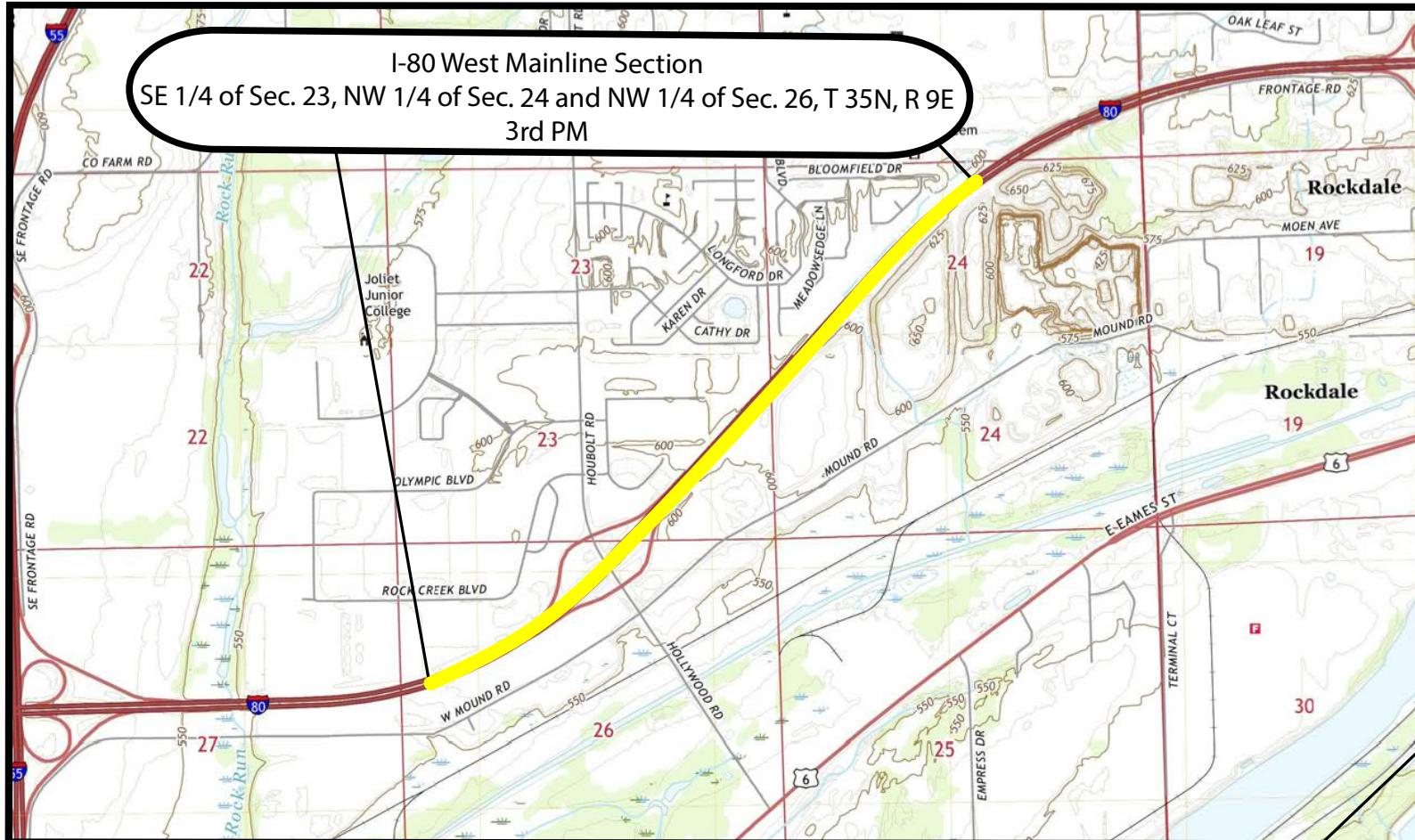
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EXHIBITS

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Scale

0 0.5 1.0 Mile

SITE LOCATION MAP: I-80 RECONSTRUCTION; WEST MAINLINE FROM STATION 0410+00 TO 0518+00, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

EXHIBIT 1

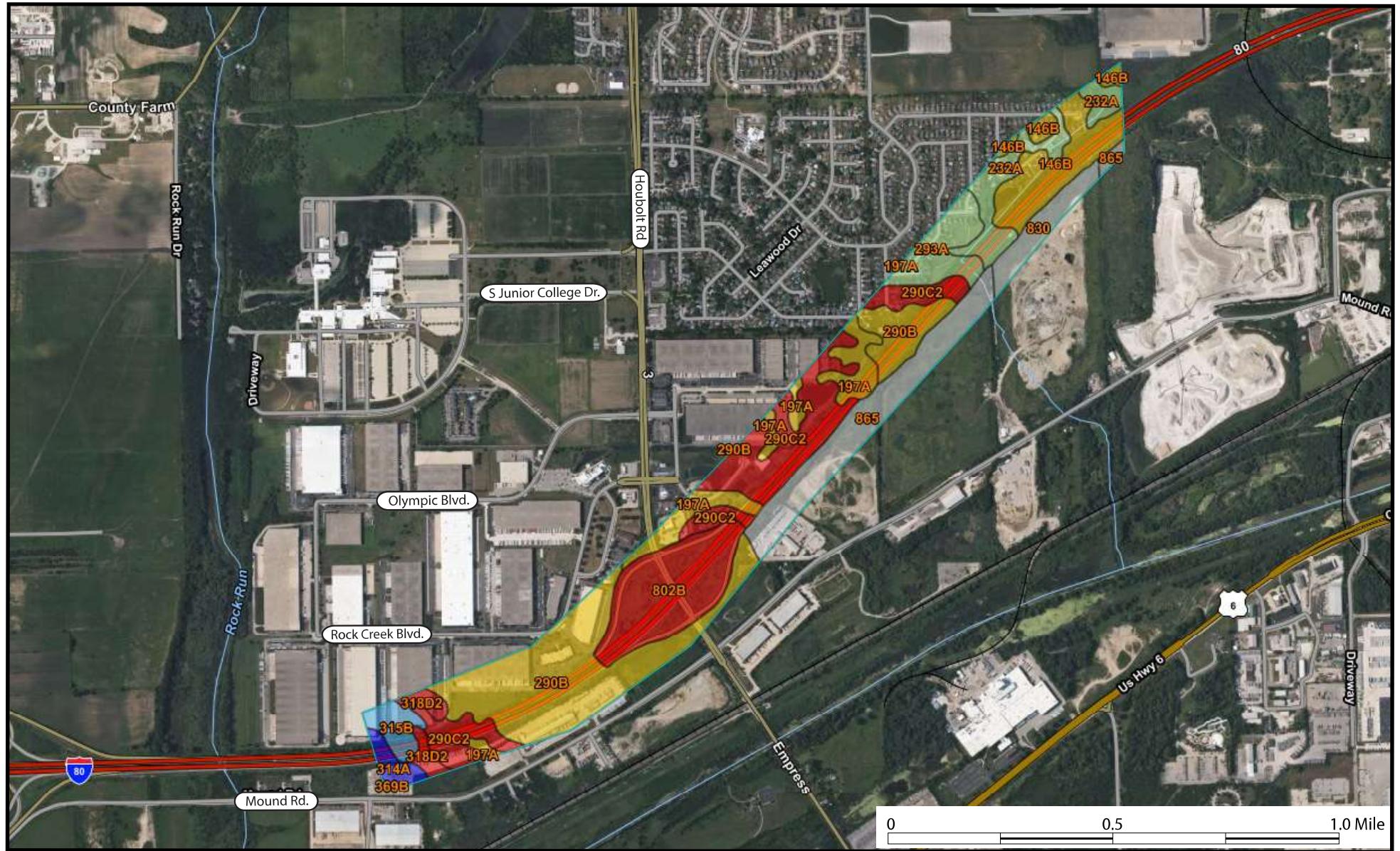
DRAWN BY: E. Greenwood
CHECKED BY: A. Kurnia



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7901-15-01



Organic Matter Soil Rating

| | |
|---|----------------------------|
| | <= 0.82 |
| | > 1.50 and <= 2.10 |
| | > 0.82 and <= 1.27 |
| | > 2.10 and <= 3.82 |
| | > 1.27 and <= 1.50 |
| | Not rated or not available |

SITE PEDOLOGY MAP: I-80 RECONSTRUCTION; WEST MAINLINE FROM STATION 0410+00 TO 0518+00, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

EXHIBIT 2-1

DRAWN BY: J. Bensen
CHECKED BY: C. Marin



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| Map unit symbol and soil name | Depth | USDA texture | Classification | Pct Fragments | | Sand | Silt | Clay | Moist bulk density | Saturated hydraulic conductivity | Organic matter | Liquid limit | Plasticity index | Erosion factors | | | Potential as a source of roadfill | Local Roads and Streets | Shallow Excavations | |
|---|-------|--|--------------------------|---------------|------------|----------|----------|----------|--------------------|----------------------------------|----------------|--------------|------------------|-----------------|-------|---|--|--|--|--|
| | | | | AASHTO | >10 inches | | | | | | | | | Kw | Kf | T | Rating class; and Limiting features | Rating class; and Limiting features | Rating class; and Limiting features | |
| | In | | | | | L-R-H | L-R-H | Pct | Pct | Pct | g/cc | micro m/sec | Pct | L-R-H | L-R-H | | | | | |
| 146B—Elliott silt loam, 2 to 4 percent slopes | | | | | | | | | | | | | | | | | | | | |
| Elliott | 0-9 | Silt loam | A-6, A-7-6 | 0-0-0 | 0-0-0 | 2-10-15 | 58-65-76 | 22-25-27 | 1.30-1.40-1.45 | 4.23-9.17-14.11 | 3.0-4.3-5.0 | 38-44-47 | 15-17-18 | 0.32 | 0.32 | 3 | Poor, Low strength, Wetness, Dusty | Very limited; Frost action, Low strength, Depth to saturated zone, Dusty, Unstable excavation walls, Ponding, Shrink-swell | Very limited; Depth to saturated zone, Dusty, Unstable excavation walls, Ponding, Too clayey | |
| | 9-13 | Silty clay loam | A-7-6 | 0-0-0 | 0-0-0 | 2-8-15 | 50-62-71 | 27-30-35 | 1.25-1.35-1.45 | 4.23-9.17-14.11 | 2.5-3.3-4.0 | 41-46-53 | 18-21-24 | 0.28 | 0.28 | | | | | |
| | 13-17 | silty clay loam, silty clay | A-7-6 | 0-0-0 | 0-0-0 | 2-7-15 | 40-51-61 | 37-42-49 | 1.35-1.45-1.55 | 1.41-2.82-4.23 | 0.5-1.0-1.6 | 46-52-60 | 26-30-35 | 0.32 | 0.32 | | | | | |
| | 17-35 | silty clay, silty clay loam | A-6, A-7-6 | 0-0-0 | 0-0-1 | 2-10-20 | 40-55-65 | 27-35-45 | 1.45-1.55-1.75 | 0.42-1.41-4.23 | 0.1-0.4-0.8 | 34-43-55 | 17-24-32 | 0.43 | 0.43 | | | | | |
| | 35-60 | silty clay loam | A-6, A-7-6 | 0-0-0 | 0-0-2 | 3-10-20 | 42-60-70 | 27-30-38 | 1.65-1.75-1.85 | 0.42-0.92-1.41 | 0.0-0.2-0.5 | 34-38-46 | 16-19-26 | 0.49 | 0.49 | | | | | |
| 197A—Troxel silt loam, 0 to 2 percent slopes | | | | | | | | | | | | | | | | | | | | |
| Troxel | 0-7 | Silt loam | A-4, A-6 | 0-0-0 | 0-0-0 | 2-9-15 | 58-69-80 | 18-23-27 | 1.20-1.30-1.40 | 4.23-9.17-14.11 | 3.0-4.0-5.0 | 20-25-30 | 5-10-15 | 0.32 | 0.32 | 5 | Poor, Low strength, Shrink-swell, Dusty | Very limited; Frost action, Low strength | Somewhat limited; Depth to saturated zone, Dusty, Unstable excavation walls | |
| | 7-32 | Silt loam | A-4, A-6 | 0-0-0 | 0-0-0 | 2-9-15 | 58-67-80 | 18-24-27 | 1.30-1.40-1.50 | 4.23-9.17-14.11 | 1.0-2.0-3.0 | 20-25-35 | 5-13-20 | 0.43 | 0.43 | | | | | |
| | 32-62 | Silty clay loam, silt loam | A-6, A-7-6 | 0-0-0 | 0-0-0 | 2-12-20 | 50-58-73 | 25-30-35 | 1.35-1.45-1.55 | 4.23-9.17-14.11 | 0.2-0.6-1.0 | 25-35-45 | 10-18-25 | 0.37 | 0.37 | | | | | |
| | 62-80 | Stratified sandy loam to gravelly clay loam | A-2-4, A-2-6, A-4, A-6 | 0-0-1 | 0-3-4 | 15-40-60 | 5-35-70 | 15-25-35 | 1.0-1.52-1.65 | 4.23-9.17-14.11 | 0.1-0.3-0.5 | 20-30-35 | 5-13-20 | 0.20 | 0.32 | | | | | |
| 232A—Ashkum silty clay loam, 0 to 2 percent slopes | | | | | | | | | | | | | | | | | | | | |
| Ashkum, drained | 0-12 | Silty clay loam | A-7-5, A-7-6 | 0-0-0 | 0-0-0 | 1-8-15 | 45-55-64 | 35-37-40 | 1.20-1.35-1.45 | 1.41-2.82-4.23 | 3.0-5.0-8.0 | 51-58-67 | 25-26-28 | 0.20 | 0.20 | 5 | Poor, Wetness, Low strength, Shrink-swell, Dusty | Very limited; Ponding, Depth to saturated zone, Shrink-swell, Frost action, Low strength | Very limited; Dusty, Unstable excavation walls, Ponding, Depth to saturated zone, Too clayey | |
| | 12-29 | Silty clay loam, silty clay | A-7-6 | 0-0-0 | 0-0-0 | 2-8-15 | 43-51-63 | 35-41-42 | 1.30-1.40-1.50 | 1.41-2.82-4.23 | 0.5-1.3-2.5 | 46-54-58 | 25-30-30 | 0.32 | 0.32 | | | | | |
| | 29-54 | Silty clay loam, silty clay | A-6, A-7-6 | 0-0-0 | 0-0-1 | 5-9-20 | 40-58-65 | 30-33-42 | 1.50-1.60-1.70 | 1.41-2.82-4.23 | 0.1-0.3-1.0 | 39-43-53 | 21-23-30 | 0.43 | 0.43 | | | | | |
| | 54-60 | Silty clay loam | A-6, A-7-6 | 0-0-0 | 0-0-1 | 5-9-20 | 45-61-68 | 27-30-35 | 1.55-1.65-1.75 | 1.41-2.82-4.23 | 0.0-0.3-1.0 | 37-41-47 | 19-21-25 | 0.43 | 0.43 | | | | | |
| 290B—Warsaw silt loam, 2 to 4 percent slopes | | | | | | | | | | | | | | | | | | | | |
| Warsaw | 0-10 | Silt loam | A-4, A-6, A-7-6 | 0-0-0 | 0-0-0 | 10-20-30 | 50-60-75 | 15-20-25 | 1.30-1.40-1.50 | 4.23-9.17-14.11 | 2.5-3.3-4.0 | 30-37-43 | 9-13-17 | 0.32 | 0.32 | 3 | Fair, Dusty | Somewhat limited; Frost action, Low strength, Shrink-swell, Depth to saturated zone | Somewhat limited; Dusty, Unstable excavation walls | |
| | 10-24 | Sandy clay loam, loam, clay loam, silty | A-6, A-7-6 | 0-0-0 | 0-1-3 | 10-35-60 | 8-36-70 | 20-29-32 | 1.35-1.48-1.60 | 4.23-9.17-14.11 | 0.5-1.3-2.0 | 32-42-47 | 13-20-23 | 0.28 | 0.28 | | | | | |
| | 24-34 | Gravelly loam, gravelly sandy clay loam, gravelly clay loam, gravelly sandy loam | A-2-6, A-2-7, A-6, A-7-6 | 0-0-1 | 0-2-4 | 30-53-70 | 0-22-50 | 18-25-30 | 1.40-1.53-1.65 | 4.23-9.17-14.11 | 0.2-1.0-1.5 | 29-38-44 | 12-17-21 | 0.15 | 0.24 | | | | | |
| | 34-60 | Stratified gravelly loamy sand to extremely gravelly coarse sand | A-1-a, A-1-b, A-3 | 0-1-1 | 1-2-3 | 80-85-98 | 0-10-18 | 2-5-8 | 1.50-1.60-1.70 | 141.14-423.42-705.00 | 0.0-0.3-0.5 | 0-17-21 | NP-2-4 | 0.02 | 0.02 | | | | | |
| 290C2—Warsaw silt loam, 4 to 6 percent slopes, eroded | | | | | | | | | | | | | | | | | | | | |
| Warsaw | 0-8 | Silt loam | A-4, A-6 | 0-0-0 | 0-0-0 | 10-20-30 | 50-60-75 | 15-20-25 | 1.30-1.40-1.50 | 4.23-9.17-14.11 | 2.0-2.5-3.0 | 20-25-30 | 4-10-15 | 0.43 | 0.43 | 3 | Fair, Dusty | Somewhat limited; Frost action, Shrink-swell | Somewhat limited; Dusty, Unstable excavation walls | |
| | 8-16 | sandy silty clay, loam, clay loam, silty clay loam | A-4, A-6 | 0-0-0 | 0-1-3 | 10-18-60 | 8-53-70 | 20-29-32 | 1.35-1.48-1.60 | 4.23-9.17-14.11 | 0.5-1.3-2.0 | 20-30-40 | 8-14-20 | 0.37 | 0.37 | | | | | |
| | 16-27 | Gravelly loam, gravelly sandy clay loam, gravelly clay loam, gravelly sandy loam | A-2-4, A-2-6, A-4, A-6 | 0-0-1 | 0-2-4 | 30-43-70 | 0-28-50 | 18-29-30 | 1.40-1.53-1.65 | 4.23-9.17-14.11 | 0.2-1.0-1.5 | 20-28-35 | 5-13-20 | 0.15 | 0.24 | | | | | |
| | 27-60 | Stratified gravelly loamy sand to extremely gravelly coarse sand | A-1-a | 0-1-1 | 1-2-3 | 80-85-98 | 0-10-18 | 2-5-8 | 1.50-1.60-1.70 | 141.14-423.42-705.00 | 0.0-0.3-0.5 | 0-8-15 | NP | 0.02 | 0.05 | | | | | |

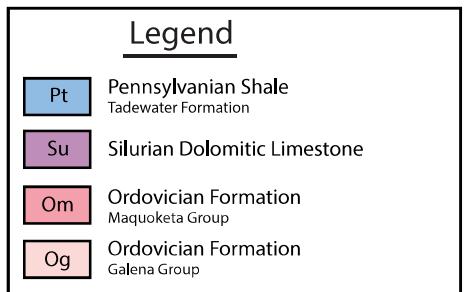
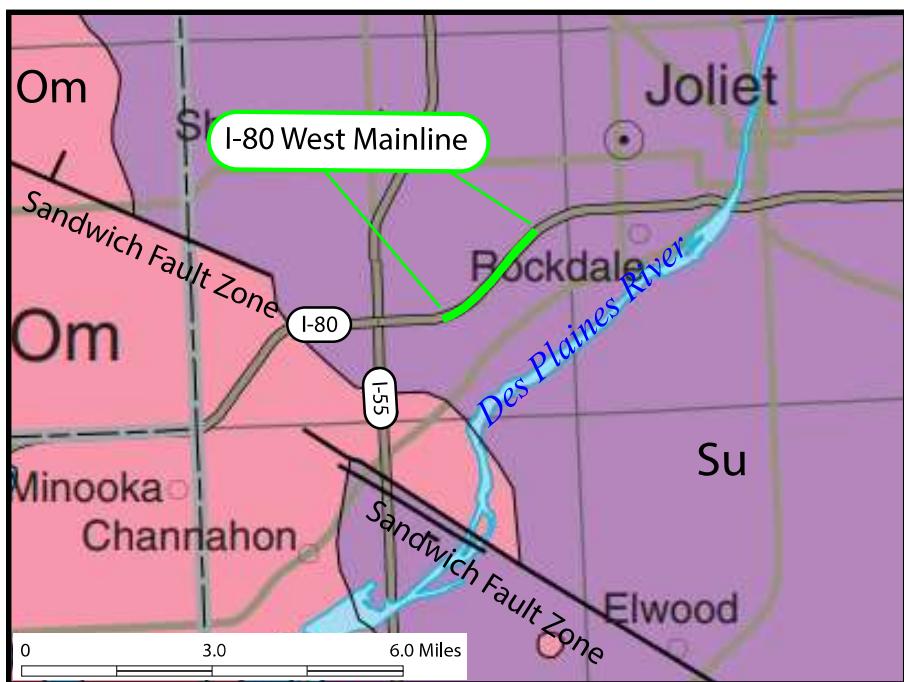
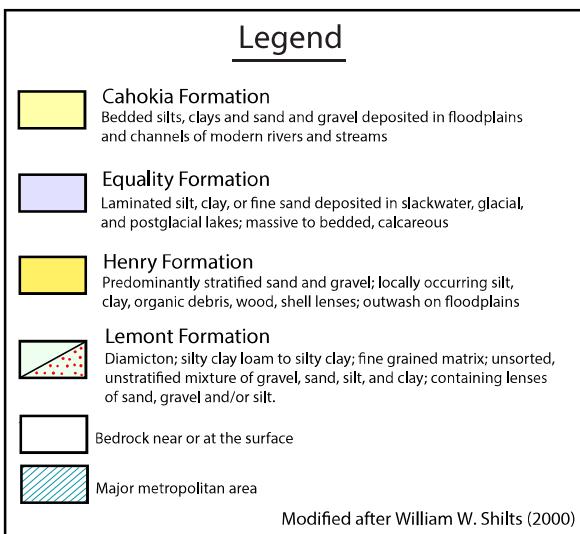
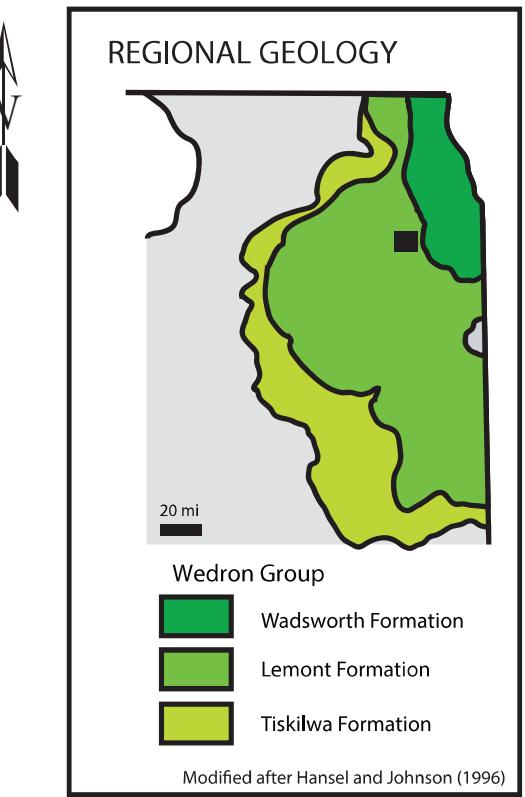
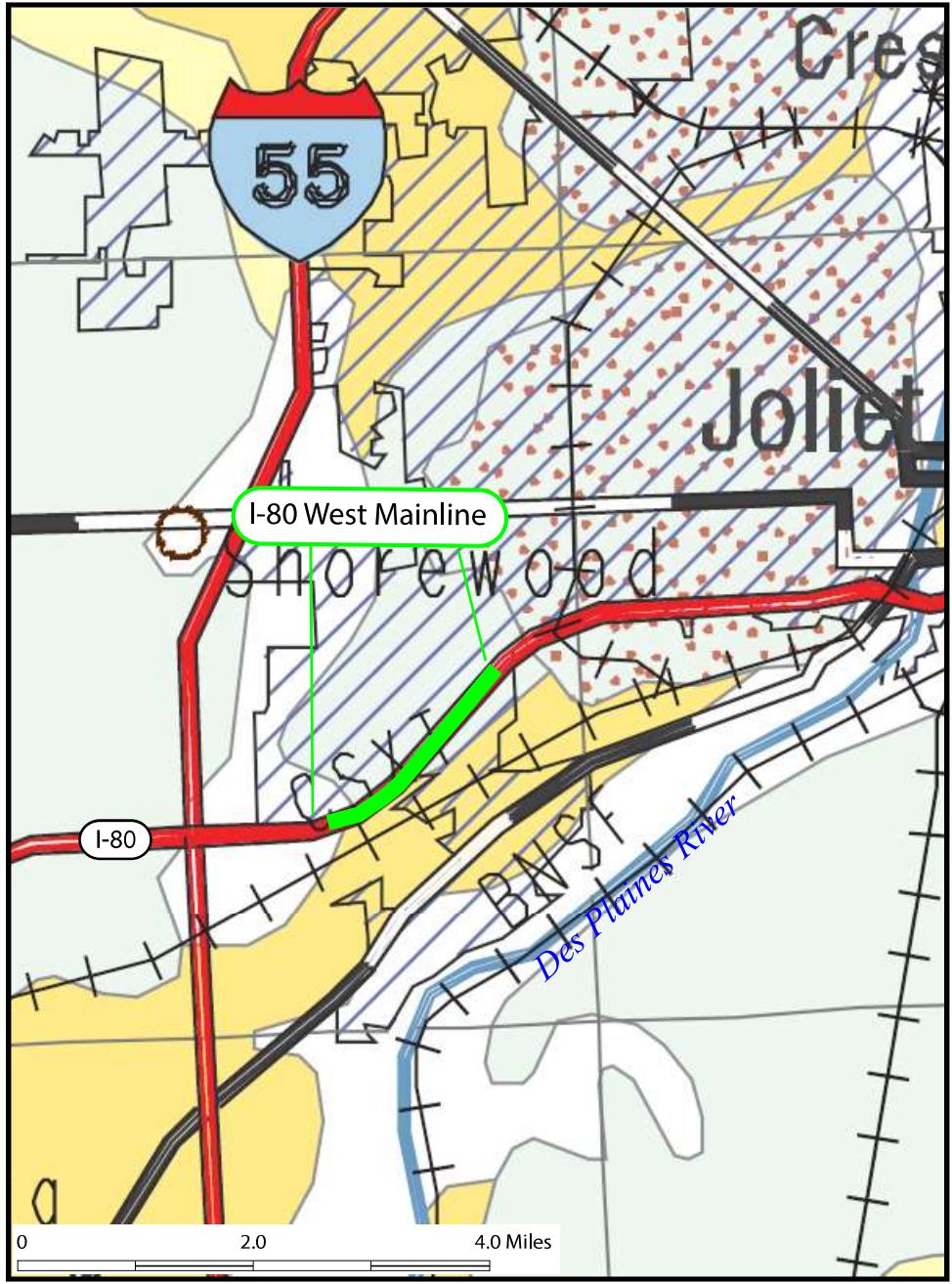
Source: USDA, Natural Resources Conservation Service; Web Soil Survey

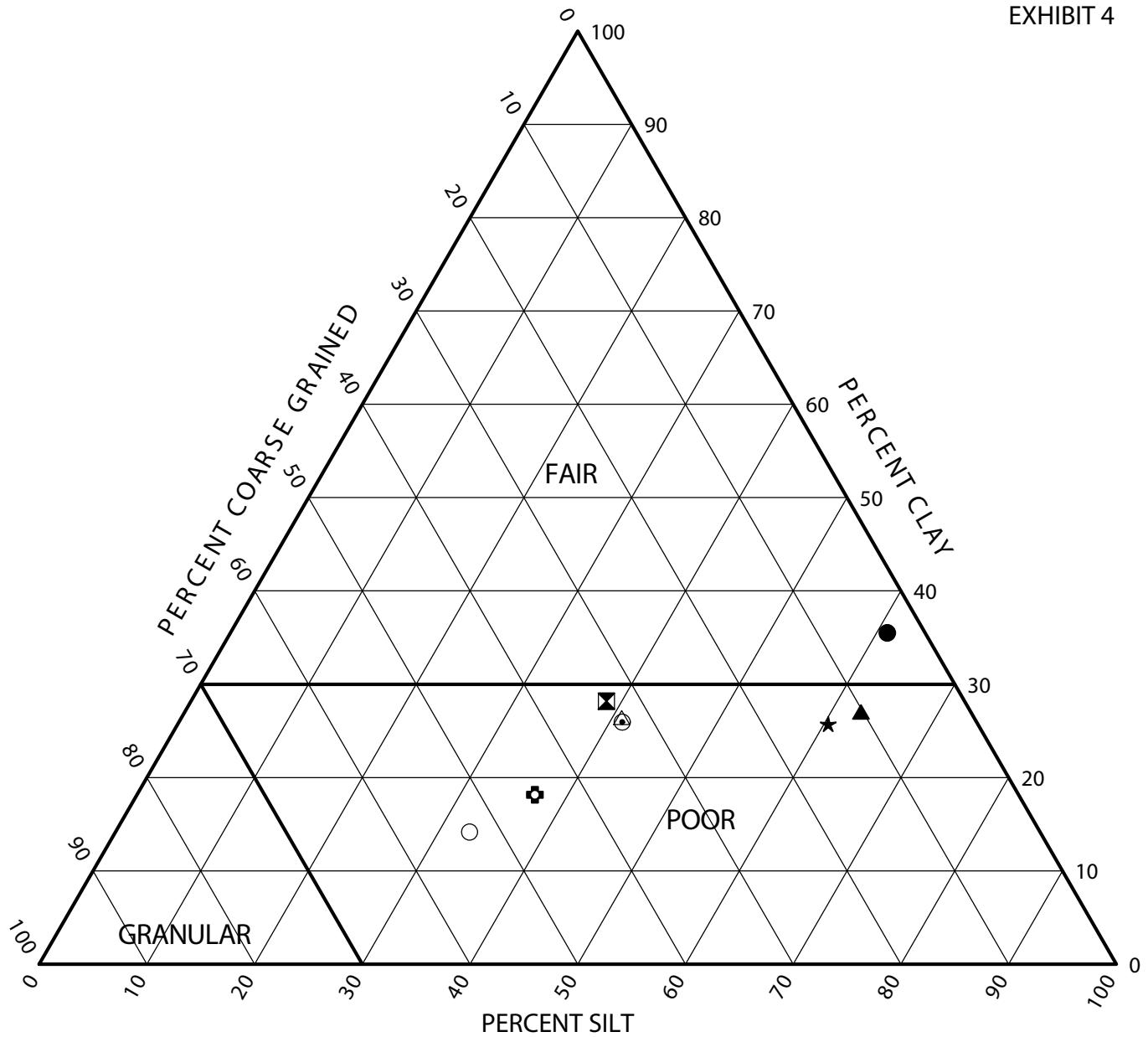
Soil Survey Area: Will County, Illinois

Survey Area Data: Version 16, Aug 31, 2021



| Map unit symbol and soil name | Depth | USDA texture | Classification | Pct Fragments | | Sand | Silt | Clay | Moist bulk density | Saturated hydraulic conductivity | Organic matter | Liquid limit | Plasticity index | Erosion factors | | | Potential as a source of roadfill | Local Roads and Streets | Shallow Excavations | | | | |
|---|-------|--|------------------------|---------------|------------|----------|----------|----------|--------------------|----------------------------------|----------------|--------------|------------------|-----------------|------|---|---|---|--|--|--|--|--|
| | | | | AASHTO | >10 inches | | | | | | | | | Kw | Kf | T | | | | | | | |
| | | | In | L-R-H | L-R-H | Pct | Pct | Pct | g/cc | micro m/sec | Pct | L-R-H | L-R-H | | | | | | | | | | |
| 293A—Andres silt loam, 0 to 2 percent slopes | | | | | | | | | | | | | | | | | | | | | | | |
| Andres | 0-11 | Silt loam | A-6, A-7-5, A-7-6 | 0-0-0 | 0-0-0 | 10-20-30 | 50-56-69 | 20-24-27 | 1.30-1.40-1.50 | 4.23-9.17-14.11 | 3.5-4.2-5.0 | 38-43-48 | 13-16-18 | 0.28 | 0.28 | 5 | Poor; Low Strength, Wetness, Dusty, Shrink-swell | Somewhat limited; Depth to saturated zone, Frost action, Shrink-swell | Very limited; Depth to saturated zone, Dusty, Unstable excavation walls, Ponding, Too clayey | | | | |
| | 11-36 | Clay loam, loam, sandy clay loam, silty | A-6, A-7-6 | 0-0-0 | 0-0-1 | 15-29-50 | 15-40-61 | 24-31-35 | 1.35-1.50-1.60 | 4.23-9.17-14.11 | 0.5-1.1-1.8 | 35-43-48 | 16-22-25 | 0.32 | 0.32 | | | | | | | | |
| | 36-50 | Silty clay loam | A-6, A-7-6 | 0-0-0 | 0-0-3 | 5-10-20 | 45-58-68 | 27-32-35 | 1.45-1.55-1.65 | 1.41-2.82-4.23 | 0.1-0.5-0.8 | 37-43-46 | 19-23-25 | 0.43 | 0.43 | | | | | | | | |
| | 50-60 | Silty clay loam, silt loam | A-6, A-7-6 | 0-0-0 | 0-1-3 | 5-10-20 | 45-61-73 | 22-29-35 | 1.50-1.65-1.70 | 0.42-0.92-1.41 | 0.0-0.2-0.5 | 32-39-46 | 15-20-25 | 0.49 | 0.49 | | | | | | | | |
| 314A—Joliet silt loam, 0 to 2 percent slopes | | | | | | | | | | | | | | | | | | | | | | | |
| Joliet | 0-15 | Silt loam | A-4, A-6 | 0-1-1 | 0-3-5 | 10-20-30 | 50-58-72 | 18-22-27 | 1.15-1.25-1.35 | 4.23-9.17-14.11 | 4.0-4.5-5.0 | 25-33-40 | 7-14-20 | 0.37 | 0.37 | 1 | Poor; Depth to bedrock, Low Strength, Dusty, Shrink-swell | Very limited; Depth to hard bedrock, Frost action, Low strength | Very limited; Depth to hard bedrock, Frost action, Low strength | | | | |
| | 15-19 | Loam, clay loam, silty clay loam | A-6, A-7-6 | 0-1-1 | 0-3-4 | 15-18-50 | 15-52-60 | 23-30-33 | 1.35-1.45-1.55 | 4.23-9.17-14.11 | 0.5-1.3-2.0 | 30-40-50 | 20-28-35 | 0.43 | 0.43 | | | | | | | | |
| | 19-60 | Bedrock | - | - | - | - | - | - | - | 0.42-2.33-4.23 | - | - | - | - | - | | | | | | | | |
| 315B—Channahon silt loam, 2 to 4 percent slopes | | | | | | | | | | | | | | | | | | | | | | | |
| Channahon | 0-11 | Silt loam | A-4, A-6 | 0-0-1 | 0-1-4 | 10-20-30 | 50-58-72 | 18-22-27 | 1.20-1.30-1.40 | 4.23-9.17-14.11 | 2.0-3.0-4.0 | 20-30-40 | 7-14-20 | 0.43 | 0.43 | 1 | Poor; Depth to bedrock, Low Strength, Dusty, Shrink-swell | Very limited; Depth to hard bedrock, Frost action, Low strength | Very limited; Depth to hard bedrock, Frost action, Low strength | | | | |
| | 11-18 | Loam, silt loam, silty clay loam, clay loam | A-6, A-7-6 | 0-1-1 | 0-3-10 | 15-18-50 | 15-52-60 | 25-30-35 | 1.35-1.47-1.60 | 4.23-9.17-14.11 | 0.0-0.7-1.5 | 30-38-45 | 15-20-25 | 0.43 | 0.43 | | | | | | | | |
| | 18-60 | Bedrock | - | - | - | - | - | - | - | 0.42-2.33-4.23 | - | - | - | - | - | | | | | | | | |
| 318D2—Lorenzo loam, 6 to 12 percent slopes, eroded | | | | | | | | | | | | | | | | | | | | | | | |
| Lorenzo | 0-5 | Loam | A-4, A-6 | 0-0-0 | 0-3-5 | 25-32-40 | 33-45-50 | 18-23-27 | 1.25-1.33-1.40 | 4.23-9.17-14.11 | 2.0-2.5-3.0 | 25-33-40 | 10-15-20 | 0.28 | 0.28 | 2 | Good | Somewhat limited; Frost action, Slope | Very limited; Slope, Dusty, Unstable excavation walls | | | | |
| | 5-15 | Loam, clay loam, gravelly sandy clay loam | A-2-4, A-6, A-7-6 | 0-0-0 | 1-2-8 | 30-42-75 | 5-28-50 | 20-30-35 | 1.60-1.65-1.70 | 14.11-28.23-42.34 | 0.0-0.5-1.0 | 30-38-45 | 10-18-25 | 0.20 | 0.20 | | | | | | | | |
| | 15-60 | Stratified gravelly loamy sand to extremely gravelly coarse sand | A-1-a, A-1-b | 0-0-0 | 4-9-13 | 85-92-99 | 0-5-14 | 1-3-5 | 1.60-1.70-1.80 | 141.14-423.42-705.00 | 0.0-0.3-0.5 | 0-8-15 | NP-3-5 | 0.02 | 0.02 | | | | | | | | |
| 369B—Waupecan silt loam, 2 to 4 percent slopes | | | | | | | | | | | | | | | | | | | | | | | |
| Waupecan | 0-11 | Silt loam | A-4, A-6, A-7-6 | 0-0-0 | 0-0-0 | 5-10-15 | 68-69-80 | 15-21-27 | 1.15-1.25-1.35 | 4.23-9.17-14.11 | 3.0-4.0-5.0 | 31-39-47 | 9-14-18 | 0.37 | 0.37 | 4 | Poor, Low strength, Dusty, Shrink-swell | Very limited; Frost action, Low strength, Shrink-swell, Depth to saturated zone | Somewhat limited; Frost action, Low strength, Shrink-swell, Depth to saturated zone | | | | |
| | 11-39 | Silty clay loam, silt loam | A-6, A-7-6 | 0-0-0 | 0-0-0 | 5-10-15 | 50-60-70 | 25-30-35 | 1.30-1.40-1.50 | 4.23-9.17-14.11 | 0.5-0.8-1.0 | 36-42-47 | 17-21-25 | 0.43 | 0.43 | | | | | | | | |
| | 39-45 | Clay loam, sandy clay loam, gravelly loam, gravelly sandy loam | A-2-4, A-2-6, A-4, A-6 | 0-0-0 | 1-4-9 | 35-50-75 | 5-32-50 | 10-18-30 | 1.55-1.65-1.75 | 4.23-23.29-42.34 | 0.2-0.3-0.5 | 21-29-40 | 6-12-21 | 0.15 | 0.28 | | | | | | | | |
| | 45-60 | Stratified gravelly loamy sand to extremely gravelly coarse sand | A-1-a, A-1-b | 0-2-3 | 4-12-36 | 80-92-99 | 0-3-20 | 0-5-10 | 1.60-1.70-1.80 | 141.14-423.42-705.00 | 0.0-0.3-0.5 | 0-17-23 | NP-2-6 | 0.02 | 0.02 | | | | | | | | |
| 802B—Orthents, loamy, undulating | | | | | | | | | | | | | | | | | | | | | | | |
| Orthents, loamy, undulating | 0-7 | Loam | A-6, A-7-6 | 0-0-0 | 0-2-4 | 23-40-50 | 28-40-50 | 22-25-27 | 1.70-1.73-1.75 | 1.41-2.82-4.23 | 0.5-1.3-2.0 | 32-37-41 | 15-17-19 | 0.37 | 0.37 | 5 | Poor; Low strength, Shrink-swell, Dusty | Somewhat limited; Frost action, Low strength, Shrink-swell | Somewhat limited; Depth to saturated zone, Dusty, Unstable excavation walls, Too clayey | | | | |
| | 7-60 | Loam, silt loam, clay loam | A-6, A-7-6 | 0-1-1 | 0-2-4 | 20-38-50 | 25-35-58 | 22-28-30 | 1.70-1.75-1.80 | 1.41-2.82-4.23 | 0.2-0.6-1.0 | 33-39-43 | 15-19-21 | 0.32 | 0.32 | | | | | | | | |
| Source: USDA, Natural Resources Conservation Service; Web Soil Survey Soil Survey Area: Will County, Illinois Survey Area Data: Version 16, Aug 31, 2021 | | | | | | | | | | | | | | | | | | | | | | | |
| SITE PEDOLOGICAL MAP: I-80 RECONSTRUCTION; WEST MAINLINE FROM STATION 0410+00 TO 0518+00; WILL COUNTY, ILLINOIS SCALE: GRAPHICAL DRAWN BY: J. Bensen EXHIBIT 2-3 CHECKED BY: C. Marin | | | | | | | | | | | | | | | | | | | | | | | |
|  Wang Engineering FOR TRANSYSTEMS CORPORATION 7901-15-01 | | | | | | | | | | | | | | | | | | | | | | | |





| | Sample | Depth (ft) | Coarse (%) | Silt (%) | Clay (%) | Classification | | |
|--------------|--------|------------|------------|----------|----------|-----------------|------------|--------|
| | | | | | | IL DOT | AASHTO | RATING |
| ●CL-SGB-04#3 | 4.0 | 3.5 | 61.0 | 35.5 | | Silty Clay | A-7-6 (31) | FAIR |
| ☒CL-SGB-08#1 | 0.0 | 33.3 | 38.6 | 28.2 | | Clay | A-6 (14) | POOR |
| ▲CL-SGB-12#2 | 2.0 | 10.2 | 62.8 | 27.0 | | Silty Clay Loam | A-7-6 (36) | POOR |
| ★CL-SGB-14#2 | 2.0 | 13.9 | 60.4 | 25.7 | | Silty Clay Loam | A-7-6 (24) | POOR |
| ○EB-SGB-10#2 | 3.0 | 32.9 | 41.2 | 25.9 | | Clay Loam | A-6 (9) | POOR |
| ☒EB-SGB-17#2 | 3.0 | 44.9 | 36.9 | 18.2 | | Loam | A-6 (4) | POOR |
| ○WB-SGB-06#4 | 7.0 | 53.0 | 32.9 | 14.2 | | Gravelly Loam | A-6 (4) | POOR |
| △WB-SGB-14#3 | 5.0 | 32.7 | 40.9 | 26.4 | | Clay Loam | A-6 (14) | POOR |
| | | | | | | | | |
| | | | | | | | | |



Wang Engineering Inc.
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Subgrade Support Rating Chart

Project: I-80 Reconstruction (Houbolt Road to Center Street)
Location: Will County, Illinois
Number: 7901-15-01



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APPENDIX A



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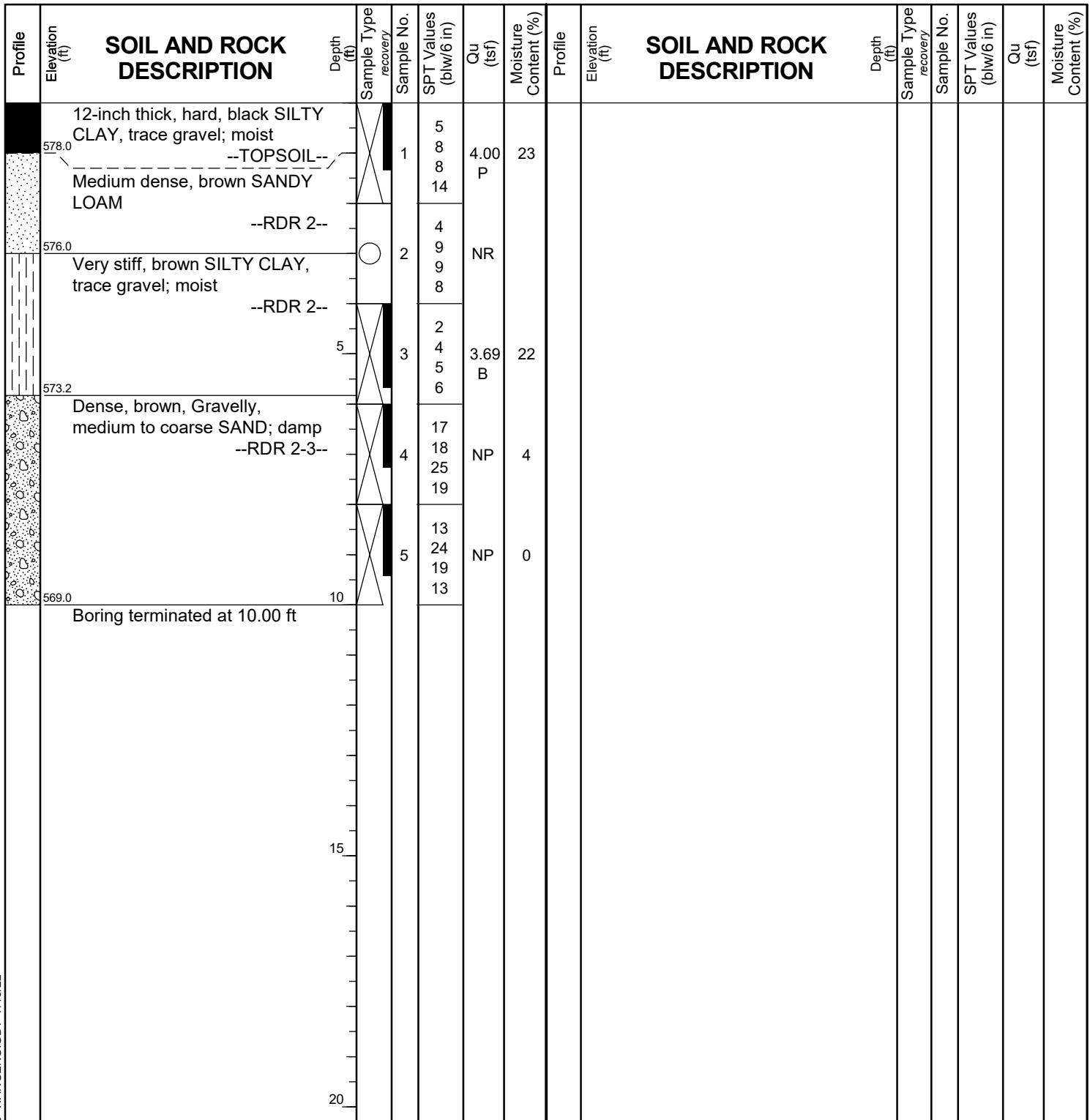
BORING LOG CL-SGB-01

WEI Job No.: 7901-15-01

TranSystems Corporation

Client **TranSystems Corporation**
Project **I-80 Reconstruction (Houbolt Road to Center Street)**
Location **Will County, Illinois**

Datum: NAVD 88
Elevation: 579.00 ft
North: 1755956.03 ft
East: 1027515.02 ft
Station: 417+60.10
Offset: 5.92 RT



WANGGENGINC 790011501.GPJ WANGGENG.GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-23-2022** Complete Drilling **05-23-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **21GeoA[96%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling | NA | |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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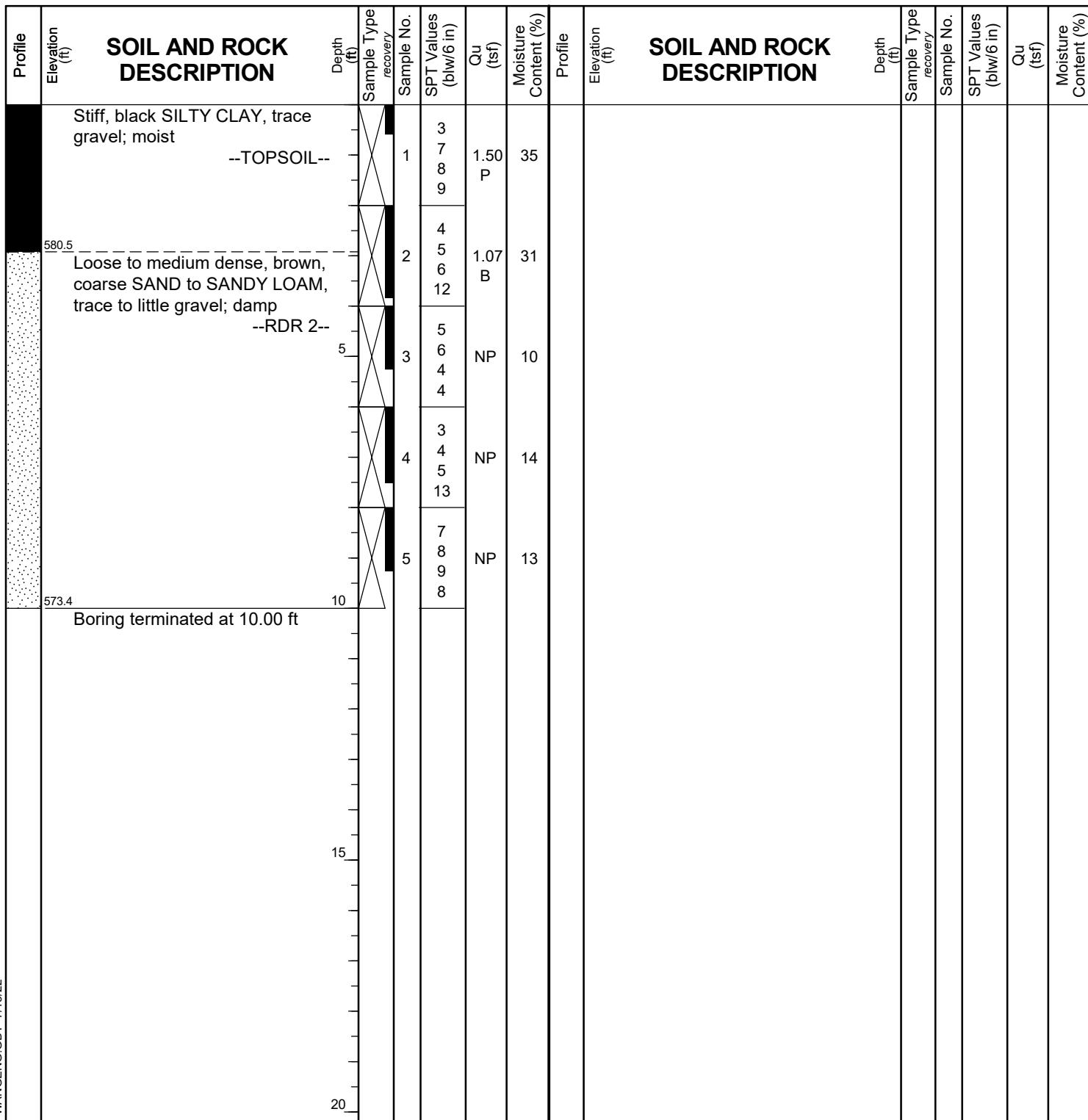
BORING LOG CL-SGB-02

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 583.38 ft
North: 1756194.40 ft
East: 1028062.43 ft
Station: 423+56.39
Offset: 13.12 RT



GENERAL NOTES

Begin Drilling **05-23-2022** Complete Drilling **05-23-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **21GeoA[96%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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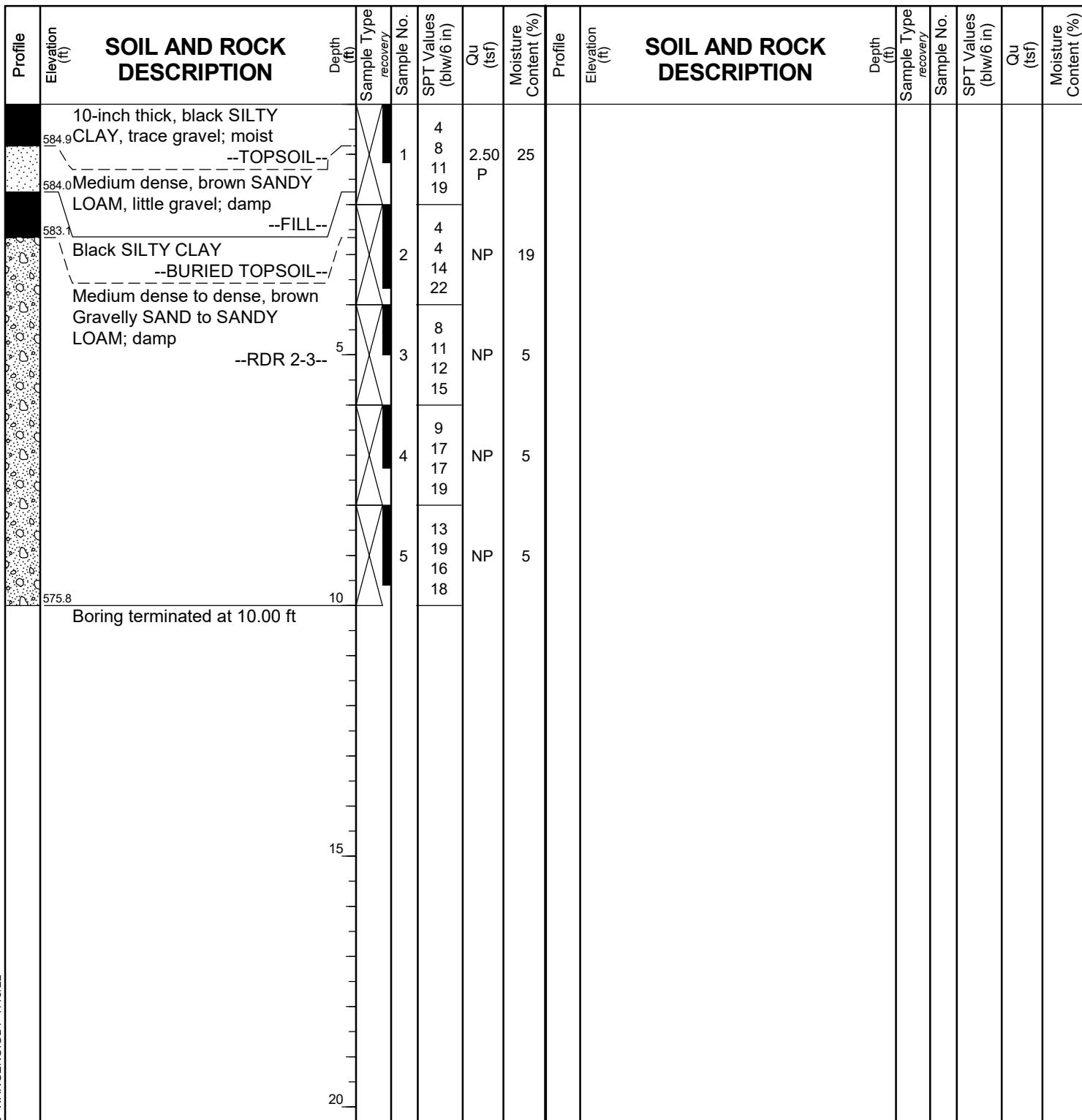
BORING LOG CL-SGB-03

WEI Job No.: 7901-15-01

TranSystems Corporation

Client **TranSystems Corporation**
Project **I-80 Reconstruction (Houbolt Road to Center Street)**
Location **Will County, Illinois**

Datum: NAVD 88
Elevation: 585.77 ft
North: 1756502.34 ft
East: 1028581.11 ft
Station: 429+58.75
Offset: 7.80 RT



WANGENGINC 79011501.GPJ WANGENG.GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-23-2022** Complete Drilling **05-23-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **21GeoA[96%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**



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BORING LOG CL-SGB-04

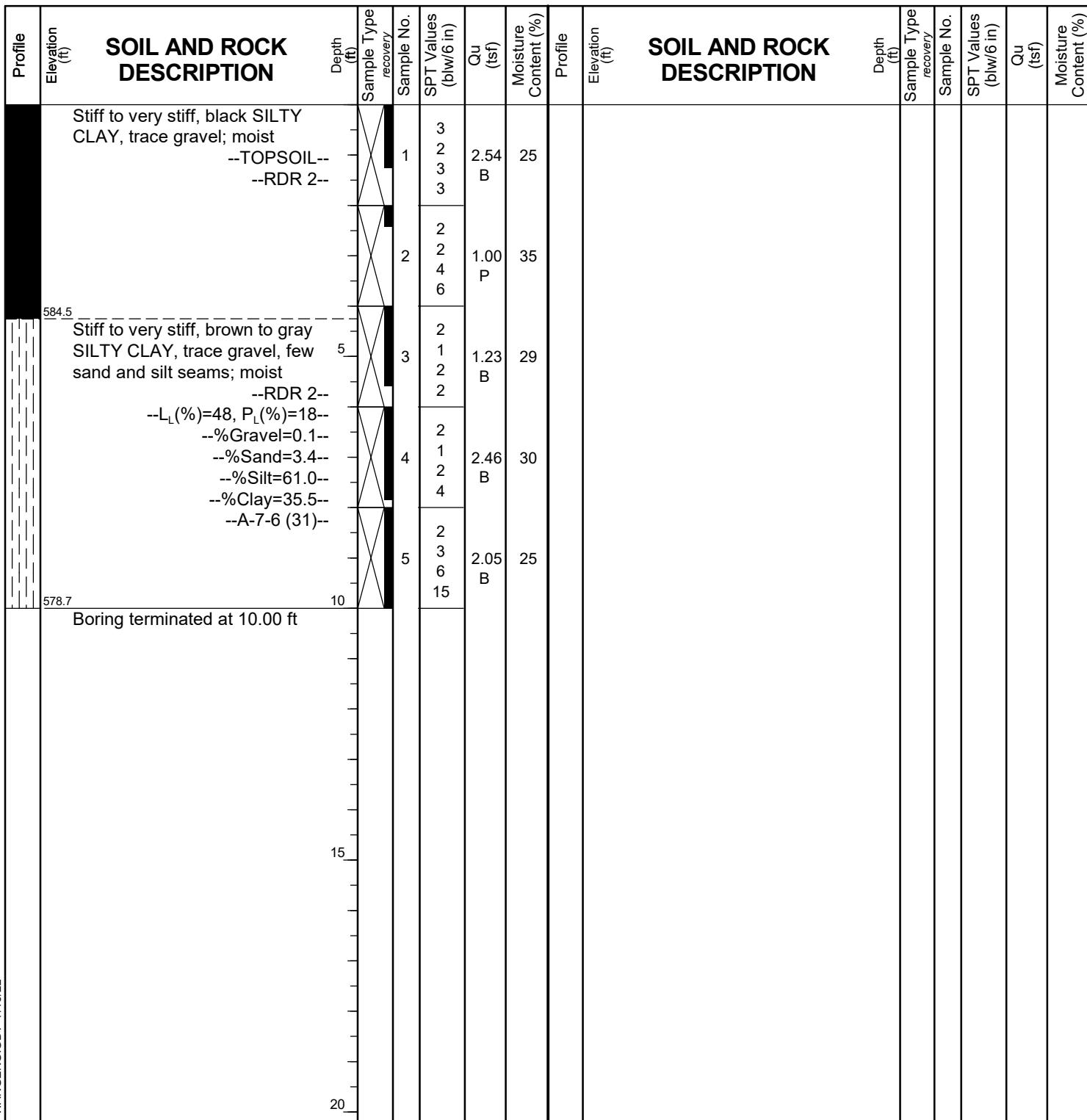
WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Page 1 of 1

Datum: NAVD 88
Elevation: 588.71 ft
North: 1756863.64 ft
East: 1029070.12 ft
Station: 435+66.34
Offset: 5.27 RT



GENERAL NOTES

Begin Drilling **05-23-2022** Complete Drilling **05-23-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **21GeoA[96%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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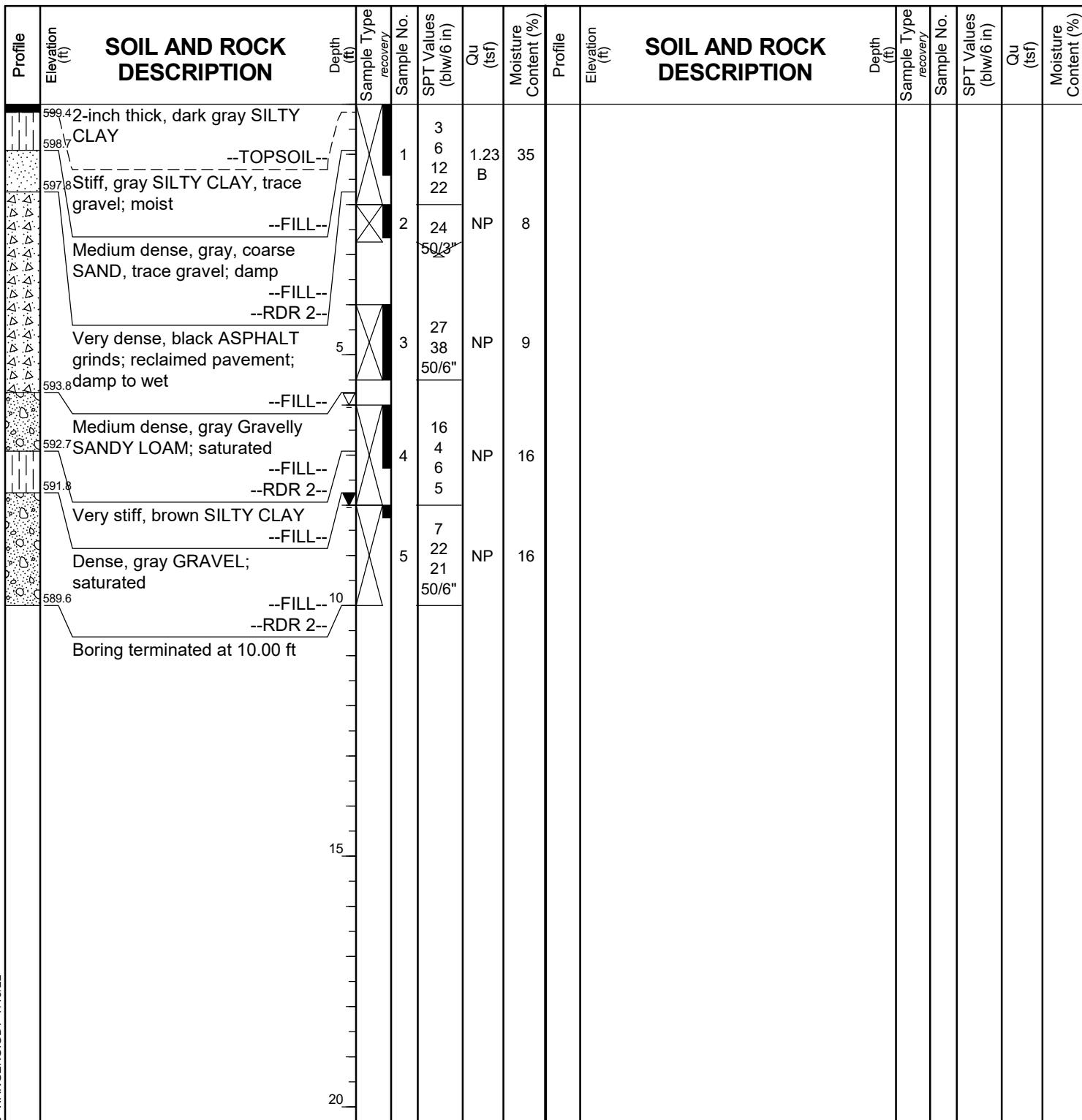
BORING LOG CL-SGB-05

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 599.60 ft
North: 1757263.33 ft
East: 1029506.17 ft
Station: 441+57.74
Offset: 2.02 RT



GENERAL NOTES

Begin Drilling 05-18-2022 Complete Drilling 05-18-2022
Drilling Contractor Wang Testing Services Drill Rig D25 ATV [93%]
Driller KG&TC Logger A. Scifers Checked by J. Bensen
Drilling Method 2.25" IDA HSA; boring backfilled upon completion

WATER LEVEL DATA

While Drilling □ 6.00 ft
At Completion of Drilling □ 8.00 ft
Time After Drilling NA
Depth to Water □ NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG CL-SGB-06

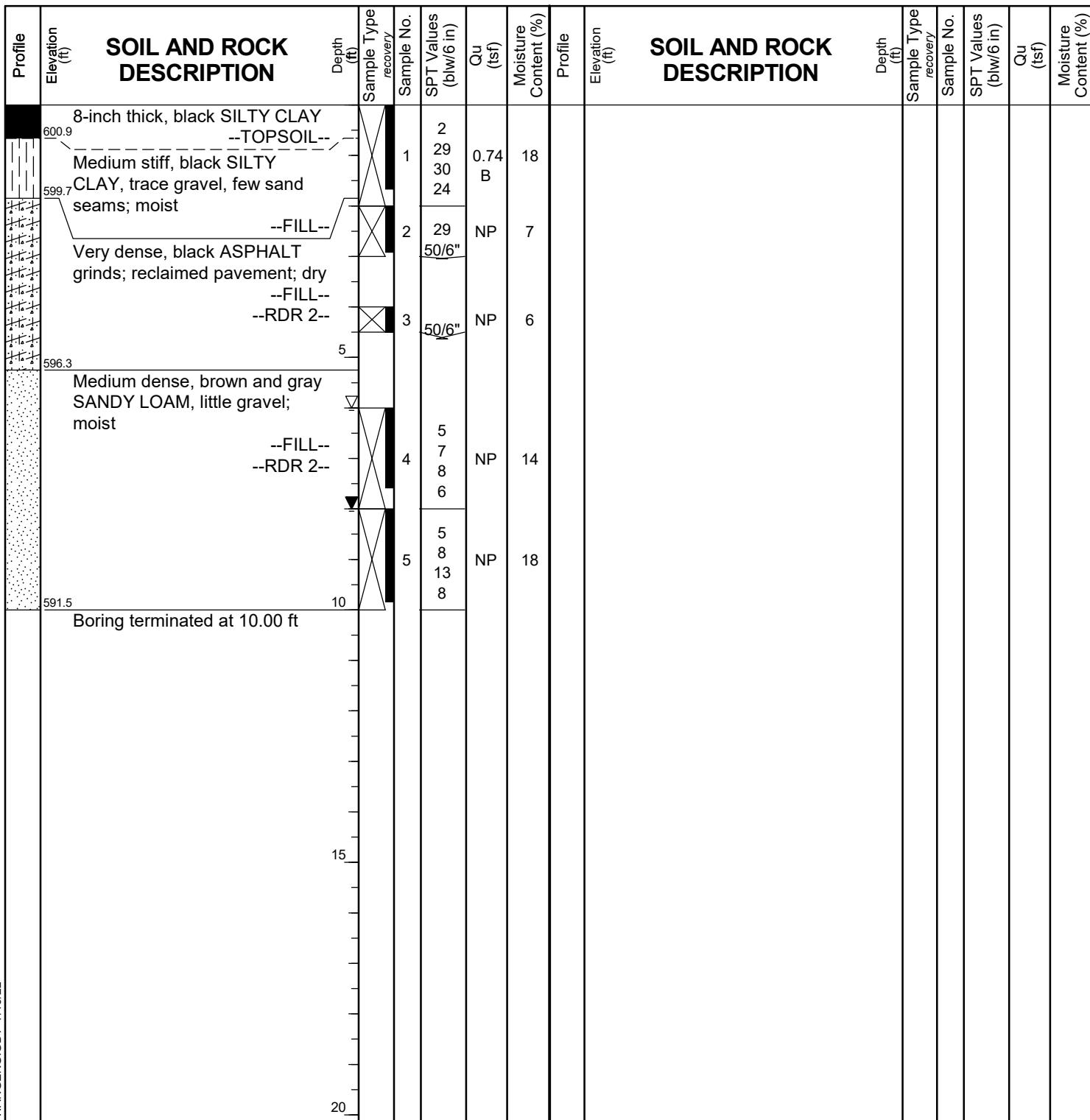
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 601.53 ft
North: 1757703.08 ft
East: 1029914.64 ft
Station: 447+57.88
Offset: 3.48 RT



GENERAL NOTES

Begin Drilling 05-18-2022 Complete Drilling 05-18-2022
Drilling Contractor Wang Testing Services Drill Rig D25 ATV [93%]
Driller KG&TC Logger A. Scifers Checked by J. Bensen
Drilling Method 2.25" IDA HSA; boring backfilled upon completion

WATER LEVEL DATA

While Drilling ▽ 6.00 ft
At Completion of Drilling ▽ 8.00 ft
Time After Drilling NA
Depth to Water ▽ NA
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-07

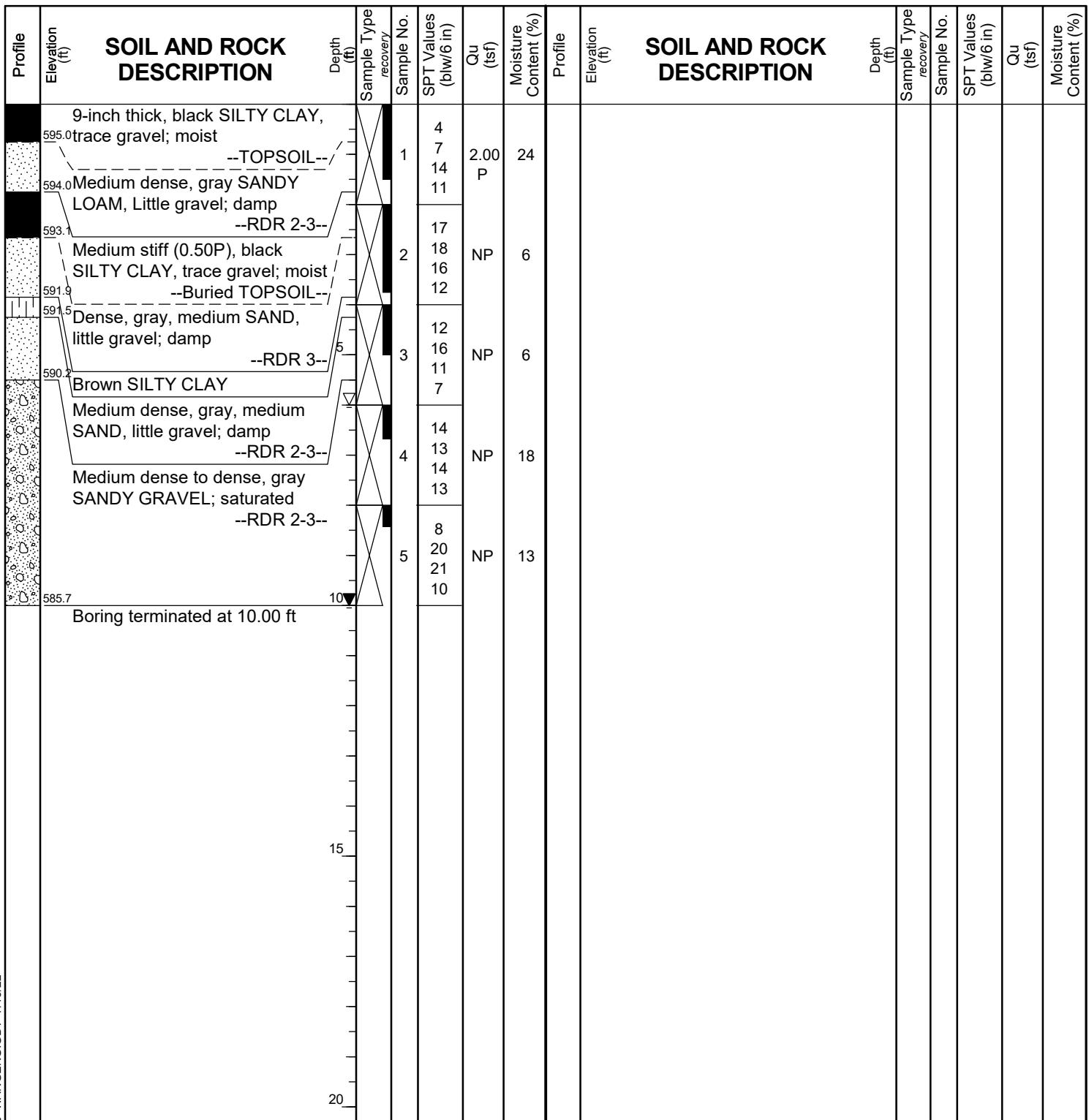
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client TranSystems Corporation
Project I-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 595.75 ft
North: 1758162.09 ft
East: 1030328.09 ft
Station: 453+75.64
Offset: 2.28 RT



WANGENG INC 79011501 GP. I WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-18-2022** Complete Drilling **05-18-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **D25 ATV [93%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|----|----------|
| While Drilling | ▼ | 6.00 ft |
| At Completion of Drilling | ▼ | 10.00 ft |
| Time After Drilling | NA | |
| Depth to Water | ▼ | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-08

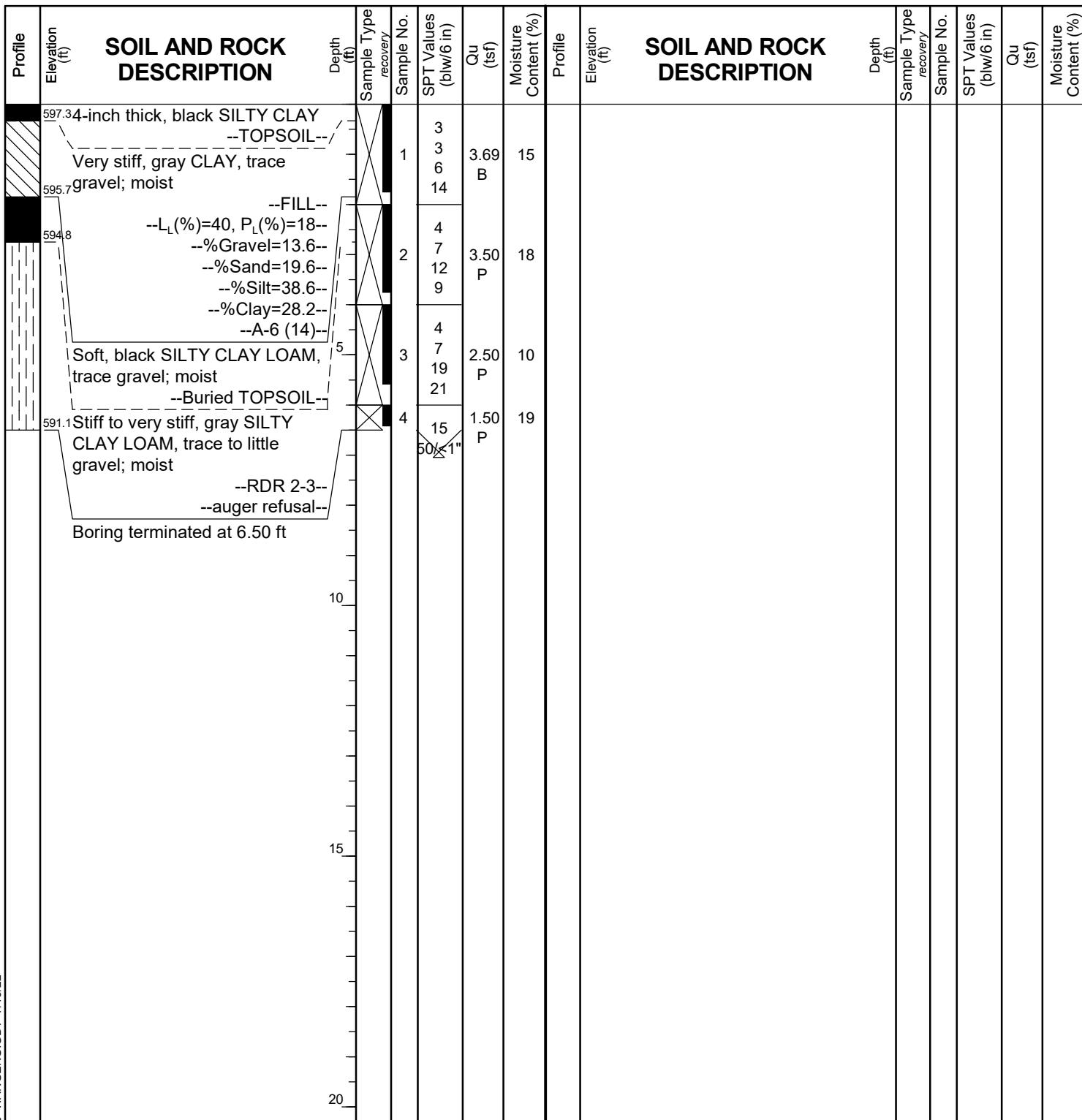
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 597.59 ft
North: 1758593.37 ft
East: 1030719.56 ft
Station: 459+58.10
Offset: 3.37 RT



GENERAL NOTES

Begin Drilling **05-18-2022** Complete Drilling **05-18-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **D25 ATV [93%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-09

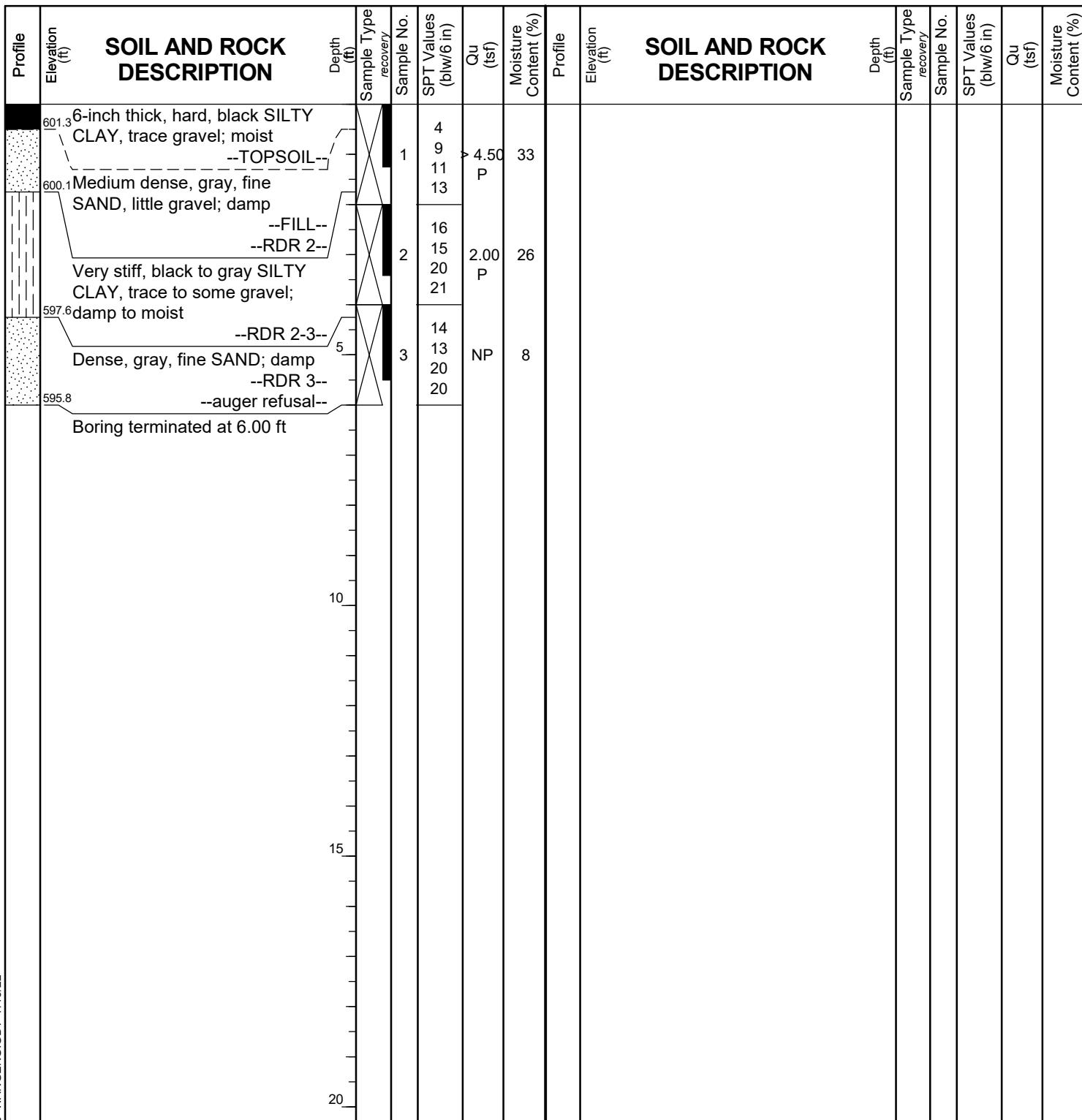
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client TranSystems Corporation
Project I-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 601.83 ft
North: 1759045.31 ft
East: 1031120.12 ft
Station: 465+61.97
Offset: 2.66 LT



WANGENGINC 79011501.GPJ WANGENG.GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-18-2022** Complete Drilling **05-18-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **D25 ATV [93%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|-------------------------------------|-----|
| While Drilling | <input checked="" type="checkbox"/> | DRY |
| At Completion of Drilling | <input checked="" type="checkbox"/> | DRY |
| Time After Drilling | <input type="checkbox"/> | NA |
| Depth to Water | <input checked="" type="checkbox"/> | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-10

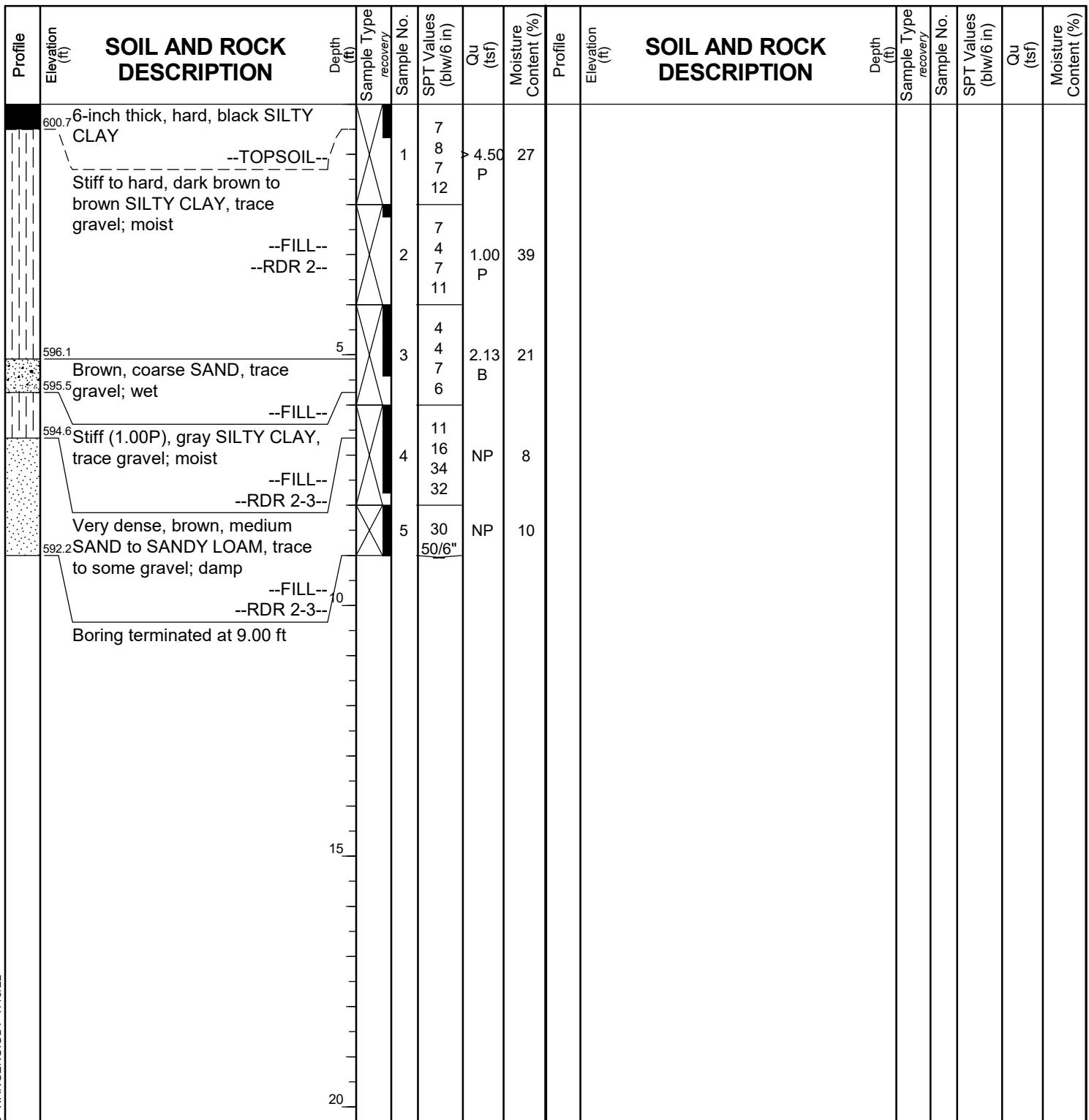
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 601.22 ft
North: 1759491.02 ft
East: 1031529.79 ft
Station: 471+67.33
Offset: 2.26 RT



GENERAL NOTES

Begin Drilling **05-18-2022** Complete Drilling **05-18-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **D25 ATV [93%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-11

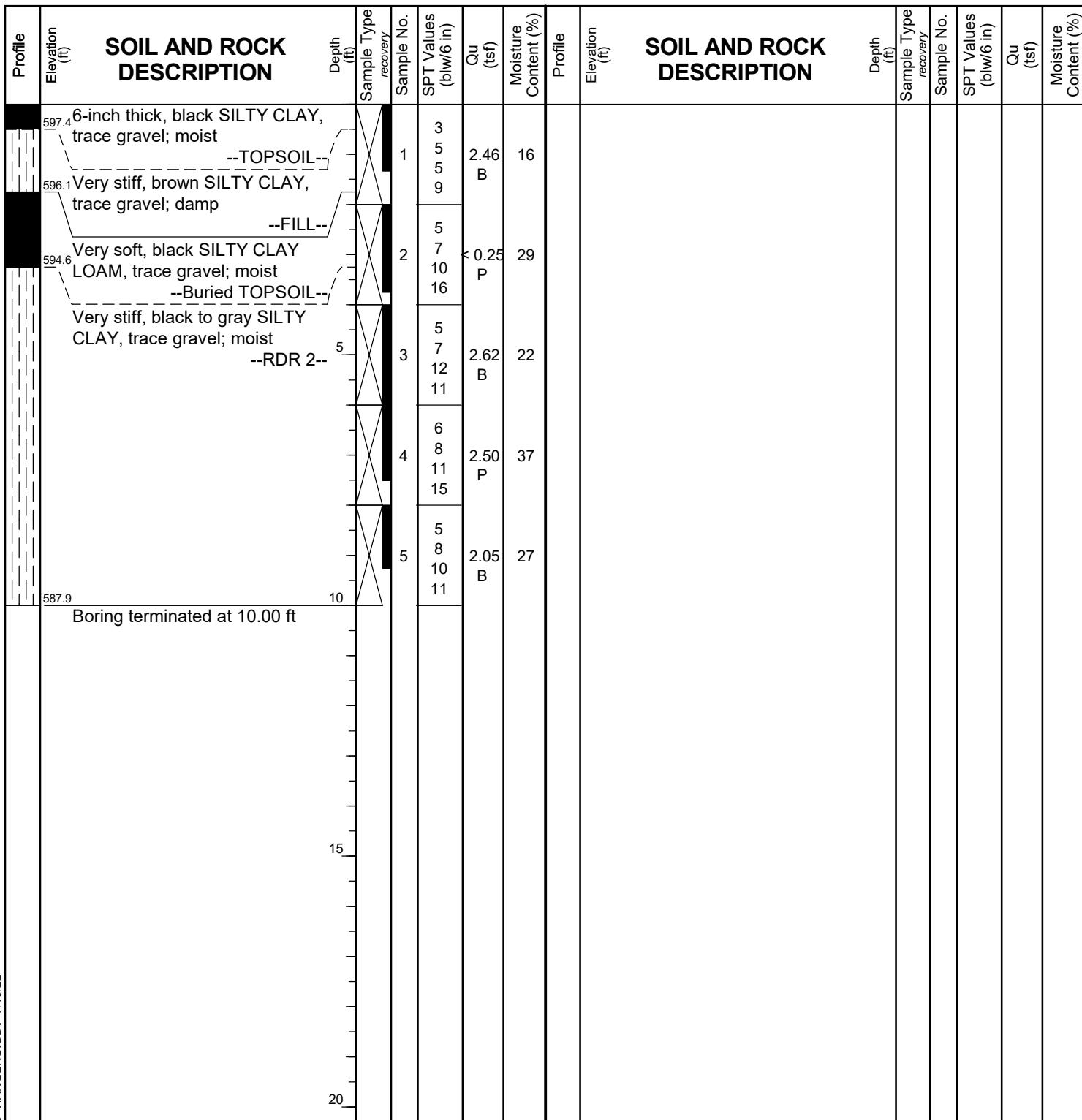
WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Page 1 of 1

Datum: NAVD 88
Elevation: 597.87 ft
North: 1759930.34 ft
East: 1031932.27 ft
Station: 477+63.98
Offset: 7.85 RT



GENERAL NOTES

WATER LEVEL DATA

Begin Drilling 05-18-2022 Complete Drilling 05-18-2022
Drilling Contractor Wang Testing Services Drill Rig D25 ATV [93%]
Driller KG&TC Logger A. Scifers Checked by J. Bensen
Drilling Method 2.25" IDA HSA; boring backfilled upon completion

While Drilling □ DRY
At Completion of Drilling □ DRY
Time After Drilling NA
Depth to Water □ NA
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-12

WEI Job No.: 7901-15-01

TranSystems Corporation

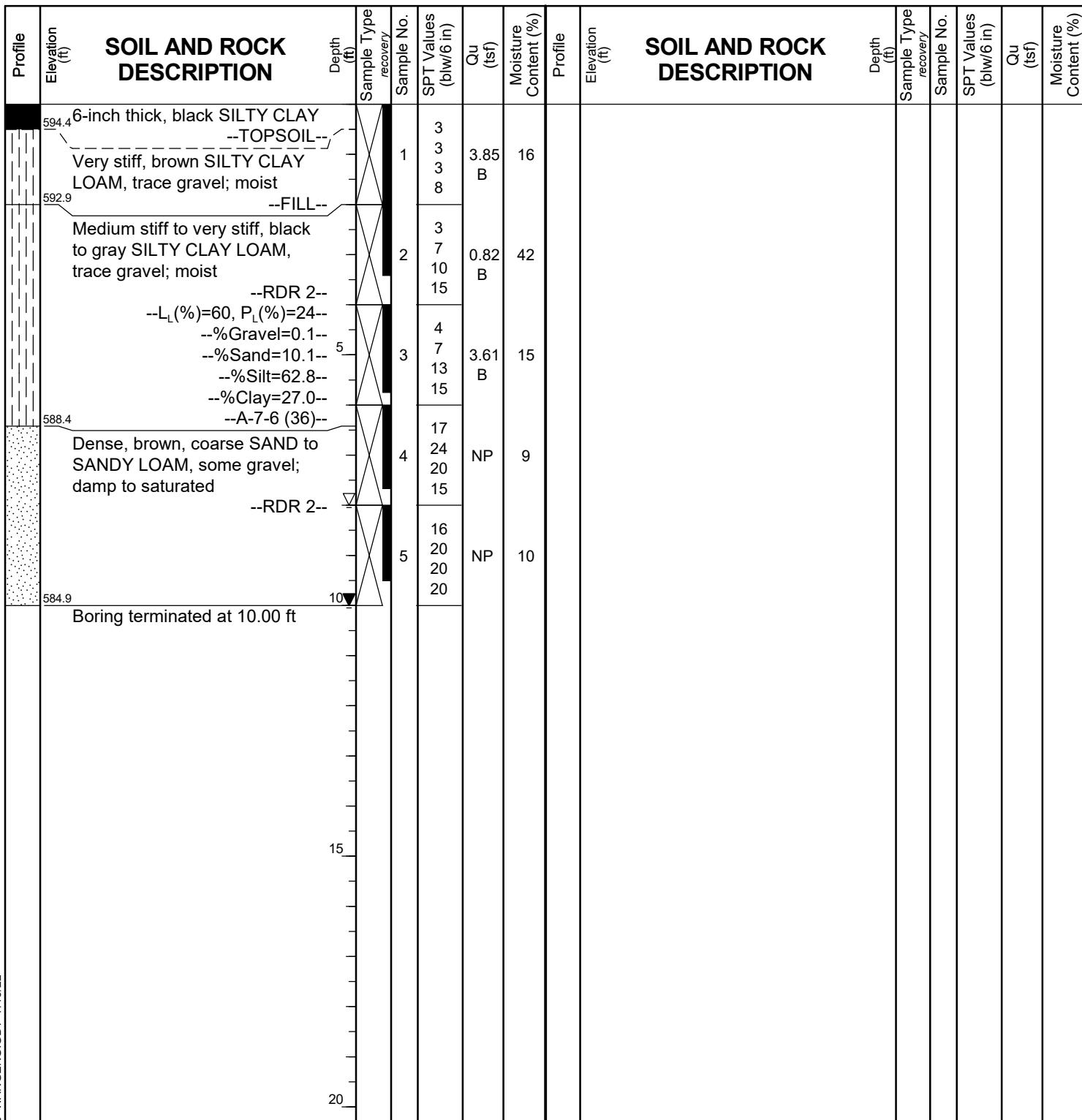
Client

Project-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 594.87 ft
North: 1760386.09 ft
East: 1032333.67 ft
Station: 483+70.39
Offset: 1.84 LT



GENERAL NOTES

Begin Drilling **05-18-2022** Complete Drilling **05-18-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **D25 ATV [93%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|--|----------|
| While Drilling | | 8.00 ft |
| At Completion of Drilling | | 10.00 ft |
| Time After Drilling | | NA |
| Depth to Water | | NA |

The stratification lines represent the approximate boundary between the two different water depths.



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BORING LOG CL-SGB-13

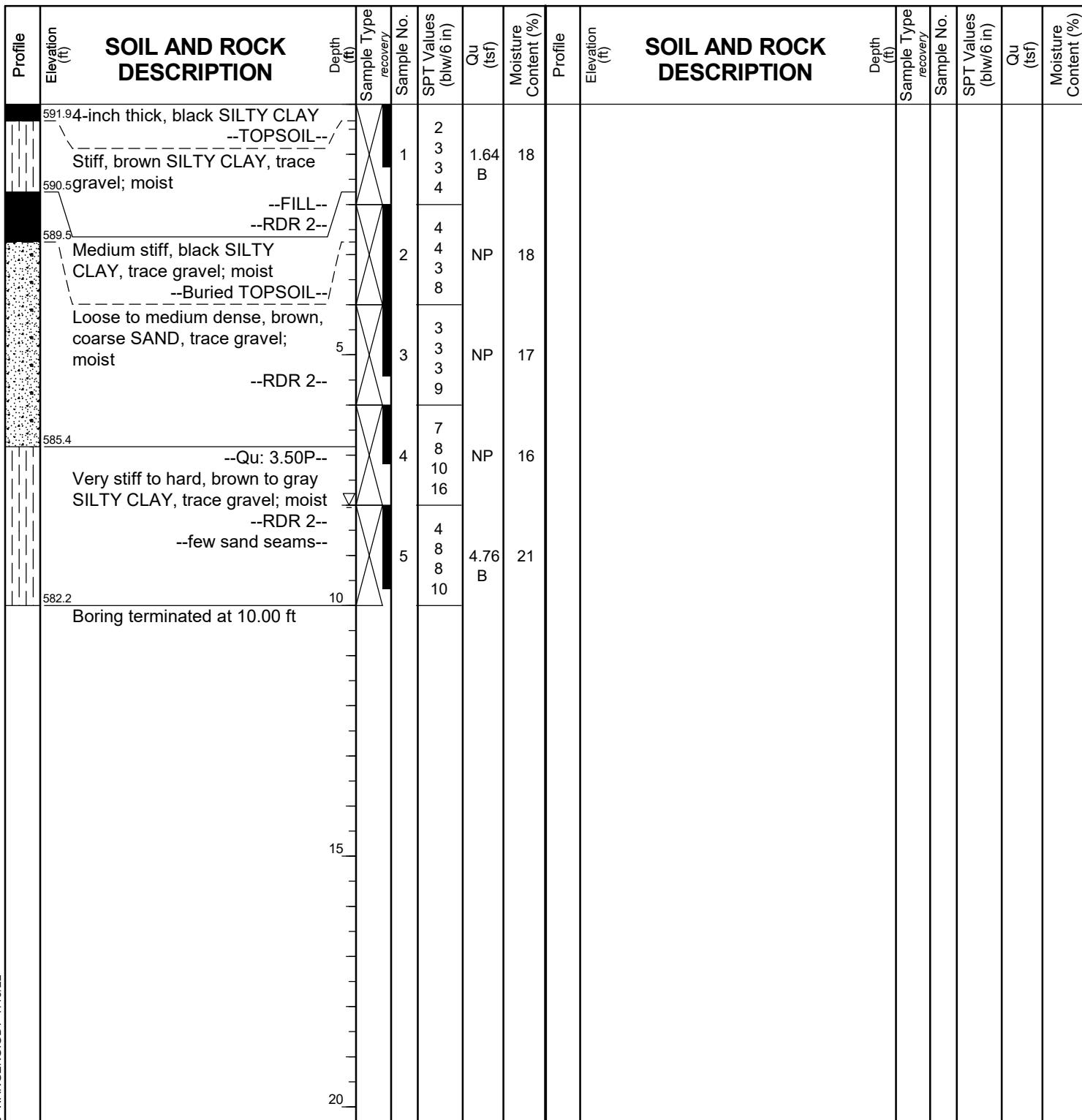
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 592.23 ft
North: 1760821.60 ft
East: 1032737.78 ft
Station: 489+64.45
Offset: 5.78 RT



GENERAL NOTES

Begin Drilling **05-18-2022** Complete Drilling **05-18-2022**
 Drilling Contractor **Wang Testing Services** Drill Rig **D25 ATV [93%]**
 Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
 Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **▽ 8.00 ft**
 At Completion of Drilling **▽ DRY**
 Time After Drilling **NA**
 Depth to Water **▽ NA**
 The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-14

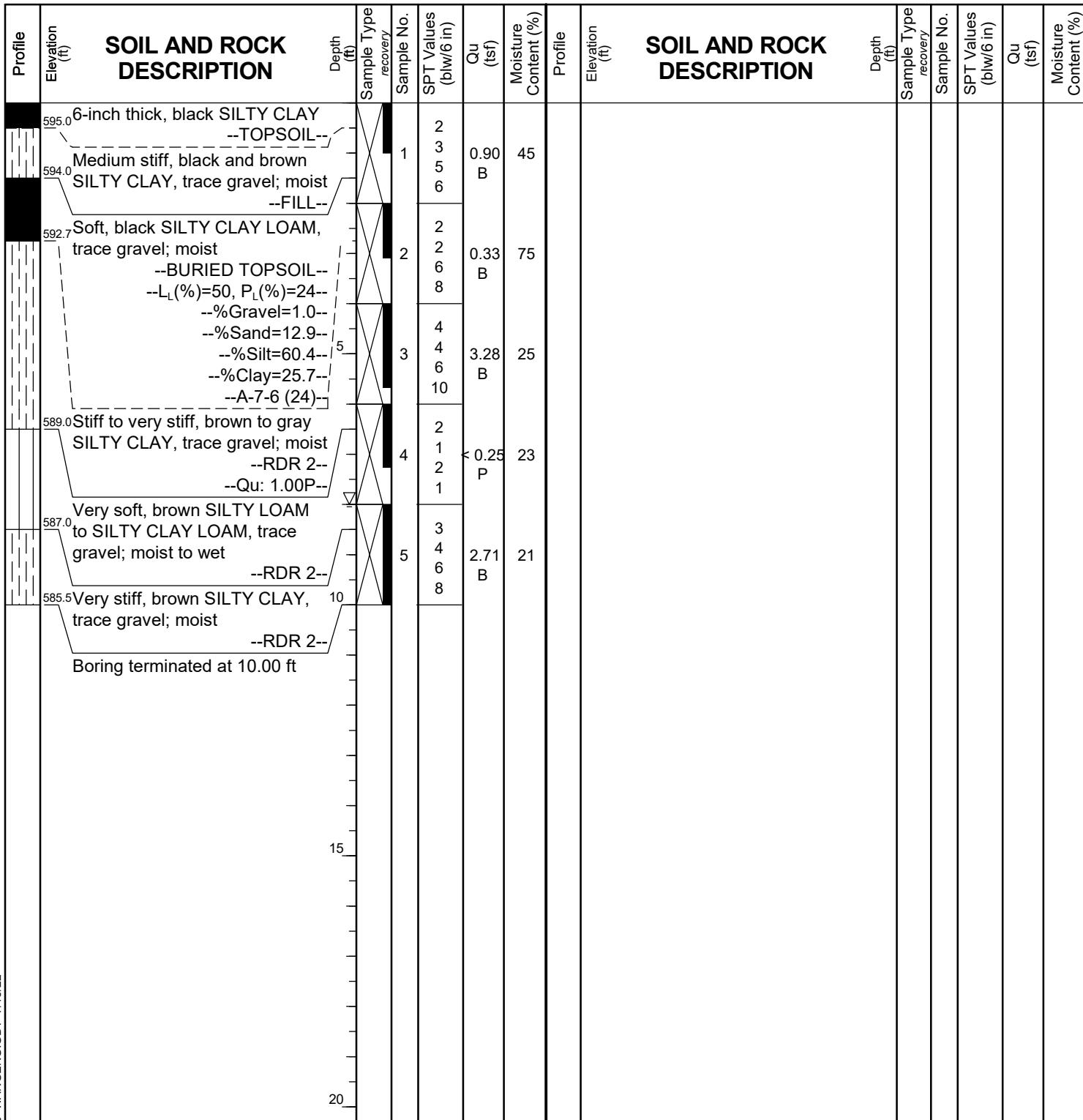
WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

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Datum: NAVD 88
Elevation: 595.49 ft
North: 1761709.04 ft
East: 1033550.92 ft
Station: 501+68.17
Offset: 13.47 RT



GENERAL NOTES

Begin Drilling 05-22-2022 Complete Drilling 05-22-2022
Drilling Contractor Wang Testing Services Drill Rig 21GeoA[96%]
Driller KG&TC Logger A. Scifers Checked by J. Bensen
Drilling Method 2.25" IDA HSA; boring backfilled upon completion

WATER LEVEL DATA

While Drilling ▽ 8.00 ft
At Completion of Drilling ▽ DRY
Time After Drilling NA
Depth to Water ▽ NA
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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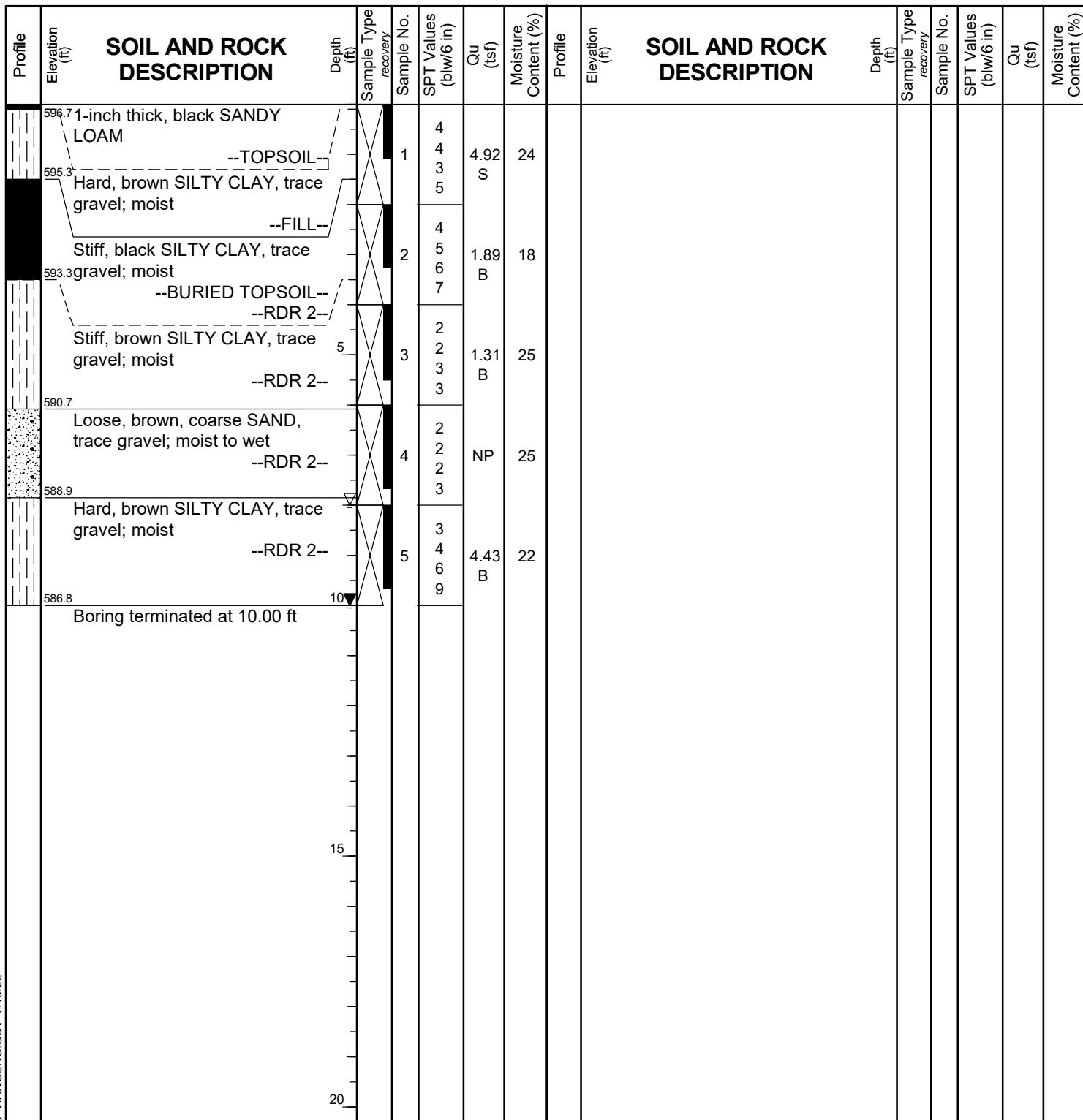
BORING LOG CL-SGB-15

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 596.77 ft
North: 1762126.17 ft
East: 1033959.57 ft
Station: 507+53.48
Offset: 14.70 RT



GENERAL NOTES

Begin Drilling **05-22-2022** Complete Drilling **05-22-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **21GeoA[96%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **▽ 8.00 ft**
At Completion of Drilling **▽ 10.00 ft**
Time After Drilling **NA**
Depth to Water **▽ NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG CL-SGB-16

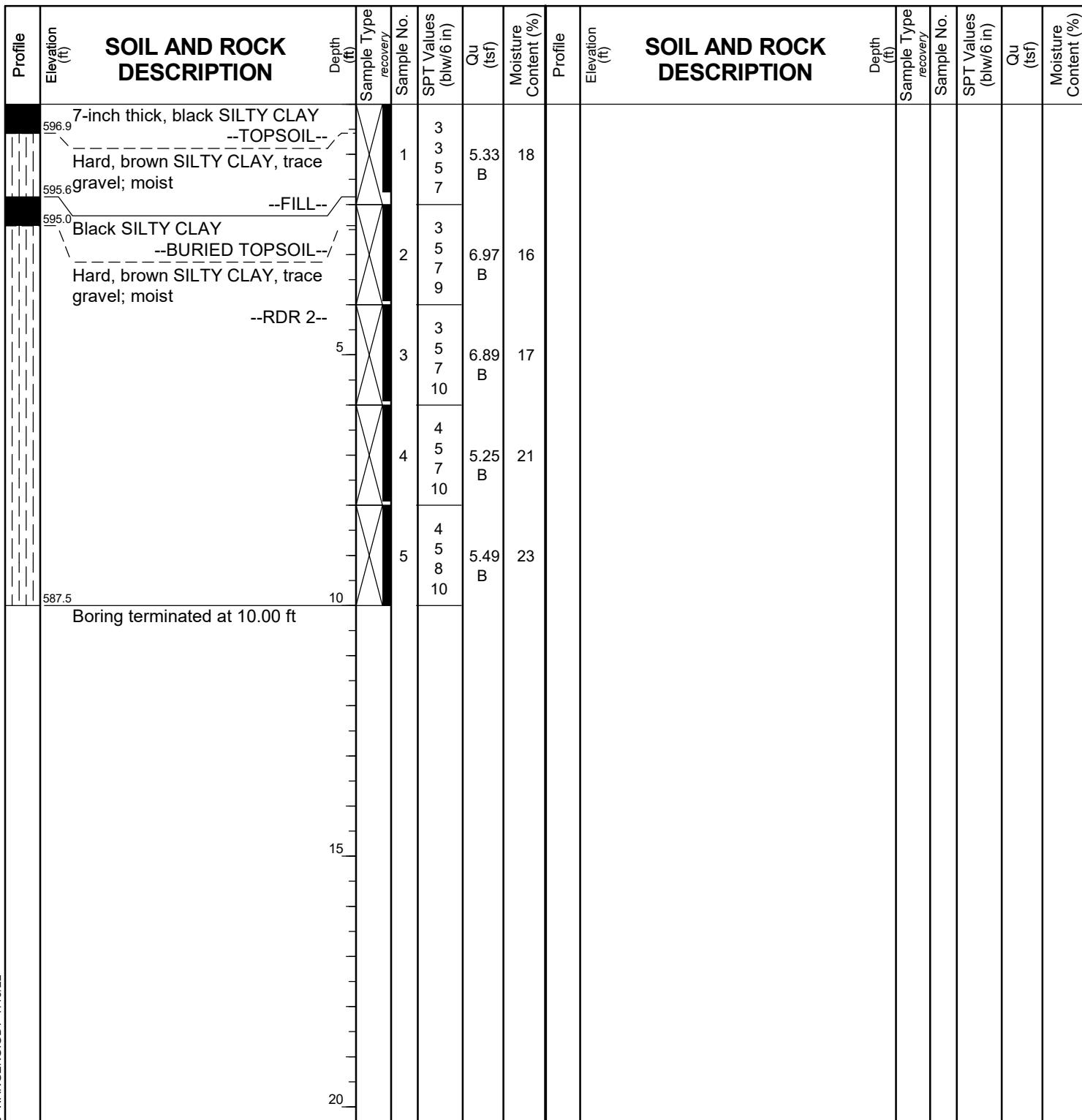
WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Page 1 of 1

Datum: NAVD 88
Elevation: 597.46 ft
North: 1762537.85 ft
East: 1034411.62 ft
Station: 513+65.23
Offset: 4.63 RT



GENERAL NOTES

Begin Drilling **05-22-2022** Complete Drilling **05-22-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **21GeoA[96%]**
Driller **KG&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-01

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WEI Job No.: 7901-15-01

TranSystems Corporation

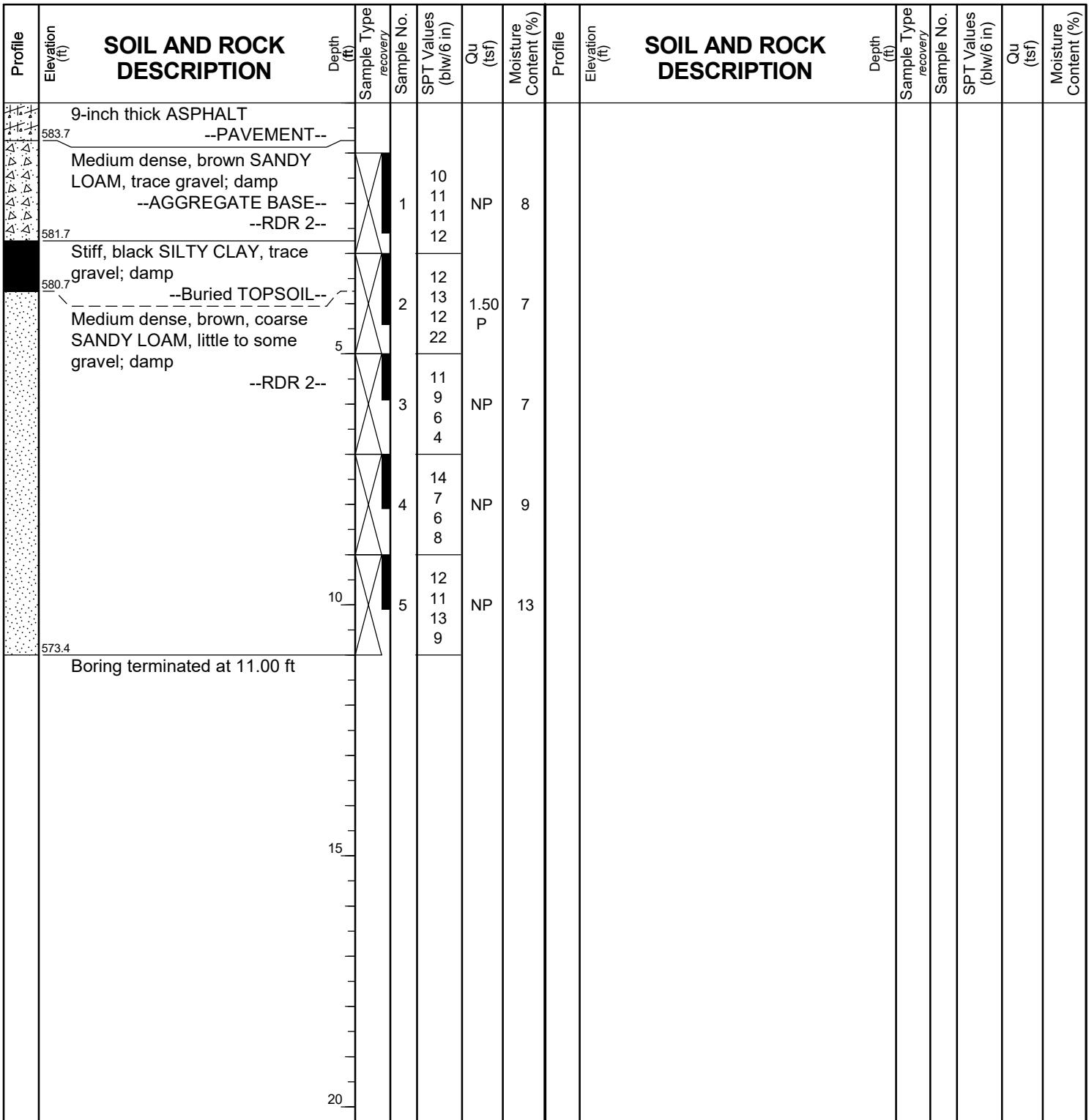
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 584.41 ft
North: 1755985.77 ft
East: 1027752.01 ft
Station: 419+89.08
Offset: 68.71 RT



WANGENG INC 79011501 GBP! WANGEENG GDT 7/13/22

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**



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BORING LOG EB-SGB-02

WEI Job No.: 7901-15-01

TranSystems Corporation

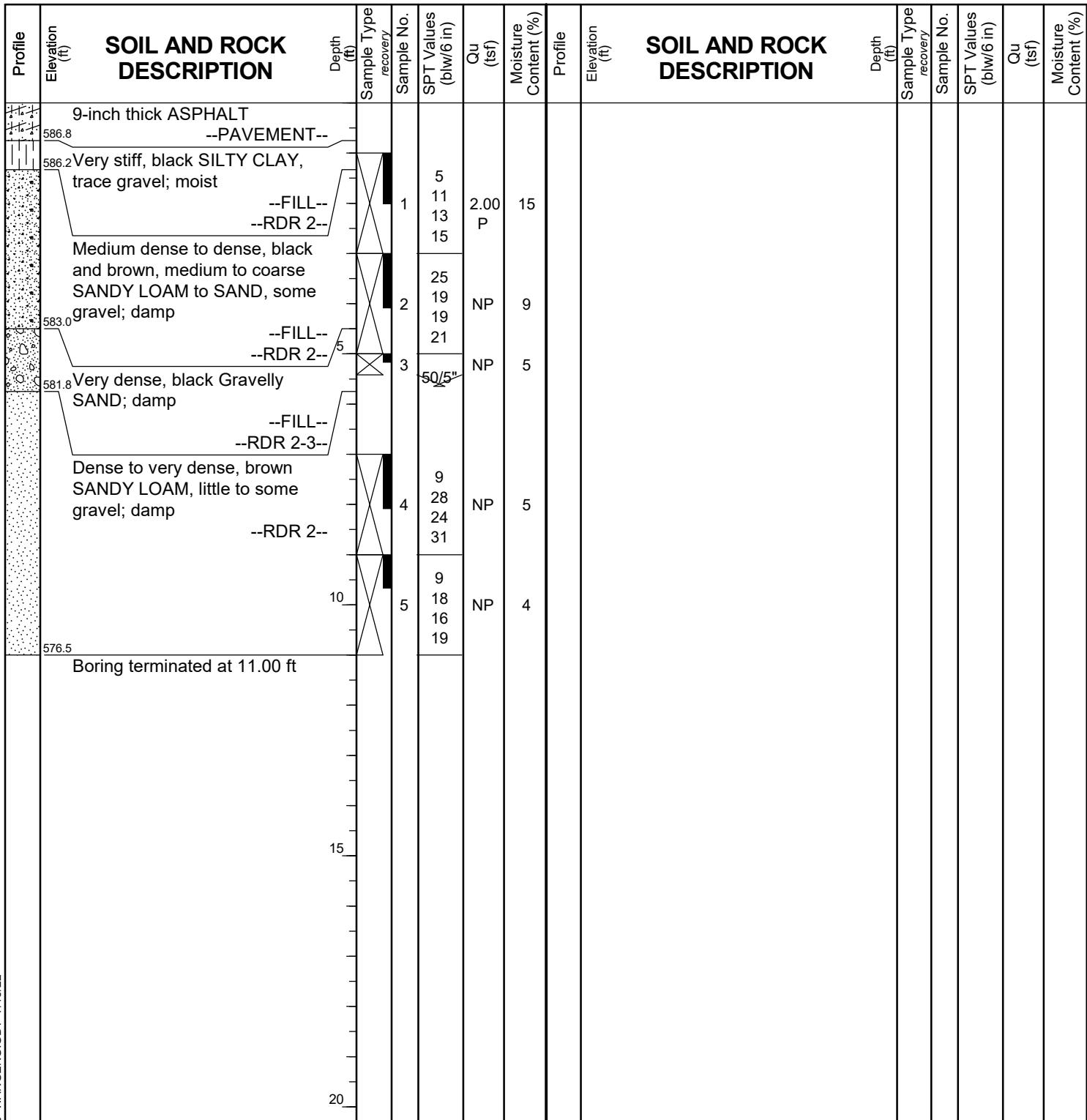
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 587.52 ft
North: 1756253.41 ft
East: 1028287.77 ft
Station: 425+81.19
Offset: 65.05 RT



GENERAL NOTES

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

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BORING LOG EB-SGB-03

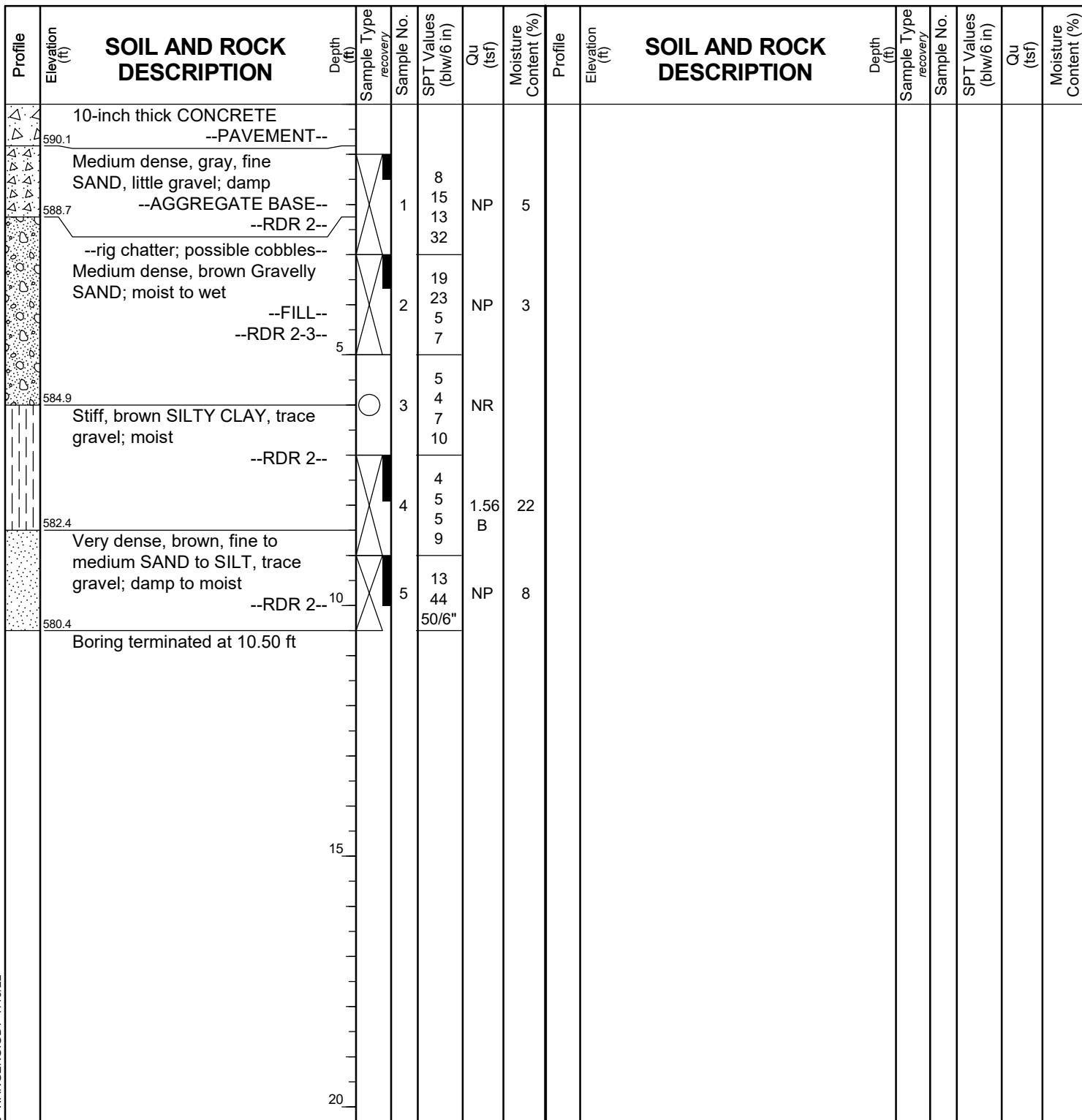
WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Page 1 of 1

Datum: NAVD 88
Elevation: 590.91 ft
North: 1756562.19 ft
East: 1028807.38 ft
Station: 431+77.65
Offset: 85.82 RT



GENERAL NOTES

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-04

WEI Job No.: 7901-15-01

TranSystems Corporation

Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 595.93 ft
North: 1756949.42 ft
East: 1029264.15 ft
Station: 437+68.62
Offset: 65.2 RT

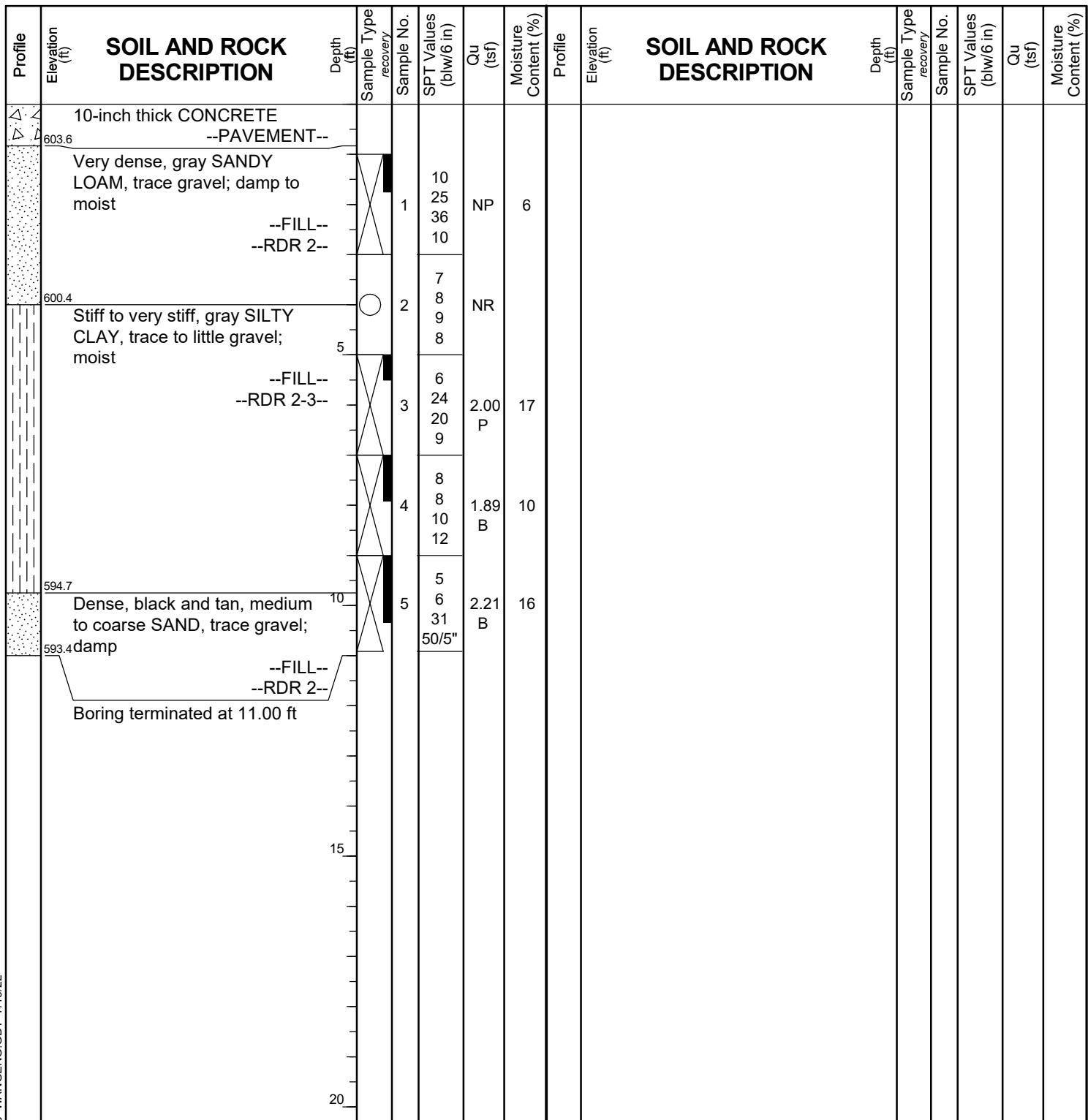


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BORING LOG EB-SGB-05

WEI Job No.: 7901-15-01
Client TranSystems Corporation
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 604.41 ft
North: 1757311.50 ft
East: 1029644.42 ft
Station: 442+87.88
Offset: 67.53 RT



GENERAL NOTES

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

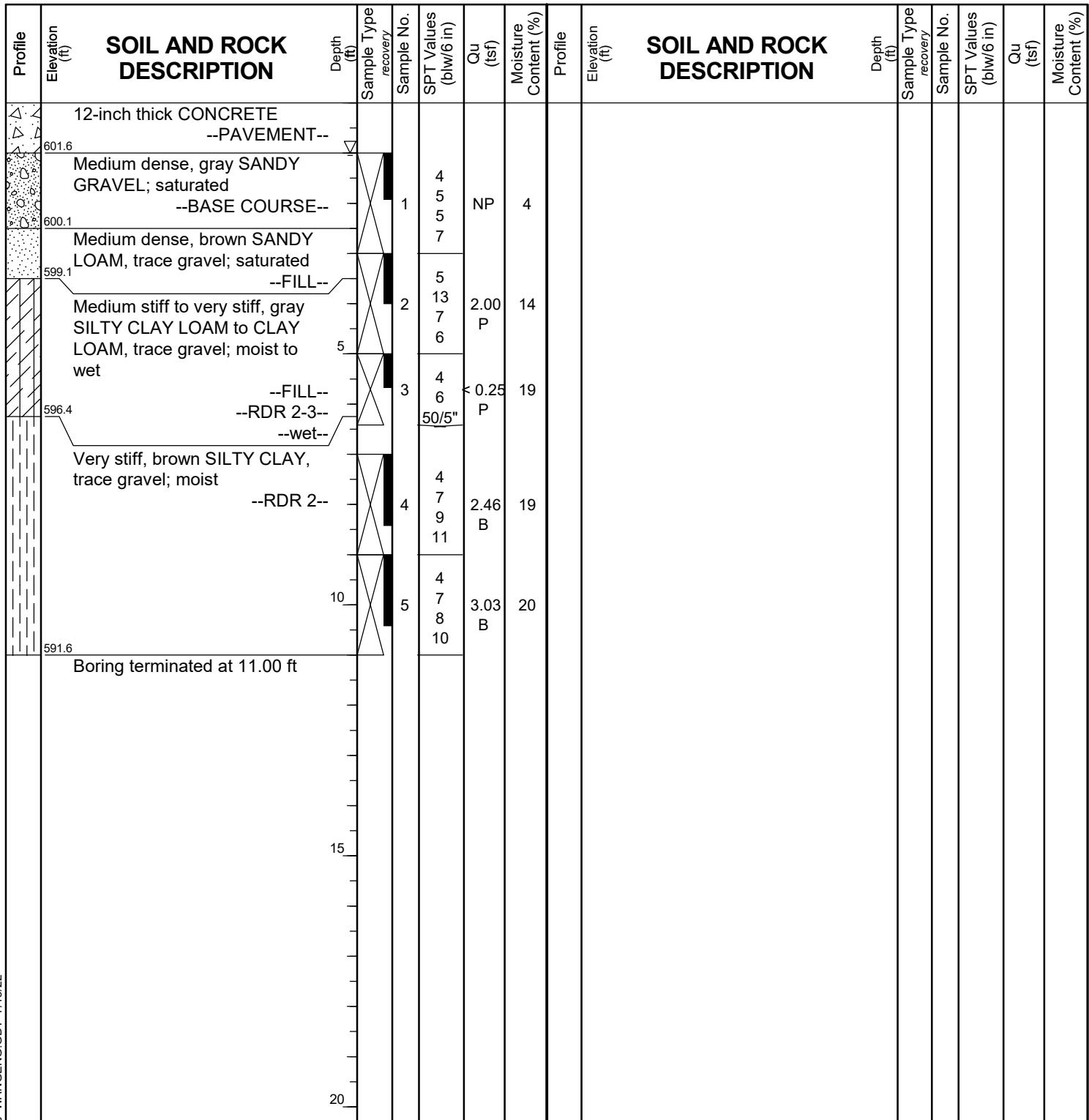


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BORING LOG EB-SGB-06

WEI Job No.: 7901-15-01
Client TranSystems Corporation
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 602.63 ft
North: 1757800.01 ft
East: 1030091.37 ft
Station: 449+48.31
Offset: 69.55 RT



GENERAL NOTES

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **V 1.00 ft**
At Completion of Drilling **V DRY**
Time After Drilling **NA**
Depth to Water **V NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-07

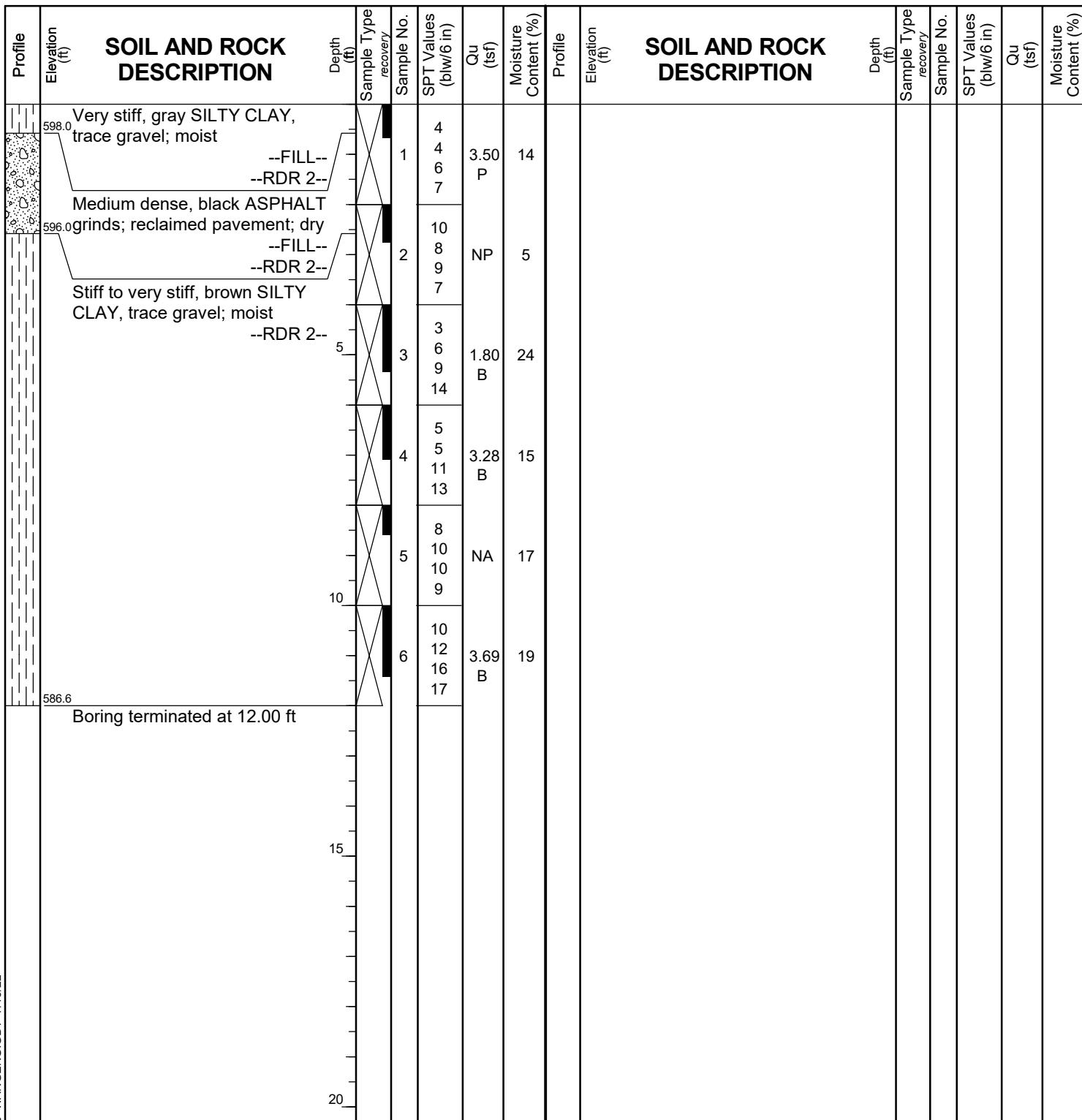
WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Page 1 of 1

Datum: NAVD 88
Elevation: 598.61 ft
North: 1758231.48 ft
East: 1030514.64 ft
Station: 455+52.23
Offset: 94.10 RT



GENERAL NOTES

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **▽ DRY**
At Completion of Drilling **▽ DRY**
Time After Drilling **NA**
Depth to Water **▽ NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-08

WEI Job No.: 7901-15-01

TranSystems Corporation

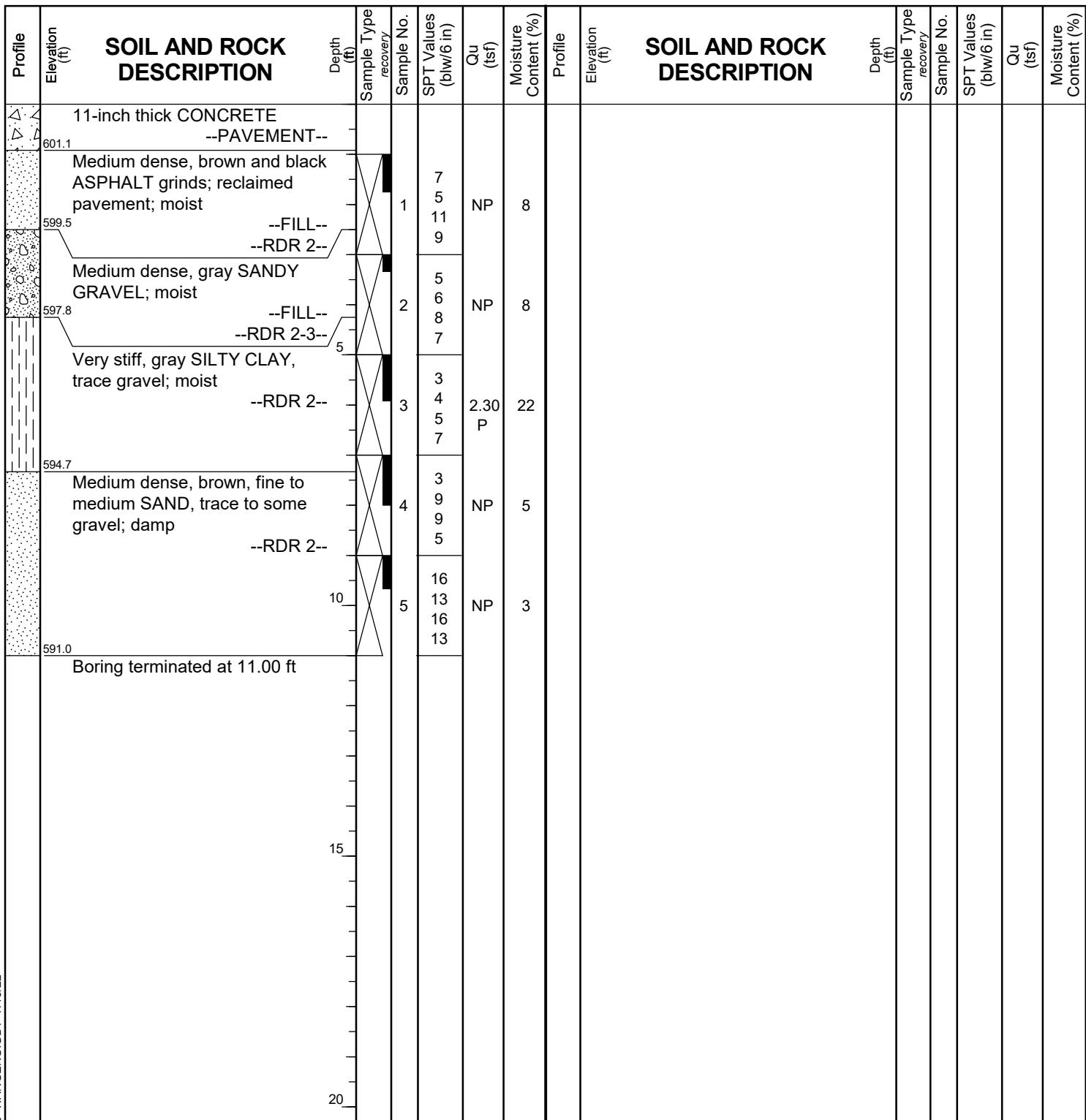
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 602.04 ft
North: 1758691.04 ft
East: 1030907.65 ft
Station: 461+56.70
Offset: 77.36 RT



GENERAL NOTES

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-09

WEI Job No.: 7901-15-01

TranSystems Corporation

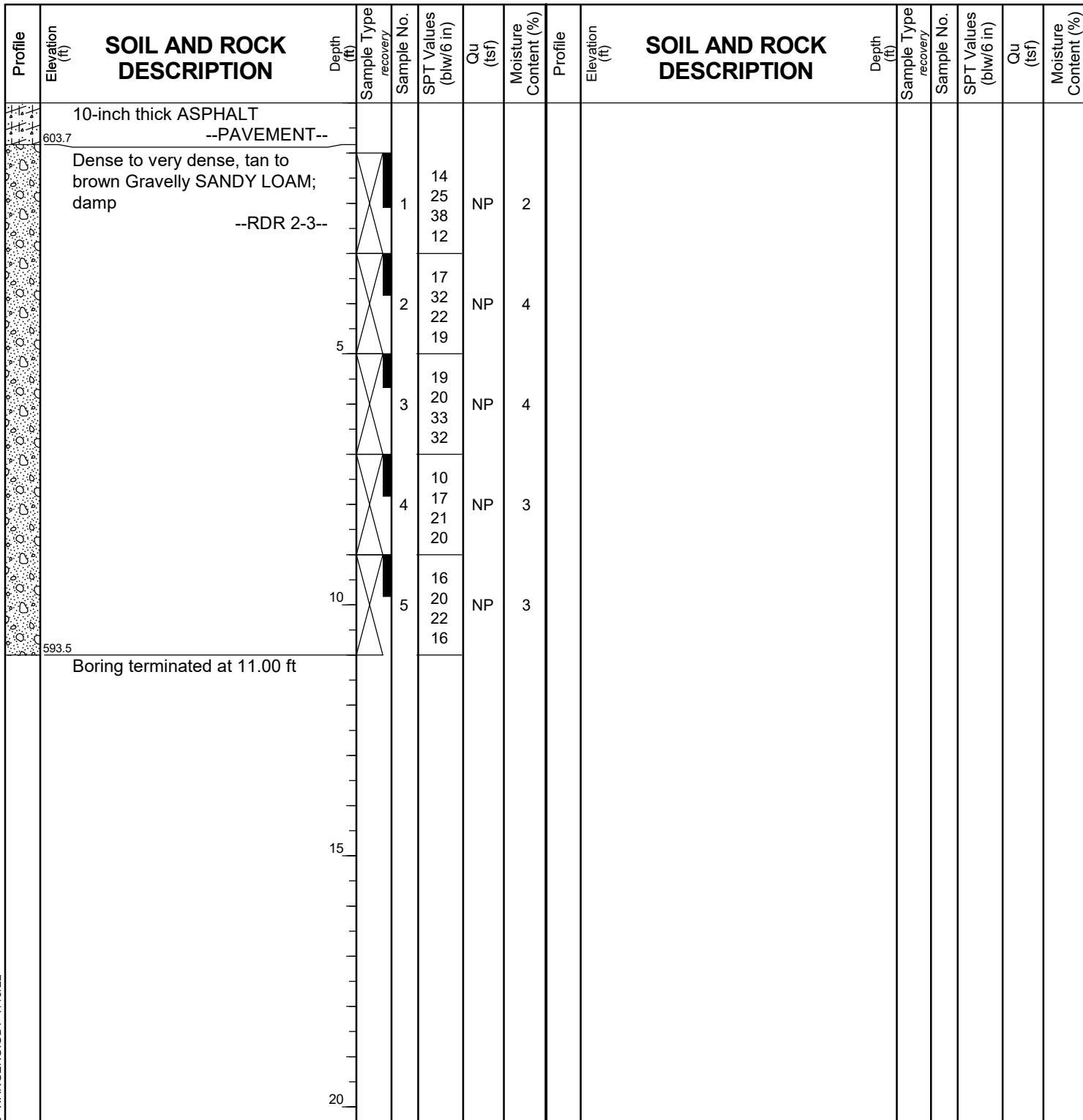
Client

Project-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 604.53 ft
North: 1759144.17 ft
East: 1031307.27 ft
Station: 467+60.81
Offset: 69.84 RT



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GENERAL NOTES

Begin Drilling **04-27-2022** Complete Drilling **04-27-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling DRY
At Completion of Drilling DRY
Time After Drilling NA
Depth to Water NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG EB-SGB-10

WEI Job No.: 7901-15-01

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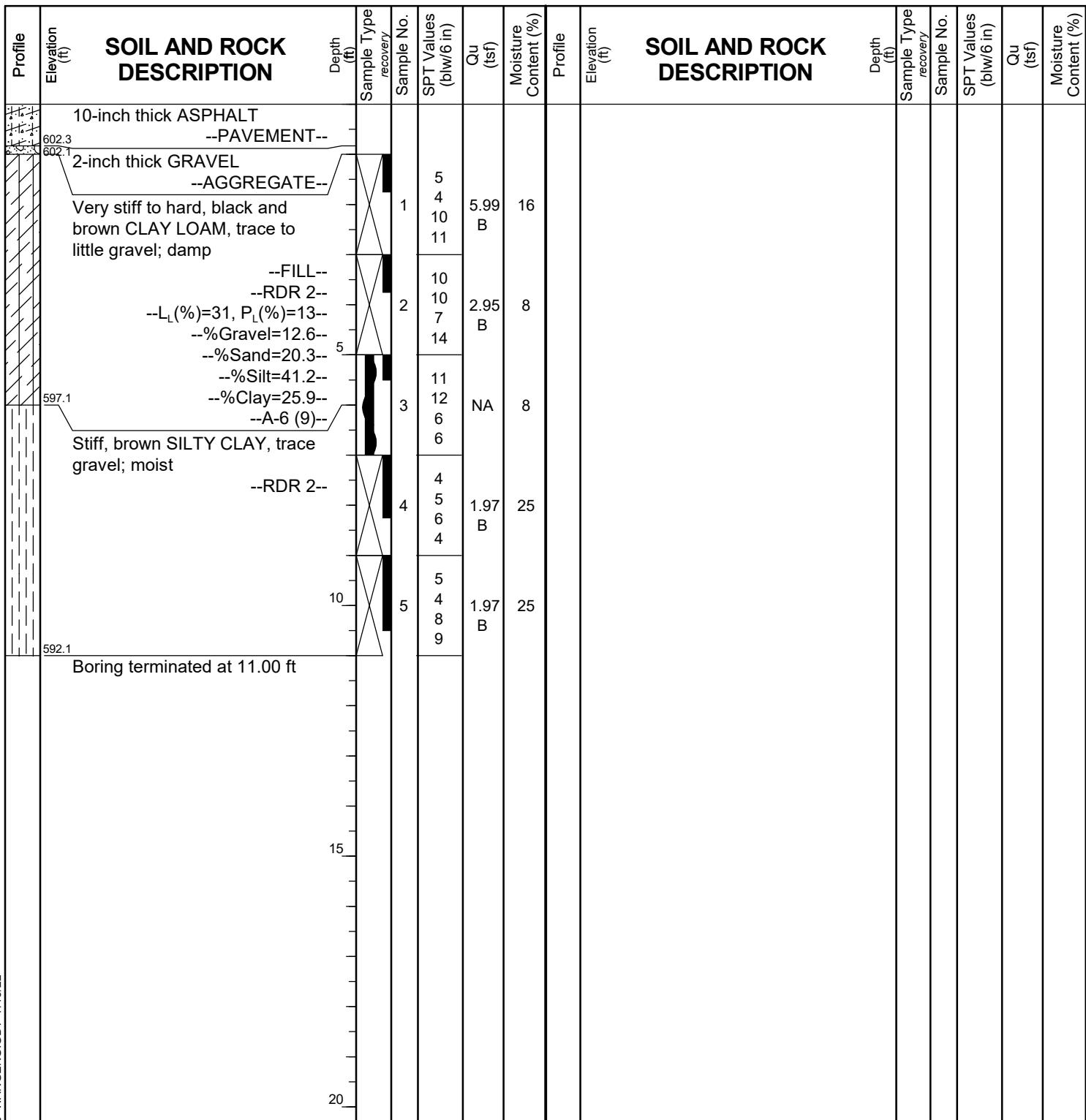
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 603.14 ft
North: 1759583.96 ft
East: 1031700.48 ft
Station: 473+50.74
Offset: 66.52 RT



GENERAL NOTES

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-11

WEI Job No.: 7901-15-01

TranSystems Corporation

Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 600.37 ft
North: 1760025.30 ft
East: 1032098.46 ft
Station: 479+45.03
Offset: 65.69 RT

GENERAL NOTES

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling DRY
At Completion of Drilling DRY
Time After Drilling NA
Depth to Water NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG EB-SGB-12

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WEI Job No.: 7901-15-01

TranSystems Corporation

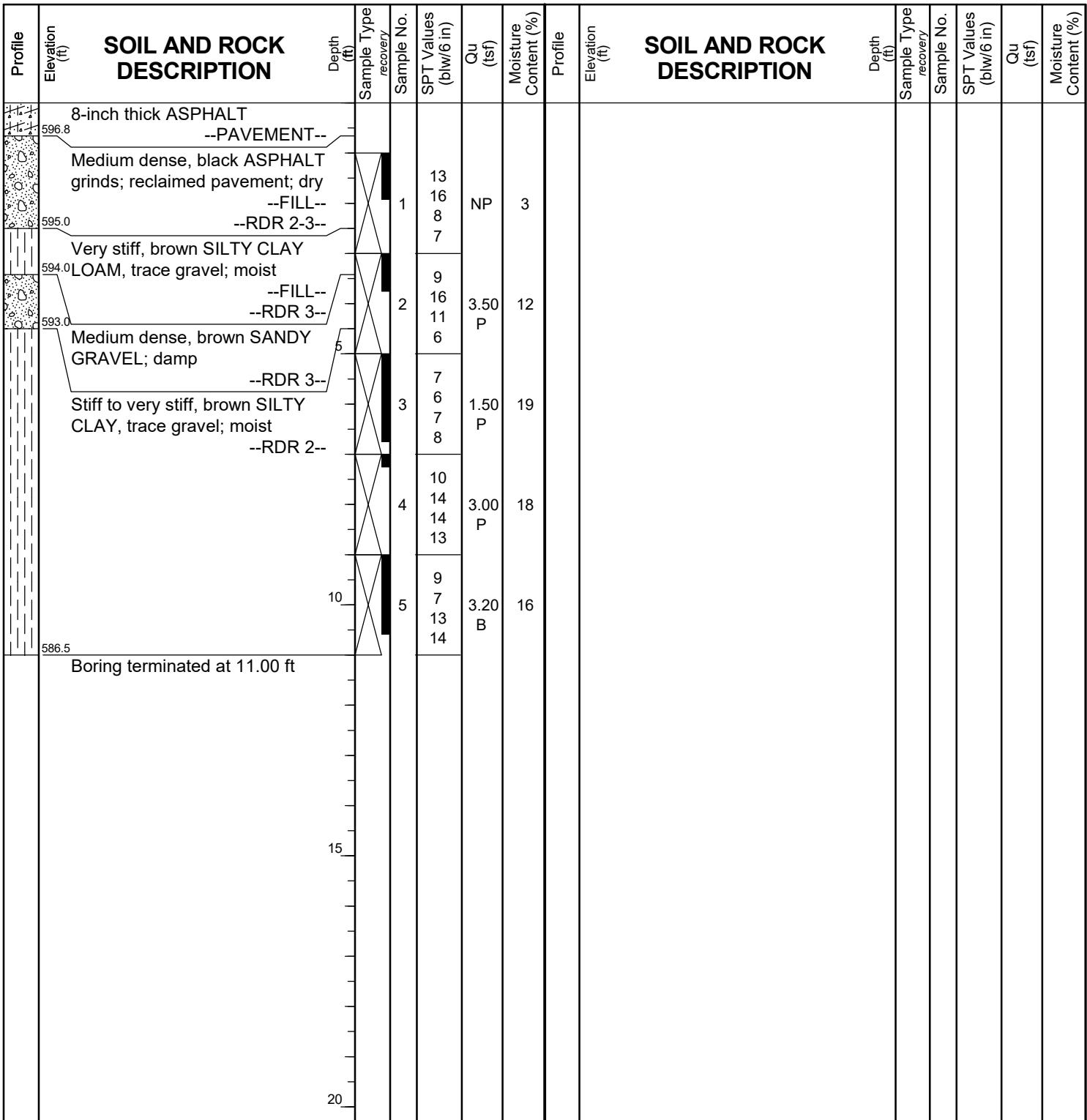
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 597.46 ft
North: 1760472.04 ft
East: 1032502.90 ft
Station: 485+48.80
Offset: 67.98 RT



WANGENG INC 79011501 GBP! WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-13

WEI Job No.: 7901-15-01

TranSystems Corporation

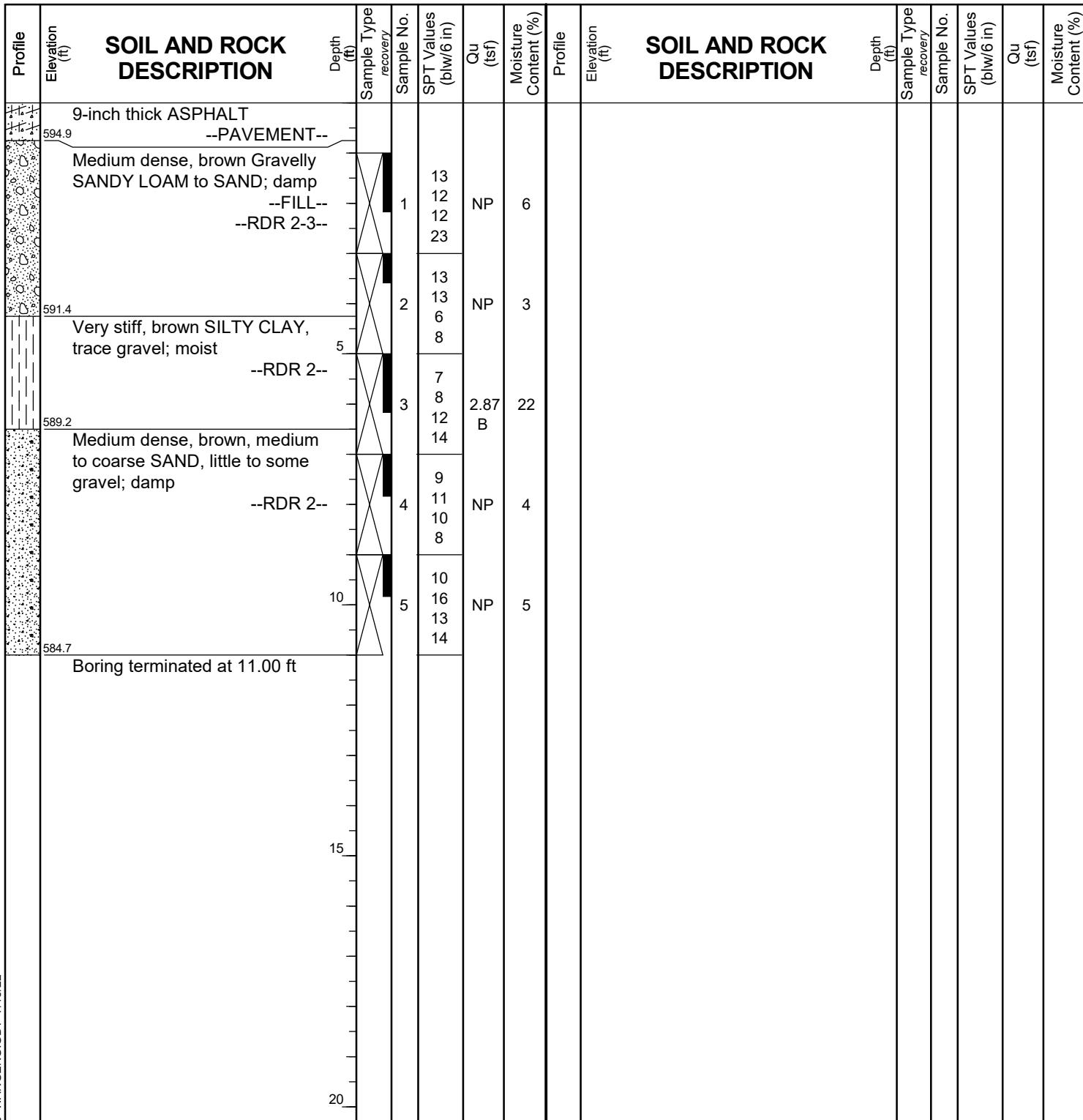
Client

Project-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 595.68 ft
North: 1760929.78 ft
East: 1032917.39 ft
Station: 491+65.16
Offset: 66.44 RT



GENERAL NOTES

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.

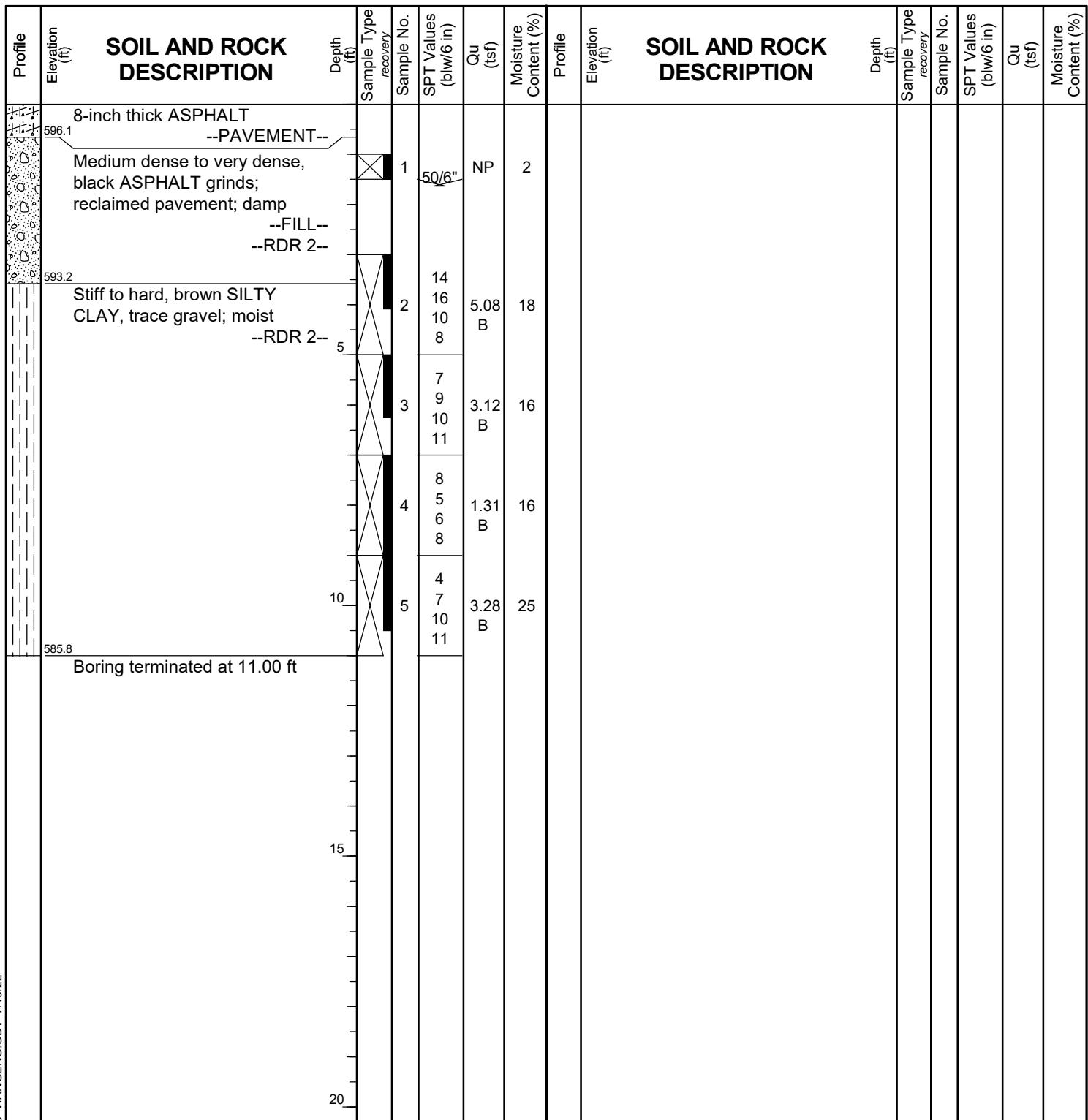


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BORING LOG EB-SGB-14

WEI Job No.: 7901-15-01
Client TranSystems Corporation
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 596.79 ft
North: 1761361.52 ft
East: 1033307.41 ft
Station: 497+46.98
Offset: 66.14 RT



GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-15

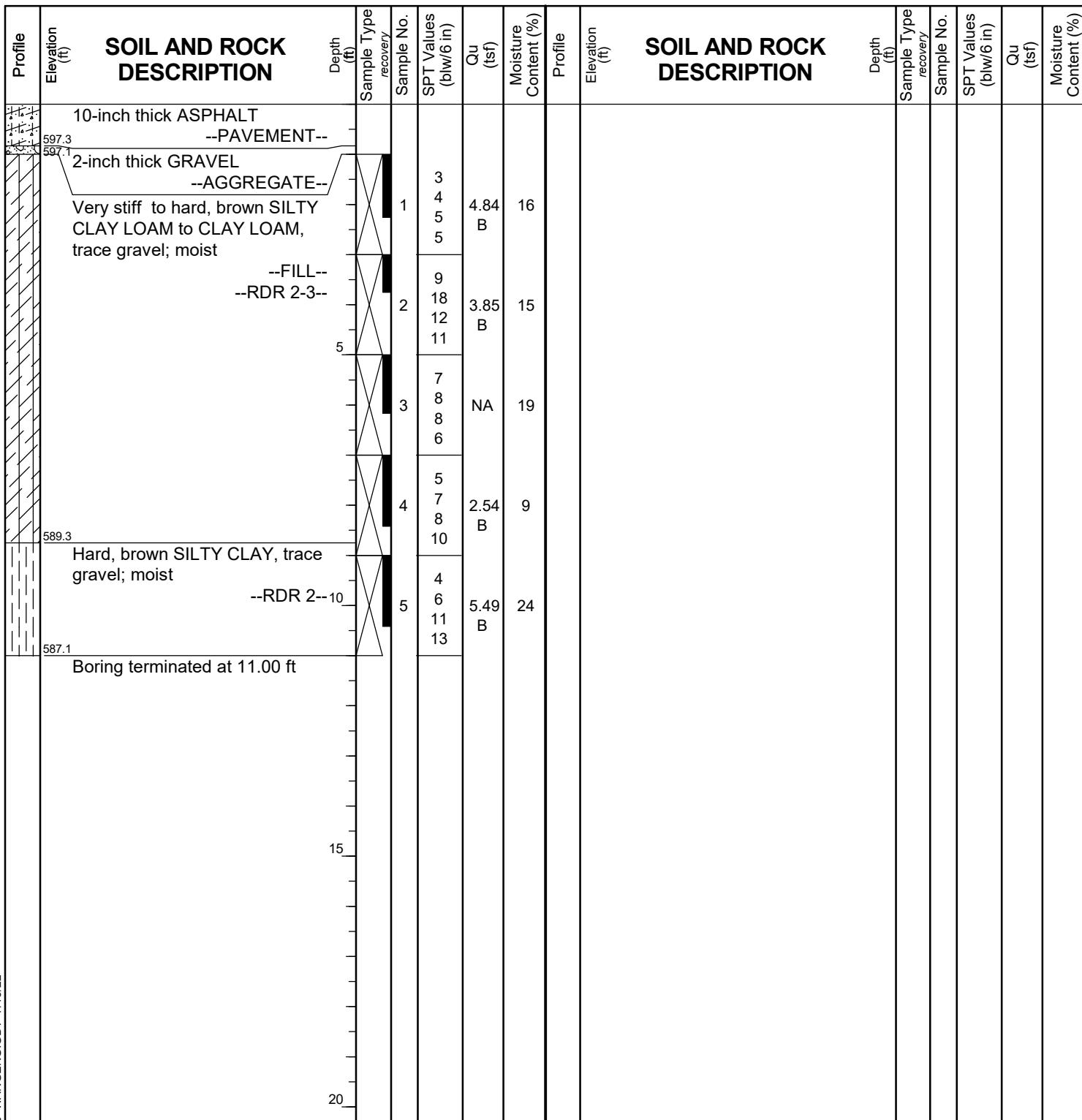
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 598.10 ft
North: 1761804.54 ft
East: 1033711.18 ft
Station: 503+48.24
Offset: 65.19 RT



GENERAL NOTES

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-16

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WEI Job No.: 7901-15-01

TranSystems Corporation

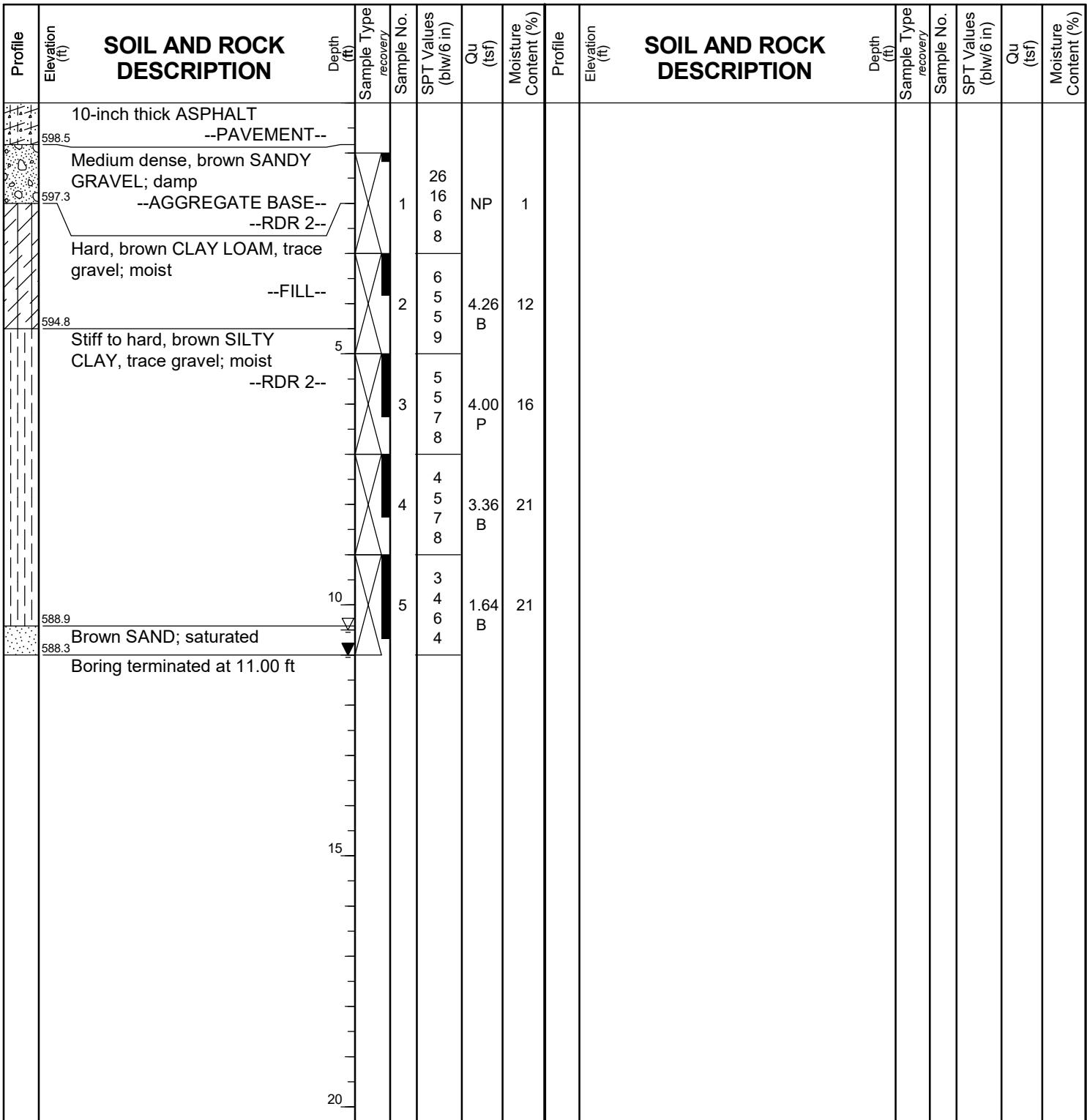
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 599.34 ft
North: 1762222.35 ft
East: 1034141.03 ft
Station: 509+52.41
Offset: 66.08 RT



WANGENG INC 79011501 GBP! WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|----|----------|
| While Drilling | ▽ | 10.50 ft |
| At Completion of Drilling | ▼ | 11.00 ft |
| Time After Drilling | NA | |
| Depth to Water | ▽ | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG EB-SGB-17

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WEI Job No.: 7901-15-01

TranSystems Corporation

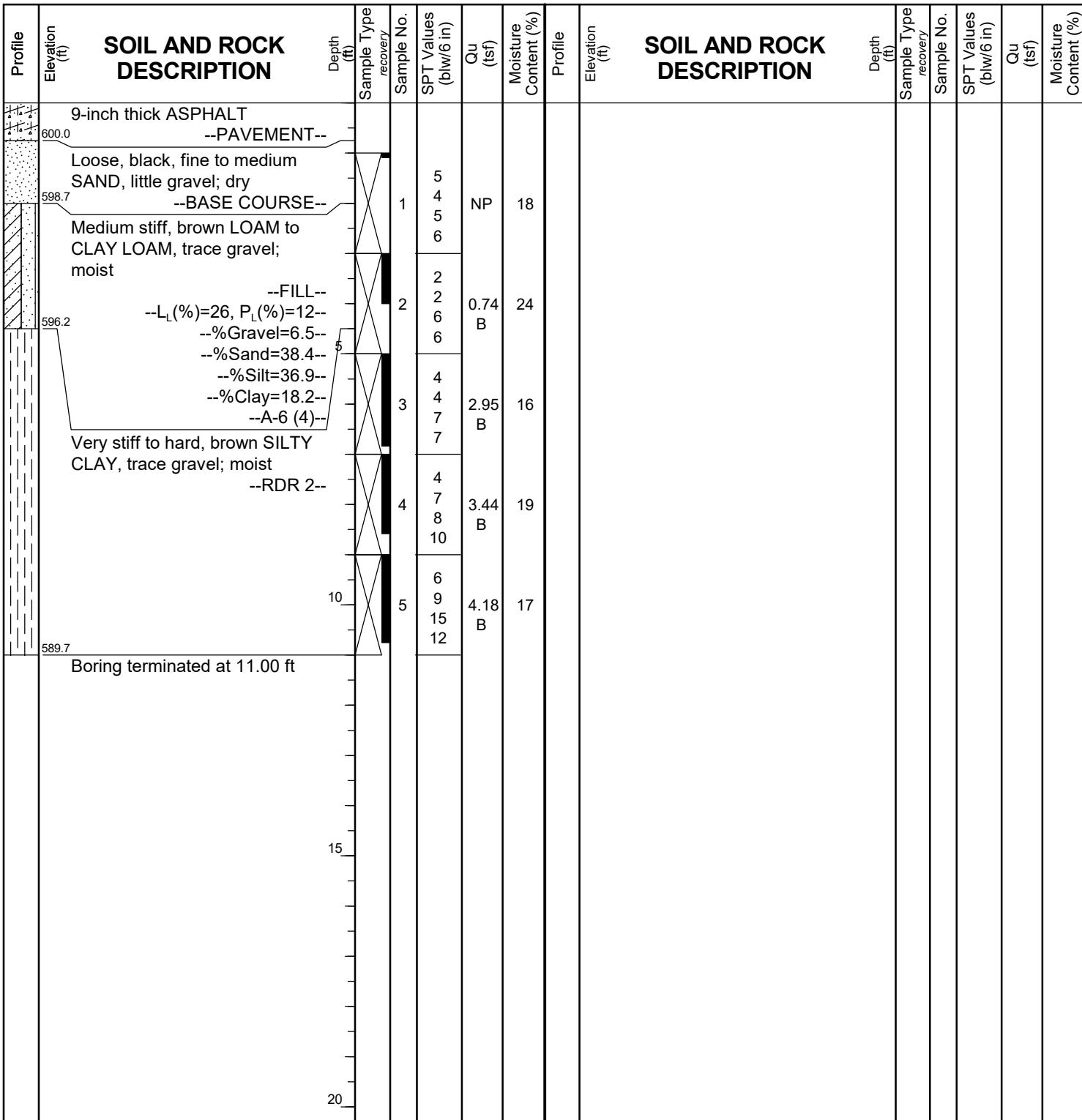
Client

Project-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 600.73 ft
North: 1762612.93 ft
East: 1034602.30 ft
Station: 515+61.70
Offset: 70.15 RT



WANGENG INC 79011501 GBP | WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **04-28-2022** Complete Drilling **04-28-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **PH&TC** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|-------------------------------------|-----|
| While Drilling | <input checked="" type="checkbox"/> | DRY |
| At Completion of Drilling | <input checked="" type="checkbox"/> | DRY |
| Time After Drilling | <input type="checkbox"/> | NA |
| Depth to Water | <input checked="" type="checkbox"/> | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG HR-BSB-01

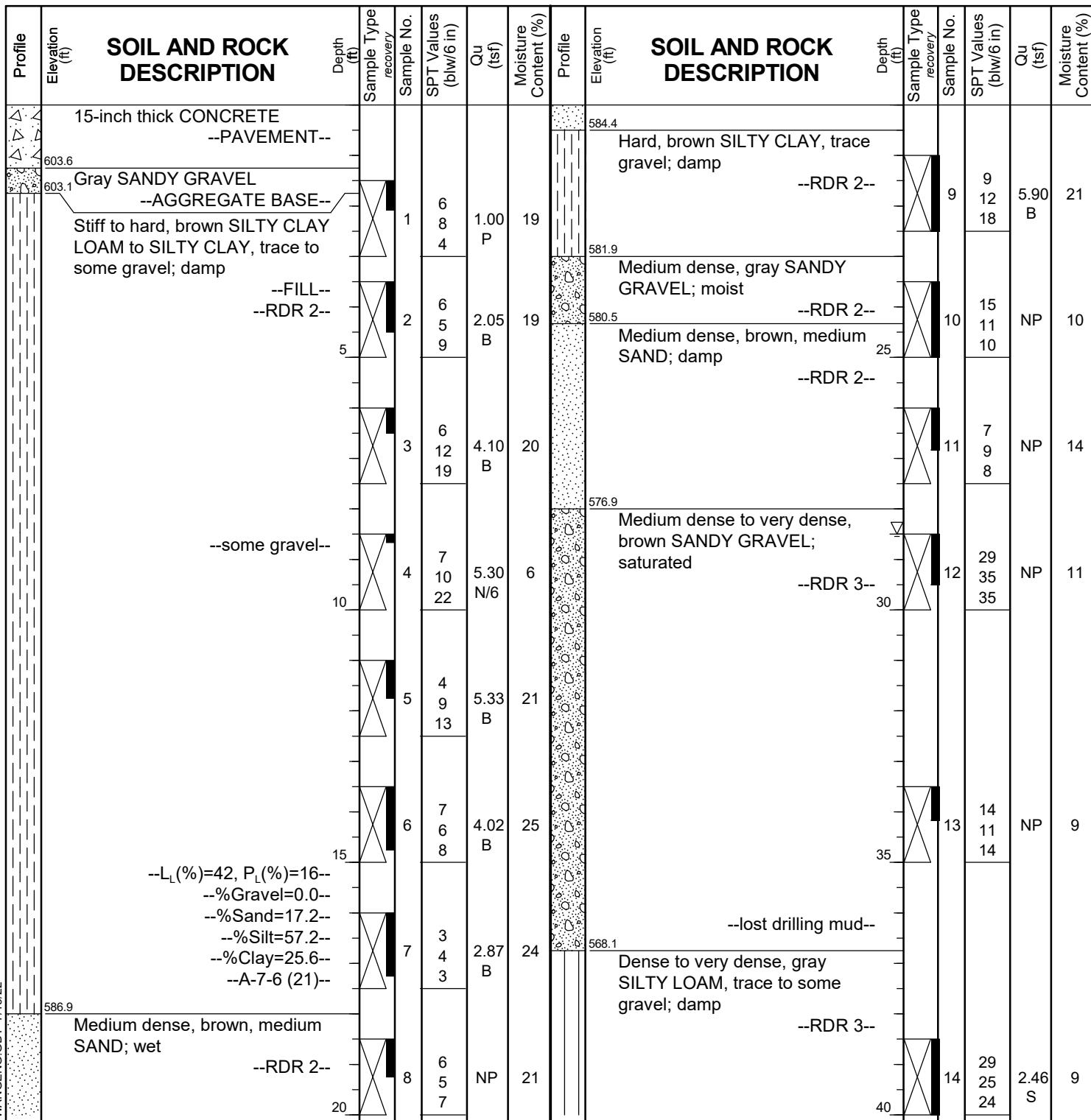
WEI Job No.: 7901-15-01

Client TranSystems Corporation

Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Page 1 of 2

Datum: NAVD 88
Elevation: 604.87 ft
North: 1757424.96 ft
East: 1029622.93 ft
Station: 443+55.59
Offset: 25.83 LT





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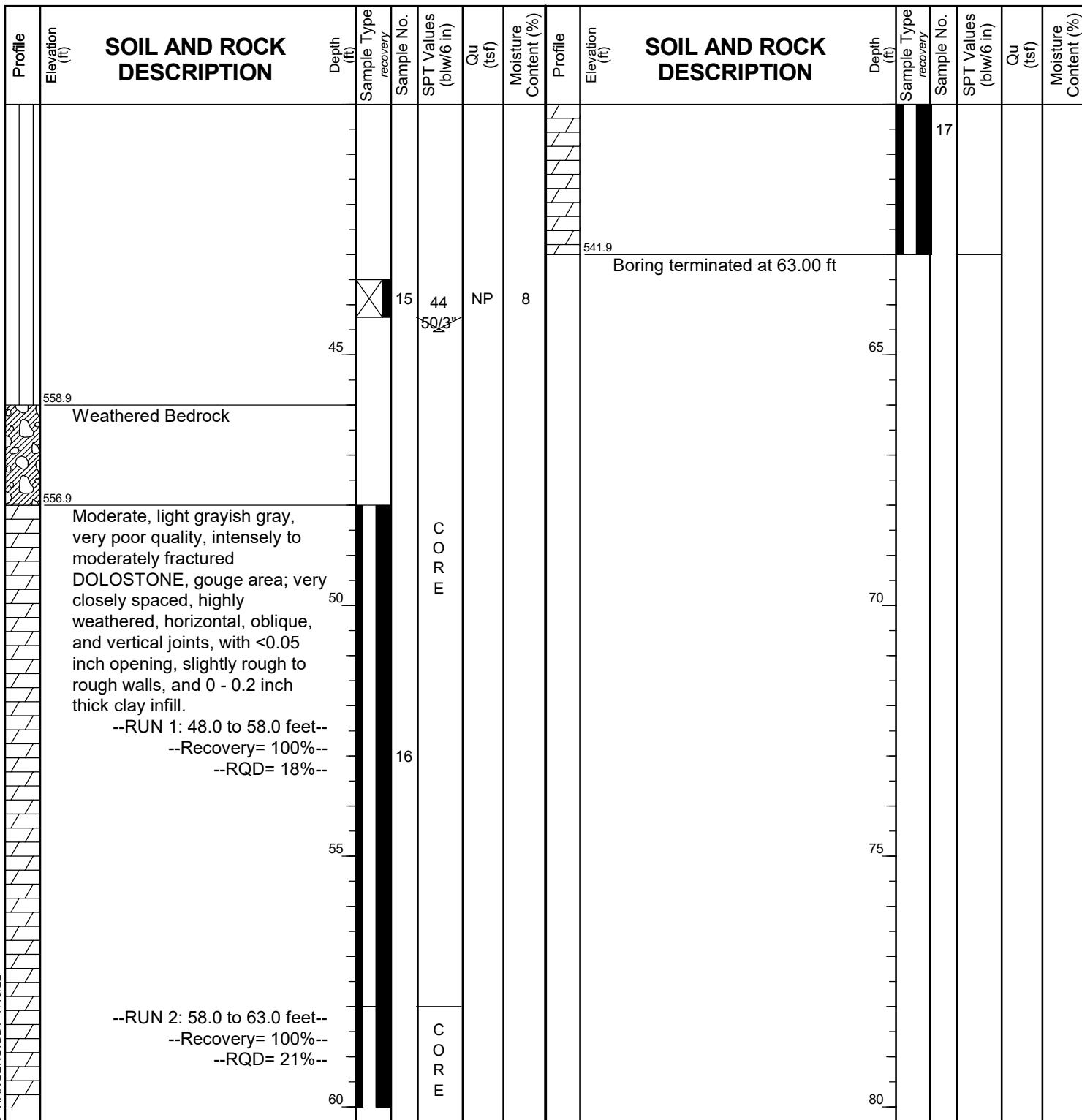
BORING LOG HR-BSB-01

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 604.87 ft
North: 1757424.96 ft
East: 1029622.93 ft
Station: 443+55.59
Offset: 25.83 LT



GENERAL NOTES

Begin Drilling **04-21-2021** Complete Drilling **04-21-2021**
Drilling Contractor **Wang Testing Services** Drill Rig **20CME55T[81%]**
Driller **R&J** Logger **I. Nenn** Checked by **C. Marin**
Drilling Method **2.25" IDA HSA to 10 ft; mud rotary thereafter; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **▽ 28.50 ft**
At Completion of Drilling **▽ mud in borehole**
Time After Drilling **NA**
Depth to Water **▽ NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.

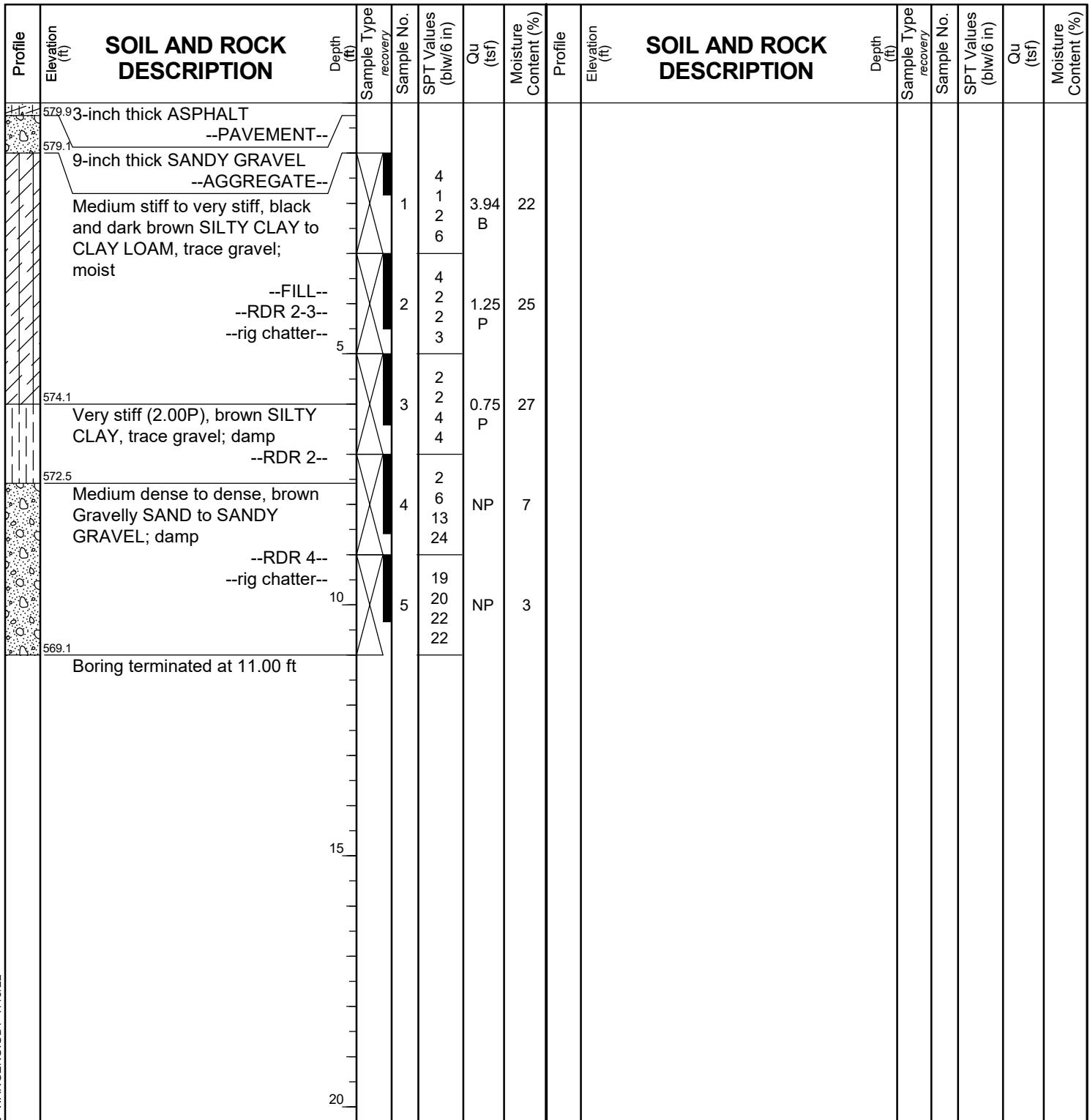


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BORING LOG WB-SGB-01

Client **TranSystems Corporation**
Project **I-80 Reconstruction (Houbolt Road to Center Street)**
Location **Will County, Illinois**

Datum: NAVD 88
Elevation: 580.10 ft
North: 1755938.39 ft
East: 1027275.97 ft
Station: 415+30.58
Offset: 57.07 LT



| GENERAL NOTES | | | | WATER LEVEL DATA | | |
|---------------------|--|-------------------|--------------|---|-----------|-----|
| Begin Drilling | 05-23-2022 | Complete Drilling | 05-23-2022 | While Drilling | ▽ | DRY |
| Drilling Contractor | Wang Testing Services | Drill Rig | 21GeoAI[96%] | At Completion of Drilling | ▽ | DRY |
| Driller | RR&AP | Logger | D. You | Checked by | J. Bensen | NA |
| Drilling Method | 2.25" IDA HSA; boring backfilled upon completion | | | Time After Drilling | NA | NA |
| | | | | Depth to Water | ▽ | NA |
| | | | | The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual. | | |



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BORING LOG WB-SGB-02

Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

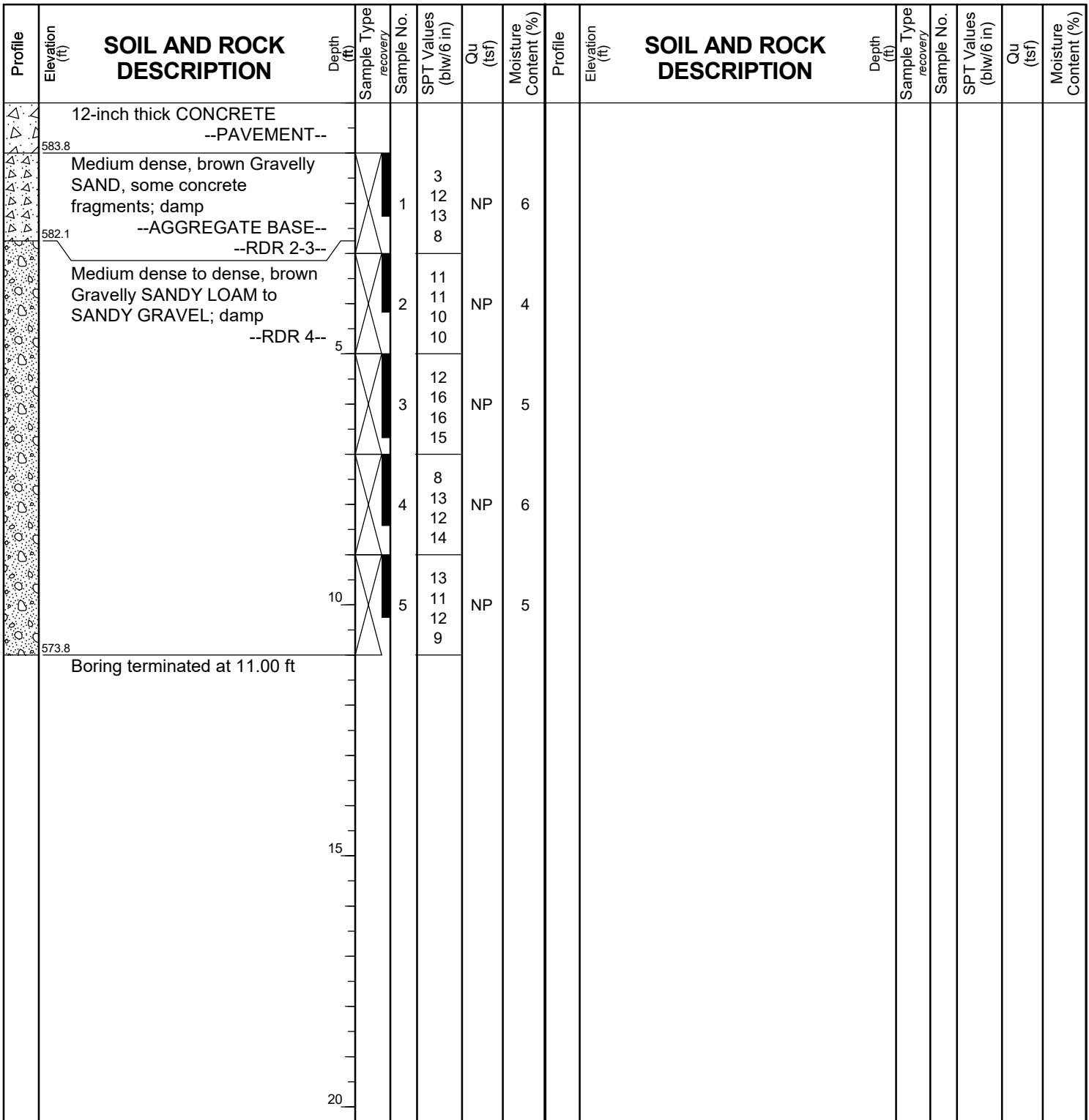
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 584.83 ft
North: 1756172.40 ft
East: 1027849.45 ft
Station: 421+54.70
Offset: 61.13 LT



WANGENG INC 79011501 GP1 WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-23-2022** Complete Drilling **05-23-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **21GeoA[96%]**
Driller **RR&AP** Logger **D. You** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG WB-SGB-03

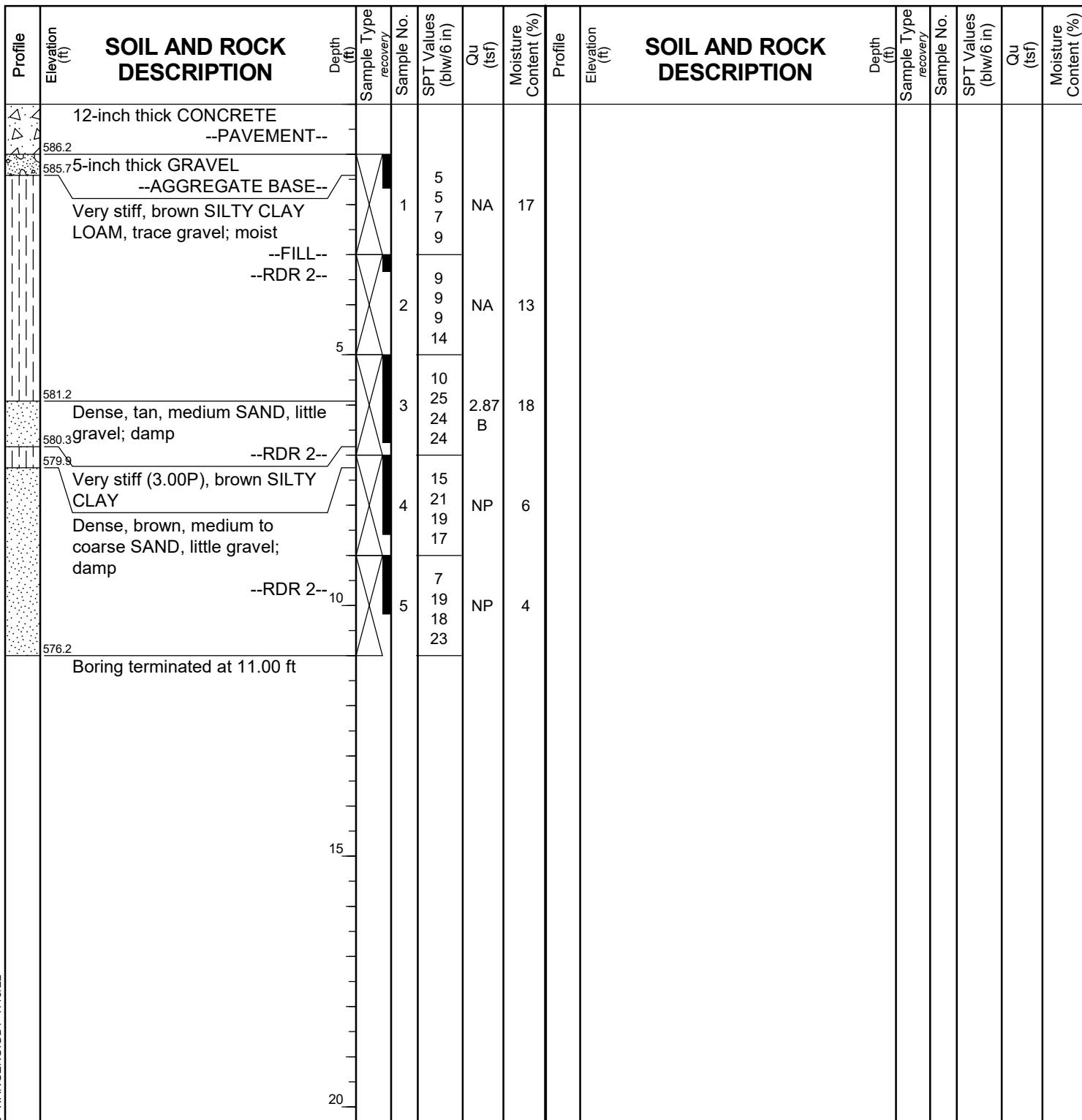
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 587.16 ft
North: 1756450.35 ft
East: 1028350.59 ft
Station: 427+34.58
Offset: 70.58 LT



GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
 Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
 Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
 Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
 At Completion of Drilling **DRY**
 Time After Drilling **NA**
 Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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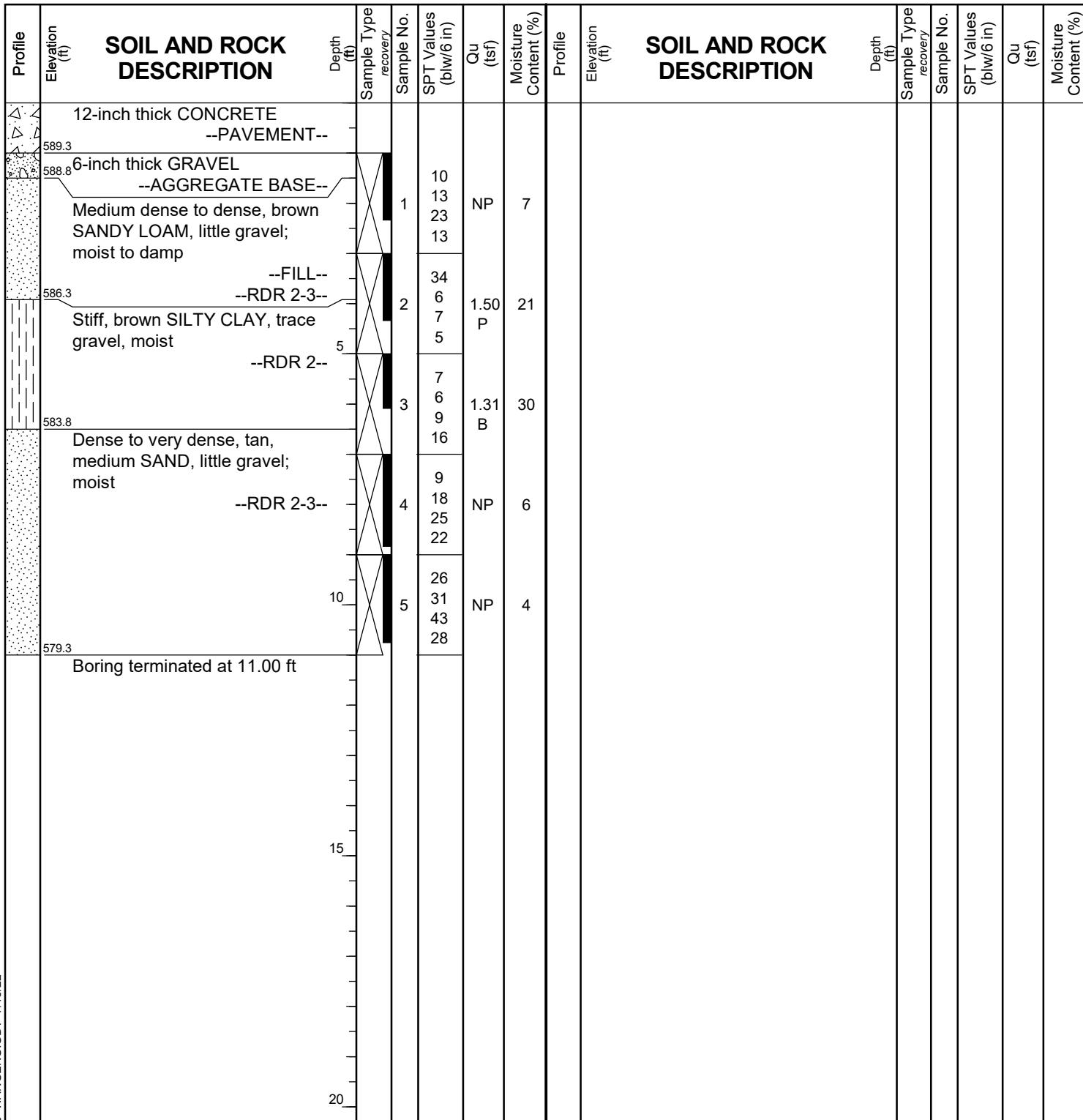
BORING LOG WB-SGB-04

WEI Job No.: 7901-15-01

TranSystems Corporation

Client TranSystems Corporation
Project I-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 590.25 ft
North: 1756779.86 ft
East: 1028859.16 ft
Station: 433+47.63
Offset: 58.27 LT



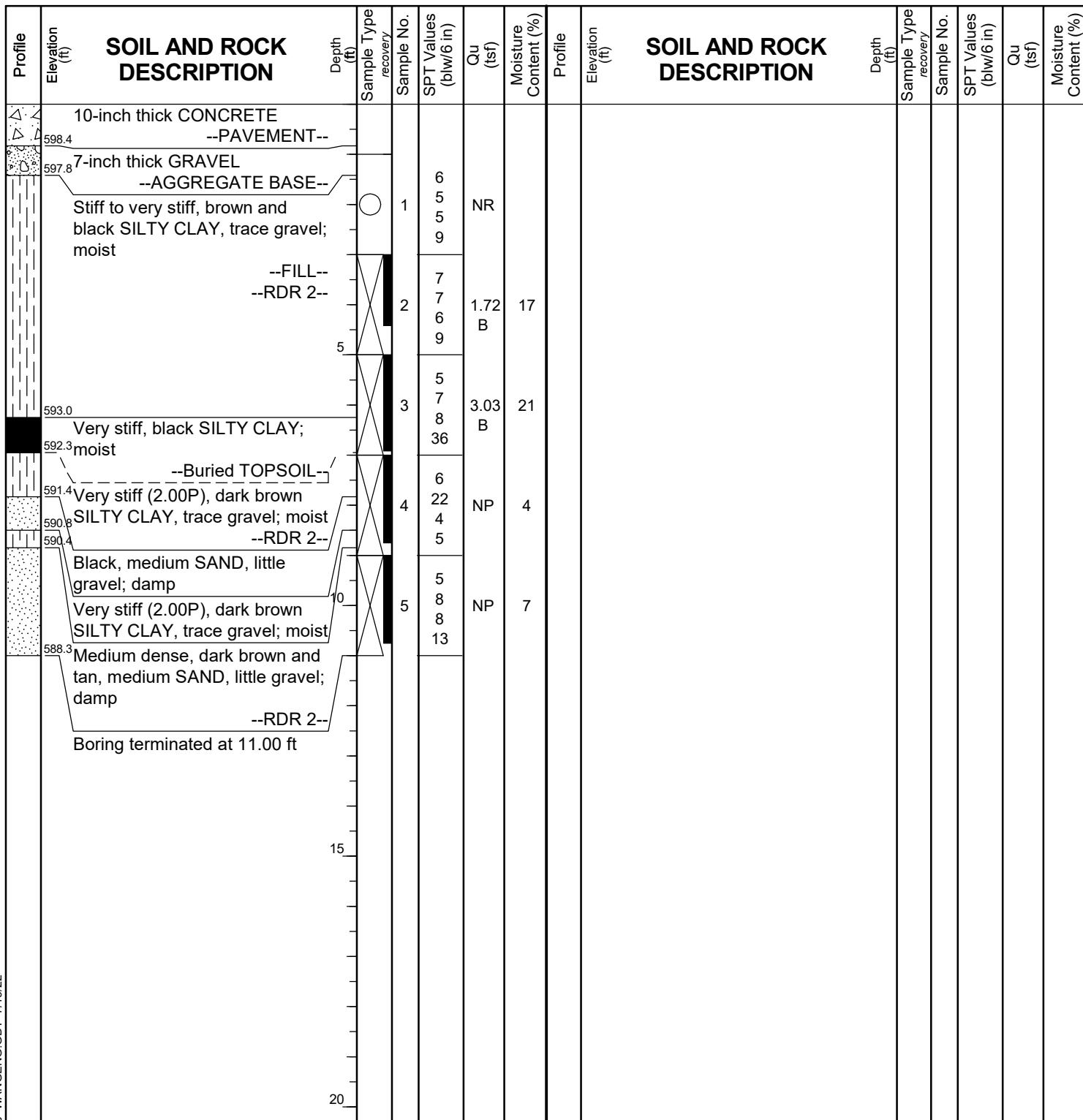


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BORING LOG WB-SGB-05

WEI Job No.: 7901-15-01
Client TranSystems Corporation
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 599.26 ft
North: 1757169.13 ft
East: 1029320.55 ft
Station: 439+57.92
Offset: 59.67 LT



GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



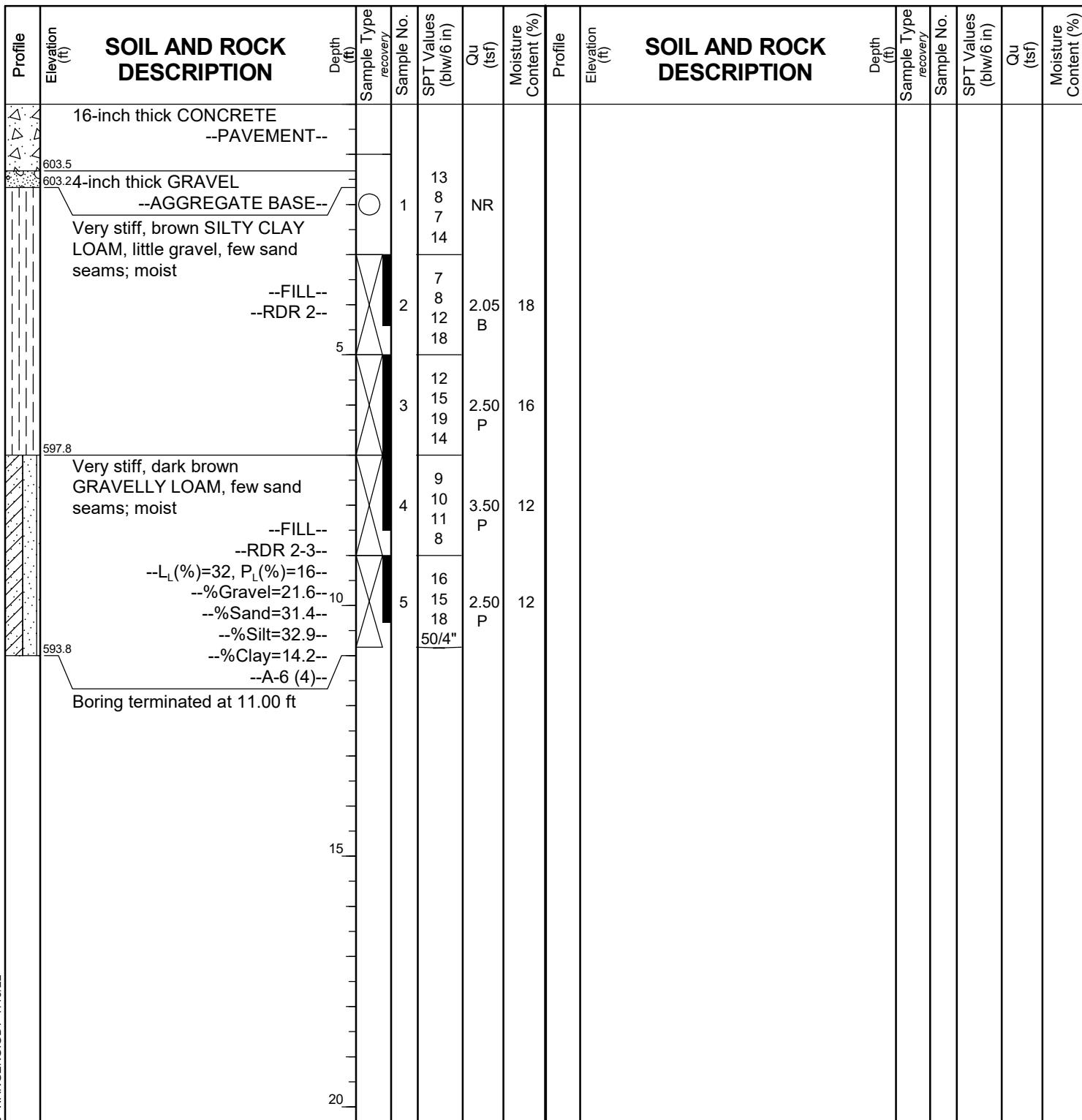
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BORING LOG WB-SGB-06

WEI Job No.: 7901-15-01
Client TranSystems Corporation
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Page 1 of 1

Datum: NAVD 88
Elevation: 604.84 ft
North: 1757626.65 ft
East: 1029761.10 ft
Station: 445+98.21
Offset: 59.14 LT



GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG WB-SGB-07

WEI Job No.: 7901-15-01

TranSystems Corporation

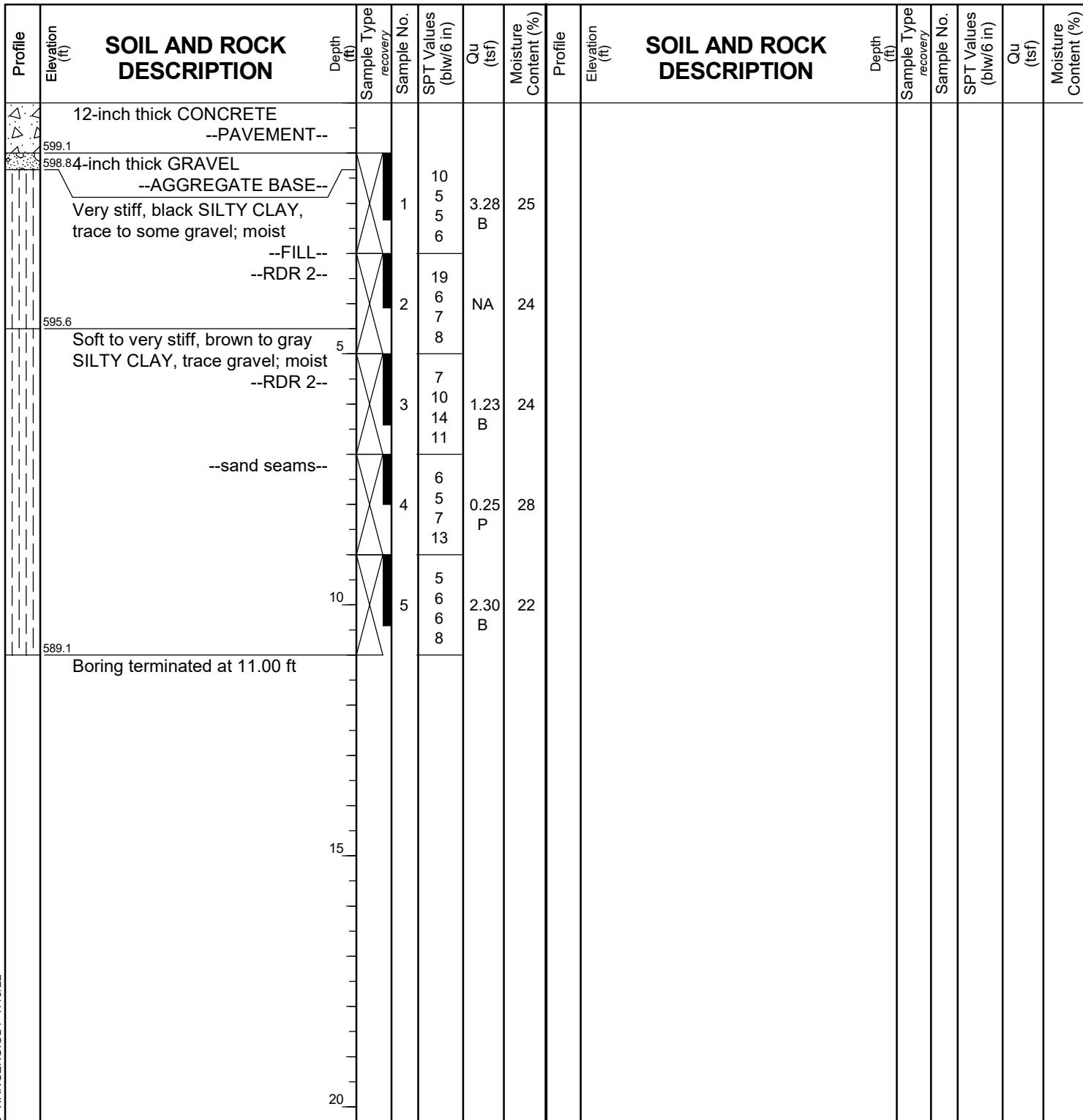
Client

Project-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 600.11 ft
North: 1758050.83 ft
East: 1030144.26 ft
Station: 451+69.82
Offset: 59.45 LT



WANGENGINC 79011501.GPJ WANGENG.GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG WB-SGB-08

Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

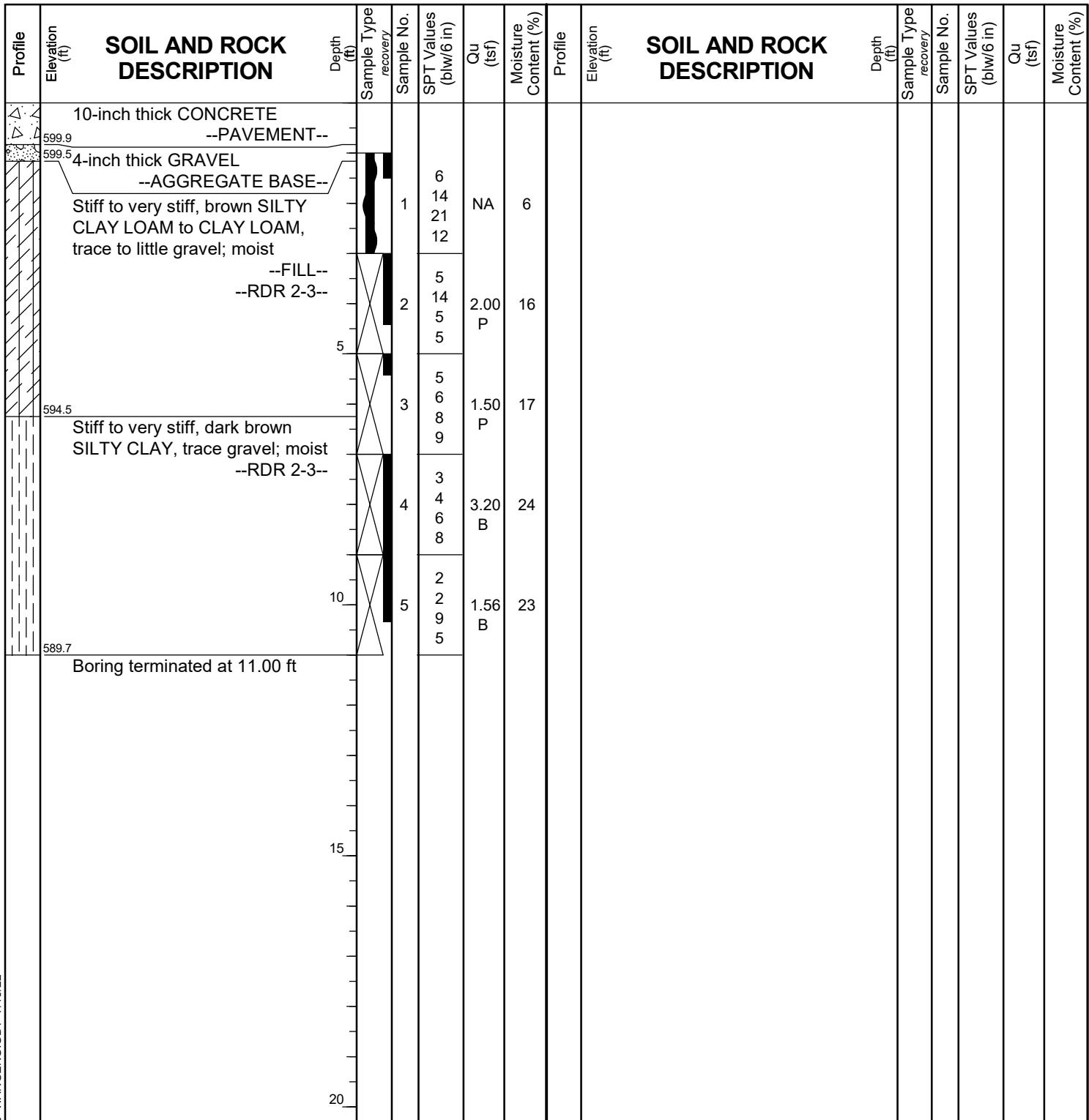
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 600.71 ft
North: 1758506.25 ft
East: 1030521.67 ft
Station: 457+60.75
Offset: 84.98 LT



WANGENG INC 79011501 GP. I WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling | NA | |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG WB-SGB-09

Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

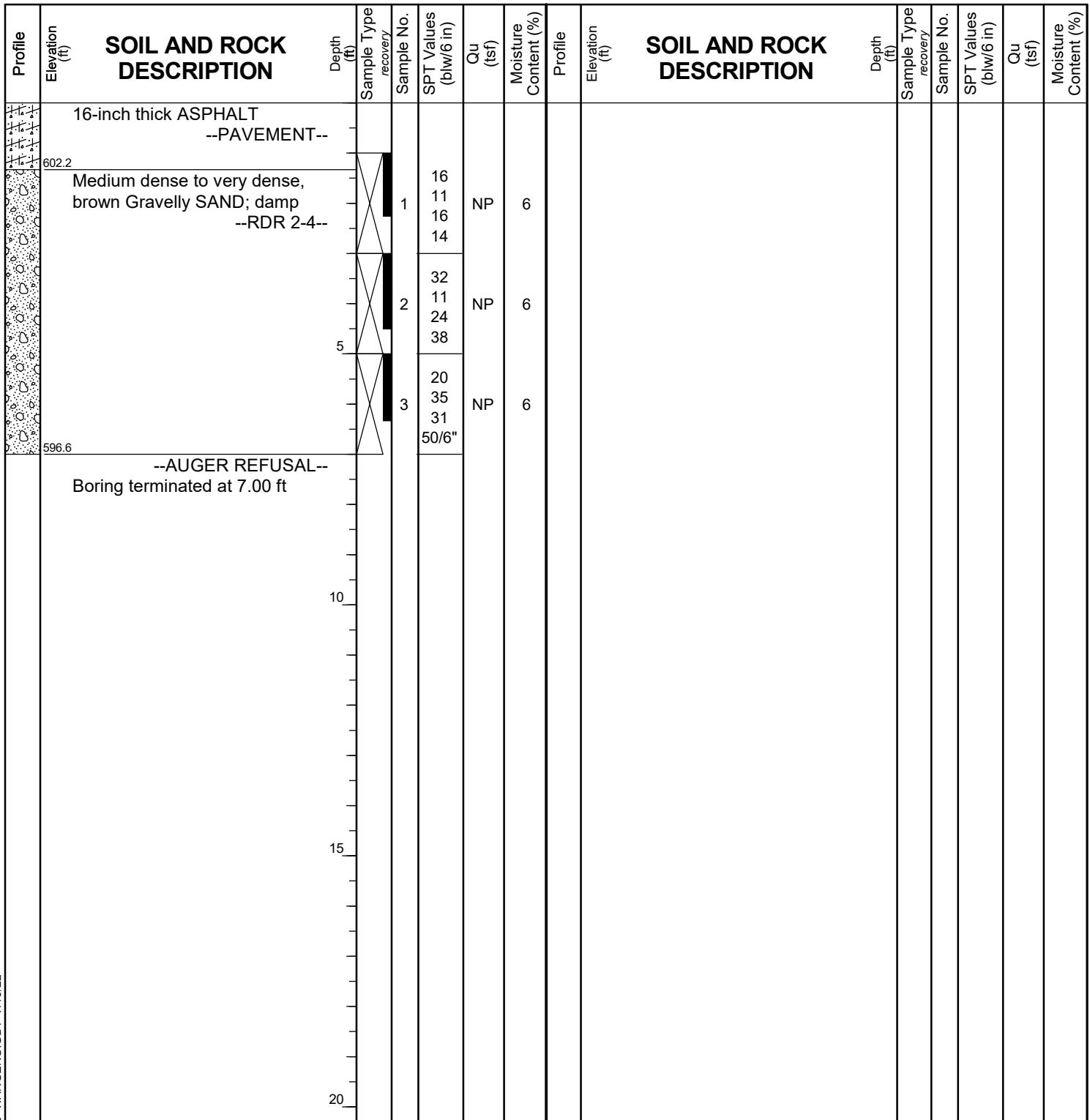
Client

Project-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 603.57 ft
North: 1758935.49 ft
East: 1030945.78 ft
Station: 463+63.58
Offset: 58.31 LT



WANGGENING 79011501 GP. I WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling  **DRY**
At Completion of Drilling  **DRY**
Time After Drilling **NA**
Depth to Water  **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

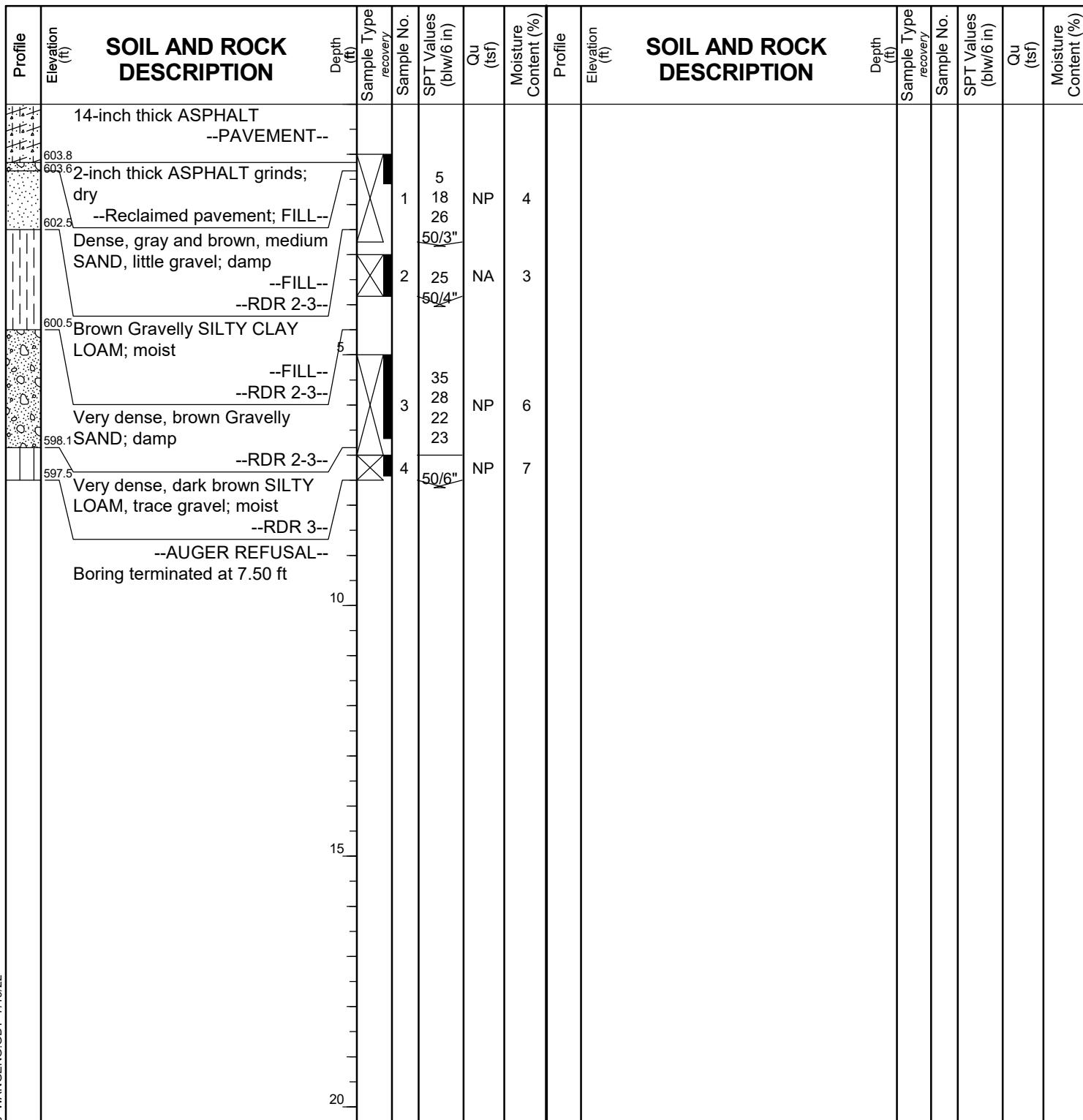


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BORING LOG WB-SGB-10

WEI Job No.: 7901-15-01
Client TranSystems Corporation
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 604.97 ft
North: 1759380.96 ft
East: 1031348.25 ft
Station: 469+64.36
Offset: 60.64 LT



GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.

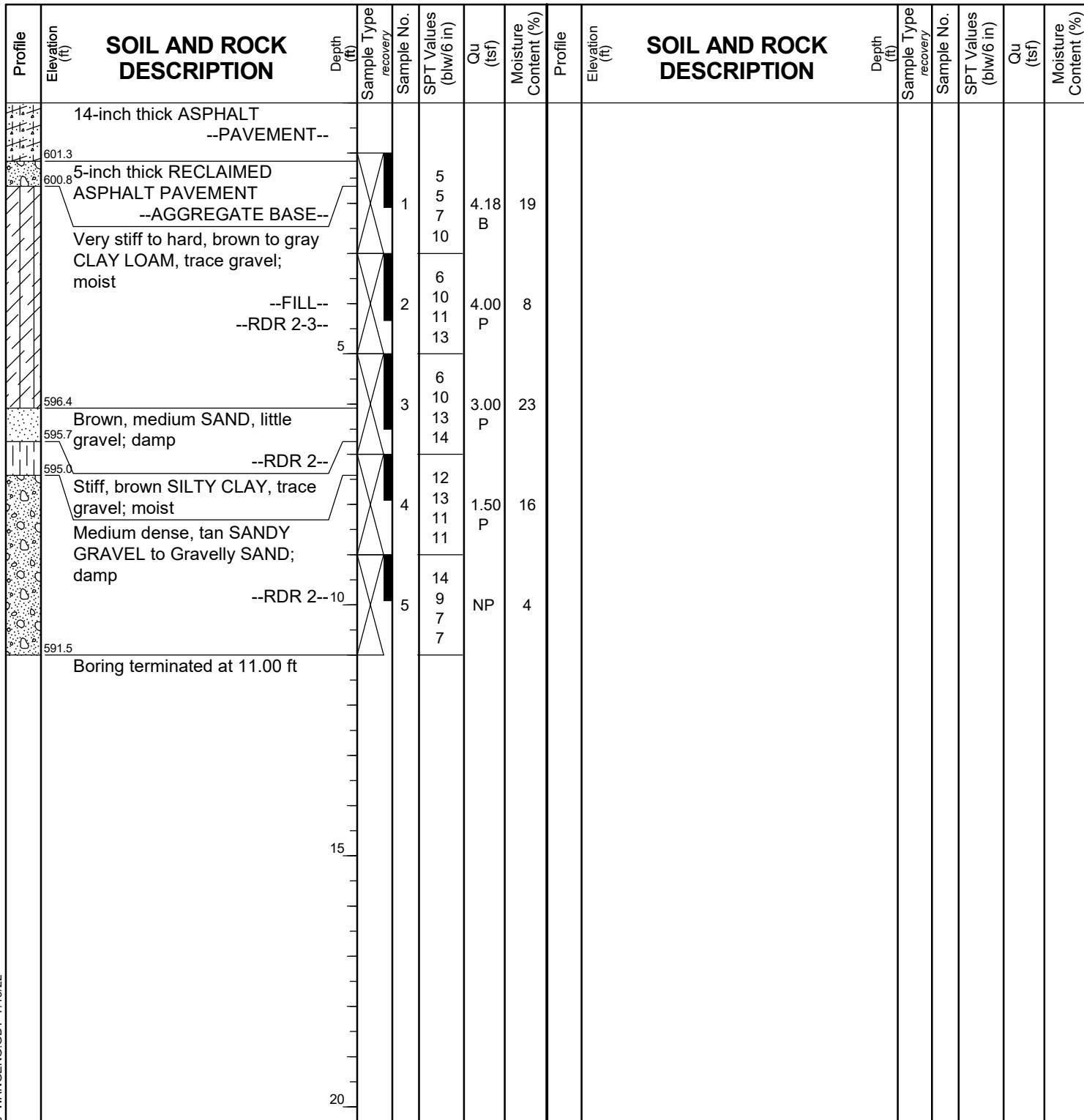


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BORING LOG WB-SGB-11

WEI Job No.: 7901-15-01
Client TranSystems Corporation
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 602.47 ft
North: 1759819.21 ft
East: 1031743.54 ft
Station: 475+54.12
Offset: 59.33 LT



GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG WB-SGB-12

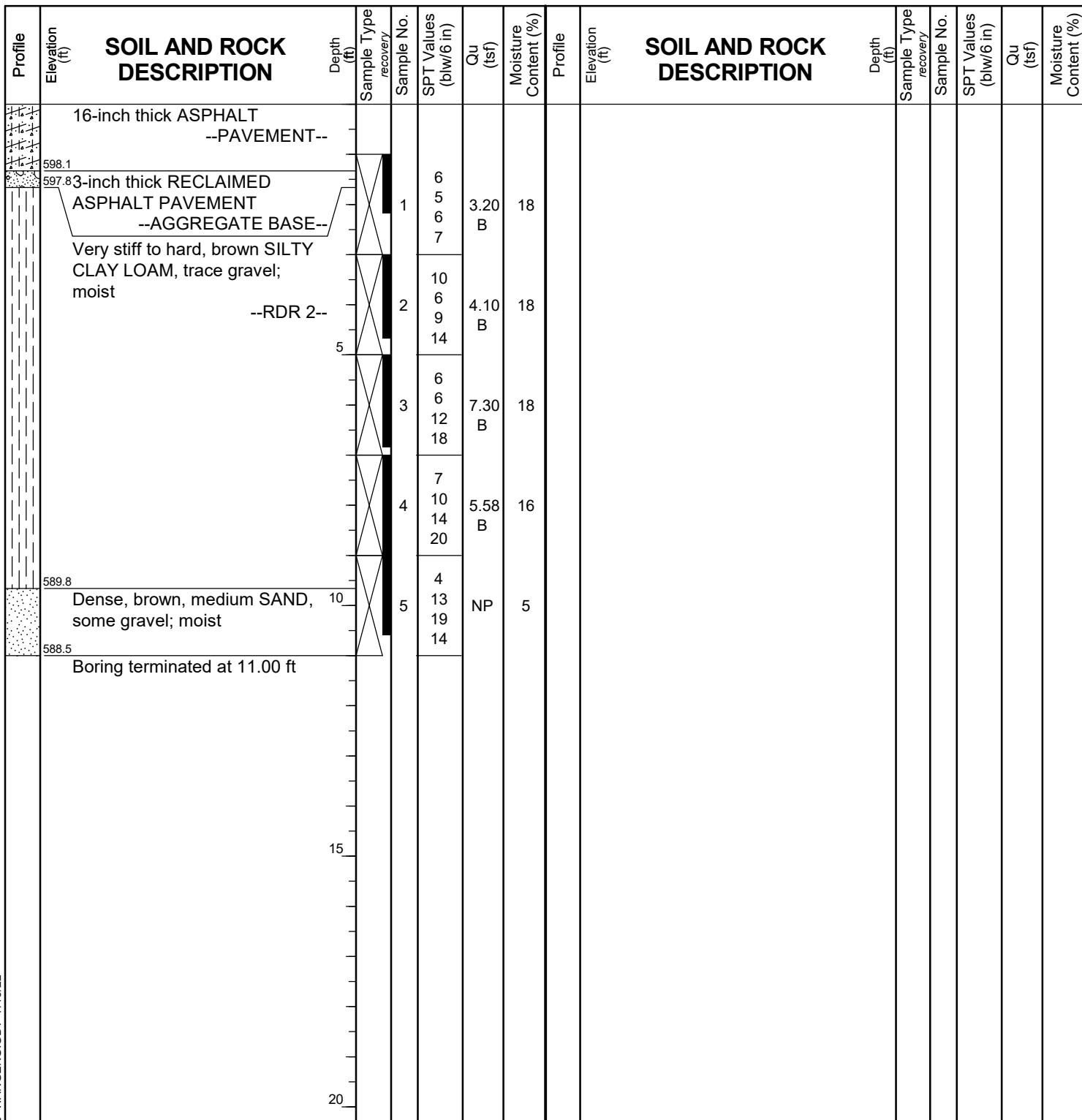
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client TranSystems Corporation
Project I-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 599.48 ft
North: 1760263.59 ft
East: 1032145.91 ft
Station: 481+53.59
Offset: 58.94 LT



WANGENG INC 79011501 GP. I WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-11-2022** Complete Drilling **05-11-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling DRY
At Completion of Drilling DRY
Time After Drilling NA
Depth to Water NA

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG WB-SGB-13

Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client

Project-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 596.67 ft
North: 1760713.59 ft
East: 1032553.93 ft
Station: 487+61.03
Offset: 58.14 LT

WANGENG INC 79011501 GP1 WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-10-2022** Complete Drilling **05-10-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling DRY
At Completion of Drilling DRY
Time After Drilling NA
Depth to Water NA

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG WB-SGB-14

Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

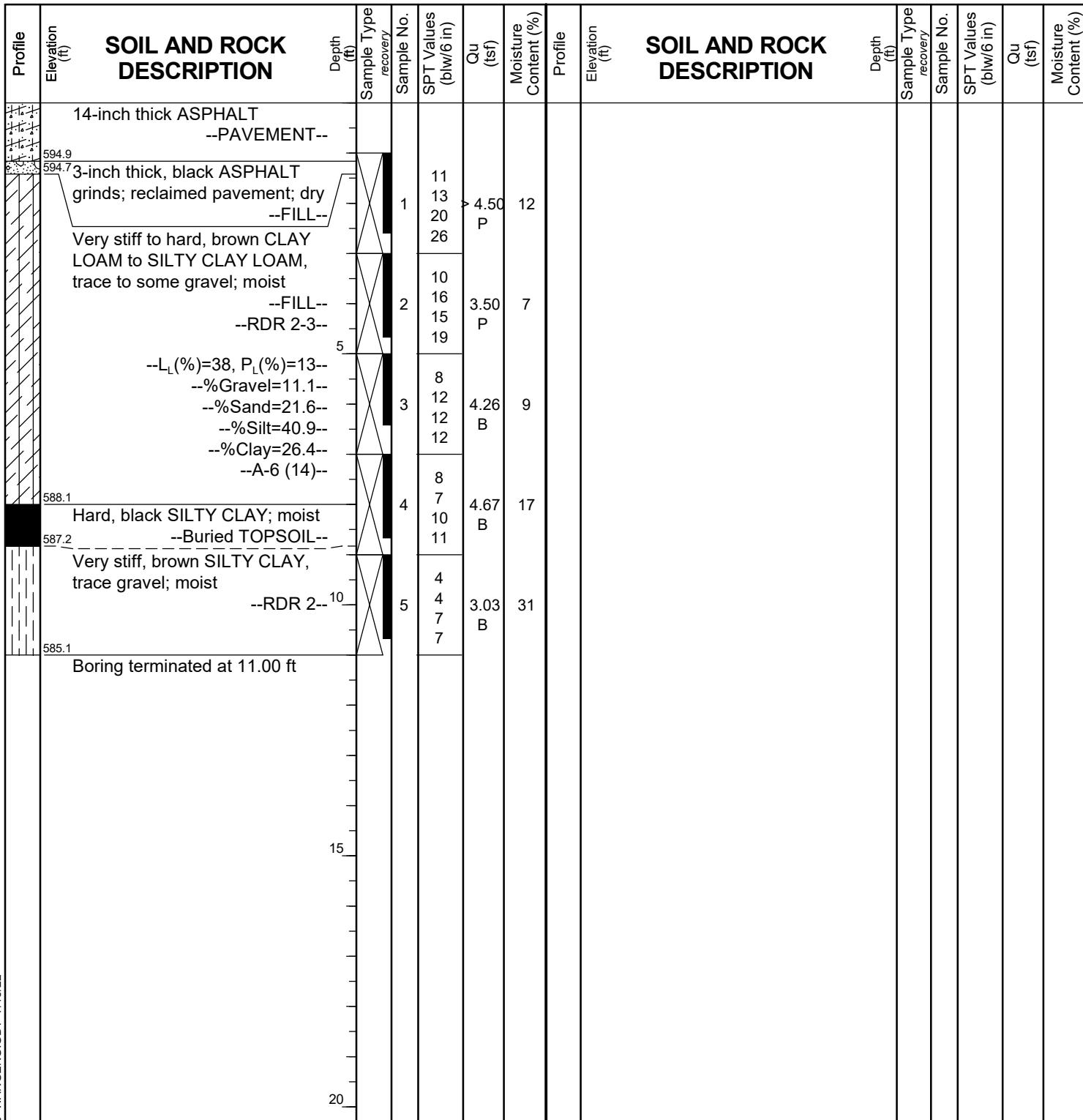
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 596.07 ft
North: 1761131.01 ft
East: 1032930.49 ft
Station: 493+23.20
Offset: 58.81 LT



GENERAL NOTES

Begin Drilling **05-10-2022** Complete Drilling **05-10-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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BORING LOG WB-SGB-15

Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

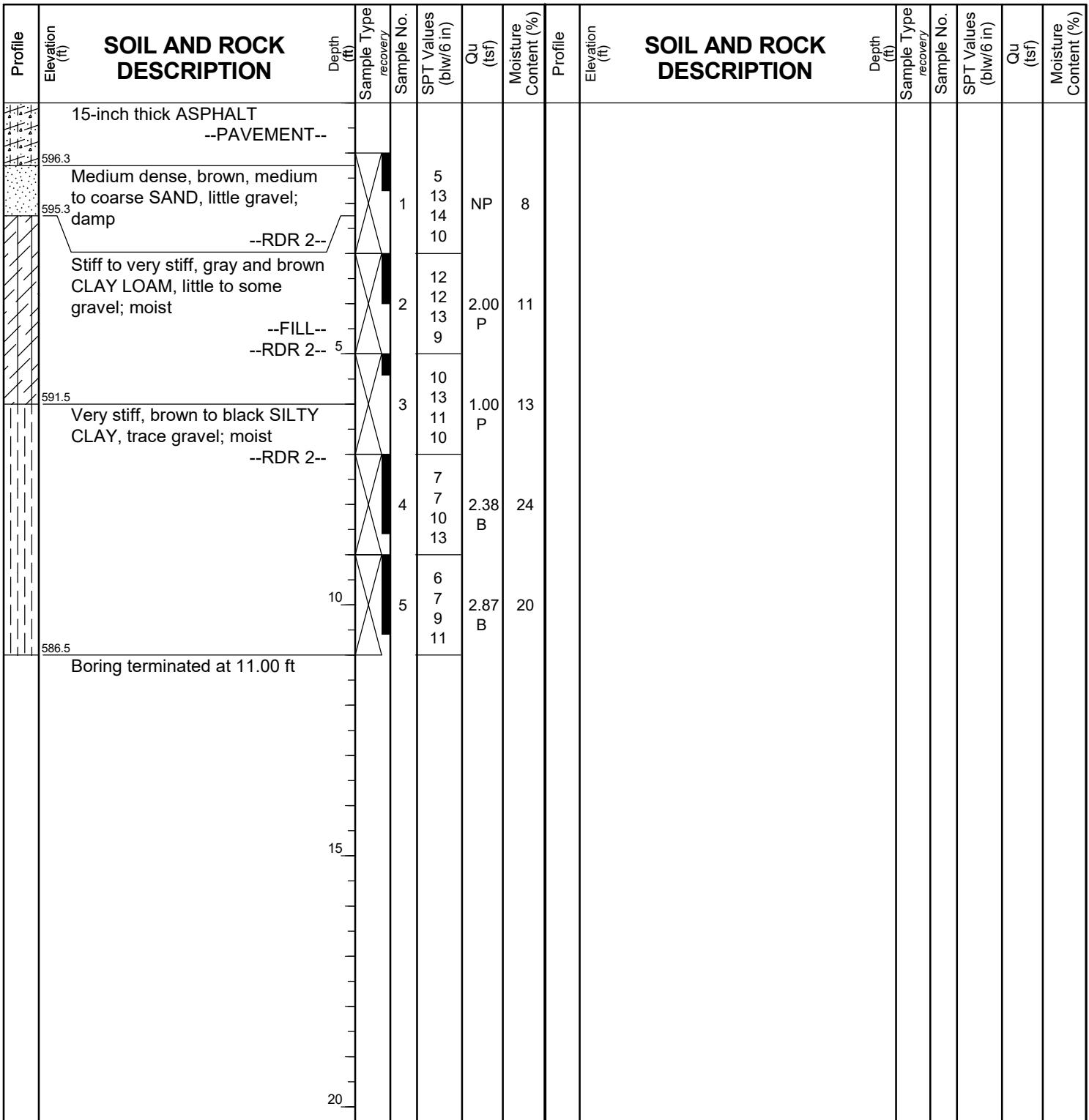
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 597.53 ft
North: 1761611.70 ft
East: 1033364.63 ft
Station: 499+70.92
Offset: 59.21 LT



WANGENG INC 79011501 GP1 WANGENG GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-10-2022** Complete Drilling **05-10-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.



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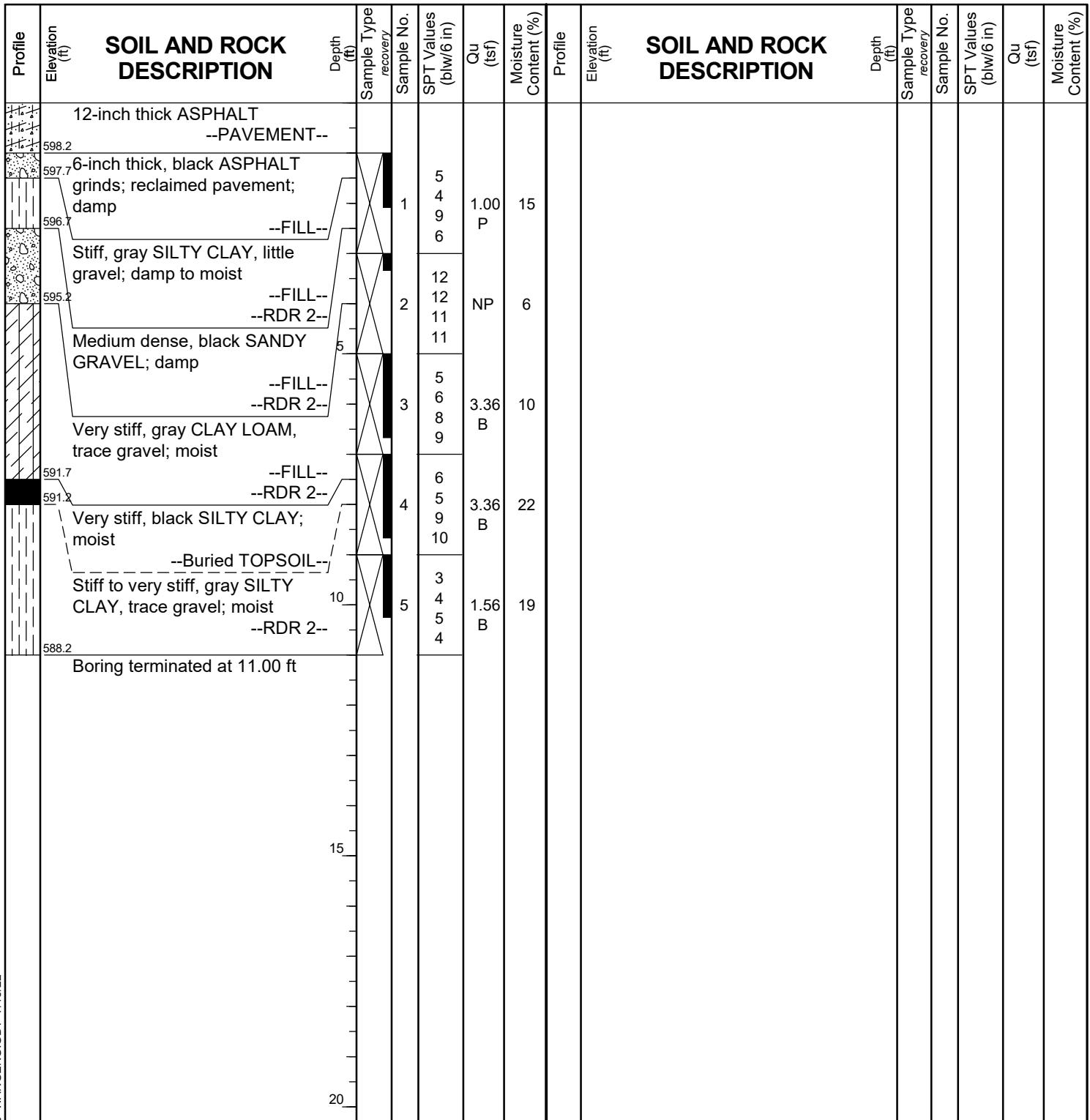
BORING LOG WB-SGB-16

WEI Job No.: 7901-15-01

TranSystems Corporation

Client TranSystems Corporation
Project I-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 599.24 ft
North: 1762035.24 ft
East: 1033762.51 ft
Station: 505+49.08
Offset: 59.51 LT



WANGENGINC 79011501.GPJ WANGENG.GDT 7/13/22

GENERAL NOTES

Begin Drilling **05-10-2022** Complete Drilling **05-10-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA, boring backfilled upon completion**

WATER LEVEL DATA

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling |  | NA |
| Depth to Water |  | NA |

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG WB-SGB-17

Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

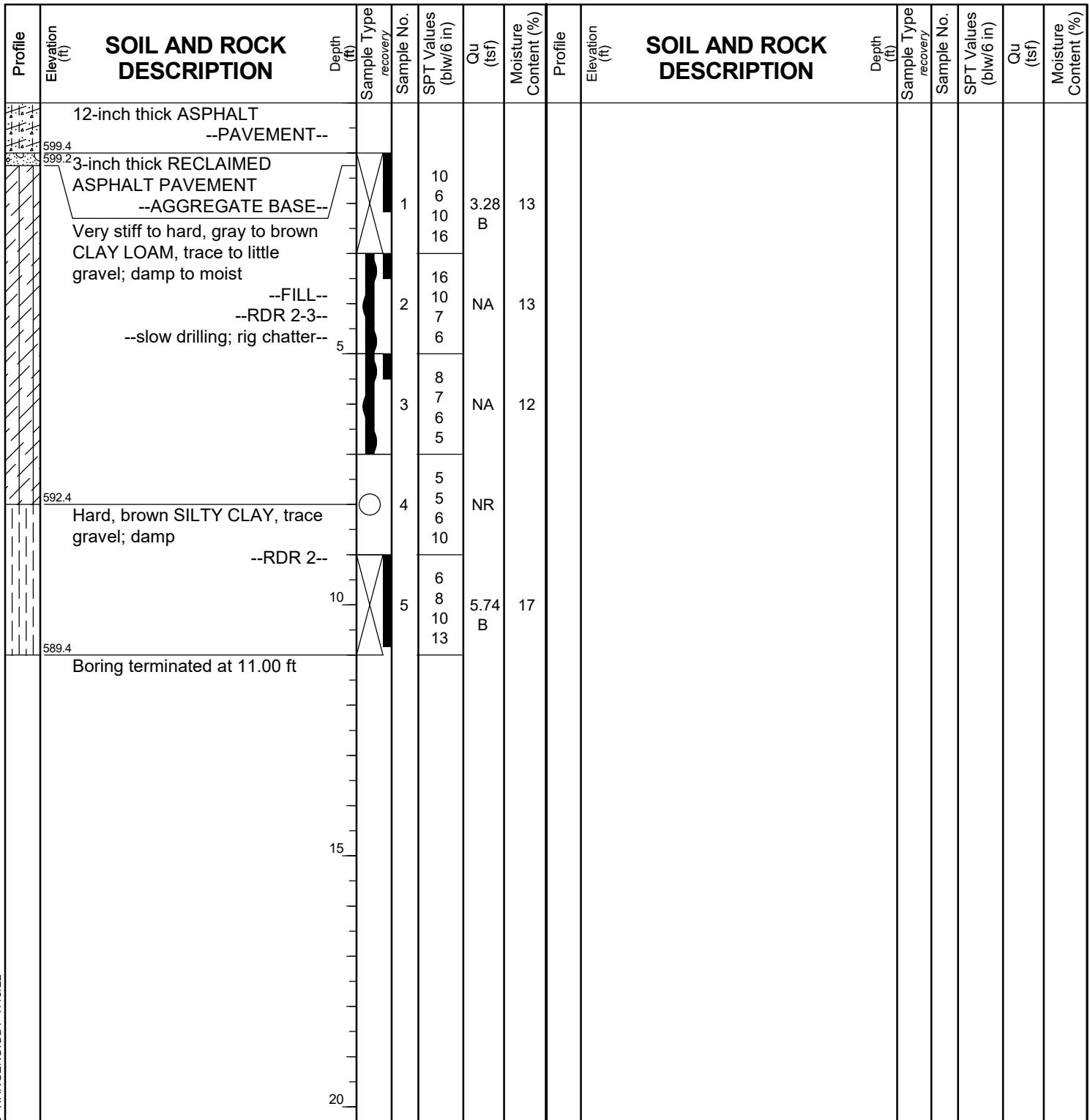
Client

Project I-80 Reconstruction (Houbolt Road to Center Street)

Location

Will County, Illinois

Datum: NAVD 88
Elevation: 600.44 ft
North: 1762454.74 ft
East: 1034213.48 ft
Station: 511+60.88
Offset: 59.74 LT



GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-10-2022** Complete Drilling **05-10-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

| | | |
|---------------------------|---|-----|
| While Drilling |  | DRY |
| At Completion of Drilling |  | DRY |
| Time After Drilling | NA | |
| Depth to Water |  | NA |



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BORING LOG WB-SGB-18

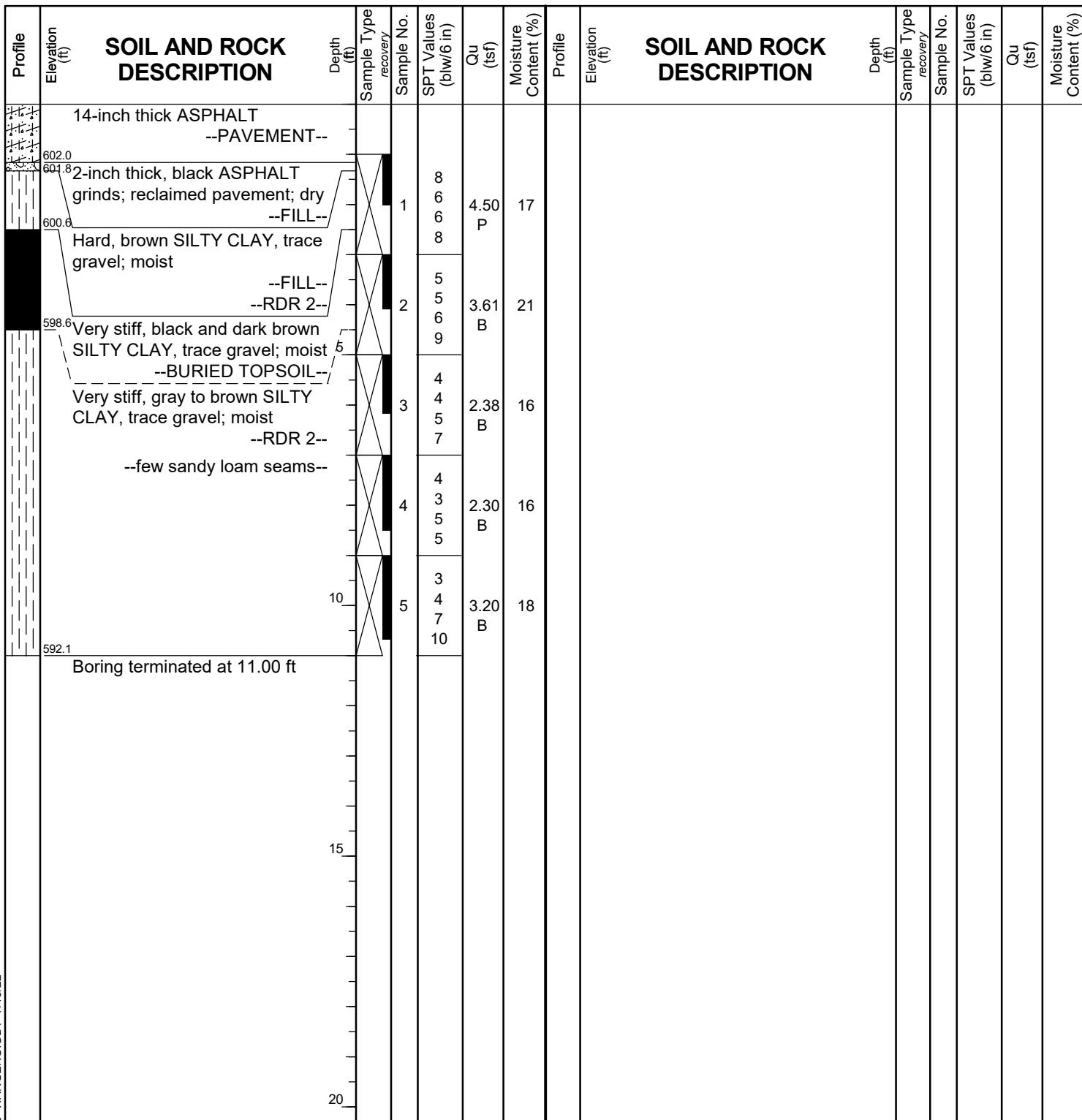
Page 1 of 1

WEI Job No.: 7901-15-01

TranSystems Corporation

Client
Project-80 Reconstruction (Houbolt Road to Center Street)
Location Will County, Illinois

Datum: NAVD 88
Elevation: 603.15 ft
North: 1762828.26 ft
East: 1034675.28 ft
Station: 517+50.83
Offset: 60.39 LT



GENERAL NOTES

Begin Drilling **05-10-2022** Complete Drilling **05-10-2022**
Drilling Contractor **Wang Testing Services** Drill Rig **20D50T [80%]**
Driller **JS&AE** Logger **A. Scifers** Checked by **J. Bensen**
Drilling Method **2.25" IDA HSA; boring backfilled upon completion**

WATER LEVEL DATA

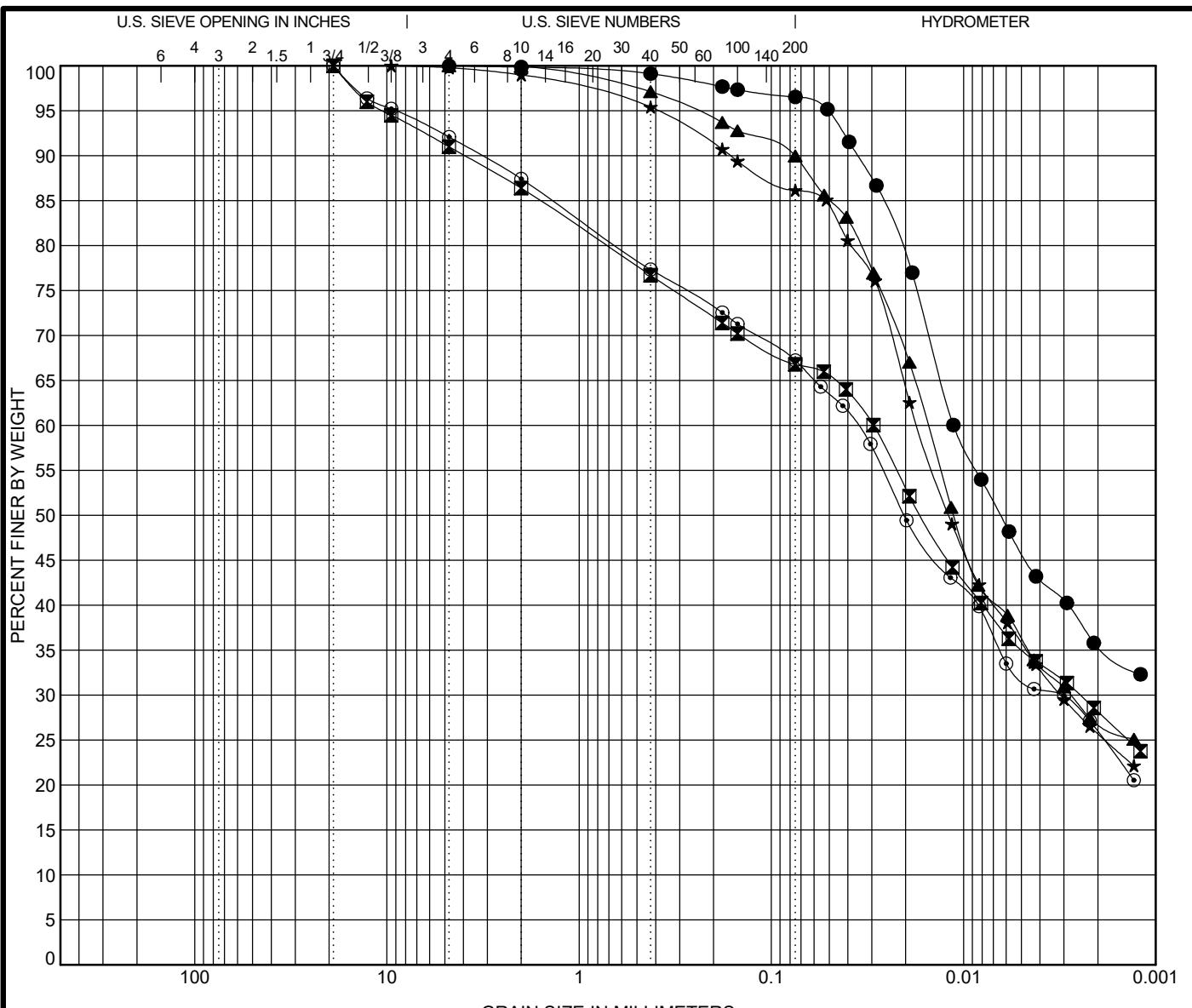
While Drilling **DRY**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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APPENDIX B



| COBBLES | GRAVEL | SAND | | SILT AND CLAY | | | |
|---------|--------|--------|------|---------------|----|----|----|
| | | coarse | fine | LL | PL | PI | Cc |

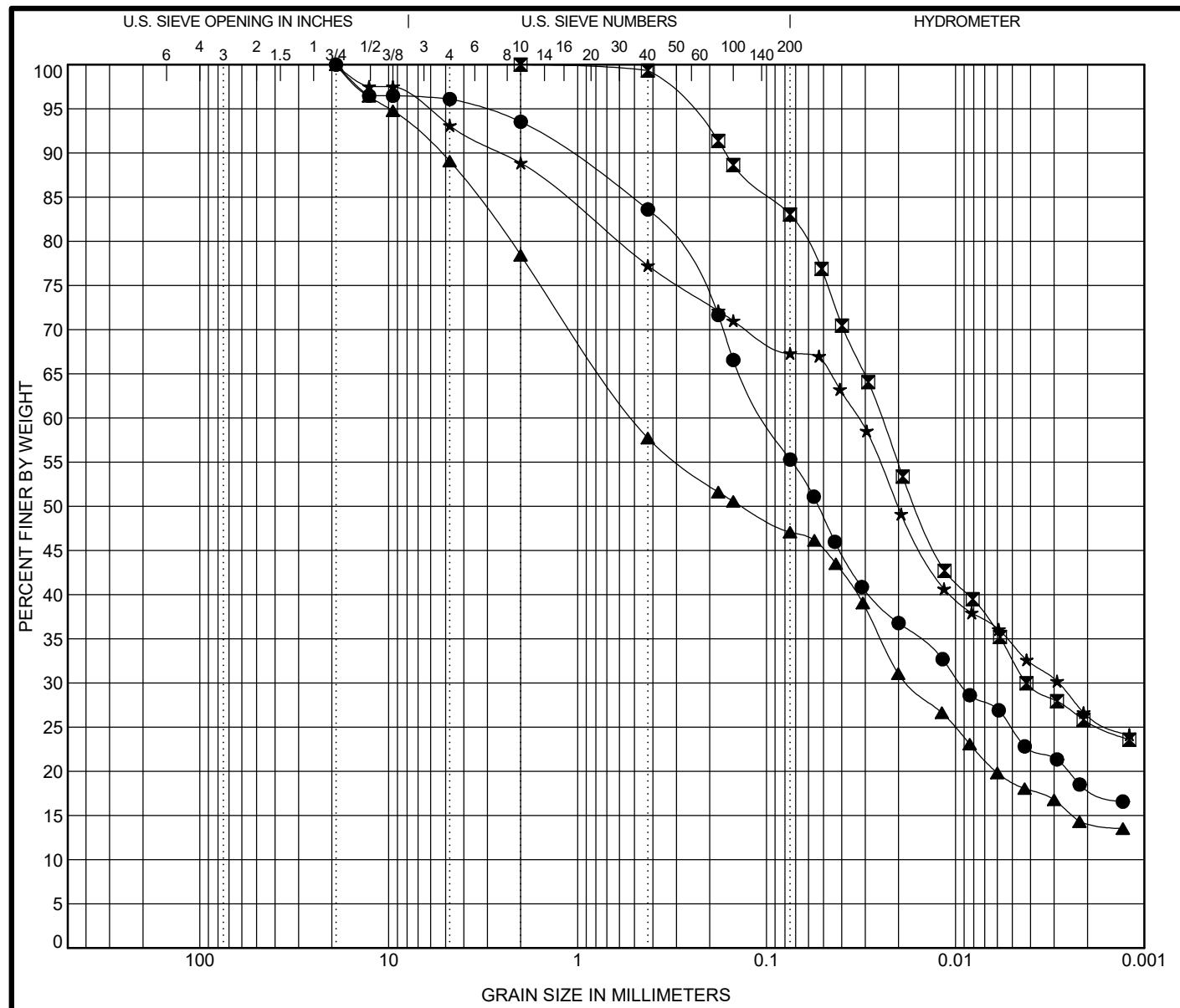
| Specimen Identification | | IDH Classification | | | | LL | PL | PI | Cc | Cu |
|-------------------------|--------------------|------------------------|--------------|--------------|-----|-------------|-------------|-------------|-------------|----|
| ● | CL-SGB-04#3 4.0 ft | Silty Clay | | | | 48 | 18 | 30 | | |
| ◻ | CL-SGB-08#1 0.0 ft | Clay | | | | 40 | 16 | 24 | | |
| ▲ | CL-SGB-12#2 2.0 ft | Silty Clay Loam | | | | 60 | 24 | 36 | | |
| ★ | CL-SGB-14#2 2.0 ft | Silty Clay Loam | | | | 50 | 24 | 26 | | |
| ◎ | EB-SGB-10#2 3.0 ft | Clay Loam | | | | 31 | 13 | 18 | | |
| Specimen Identification | | D100 | D60 | D30 | D10 | %Gravel | %Sand | %Silt | %Clay | |
| ● | CL-SGB-04#3 4.0 ft | 4.75 | 0.011 | | | 0.1 | 3.4 | 61.0 | 35.5 | |
| ◻ | CL-SGB-08#1 0.0 ft | 19 | 0.029 | 0.002 | | 13.6 | 19.6 | 38.6 | 28.2 | |
| ▲ | CL-SGB-12#2 2.0 ft | 4.75 | 0.015 | 0.003 | | 0.1 | 10.1 | 62.8 | 27.0 | |
| ★ | CL-SGB-14#2 2.0 ft | 9.5 | 0.017 | 0.003 | | 1.0 | 12.9 | 60.4 | 25.7 | |
| ◎ | EB-SGB-10#2 3.0 ft | 19 | 0.036 | 0.003 | | 12.6 | 20.3 | 41.2 | 25.9 | |



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GRAIN SIZE DISTRIBUTION

Project: I-80 Reconstruction (Houbolt Road to Center Street)
Location: Will County, Illinois
Number: 7901-15-01



| COBBLES | GRAVEL | SAND | | SILT AND CLAY | | | |
|---------|--------|--------|------|---------------|--|--|--|
| | | coarse | fine | | | | |

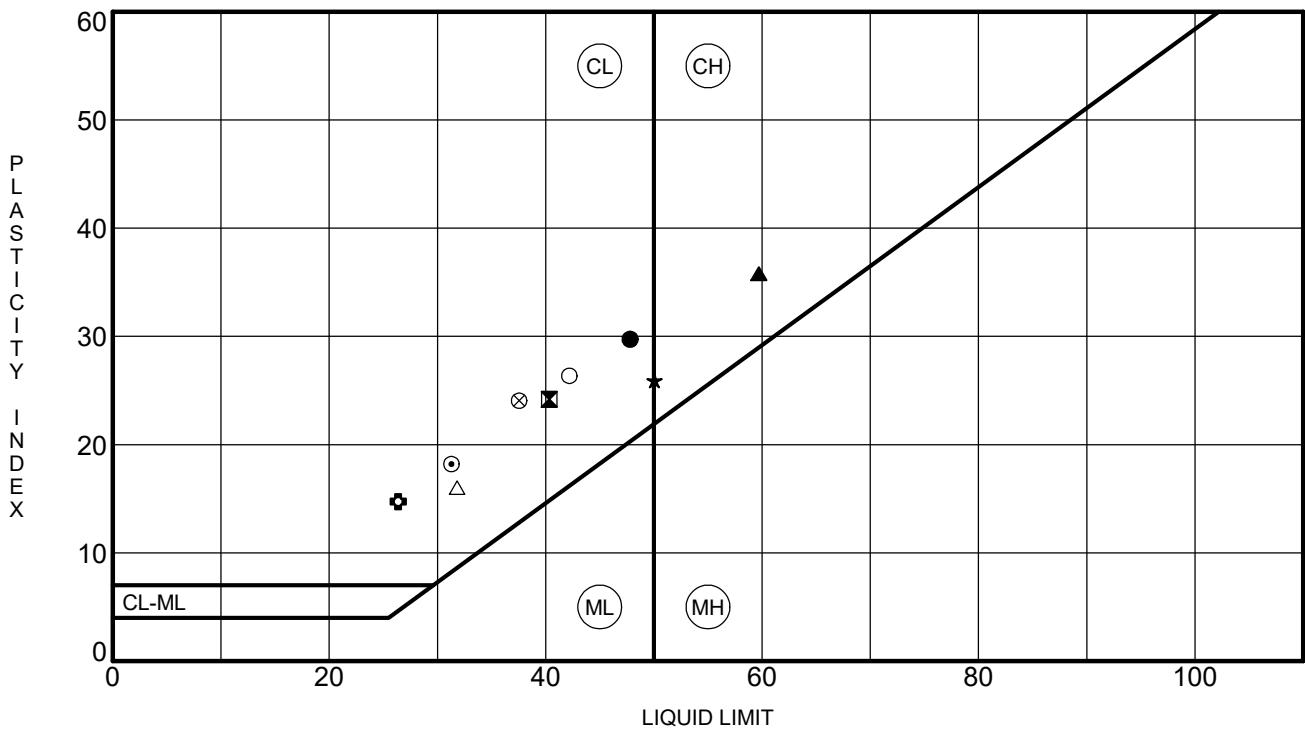
| Specimen Identification | | IDH Classification | | | | | LL | PL | PI | Cc | Cu |
|-------------------------|---------------------|------------------------|--------------|--------------|-----|-------------|-------------|-------------|-------------|----|----|
| ● | EB-SGB-17#2 3.0 ft | Loam | | | | | 26 | 12 | 14 | | |
| ■ | HR-BSB-01#7 16.0 ft | Silty Clay Loam | | | | | 42 | 16 | 26 | | |
| ▲ | WB-SGB-06#4 7.0 ft | Gravelly Loam | | | | | 32 | 16 | 16 | | |
| ★ | WB-SGB-14#3 5.0 ft | Clay Loam | | | | | 38 | 13 | 25 | | |
| Specimen Identification | | D100 | D60 | D30 | D10 | %Gravel | %Sand | %Silt | %Clay | | |
| ● | EB-SGB-17#2 3.0 ft | 19 | 0.1 | 0.009 | | 6.5 | 38.4 | 36.9 | 18.2 | | |
| ■ | HR-BSB-01#7 16.0 ft | 2 | 0.025 | 0.004 | | 0.0 | 17.2 | 57.2 | 25.6 | | |
| ▲ | WB-SGB-06#4 7.0 ft | 19 | 0.504 | 0.018 | | 21.6 | 31.4 | 32.9 | 14.2 | | |
| ★ | WB-SGB-14#3 5.0 ft | 19 | 0.033 | 0.003 | | 11.1 | 21.6 | 40.9 | 26.4 | | |



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GRAIN SIZE DISTRIBUTION

Project: I-80 Reconstruction (Houbolt Road to Center Street)
Location: Will County, Illinois
Number: 7901-15-01



| Specimen Identification | | LL | PL | PI | Fines | IDH Classification | |
|-------------------------|-------------|---------|----|----|-------|--------------------|-----------------|
| ● | CL-SGB-04#3 | 4.0 ft | 48 | 18 | 30 | 97 | Silty Clay |
| ■ | CL-SGB-08#1 | 0.0 ft | 40 | 16 | 24 | 67 | Clay |
| ▲ | CL-SGB-12#2 | 2.0 ft | 60 | 24 | 36 | 90 | Silty Clay Loam |
| ★ | CL-SGB-14#2 | 2.0 ft | 50 | 24 | 26 | 86 | Silty Clay Loam |
| ○ | EB-SGB-10#2 | 3.0 ft | 31 | 13 | 18 | 67 | Clay Loam |
| ◆ | EB-SGB-17#2 | 3.0 ft | 26 | 12 | 14 | 55 | Loam |
| ○ | HR-BSB-01#7 | 16.0 ft | 42 | 16 | 26 | 83 | Silty Clay Loam |
| △ | WB-SGB-06#4 | 7.0 ft | 32 | 16 | 16 | 47 | Gravelly Loam |
| ⊗ | WB-SGB-14#3 | 5.0 ft | 38 | 13 | 25 | 67 | Clay Loam |

ATTERBERG LIMITS' RESULTS

Project: I-80 Reconstruction (Houbolt Road to Center Street)
Location: Will County, Illinois
Number: 7901-15-01

ORGANIC CONTENT in SOILS by LOSS on IGNITION
ASTM D 2974, Method C

Client: Transystems
Project: I-80
WEI Job: 7901-15-01
Type/Condition: SS
Testing Furnace Temp °C.: 440

Analyst Name: M. Ciapas
Date Received: Various
Date Tested: 7/7/2022

| Sample No./Depth | CL-SGB-04 SS#2 (2-4ft.) | CL-SGB-11 SS#2 (2-4ft.) | CL-SGB-19 SS#4 (6-8ft.) | CL-SGB-19 SS#1 (0-2ft.) | |
|----------------------------|----------------------------|----------------------------|----------------------------|----------------------------|--|
| | | | | | |
| Wet Soil + Tare | 70.3 | 83.3 | 76.77 | 86.82 | |
| Dry Soil + Tare | 64.02 | 75.16 | 69.96 | 78.18 | |
| Tare Mass | 42.55 | 43.73 | 42.61 | 43.71 | |
| w (%) | 29 | 26 | 25 | 25 | |
| Dry Soil + Tare | 64.02 | 75.16 | 69.96 | 78.18 | |
| Ash+ Tare | 61.87 | 73.26 | 68.47 | 76.35 | |
| Tare Mass | 42.55 | 43.73 | 42.61 | 43.71 | |
| Ash Content (%) | 90 | 94 | 95 | 95 | |
| Organic Content (%) | 10.0 | 6.0 | 5.4 | 5.3 | |

Prepared By: _____

Reviewed By: _____



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APPENDIX C



**Illinois Department
of Transportation**

**Summary Report on Pavement,
Base and Subbase Design**

State Job Number: 7901-15-01 Project: I-80 Reconstruction Route: I-80

Section: _____ City or County: Will Date: 07/13/2022

ADT: _____ Year: _____ Design Period: _____ Class Highway: _____

Passenger Cars Per Day: _____ Trucks S.U. Per Day: _____ Trucks M.U. Per Day: _____

Pavement Structure: _____

Type Surface Course: _____ Thickness: _____

Type Base Course: _____ Thickness: _____

Type Subbase Material: _____ Thickness: _____

| Sta. to Sta. | 410+00 to 518+00 | + to + | + to + | + to + |
|---|------------------|--------|--------|--------|
| *Sta. of Test | 483+70.39 | | | |
| *Drainage Class | Poor | | | |
| *Ave. Frost Penetration | 45 to 60 in. | | | |
| Illinois Textural Classification | Silty Clay Loam | | | |
| Classification and Group Index (AASHTO M 145) | A-7-6 (36) | | | |
| *Percent Silt (AASHTO T 88) | 62.8 | | | |
| *Illinois Bearing Ratio (%) | | | | |
| Std. Dry Density (IL Mod. AASHTO T 99) | | | | |
| Optimum Moisture (IL Mod AASHTO T 99) | | | | |

* Indicates worst condition within the above station limits.

Remarks: _____

SOIL TEST DATA

| | | ROUTE | PROJECT | | | |
|---|--|---------------------|---------------------------|-----------------|-----------------|----------------|
| | | I-80 Reconstruction | 7901-15-01/ KE225089 I 80 | | | |
| SECTION | | | | | | COUNTY |
| I-80 (Sta. 410+00 to Sta. 518+00) | | | | | | Will |
| Lab. No. | | CL-SGB-04 No.3 | CL-SGB-08 No.1 | CL-SGB-12 No.2 | CL-SGB-14 No.2 | EB-SGB-10 No.2 |
| Station ft) | | 435+66.34 | 459+58.10 | 483+70.39 | 501+68.17 | 473+50.74 |
| Offset (ft) | | 5.27 RT | 3.37 RT | 1.84 LT | 13.47 RT | 66.52 RT |
| Depth (ft) | | 4.0 | 0.0 | 2.0 | 2.0 | 3.0 |
| AASHTO M 145 | | A-7-6 (31) | A-6 (14) | A-7-6 (36) | A-7-6 (24) | A-6 (9) |
| Classification and Group Index | | A-6 (4) | | | | |
| Illinois Textural Classification (Illinois Method) | | Silty Clay | Clay | Silty Clay Loam | Silty Clay Loam | Clay Loam |
| Gradation--Passing 1" Sieve % | | | | | | Loam |
| --" 3/4" Sieve % | | | 100.0 | | 100.0 | 100 |
| --" 1/2" Sieve % | | | 96.0 | | 96.4 | 96.5 |
| --" No.4 Sieve % | | 100.0 | 91.0 | 100.0 | 99.8 | 92.1 |
| --" No.10 Sieve % | | 99.9 | 86.4 | 99.9 | 99.0 | 87.4 |
| --" No.40 Sieve % | | 99.1 | 76.7 | 97.1 | 95.4 | 77.3 |
| --" No.100 Sieve % | | 97.3 | 70.2 | 92.8 | 89.4 | 71.3 |
| --" No.200 Sieve % | | 96.5 | 66.7 | 89.8 | 86.1 | 67.1 |
| Sand % (AASHTO T 88) | | 3.4 | 19.6 | 10.1 | 12.9 | 20.3 |
| Silt % (AASHTO T 88) | | 61.0 | 38.6 | 62.8 | 60.4 | 41.2 |
| Clay % (AASHTO T 88) | | 35.5 | 28.2 | 27.0 | 25.7 | 25.9 |
| Liquid limit % (AASHTO T 89) | | 48 | 40 | 60 | 50 | 31 |
| Plasticity index % (AASHTO T 90) | | 30 | 24 | 36 | 26 | 18 |
| IBR % (Illinois Method) | | | | | | 15 |
| Standard Dry Density % (AASHTO T 99) | | | | | | |
| Optimum Moisture % (AASHTO T 99) | | | | | | |
| Subgrade Support Rating | | FAIR | POOR | POOR | POOR | POOR |
| Insitu Moisture % (AASHTO T 99) | | 29 | 15 | 42 | 75 | 8 |
| | | | | | | 24 |

SOIL TEST DATA**SECTION**

I-80 (Sta. 410+00 to Sta. 518+00)

| Lab. No. | WB-SGB-06 No.4 | WB-SGB-14 No.3 |
|---|----------------|----------------|
| Station ft) | 445+98.21 | 493+23.20 |
| Offset (ft) | 59.14 LT | 58.81 LT |
| Depth (ft) | 7 | 5.0 |
| AASHTO M 145 | | |
| Classification and Group Index | A-6 (4) | A-6 (14) |
| Illinois Textural Classification (Illinois Method) | Gravelly Loam | Clay Loam |
| Gradation--Passing 1" Sieve % | | |
| --"-- 3/4" Sieve % | 100 | 100.0 |
| --"-- 1/2" Sieve % | 96.4 | 97.5 |
| --"-- No.4 Sieve % | 89.1 | 93.1 |
| --"-- No.10 Sieve % | 78.4 | 88.9 |
| --"-- No.40 Sieve % | 57.7 | 77.3 |
| --"-- No.100 Sieve % | 50.6 | 71.0 |
| --"-- No.200 Sieve % | 47 | 67.3 |
| Sand % (AASHTO T 88) | 31.4 | 21.6 |
| Silt % (AASHTO T 88) | 32.9 | 40.9 |
| Clay % (AASHTO T 88) | 14.2 | 26.4 |
| Liquid limit % (AASHTO T 89) | 32 | 38 |
| Plasticity index % (AASHTO T 90) | 16 | 24 |
| IBR % (Illinois Method) | | |
| Standard Dry Density % (AASHTO T 99) | | |
| Optimum Moisture % (AASHTO T 99) | | |
| Subgrade Support Rating | POOR | POOR |
| Insitu Moisture % (AASHTO T 99) | 12 | 9 |



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APPENDIX D

Pavement Composition - Shoulder
I-80 West Mainline
Station 410+00 to Station 518+00

| | Station | Offset | Asphalt (in) | Concrete (in) | Basecourse Type / Thickness (in) | Topsoil Type | Topsoil Thickness (in) | Edge of Pavement (ft) |
|-----------|-----------|--------|--------------|---------------|----------------------------------|----------------------|------------------------|-----------------------|
| EB-SGB-01 | 419+89.08 | 68.7 | 9 | - | Gravel / 3 | Black Sandy Loam | 4 | 1 |
| EB-SGB-02 | 425+81.19 | 65.1 | 9 | - | Gravel / 3 | Black Sandy Loam | 4 | 1 |
| EB-SGB-03 | 431+77.65 | 85.8 | - | 10 | Gravel / 2 | Black Sandy Loam | 4 | 1 |
| EB-SGB-04 | 437+68.62 | 65.2 | - | 11 | Gravel / 1 | Black Sandy Loam | 4 | 1 |
| EB-SGB-05 | 442+87.88 | 67.5 | - | 10 | Gravel / 2 | Black Sandy Loam | 4 | 1.3 |
| EB-SGB-06 | 449+48.31 | 69.6 | - | 12 | Gravel / 2 | Black Sandy Loam | 3 | 1 |
| EB-SGB-07 | 455+52.23 | 94.1 | - | - | - | Black Sandy Loam | 3 | 1 |
| EB-SGB-08 | 461+56.7 | 77.4 | - | 11 | Gravel / 2 | Black Sandy Loam | 4 | 1.3 |
| EB-SGB-09 | 467+60.81 | 69.8 | 10 | - | Gravel / 2 | Black Sandy Loam | 7 | 1.3 |
| EB-SGB-10 | 473+50.74 | 66.5 | 10 | - | Gravel / 2 | Black Sandy Loam | 7 | 10 |
| EB-SGB-11 | 479+45.03 | 65.7 | 8 | - | Gravel / 4 | Black Sandy Loam | 6 | 10 |
| EB-SGB-12 | 485+48.8 | 68.0 | 8 | - | Gravel / 4 | Black Sandy Loam | 4 | 10 |
| EB-SGB-13 | 491+65.16 | 66.4 | 9 | - | Gravel / 3 | Black Sandy Loam | 5 | 10 |
| EB-SGB-14 | 497+46.98 | 66.1 | 8 | - | Gravel / 4 | Black Sandy Loam | 5 | 10 |
| EB-SGB-15 | 503+48.24 | 65.2 | 10 | - | Gravel / 2 | Black Sandy Loam | 9 | 10 |
| EB-SGB-16 | 509+52.41 | 66.1 | 10 | - | Gravel / 2 | Black Sandy Loam | 6 | 10 |
| EB-SGB-17 | 515+61.7 | 70.1 | 9 | - | Gravel / 3 | Black Sandy Loam | 6 | 10 |
| CL-SGB-01 | 417+60.1 | 5.9 | - | - | - | Black Silty Clay | 12 | - |
| CL-SGB-02 | 423+56.39 | 13.1 | - | - | - | Black Silty Clay | 35 | - |
| CL-SGB-03 | 429+58.75 | 7.8 | - | - | - | Black Silty Clay | 10 | - |
| CL-SGB-04 | 435+66.34 | 5.3 | - | - | - | Black Silty Clay | 51 | - |
| CL-SGB-05 | 441+57.74 | 2.0 | - | - | - | Dark gray Silty Clay | 2 | - |
| CL-SGB-06 | 447+57.88 | 3.5 | - | - | - | Black Silty Clay | 8 | - |
| CL-SGB-07 | 453+75.64 | 2.3 | - | - | - | Black Silty Clay | 9 | - |
| CL-SGB-08 | 459+58.1 | 3.4 | - | - | - | Black Silty Clay | 4 | - |
| CL-SGB-09 | 465+61.97 | -2.7 | - | - | - | Black Silty Clay | 6 | - |
| CL-SGB-10 | 471+67.33 | 2.3 | - | - | - | Black Silty Clay | 6 | - |
| CL-SGB-11 | 477+63.98 | 7.8 | - | - | - | Black Silty Clay | 6 | - |
| CL-SGB-12 | 483+70.39 | -1.8 | - | - | - | Black Silty Clay | 6 | - |
| CL-SGB-13 | 489+64.45 | 5.8 | - | - | - | Black Silty Clay | 4 | - |
| CL-SGB-14 | 501+68.17 | 13.5 | - | - | - | Black Silty Clay | 6 | - |
| CL-SGB-15 | 507+53.48 | 14.7 | - | - | - | Black Sandy Loam | 1 | - |
| CL-SGB-16 | 513+65.23 | 4.6 | - | - | - | Black Silty Clay | 7 | - |
| WB-SGB-01 | 415+30.58 | -57.1 | 3 | - | Sandy gravel / 9 | Not Measured | - | - |
| WB-SGB-02 | 421+54.7 | -61.1 | - | 12 | Gravelly sand / 21 | Not Measured | - | - |
| WB-SGB-03 | 427+34.58 | -70.6 | - | 12 | Gravel / 5 | Black Sandy Loam | 7 | 10 |
| WB-SGB-04 | 433+47.63 | -58.3 | - | 12 | Gravel / 6 | Black Sandy Loam | 8 | 10 |
| WB-SGB-05 | 439+57.92 | -59.7 | - | 10 | Gravel / 7 | Black Sandy Loam | 6 | 10 |
| WB-SGB-06 | 445+98.21 | -59.1 | - | 16 | Gravel / 4 | Black Sandy Loam | 6 | 10 |
| WB-SGB-07 | 451+69.82 | -59.4 | - | 12 | Gravel / 4 | Black Sandy Loam | 7 | 10 |
| WB-SGB-08 | 457+60.75 | -85.0 | - | 10 | Gravel / 4 | Black Sandy Loam | 5 | 10 |
| WB-SGB-09 | 463+63.58 | -58.3 | 16 | - | - | Black Sandy Loam | 4 | 10 |
| WB-SGB-10 | 469+64.36 | -60.6 | 14 | - | RAP / 2 | Black Sandy Loam | 5 | 10 |
| WB-SGB-11 | 475+54.12 | -59.3 | 14 | - | RAP / 5 | Black Sandy Loam | 6 | 10 |
| WB-SGB-12 | 481+53.59 | -58.9 | 16 | - | RAP / 3 | Black Sandy Loam | 7 | 10 |
| WB-SGB-13 | 487+61.03 | -58.1 | 14 | - | RAP / 3 | Black Sandy Loam | 5 | 10 |
| WB-SGB-14 | 493+23.2 | -58.8 | 14 | - | RAP / 3 | Black Sandy Loam | 7 | 10 |
| WB-SGB-15 | 499+70.92 | -59.2 | 15 | - | - | Black Sandy Loam | 7 | 10 |
| WB-SGB-16 | 505+49.08 | -59.5 | 12 | - | Gravel / 6 | Black Sandy Loam | 6 | 10 |
| WB-SGB-17 | 511+60.88 | -59.7 | 12 | - | RAP / 3 | Black Sandy Loam | 6 | 10 |
| WB-SGB-18 | 517+50.83 | -60.4 | 14 | - | RAP / 2 | Black Sandy Loam | 6 | 10 |

Pavement Composition - Mainline Lanes
I-80 West Mainline
Station 410+00 to Station 518+00

| | Station | Offset | Asphalt (in) | Concrete (in) | Total Thickness (in) | Basecourse Type / Thickness (in) |
|----------|-----------|--------|--------------|---------------|----------------------|----------------------------------|
| PC-EB-01 | 436+39.10 | 58.37 | - | 13.375 | 13.375 | Gravel |
| PC-EB-02 | 462+85.52 | 38.87 | - | 13.5 | 13.5 | RAP |
| PC-EB-03 | 489+23.88 | 55.08 | 4 | 8 | 12 | Gravel |
| PC-EB-04 | 515+61.42 | 37.03 | 4 | 10 | 14 | Gravel |
| PC-WB-02 | 449+51.9 | -52.68 | - | 12.75 | 12.75 | Gravel |
| PC-WB-03 | 475+98.01 | -32.63 | 4 | 8.25 | 12.25 | RAP |
| PC-WB-04 | 501+97.71 | -52.75 | 4 | 8.25 | 12.25 | Gravel |



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APPENDIX E



PC-EB-01

Core Length: 13.50 in

Asphalt: --

Concrete: 13.50 in

Base Course: Gravel



PC-EB-02

Core Length: 13.50 in

Asphalt: --

Concrete: 13.50 in

Base Course: RAP

PAVEMENT CORES: I-80 RECONSTRUCTION; WEST MAINLINE FROM STATION 410+00 TO STATION 518+00, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

APPENDIX E-1

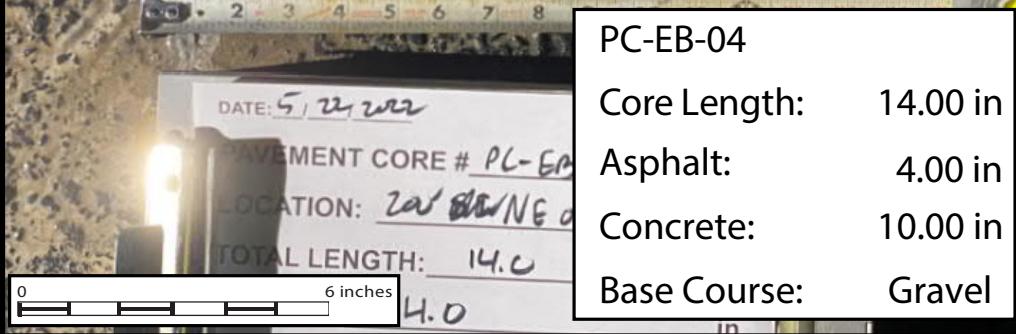
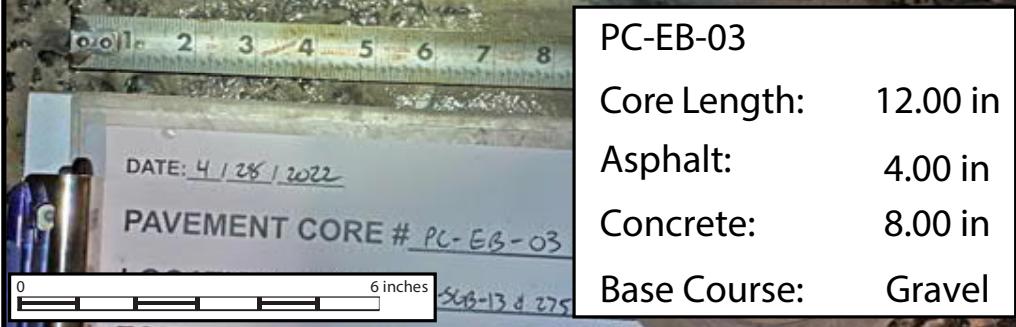
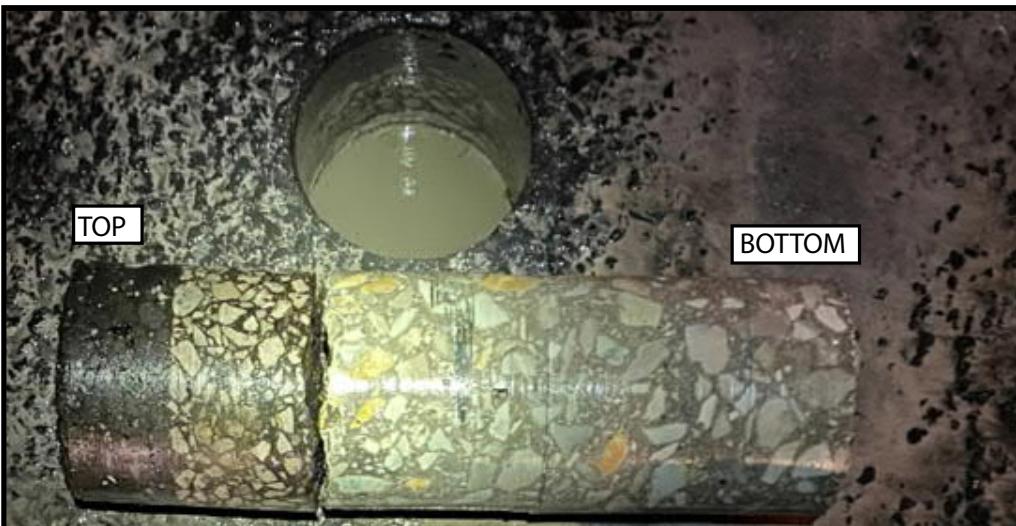
DRAWN BY: J. Bensen
CHECKED BY: A. Kurnia



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7901-15-01



PAVEMENT CORES: I-80 RECONSTRUCTION ; WEST MAINLINE FROM STATION 410+00 TO STATION 518+00, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

APPENDIX E-2

DRAWN BY: J. Bensen

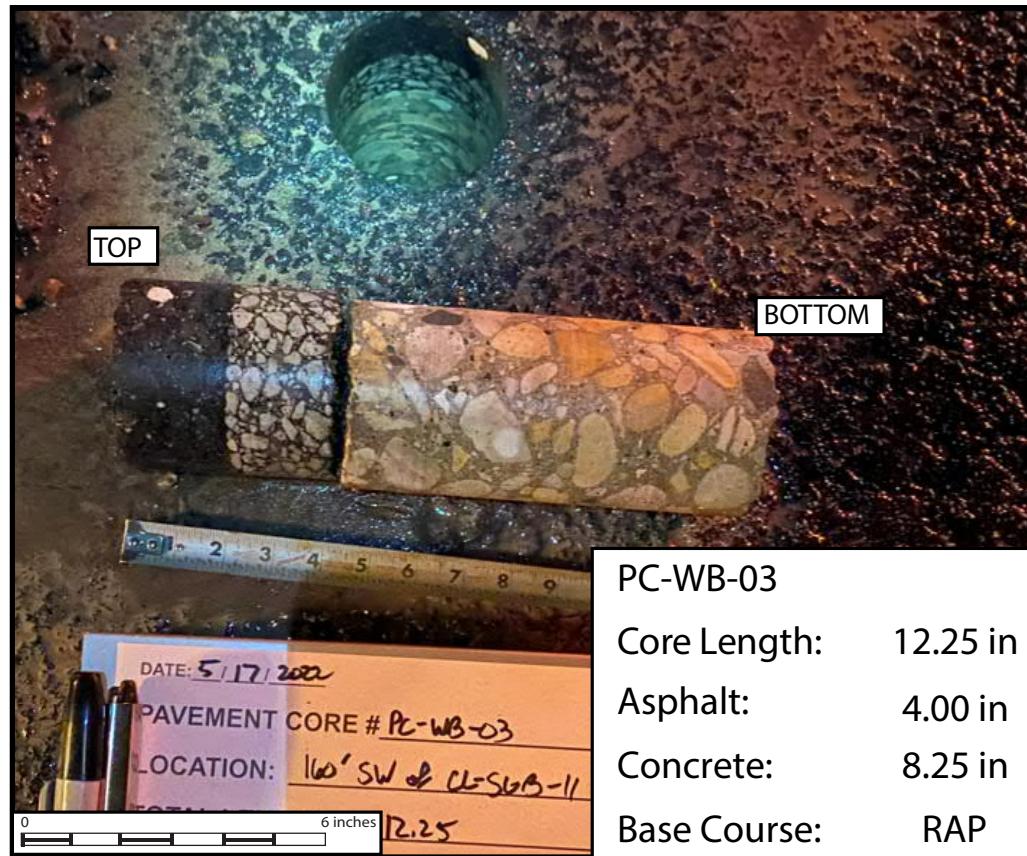
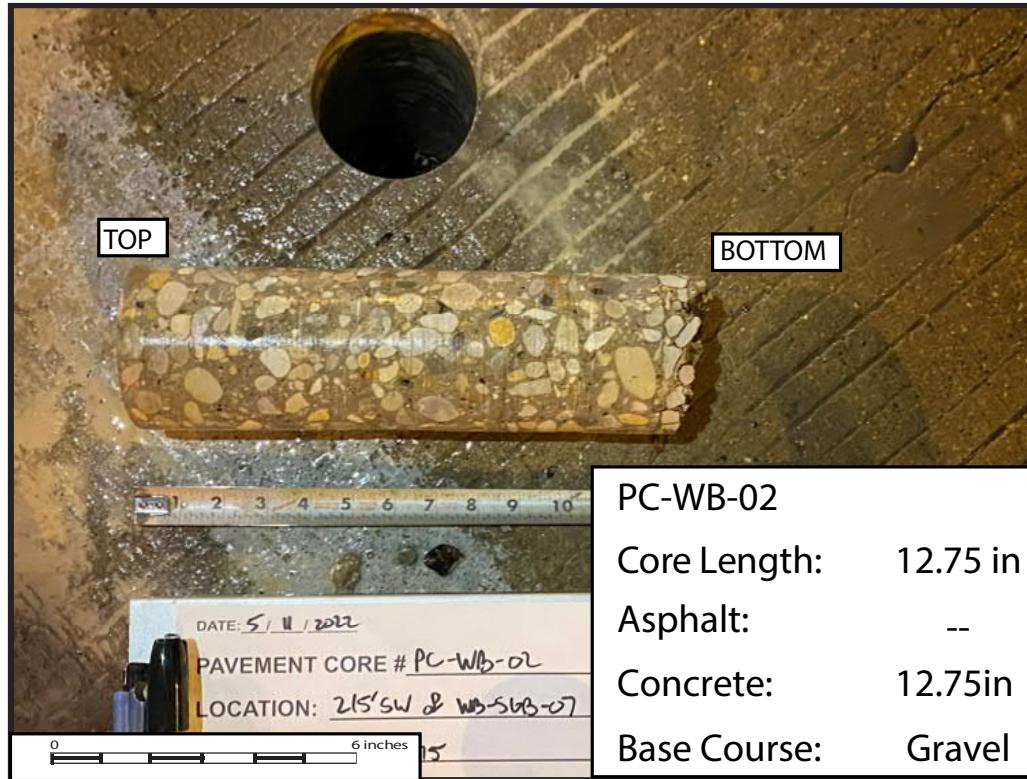
CHECKED BY: A. Kurnia



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7901-15-01



PAVEMENT CORES: I-80 RECONSTRUCTION; WEST MAINLINE FROM STATION 410+00 TO STATION 518+00, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

APPENDIX E-3

DRAWN BY: J. Bensen

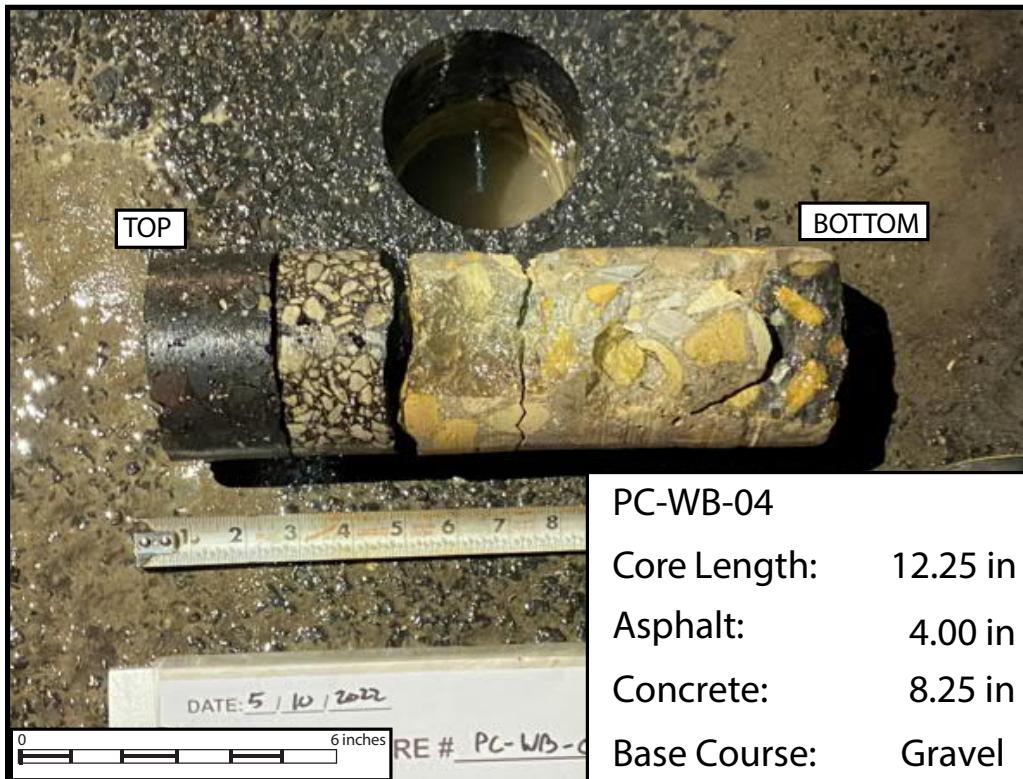
CHECKED BY: A. Kurnia



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7901-15-01



PC-WB-04

Core Length: 12.25 in

Asphalt: 4.00 in

Concrete: 8.25 in

Base Course: Gravel

PAVEMENT CORES: I-80 RECONSTRUCTION FROM HOBOLT RD TO WEST OF
CENTER ST & LARKIN AVE; CONTRACT D-91-207-19, PTB 194/11, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

APPENDIX E-4

DRAWN BY: J. Bensen

CHECKED BY: A. Kurnia



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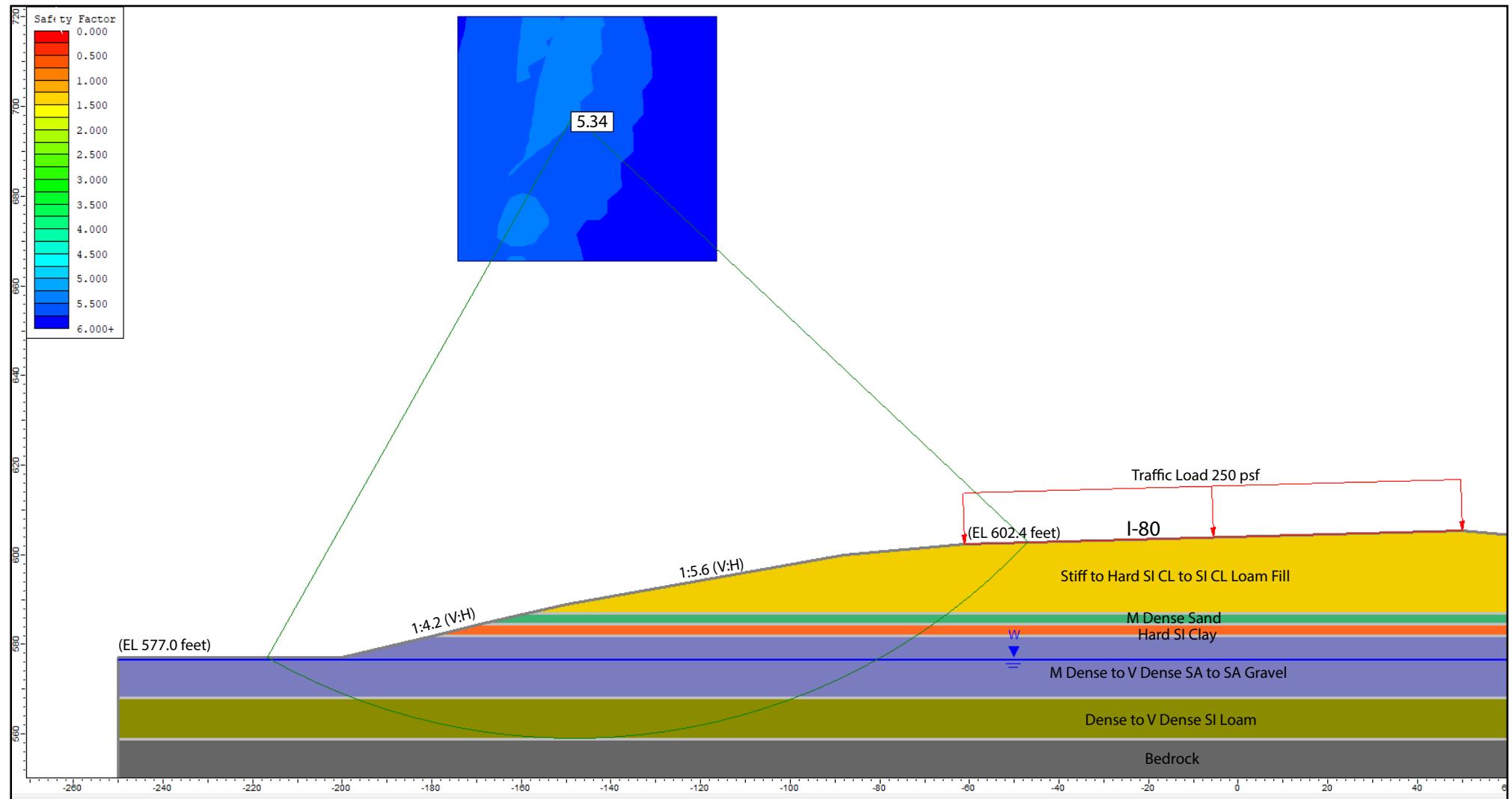
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7901-15-01



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APPENDIX F



Undrained Analysis, Mainline-West 62R27, Station 443+00, Ref Boring: HR-BSB-01

| Layer ID | Description | Total Unit Weight (pcf) | Undrained Cohesion (psf) | Undrained Friction Angle (degrees) |
|----------|--|-------------------------|--------------------------|------------------------------------|
| 1 | Stiff to Hard SI CL to SI CL Loam Fill | 120 | 3500 | 0 |
| 2 | M Dense Sand | 115 | 0 | 30 |
| 3 | Hard SI Clay | 125 | 5000 | 0 |
| 4 | M Dense to V Dense SA to SA Gravel | 120 | 0 | 31 |
| 5 | Dense to V Dense SI Loam | 125 | 0 | 32 |
| 6 | Bedrock | 150 | -- | -- |

GLOBAL STABILITY: I-80 RECONSTRUCTION; WEST MAINLINE FROM STATION 410+00, TO 518+00, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

APPENDIX F-1

DRAWN BY: RKC
CHECKED BY: A. Kurnia

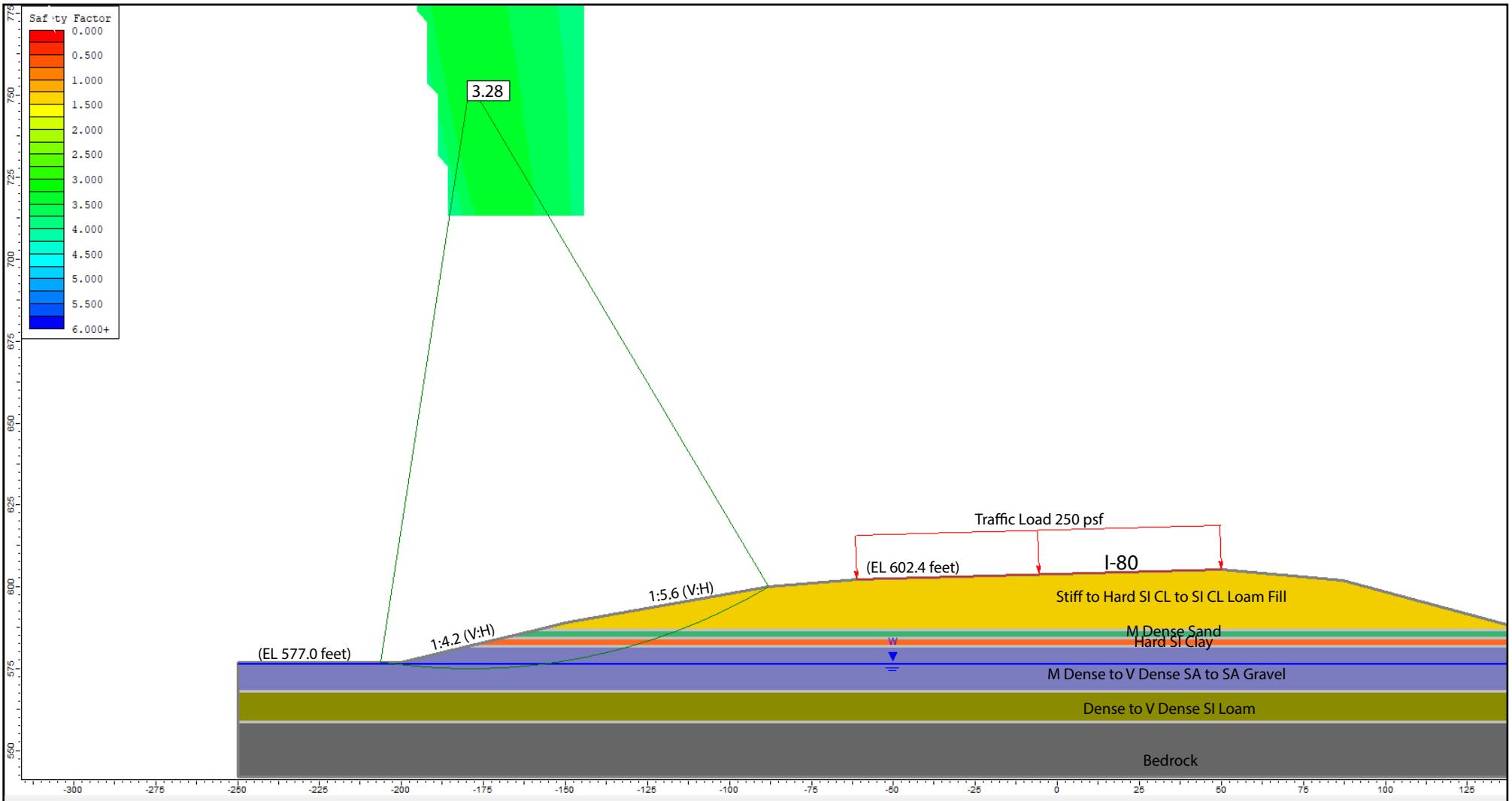


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7901-15-01



Drained Analysis, Mainline-West 62R27, Station 443+00, Ref Boring: HR-BSB-01

| Layer ID | Description | Total Unit Weight (pcf) | Drained Cohesion (psf) | Drained Friction Angle (degrees) |
|----------|--|-------------------------|------------------------|----------------------------------|
| 1 | Stiff to Hard SI CL to SI CL Loam Fill | 120 | 100 | 31 |
| 2 | M Dense Sand | 115 | 0 | 30 |
| 3 | Hard SI Clay | 125 | 100 | 32 |
| 4 | M Dense to V Dense SA to SA Gravel | 120 | 0 | 31 |
| 5 | Dense to V Dense SI Loam | 125 | 0 | 32 |
| 6 | Bedrock | 150 | -- | -- |

GLOBAL STABILITY: I-80 RECONSTRUCTION; WEST MAINLINE FROM STATION 0410+00, TO 0518+00, WILL COUNTY, ILLINOIS

SCALE: GRAPHICAL

APPENDIX F-2

DRAWN BY: RKC
CHECKED BY: A. Kurnia



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APPENDIX G

LEGEND:

-  Soil Boring
-  Pavement Core

APPENDIX G
BORING AND PAVEMENT CORE LOCATION PLANS
AND SOIL PROFILES

ROADWAY GEOTECHNICAL REPORT

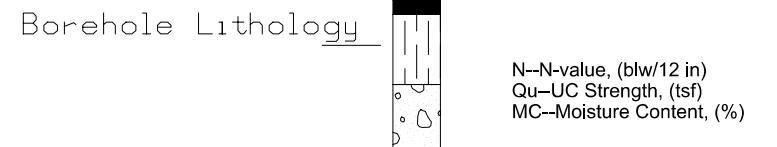
I-80 IMPROVEMENTS
WEST MAINLINE
CONTRACT 62R27

STATION 410+00 AND STATION 518+00
WILL COUNTY, ILLINOIS

FOR
FOR TRANSYSTEMS CORPORATION
1475 EAST WOODFIELD ROAD, SUITE 600
SCHAUMBURG, IL 60173

PREPARED BY
WANG ENGINEERING
1145 NORTH MAIN STREET
LOMBARD, IL 60148

WB-SGB-01 Borehole Number
580.10 ft, Elevation
415+30.58; 57.07 LT Station, offset



- ✖ Water Level Reading at time of drilling.
- ▼ Water Level Reading 24-hr after drilling or at end of drilling

Lithology Graphics

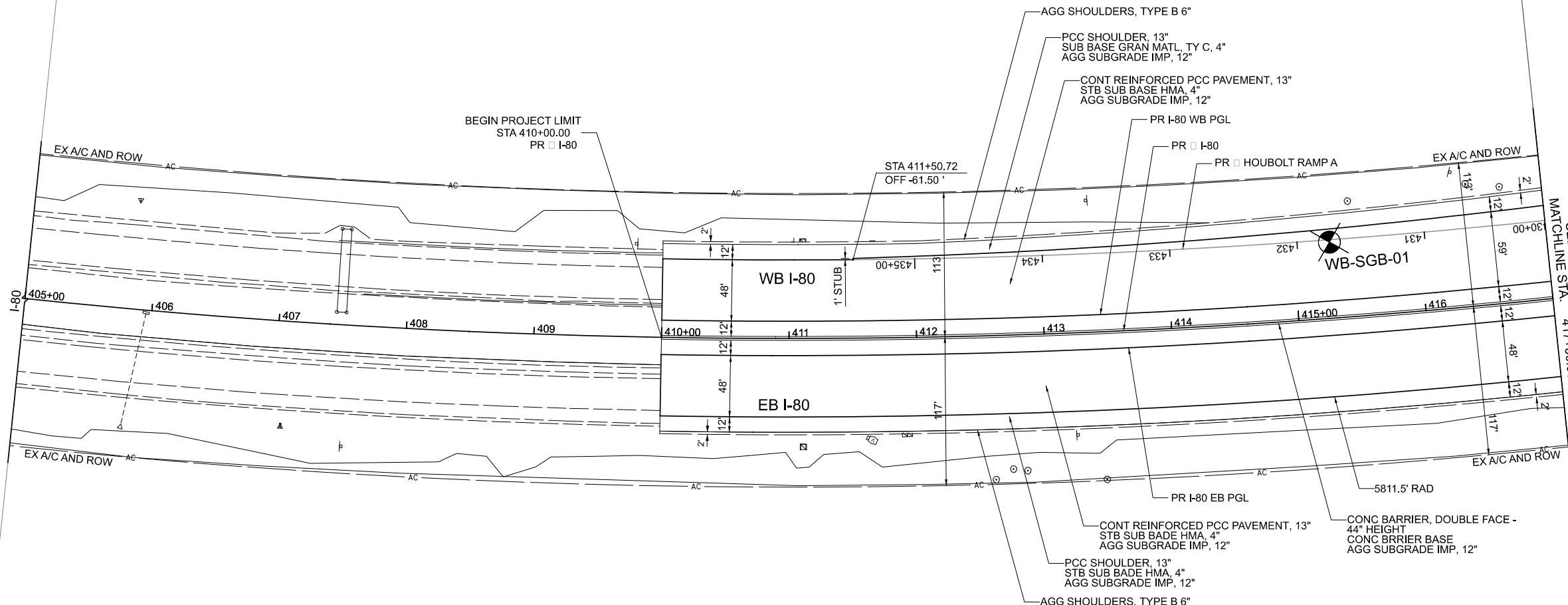
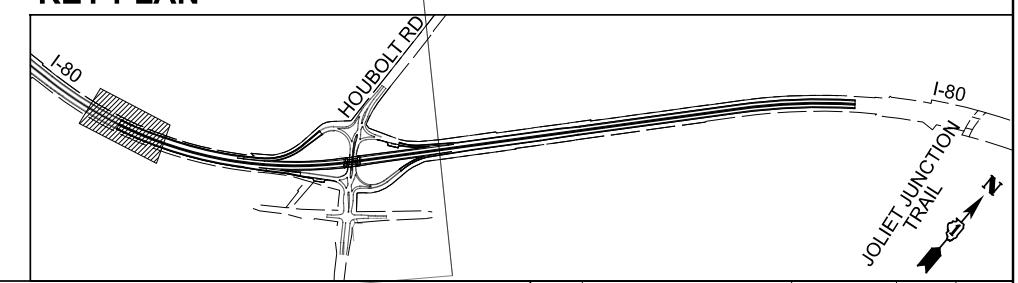
-  Topsoil
-  IDH Sand, Sandy Loam
-  IDH Clay
-  Gravelly sand, sandy gravel
-  IDH Loam
-  IDH Clay Loam
-  IDH Silt, Silty Loam
-  Pavement
-  IDH Silty Clay, Silty Clay Loam
-  Crushed stone

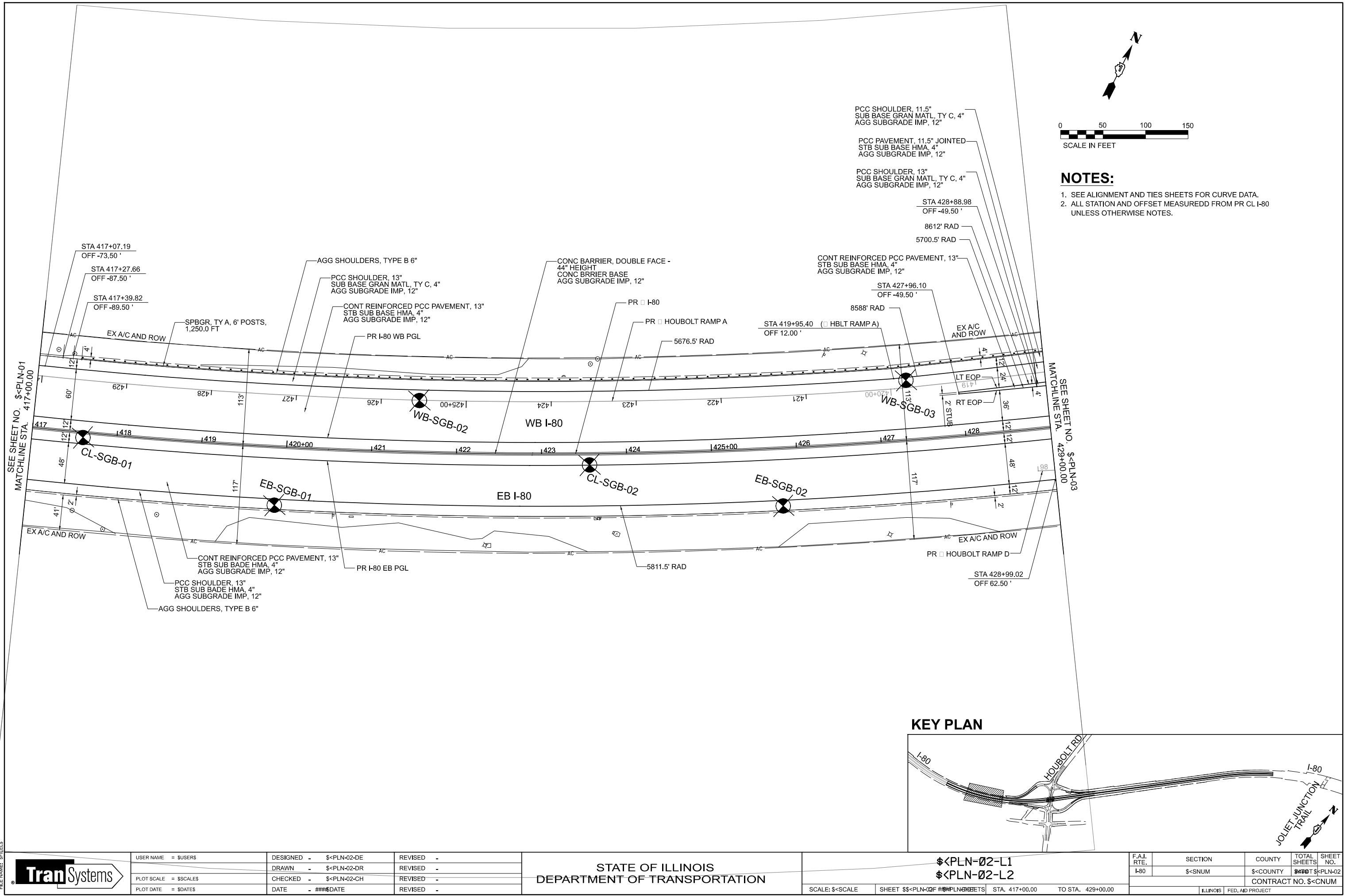
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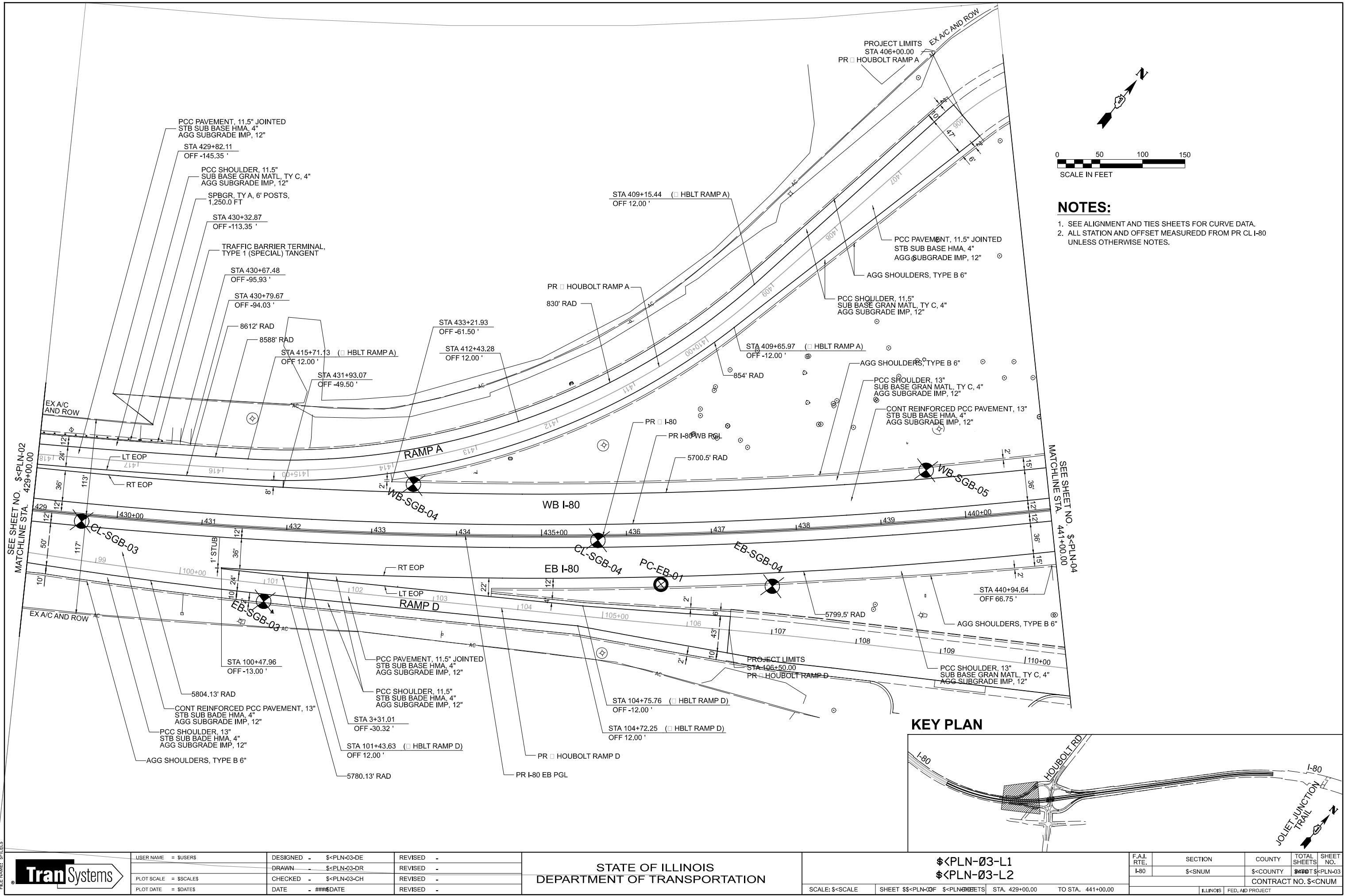
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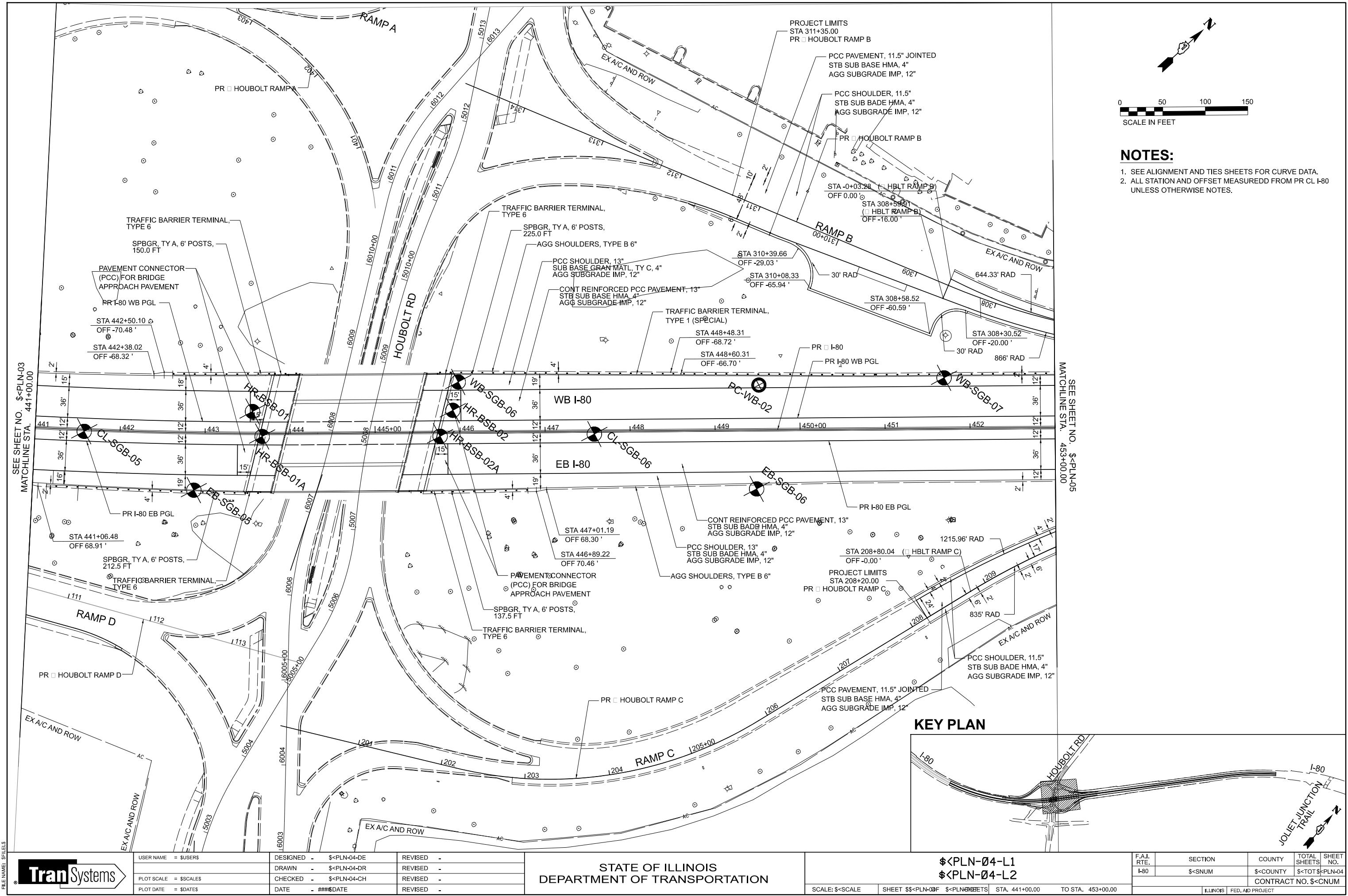
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2. ALL STATION AND OFFSET MEASURED FROM PR CL I-80 UNLESS OTHERWISE NOTES.

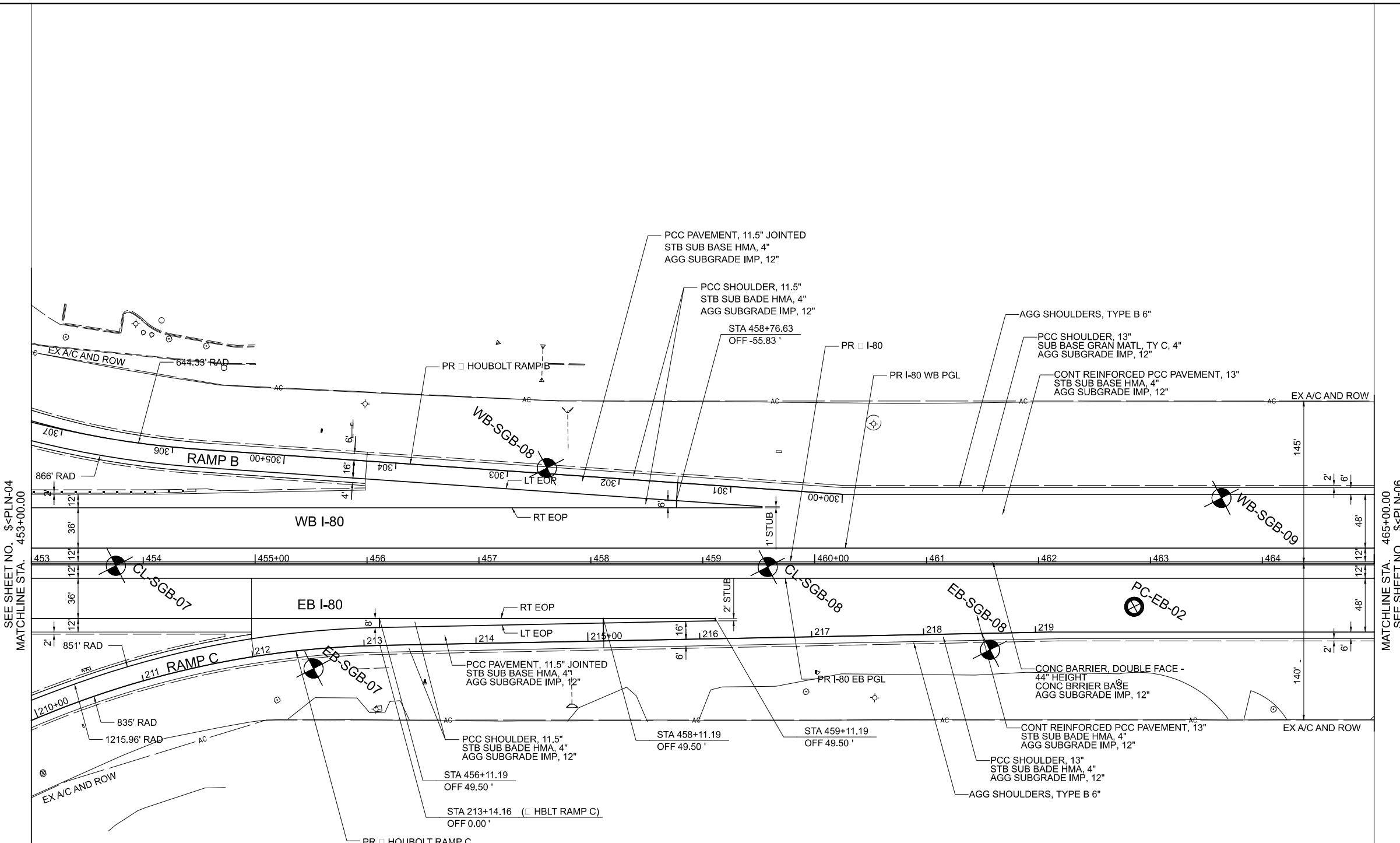
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MATCHLINE STA. 417+00.00

**KEY PLAN**







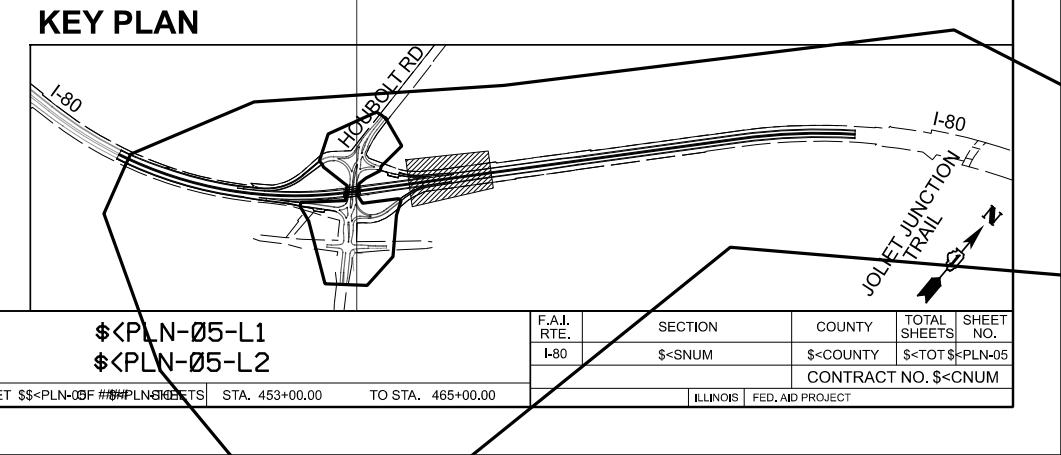


0 50 100 150

SCALE IN FEET

NOTES:

1. SEE ALIGNMENT AND TIES SHEETS FOR CURVE DATA.
 2. ALL STATION AND OFFSET MEASURED FROM PR CL I-80
UNLESS OTHERWISE NOTES.



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

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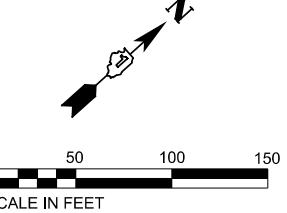
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- - - - - STATE OF ILLINOIS
- - - - - DEPARTMENT OF TRANSPORTATION

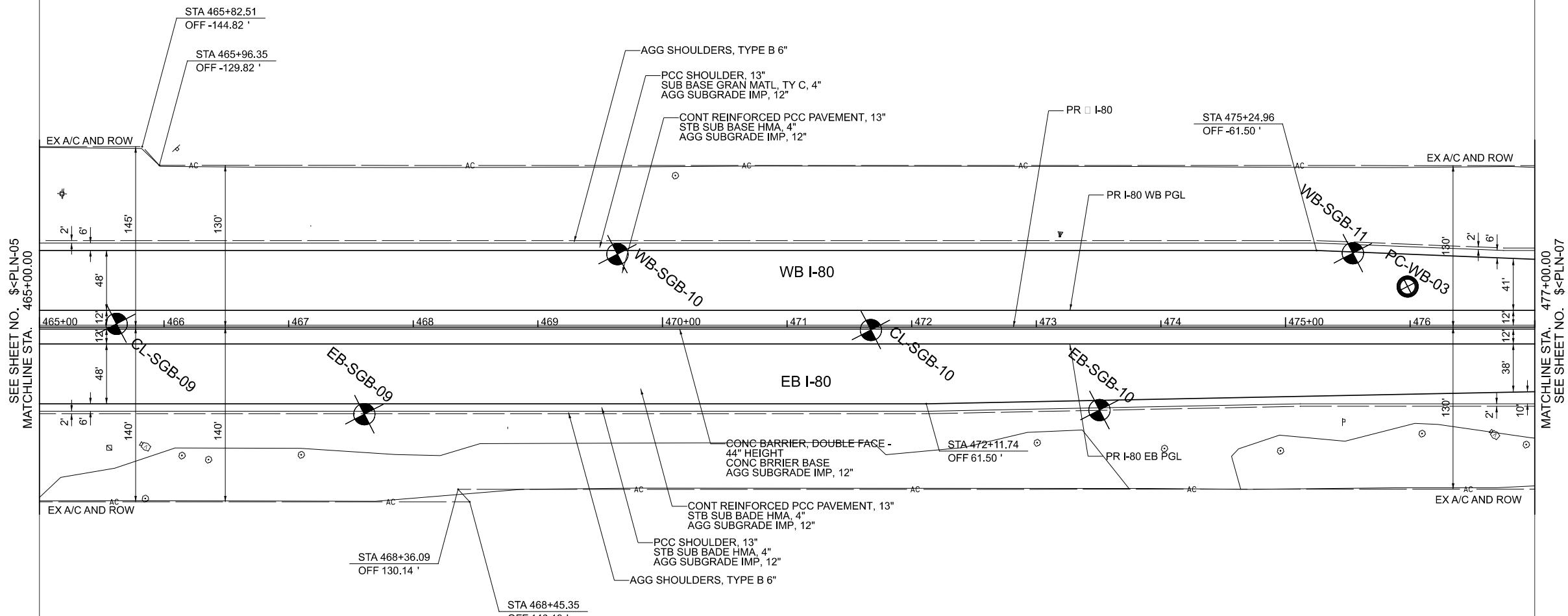
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\$<PLN-05-L2~~

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| | ILLINOIS | FED. AID PROJECT | | |

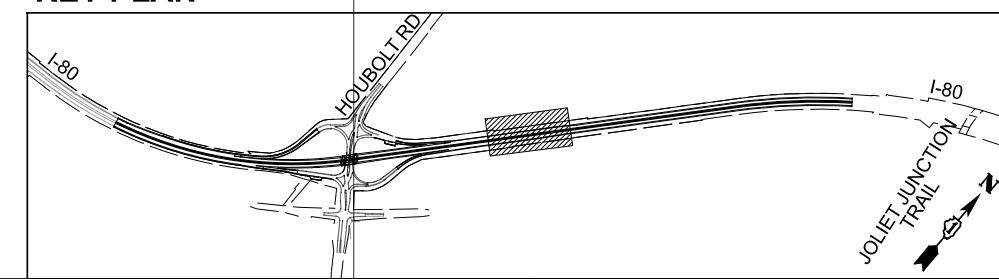


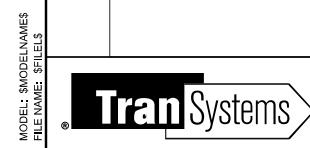
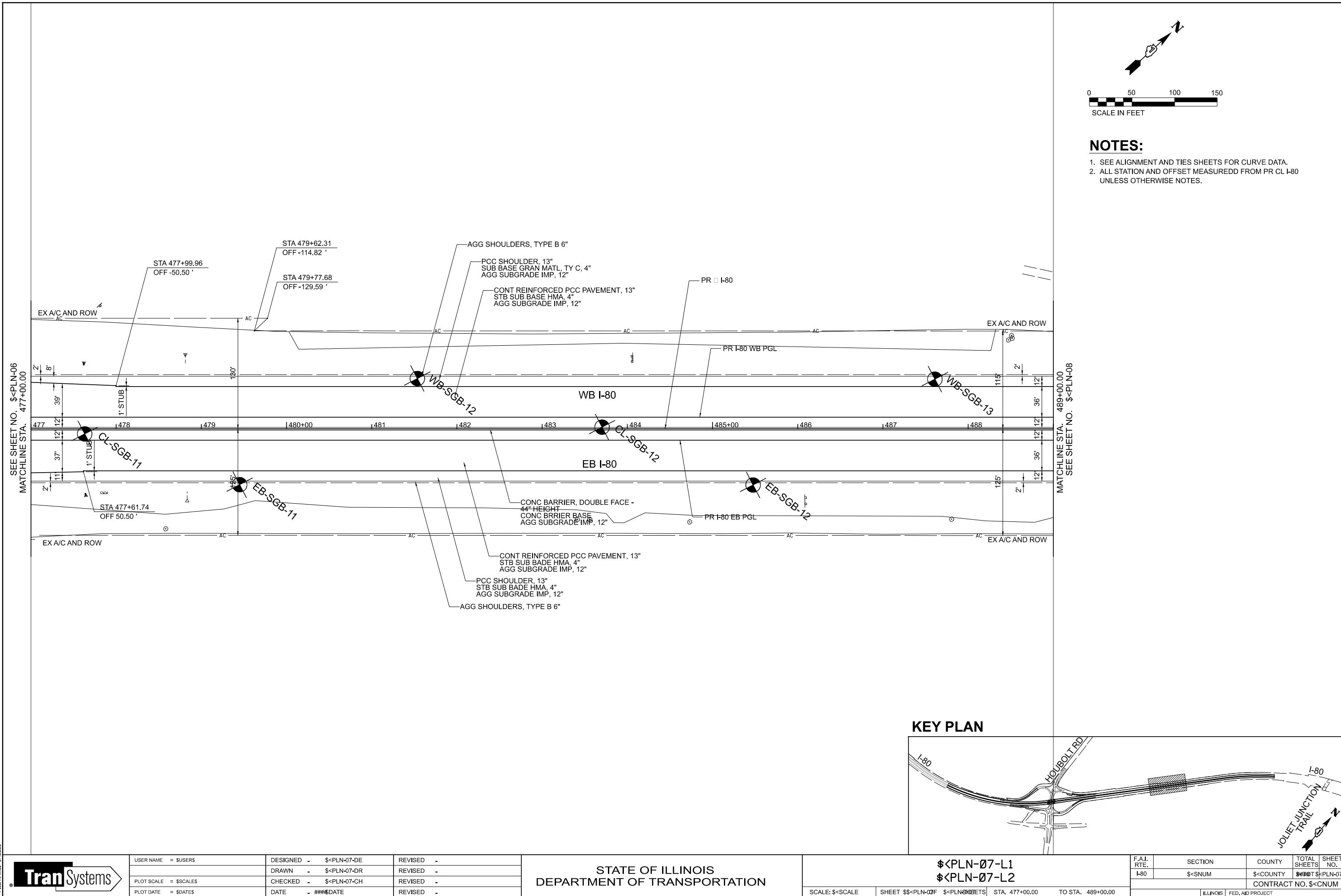
NOTES:

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KEY PLAN

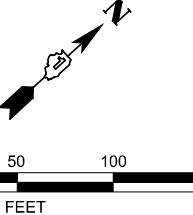
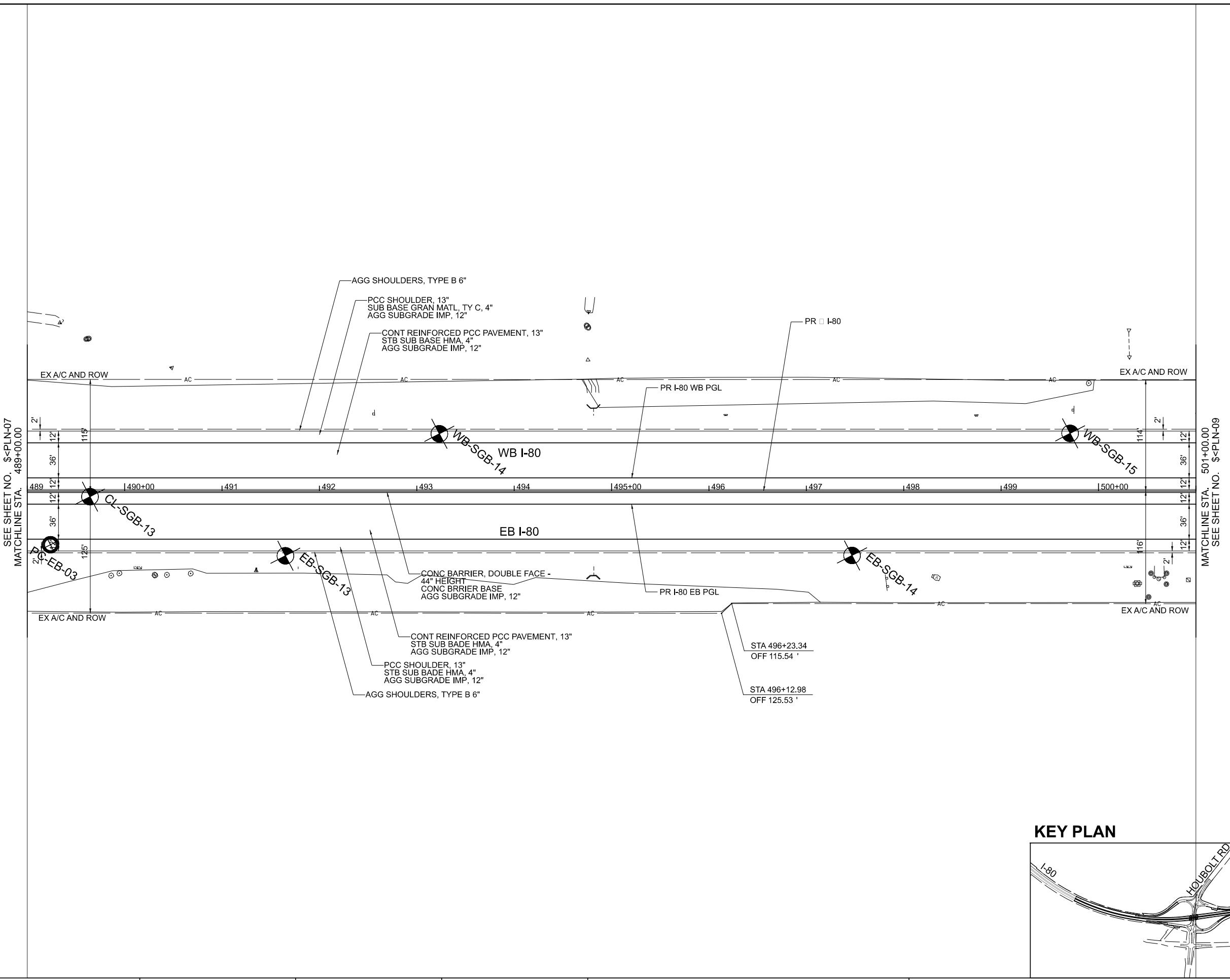




STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

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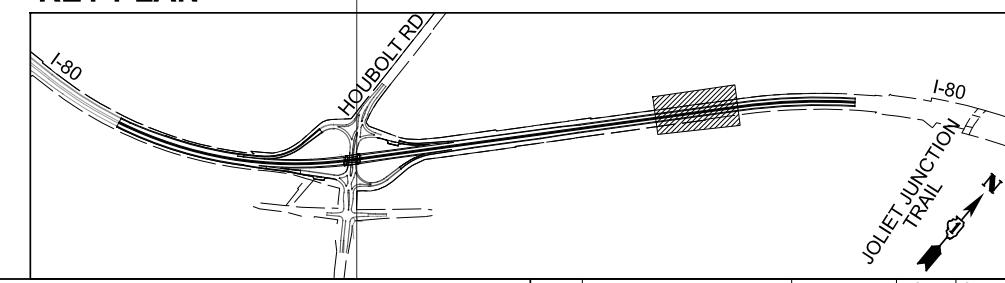
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| CONTRACT NO. \$<CNUM> | | | | |
| 489+00.00 | ILLINOIS FED. AID PROJECT | | | |



NOTES:

1. SEE ALIGNMENT AND TIES SHEETS FOR CURVE DATA.
 2. ALL STATION AND OFFSET MEASURED FROM PR CL I-80 UNLESS OTHERWISE NOTES.

KEY PLAN



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

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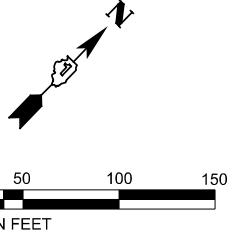
Tran Systems

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

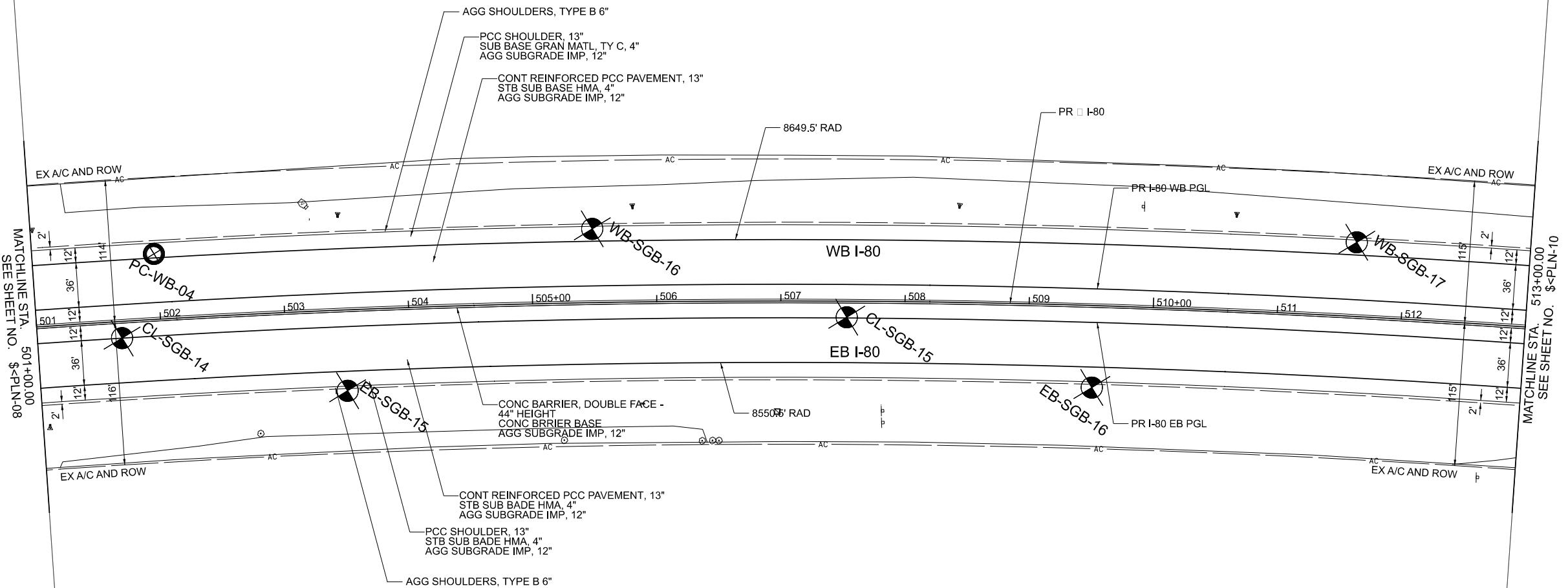
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| | | ILLINOIS | FED. AID PROJECT | | |

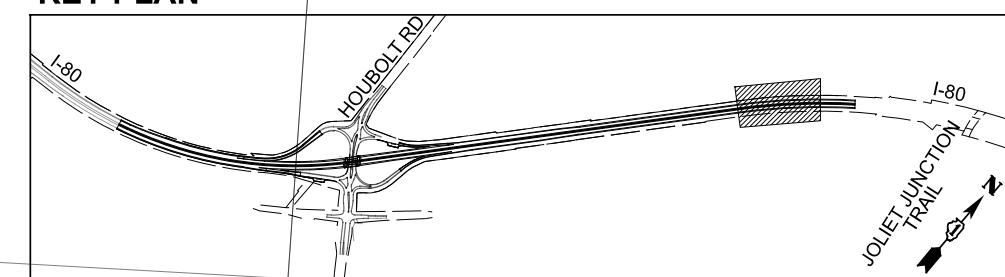


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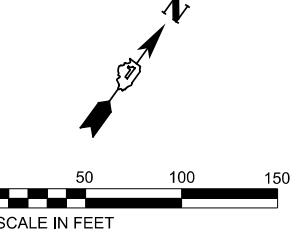
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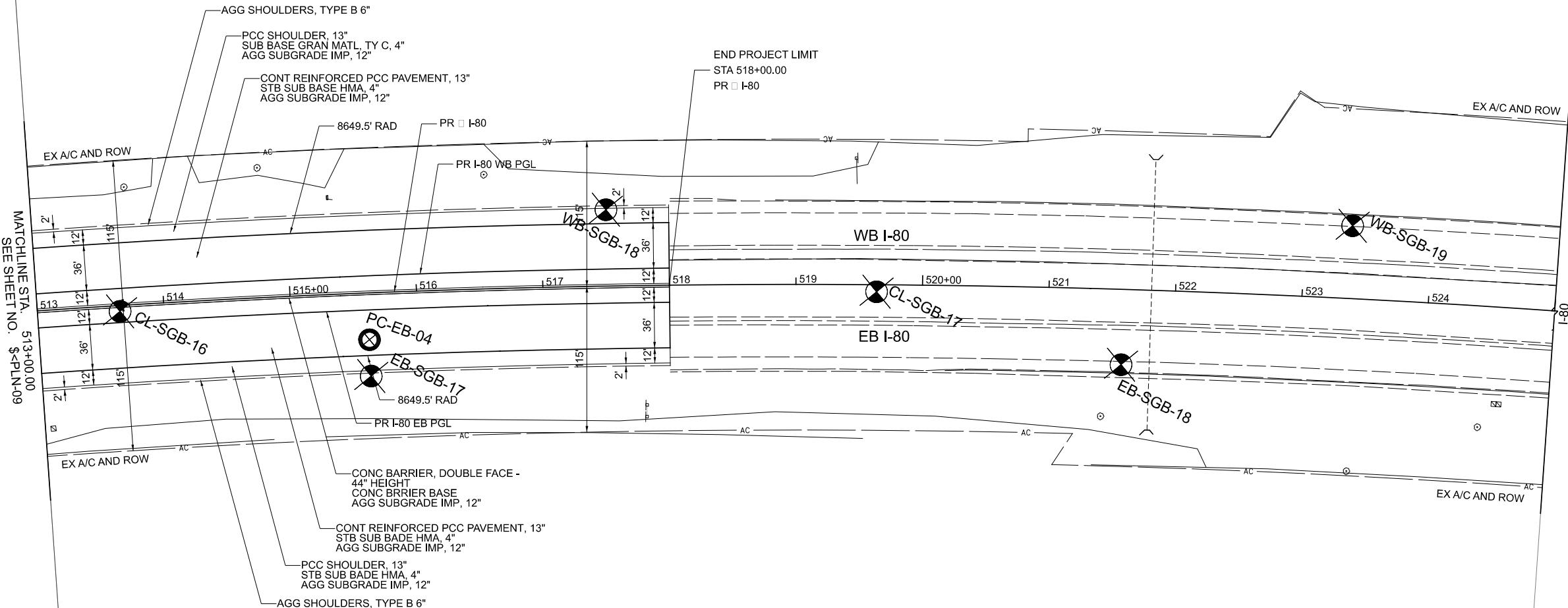


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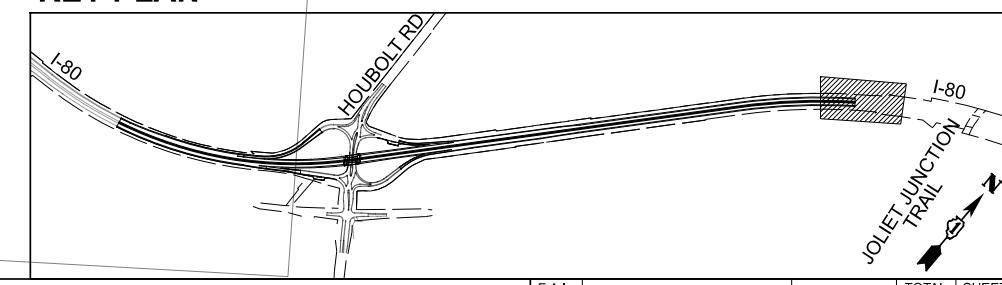


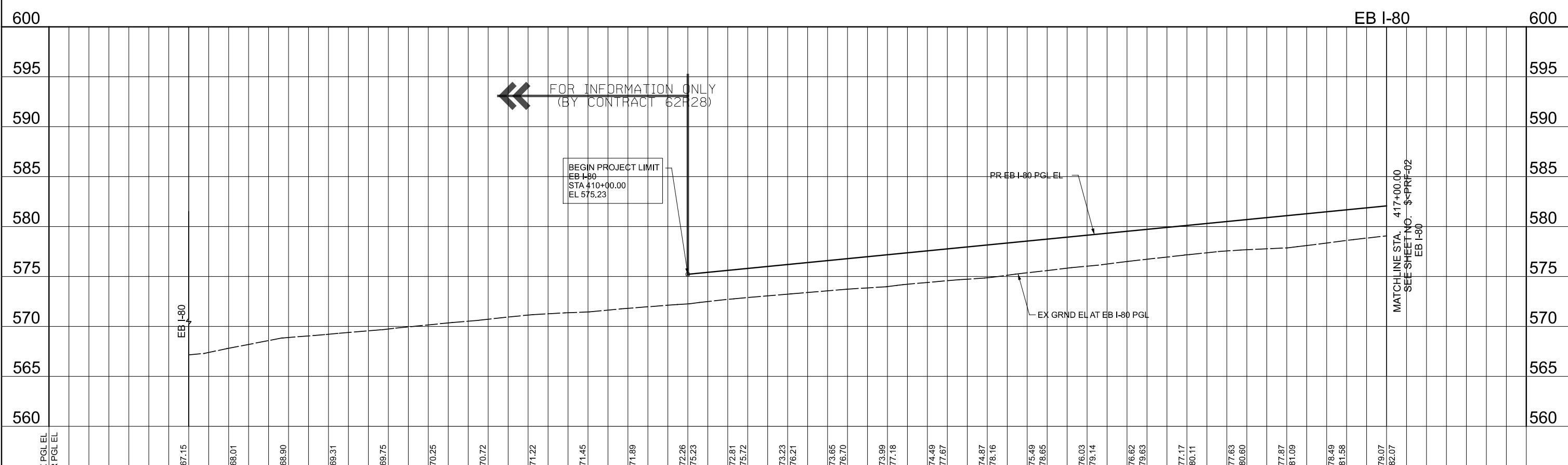
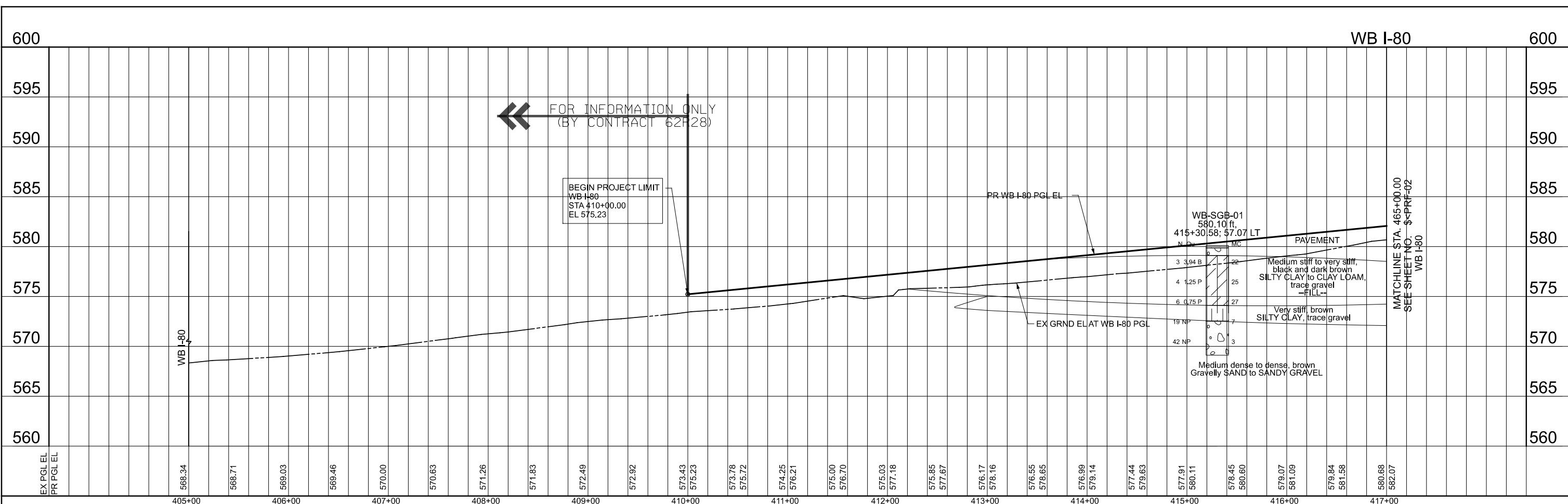
NOTES:

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2. ALL STATION AND OFFSET MEASURED FROM PR CL I-80 UNLESS OTHERWISE NOTES.



KEY PLAN





STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

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MODEL: \$MODELNAME\$



405+00
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PLOT SCALE = \$\$CALE\$
PLOT DATE = \$DATE\$

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| 406+00 | | 407+00 |
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| | DRAWN | - \$<PRF-01 |
| | CHECKED | - \$<PRF-01 |
| | DATE | - \$<DATE |

| | | |
|-----|---------|---|
| | 408+00 | |
| -DE | REVISED | - |
| -DR | REVISED | - |
| -CH | REVISED | - |
| | REVISED | - |

409+00 410+00
S
DEPARTMENT

411+00

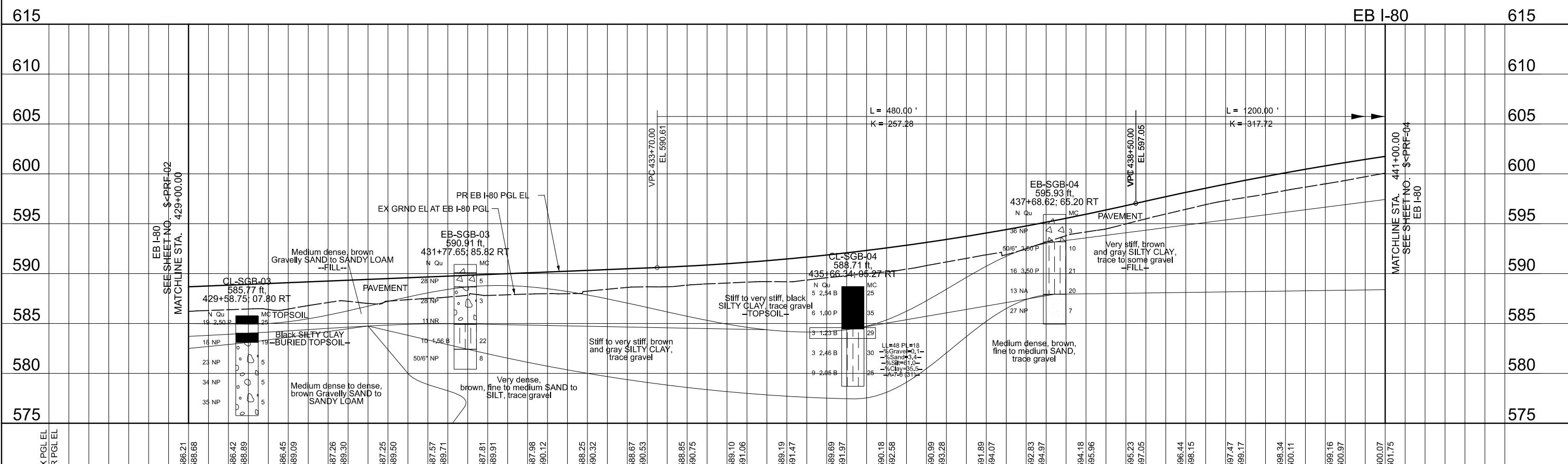
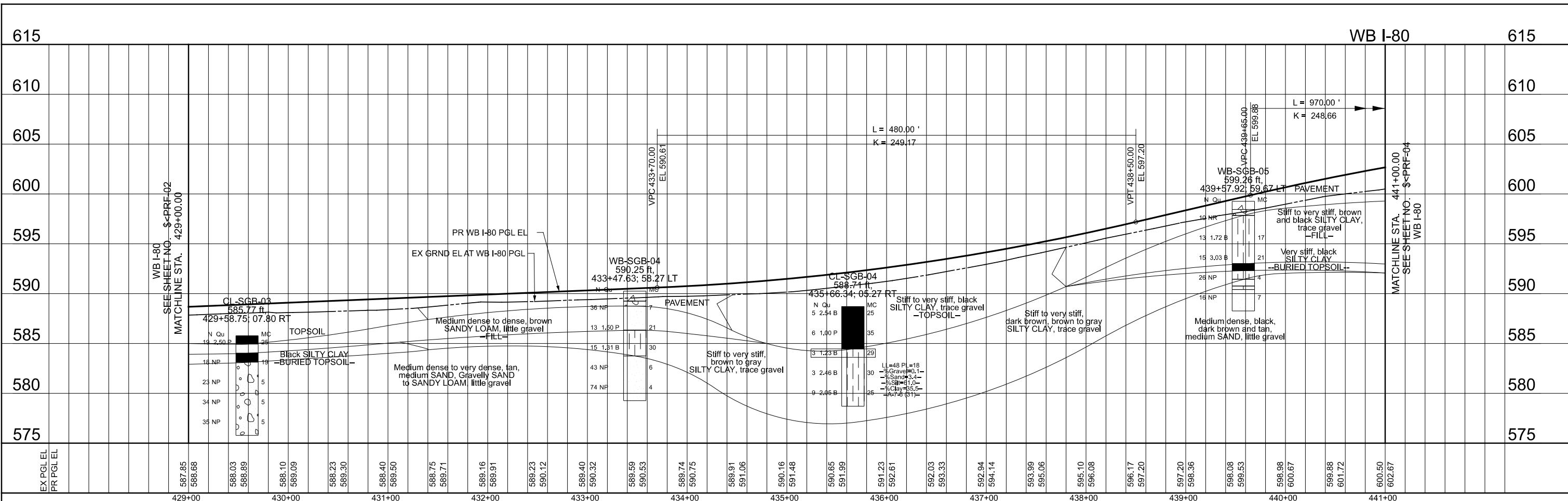
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TATION SCALE: HOF
VERP

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RZ: 1"-50'
T: 1"-5'

415+00 4
F-01-L1
F-01-L2

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| | I-80 | \$<SNUM |
| 17+00,00 | | ILLIN |

| | | | |
|------------------|----------------------|--------------|-----------|
| | COUNTY | TOTAL SHEETS | SHEET NO. |
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| IOIS | CONTRACT NO. \$<CNUM | | |
| FED. AID PROJECT | | | |



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

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\$<PRF-03-L2

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FILE NAME: \$FILEL\$

EX PR
PR PR

| | |
|------------|-------------|
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| | |
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| PLOT DATE | = \$DATES\$ |

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|--------|------------|-------------|
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| | DRAWN - | \$<PRF-03-D |
| | CHECKED - | \$<PRF-03-C |
| | DATE - | \$<DATE |

| | |
|----|-----------|
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| E | REVISED - |
| R | REVISED - |
| CH | REVISED - |
| | REVISED - |

433+00 434+00
|
|
|
|
|
**STA
DEPARTMENT**

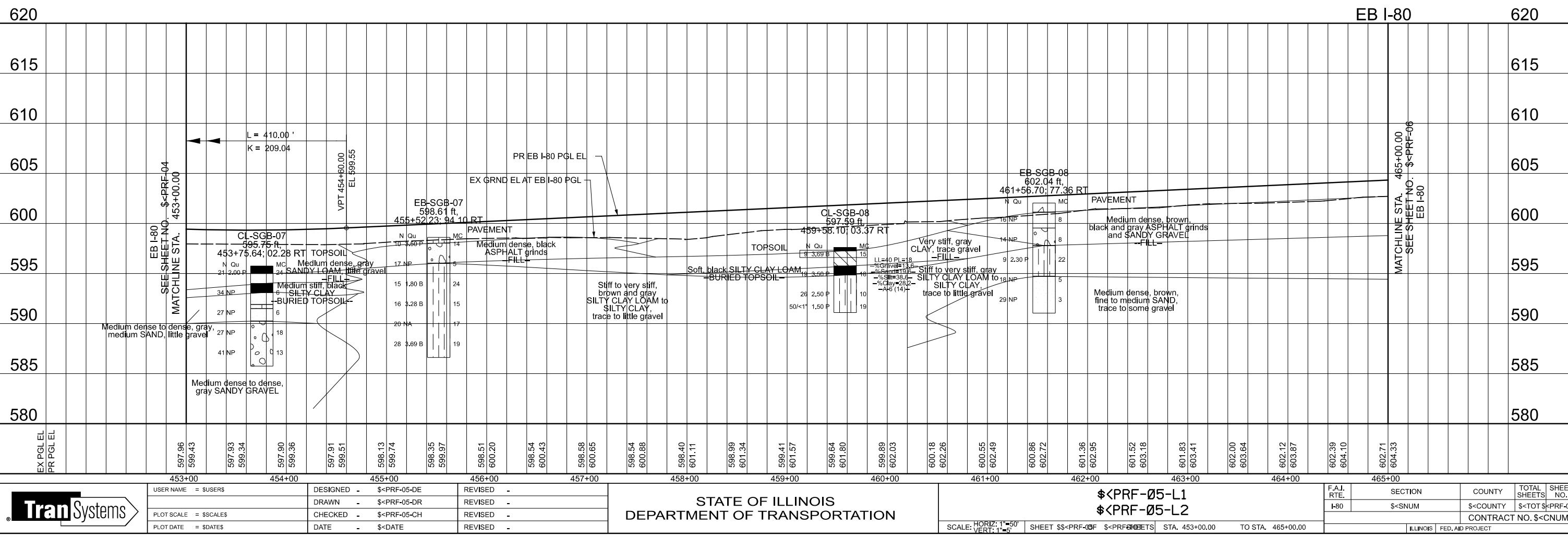
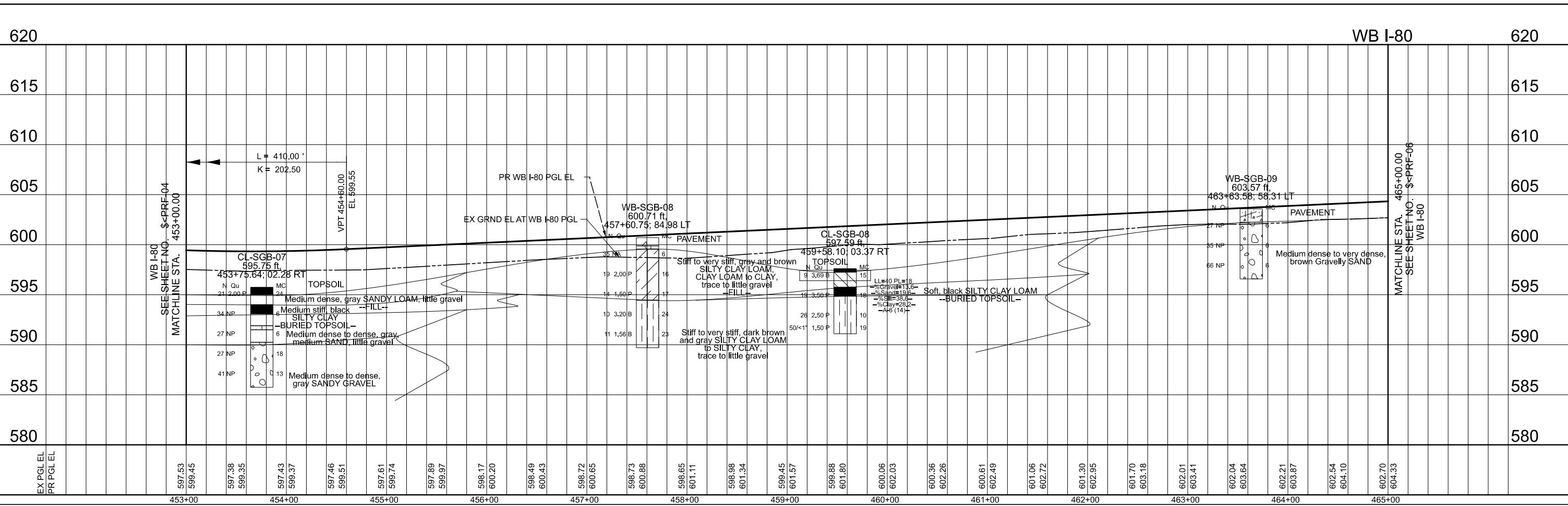
435+00 4

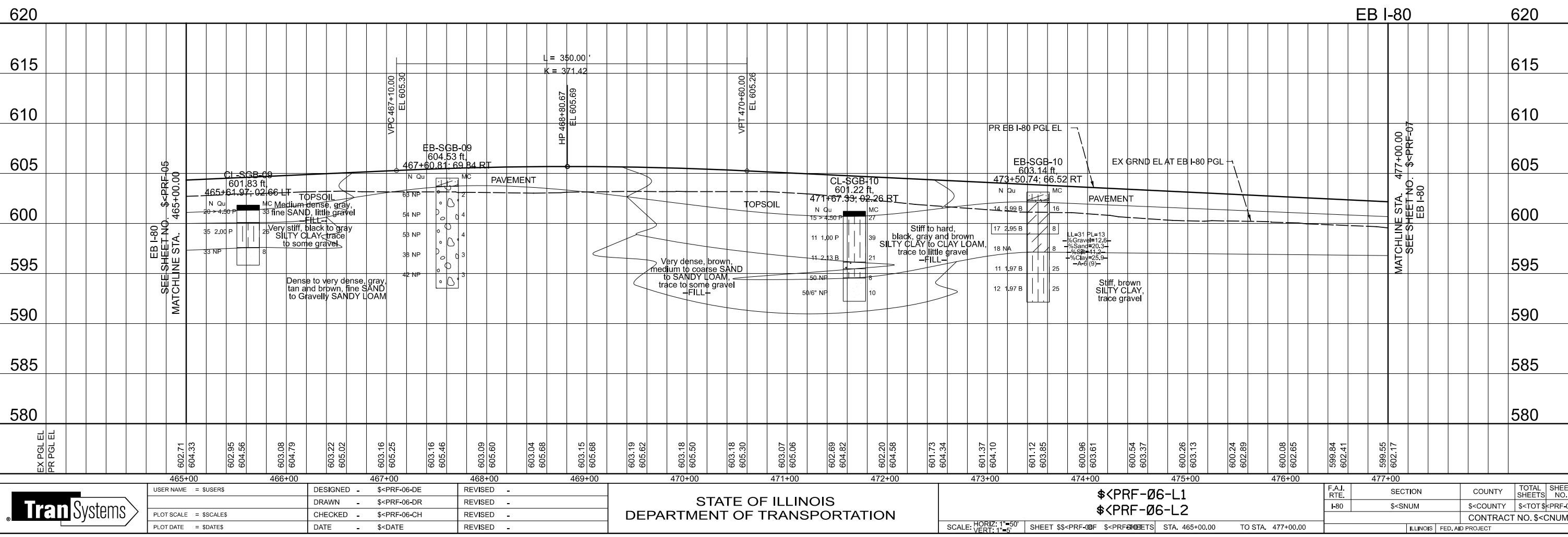
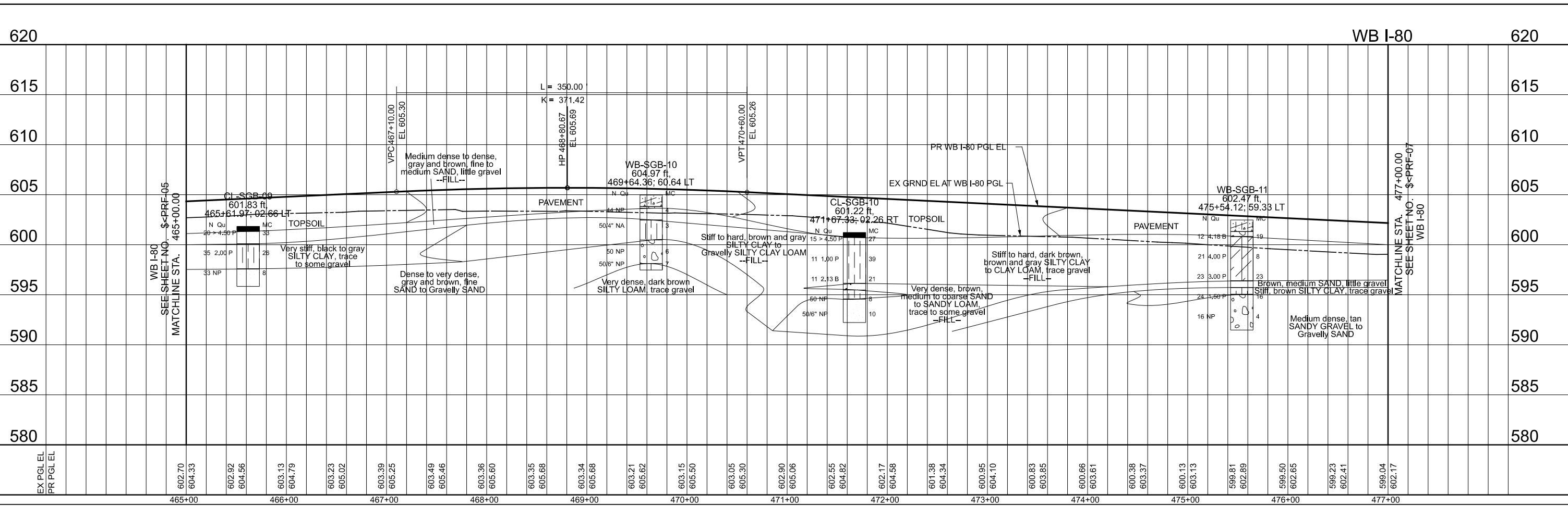
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| 36+00 | 437+00 |
| ON | SCALE: HORIZ: 1"-50' VERT: 1"-5' |

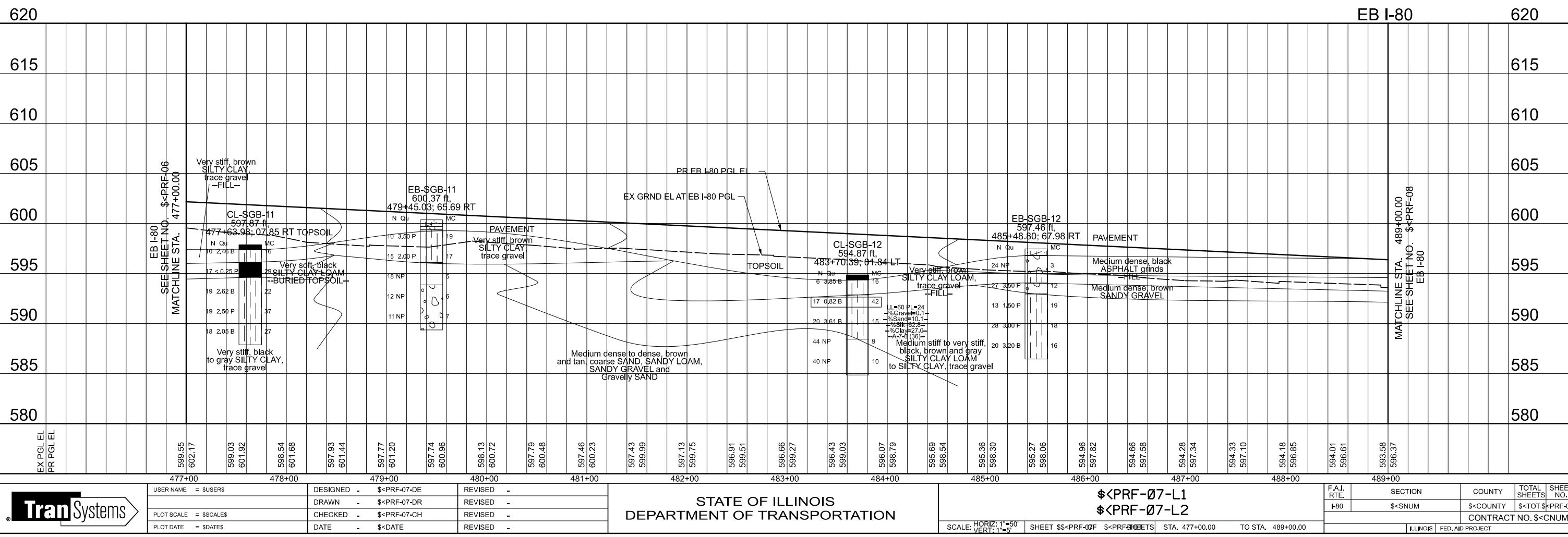
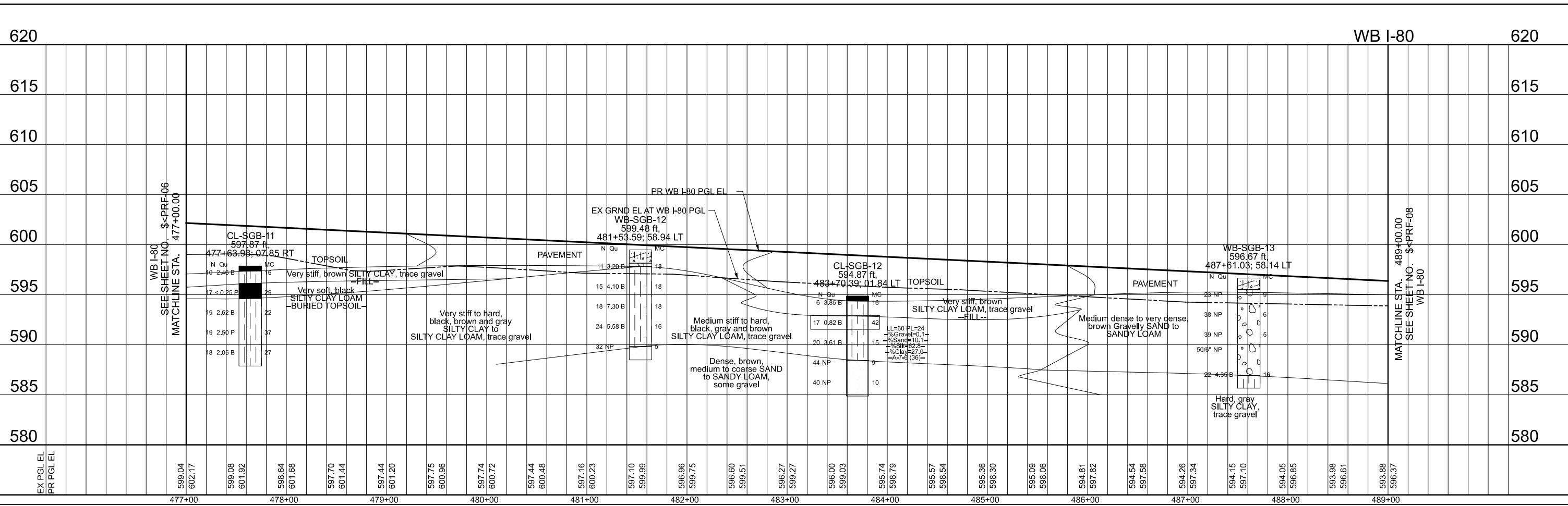
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SHEET \$\$<PRF-03F \$<PRF03LETS STA.

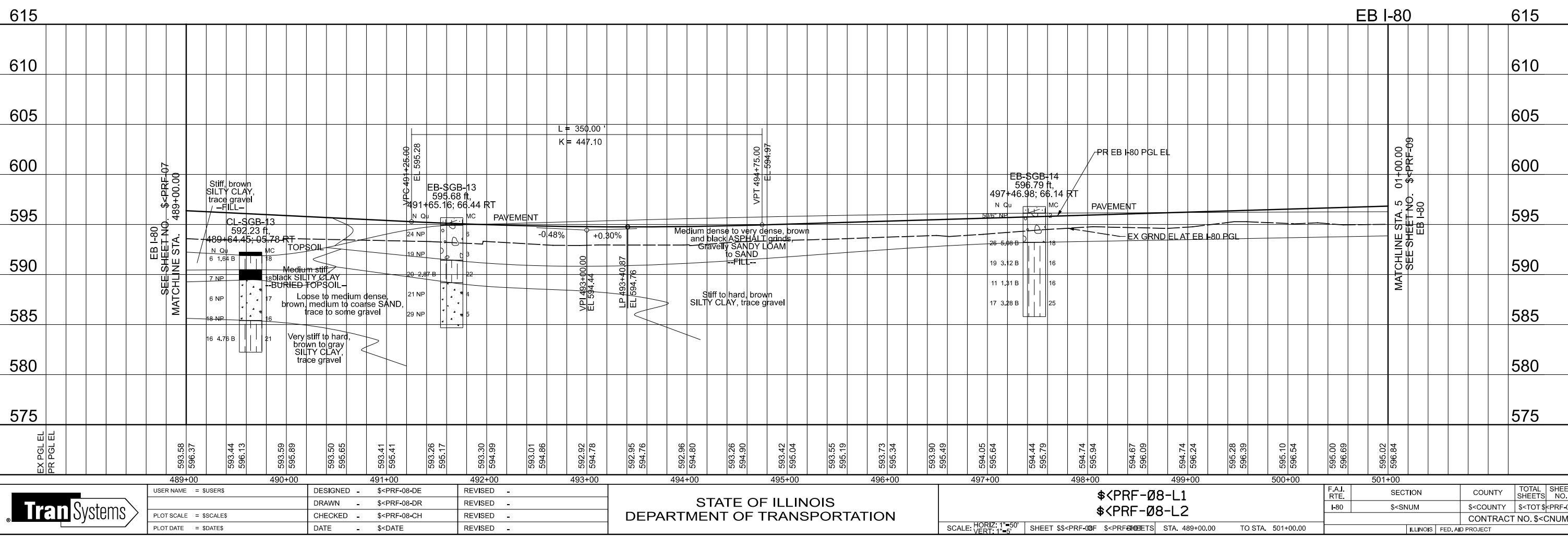
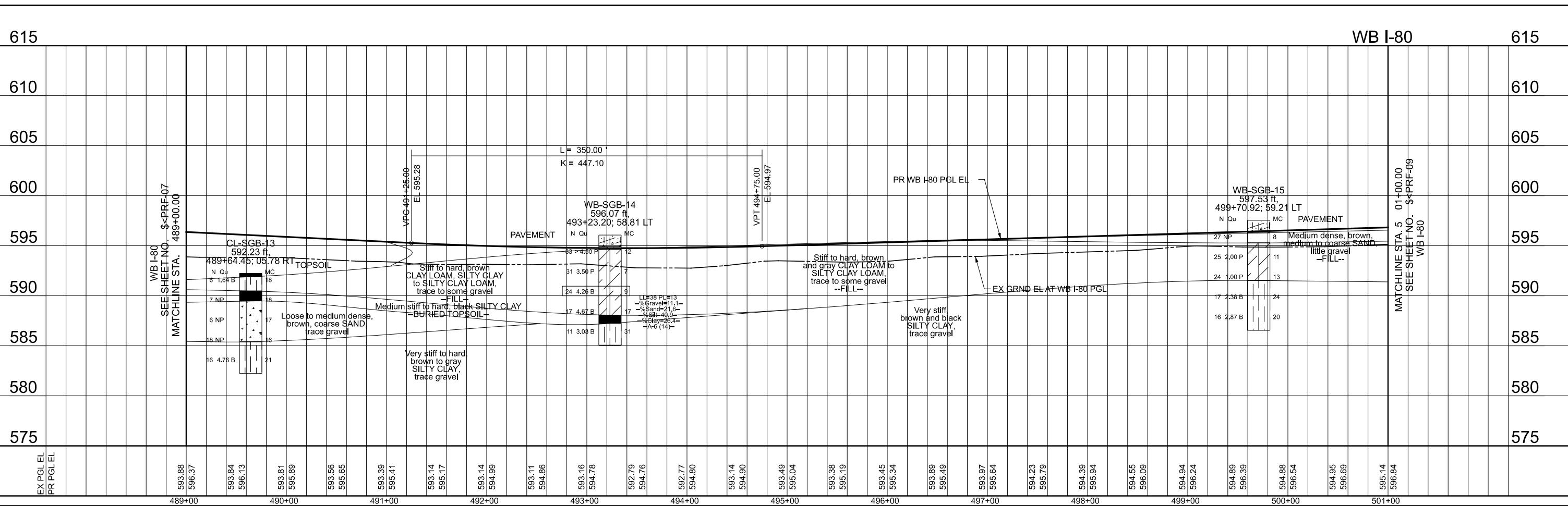
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|------|--------|----------------|
| 9+00 | 440+00 | F. R. I. |
| 1 | | |
| 2 | | |

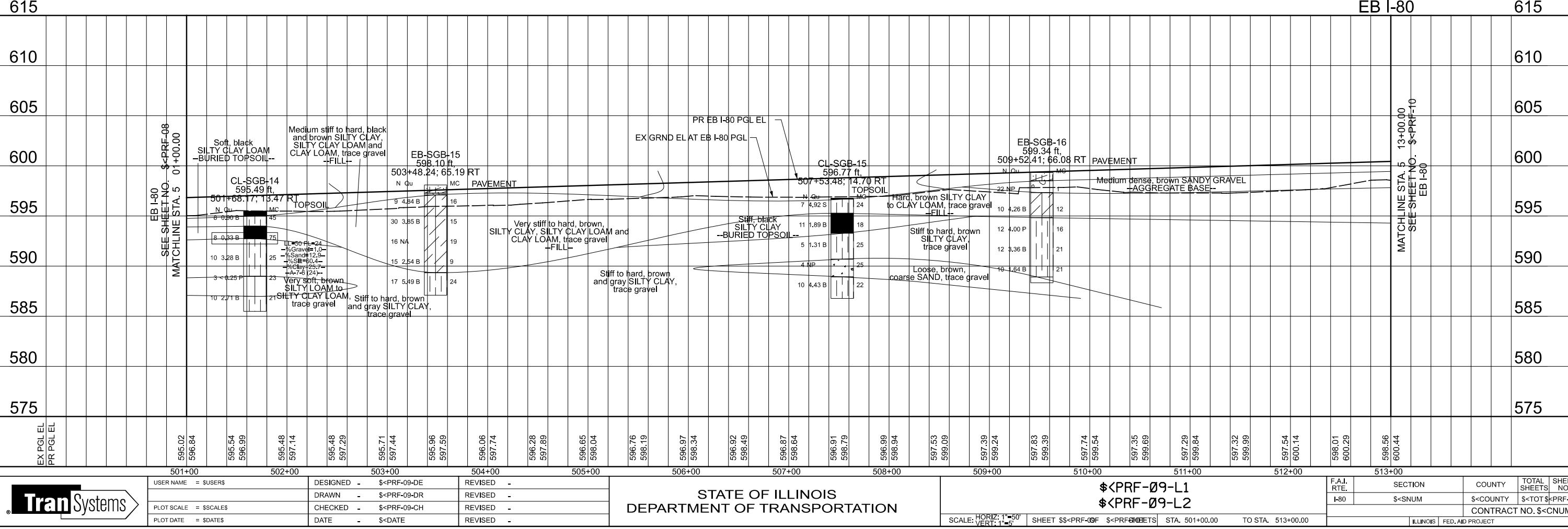
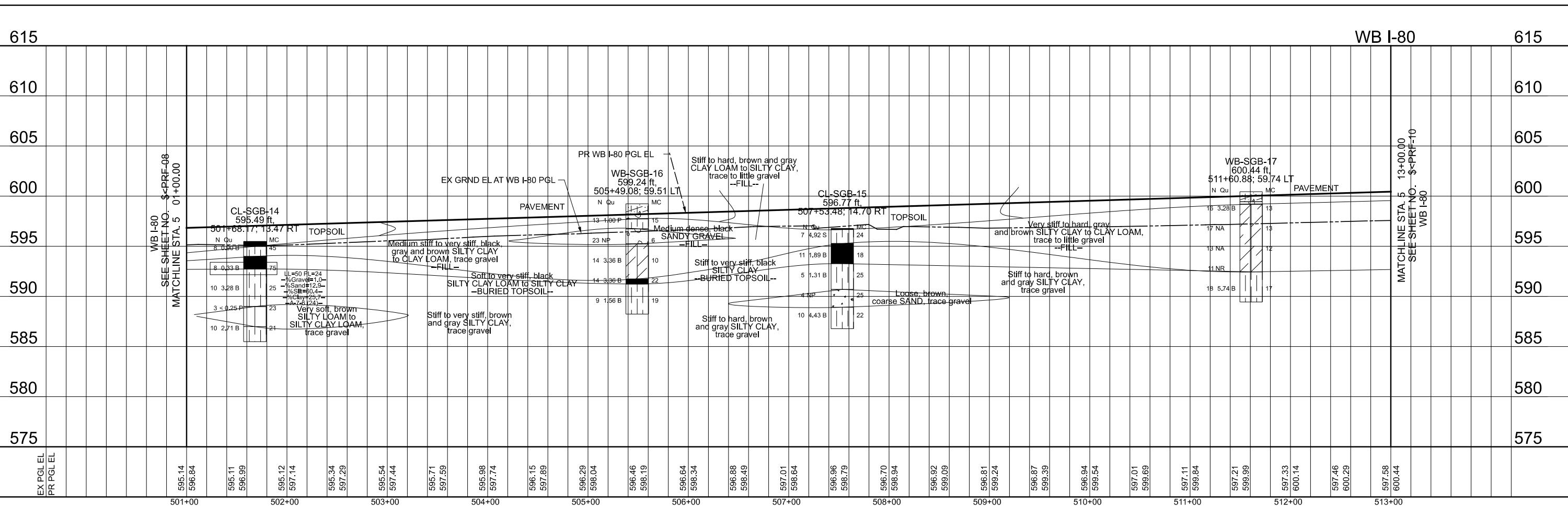
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|--------------|----------|----------------------|-----------------|--------------|
| 441+00 | | | | |
| A.I. T.E. | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
| 80 | \$<NUM> | \$<COUNTY | \$<TOT\$<PFR-03 | |
| | | CONTRACT NO. \$<CNUM | | |
| | ILLINOIS | FED. AID PROJECT | | |

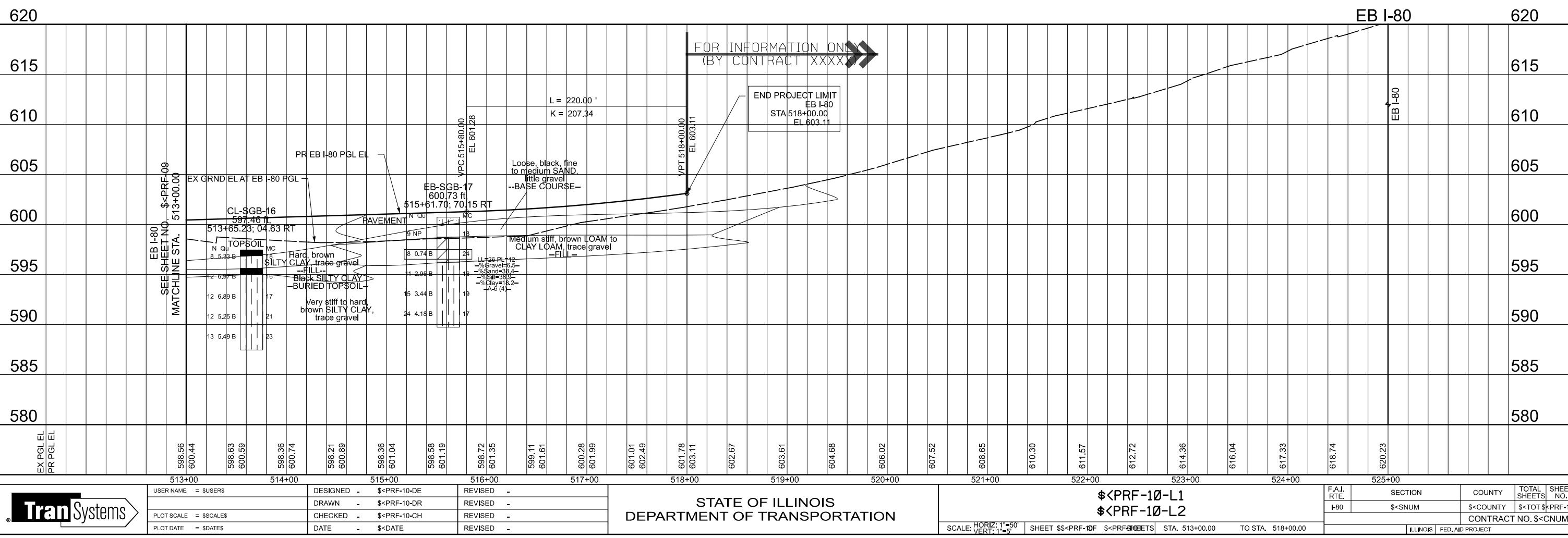
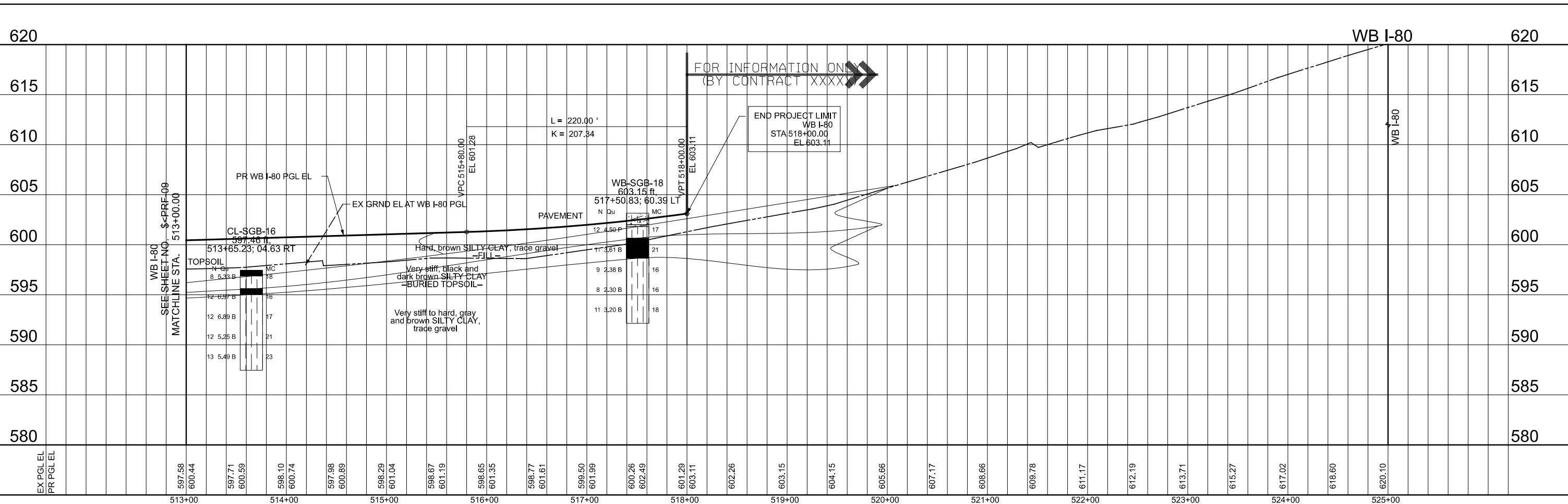








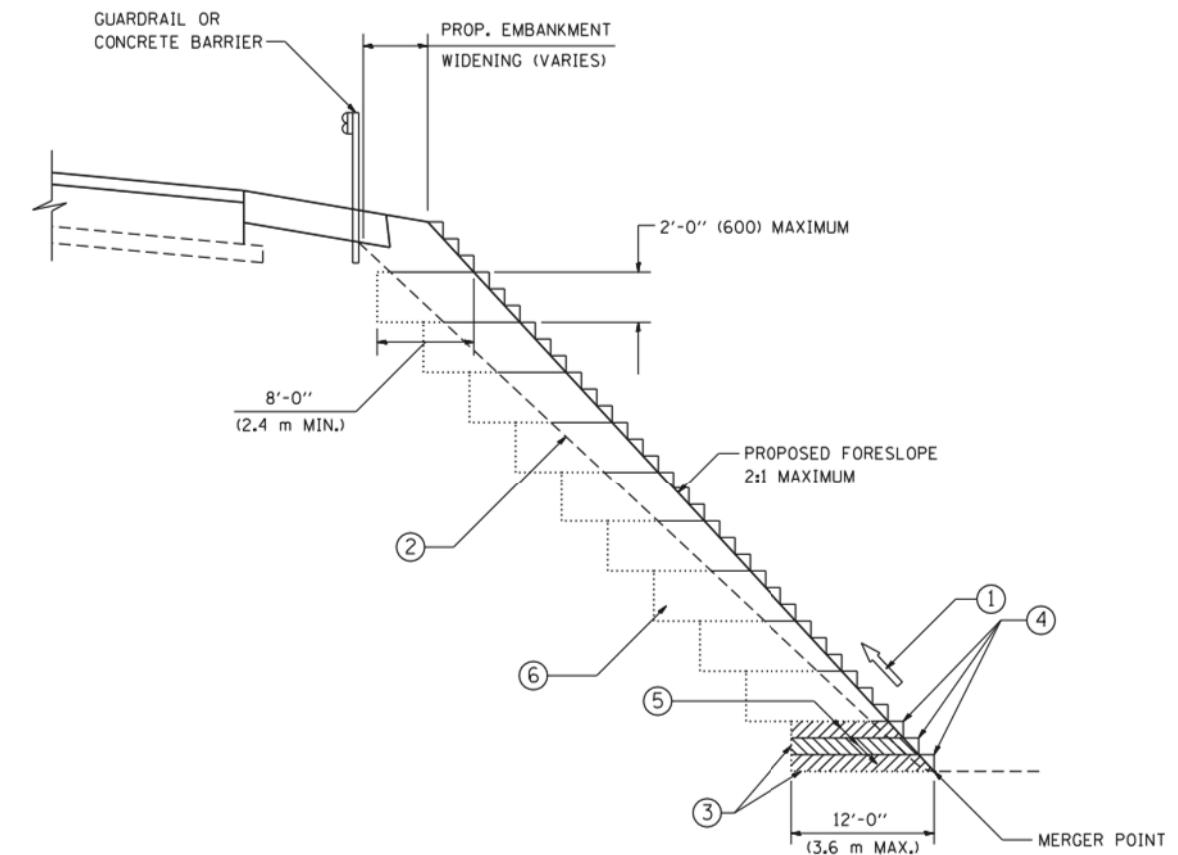






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APPENDIX H



TYPICAL BENCHING DETAIL
FOR EMBANKMENT

NOTES:

- ① CONSTRUCT SUCCEEDING BENCH CUTS AND EMBANKMENT PLACEMENT AND COMPACTION FROM BOTTOM TO TOP IN STAIRSTEP FASHION.
- ② EXISTING FORESLOPE PREPARED IN ACCORDANCE WITH ARTICLE 205.03 OF THE STANDARD SPECIFICATIONS.
- ③ BENCH CUT EXISTING SLOPE TYPICAL FOR EACH STEP.
- ④ TRIM TO FINAL SLOPE.
- ⑤ EQUAL 8-INCH (200) LIFTS OF EMBANKMENT COMPACTED IN ACCORDANCE WITH ARTICLE 205.05 OF THE STANDARD SPECIFICATIONS.
- ⑥ EXCAVATION OF BENCH CUTS WITHIN EXISTING EMBANKMENT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC METER OR CUBIC YARD FOR "EARTH EXCAVATION". THIS PRICE WILL INCLUDE ALL LABOR AND MATERIAL, NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- ⑦ SLOPES SHALL BE BENCHING ACCORDING TO THIS DETAIL WHEN THE SLOPE IS STEEPER THAN 4:1 AND THE HEIGHT IS GREATER THAN 5' (1.5 m).

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS)
UNLESS OTHERWISE SHOWN.

| FILE NAME = W:\diststd\22x34\bd51.dgn | USER NAME = gaglionobt | DESIGNED - | REVISED - | BENCHING DETAIL FOR EMBANKMENT WIDENING | | | F.A.P. RTE. | SECTION | COUNTY | TOTAL SHEETS | SHEET NO. |
|--|------------------------|----------------------|-----------------|--|-------------|--------------------------------------|----------------|-------------|--------------------|---|--------------|
| | | DRAWN - CADD | REVISED - | | | | 326 | 105-N-2(15) | MCHENRY | 473 | 380 |
| | | CHECKED - S.E.B. | REVISED - | | | | | | | | |
| | | PLOT DATE = 1/4/2008 | DATE - 06-16-04 | REVISED - | SCALE: NONE | SHEET NO. 1 OF 1 SHEETS STA. TO STA. | | BD-51 | CONTRACT NO. 62B43 | FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT | |