

Benchmark: BM #402 "\(\)" on west headwall of box culvert by old railroad bridge (Sta. 22+46, 30' Rt, Elev. 642.03). BM #413 "[" southwest corner of ComEd tower foundation west of railroad arch (Sta. 14+21, 128' Lt, Elev. 652.32).

Existing Structure:

The original single-track railroad bridge was 1,000'-0" long from Q-to-Q of bearing and was built in the 1920s by the Illinois Central Railroad. The substructure consists of cast-in-place concrete abutments and fifteen piers and is all that remains of the existing structure. When the bridge was in service, it also included non-ballasted steel deck girders and steel trestles on the concrete piers. The girders and trestles were removed in the 1980s.

Vehicular and river traffic under the bridge to be maintained during construction.

— Prop. Ground Line

36" to 60" Haunched

Web Plate Girder

(composite full

length), typ.

Salvage: None

Footing

Elev.

with Pile

 \bigcirc

 \bigcirc

Shoes, typ.

671.82

Steel H-piles

Stone Riprap,

Class A5, typ.

DESIGN SCOUR ELEVATION TABLE

Event / Limit		Design Scour Elevations (ft.)											
State	S. Abut.	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Pier 8	Pier 9	N. Abut.	Item 113	
Q100 (base)	665.60	627.51	623.01	622.51	624.01	627.28	632.45	639.16	639.16	642.38	673.70		
Q200	665.60	621.50	613.92	613.42	614.92	620.91	626.34	633.37	633.35	636.57	673.70	_ [
Design	665.60	627.51	623.01	622.51	624.01	627.28	632.45	639.16	639.16	642.38	673.70	1 ³	
Check	665.60	621.50	613.92	613.42	614.92	620.91	626.34	633.37	633.35	636.57	673.70		

WATERWAY INFORMATION

Drainage Area = 8,620 sq.	Low Grade Elev. 669.72 at Sta. 22+79 to 26+75								
Flood	Freq.	Q	Opening	g Sq. Ft.	Nat.	Head - Ft.		Headwa	ater El.
F1000	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
10-Year	10	42,600	7,350	7,644	645.59	N/A	0	N/A	645.59
Design	50	56,800	9,287	9,634	647.98	N/A	0	N/A	647.98
Base	100	62,400	9,980	10,345	648.82	N/A	0	N/A	648.82
Scour Check	200	67,584	10,696	11,079	649.68	N/A	0	N/A	649.68
Max. Calc.	500	74,500	11,400	11,804	650.53	N/A	0	N/A	650.53

-Exist.

NOTES.

Ground

Low Steel

Elev. 663.07

Exist. Pier

removed),

(to be

typ.

1. For Sections A-A and B-B, see Sheet S-3.

2. Refer to Ramp Plans for North Abutment

10-Yr Velocity through Existing Bridge = 5.8 fps 10-Yr Velocity through Proposed Bridge = 5.6 fps

5'-2" min. Horiz. Clr.

21'-8" min. Horiz. Clr.

- **©** Page Dr.

@ River Shared-Use Path

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge

Design Specifications, 9th Edition

2009 AASHTO LRFD Guide Specifications for the Design of Pedestrian Bridges, 2nd Edition with 2015 Interims

LOADING H10

Pedestrian Live Load = 90 psf uniform load

DESIGN STRESSES

FIELD UNITS

f'c = 4,000 psi (Superstructure)f'c = 3,500 psi (Substructure)fy = 60,000 psi (Reinforcement) fy = 50,000 psi (M270 Grade 50)All structural steel shall be painted

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1Design Spectral Acceleration at 1.0 sec. (SD1) = 0.086 Design Spectral Acceleration at 0.2 sec. (Sps) = 0.143 Soil Site Class = D

- 15' Bridge Approach

Bridge Approach

Footing, typ.

(to be removed)

Exist. Abut.

Bot, of Fta.

Elev. 660.93

6" Conc. Slopewall

(see Ramp Plans)

-Ramp (see

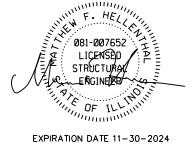
Ramp Plans)

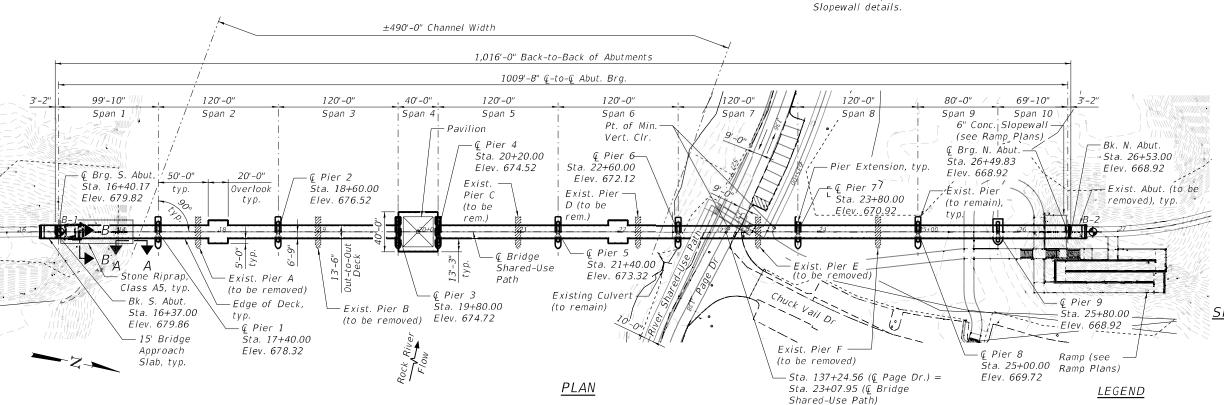
Soil Boring

Slab, typ.



DATE: 07-22-2024





Low Steel over

Exist. Pier

(to remain),

typ.

Flev. 666.23

ELEVATION (Looking west)

Channel

Pavilion

DHWE 647.98

Pedestrian

— Light Pole, typ.

Pier Extension,

EWSE 637.70

Range 9E, 4th P.M.

LOCATION SKETCH

GENERAL PLAN AND ELEVATION SHARED-USE PATH EXTENSION OVER ROCK RIVER AND PAGE DRIVE PUBLIC WATER SEC. 22-00183-00-BR LEE COUNTY STATION 23+07.95 <u>STRUCTURE</u> NO. 052-0082

.	REVISION	DATE	BY	REMARKS	DESIGNED	AED	Г
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2					CHECKED	MFH	
ì١					DRAWN	RMG	
ŭ					CHECKED	KMG	
`					CHECKED	MELL	

benesch	Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10869.00
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CITY OF DIXON RIVER CROSSING SHARED-USE PATH 2024



	F.A.P. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
		22-00183-00-BR LE		LEE	315	104	
011557 NO. 0 4 05 0 50 0115570		WHA# 1369	D22		CONTRAC	T NO. 8	5762
SHEET NO. S-1 OF S-50 SHEETS			ILLINOIS	FED, All	D PROJECT 5LY7(9	16)	

- 1. Fasteners shall be ASTM F 3125 Grade A325 Type 1, mechanically galvanized bolts in painted areas. Bolts shall be $\frac{7}{8}$ " Ø, in $\frac{15}{16}$ " Ø holes, unless otherwise noted.
- 2. Calculated weight of Structural Steel = 337,118 lbs.
- 3. All structural steel shall be AASHTO M270 Grade 50.
- 4. No field welding is permitted except as specified in the contract documents.
- 5. Reinforcement bars designated (E) shall be epoxy coated.
- 6. If the Contractor elects to use cantilever forming brackets on the exterior girders or outlook supports, the brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and the first interior girder at each of these additional bracket locations.
- 7. Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of $\frac{1}{8}$ inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- 8. Concrete stain and anti-graffiti coating shall be applied to all exposed surfaces of the abutments and piers, including existing concrete. The bottom limit shall be taken 1'-0" below grade on land and the EWSE in the
- 9. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- 10. The Contractor shall obtain a construction permit from the Illinois Department of Natural Resources (IDNR), Office of Water Resources for any temporary construction activity placed in the water except cofferdams. This shall include the placement of material for run-arounds, causeways, etc. Any permit application by the Contractor shall refer to the IDNR 3704 Floodway Construction permit number allowing permanent construction as shown in the contract plans (Permit No. DS2024051).
- 11. Plan dimensions and details relative to the existing structure have been taken from survey and inspection notes and are subject to nominal variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.
- 12. The Inorganic Zinc Rich Primer/Acrylic/Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all steel surfaces shall be gray, Munsell No 5B 7/1 unless noted otherwise.
- 13. No in stream work shall be conducted from April 1st through July 31st of any given year.
- 14. The Contractor shall determine the means and methods of temporary river access and temporary works for pier removal and bridge construction. All work associated with temporary river access and temporary works for pier removal and bridge construction is not paid separately and is incidental to the items requiring work in or above the river.

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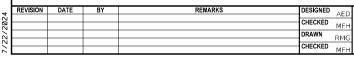
5-49 Existing Heritage Crossing Pavilion Details (1 of 2) Existing Heritage Crossing Pavilion Details (2 of 2)

TOTAL BILL OF MATERIAL

85762

ITEM	UNIT	SUPER	SUB	TOTAL
Stone Riprap, Class A5	Sq. Yd.		238	238
Filter Fabric	Sq. Yd.		238	238
Concrete Removal	Cu. Yd.		1,400.1	1,400.1
Structure Excavation	Cu. Yd.		203	203
Concrete Structures	Cu. Yd.		541.4	541.4
Concrete Superstructure	Cu. Yd.	378.1		378.1
Form Liner Textured Surface	Sq. Ft.		4,136	4,136
Protective Coat	Sq. Yd.	1,729		1,729
Concrete Superstructure (Approach Slab)	Cu. Yd.	10.0		10.0
Furnishing and Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	5,688		5,688
Reinforcement Bars, Epoxy Coated	Pound	101,880	75,220	177,100
Pedestrian Railing	Foot	2,191		2,191
Furnishing Steel Piles HP12X53	Foot		390	390
Driving Piles	Foot		390	390
Test Pile Steel HP12x53	Each		2	2
Pile Shoes	Each		12	12
Name Plates	Each	1		1
Preformed Joint Strip Seal	Foot	54		54
Elastomeric Bearing Assembly, Type I	Each	16		16
Elastomeric Bearing Assembly, Type II	Each	4		4
Elastomeric Bearing Assembly, Type III	Each	2		2
Anchor Bolts, 3/4"	Each	32		32
Anchor Bolts, 1"	Each	20		20
Anchor Bolts, 1 1/2"	Each	8		8
Granular Backfill for Structures	Cu. Yd.		30	30
Epoxy Crack Injection*	Foot		270	270
Geocomposite Wall Drain	Sq. Yd.		16	16
Concrete Headwalls for Pipe Drains	Each		2	2
Pipe Underdrains for Structures 4"	Foot		71	71
Anti-Graffiti Coating	Sq. Ft.		19,924	19,924
Potholing	Each		1	1
Staining Concrete Structures	Sq. Ft.		19,924	19,924
Bar Terminators	Each	60		60
Underwater Concrete Repair*	Sq. Ft.		120	120
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)*	Sg. Ft.		720	720
Structural Repair of Concrete (Depth Greater Than 5 Inches)*	Sg. Ft.		180	180
Pavilion Structure	L. Sum	1		1

^{*}Pay item quantity includes an allowance. Final repair areas shall be determined in the field and coordinated with the Engineer during construction. See Sheet S-7 for additional information.

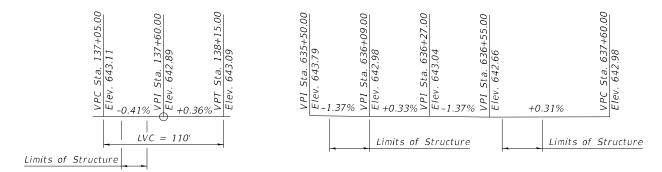






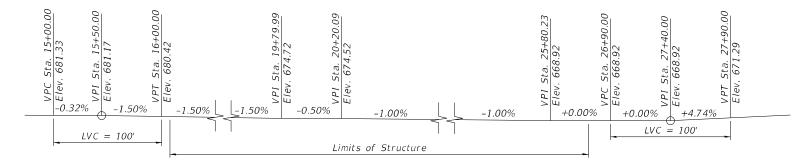


GENERAL NOTES, BILL OF MATERIALS AND INDEX OF SHEETS	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
STRUCTURE NO. 052-0082		22-00183-00-BR	LEE	315	105	
		WHA# 1369D22	CONTRAC	T NO. 8	35762	
SHEET NO. S-2 OF S-50 SHEETS	ILLINOIS EED AID PROJECT 5177(916)					



PROFILE GRADE (Along & Page Drive)

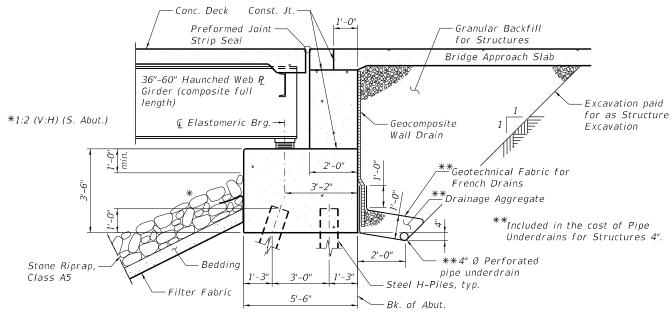
PROFILE GRADE (Along & River Shared-Use Path)



PROFILE GRADE (Along & Bridge Shared-Use Path)

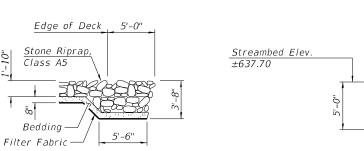
ROCK RIVER BUILT 202_ BY CITY OF DIXON LEE COUNTY STA 23+07.95 STR. NO. 052-0082 - H10 LOADING

> NAME PLATE (See Std. 515001)

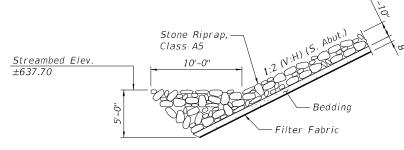


SECTION THRU PILE SUPPORTED STUB ABUTMENT

(S. Abut. with riprap shown, N. Abut. with slopewall similar; see Ramp plans for slopewall details)



SECTION B-B



SECTION A-A

NOTES:

1. All drainage system components shall extend parallel to the abutment back wall until they intersect the wingwalls. The pipe shall extend under the west wingwall until intersecting the west side slope. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

LEE

315 106 CONTRACT NO. 85762

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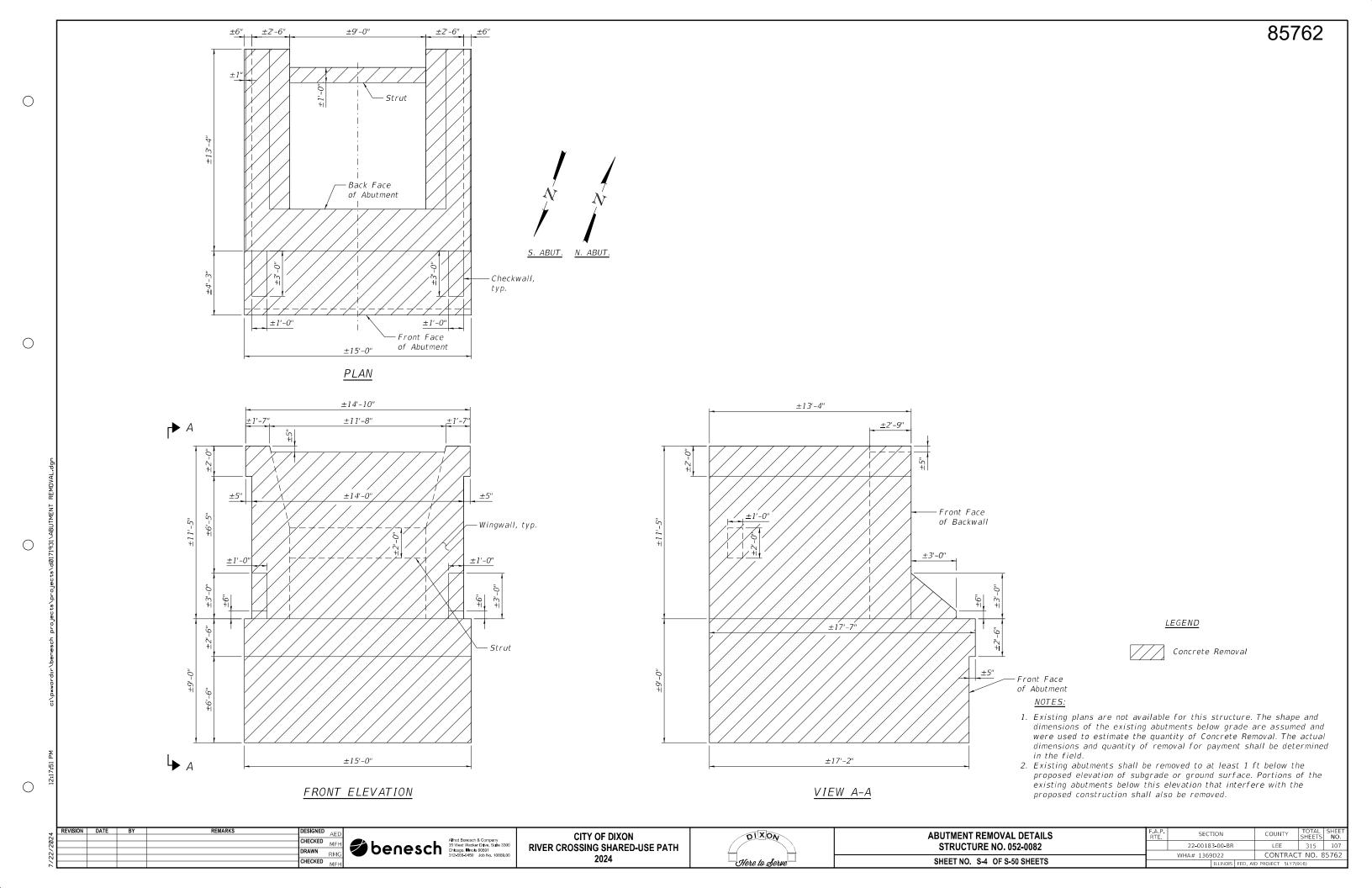
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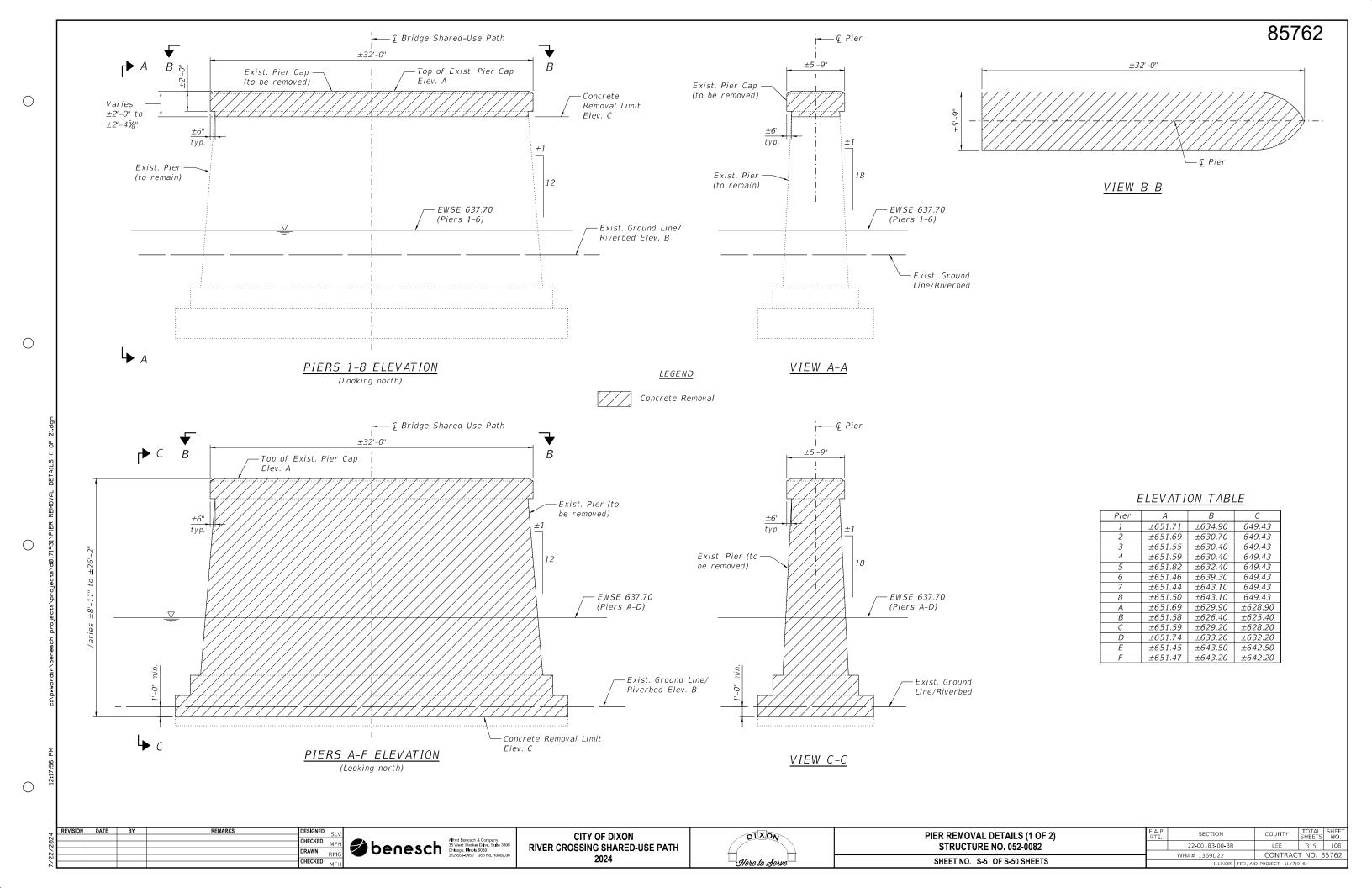


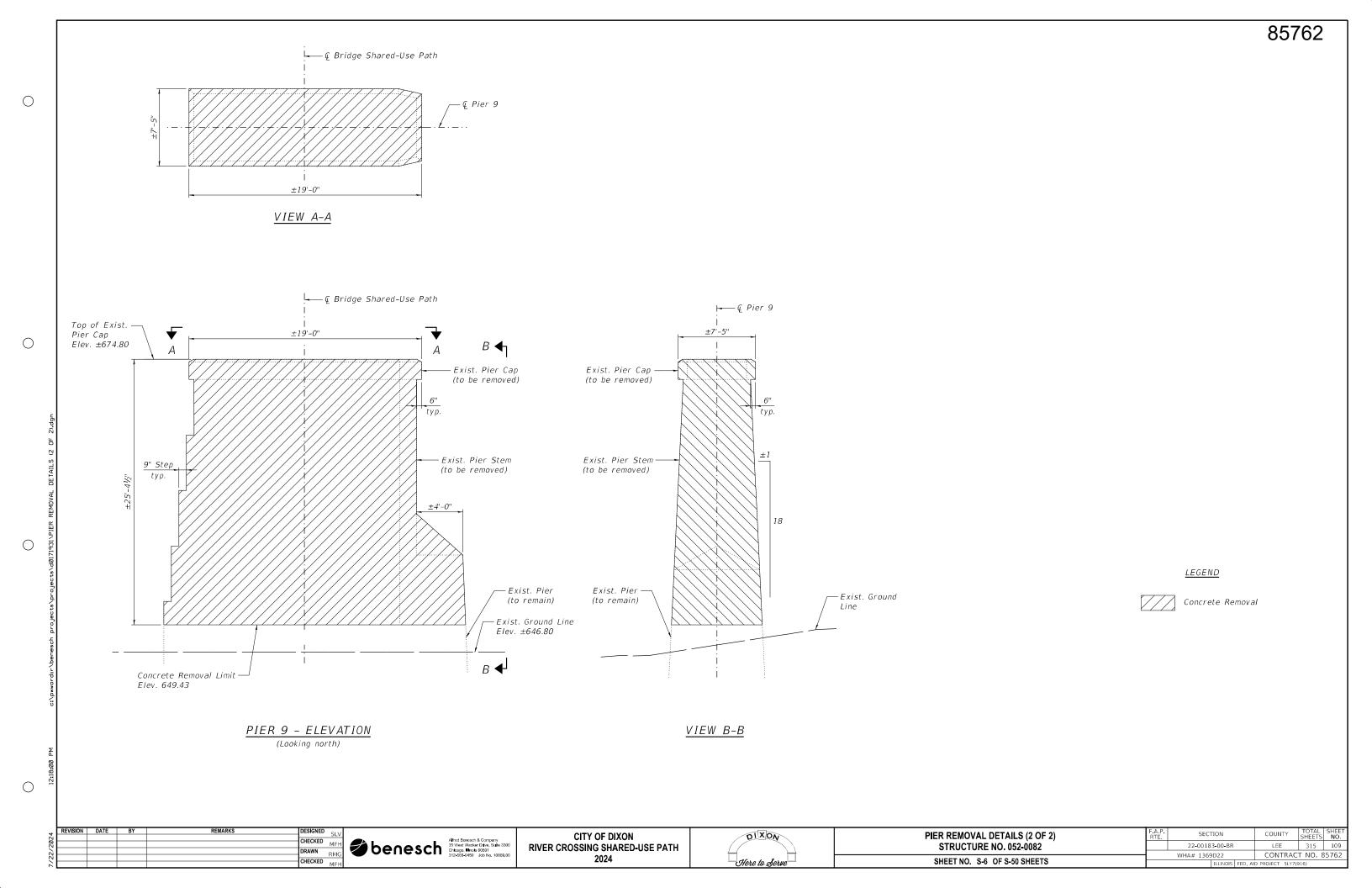
CITY OF DIXON RIVER CROSSING SHARED-USE PATH 2024

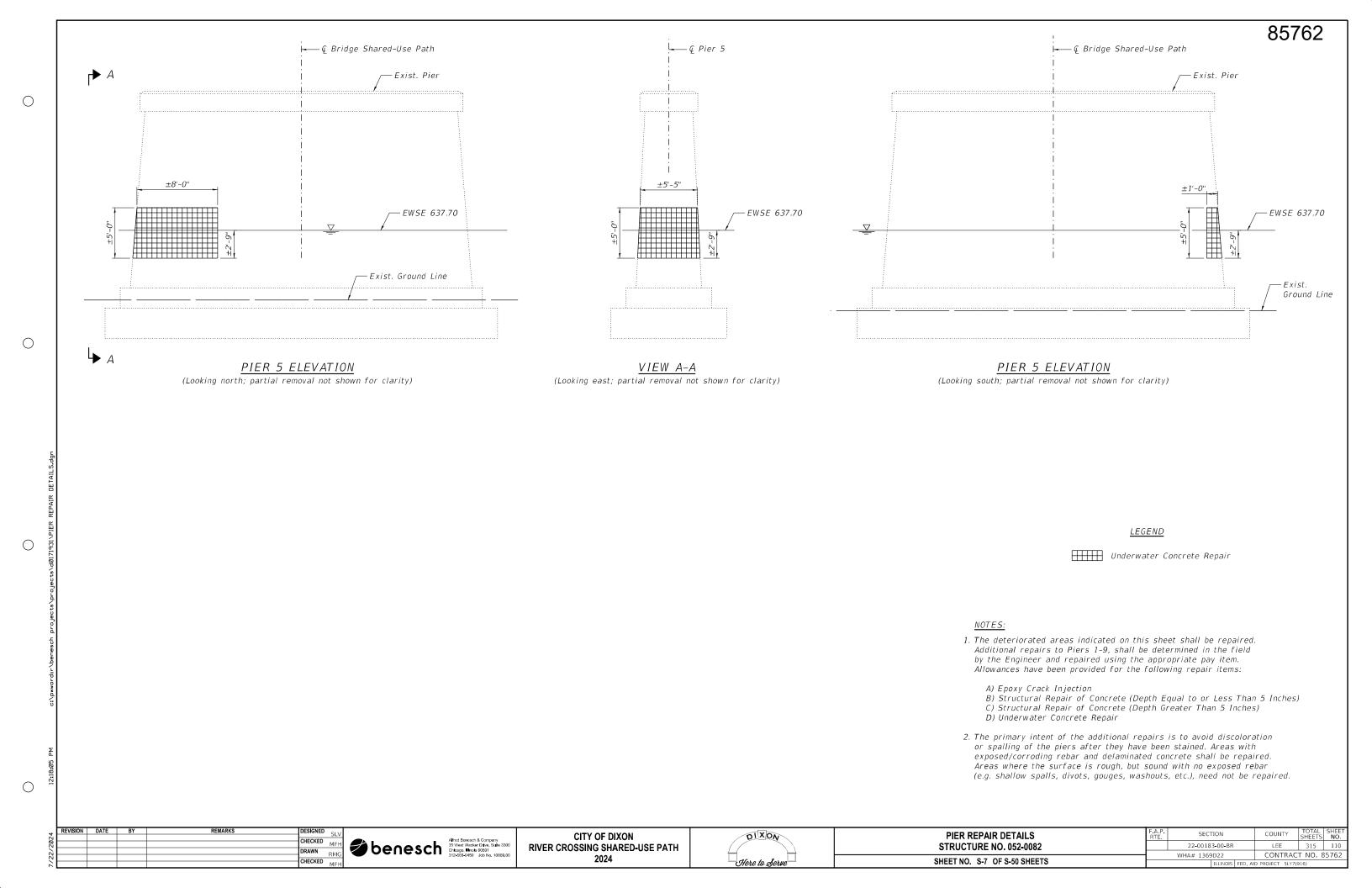


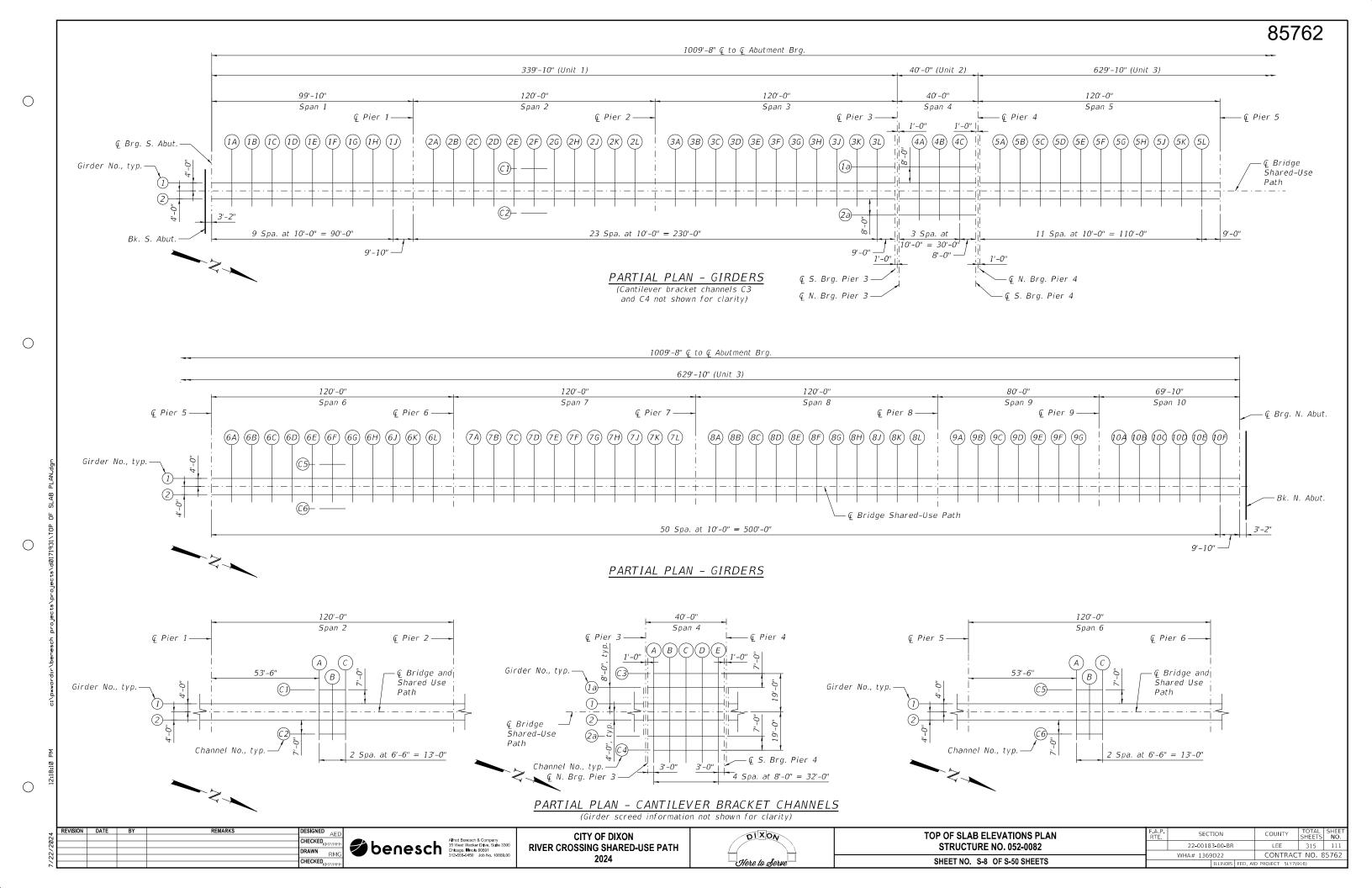
GENERAL DATA	F.A.P. RTE	SEC	TION
STRUCTURE NO. 052-0082		22-0018	3-00-BR
	WHA# 1369D22		
SHEET NO. S-3 OF S-50 SHEETS			ILLINOIS











GIRDER 1 GIRDER 1 (CONT'D) © BRIDGE SHARED-USE PATH heoretical Grade Theoretical Grade Theoretical Grade heoretica heoretica. heoretica Elevations Elevations Elevations Offset Offset Station Off set Location Station Grade Location Station Grade Location Grade diusted For Dead Adiusted For Dead Adiusted For Dea Elevations Elevations Flevations Load Deflection Load Deflection Load Deflection Bk. S. Abut. 16+37.00 -4.00679.80 679.80 CL. Brg. Pier 5 21+40.00 673.26 673.26 Bk. S. Abut. 16+37.00 0.00 679.86 679.86 CL. Brg. S. Abut. 16+40.17 -4.00 679.76 679.76 21+50.00 -4.00 673.16 673.13 CL. Brg. S. Abut. 16+40.17 0.00 679.82 679.82 -4.00 673.02 6B 21+60.00 673.06 16+50.17 -4.00679.61 679.68 6C 21+70.00 -4.00672.96 672.93 16+50.17 0.00 679.67 679.74 1B 16+60.17 -4.00679.46 679.59 6D 21+80.00 -4.00 672.86 672.84 1B 16+60.17 0.00 679.52 679.65 679.55 1 C 16+70.17 -4 00 679 31 679 49 6E -4 00 672.76 1 C 0.00 21+90.00 672 76 16+70.17 679.37 1 D 16+80.17 -4.00 679.16 679.35 6F 22+00.00 -4.00 672.66 672.68 1 D 16+80.17 0.00 679.22 679.41 1 E 679.20 672.58 1 E 679.26 16+90.17 -4.00679.01 22+10.00 -4.00672.56 16+90.17 0.00 679.07 6G 1 F 17+00.17 -4.00678.86 679.02 6Н 22+20.00 -4.00672.46 672.48 1 F 17+00.17 0.00 678.92 679.08 1 G 17+10.17 -4.00 678.71 678.83 6J 22+30.00 -4.00 672.36 672.37 1 G 17+10.17 0.00 678.77 678.89 17+20.17 678.63 22+40.00 -4 00 1 H 17+20.17 678.62 1 H -4 00 678.56 6K 672.26 672.26 0.00 678.69 1 J 17+30.17 -4.00 678.41 678.44 6L 22+50.00 -4.00 672.16 672.16 1 J 17+30.17 0.00 678.47 678.50 CL. Brg. Pier 1 17+40.00 -4.00 678.26 678.26 CL. Brg. Pier 6 22+60.00 -4.00 672.06 672.06 CL. Brg. Pier 1 17+40.00 0.00 *678.32 678.32* 17+50.00 678 11 678.09 -4 00 671 98 678.17 678.15 2A -4.007 A 22+70.00 671.96 2A 17+50.00 0.00 2B 17+60.00 -4.00677.96 677.94 7*B* 22+80.00 -4.00 671.86 671.90 2B 17+60.00 0.00 678.02 678.00 17+70.00 677.81 677 79 22+90.00 -4 00 671.83 17+70.00 0.00 677.87 677.85 2C -4 00 7.0 67176 2C 2D 17+80.00 -4.00 677.66 677.64 7 D 23+00.00 -4.00671.66 671.76 2D 17+80.00 0.00 677.72 677.70 2E 17+90.00 -4.00 677.51 677.50 7 E 23+10.00 -4.00 671.56 671.68 2E 17+90.00 0.00 677.57 677.56 2F 677 34 7 F 2F 677 40 18+00.00 -4 00 677 36 23+20.00 -4 00 671 46 671 58 18+00.00 0.00 677 42 2G 18+10.00 -4.00677.21 677.18 7 G 23+30.00 -4.00671.36 671.46 2G 18+10.00 0.00 677.27 677.24 2H -4 00 2H 677.08 18+20.00 -4 00 677.06 677.02 7 H 23+40.00 671.26 671 34 18+20.00 0.00 677 12 2J 18+30.00 -4.00 676.91 676.86 7 J 23+50.00 -4.00 671.16 671.21 2J 18+30.00 0.00 676.97 676.92 2K 18+40.00 -4.00 676.76 676.71 7 K 23+60.00 -4.00 671.06 671.08 2K 18+40.00 0.00 676.82 676.77 2L 18+50.00 -4.00 676.61 676.58 7L 23+70.00 -4.00 670.96 670.96 2L 18+50.00 0.00 676.67 676.64 18+60.00 23+80.00 -4.00 CL. Brg. Pier 2 -4.00 676.46 676.46 CL. Brg. Pier 7 670.86 670.86 CL. Brg. Pier 2 18+60.00 0.00 676.52 676.52 18+70.00 676.36 23+90.00 -4.00 670.76 670.78 18+70.00 676.37 676.42 3A -4.00 676.31 8A 3A 0.00 3B 18+80.00 -4.00 676.16 676.28 8B 24+00.00 -4.00 670.66 670.71 3B 18+80.00 0.00 676.22 676.34 3C 18+90.00 -4 00 676.01 676.20 8C 24+10.00 -4.00 670.56 670.65 3C 18+90.00 0.00 676.07 676.26 3D 19+00.00 -4 00 675.86 8D -4 00 670 46 670 59 3D 0.00 675.92 676.19 676 13 24+20.00 19+00.00 3E 19+10.00 -4.00 675.71 676.04 8E 24+30.00 -4.00 670.36 670.52 3E 19+10.00 0.00 675.77 676.10 3F 19+20.00 675.94 3F -4 00 675 56 8F 24+40.00 -4 00 670 26 670 44 19+20.00 0.00 67562 676.00 3G 19+30.00 -4.00 675.41 675.80 8G 24+50.00 -4.00 670.16 670.34 3G 19+30.00 0.00 675.47 675.86 ЗН 3H 19+40.00 -4.00 675.26 675.62 8Н 24+60.00 -4.00 670.06 670.22 19+40.00 0.00 675.32 675.68 ЗJ 19+50.00 675.41 ЗJ -4.00 24+70.00 -4.00 670.08 19+50.00 0.00 675.17 675.47 675.11 8 J 669.96 3K 19+60.00 -4.00 674.96 675.18 24+80.00 -4.00 669.94 3K 19+60.00 675.02 675.24 8K 669.86 0.00 31 19+70.00 -4.00 674.81 674.92 81 24+90.00 -4.00 669.76 669.79 31 19+70.00 0.00 674.87 674.98 CL. S. Brg. Pier 3 19+79.00 -4.00 674.67 674.67 CL. Brg. Pier 8 25+00.00 -4.00 669.66 669.66 CL. S. Brg. Pier 3 19+79.00 0.00 674.73 674.73 19+80.00 674.66 25+10.00 -4.00 669.54 19+80.00 674.72 674.72 CL. Pier 3 -4.00 674.66 9A 669.56 CL. Pier 3 0.00 25+20.00 -4.00 669.43 9B669.46 CL. N. Brg. Pier 3 19+81.00 -4.00 674.65 674.65 9C 25+30.00 -4.00 669.36 669.33 CL. N. Brg. Pier 3 19+81.00 0.00 674.71 674.71 9D 25+40.00 -4.00 669.26 669.24 19+91.00 674.61 674.62 67467 674.68 -4.009E 25+50.00 -4.00 669.16 669.14 19+91.00 0.00 674.56 674.58 25+60.00 669.06 674.64 20+01.00 -4.00 9F -4.00 669.06 20+01.00 0.00 674.62 674.52 674.58 4C 20+11.00 -4.00 674.51 9G 25+70.00 -4.00 668.96 668.96 40 20+11.00 0.00 674.57 CL. S. Brg. Pier 4 20+19.00 -4.00 674.47 674.47 CL. Brg. Pier 9 25+80.00 -4.00 668.86 668.86 CL. S. Brg. Pier 4 20+19.00 0.00 674.53 674.53 CL. Pier 4 20+20.00 -4.00 674.46 674.46 10A 25+90.00 -4.00 668.86 668.88 CL. Pier 4 20+20.00 0.00 674.52 674.52 10B 26+00.00 -4.00668.86 668.89 CL. N. Brg. Pier 4 20+21.00 -4.00 674.45 674.45 10C 26+10.00 -4.00 668.86 668.91 CL. N. Brg. Pier 4 20+21.00 674.51 674.51 0.00 10D 26+20.00 -4.00 668.86 668.91 20+31.00 -4.00674.35 674.47 10E 26+30.00 -4.00 668.86 668.91 5A 20+31.00 0.00 674.41 674.53 674.53 5B 20+41.00 -4.00 674.25 674.47 10F 26+40.00 -4.00 668.86 668.89 5B 20+41.00 0.00 674.31 5C 5C -4.00674.15 674.45 0.00 674.21 674.51 20+51.00 20+51.00 5D 20+61.00 -4.00 674.05 674.40 CL. Brg. N. Abut. 26+49.83 -4.00 668.86 5D 0.00 674.11 674.46 668.86 20+61.00 5E 20+71.00 -4.00 673.95 674.32 5E 20+71.00 0.00 674.01 674.38 5F -4.00 668.86 5F 20+81.00 -4.00 673.85 674.21 Bk. N. Abut 26+53.00 668.86 20+81.00 0.00 673.91 674.27 5G 5G 20+91.00 -4.00 673.75 674.06 20+91.00 0.00 673.81 674.12 5H 21+01.00 673.65 673.89 5H 673.95 -4.0021+01.00 0.00 673.71 5J 21+11.00 -4.00 673.55 673.72 5J 21+11.00 673.61 673.78 0.00 5K -4.00 5K 21+21.00 673.45 673.55 21+21.00 0.00 673.51 673.61 51 21+31.00 -4.00673.35 673.39 5L 21+31.00 0.00 673.41 673.45 SECTION COUNTY CITY OF DIXON DIXON **TOP OF SLAB ELEVATIONS (1 OF 3)** SHEETS NO. CHECKED benesch 35 West Wacker Drive, Suite 3300 Chicago, Illnois 60600, John 10,0000 22-00183-00-BR LEE 315 112 RIVER CROSSING SHARED-USE PATH STRUCTURE NO. 052-0082 DRAWN WHA# 1369D22 CONTRACT NO. 85762

2024

Here to Serve

SHEET NO. S-9 OF S-50 SHEETS

CHECKED

	⊊ BRI	DGE SHARE	D-USE P	PATH (CO	<u>NT'D)</u>		<u> G11</u>	RDER 2				<u>GIRDE F</u>	R 2 (CONT	<u> </u>	
				T/	Theoretical Grade				T (Theoretical Grade				T/:	T7
Locati	ion	Station	Offset	Theoretical Grade Elevations	Elevations Adjusted For Dead Load Deflection	Location	Station	Offset	Theoretical Grade Elevations	Elevations Adjusted For Dead Load Deflection	Location	Station	Offset	Theoretical Grade Elevations	Ac
CL. Brg.	Pier 5	21+40.00	0.00	673.32	673.32	Bk. S. Abut.	16+37.00	+4.00	679.80	679.80	CL. Brg. Pier 5	21+40.00	+4.00	673.26	
	6A	21+50.00	0.00	673.22	673.19	CL. Brg. S. Abut.	16+40.17	+4.00	679.76	679.76	6A	21+50.00	+4.00	673.16	
	6B	21+60.00	0.00	673.12	673.08						6B	21+60.00	+4.00	673.06	
	6C	21+70.00	0.00	673.02	672.99	1A	16+50.17	+4.00	679.61	679.68	6C	21+70.00	+4.00	672.96	
	6D	21+80.00	0.00	672.92	672.90	18	16+60.17	+4.00	679.46	679.59	6D	21+80.00	+4.00	672.86	
	6E	21+90.00	0.00	672.82	672.82	1C	16+70.17	+4.00	679.31	679.49	6E	21+90.00	+4.00	672.76	
	6F	22+00.00	0.00	672.72	672.74	1D	16+80.17	+4.00	679.16	679.35	6F	22+00.00	+4.00	672.66	
	6G 6H	22+10.00 22+20.00	0.00	672.62 672.52	672.64 672.54	1 <i>E</i> 1 <i>F</i>	16+90.17 17+00.17	+4.00 +4.00	679.01 678.86	679.20 679.02	6G 6H	22+10.00 22+20.00	+4.00 +4.00	672.56 672.46	
	6J	22+30.00	0.00	672.42	672.43	1 G	17+00.17 17+10.17	+4.00	678.71	678.83	6J	22+20.00 22+30.00	+4.00	672.36	
	6K	22+30.00 22+40.00	0.00	672.32	672.32	1H	17+10.17	+4.00	678.56	678.63	6K	22+40.00	+4.00	672.26	
	6L	22+50.00	0.00	672.22	672.22	17	17+30.17	+4.00	678.41	678.44	6L	22+50.00	+4.00	672.16	
CL. Brg.	Pier 6	22+60.00	0.00	672.12	672.12	CL. Brg. Pier 1	17+40.00	+4.00	678.26	678.26	CL. Brg. Pier 6	22+60.00	+4.00	672.06	
	7 <i>A</i>	22+70.00	0.00	672.02	672.04	2A	17+50.00	+4.00	678.11	678.09	7A	22+70.00	+4.00	671.96	
	7 <i>B</i>	22+80.00	0.00	671.92	671.96	2B	17+60.00	+4.00	677.96	677.94	7 <i>B</i>	22+80.00	+4.00	671.86	
	7C	22+90.00	0.00	671.82	671.89	2C	17+70.00	+4.00	677.81	677.79	7 <i>C</i>	22+90.00	+4.00	671.76	
	7D	23+00.00	0.00	671.72	671.82	2D	17+80.00	+4.00	677.66	677.64	7 <i>D</i>	23+00.00	+4.00	671.66	
	7 <i>E</i>	23+10.00	0.00	671.62	671.74	2E	17+90.00	+4.00	677.51	677.50	7 <i>E</i>	23+10.00	+4.00	671.56	
	7 <i>F</i>	23+20.00	0.00	671.52	671.64	2F	18+00.00	+4.00	677.36	677.34	7 <i>F</i>	23+20.00	+4.00	671.46	
	7 <i>G</i>	23+30.00	0.00	671.42	671.52	2 <i>G</i>	18+10.00	+4.00	677.21	677.18	7 <i>G</i>	23+30.00	+4.00	671.36	
	7 <i>H</i>	23+40.00	0.00	671.32	671.40	2H	18+20.00	+4.00	677.06	677.02	7 <i>H</i>	23+40.00	+4.00	671.26	
	7J	23+50.00	0.00	671.22	671.27	2J	18+30.00	+4.00	676.91	676.86	7J	23+50.00	+4.00	671.16	
	7 <i>K</i> 7 <i>L</i>	23+60.00 23+70.00	0.00 0.00	671.12 671.02	671.14 671.02	2K 2L	18+40.00 18+50.00	+4.00 +4.00	676.76 676.61	676.71 676.58	7 <i>K</i> 7 <i>L</i>	23+60.00 23+70.00	+4.00 +4.00	671.06 670.96	
CL. Brg.	Pier 7	23+80.00	0.00	670.92	670.92	CL. Brg. Pier 2	18+60.00	+4.00	676.46	676.46	CL. Brg. Pier 7	23+80.00	+4.00	670.86	
	8A	23+90.00	0.00	670.82	670.84	3A	18+70.00	+4.00	676.31	676.36	8A	23+90.00	+4.00	670.76	
	8B	24+00.00	0.00	670.72	670.77	3 <i>B</i>	18+80.00	+4.00	676.16	676.28	8B	24+00.00	+4.00	670.66	
	8C	24+10.00	0.00	670.62	670.71	3 <i>C</i>	18+90.00	+4.00	676.01	676.20	8C	24+10.00	+4.00	670.56	
	8D	24+20.00	0.00	670.52	670.65	3D	19+00.00	+4.00	675.86	676.13	8D	24+20.00	+4.00	670.46	
	8E	24+30.00	0.00	670.42	670.58	3E	19+10.00	+4.00	675.71	676.04	8E	24+30.00	+4.00	670.36	
	8F	24+40.00	0.00	670.32	670.50	3F	19+20.00	+4.00	675.56	675.94	8F	24+40.00	+4.00	670.26	
	8G	24+50.00	0.00	670.22	670.40	3G	19+30.00	+4.00	675.41	675.80	8G	24+50.00	+4.00	670.16	
	8H	24+60.00	0.00	670.12	670.28	3H	19+40.00	+4.00	675.26	675.62	8H	24+60.00	+4.00	670.06	
	8 J	24+70.00	0.00	670.02	670.14	3J	19+50.00	+4.00	675.11	675.41	8.7	24+70.00	+4.00	669.96	
	8K 8L	24+80.00 24+90.00	0.00 0.00	669.92 669.82	670.00 669.85	3K 3L	19+60.00 19+70.00	+4.00 +4.00	674.96 674.81	675.18 674.92	8K 8L	24+80.00 24+90.00	+4.00 +4.00	669.86 669.76	
CL. Brg.		25+00.00	0.00	669.72	669.72	CL. S. Brg. Pier 3	19+79.00	+4.00	674.67	674.67	CL. Brg. Pier 8	25+00.00	+4.00	669.66	
9.		25+10.00	0.00	669.62	669.60	CL. Pier 3	19+80.00	+4.00	674.66	674.66	94	25+10.00	+4.00	669.56	
	9A 9B	25+20.00	0.00	669.52	669.49	CL. Tier 5	19+00.00	74.00	074.00	074.00	9B	25+20.00	+4.00	669.46	
	9C	25+30.00	0.00	669.42	669.39	CL. N. Brg. Pier 3	19+81.00	+4.00	674.65	674.65	90	25+30.00	+4.00	669.36	
	9D	25+40.00	0.00	669.32	669.30	02, 2, g	13101700	' ' ' '	0, 1,00	37 7,05	9D	25+40.00	+4.00	669.26	
	9E	25+50.00	0.00	669.22	669.20	4A	19+91.00	+4.00	674.61	674.62	9 <i>E</i>	25+50.00	+4.00	669.16	
	9F	25+60.00	0.00	669.12	669.12	4B	20+01.00	+4.00	674.56	674.58	9F	25+60.00	+4.00	669.06	
	9G	25+70.00	0.00	669.02	669.02	4C	20+11.00	+4.00	674.51	674.52	9G	25+70.00	+4.00	668.96	
CL. Brg.	Pier 9	25+80.00	0.00	668.92	668.92	CL. S. Brg. Pier 4	20+19.00	+4.00	674.47	674.47	CL. Brg. Pier 9	25+80.00	+4.00	668.86	
	10A	25+90.00	0.00	668.92	668.94	CL. Pier 4	20+20.00	+4.00	674.46	674.46	10A	25+90.00	+4.00	668.86	
	10B	26+00.00	0.00	668.92	668.95			1			10B	26+00.00	+4.00	668.86	1
	10C	26+10.00	0.00	668.92	668.97	CL. N. Brg. Pier 4	20+21.00	+4.00	674.45	674.45	10C	26+10.00	+4.00	668.86	1
	10D	26+20.00	0.00	668.92	668.97	1		1 .			10D	26+20.00	+4.00	668.86	
	10E	26+30.00	0.00	668.92	668.97	5A	20+31.00	+4.00	674.35	674.47	10E	26+30.00	+4.00	668.86	
	10F	26+40.00	0.00	668.92	668.95	5B	20+41.00	+4.00	674.25	674.47	10F	26+40.00	+4.00	668.86	
61 5		26:40.22	0.00	660.00	660.03	5C	20+51.00	+4.00	674.15	674.45	61.5	26:46.55		650.55	
CL. Brg. N	i. Abut.	26+49.83	0.00	668.92	668.92	5D	20+61.00	+4.00	674.05	674.40	CL. Brg. N. Abut.	26+49.83	+4.00	668.86	
DI: **	l Abut	26 / 52 00	0.00	660.00	660.03	5E	20+71.00	+4.00	673.95	674.32	DI. N. Alexa	26,52,00	1 , 4 00	660.06	
BK. N	I. Abut.	26+53.00	0.00	668.92	668.92	5F	20+81.00	+4.00	673.85	674.21	Bk. N. Abut.	26+53.00	+4.00	668.86	上
						5G 5H	20+91.00 21+01.00	+4.00 +4.00	673.75 673.65	674.06 673.89					
						5H 5J	21+01.00 21+11.00	+4.00	673.55	673.72					
						5K	21+11.00 21+21.00	+4.00	673.45	673.55					
							, ,	, ,,,,,,	3,3,73	2.2.22					

REMARKS	DESIGNED AED	Τ
	KMP/MFH	
	DRAWN RMG	
		CHECKED

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Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10869.00
3





TOP OF SLAB ELEVATIONS (2 OF 3)	F.A.P. RTE	SEC*	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 052-0082		22-0018	3-00-BR		LEE	315	113
		WHA# 1369	מחמ		CONTRAC	T NO S	5762
SHEET NO. S-10 OF S-50 SHEETS		WITH# 150:	1022		CONTINAC	, i ivo. c	37702
			HUMOR	EED AH	D DROJECT ELV7	0.163	

85762

Theoretical Grade Elevations Adjusted For Dead Load Deflection

673.26

673.13 673.02 672.93

672.84

672.76

672.68

672.58 672.48

672.37

672.26

672.16

672.06

671.98

671.90

671.83

671.76

671.68

671.58 671.46

671.34 671.21

671.08

670.96

670.86

670.78 670.71

670.65

670.59

670.52

670.44 670.34

670.22

670.08

669.94

669.79

669.66

669.54 669.43 669.33

669.24

669.14 669.06

668.96

668.86

668.88 668.89 668.91

668.91

668.91

668.89

668.86

668.86

85762

Theoretical Grade heoretica Elevations Location Station Off set Grade Adjusted For Dead Elevations Load Deflection CL. Pier 3 19+80.00 674.54 674.54 CL. N. Brg. Pier 3 19+81.00 -12.00 674.53 674.53 674.49 19+91.00 -12.00 674.50 4B 20+01.00 -12.00 674.44 674.46 -12.00 674.39 674.40 20+11.00 4C CL. S. Brg. Pier 4 20+19.00 -12.00 674.35 674.35 CL. Pier 4 20+20.00 -12.00 674.34 674.34

GIRDER 2a

Offset

12.00

12.00

12.00

12.00

12.00

12.00

12.00

Off set

-11.00

-11.00

-11.00

674.39

674.35

674.34

heoretica

Grade

Elevations

677.35

677.26

677.16

heoretical

Grade

Elevations

674.42

674.38

674.34

674.30

674.26

674.40

674.35

674.34

Theoretical Grade

Elevations

Adjusted For Dead

Load Deflection

677.34

677.24

677.13

heoretical Grade

Elevations

Adjusted For Dead

Load Deflection

674.43

674.40

674.36

674.32

674.27

Station

19+80.00

19+81.00

19+91.00

20+01.00

20+11.00

20+19.00

20+20.00

Station

17+93.50

18+00.00

18+06.50

Station

19+84.00

19+92.00

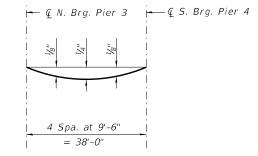
20+00.00

20+08.00

20+16.00

GIRDER 1a

4 Spa. at 24'-111/2" | 4 Spa. at 30'-0" 4 Spa. at 29'-9" = 99'-10" = 120'-0''= 119'-0''



DEAD LOAD DEFLECTION DIAGRAM - UNIT 1

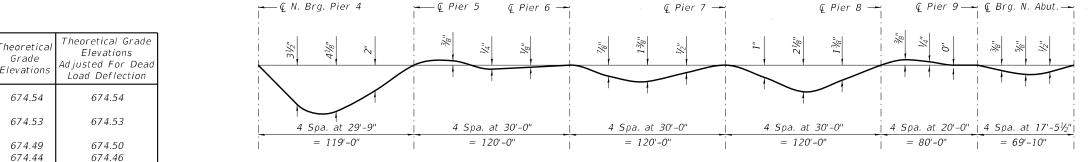
Pier 1 & Pier 2

- G Brg. S. Abut.

(Includes weight of concrete only.)

DEAD LOAD DEFLECTION DIAGRAM - UNIT 2

(For Girders 1a, 1, 2, & 2a. Includes weight of concrete only.)



DEAD LOAD DEFLECTION DIAGRAM - UNIT 3

€ S. Brg. Pier 3 -

(Includes weight of concrete only.)

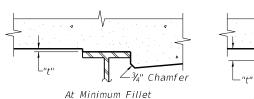
Note:

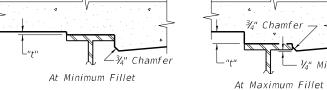
The above deflections are not to be used in the field if the Engineer is working from the grade elevations adjusted for dead load deflections as shown on this sheet and Sheets S-9 to S-10.

CANTILEVER BRACKET CHANNEL C1

Location	Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
A	19+84.00	19.00	674.42	674.43
B	19+92.00	19.00	674.38	674.40
C	20+00.00	19.00	674.34	674.36
D	20+08.00	19.00	674.30	674.32
E	20+16.00	19.00	674.26	674.27

CANTILEVER BRACKET CHANNEL C4





CANTILEVER BRACKET CHANNEL C2

Location	Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
А	17+93.50	11.00	677.35	677.34	
В	18+00.00	11.00	677.26	677.24	
С	18+06.50	11.00	677.16	677.13	

CANTILEVER BRACKET CHANNEL C3

Offset

-19.00

-19.00

-19.00

-19.00

-19.00

CANTILEVER BRACKET CHANNEL C5

Location	Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
A	21+93.50	-11.00	672.62	672.63
B	22+00.00	-11.00	672.56	672.57
C	22+06.50	-11.00	672.49	672.51

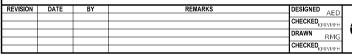
To determine "t": After all structural steel has been erected, elevations of the top flanges of the girders shall be taken at intervals shown on Sheet S-8. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on this sheet and on Sheets S-9 to S-10, minus the 7" slab thickness, equals the fillet heights "t" above top flange of girders.

CANTILEVER BRACKET CHANNEL C6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
A	21+93.50	11.00	672.62	672.63		
B	22+00.00	11.00	672.56	672.57		
C	22+06.50	11.00	672.49	672.51		

FILLET HEIGHTS

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
A	21+93.50	11.00	672.62	672.63	
B	22+00.00	11.00	672.56	672.57	
C	22+06.50	11.00	672.49	672.51	



Location

В

D

Location

CL. N. Brg. Pier 3

CL. S. Brg. Pier 4

Location

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CL. Pier 4

CL. Pier 3

4A

4B

4C

Α

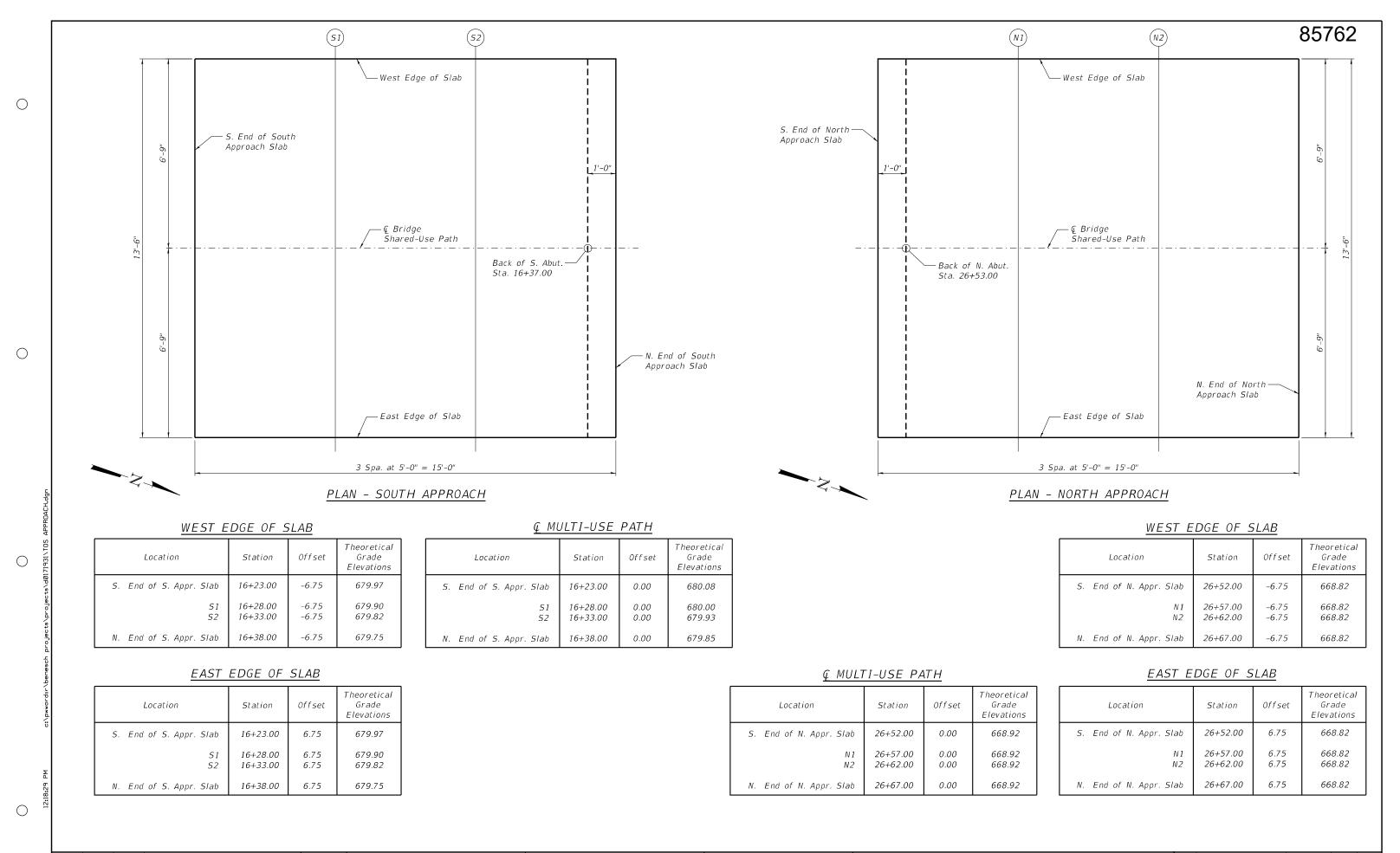
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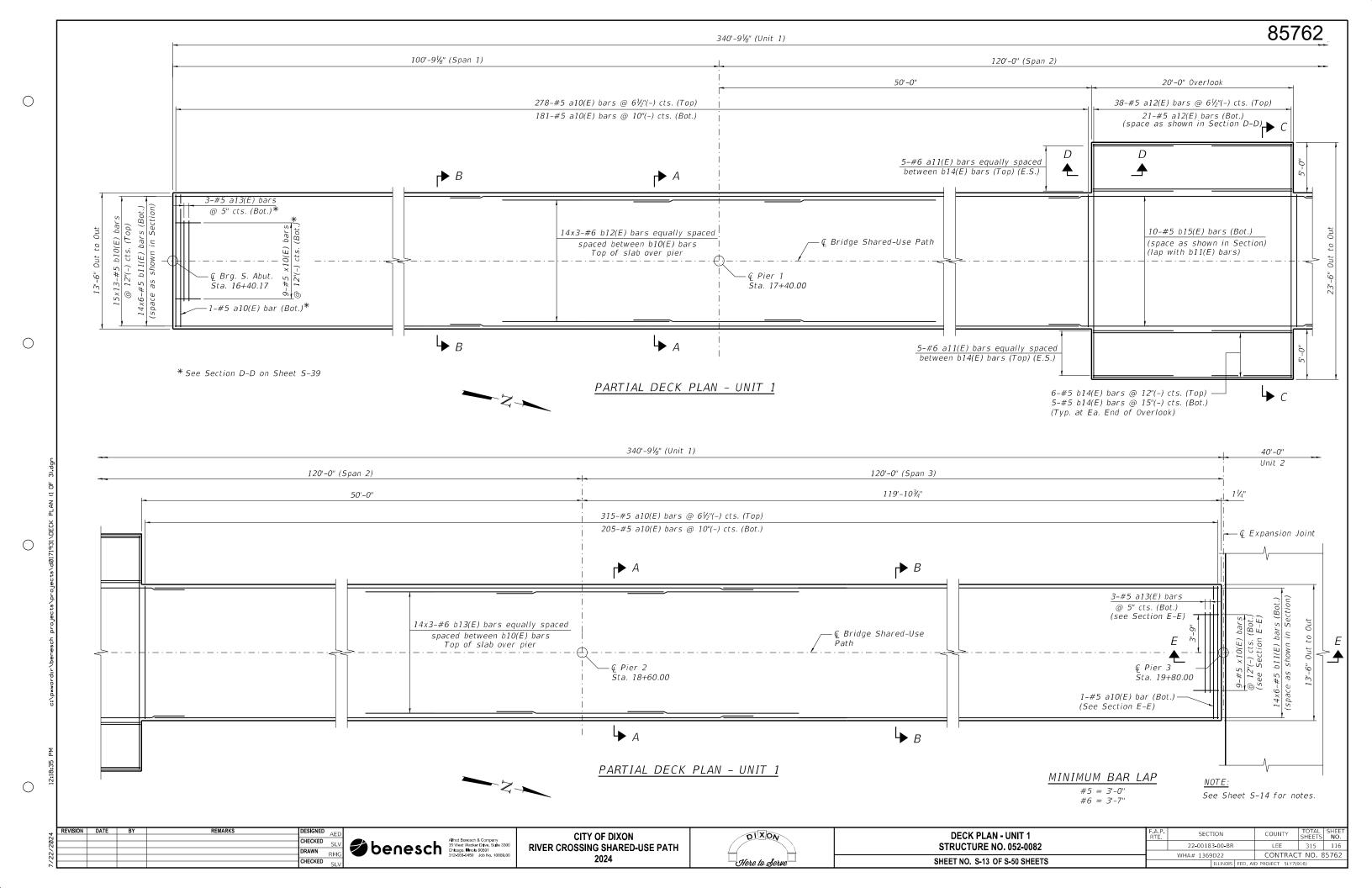


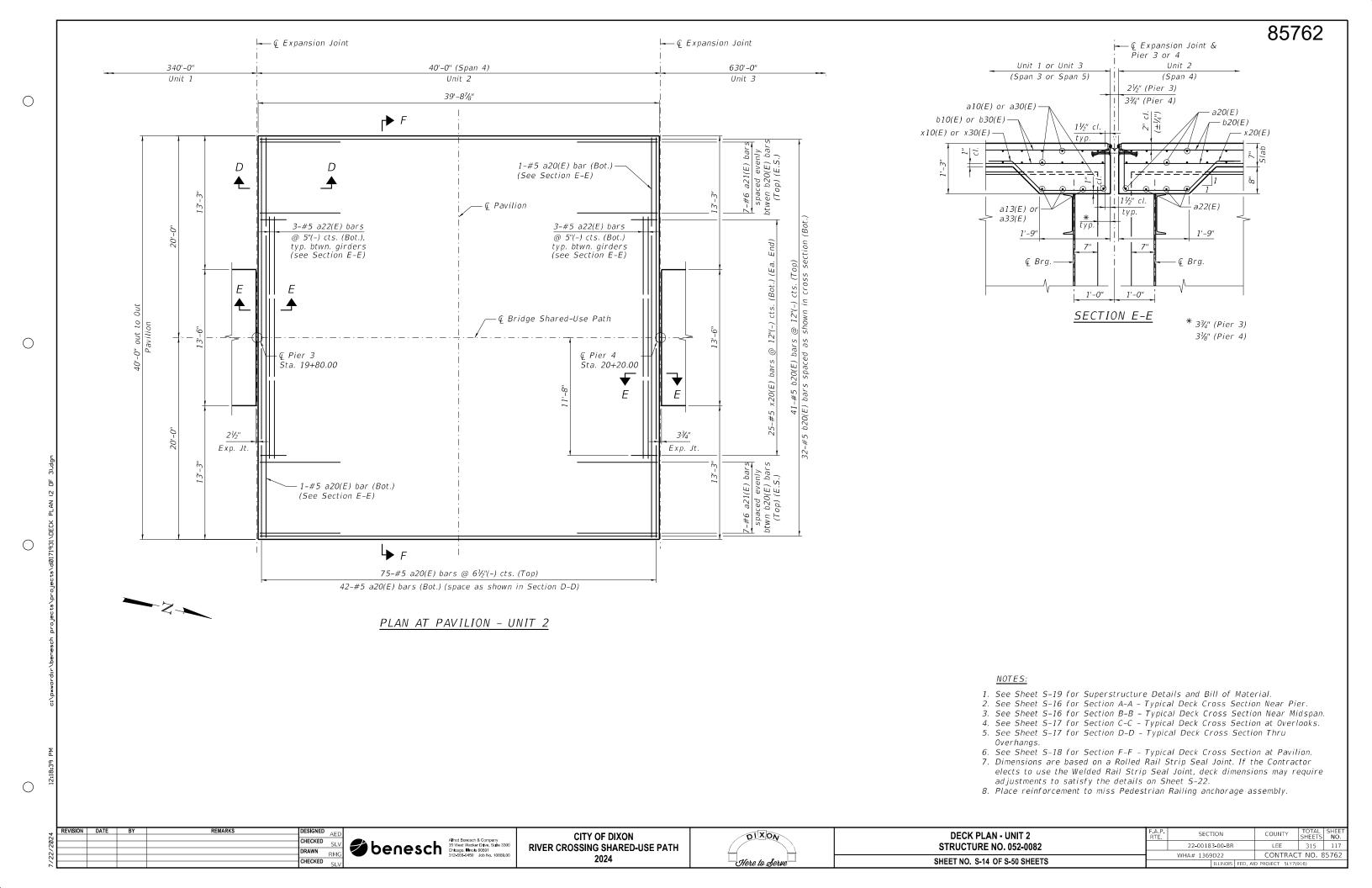


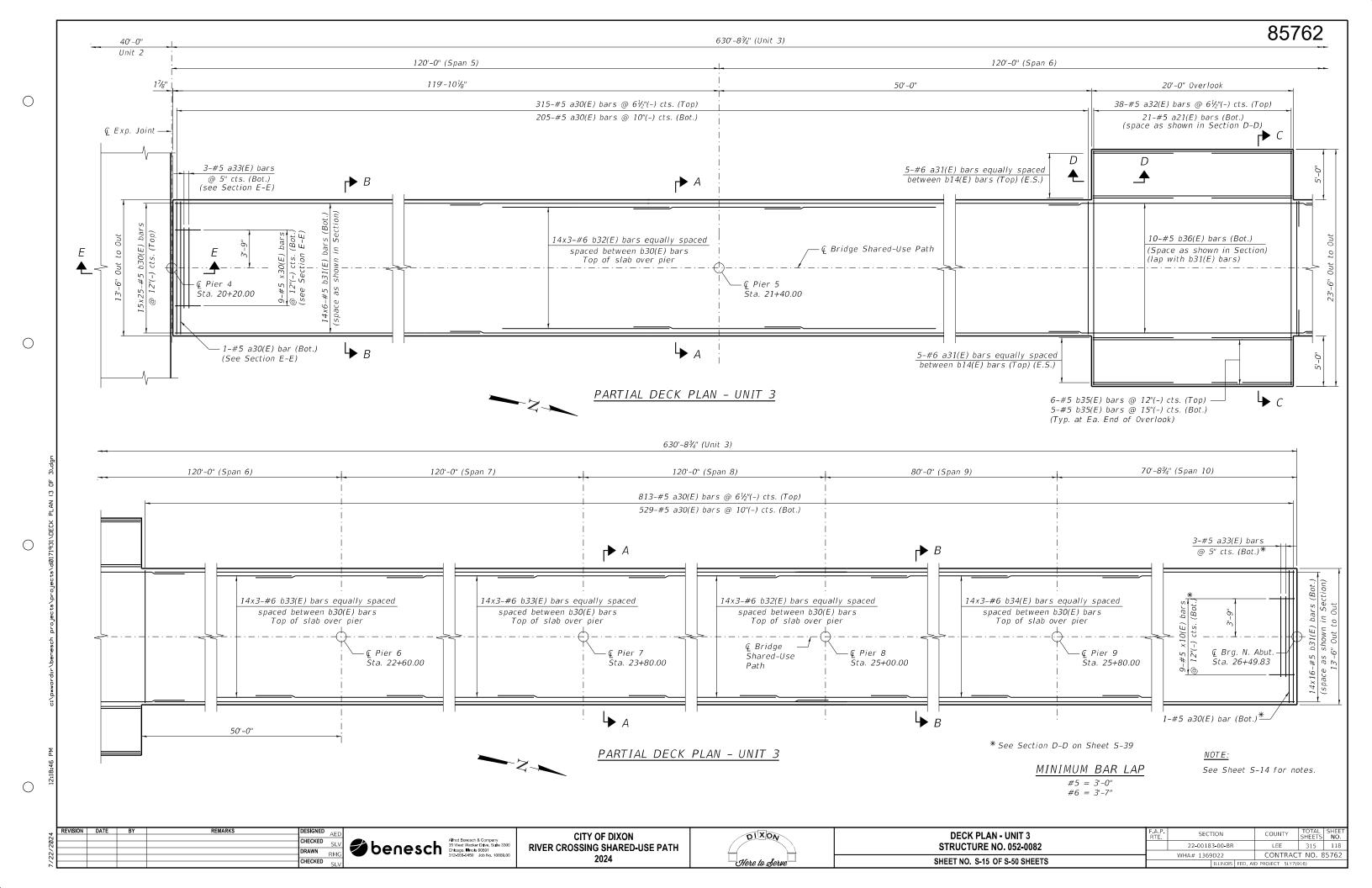
TOP OF SLAB ELEVATIONS (3 OF 3)	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
STRUCTURE NO. 052-0082		22-00183-00-BR	LEE	315	114
		WHA# 1369D22	CONTRAC	T NO. 8	5762
SHEET NO. S-11 OF S-50 SHEETS		THINOIS EED AL	D PROJECT 51 V7/	216)	

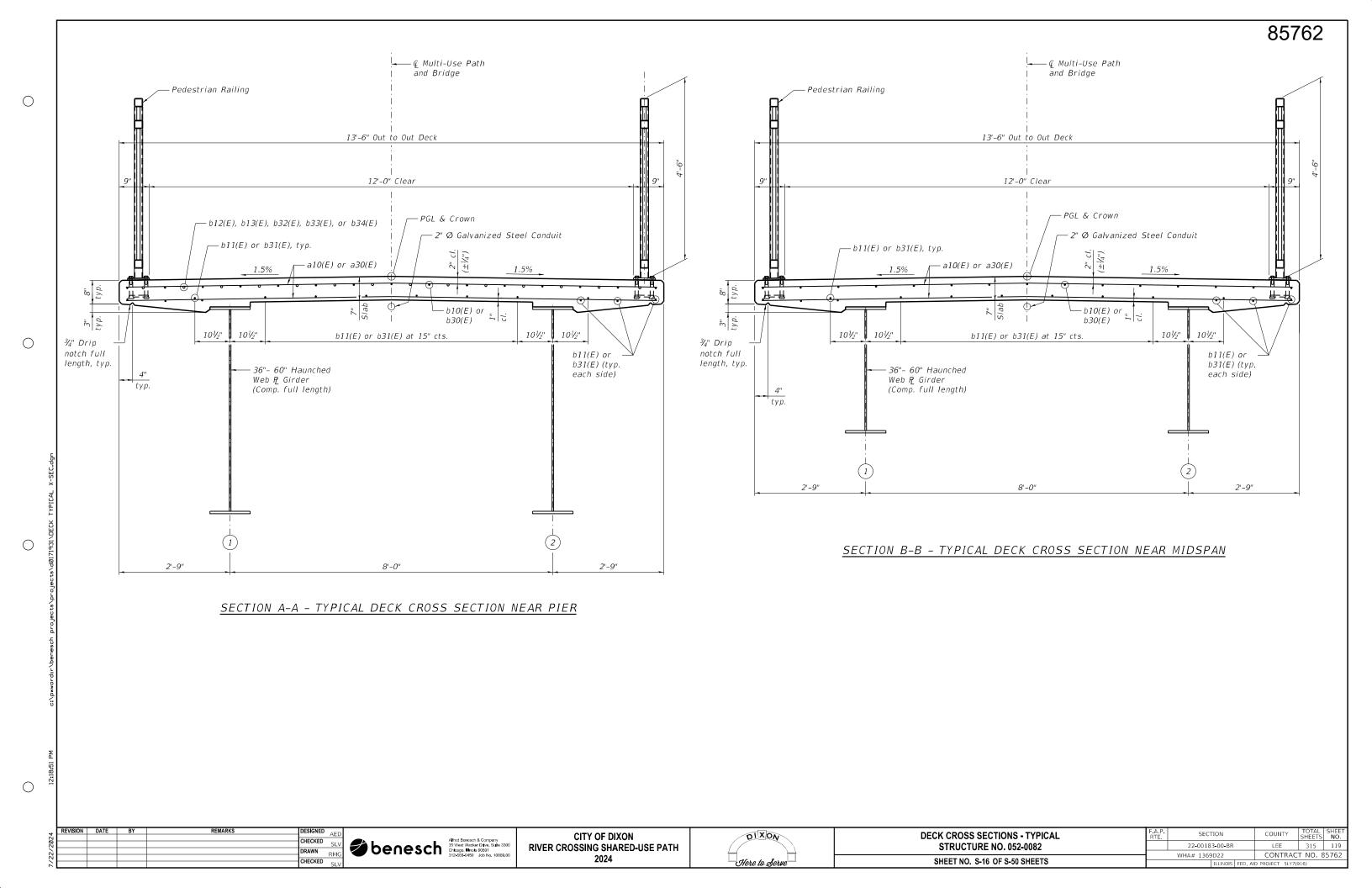


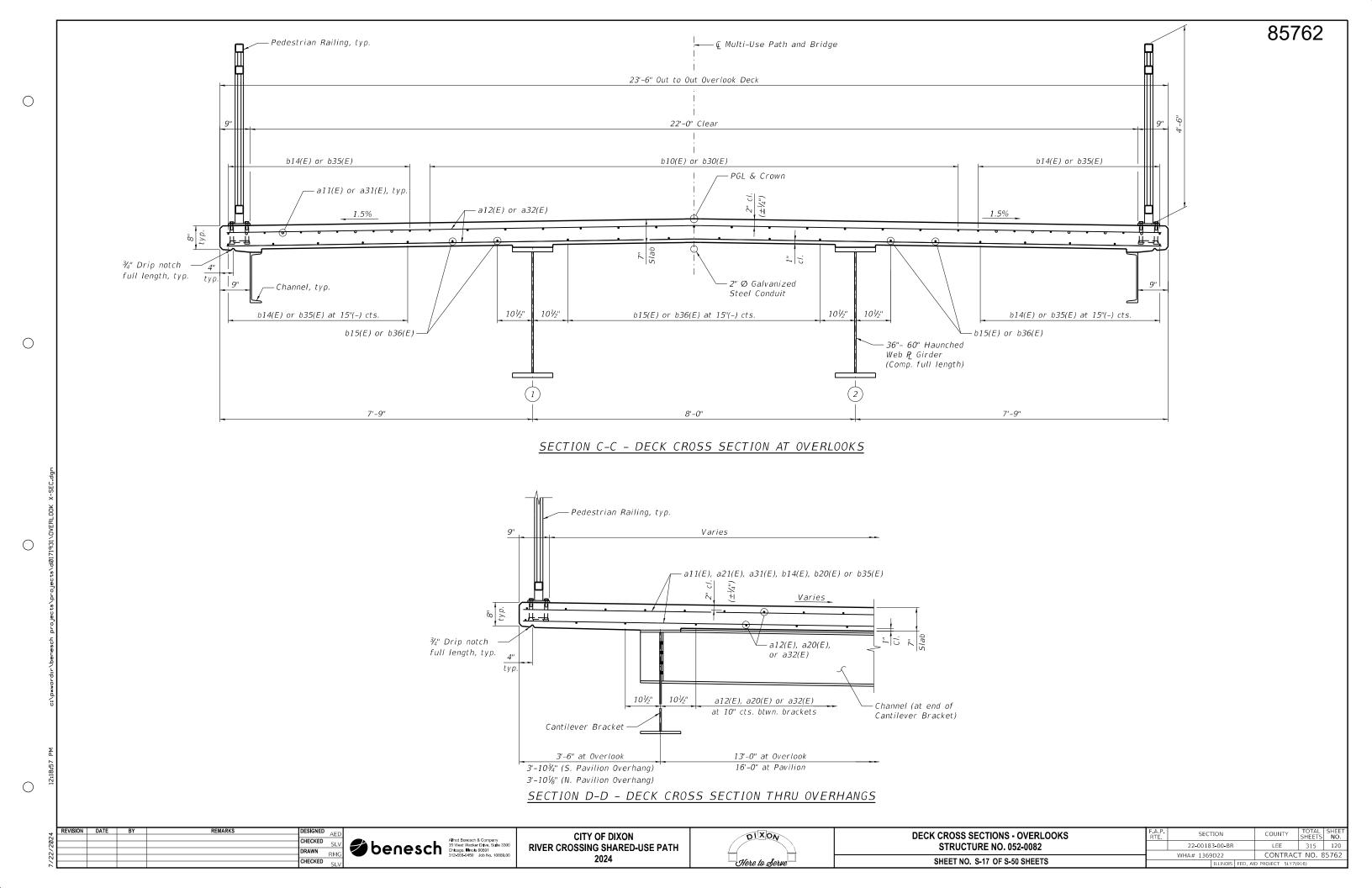
REVISION	DATE BY	REMARKS	DESIGNED AED		CITY OF DIXON	DIXON	TOP OF APPROACH SLAB ELEVATIONS	F.A.P. RTF	SECTION	COUNTY	TOTAL SHEET SHEET NO.
/500/			CHECKED SLV	Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, lineis 60601	RIVER CROSSING SHARED-USE PATH		STRUCTURE NO. 052-0082		22-00183-00-BR	LEE	315 115
1/22			CHECKED SLV	312-565-0450 Job No. 10869.00	2024	Here to Serve	SHEET NO. S-12 OF S-50 SHEETS	- WH	HA# 1369D22 ILLINOIS FEE	CONTRAC , AID PROJECT 5LY7(9	T NO. 85762









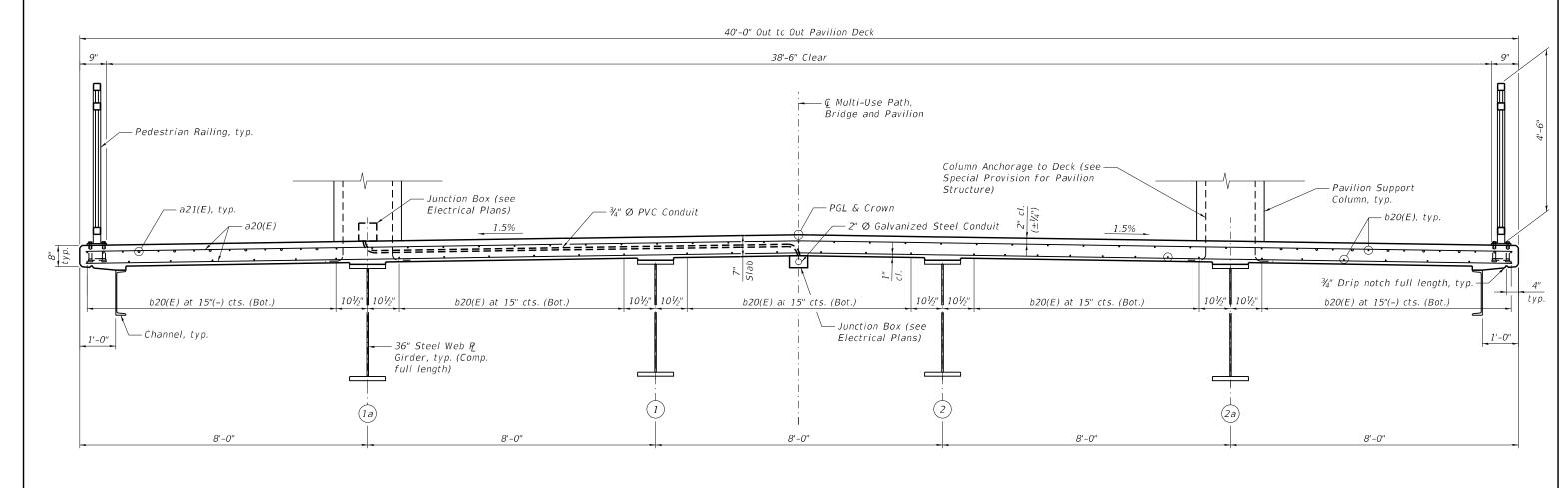




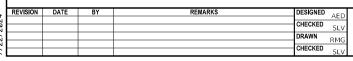
COUNTY TOTAL SHEET NO.

LEE 315 121

CONTRACT NO. 85762



SECTION F-F - DECK CROSS SECTION AT PAVILION



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benesch	Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10869.00
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DECK CROSS SECTIONS - PAVILION	F.A.P. RTE	SECTION
STRUCTURE NO. 052-0082		22-00183-00-B
		WHA# 1369D22
SHEET NO. S-18 OF S-50 SHEETS		ILLINOI

Bar	No.	Size	Length	Shape
a10(E)	981	#5	13'-3"	
a11(E)	20	#6	8'-0"	
a12(E)	a12(E) 59		23'-3"	
a13(E)	6	#5	7'-9"	
b10(E)	195	#5	28'-11"	
b11(E)	168	#5	30'-10"	
b12(E)	42	#6	26'-6"	
b13(E)	42	#6	31'-6"	
b14(E)	22	#5	19'-9"	
b15(E)	10	#5	26'-4"	
x10(E)	18	#5	5'-11"	7
Concrete	Supersti	ucture	Cu. Yd.	120.8
Reinforce	ment Bai	´S,	Pound	31,060
Epoxy Co	ated		Found	51,000

Bars indicated thus 1 \times 2-#5 etc. indicates 1 line of bars with 2 lengths per line.

<u>UNIT 2</u> BILL OF MATERIAL

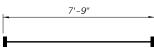
Bar	No.	Size	Length	Shape
a20(E)	119	#5	39'-9"	
a21(E)	28	#6	8'-0"	
a22(E)	18	#5	7'-9"	
b20(E)	73	#5	39'-9"	
x20(E)	50	#5	5'-11"	7
Concrete	Supersti	Cu. Yd.	38.6	
Reinforce Epoxy Co		Pound	8,760	
L poxy co	асси			

Bars indicated thus 1 x 2-#5 etc. indicates 1 line of bars with 2 lengths per line.

<u>UNIT 3</u> BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a30(E)	1864	#5	13'-3"	
a31(E)	20	#6	8'-0"	
a32(E)	59	#5	23'-3"	
a33(E)	6	#5	7'-9"	
b30(E)	375	#5	28'-1"	
b31(E)	308	#5	30'-10"	
b32(E)	84	#6	28'-6"	
b33(E)	84	#6	26'-6"	
b34(E)	42	#6	31'-6"	
b35(E)	22	#5	19'-9"	
b36(E)	10	#5	26'-4"	
x10(E)	18	#5	5'-11"	7
C	<u></u>		C V.1	2107
Concrete	<u> </u>		Cu. Yd.	218.7
Reinforce		rs,	Pound	58,140
Ероху Со	atea			

Bars indicated thus 1 x 2-#5 etc. indicates 1 line of bars with 2 lengths per line.



7.0,

BARS a14(E), a22(E), and a34(E) (Headed, 60-#5 Bar Terminators)

BARS x10(E), x20(E), and x30(E)

NED AED	DESIGNED	REMARKS	BY	DATE	REVISION
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ED	CHECKED				
SLV	TCHECKED				
N RMG	DRAWN				
MMG					
ED	CHECKED				
SLV	CHECKED				

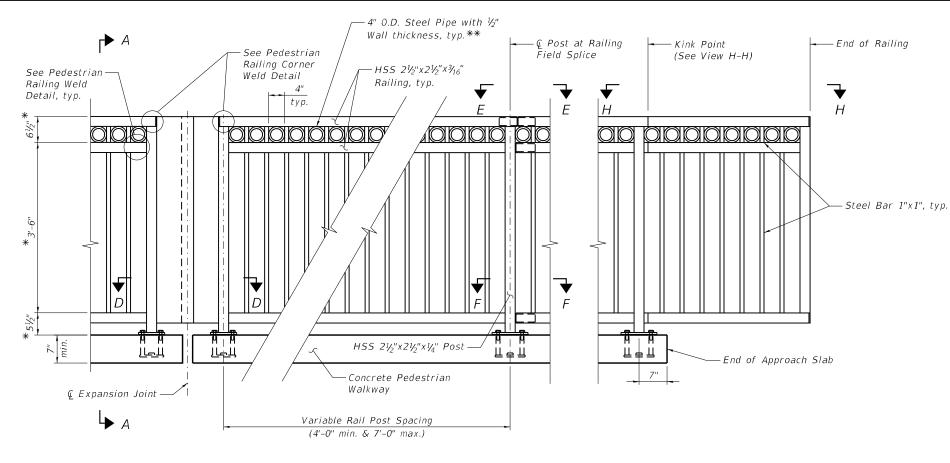


CITY OF DIXON RIVER CROSSING SHARED-USE PATH 2024

1'-6"



SUPERSTRUCTURE DETAILS	F.A.P. RTE	SEC [*]	ΠΟN		COUNT	ΓΥ	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 052-0082		22-0018	3-00-BR		LEE		315	122
		WHA# 1369	D22		CONT	RACT	NO.	35762
			ILLINOIS	FED, All	D PROJECT	5LY7(9	16)	

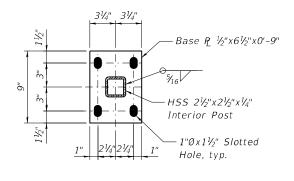


HSS 21/2"x21/2"x3/16" Railing, typ. Steel Bar 1"x1", typ. HSS 21/2"x21/2"x1/4" -Post, typ. ¾"Ø H.S. Bolt, typ.-Concrete Pedestrian --Base R 1/2"x61/2"x0'-9" or Walkway Base P 1/2"x61/2"x0'-61/2" Intermediate Post or Corner Post 53/4" Anchorage Assembly See Base Plate Weld Detail

SECTION A-A

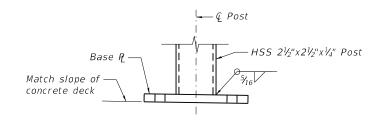
- *Dimensions measured along © of Post
- **Omit Steel Pipe adjacent to railing posts when spacing is less than 4"

ELEVATION - PEDESTRIAN RAILING

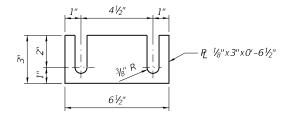


Base R 1/2x61/2x0'-61/2" Base R 1/2x61/2x0'-61/2" HSS 21/2"x21/2"x1/4" Corner Post 1" 21/4"21/4" 1" Hole, typ.

BASE PLATE - INTERMEDIATE POST



BASE PLATE - CORNER POST



BASE PLATE WELD DETAIL

POST SHIM PLATE DETAIL

Shim plates shall be galvanized after shop fabrication according to AASHTO M232.

NOTES:

- 1. Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for Pedestrain Railing.
- 2. Hollow Structural Steel Tubing shall conform to the requirements of ASTM designation A500, Grade B, Structural Steel Tubing.
- 3. All other steel shapes and plates shall conform to the requirements of AASHTO M270, Grade 36.
- 4. Railing shall be fabricated in lengths that include a minimum of 3 posts unless section length is less than 8'-0".
- 5. Post base plates shall be flat with all surfaces smooth and free from warp, and all edges smooth, straight and vertical.
- 6. Posts shall be vertical with bottom edge cut to match slope of deck before welding to base plates.
- 7. Galvanized and painted steel post shims may be used under posts where required for alignment.
- 8. All posts, railings, splices, anchor devices and plates shall be galvanized according to Article 509.05 and painted according to Article 505.06 of the Standard Specifications.
- 9. New steel shall receive a 3 coat of paint system. The organic zinc rich primer / epoxy / urethane paint system shall be used for painting of the steel railing except where otherwise noted. The entire system shall be shop applied, with the exception of masked off connection surfaces, field installed fasteners and damaged areas shall be touched up in the field. The color of the final finish coat for the steel surfaces shall be dark green, Munsell 7.5G 2/4.
- 10. For Views B-B, C-C, E-E, H-H, Sections D-D, F-F, G-G, Pedestrian Railing Weld Detail, and Pedestrian Railing Corner Weld Detail see Sheet S-21.

REVISION DATE BY REMARKS DESIGNED RPC
CHECKED SLV
DRAWN RPC
CHECKED SLV

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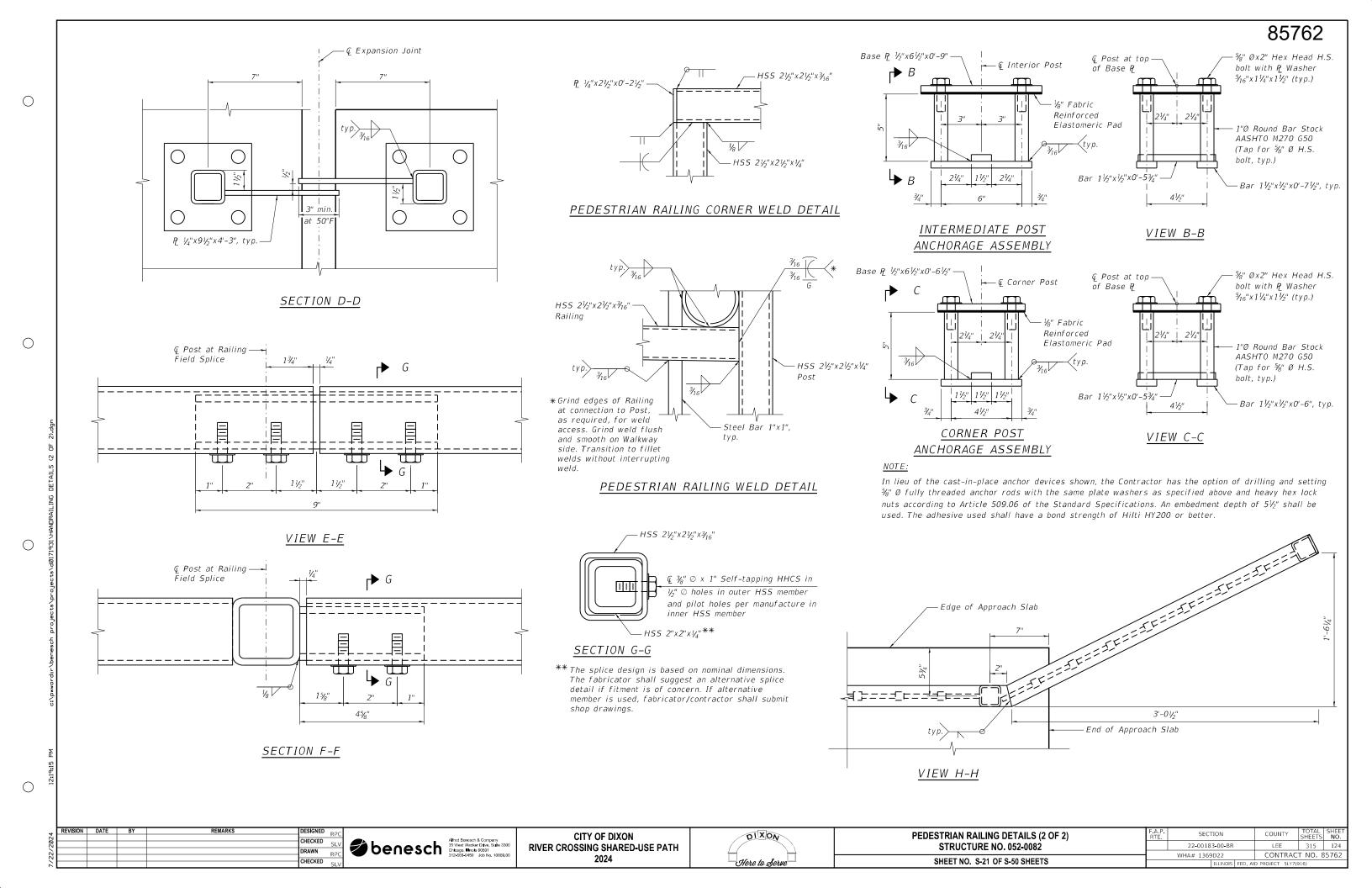


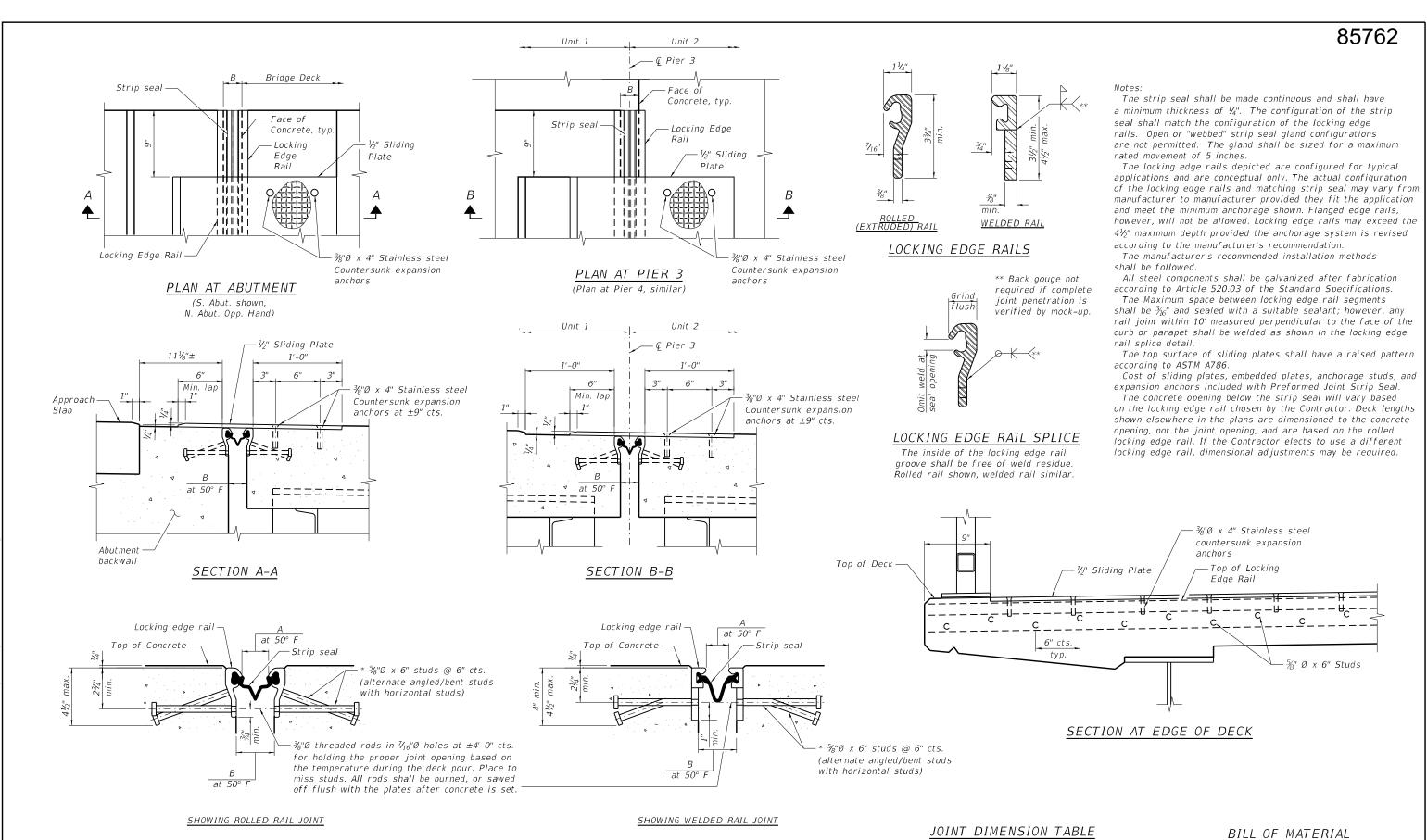
1.8 Company
of Other, Safe 3300
Job No. 10869.00

CITY OF DIXON
RIVER CROSSING SHARED-USE PATH
2024



PEDESTRIAN RAILING DETAILS (1 OF 2)	F.A.P. RTE	SE
STRUCTURE NO. 052-0082		22-00
011557 110 0 00 05 0 50 0115570		WHA# 13
SHEET NO. S-20 OF S-50 SHEETS		





STRIP SEAL ASSEMBLY

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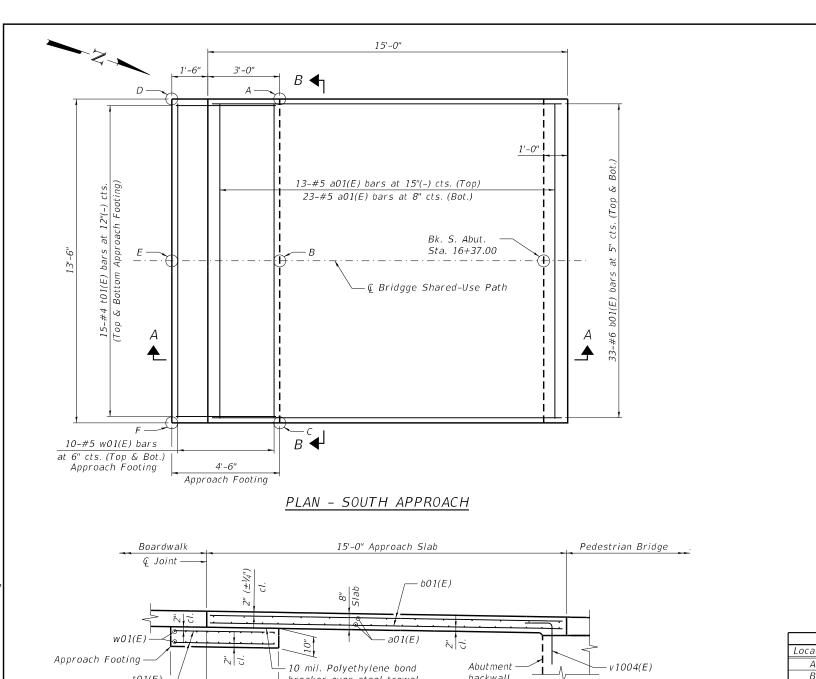
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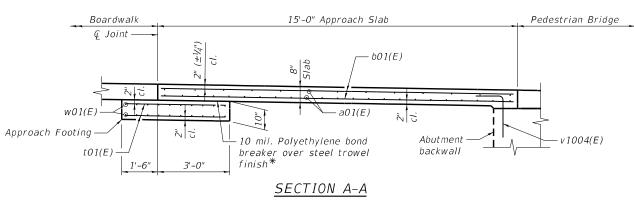
* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

ITEM	S. ABUT.	PIER 3	PIER 4	N. ABUT.
Α	2"	15/8"	27/8"	23/8"
В	27/2"	21/5"	33/1"	31/4"

Item	Unit	Total
Preformed Joint Strip Seal	Foot	54

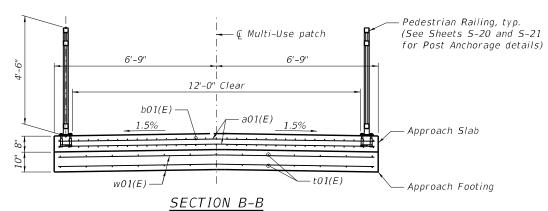
24	REVISION	DATE	 REMARKS	DESIGNED AED	Alfred Benesch & Company	CITY OF DIXON	DIXON	PREFORMED JOINT STRIP SEAL	RTE SECTION	COUNTY SHEETS NO.
750				CHECKED SLV Shoot	esch 35 West Wacker Drive, Suite 3300 Chicago, Minols 60601	RIVER CROSSING SHARED-USE PATH		STRUCTURE NO. 052-0082	22-00183-00	-BR LEE 315 125
- 25 - 25				KMO	312-565-0450 Job No. 10869.00	2024			WHA# 1369D22	CONTRACT NO. 85762
٦Ł				CHECKED SLV		2024	Here to Serve	SHEET NO. S-22 OF S-50 SHEETS	ILLIN	OIS FED. AID PROJECT 5LY7(916)

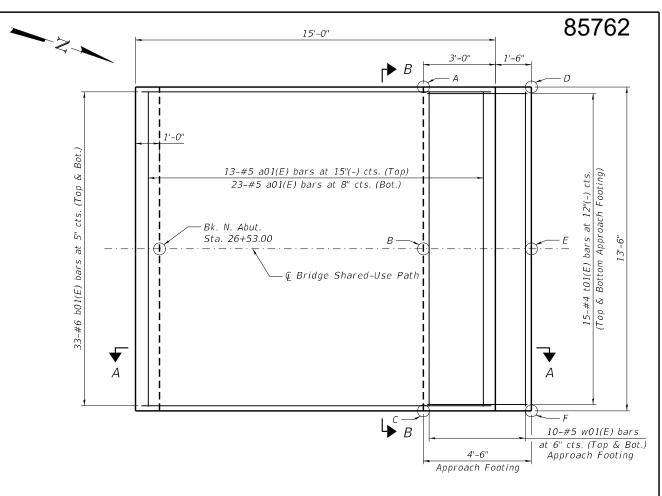




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PLAN - NORTH APPROACH

TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

500	TH APPRO	ACH	NORTH APPROACH				
Location	Location Top		Location	Тор	Bottom		
Α	679.26	678.43	Α	668.15	667.32		
В	679.36	678.53	В	668.25	667.42		
С	679.26	678.43	С	668.15	667.32		
D	679.33	678.50	D	668.15	667.32		
Ε	679.43	678.60	E	668.25	667.42		
F	679.33	678.50	F	668.15	667.32		

BILL OF MATERIAL

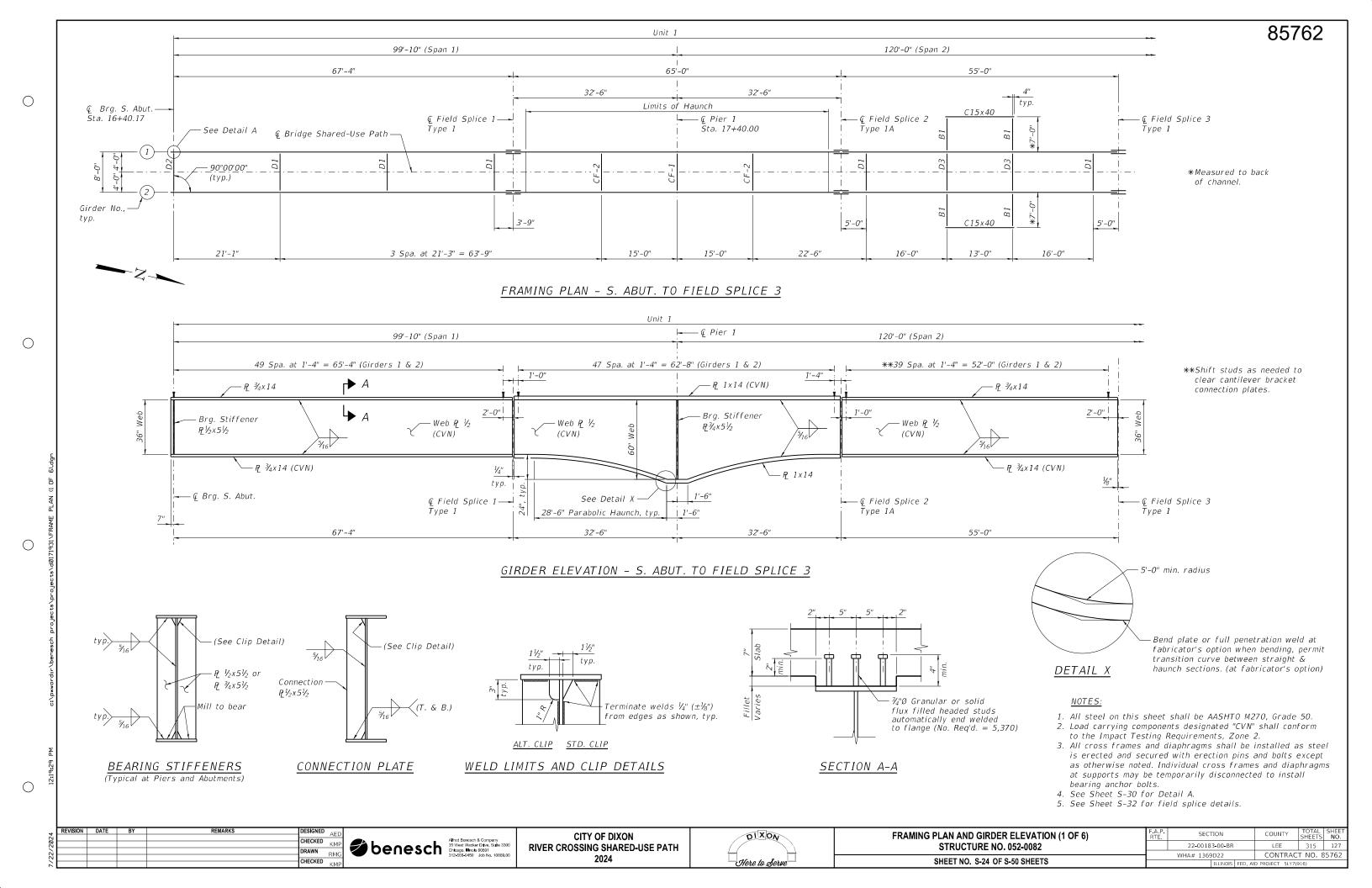
Bar	No.	Size	Length	Shape
a01(E)	72	#5	13'-2"	
b01(E)	132	#6	14'-9"	
t01(E)	60	#4	4'-3"	
w01(E)	40	#5	13'-3"	
Concrete (Approach		Cu. Yd.	10.0	
Concrete	Structur	Cu. Yd.	3.8	
Reinforce Epoxy Co		rs,	Pound	4,640

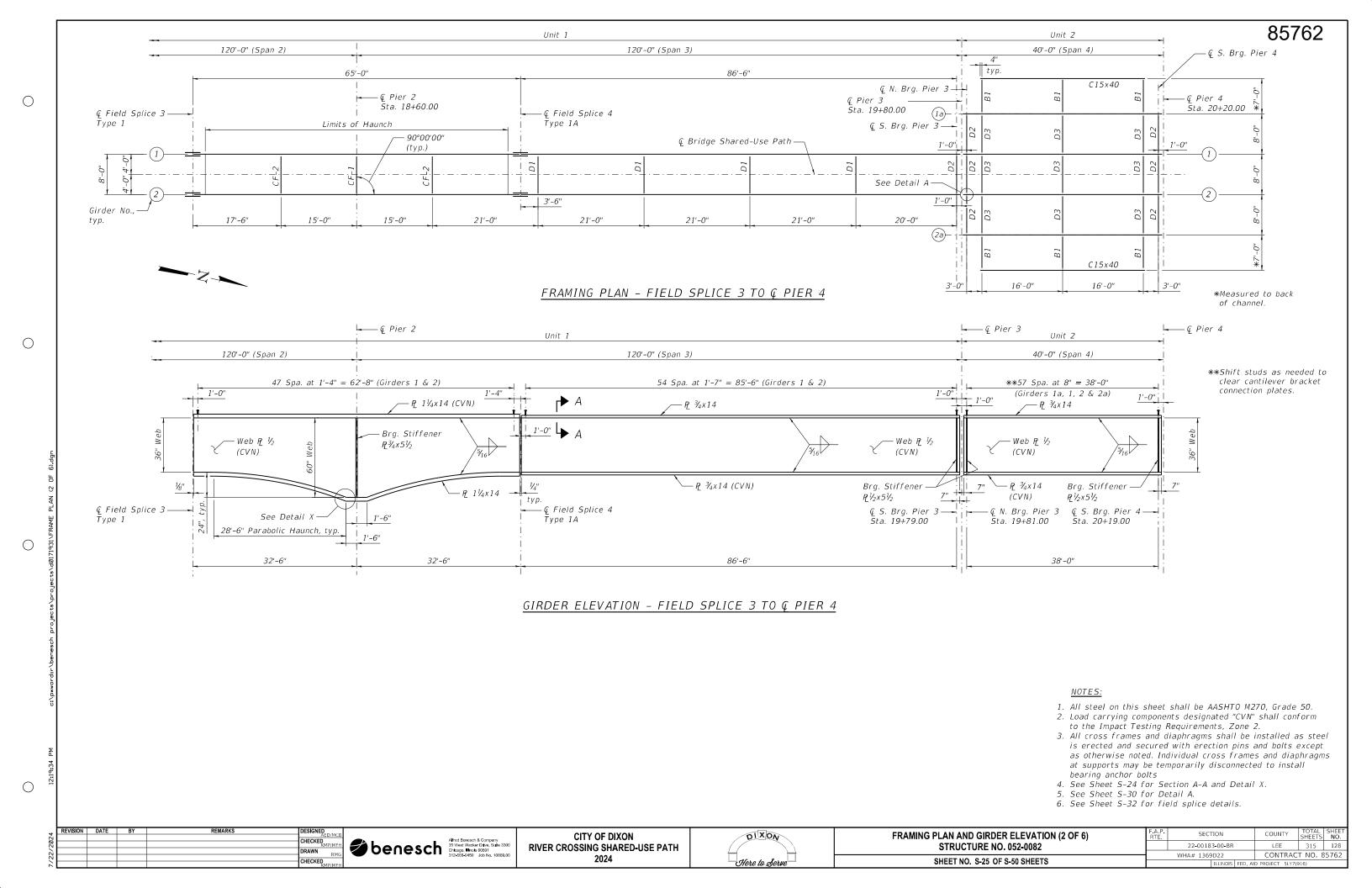
NOTES:

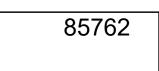
- 1. See Sheet S-39 for v1004(E) bar details.
- 2. Approach Slab shall be paid for as Concrete Superstructure (Approach Slab).
- 3. Approach footing concrete shall be paid for as Concrete Structures.
- 4. The Approach Slab footing maximum applied service bearing pressure is $(Q_{max}) = 0.50$ ksf.
- 5. Cost of excavation for approach footing included with Concrete
- 6. Place reinforcement to miss Pedestrian Railing anchorage assembly.

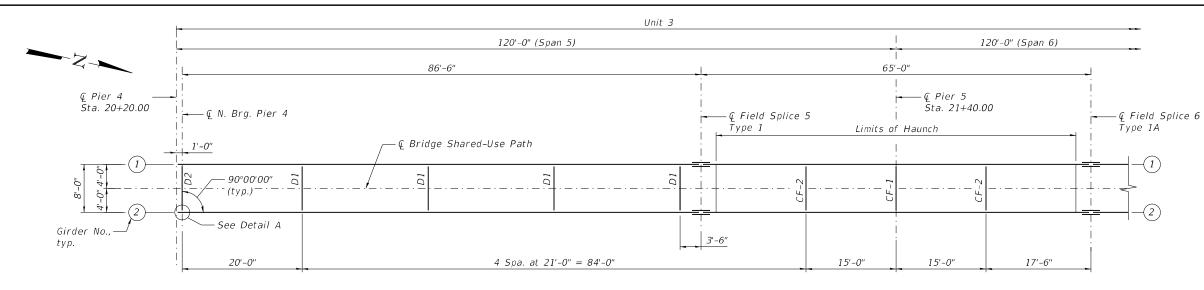
* Cost included with Concrete Superstructure (Approach Slab)

	(101011	DATE	DV	DEMARKO	Inesignen I			1	<u></u>	512		TOTAL CUEST
4 KEA	ISION	DATE	ВТ	REMARKS	AED AED	Alfred Benesch & Company	CITY OF DIXON	DIXON	APPROACH SLAB DETAILS	RTE SECTION	COUNTY	SHEETS NO.
7,20					CHECKED SLV	becesch 35 West Wacker Drive, Suite 3300 Chicago, Ilhois 60601	RIVER CROSSING SHARED-USE PATH 2024		STRUCTURE NO. 052-0082	22-00183-00-BR	LEE	315 126
-Ì					DRAWN RMG	312-565-0450 Job No. 10869.00				WHA# 1369D22	CONTRACT	T NO. 85762
`					CHECKED SLV			Here to Serve	SHEET NO. S-23 OF S-50 SHEETS	ILLINOIS	FED, AID PROJECT 5LY7(91	J16)

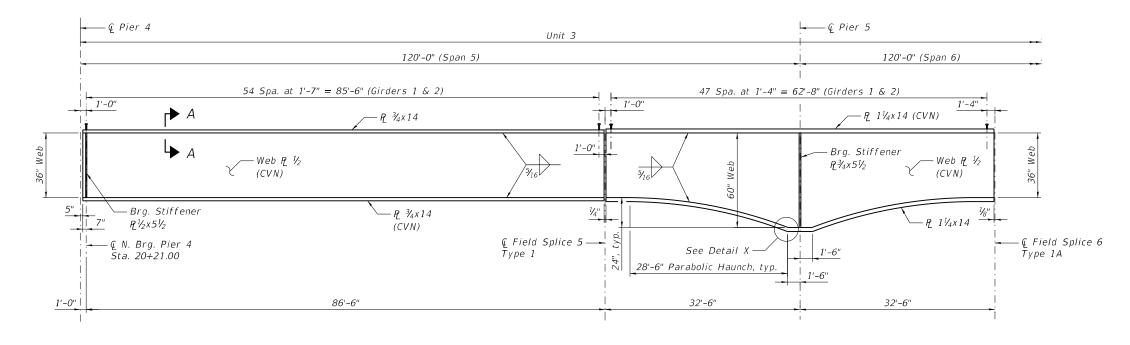








FRAMING PLAN - Q PIER 4 TO FIELD SPLICE 6



GIRDER ELEVATION - Q PIER 4 TO FIELD SPLICE 6

<u>NOTES:</u>

- 1. All steel on this sheet shall be AASHTO M270, Grade 50.
- 2. Load carrying components designated "CVN" shall conform to the Impact Testing Requirements, Zone 2.
- 3. All cross frames and diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames and diaphragms at supports may be temporarily disconnected to install bearing anchor bolts

COUNTY

LEE 315 129 CONTRACT NO. 85762

- 4. See Sheet S-24 for Section A-A and Detail X.
- 5. See Sheet S-30 for Detail A.
- 6. See Sheet S-32 for field splice details.

	DE1/101011			BEMARKS.	I		
24	REVISION	DATE	BY	REMARKS	DESIGNED	AED	
202					CHECKED	KMP	
2/					DRAWN	RMG	
7/2					CHECKED	KMP	

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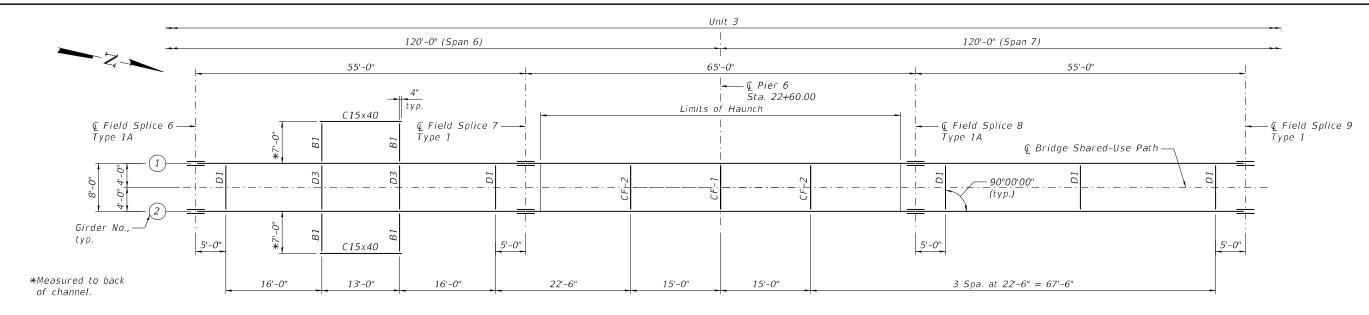
benesch	Alfred Benesch & Company 35 West Wacker Drive, Sufte 3300 Chicago, Illinois 60801 312-565-0450 Job No. 10869.00
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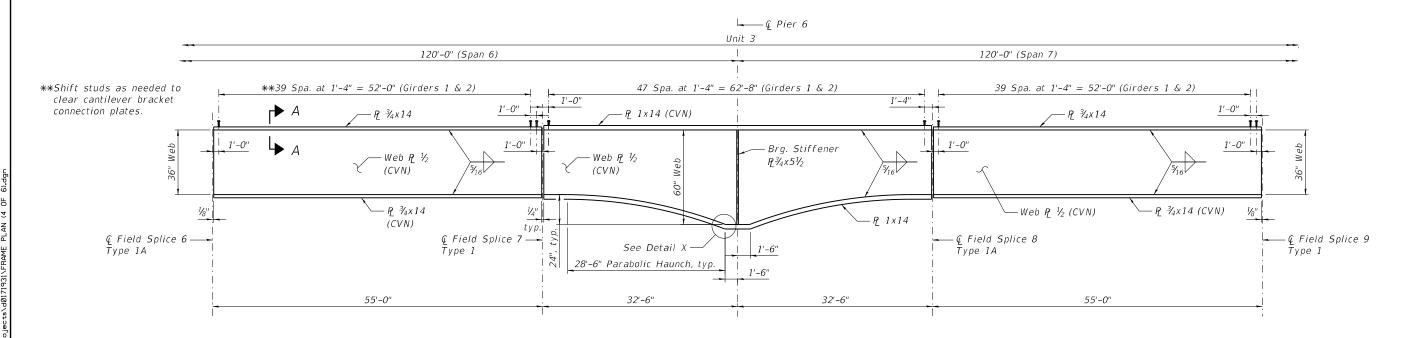


FRAMING PLAN AND GIRDER ELEVATION (3 OF 6)	F.A.P. RTE	SECTION
STRUCTURE NO. 052-0082		22-00183-00-BR
CUEET NO. C. CO. C.		WHA# 1369D22
SHEET NO. S-26 OF S-50 SHEETS		HI INOIS E





FRAMING PLAN - FIELD SPLICE 6 TO FIELD SPLICE 9



GIRDER ELEVATION - FIELD SPLICE 6 TO FIELD SPLICE 9

NOTES

- 1. All steel on this sheet shall be AASHTO M270, Grade 50.
- 2. Load carrying components designated "CVN" shall conform to the Impact Testing Requirements, Zone 2.
- 3. All cross frames and diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames and diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.
- 4. See Sheet S-24 for Section A-A and Detail X.
- 5. See Sheet S-32 for field splice details.

4	REVISION	DATE	BY	REMARKS	DESIGNED	AED	
02					CHECKED	KMP	
2/2					DRAWN	RMG	
7/2					CHECKED	KMP	`

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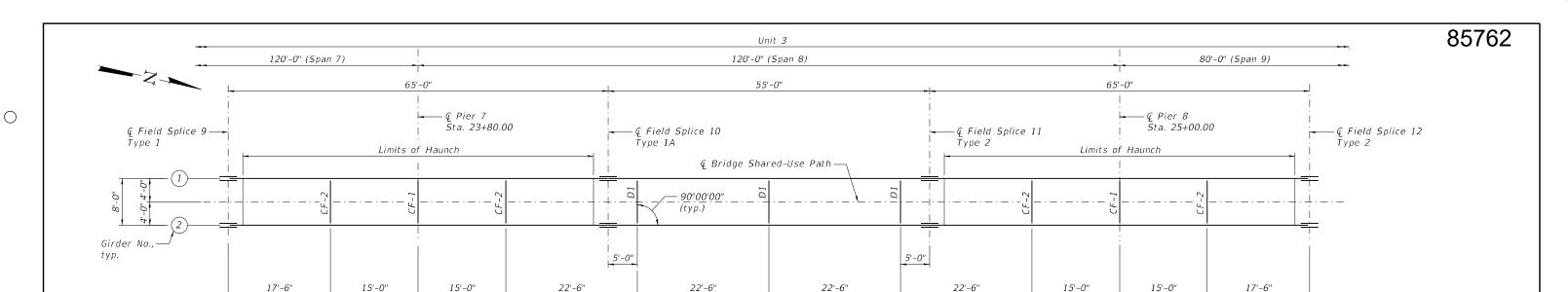
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benesch	Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10869.00
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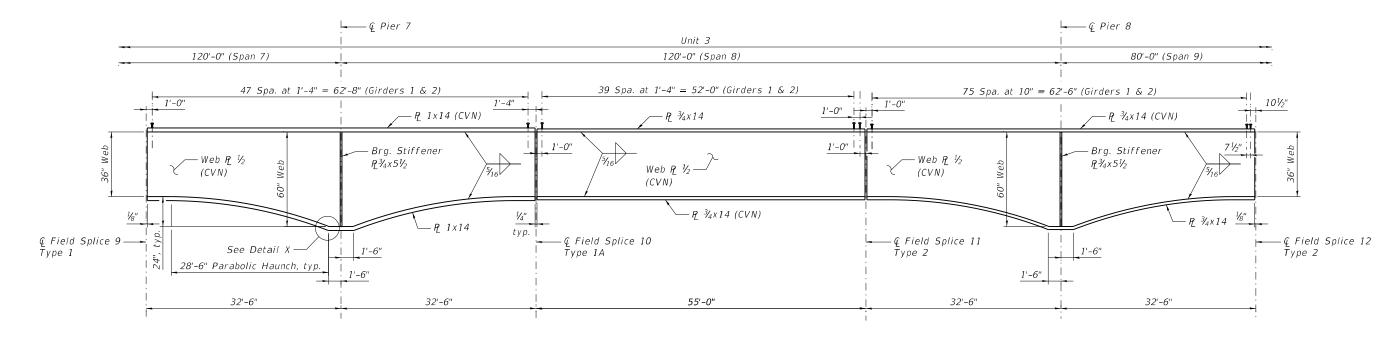




FRAMING PLAN AND GIRDER ELEVATION (4 OF 6)	F.A.P. RTE	SECT	10N		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 052-0082		22-0018	3-00-BR		LEE	315	130
		WHA# 1369	D22		CONTRAC	T NO. 8	5762
SHEET NO. S-27 OF S-50 SHEETS			ILLINOIS	FED, All	D PROJECT 5LY7(9	916)	



FRAMING PLAN - FIELD SPLICE 9 TO FIELD SPLICE 12



GIRDER ELEVATION - FIELD SPLICE 9 TO FIELD SPLICE 12

- 1. All steel on this sheet shall be AASHTO M270, Grade 50.
- 2. Load carrying components designated "CVN" shall conform to the Impact Testing Requirements, Zone 2.
- 3. All cross frames and diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames and diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.

- 4. See Sheet S-24 for Section A-A and Detail X.
- 5. See Sheet S-32 for field splice details.

_	REVISION	DATE	BY	REMARKS	DESIGNED	AED	
2024					CHECKED	AED KMP	
22/2					DRAWN	RMG	
1/2					CHECKED	KMP	

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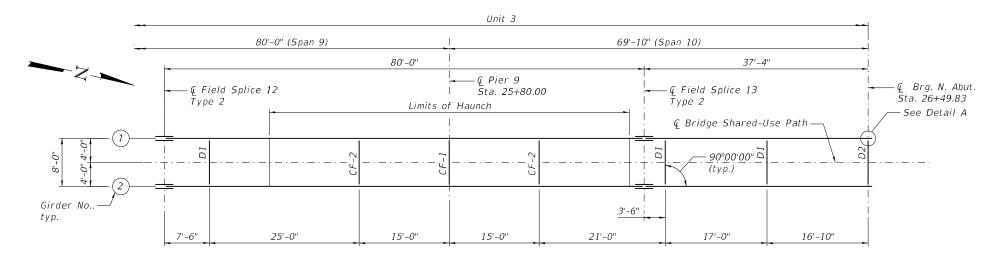
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benesch	Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10869.00

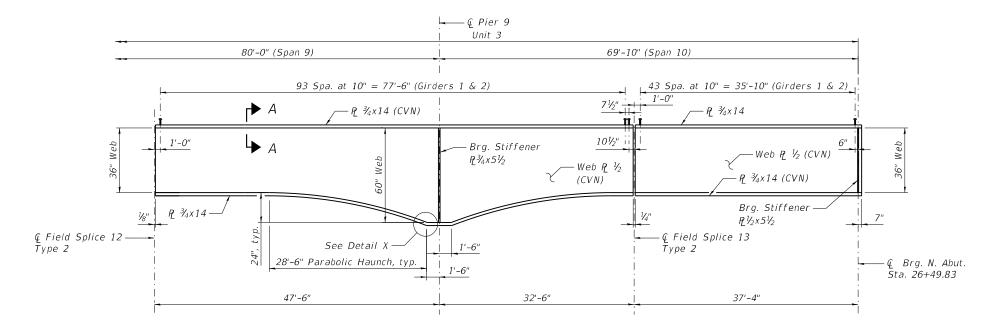




FRAMING PLAN AND GIRDER ELEVATION (5 OF 6)	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 052-0082		22-00183-00-BR	LEE	315	131
		WHA# 1369D22	CONTRAC	T NO. 8	5762
SHEET NO. S-28 OF S-50 SHEETS		ILLINOIS FED AL	D PROJECT 51 Y70	916)	



FRAMING PLAN - FIELD SPLICE 12 TO N. ABUT.



GIRDER ELEVATION - FIELD SPLICE 12 TO N. ABUT.

NOTES:

- 1. All steel on this sheet shall be AASHTO M270, Grade 50.
- 2. Load carrying components designated "CVN" shall conform to the Impact Testing Requirements, Zone 2.
- 3. All cross frames and diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames and diaphragms at supports may be temporarily disconnected to install bearing anchor bolts
- 4. See Sheet S-24 for Section A-A and Detail X.
- 5. See Sheet S-30 for Detail A.
- 6. See Sheet S-32 for field splice details.

. I	REVISION	DATE	BY	REMARKS	DESIGNED	AED	
70					CHECKED	KMP	
7					DRAWN		
7					CHECKED	RMG	_
۱:					CHECKED	KMP	

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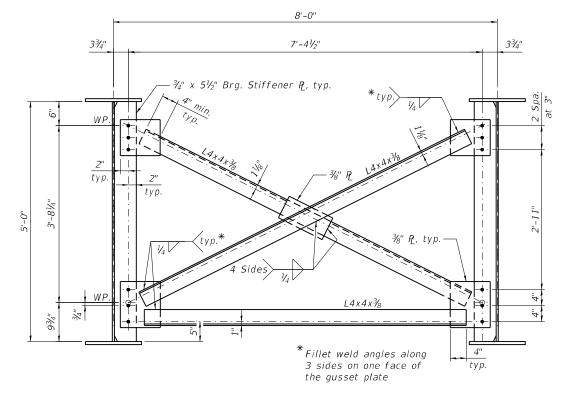
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benesch	Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10869.00
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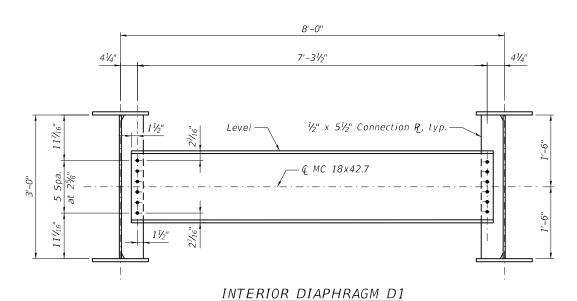
CITY OF DIXON
RIVER CROSSING SHARED-USE PATH
2024

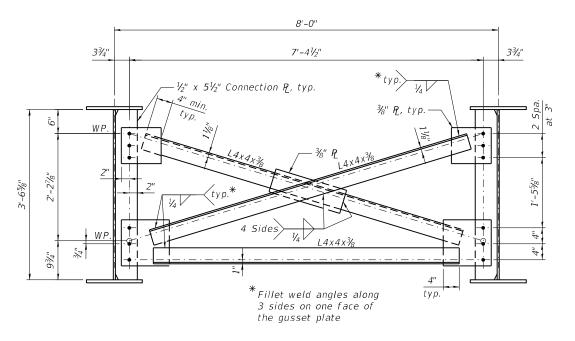
DIXON	
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Here to Serve	

FRAMING PLAN AND GIRDER ELEVATION (6 OF 6) STRUCTURE NO. 052-0082		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		22-00183-00-BR		315	132
		WHA# 1369D22 CONTR			5762
SHEET NO. S-29 OF S-50 SHEETS	ILLINOIS FED AID PROJECT 51Y7(916)				

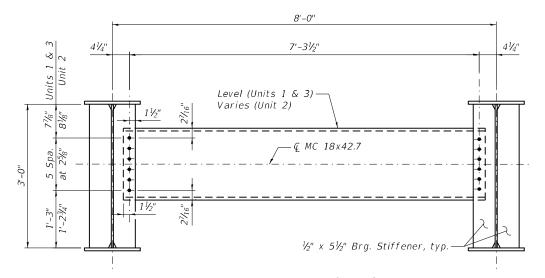


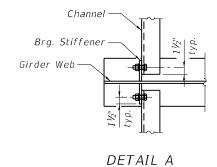
INTERMEDIATE CROSS-FRAME CF-1 AT CENTERLINE OF PIER





INTERMEDIATE CROSS-FRAME CF-2





END DIAPHRAGM D2

NOTES.

- 1. See Framing Plans for location of Girder cross frames and diaphragms.
- 2. AASHTO M270 Grade 50 steel shall be used for all cross frames, diaphragms, connections plates and bearing stiffeners, unless otherwise noted.
- 3. Load carrying components designated "CVN" shall conform to the Impact Testing Requirements, Zone 2.
- 4. Fasteners shall be ASTM F 3125 Grade A325 Type I, mechanically galvanized bolts. Bolts shall be $\frac{7}{6}$ " Ø, in holes $\frac{15}{16}$ " Ø, unless otherwise noted.
- 5. All cross frames and diaphragms between girders shall be installed with erection pins and bolts in accordance with the erection plan approved by the Engineer. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor bolts.

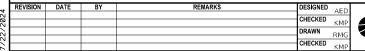
COUNTY

LEE

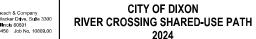
315 133

CONTRACT NO. 85762

6. All cross frames and diaphragms shall be oriented, at all locations, perpendicular to the girders.

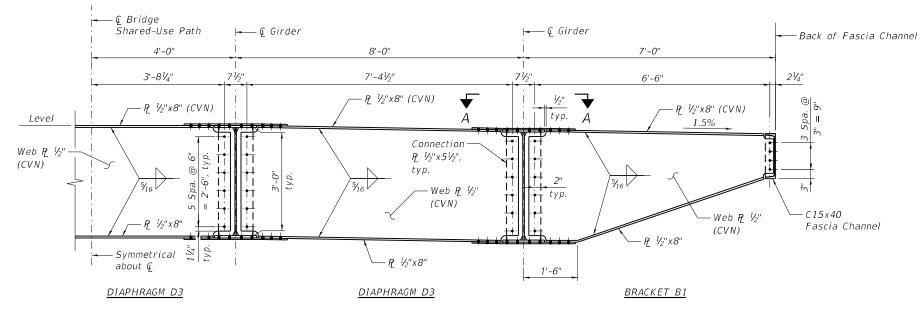




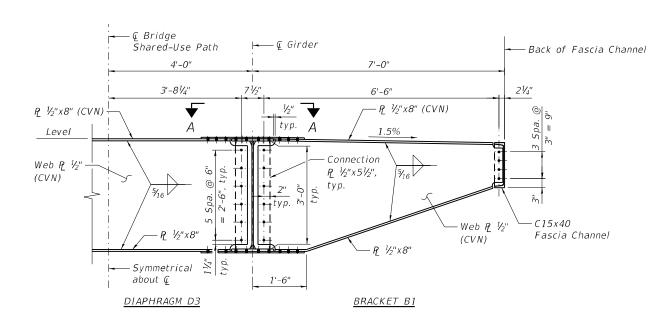




STEEL DETAILS (1 OF 2)	F.A.P. RTE	SEC ⁻	TION		
STRUCTURE NO. 052-0082		22-0018	3-00-BR		
OUEET NO. O OO OE O CO OUEETO		WHA# 1369D22			
SHEET NO. S-30 OF S-50 SHEETS			ILLINOIS	FED	



PARTIAL SECTION AT PAVILION



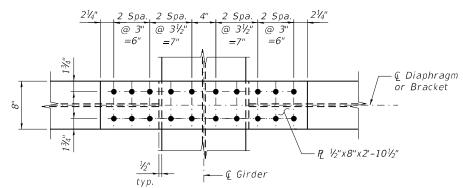
PARTIAL SECTION AT OVERLOOK

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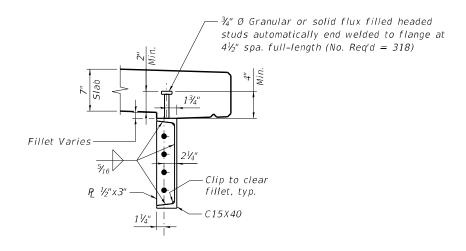
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 $\frac{VIEW A-A}{(top flange shown, bottom flange similar)}$

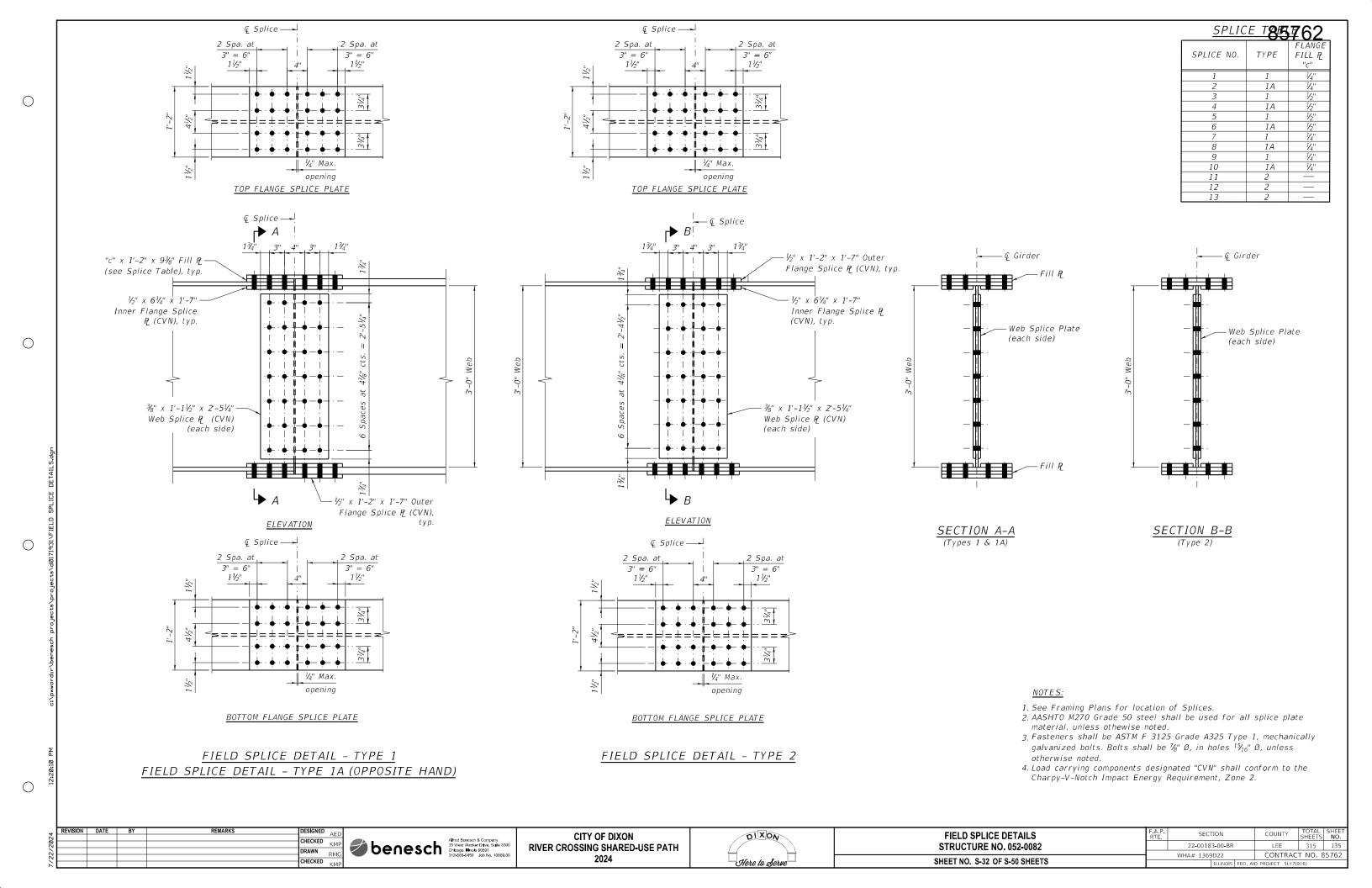


<u>FASCIA CHANNEL</u> <u>DETAIL</u>

NOTES:

- 1. See Framing Plans for location of diaphragms and brackets.
- 2. All steel on this sheet shall be AASHTO M270, Grade 50.
- 3. Load carrying components designated "CVN" shall conform to the Impact Testing Requirements, Zone 2.
- 4. All brackets and diaphragms shall be installed with erection pins and bolts in accordance with the erection plan approved by the Engineer.
- 5. Fasteners shall be ASTM F 3125 Grade A325 Type 1, mechanically galvanized bolts. Bolts shall be $\frac{7}{6}$ " Ø, in $\frac{15}{16}$ " Ø holes unless otherwise noted.

						-					· ·			
/					CHECKED MF	1	2024	Here to Serve	SHEET NO. S-31 OF S-50 SHEETS		ILLINOIS FED, AID	PROJECT 5LY7(9	J16)	
73					TAPIC	312-555-0450 Job No. 10869-00	2024	1	CHEET NO. O ALOE O SO CHEETO	WHA# 13691	DZZ	CONTRACT	1 110. 8	30/02
2					DRAWN DAY	UEIIESCII Chicago, Minols 60601			011100101121101002	WHA# 1369D22		CONTRACT NC		05765
.56					IVIF	35 West Wacker Drive, Suite 3300	RIVER CROSSING SHARED-USE PATH		STRUCTURE NO. 052-0082	22-00183	3-00-BR	LEE	315	134
2					CHECKED ME	Alfred Benesch & Company	CITY OF DIXON	DIXON	1 ,	RIE.			SHEETS	1 NO
4	REVISION	DATE	BY	REMARKS	DESIGNED MCI	3	CITY OF DIXON	N VO	STEEL DETAILS (2 OF 2)	F.A.P. SECT	ION	COUNTY	TOTAL	SHEE

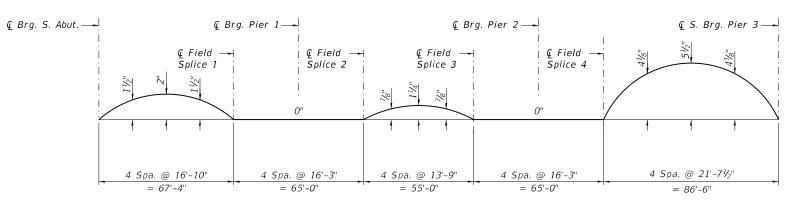


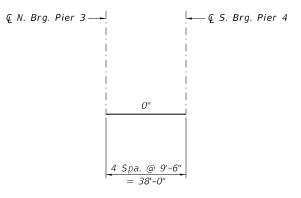
TOTAL SHEET NO.

LEE 315 136

CONTRACT NO. 85762

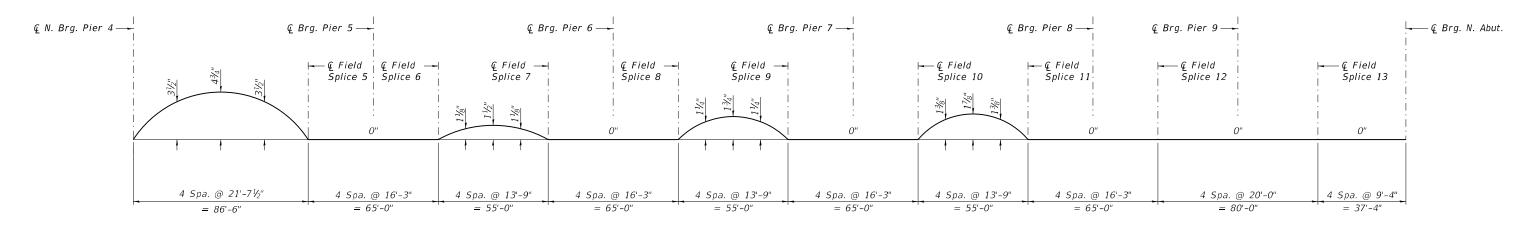
COUNTY





CAMBER DIAGRAM - UNIT 1

CAMBER DIAGRAM - UNIT 2 (For Girders 1a, 1, 2, & 2a.)



CAMBER DIAGRAM - UNIT 3

UNIT 1									
	ℚ Brg. S. Abut.	FS 1	⊊ Brg. Pier 1	FS 2	FS 3	⊊ Brg. Pier 2	FS 4	⊊ S. Brg. Pier 3	
Girder 1	678.97	678.07	<i>677.52</i>	676.97	676.06	675.69	675.32	673.88	
Girder 2	678.97	678.07	<i>677.52</i>	676.97	676.06	675.69	675.32	673.88	

	UNII 2	
	ℚ N. Brg.	
	Pier 3	Pier 4
Girder 1a	673.76	673.57
Girder 1	673.88	673.69
Girder 2	673.88	673.69
Girder 2a	673.76	673.57

	UNIT 3															
	ℚ N. Brg. Pier 4	FS 5	⊈ Brg. Pier 5	FS 6	FS 7	⊈ Brg. Pier 6	FS 8	FS 9	⊊ Brg. Pier 7	FS 10	FS 11	⊊ Brg. Pier 8	FS 12	⊊ Brg. Pier 9	FS 13	⊊ Brg. N. Abut.
Girder 1	673.66	672.91	672.49	672.08	671.63	671.31	670.99	670.40	670.11	669.83	669.35	668.86	668.37	668.14	667.98	668.07
Girder 2	673.66	672.91	672.49	672.08	671.63	671.31	670.99	670.40	670.11	669.83	669.35	668.86	668.37	668.14	667.98	668.07

TOP OF WEB ELEVATIONS

For fabrication only. Elevations based on a "No-Load" condition (fully supported with the web horizontal).

4	REVISION	DATE	BY	REMARKS	DESIGNED AED	
02					CHECKED KMP/MEH	_
2/2					DDAWN	
/22					CHECKED	\ \ \
7					KMP/MFH	

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CITY OF DIXON RIVER CROSSING SHARED-USE PATH 2024



STEEL GIRDER TOP OF WEB ELEVATIONS AND CAMBER	F.A.P. RTE	SEC ⁻	ΠΟΝ	
STRUCTURE NO. 052-0082		22-0018	3-00-BR	
CUEET NO. O CO. OF C. SO CUEETO	WHA# 1369D22			
SHEET NO. S-33 OF S-50 SHEETS			TELEVIOLE	EED /

								GIRDER	MOMENT T	4 <i>BLE</i>								
				UNIT 1			UNIT 2						UNIT 3					
		0.4 Span 1	Pier 1	0.5 Span 2	Pier 2	0.6 Span 3	0.5 Span 4	0.4 Span 5	Pier 5	0.5 Span 6	Pier 6	0.5 Span 7	Pier 7	0.5 Span 8	Pier 8	0.5 Span 9	Pier 9	0.6 Span 1
I_S	(in⁴)	9,035	35,049	9,035	41,831	9,035	9,035	9,035	41,831	9,035	35,049	9,035	35,049	9,035	28,376	9,035	28,376	9,035
$I_c(n)$	(in ⁴)	24,428	-	24,428	-	24,428	22,948	24,428	-	24,428	-	24,428	-	24,428	-	24,428	-	24,428
$I_c(3n)$	(in ⁴)	18,094	-	18,094	-	18,094	17,635	18,094	-	18,094	-	18,094	-	18,094	-	18,094	-	18,094
$I_c(cr)$	(in ⁴)	-	43,505	-	50,508	_	-	-	50,507	-	43,506	-	43,506	-	36,597	-	36,597	_
S₅	(in³)	482	1,131	482	1,339	482	482	482	1,339	482	1,131	482	1,131	482	923	482	94	482
Sc(n)	(in³)	704	-	704	-	704	668	704	-	704	-	704	-	704	-	704	-	704
Sc(3n)	(in³)	641	-	641	-	641	618	641	-	641	-	641	-	641	-	641	-	641
Sc(cr)	(in³)	-	1,242	-	1,447	_	-	-	1,447	-	1,242	-	1,242	-	1,039	-	1,039	_
Sx	(in³)	631	1,209	665	1,406	592	665	604	1,407	656	1,212	657	1,206	643	1,008	701	-274	662
DC1	(k/')	1.09	1.17	1.53	1.19	1.09	1.20	1.09	1.19	1.53	1.17	1.09	1.17	1.09	1.14	1.09	1.14	1.09
MDC1	('k)	518	1,136	283	1,678	803	217	717	1,662	341	1,037	336	1,237	435	818	22	437	299
DC2	(k/')	0.045	0.045	0.045	0.045	0.045	0.660	0.045	0.045	0.045	0.045	0.045	0.045	0.045	0.045	0.045	0.045	0.045
MDC2	('k)	31	58	11	82	44	119	43	83	13	<i>52</i>	21	68	24	45	1	24	17
DW	(k/')	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Mow	('k)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
MŁ	('k)	645	1,193	817	1,395	793	182	808	1,451	879	1,295	647	1,172	610	936	350	<i>562</i>	364
f∈ (Strength I)	(ksi)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
$M_U + \frac{1}{3} f_{\ell} S_X$	('k)	1,693	3,581	1,798	4,643	2,338	738	2,254	4,721	1,981	<i>3,627</i>	1,480	3,574	1,640	2,717	627	1,545	1,032
$Ø_f M_n$	('k)	3,414	-	3,542	-	3,205	3,336	3,269	-	3,498	-	3,553	-	3,480	-	2,998	-	3,586
f _s DC1	(ksi)	12.9	12.1	7.0	15.0	20.0	5.4	17.9	14.9	8.5	11.0	8.4	13.1	10.8	10.6	0.5	55.6	7.4
f _s DC2	(ksi)	0.6	0.6	0.2	0.7	0.8	2.3	0.8	0.7	0.2	0.5	0.4	0.7	0.4	0.5	0.0	0.3	0.3
fs DW	(ksi)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
f_s (4)	(ksi)	11.0	11.5	13.9	11.6	13.5	3.3	13.8	12.0	15.0	12.5	11.0	11.3	10.4	10.8	6.0	6.5	6.2
f: (Service II)	(ksi)		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
f_s + $f_\ell/_2$ (Service II,) (ksi)	27.8	27.6	25.4	30.8	38.4	11.0	36.6	31.2	28.2	27.8	23.1	28.5	24.8	<i>25.2</i>	8.3	64.3	15.8
Service II Resistant	ce (ksi)	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5	47.5
f_s + $f_\ell/_3$ (Strength I	') (ksi)	36.1	35.9	33.5	39.9	49.7	15.4	47.4	40.5	37.2	36.3	30.3	37.0	32.3	32.9	11.1	81.2	20.6
$Ø_f F_n$	(ksi)	-	50.0	-	50.0	-	50.0	-	50.0	-	50.0	-	50.0	-	50.0	-	50.0	-

GIRDER REACTIO	V TABLE -	- UNIT 1		
	S. Abut.	Pier 1	Pier 2	Pier 3
RDC1 (k	29.8	102.7	123.9	41.6
R_{DC2} (k)	1.7	5.3	6.3	2.0
R _{DW} (k	0.0	0.0	0.0	0.0
$R \pm$ (k)	28.3	91.4	98.9	31.1
RTotal (Strength I)(No Impact) (k	88.8	295.0	335.7	108.9

GIRDER REACTION TABLE -	- UNIT 2	
	Pier 3	Pier 4
R_{DC1} (k)	26.8	26.8
R_{DC2} (k)	16.6	16.6
R_{DW} (k)	0.0	0.0
R + (k)	22.5	22.5
RTotal (Strength I)(No Impact) (k)	93.6	93.6

GIRDER REACTION TABLE - UNIT 3										
Pier 4 Pier 5 Pier 6 Pier 7 Pier 8 Pier 9 N. Abut										
R _{DC1} (k) 35.0	124.0	96.4	112.5	83.1	62.9	22.7			
R_{DC2} (k) 2.0	6.3	5.0	5.7	4.6	3.5	1.2			
R _{DW} (k) 0.0	0.0	0.0	0.0	0.0	0.0	0.0			
R Ł (k) 31.4	101.3	95.5	88.6	78.5	62.1	21.5			
RTotal (Strength I)(No Impact) (k) 101.2	340.2	293.8	302.8	247.0	191.6	67.5			

 I_s , S_s : Non-composite moment of inertia and section modulus of the steel section used for computing $f_s(Total-Strength\ I$, and Service II) due to non-composite dead loads (in.⁴ and in.³).

 $I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing $f_s(Total-Strength\ I,\ and\ Service\ II)$ in uncracked sections due to short-term composite live loads (in.4 and in.3).

 $I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing $f_s(Total-Strength\ I$, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.4 and in.3).

Ic(cr), Sc(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total–Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.4 and in.3).

Sx: Section modulus about the major axis of a section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.3).

DC1: Un-factored non-composite dead load (kips/ft.).

M DCI: Un-factored moment due to non-composite dead load (kip-ft.).
DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M DC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M &: Un-factored live load moment (kip-ft).

 M_u : Strength I load combination of factored design moments (kip-ft.). 1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{\odot}

fi: Factored calculated flange lateral bending stress as calculated using Article 6.10.1.6 and as further simplified by IDOT provisions (ksi).

 $\phi_f M_n$: Factored nominal flexural resistance of the section determined as specified in Article 6.10.7.1 or A6 as applicable (kip-ft.).

fs DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

M DC1 / Ss

fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

 M_{DC2} / $S_c(3n)$ or M_{DC2} / $S_c(cr)$ as applicable.

 f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi). $M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

fs (4): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load as calculated below (ksi).

 $M4 / S_c(n)$ or $M4 / S_c(cr)$ as applicable. $f_s + f_l/2$ (Service II): Sum of stresses as computed below (ksi). $f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (4) + f_l/2$

Service II Resistance: Composite $(0.95R_hF_{yf})$ or noncomposite $(0.80R_hF_{yf})$ stress capacity according to Article 6.10.4.2 (ksi).

 f_s + $f_\ell/3$ (Strength I): Sum of stresses as computed below on non-compact sections (ksi). 1.25 (f_s DC1 + f_s DC2) + 1.5 f_s DW + 1.75 f_s (ξ) + $f_\ell/3$

 $\varphi_f F_n$: Factored nominal flexural resistance of the section as specified in Article 6.10.7.2 or 6.10.8 as applicable (ksi).

Vr: Maximum factored shear range in span computed according to Article 6.10.10.

R_{DC1}: Un-factored reaction due to non-composite dead load (kip).

R_{DC2}: Un-factored reaction due to long-term composite (superimposed excluding future wearing surface) dead load (kip).

R DW: Un-factored reaction due to long-term composite (superimposed future wearing surface only) dead load (kip).

R & : Un-factored live load reaction (kip).

R_{Total} (Strength I)(No Impact): Strength I load combination of factored design reactions, not including dynamic load allowance (Impact) (kip).

 $1.25 (R_{DC1} + R_{DC2}) + 1.5R_{DW} + 1.75 (R_4)$

REVISION	DATE	BY	REMARKS	DESIGNED
				AED/MCB CHECKED
				KMP/MFH
				DRAWN RMG
				CHECKED KMP/MFH

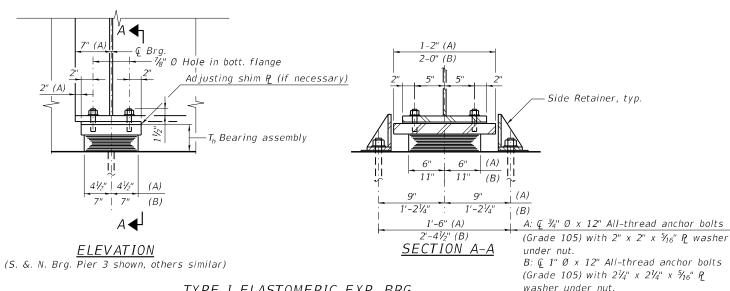






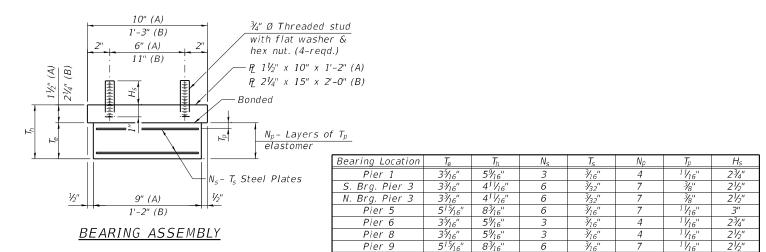
MOMENT AND REACTION TABLES	F.A.P. RTE	SECT	ΠΟN		COUNTY	TOTAL SHEETS	SHE	
STRUCTURE NO. 052-0082		22-00183-00-BR			LEE	315	13	
011557 110 0 0 4 0 5 0 50 0115570		WHA# 1369	D22		CONTRAC	T NO. 8	3576	
SHEET NO. S-34 OF S-50 SHEETS			ILLINOIS	FED. All	D PROJECT 5LY7(916)		

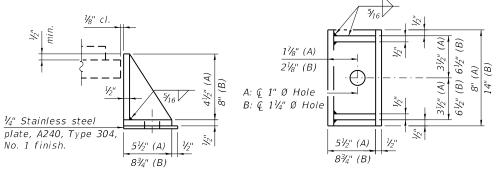




TYPE I ELASTOMERIC EXP. BRG.

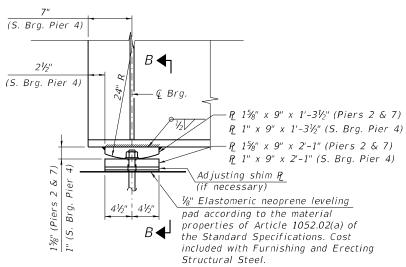
(A = S. & N. Brg. Pier 3; B = Piers 1, 5, 6, 8, & 9





SIDE RETAINER

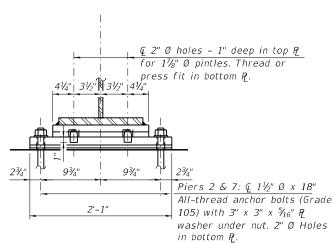
Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



ELEVATION

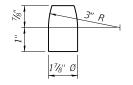
(S. Brg. Pier 4 shown, others similar)

FIXED BEARING (Piers 2, 7, & S. Brg. Pier 4)



SECTION B-B

S. Brg. Pier 4: Q ¾" Ø x 12" All-thread anchor bolts (Grade 105) with 2" x 2" x 1/16" P washer under nut. $1\frac{1}{4}$ " Ø Holes in bottom R.



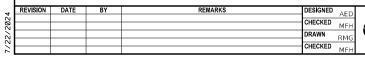
PINTLE

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	16
Anchor Bolts, ¾"	Each	20
Anchor Bolts, 1"	Each	20
Anchor Bolts, 1½"	Each	8

NOTES:

- 1. Side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.
- 2. Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
- 3. Shim plates shall not be placed under Elastomeric Bearing, Type I bearing assemblies.
- 4. The cost of the fixed bearing assembly, excluding Anchor Bolts, shall be included with "Furnishing and Erecting Structural Steel".
- 5. The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M270 Grade 50.
- 6. Two $\frac{1}{8}$ " adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on the bearing details.



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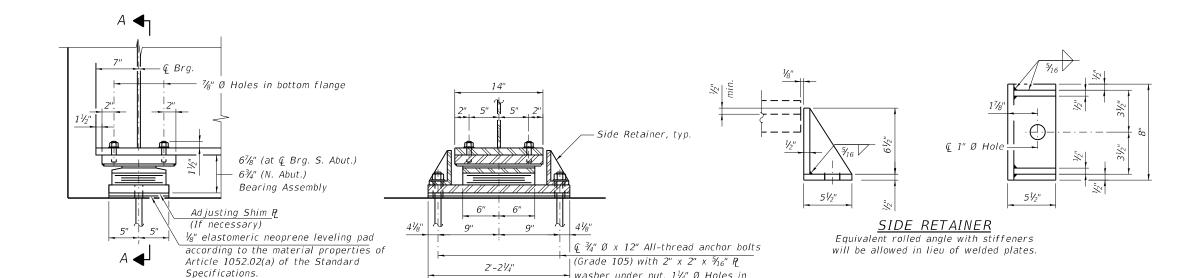
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ELASTOMERIC BEARING TYPE I & LOW PROFILE FIXED BEARING DETAILS	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 052-0082		22-00183-00-BR	LEE	315	138
		WHA# 1369D22	CONTRAC	CT NO. 8	35762
SHEET NO. S-35 OF S-50 SHEETS	HUNDE EED AID DROIECT ELYTOIS				



bottom R.

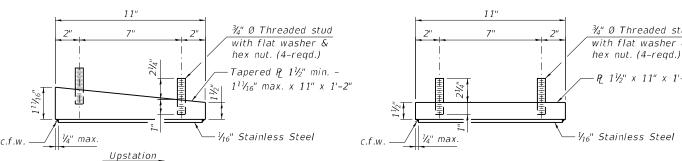
washer under nut. 11/4" Ø Holes in

ELEVATION

(N. Abut. shown, S. Abut. similar)

TYPE II ELASTOMERIC EXP. BRG.

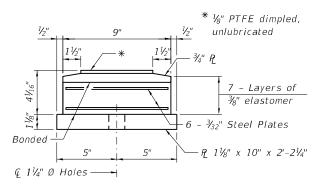
(South and North Abutments)



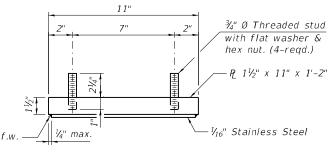
TOP BEARING ASSEMBLY (S. Abut.)

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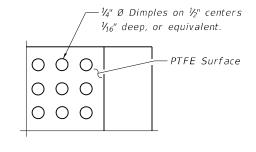


BOTTOM BEARING ASSEMBLY

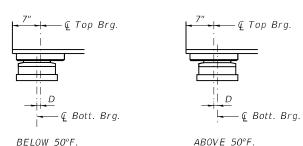


TOP BEARING ASSEMBLY (N. Abut.)

SECTION A-A



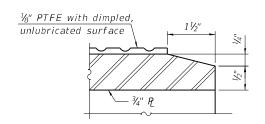
PLAN-PTFE SURFACE



BELOW 50°F. $D=\frac{1}{8}$ " per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

EXPANSION BEARING ORIENTATION

The above diagrams are for informational purposes only to show the amount of expected offset "D" for the current temperature in the field.



SECTION THRU PTFE

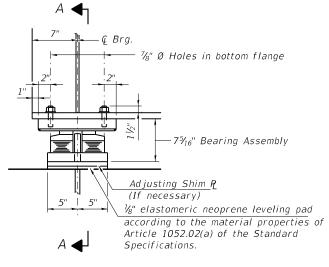
BILL OF MATERIAL

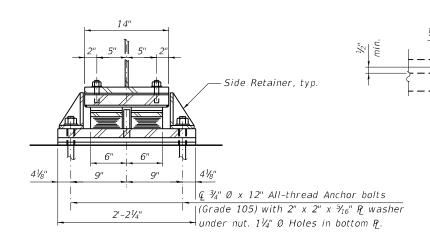
Item	Unit	Total
Elastomeric Bearing Assembly, Type II	Each	4
Anchor Bolts, ¾"	Each	8

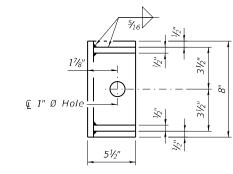
NOTES:

- 1. Side retainers and leveling pad required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type II.
- 2. The 1/8" PTFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.
- 3. Bonding of $\frac{1}{8}$ " PTFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.
- 4. Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used. 5. The structural steel plates of the Bearing Assembly shall
- conform to the requirements of AASHTO M270 Grade 50.
- 6. Two $\frac{1}{8}$ " adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on the bearing details.

DRAWN RMG CHECKED MEH CHECKED	2024	REVISION	DATE	BY	REMARKS	CHECKED MFH	Alfred Benesch & Company 35 West Wacker Onive, Sutie 3300	CITY OF DIXON RIVER CROSSING SHARED-USE PATH	DIXON	ELASTOMERIC BEARING TYPE II DETAILS	F.A.P. SECT 22-0018	rion cour	TOTA SHEE	TS NO.
	λE					DRAWN DMG				31RUCTURE NO. 052-0082			TDACT NO	05762
	7/2 -					CHECKED MFH	312-0654/450 Job No, 10869.00	2024	Here to Serve	SHEET NO. S-36 OF S-50 SHEETS	WHA# 1369	ILLINOIS FED. AID PROJECT	5LY7(916)	. 83/62







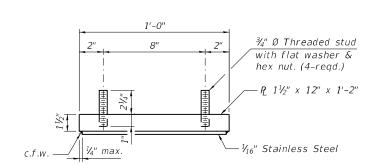
SIDE RETAINER

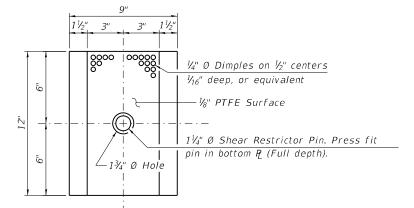
Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

<u>ELEVATION</u>

<u>SECTION A-A</u>

TYPE III ELASTOMERIC EXP. BRG. (N. Brg. Pier 4)

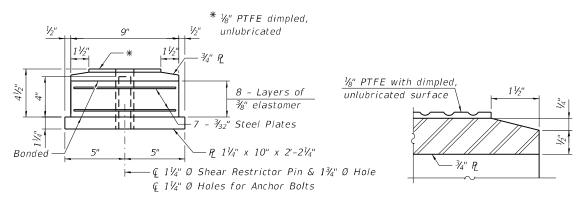




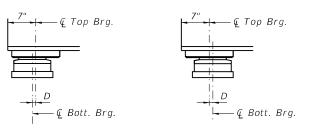
TOP BEARING ASSEMBLY

 \bigcirc

PLAN-PTFE ELASTOMERIC BRG.



SECTION THRU PTFE



BELOW 50°F. $D=\frac{1}{6}$ " per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

EXPANSION BEARING ORIENTATION

The above diagrams are for informational purposes only to show the amount of expected offset "D" for the current temperature in the field.

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type III	Each	2
Anchor Bolts, ¾"	Each	4

NOTES:

- Side retainers and leveling pad required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type III.
- 2. The 1/8" PTFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.
- 3. Bonding of 1/8" PTFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.
- Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
 The structural steel plates of the Bearing Assembly shall
- conform to the requirements of AASHTO M270 Grade 50.

 Two ¼" adjusting shims shall be provided for each
- 6. Two $\frac{V_8}{8}$ adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on the bearing details.

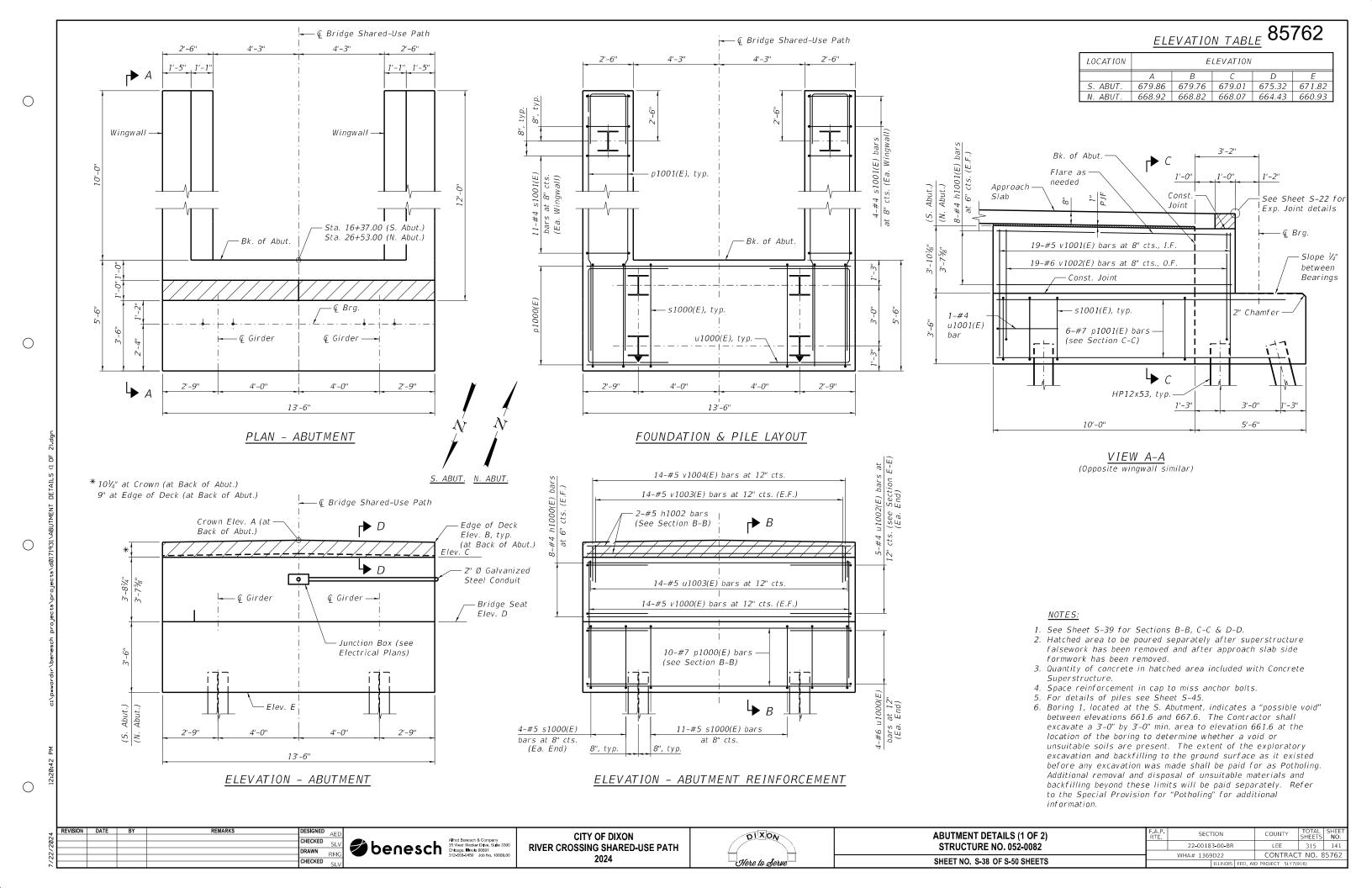
BOTTOM BEARING ASSEMBLY





ELASTOMERIC BEARING TYPE III DETAILS STRUCTURE NO. 052-0082	
SHEET NO. S-37 OF S-50 SHEETS	

	F.A.P. RTE	SEC ⁻	COUNTY	TOTAL SHEETS	SHEET NO.		
		22-00183-00-BR			LEE	315	140
-	WHA# 1369D22				CONTRAC	T NO. 8	5762
	ILLINOIS FED. AI				D PROJECT 5LY7(9	916)	



ABUTMENT

#4

#5

#6

#4

#5

#5

#5

#6

#5

#5

16

13'-2"

13'-2"

13'-2"

15'-2"

11'-1"

13'-9"

7'-11"

3'-4"

6'-9"

6'-9"

6'-2"

2'-3"

3'-8"

Cu. Yd.

Cu. Yd.

Pound

Foot

Foot

Each

Each

n

203

47.4

5,510

390

390

2

12

— @ Brg. Unit 1 or Unit 3 BILL OF MATERIAL Abutment (Span 1 or Span 10) (Quantity for N. & S. Abutments) 1'-0" — h1002(E) 21/8" (S. Abut.) Bar No. | Size | Length | Shape v1004(E)-31/4" (N. Abut.) h1000(E) 32 1½" cl. PILE DATA h1001(E) 32 #4 11'-8" *S. Abut.: 9% at Crown (at F.F./Backwall) h1002(E), typ. SOUTH ABUTMENT h1002(E) 8 h<u>1002(E</u> 85%" at Edge of Deck (at F.F./Backwall) Type: HP12x53 with Pile Shoes N. Abut.: $10\frac{1}{4}$ " at Crown (at F.F./Backwall) p1000(E) 20 #7 onst Joint Nominal Required Bearing: 164 kips 9" at Edge of Deck (at F.F./Backwall) Factored Resistance Available: 90 kips p1001(E) 24 #7 v 1003(E) u1003(E) Est. Length: 38 ft. u1003(E) No. Production Piles: 5 s1000(E) s1001(E) No. Test Piles: 1 60 #4 Back of Abut. v1003(E),-Slope 1/4" between bearings typ. NORTH ABUTMENT - h1000(E), typ. u1000(E) 2" Chamfer h1000(E) Type: HP12x53 with Pile Shoes u1001(E) 4 Const. Joint -Nominal Required Bearing: 147 kips 20 #4 9'-11" u1002(E) (S. Factored Resistance Available: 81 kips 28 u1003(E) Est. Length: 40 ft. v1000(E) No. Production Piles: 5 v1000(E) v1000(E) 56 No. Test Piles: 1 76 v1001(E) - s1000(E) v1002(E) 76 p1000(E), typ. * 41/8" (S. Abut.) v1003(E) 56 1'-2" 28 3¾" (N. Abut.) v 1004(E) SECTION D-D Structure Excavation Concrete Structures Reinforcement Bars, Epoxy Coated Furnishing Steel Piles - Stub HP12x53 1'-3" 3'-0" Driving Piles - h1000(E) 1000(E) Test Pile Steel Backwall 5'-6" – u1003(E) u1002(E) HP12x53 Pile Shoes -Wingwall SECTION B-B (s1000(E)) Approach Slab — (S1001(E)) – h1001(E) Bk. of Abut. -v1001(E) h1001(E), typ. v1002(E) 5'-6" BAR v1002(E) BAR v1004(E) Ε v1001(E) 1'-1" 1'-5" - Const. Joint 2'-6" BAR s1000(E) AND s1001(E) u(E) Bar Dimensions v 1002(E) s1001(E) SECTION E-E u1000(E) 5'-1" 4'-4" u1001(E) 2'-1" 2'-11" p1001(E), typ. – u1002(E) 1'-6½" 4'-2" u1003(E) 1'-8" 10" BARS u1000(E), u1001(E), u1002(E), & u1003(E)

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1'-3"

2'-6" SECTION C-C

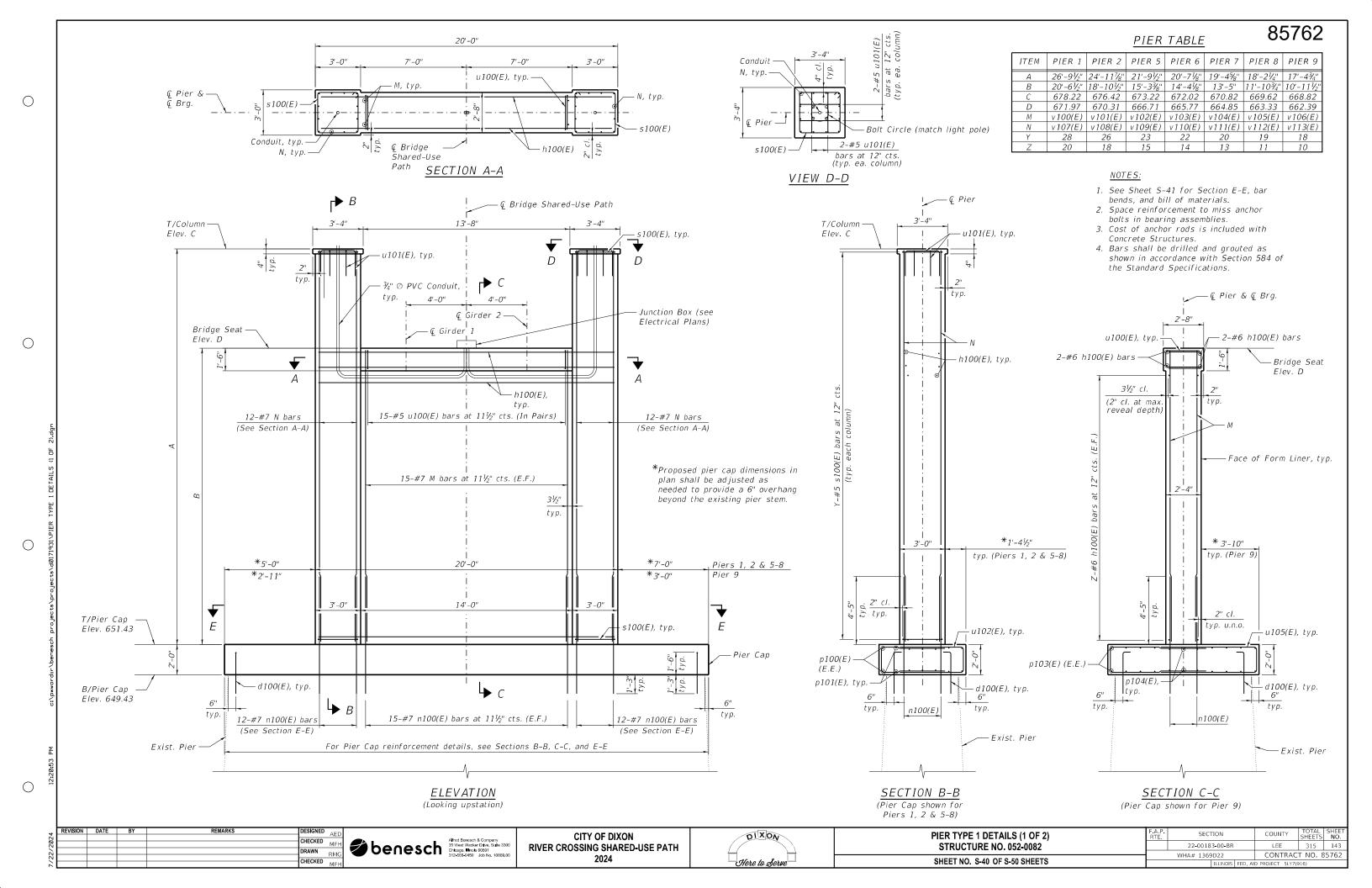
1'-3"

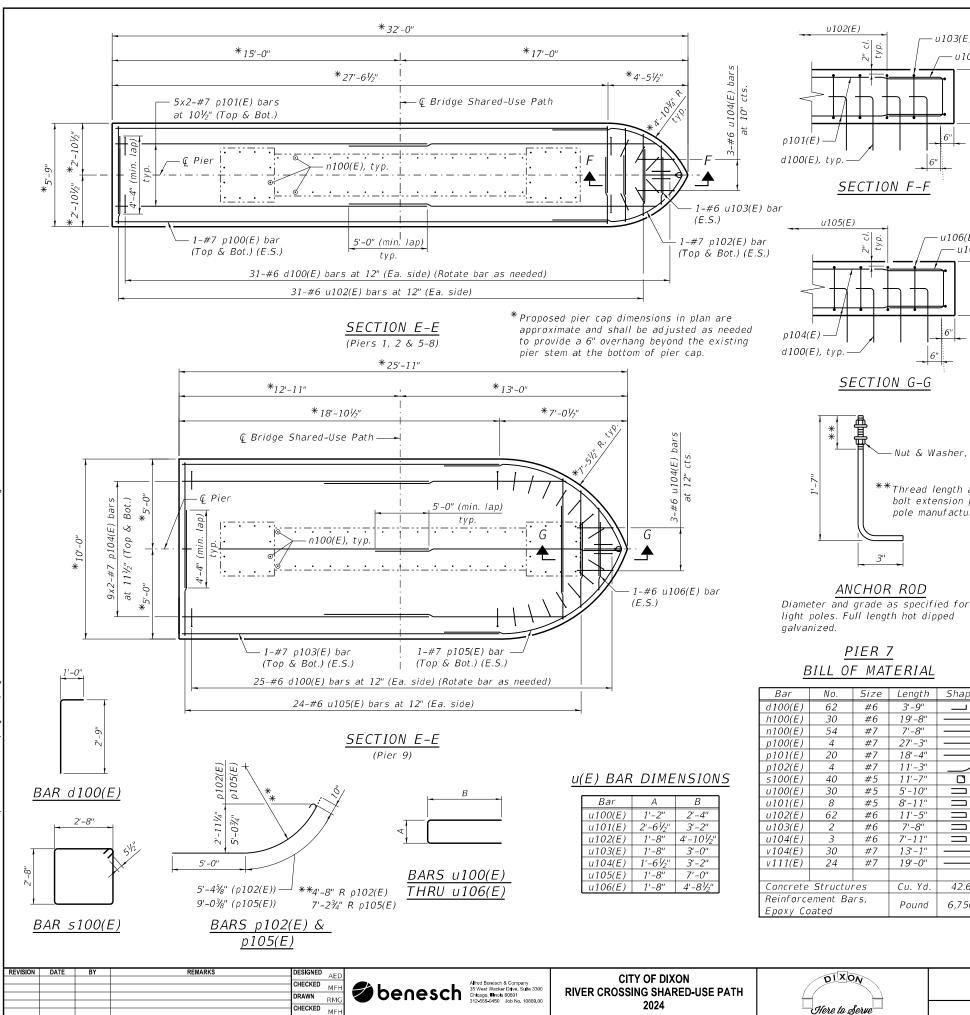
For superstructure details in Section D-D, see Sheets S-13 to S-19.

Bar A

В

24 F	EVISION	DATE	BY	REMARKS	DESIGNED AE	Alfred Benesch & Company	CITY OF DIXON	DIXON	ABUTMENT DETAILS (2 OF 2)	RTE.	SECTION	COUNTY	SHEETS	NO.
750 -					CHECKED SL	benesch 35 West Wacker Drive, Sulte 3300 Chicago, Minois 60601	RIVER CROSSING SHARED-USE PATH		STRUCTURE NO. 052-0082		22-00183-00-BR	LEE	315	142
7/22					CHECKED SL	312-565-0450 Job No. 10869.00	2024	Here to Serve	SHEET NO. S-39 OF S-50 SHEETS	WH	HA# 1369D22 ILLINOIS FED	CONTRAC D. AID PROJECT 5LY7	CT NO. (916)	35762





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PIER 1 BILL OF MATERIAL

Bar	No.	Size	Length	Shape
				Silape
d100(E)	62	#6	3'-9"	
h100(E)	44	#6	19'-8"	
n100(E)	54	#7	7'-8"	
p100(E)	4	#7	27'-3"	
p101(E)	20	#7	18'-4"	
p102(E)	4	#7	11'-3"	
s100(E)	56	#5	11'-7"	C)
u100(E)	30	#5	5'-10"	
u101(E)	8	#5	8'-11"	
u102(E)	62	#6	11'-5"	
u103(E)	2	#6	7'-8"	
u104(E)	3	#6	7'-11"	
v100(E)	30	#7	20'-2"	
v107(E)	24	#7	26'-5"	
Concrete	Structu	Cu. Yd.	56.2	
Reinforce	ement Ba	Pound	8,150	
Ероху Са	ated		Pouna	0,150

PIER 2 85762 BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d100(E)	62	#6	3'-9"	7
h100(E)	40	#6	19'-8"	
n100(E)	54	#7	7'-8"	
p100(E)	4	#7	27'-3"	
p101(E)	20	#7	18'-4"	
p102(E)	4	#7	11'-3"	
s100(E)	52	#5	11'-7"	
u100(E)	30	#5	5'-10"	П
u101(E)	8	#5	8'-11"	П
u102(E)	62	#6	11'-5"	П
u103(E)	2	#6	7'-8"	П
u104(E)	3	#6	7'-11"	П
v101(E)	30	#7	18'-6"	
v108(E)	24	#7	24'-7"	
Concrete	Structui	Cu. Yd.	53.0	
Reinforce		Pound	7,790	
Epoxy Co	pated			. ,. 50

PIER 5 BILL OF MATERIAL

d100(E) 62 #6 3'-9" - h100(E) 34 #6 19'-8" - n100(E) 54 #7 7'-8" - p100(E) 4 #7 27'-3" - p101(E) 20 #7 18'-4" - p102(E) 4 #7 11'-3" _ s100(E) 46 #5 11'-7" _ u100(E) 30 #5 5'-10" _ u101(E) 8 #5 8'-11" _ u102(E) 62 #6 11'-5" _ u103(E) 2 #6 7'-8" _ u104(E) 3 #6 7'-11" =	nape
h100(E) 34 #6 19-8" — n100(E) 54 #7 7'-8" — p100(E) 4 #7 27'-3" — p101(E) 20 #7 18'-4" — p102(E) 4 #7 11'-3" — s100(E) 46 #5 11'-7" — u100(E) 30 #5 5'-10" — u101(E) 8 #5 8'-11" — u102(E) 62 #6 11'-5" — u103(E) 2 #6 7'-8" — u104(E) 3 #6 7'-11" —	
n100(E) 54 #7 7'-8" — p100(E) 4 #7 27'-3" — p101(E) 20 #7 18'-4" — p102(E) 4 #7 11'-3" _ s100(E) 46 #5 11'-7" _ u100(E) 30 #5 5'-10" _ u101(E) 8 #5 8'-11" _ u102(E) 62 #6 11'-5" _ u103(E) 2 #6 7'-8" _ u104(E) 3 #6 7'-11" _	-
p100(E) 4 #7 27'-3" — p101(E) 20 #7 18'-4" — p102(E) 4 #7 11'-3" _ s100(E) 46 #5 11'-7" _ u100(E) 30 #5 5'-10" _ u101(E) 8 #5 8'-11" _ u102(E) 62 #6 11'-5" _ u103(E) 2 #6 7'-8" _ u104(E) 3 #6 7'-11" _	
p101(E) 20 #7 18'-4" — p102(E) 4 #7 11'-3" — s100(E) 46 #5 11'-7" — u100(E) 30 #5 5'-10" — u101(E) 8 #5 8'-11" — u102(E) 62 #6 11'-5" — u103(E) 2 #6 7'-8" — u104(E) 3 #6 7'-11" —	
p102(E) 4 #7 11'-3"	
\$100(E) 46 #5 \$11'-7" \$u100(E) 30 #5 \$5'-10" = \$u101(E) 8 #5 \$8'-11" = \$u102(E) 62 #6 \$11'-5" = \$u103(E) 2 #6 \$7'-8" = \$u104(E) 3 #6 \$7'-11" =	
u100(E) 30 #5 5'-10" = u101(E) 8 #5 8'-11" = u102(E) 62 #6 11'-5" = u103(E) 2 #6 7'-8" = u104(E) 3 #6 7'-11" =	ノ
u101(E) 8 #5 8'-11" = u102(E) 62 #6 11'-5" = u103(E) 2 #6 7'-8" = u104(E) 3 #6 7'-11" =	
u102(E) 62 #6 11'-5" = u103(E) 2 #6 7'-8" = u104(E) 3 #6 7'-11" =	\supset
u103(E) 2 #6 7'-8" = u104(E) 3 #6 7'-11" =	\supset
u104(E) 3 #6 7'-11" =	–
0.10 N(2)	\supset
v102/E) 20 #7 1// 11//	\supset
v102(E) 30 #7 14'-11" 	
v109(E) 24 #7 21'-5" —	
Concrete Structures Cu. Yd. 4	6.5
Reinforcement Bars, Epoxy Coated Pound 7,	170

PIER 6 BILL OF MATERIAL

	L / \1 / \L			
	1	0.1		
Bar	No.	Size	Length	Shape
d100(E)	62	#6	3'-9"	
h100(E)	32	#6	19'-8"	
n100(E)	54	#7	7'-8"	
p100(E)	4	#7	27'-3"	
p101(E)	20	#7	18'-4"	
p102(E)	4	#7	11'-3"	
s100(E)	44	#5	11'-7"	
u100(E)	30	#5	5'-10"	П
u101(E)	8	#5	8'-11"	
u102(E)	62	#6	11'-5"	
u103(E)	2	#6	7'-8"	
u104(E)	3	#6	7'-11"	
v103(E)	30	#7	14'-0"	
v110(E)	24	#7	20'-3"	
Concret	e Structui	Cu. Yd.	44.5	
Reinfor	cement Ba	Pound	6,970	
Ероху (Coated		i oana	0,970

PIER 7 BILL OF MATERIAL

- Nut & Washer, typ.

**Thread length and

bolt extension per

pole manufacturer.

— u103(E)

— и104(Е)

- u106(E)

— u104(Е)

Bar	No.	Size	Length	Shape				
d100(E)	62	#6	3'-9"	L				
h100(E)	30	#6	19'-8"					
n100(E)	54	#7	7'-8"					
p100(E)	4	#7	27'-3"					
p101(E)	20	#7	18'-4"					
p102(E)	4	#7	11'-3"					
s100(E)	40	#5	11'-7"	C)				
u100(E)	30	#5	5'-10"					
u101(E)	8	#5	8'-11"					
u102(E)	62	#6	11'-5"					
u103(E)	2	#6	7'-8"	IJ				
u104(E)	3	#6	7'-11"					
v104(E)	30	#7	13'-1"					
v111(E)	24	#7	19'-0"					
Concrete	Structui	Cu. Yd.	42.6					
Reinforce	ement Ba	Pound	6,750					
Ероху Сс	pated		Found	0,730				

PIER 8 BILL OF MATERIAL

=	BILL OF THIN LINE							
Bar	No.	Size	Length	Shape				
d100(E)	62	#6	3'-9"					
h100(E)	26	#6	19'-8"					
n100(E)	54	#7	7'-8"					
p100(E)	4	#7	27'-3"					
p101(E)	20	#7	18'-4"					
p102(E)	4	#7	11'-3"					
s100(E)	38	#5	11'-7"	0				
u100(E)	30	#5	5'-10"					
u101(E)	8	#5	8'-11"					
u102(E)	62	#6	11'-5"					
u103(E)	2	#6	7'-8"					
u104(E)	3	#6	7'-11"					
v105(E)	30	#7	11'-6"					
v112(E)	24	#7	17'-10"					
Concrete	Structu	Cu. Yd.	40.0					
Reinforce Epoxy Co	ement Ba pated	Pound	6,450					

PIER 9 BILL OF MATERIAL

h100(E) 24 # n100(E) 54 # p103(E) 4 # p104(E) 36 #	
n100(E) 54 # p103(E) 4 # p104(E) 36 #	#7 7'-8" ————————————————————————————————————
p103(E) 4 # p104(E) 36 #	†7 18'-7" ———— †7 15'-4" ———— †7 14'-11"
p104(E) 36 #	±7 15'-4" ————————————————————————————————————
	±7 14'-11"
p105(E) 4 #	
	£5 11'-7" 🖸
s100(E) 36 #	
u100(E) 30 #	£5 5'-10" ===
u101(E) 8 #	£5 8'-11" ===
u104(E) 3 #	£6 7'-11" 🗀
u105(E) 48 #	£6 15'-8" —
u106(E) 2 #	£6 11'-1" ===
v106(E) 30 #	£7 10'-7" —
v113(E) 24 #	£7 17'-0" ———
Concrete Structures	Cu. Yd. 43.0
Reinforcement Bars, Epoxy Coated	Pound 6,620



PIER TYPE 1 DETAILS (2 OF 2) SECTION 22-00183-00-BR LEE **STRUCTURE NO. 052-0082** 315 144 WHA# 1369D22 CONTRACT NO. 85762 SHEET NO. S-41 OF S-50 SHEETS

CONCRETE PEDESTAL THICKNESS & SEAT ELEVATION TABLE

PIER 3 PIER 4 BRG.GIRDER Elev. Elev. t 1 a 670.31 1½" 1½" 670.43 North 670.43 2a 670.31 41/4" 1 a 670.43 5¾" South 11/2" 41/4" 670.34

See Top Plan for

u201(E), typ

Locations, typ.

669.98 669.98 670.34 670.46 670.43 5¾" 670.46

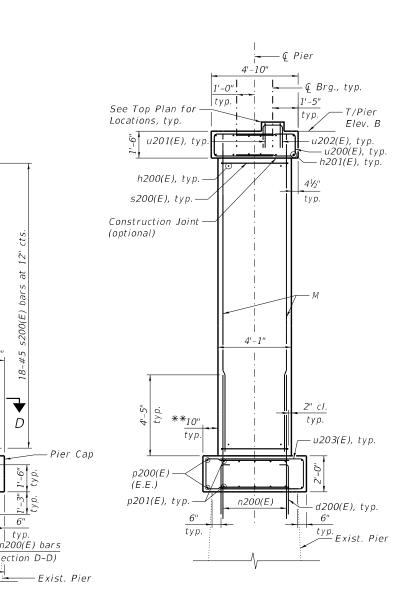
NOTES:

- 1. See Sheet S-43 for Section C-C, Section D-D, bar bends, and bill of materials.
- 2. Space reinforcement to miss anchor bolts in bearing assemblies.
- 3. Bars shall be drilled and grouted as shown in accordance with Section 584 of the Standard Specifications.

4'-10"

typ.

4. All concrete pedestals shall be poured monolithically with the cap.



*Pier 4 only. Pier 3 pedestals

*3-#5 u202(E)

bars at 12" cts.

are unreinforced.

DETAIL A



3½" c1. (2" cl. at max reveal depth — Construction Joint **1'-2½" typ. typ. u.n.o. p200(E) (E.E.) p201(E), typ. n200(E)

SECTION B-B (Pier 4 shown, Pier 3 similar)

typ.

CHECKED DRAWN RMG CHECKED

**1'-3"

D

typ.

14-#7 n200(E) bars (See Section D-D)

3'-0"

d200(E), typ.



22-#7 n200(E) bars at 12" cts. (E.F.)

For Pier Cap reinforcement details, see Sections A-A, B-B, and Section D-D

ELEVATION

(Looking upstation;

Pier 4 shown, Pier 3 similar)

28'-3"

29-#5 u200(E) bars at 12" cts. (In Pairs)

Concrete

Pedestal,

typ.

TOP PLAN

(v200(E) and v201(E) bars not shown for clarity)

28'-3"

8'-0"

u202(E),

22-#7 M bars at 12" cts. (E.F.)

27'-6"

21'-6"

- h201(E)

Construction

Joint (optional) 14-#7 M bars

-Ç Bridge Shared-Use Path

 \downarrow_B

– u201(E), typ.

.| ġ typ. Ì

Pier 3 Only -

- Ç N. Brg.

- & S. Brg.

14'-11/2"

⊈ Girder 1—

See Top Plan for

u200(E),

Junction Box (see -

Electrical Plans)

typ.

14-#7 M bars

(See Section C-C)

Locations, typ.

8'-0"

— Ç Girder 1a

- Pier 4 Only

€ Girder, typ.-

T/Pier

Elev. B

2'-11/2"

 \bigcirc

 \bigcirc

 \bigcirc

T/Pier Cap

Elev. 651.43

B/Pier Cap

Elev. 649.43

– Ç Bridge Shared-Use Path

Pier 3 Only

14'-11/2"

G Girder 2

(See Section C-C)

8'-0"

€ Girder 2a-

typ.

- See Detail A

CITY OF DIXON RIVER CROSSING SHARED-USE PATH 2024

6-#6 h201(E) at 10" cts. (Top & Bot.)

— Pier 4 Only

2'-11/2"

41/2"

typ.

**3'-3"

D

6"

typ.

14-#7 n200(E) bars

(See Section D-D)

**Proposed pier cap dimensions in

beyond the existing pier stem.

needed to provide a 6" overhang

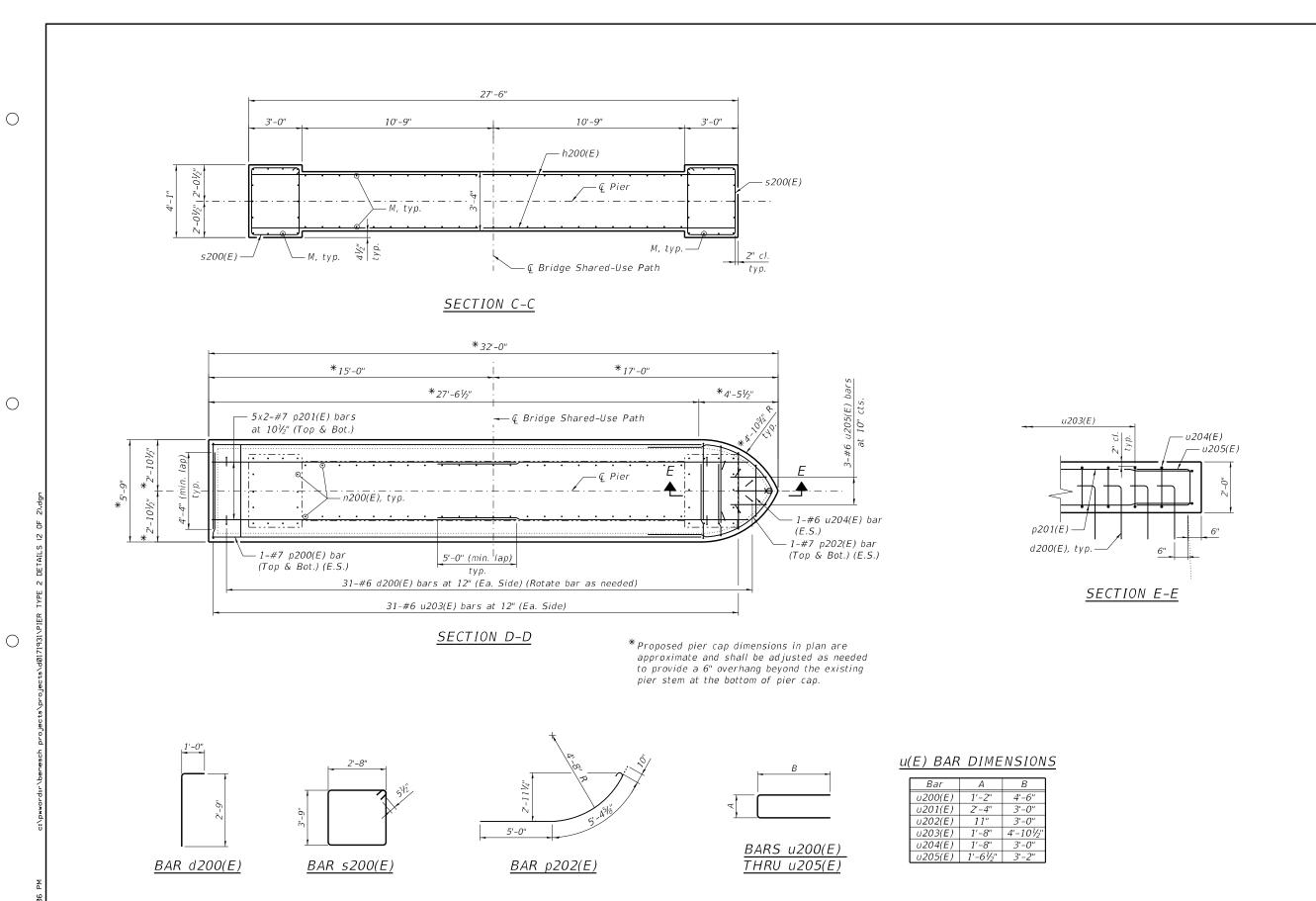
plan shall be adjusted as

3'-0"



PIER TYPE 2 DETAILS (1 OF 2)				
STRUCTURE NO. 052-0082				
SHEET NO. S.42 OF S.50 SHEETS				

SECTION COUNTY 22-00183-00-BR LEE 315 145 WHA# 1369D22 CONTRACT NO. 85762



PIER 3 85762 BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d200(E)	62	#6	3'-9"	
h200(E)	36	#6	27'-2"	
h201(E)	12	#6	27'-11"	
n200(E)	72	#7	7'-8"	
p200(E)	4	#7	27'-3"	
p201(E)	20	#7	18'-4"	
p202(E)	4	#7	11'-3")
s200(E)	36	#5	13'-9"	
u200(E)	58	#5	10'-2"	П
u203(E)	62	#6	11'-5"	П
u204(E)	2	#6	7'-8"	П
u205(E)	3	#6	7'-11"	П
v200(E)	72	#7	18'-6"	
Concrete	Structui	Cu. Yd.	82.7	
Reinforce Epoxy Co		Pound	9,490	

<u>PIER 4</u> BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d200(E)	62	#6	3'-9"	7
h200(E)	36	#6	27'-2"	
h201(E)	12	#6	27'-11"	
n200(E)	72	#7	7'-8"	
p200(E)	4	#7	27'-3"	
p201(E)	20	#7	18'-4"	
p202(E)	4	#7	11'-3"	
s200(E)	36	#5	13'-9"	ľ
u200(E)	58	#5	10'-2"	П
u201(E)	8	#5	8'-4"	П
u202(E)	12	#5	6'-11"	П
u203(E)	62	#6	11'-5"	П
u204(E)	2	#6	7'-8"	П
u205(E)	3	#6	7'-11"	П
v201(E)	72	#7	18'-2"	
Concrete	Concrete Structures			81.7
Reinforcement Bars,			Pound	9,600
Ероху Сс	ated		1 ound	9,000

AED	DESIGNED	REMARKS	BY	DATE	REVISION
ALL	CHECKED				
MFH	CHECKED				
RMG	DRAWN				
KIMC	1				
MEH	CHECKED				







PIER TYPE 2 DETAILS (2 OF 2) STRUCTURE NO. 052-0082		SECTION			COUNTY	TOTAL SHEETS	SHEE
		22-00183-00-BR L			LEE	315	146
		WHA# 1369	D22		CONTRAC	T NO. 8	3576
SHEET NO. S-43 OF S-50 SHEETS			ILLINOIS	FED, All	D PROJECT 5LY7(916)	

2024

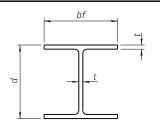
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CHECKED

WHA# 1369D22

SHEET NO. S-44 OF S-50 SHEETS

CONTRACT NO. 85762



STEEL PILE TABLE

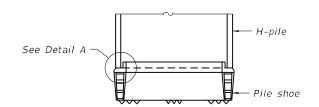
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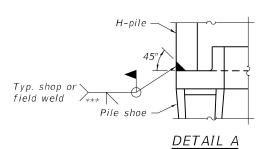
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Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	141/4"	1 4 ⁷ / ₈ "	¹³ / ₁₆ "	30"
x102	14"	143/4"	11/16"	30"
x89	137/8"	1 43/4"	5/8"	30"
x73	135/8"	145%"	1/2"	30"
HP 12x84	121/4"	121/4"	¹ ½16"	24"
x74	12½"	121/4"	5/8"	24"
x63	12"	121/8"	1/2"	24"
x53	1 1 3/4"	12"	7/ ₁₆ "	24"
HP 10x57	10"	101/4"	%16"	24"
x42	9¾"	101/8"	7/ ₁₆ "	24"
HP 8x36	8"	8½"	⁷ / ₁₆ "	18"

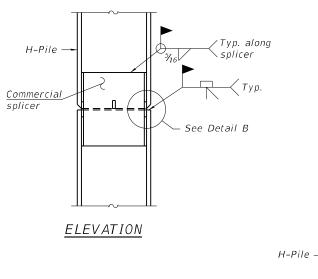


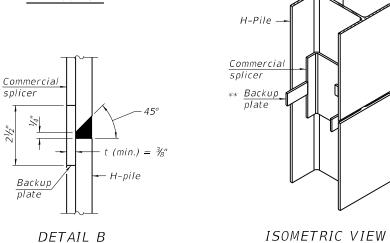
ELEVATION



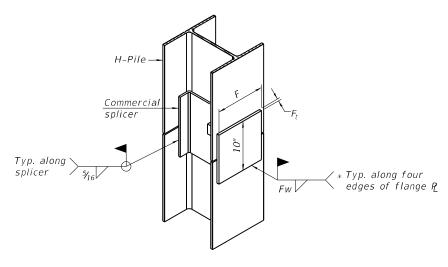
SHOE ATTACHMENT

Note: The steel H-piles shall be according to AASHTO M270 Grade 50.





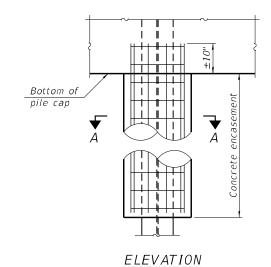
WELDED COMMERCIAL SPLICE



ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- $_{*}$ Interrupt welds $\mathcal{V}_{\!\!4}"$ from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).



Welded wire fabric 6 x 6W4.0 x W4.0 weighing
58#/100 sq. ft. Bend as
required to fit into wall.

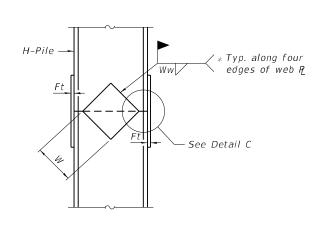
Forms for encasement
may be omitted when
soil conditions permit.

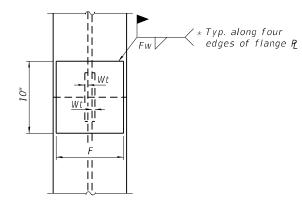
H-pile

<u>SECTION A-A</u>

INDIVIDUAL PILE CONCRETE ENCASEMENT

(when specified)





<u>ELEVATION</u>

Splice plate thickness Ft

DETAIL C

Designation	F	Ft	Fw	W	Wt	Ww
HP 14×117	121/2"	1 1/4"	7/8"	73/4"	5/8"	1/2"
x102	121/2"	1"	3/4"	73/4"	5/8"	1/2"
x89	12½"	7/8"	11/16"	73/4"	5/8"	1/2"
x73	121/2"	3/4"	%16"	73/4"	5/8"	1/2"
HP 12x84	10"	1"	11/16"	6½"	5/8"	1/2"
x74	10"	7/8"	11/16"	6½"	5/8"	1/2"
x63	10"	3/4"	1/2"	6½"	1/2"	3/8"
x53	10"	3/4"	1/2"	6½"	1/2"	3/8"
HP 10x57	8"	7/8"	%16"	5½"	1/2"	3/8"
x42	8"	3/4"	%16"	5½"	1/2"	3/8"

7/₁₆"

4"

1/2"

3/8"

END VIEW

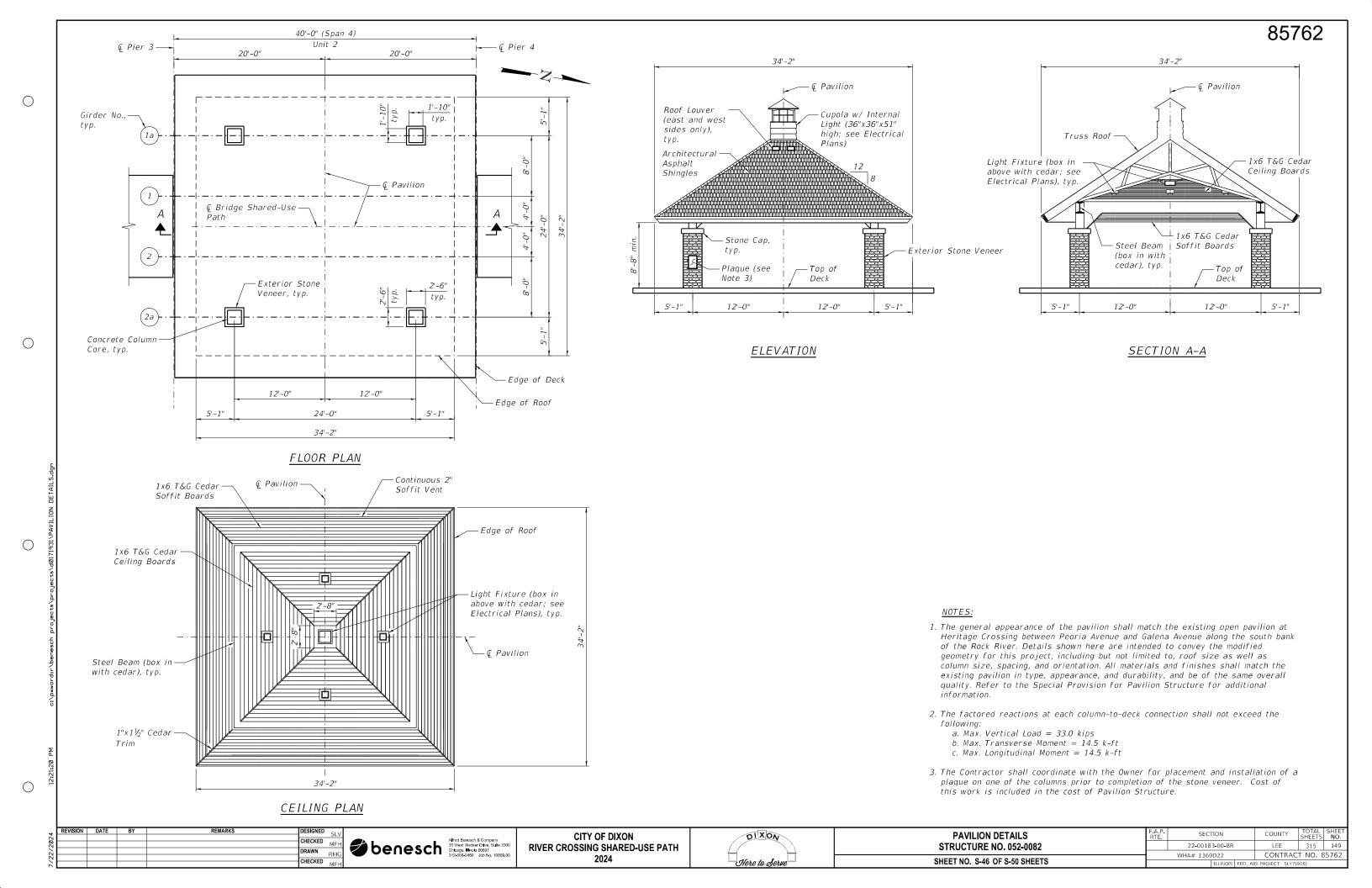
WELDED PLATE FIELD SPLICE

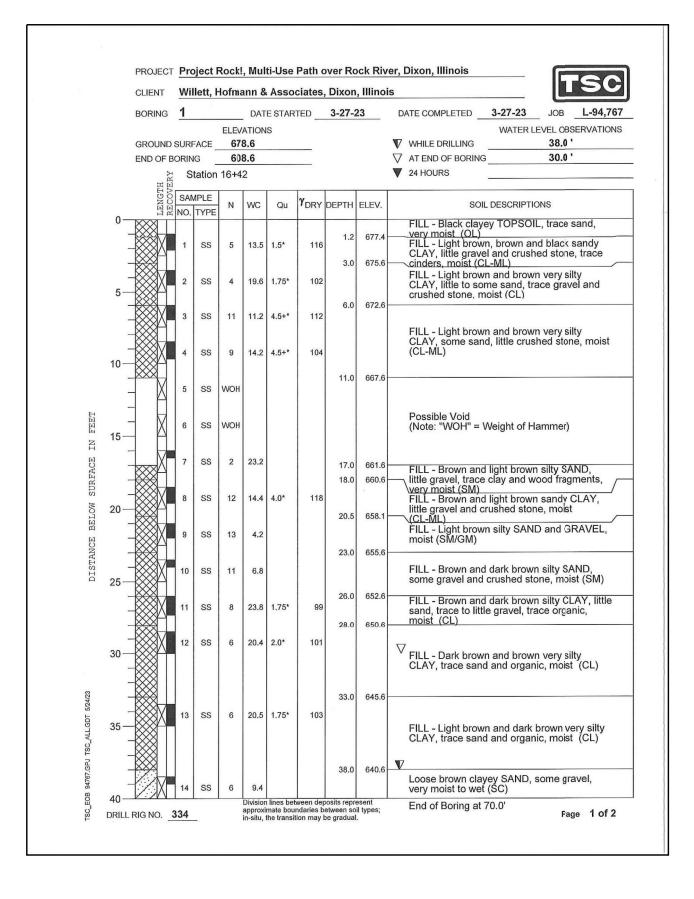
HP 8x36

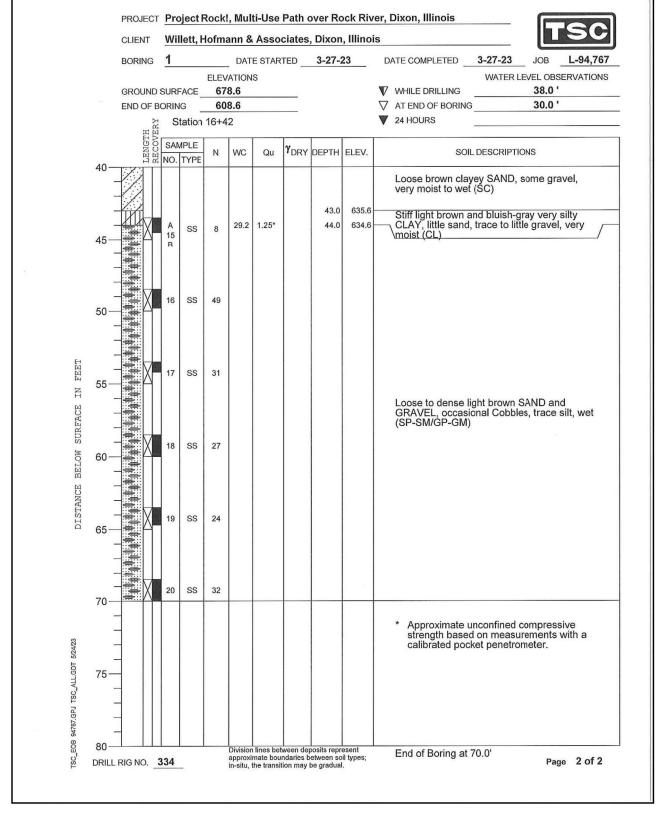
6¾"

5/8"

				,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	ser prie sinee manaractarer (116 min),				
REVIS	ION DATE	BY REMARKS	DESIGNED AFT		CITY OF DIVON	DIXON	HP-PILE DETAILS	F.A.P. SECTION	COUNTY TOTAL SHEET
			CHECKED ME	Alfred Benesch & Company	CITY OF DIXON	DIVON		RTE. SECTION	SHEETS NO.
i			IVIE	35 West Wacker Drive, Suite 3300	RIVER CROSSING SHARED-USE PATH		STRUCTURE NO. 052-0082	22-00183-00-BR	LEE 315 148
i			DRAWN RMC	benesch Chies Michels 60601			01100101C 1101002	WHA# 1369D22	CONTRACT NO 95762
			CHECKED ME	312-305-0450 JOB NO. 10809-00	2024	Clara to Large	SHEET NO. S-45 OF S-50 SHEETS	WHA# 1369D22	CONTRACT NO. 63762
			CHECKED MFH	1		CHere to serve	3nee No. 3-45 Or 3-50 3nee 13	ILLINOIS	FED, AID PROJECT 5LY7(916)







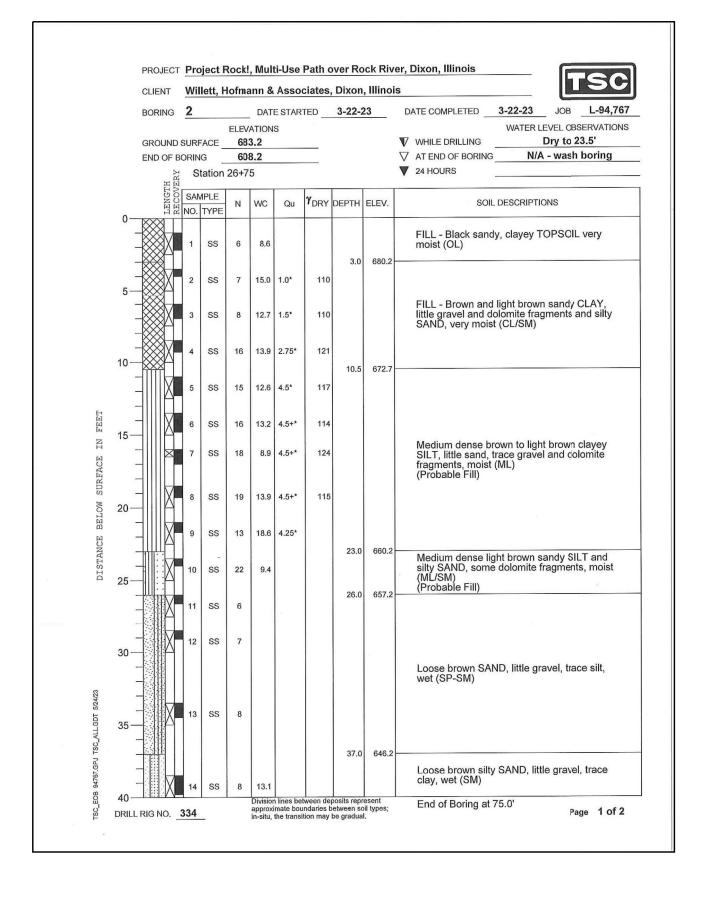
REV	/ISION	DATE	BY	REMARKS	DESIGNED	AED
					CHECKED	ALU
					CHECKED	MFH
					DRAWN	RMG
					CHECKED	KMG
					CHECKED	MFH

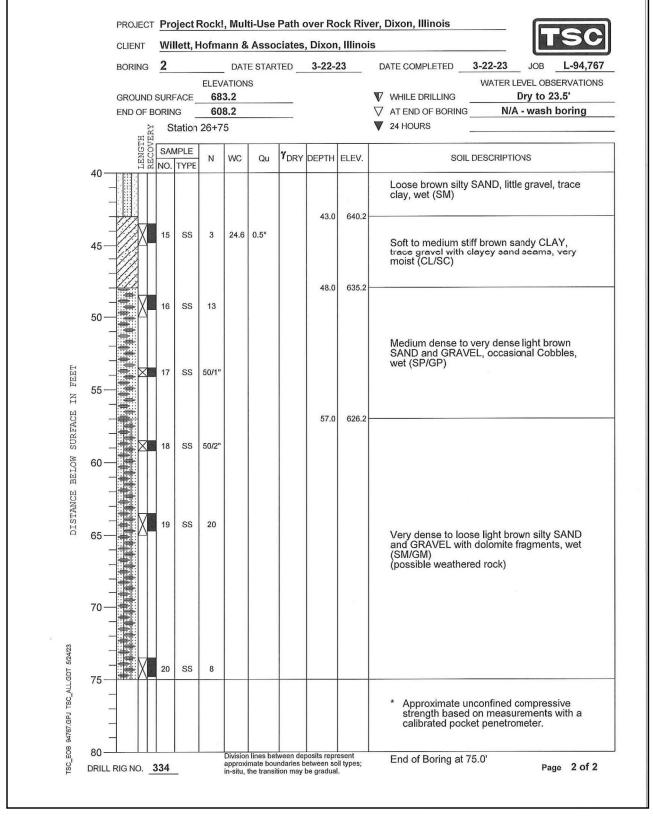
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SOIL BORING LOGS (1 OF 2)		SECTION			COUNTY	TOTAL SHEETS	SHEE NO.
STRUCTURE NO. 052-0082		22-00183-00-BR			LEE	315	150
011555 110 0 45 05 0 50 0115550		WHA# 1369	D22		CONTRAC	T NO. 8	35762
SHEET NO. S-47 OF S-50 SHEETS	ILLINOIS FED. AI			D PROJECT 5LY70	916)		





REVISION	DATE	BY	REMARKS D	DESIGNED	AED
				CHECKED	
					MFH
			D	DRAWN	RMG
			<u></u>	CHECKED	
				SHECKED	MFH

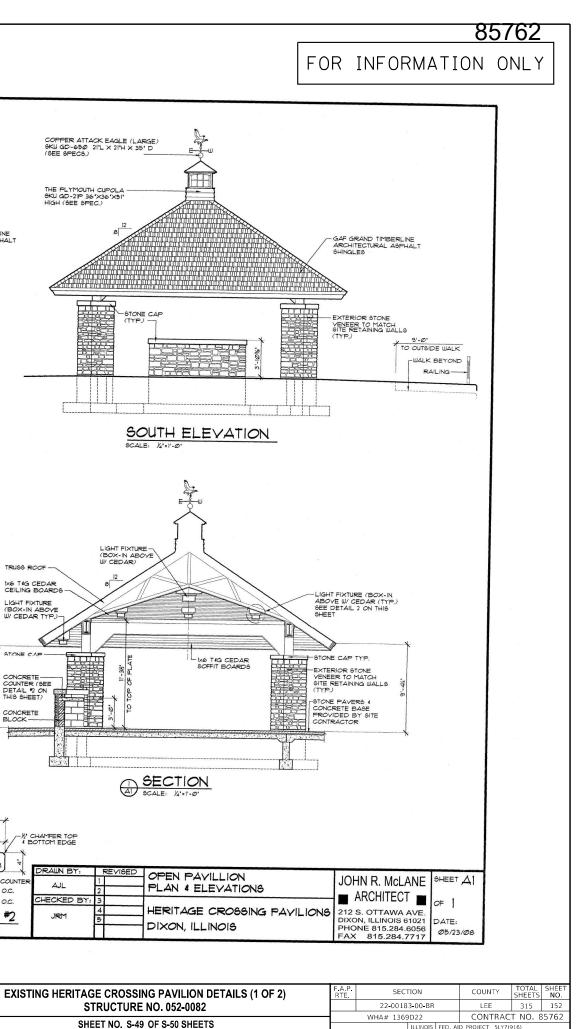
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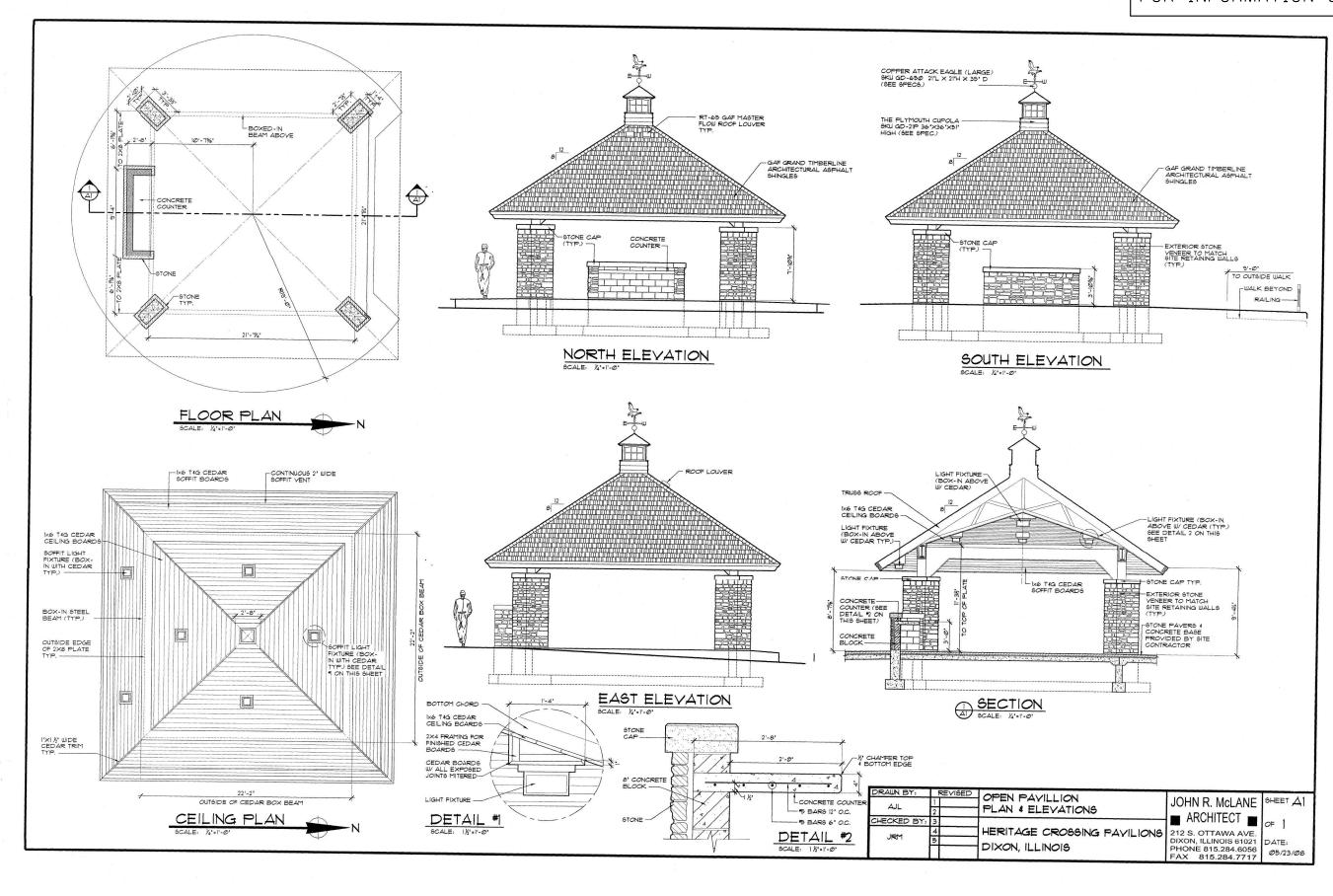
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OIL BORING LOGS (2 OF 2)		SECTION	COUNTY	TOTAL SHEETS	SHEE
STRUCTURE NO. 052-0082		22-00183-00-BR	LEE	315	15
011555 10 0 40 05 0 50 0115550		WHA# 1369D22	CONTRAC	T NO. 8	35762
SHEET NO. S-48 OF S-50 SHEETS		ILLINOIS FED. AI	D PROJECT 5LY7(916)	





DIXON

Here to Serve

CITY OF DIXON

RIVER CROSSING SHARED-USE PATH

2024

Affred Benesch & Company
35 West Wacker Dylve, Style 3300
Chicago, March 6 06031
312-565-4450 Joh No. 10869,00

CHECKED

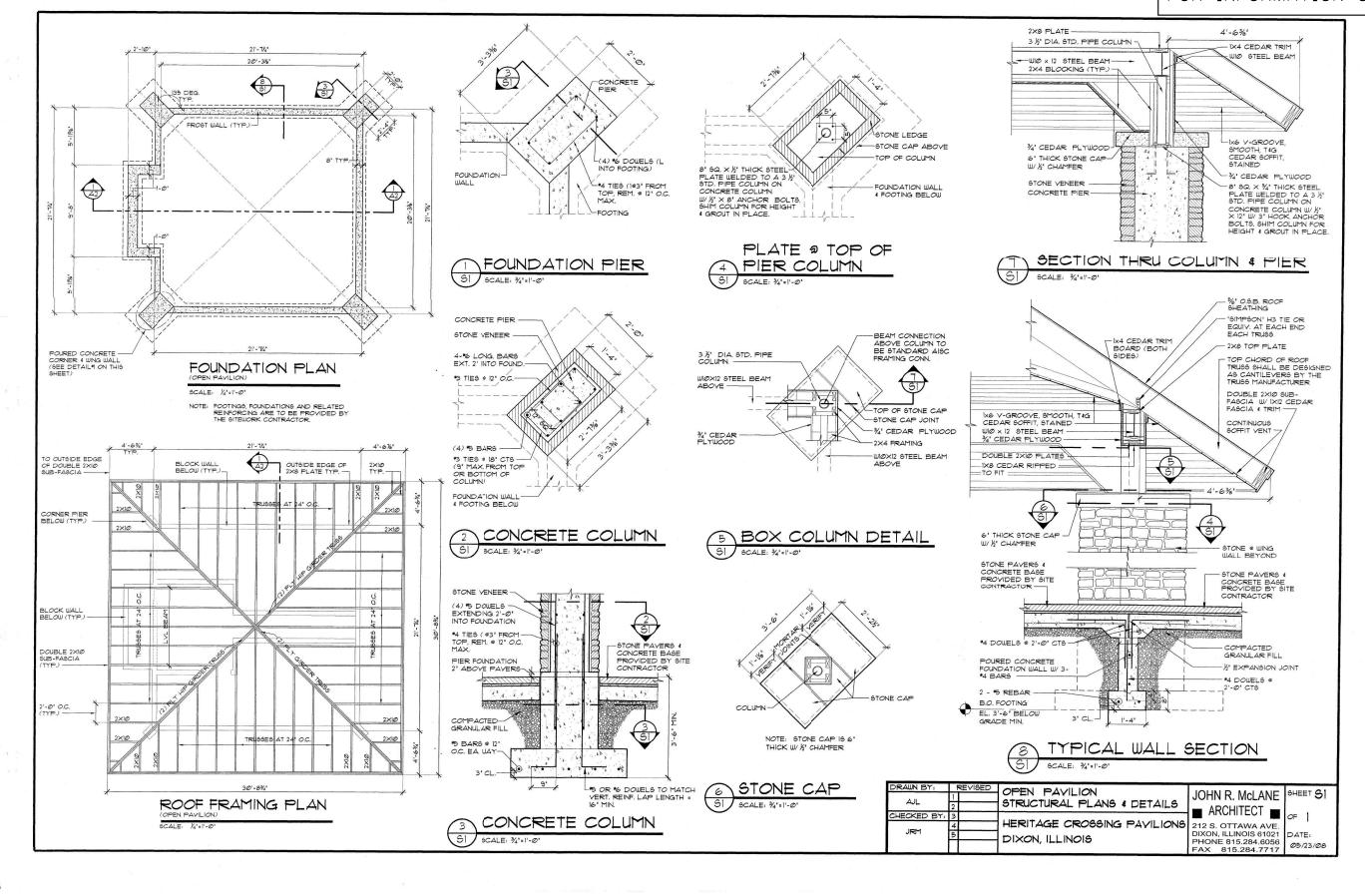
DRAWN RMG

CHECKED

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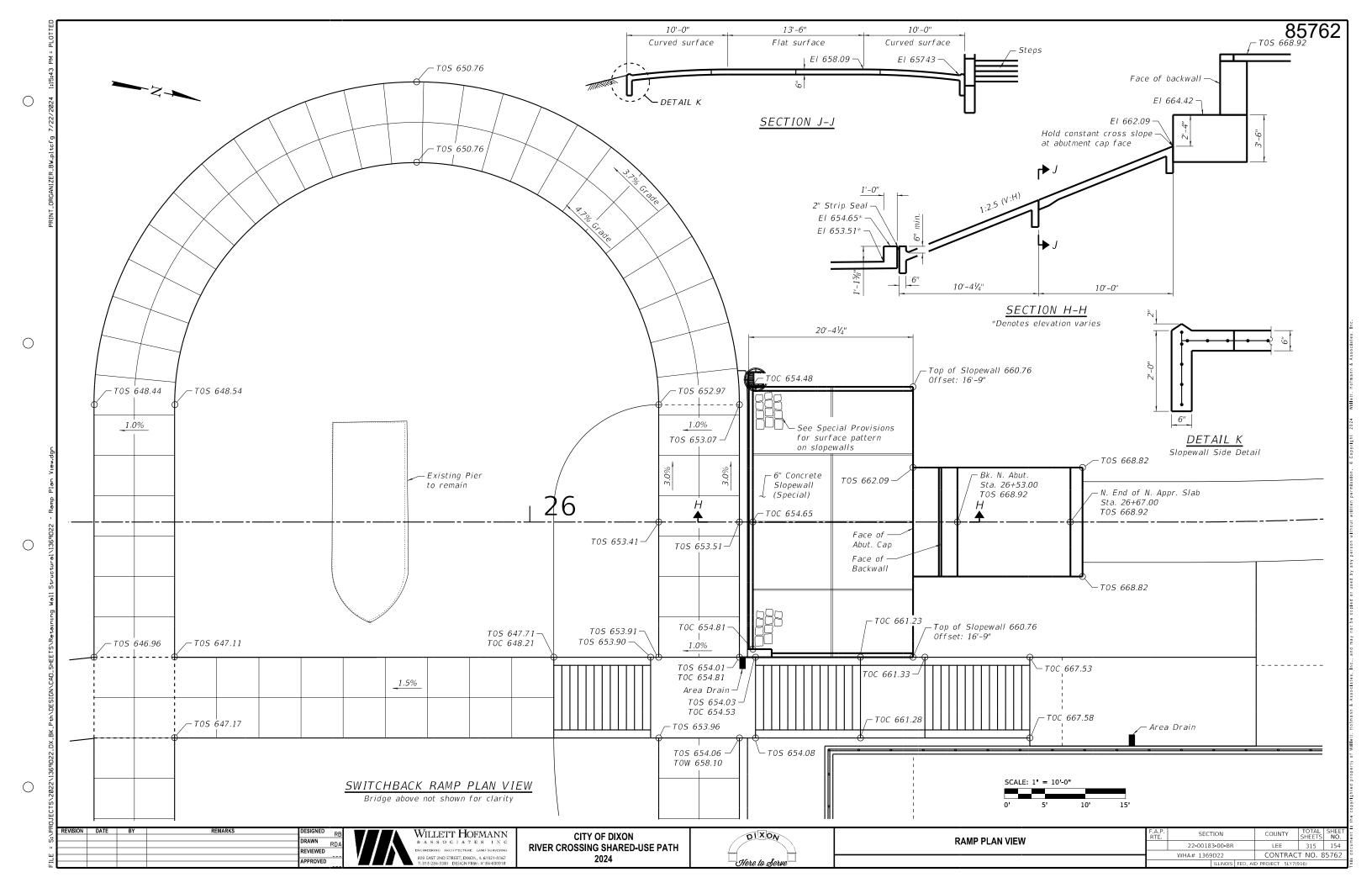
FOR INFORMATION ONLY

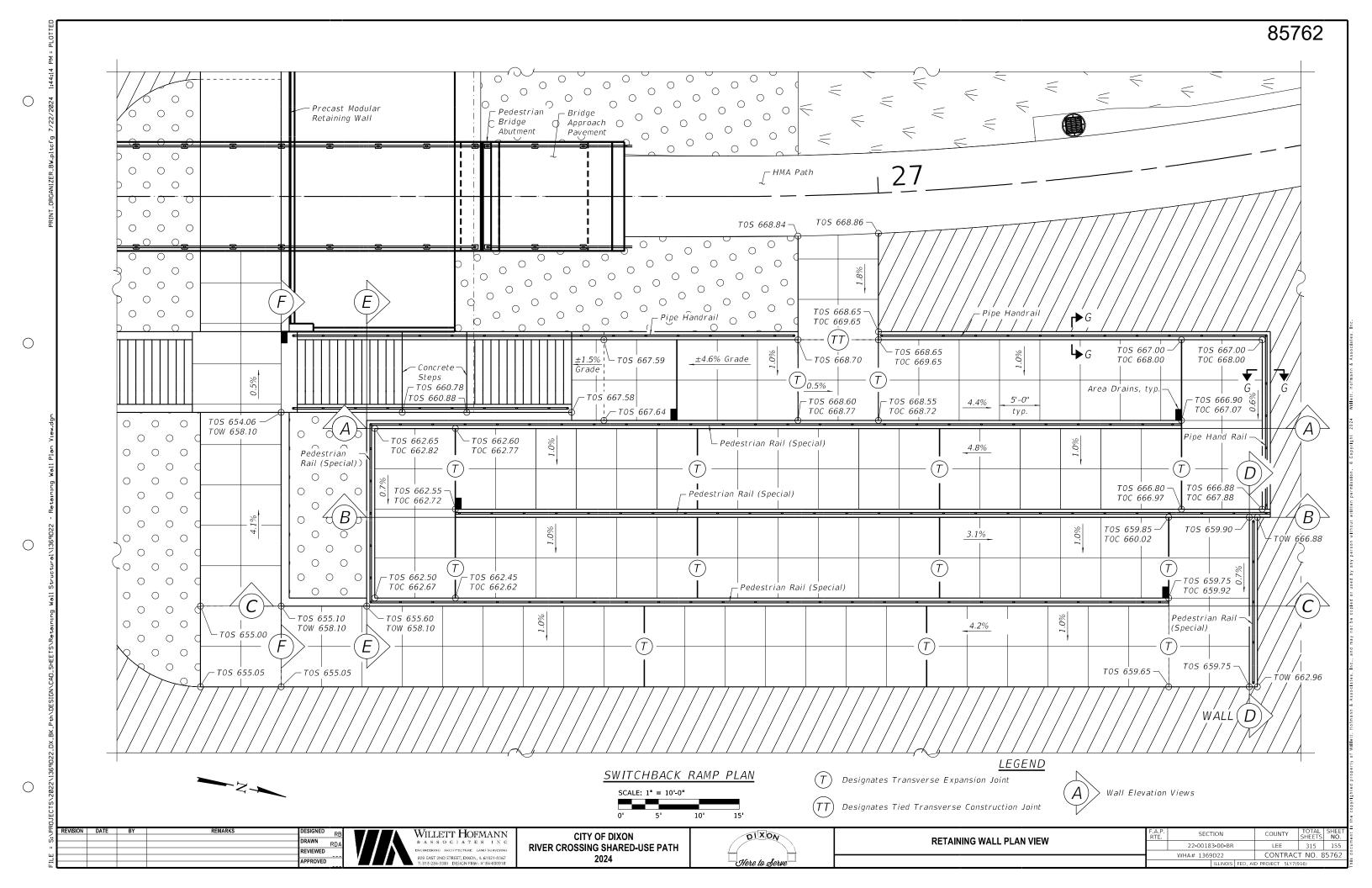


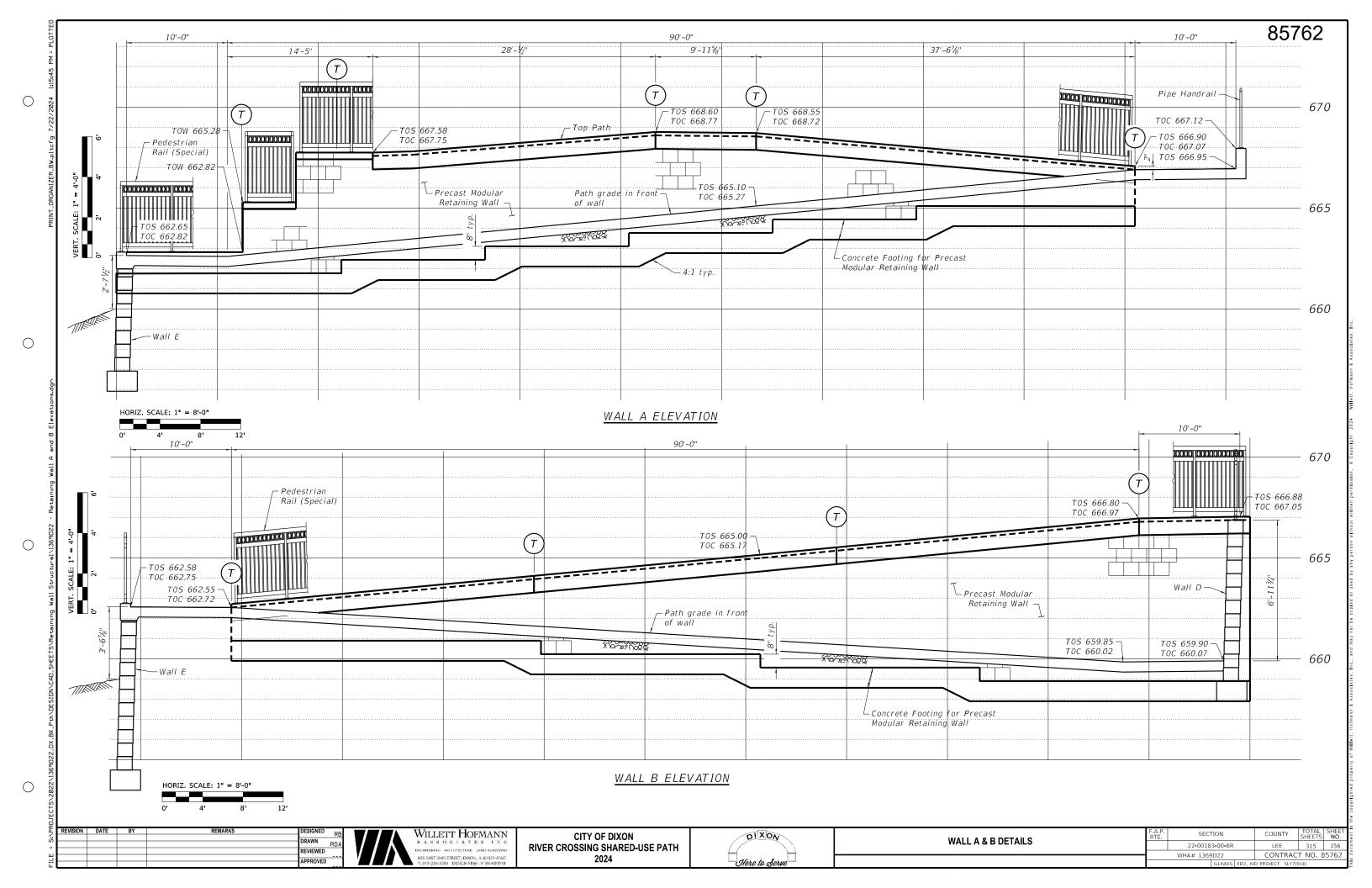
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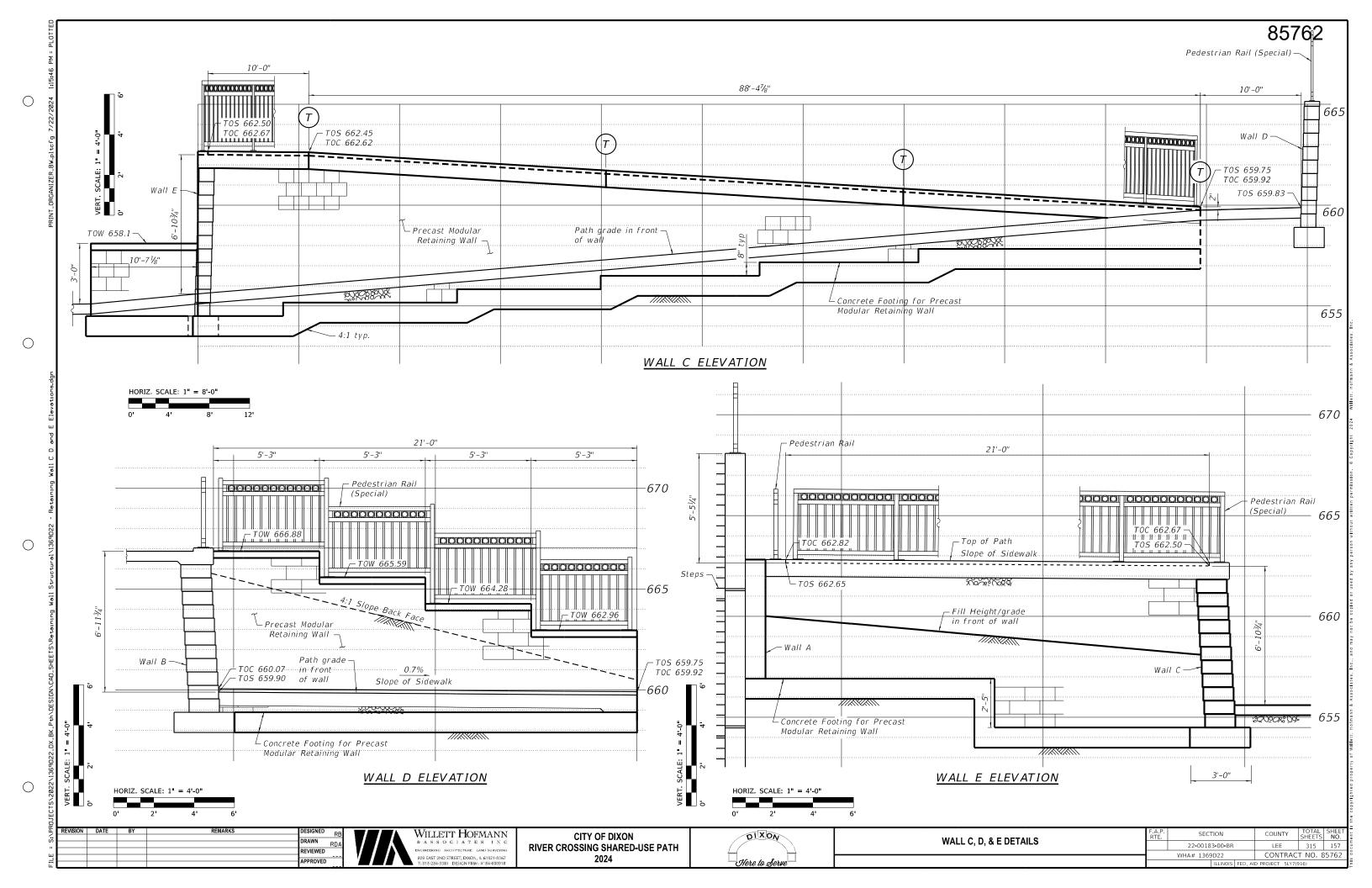
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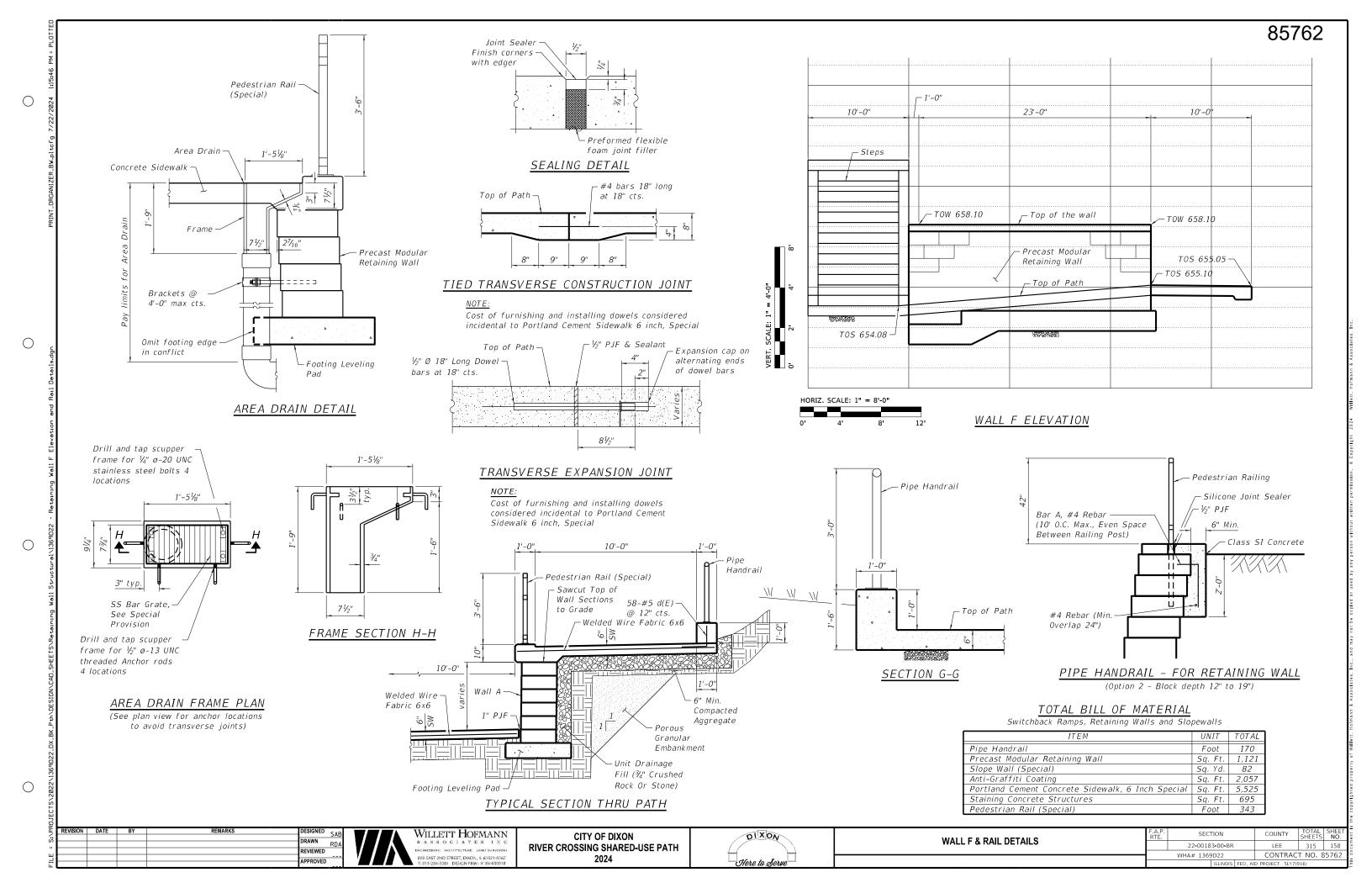
24	REVISION	DATE BY	REMARKS	DESIGNED AED	Affect Bonosch & Company	CITY OF DIXON	DIXON	EXISTING HERITAGE CROSSING PAVILION DETAILS (2 OF 2)	RTE.	SECTION	COUNTY SHEETS NO.
/20				CHECKED MFH	benesch 35 West Wacker Drive, Suite 3300 Chicago, Minols 60601	RIVER CROSSING SHARED-USE PATH		STRUCTURE NO. 052-0082		22-00183-00-BR	LEE 315 153
,22,				CHECKED	312-565-0450 Job No. 10869.00	2024		SHEET NO. S-50 OF S-50 SHEETS		WHA# 1369D22	CONTRACT NO. 85762
				CHECKED MFH			CHERE IO DERVE	SHEET NO. 3-30 OF 3-30 SHEETS		ILLINOIS FED. A	iD PROJECT 5LY7(916)

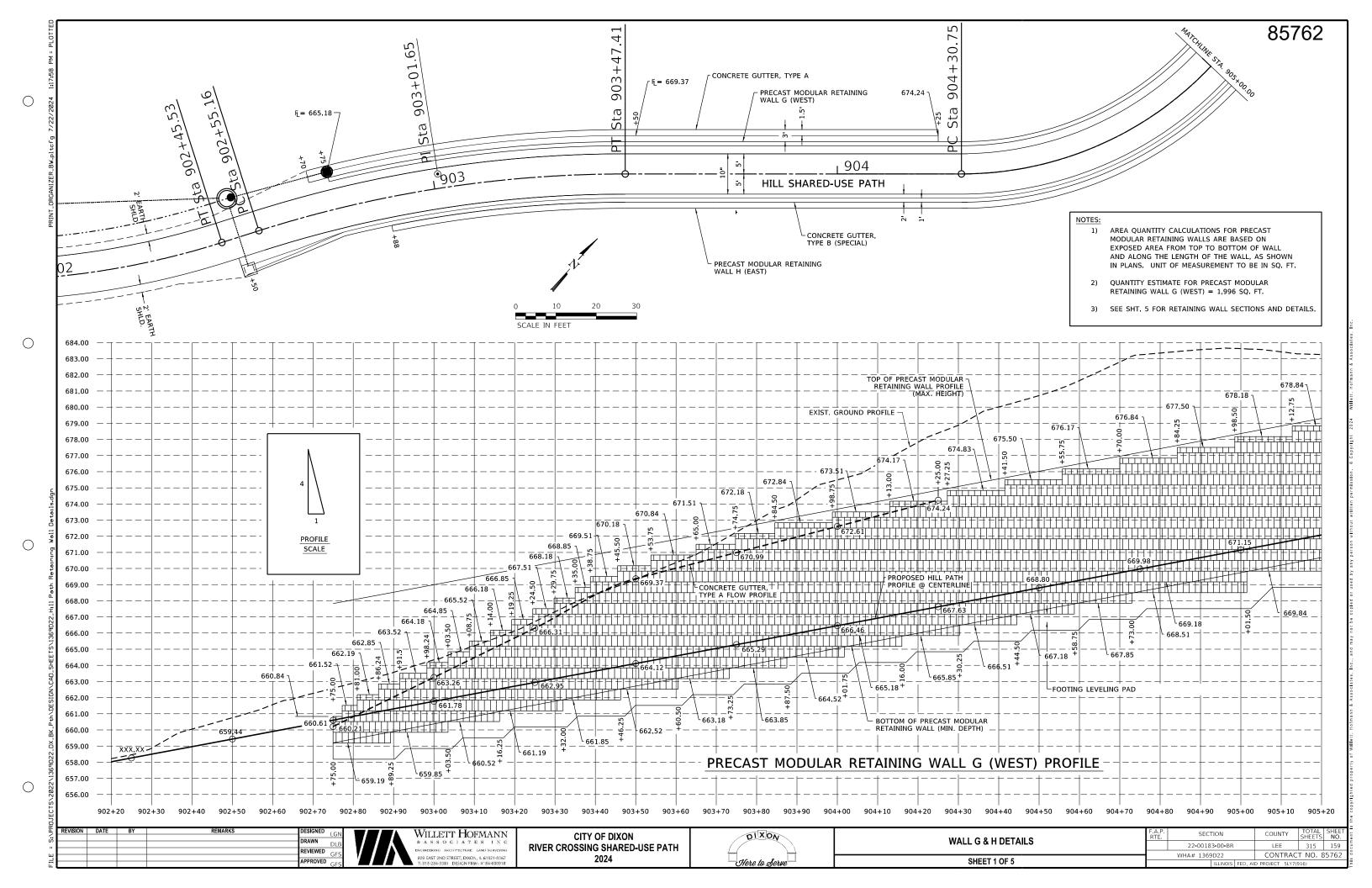


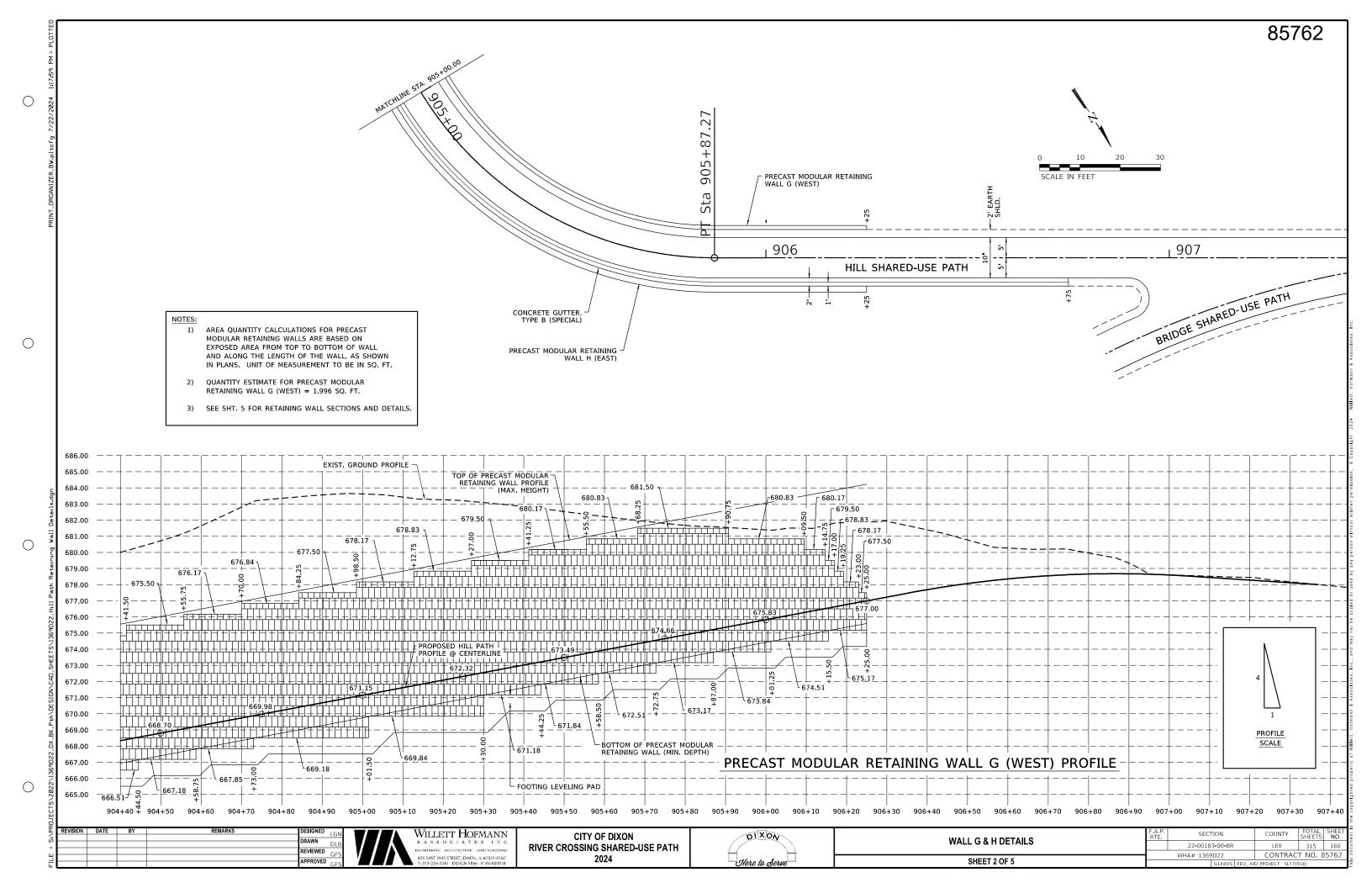


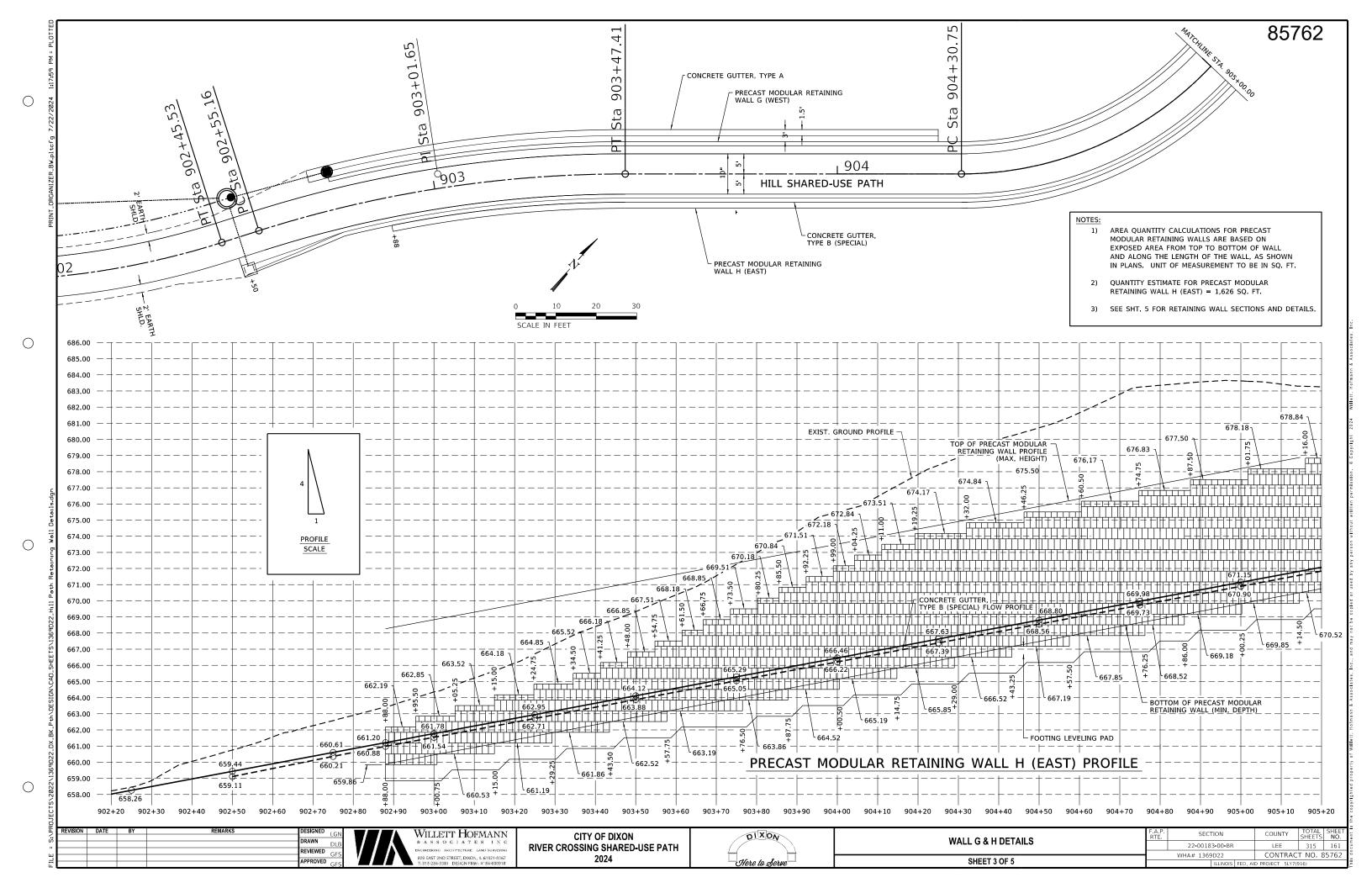


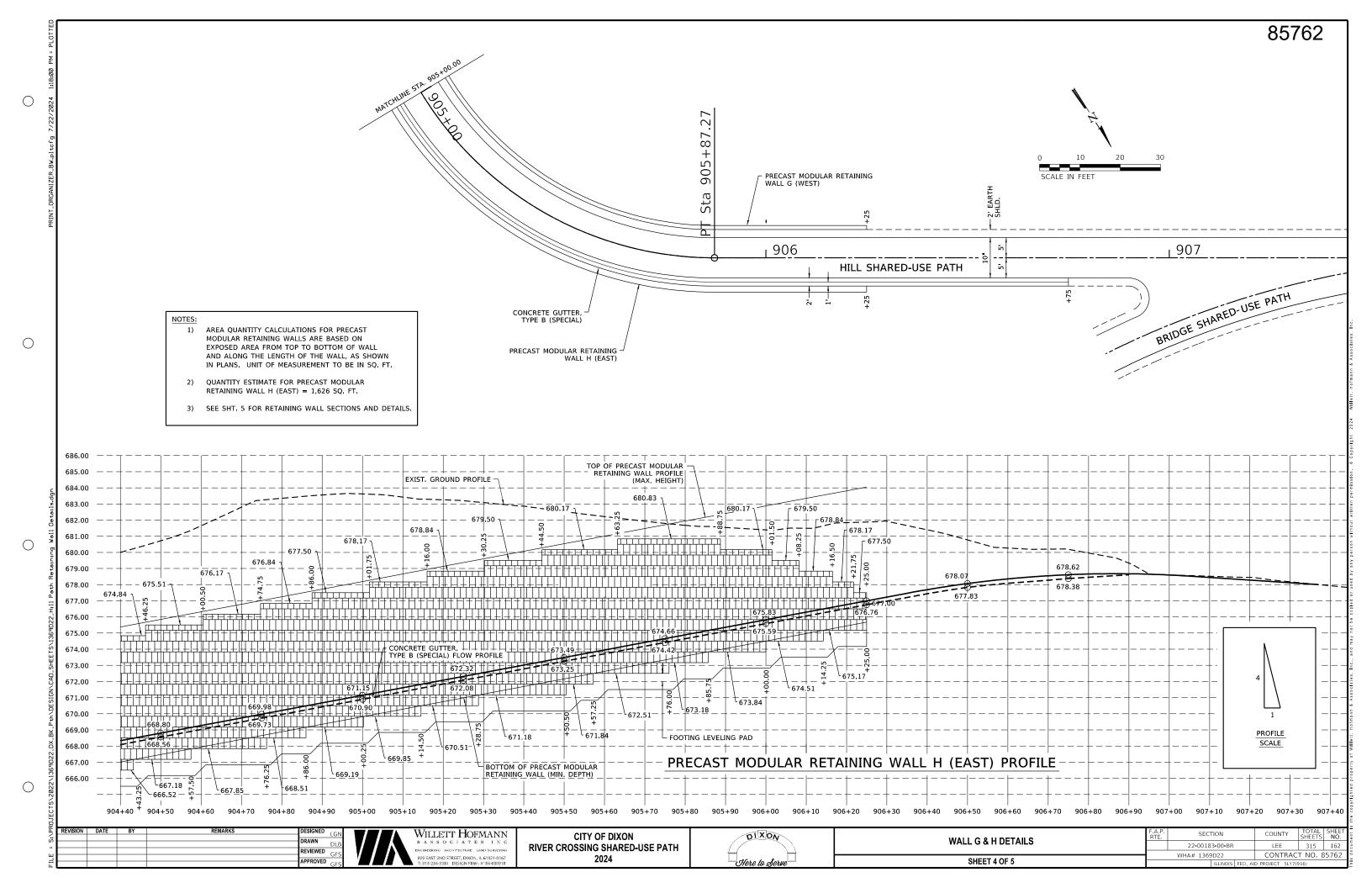


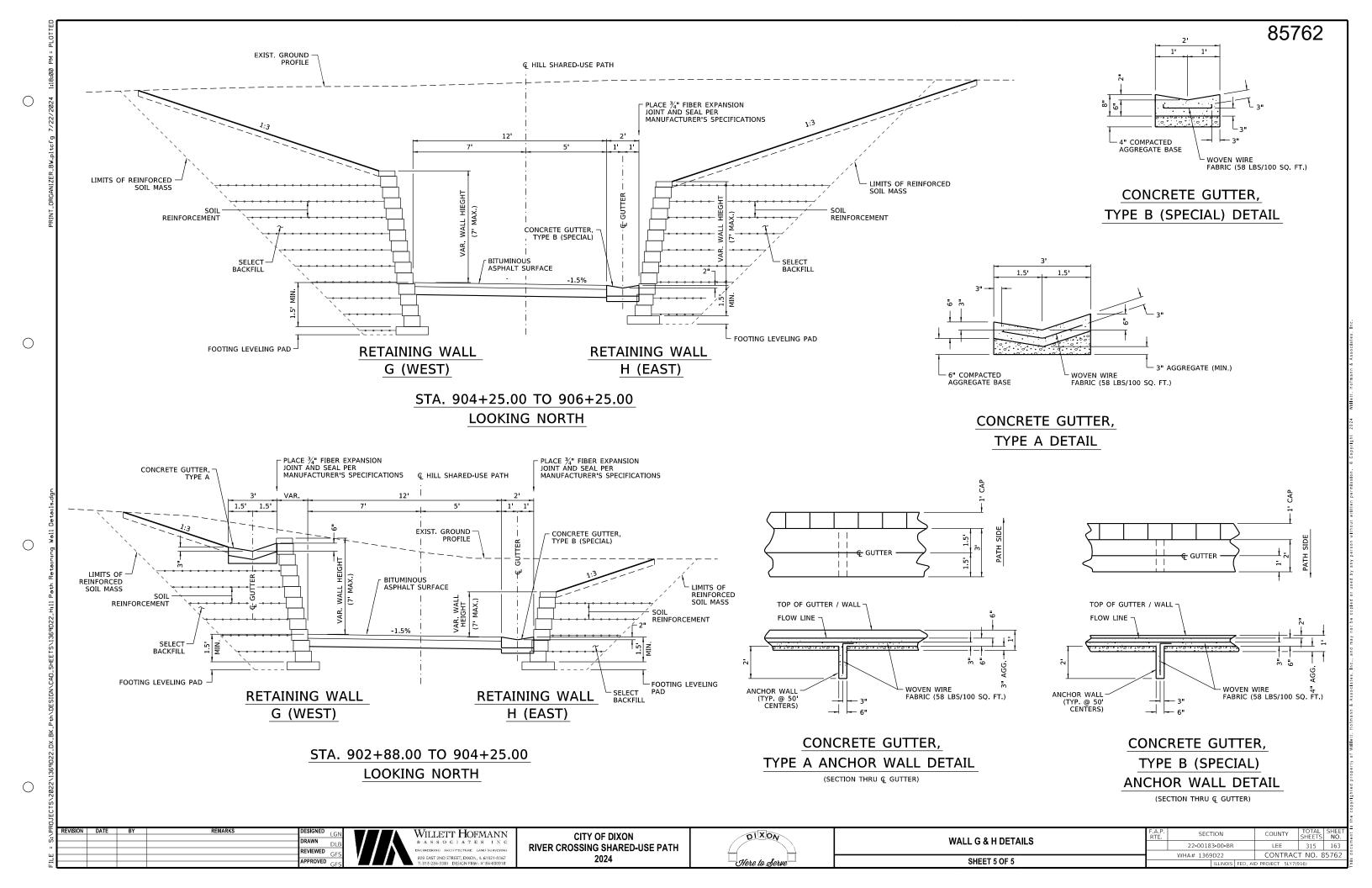


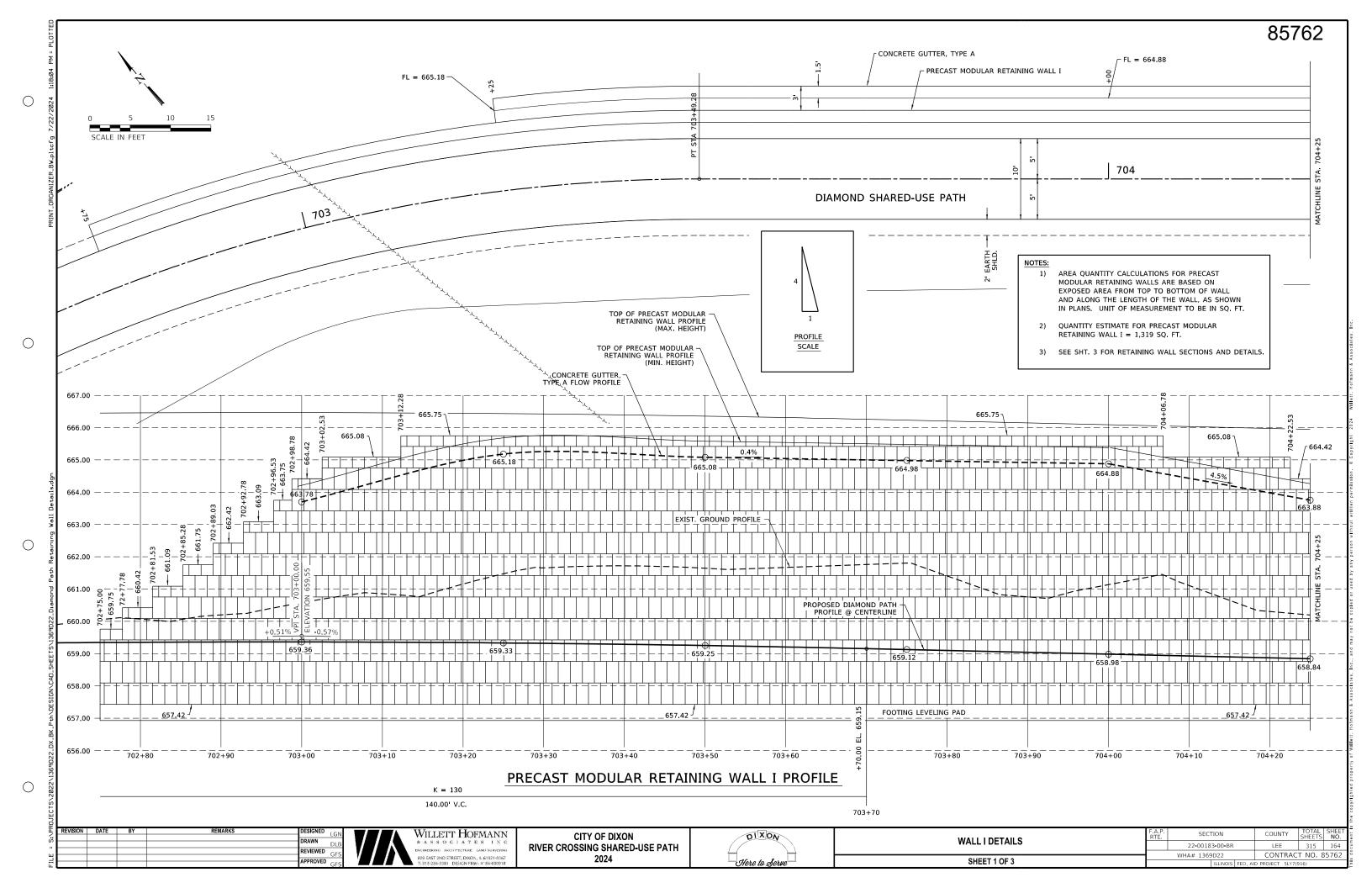


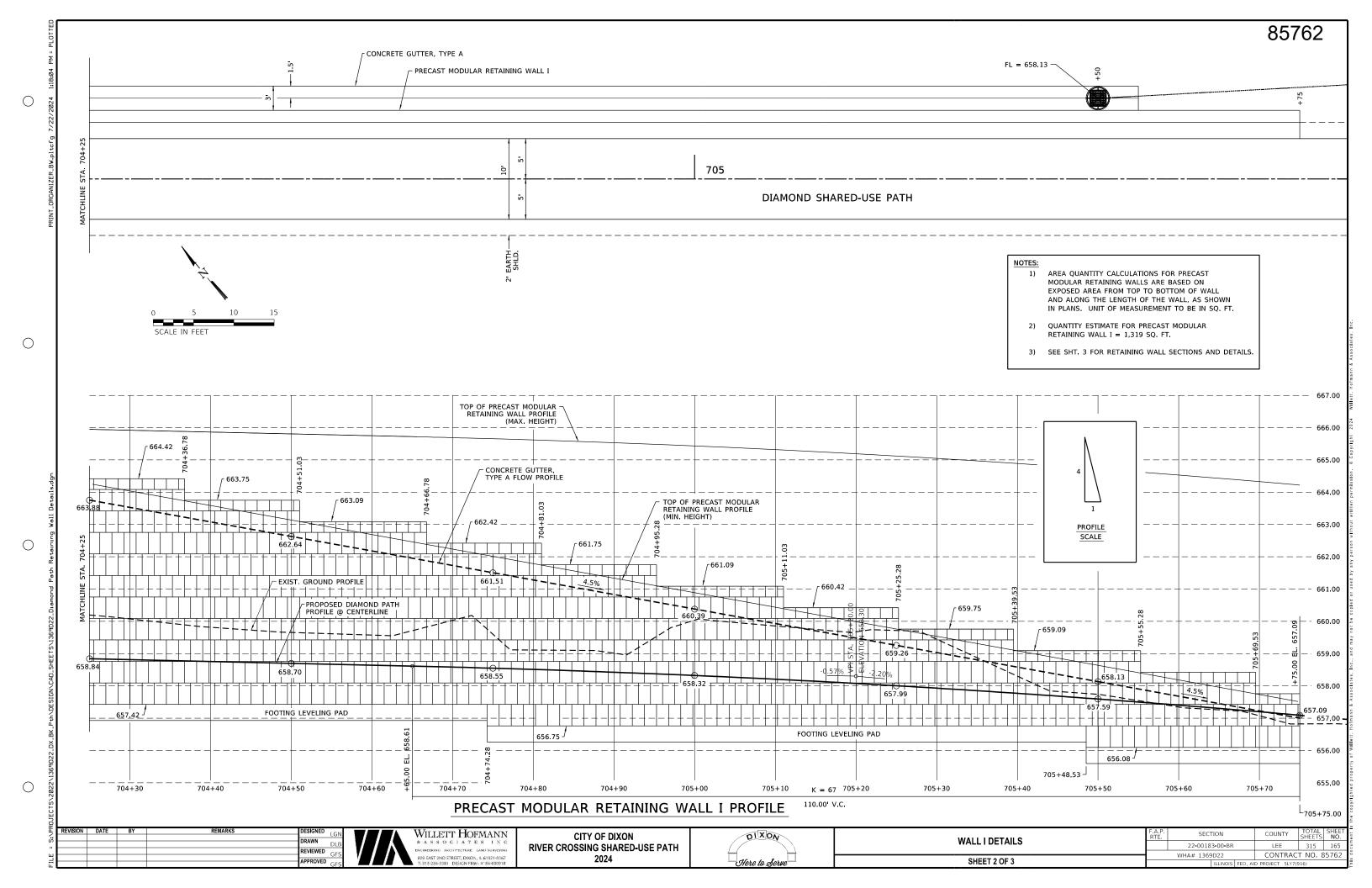


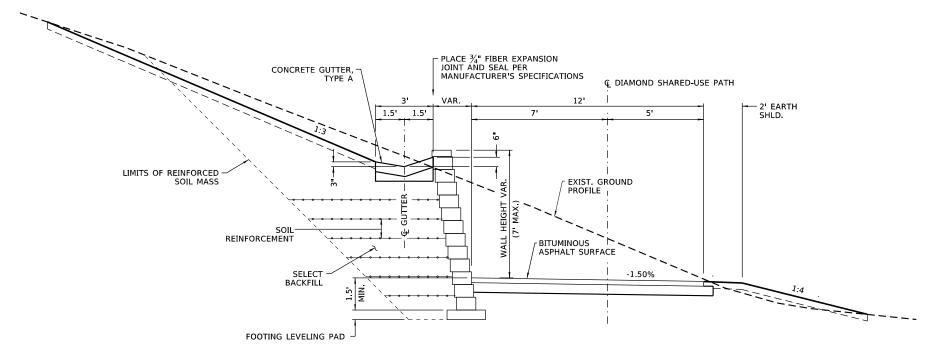








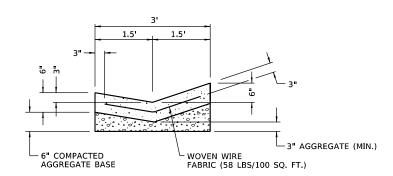




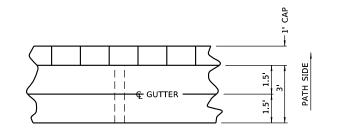
PRECAST MODULAR RETAINING WALL I W/ CONC. GUTTER

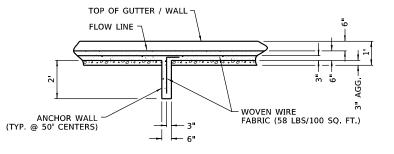
STA. 703+00.00 TO 705+75.00

LOOKING SOUTH



CONCRETE GUTTER, TYPE A DETAIL





CONCRETE GUTTER, TYPE A ANCHOR WALL DETAIL

(SECTION THRU Q GUTTER)

REVISION	DATE	BY	REMARKS	DESIGNED	LGN
					LGIV
				DRAWN	DLB
					DLD
				REVIEWED	GFS
				ADDDOVED	0, 5
				APPROVED	GFS

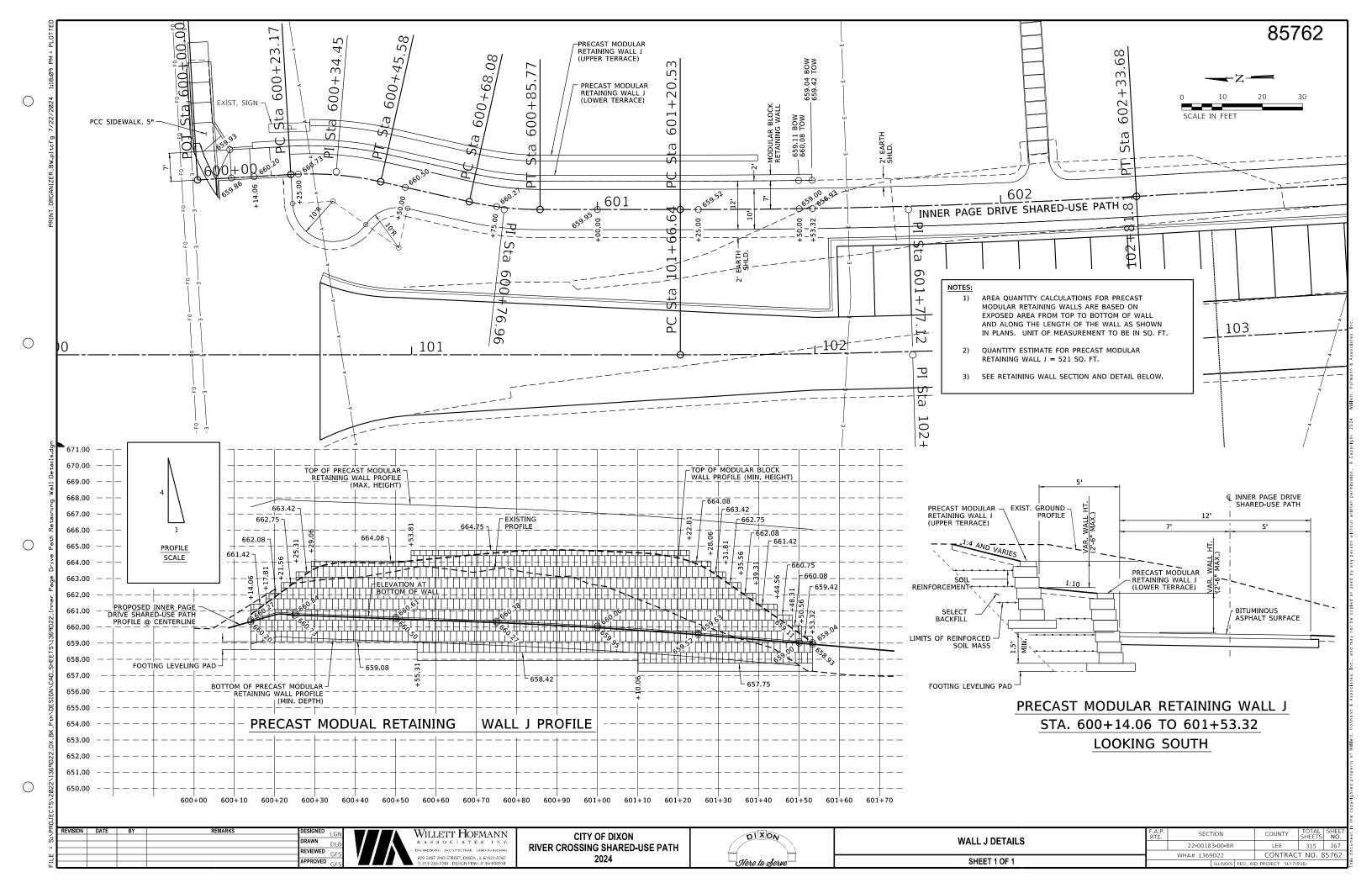
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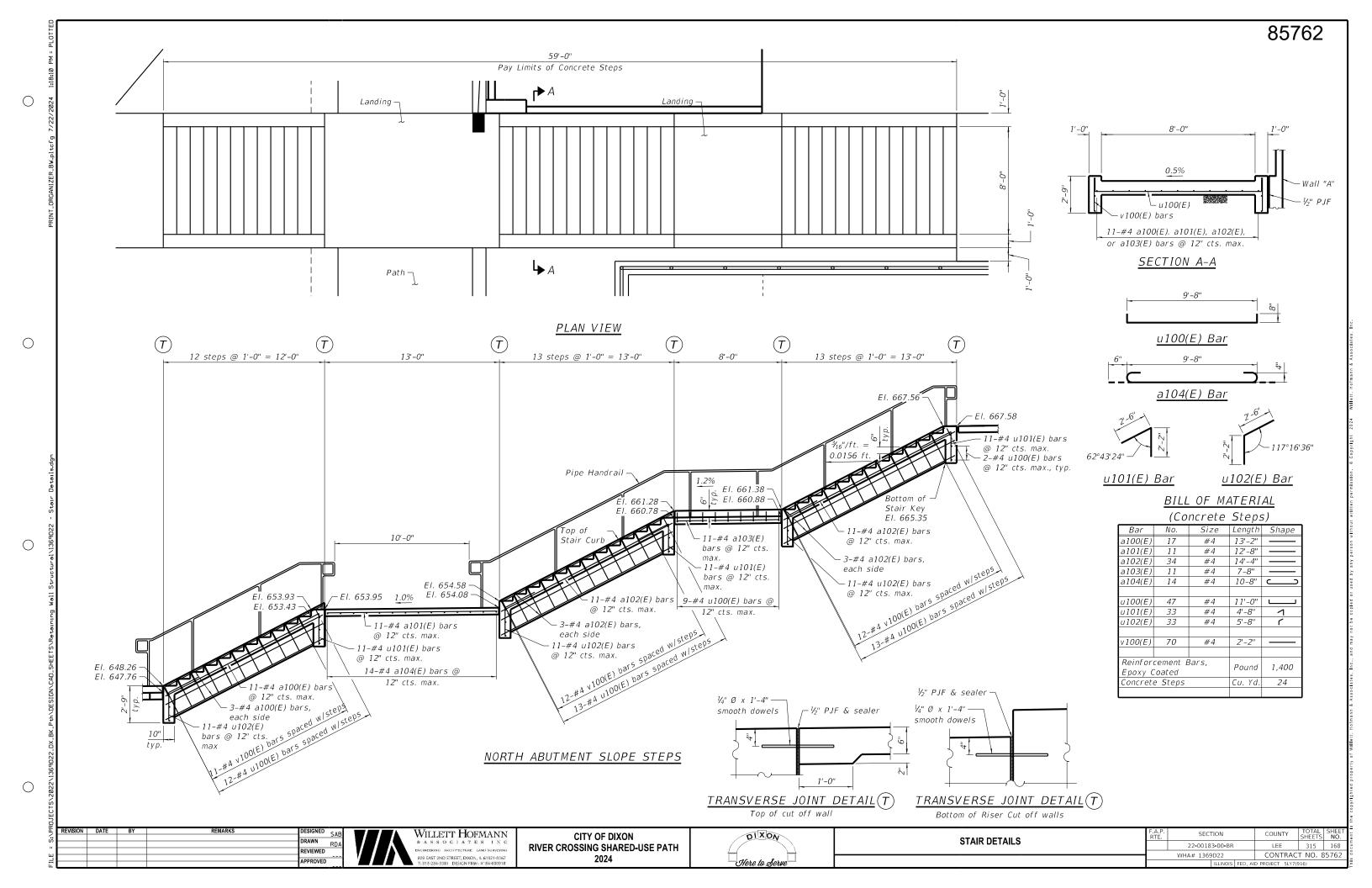






WALL I DETAILS		SECTION			COUNTY	TOTAL SHEETS	SHEE NO.
		22-00183-00-BR			LEE	315	166
0.00000		WHA# 1369	D22		CONTRAC	T NO. 8	35762
SHEET 3 OF 3			ILLINOIS	FED, AI	D PROJECT 5LY7(916)	





HANDRAIL FOR CONCRETE STEPS

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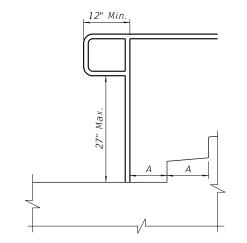
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Top of Concrete

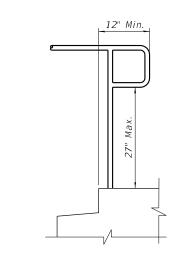
Top of

two Hex Nuts

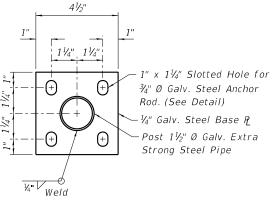
<u>ANCHOR ROD DETAIL</u> (Included in the cost of Pipe Handrail)



EXTENSION AT BOTTOM OF RUN DETAIL

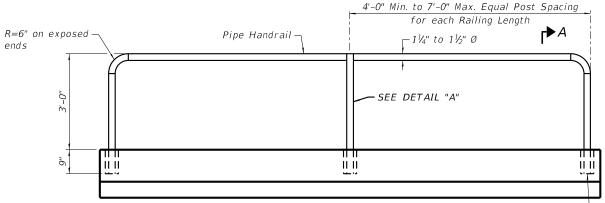


EXTENSION AT TOP OF RUN DETAIL



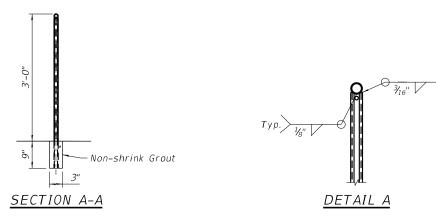
POST BASE PLATE DETAIL

(Included in the cost of Hand or Safety Rail)



<u>PIPE HANDRAIL - FOR CURB</u>

3" Ø Hole Fill Annular Ring with Non-Shrink Grout then Sealer for top ½"



PIPE HANDRAIL NOTES:

Gripping surfaces shall be uninterrupted by newel posts, other construction elements, or obstructions.

Ends of handrail shall be either rounded or returned smoothly to floor, wall, or post.

Hand & safety rails shall not rotate within their fittings.

Handrail shall conform to Section 509 with the exception that all pipe and connections shall be welded galvanized or aluminum according to Article 1006.30 or 1006.34.

The \emptyset of the gripping surface of the handrail shall be $1\frac{1}{4}$ " to $1\frac{1}{4}$ ".

Drilling of blocks will be necessary for reinforcement placement.

This work shall consist of furnishing and erecting Handrails as listed above and according to this detail. This work shall be paid for at the contract unit price per Foot for Pipe Handrail.

Stairways shall have continuous handrails both sides of all stairs.

The clear space between handrails and any wall shall be 11/2"

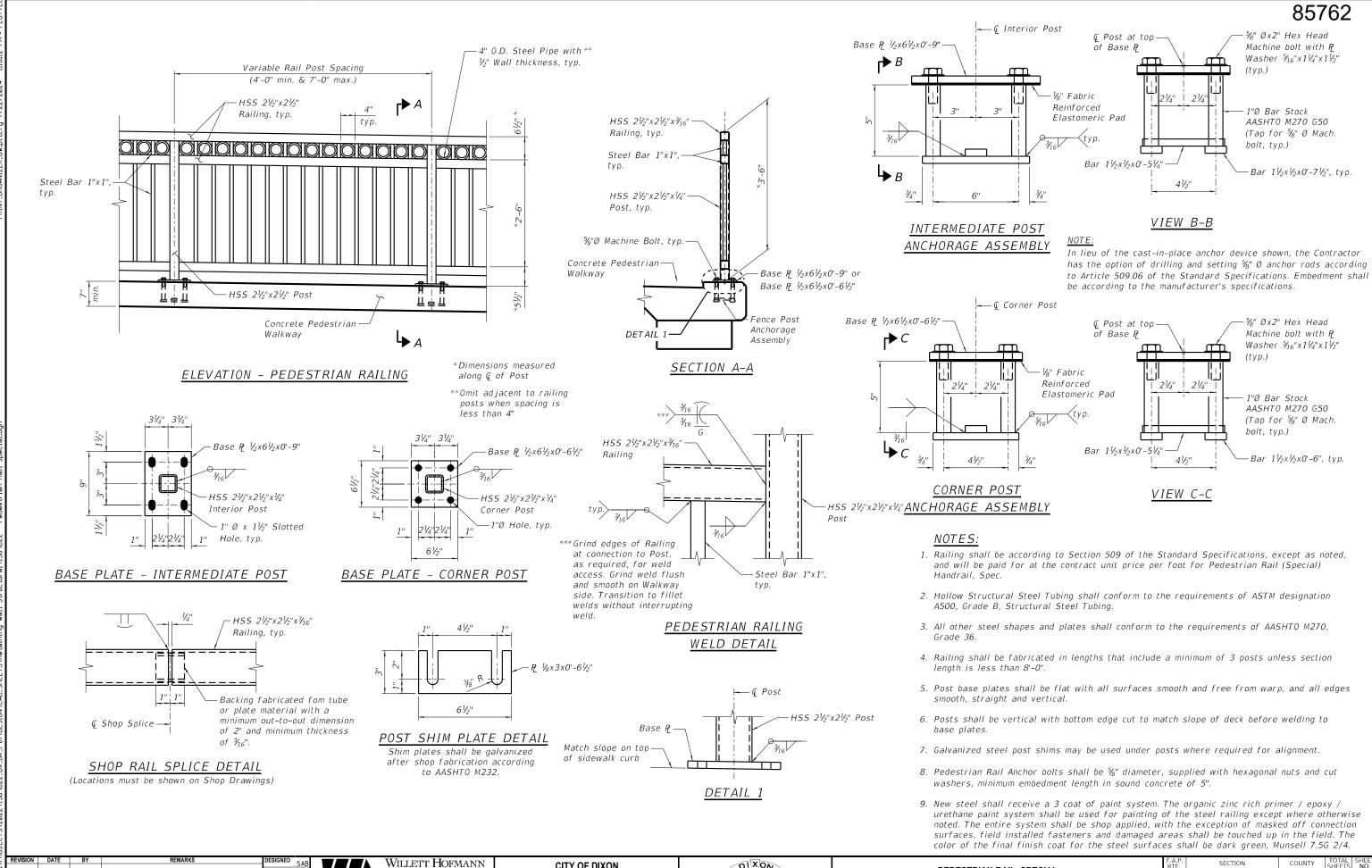
New steel shall receive a 3 coat of paint system. The organic zinc rich primer / epoxy / urethane paint system shall be used for painting of the steel railing except where otherwise noted. The entire system shall be shop applied, with the exception of masked off connection surfaces, field installed fasteners and damaged areas shall be touched up in the field. The color of the final finish coat for the steel surfaces shall be dark green, Munsell 7.5G 2/4.

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	REVISION	DATE	BY	REMARKS	DESIGNED	SAB
ı					DD 414/41	JAD
Г					DRAWN	RDA
					REVIEWED	
L					APPROVED	
L					AFFROVED	





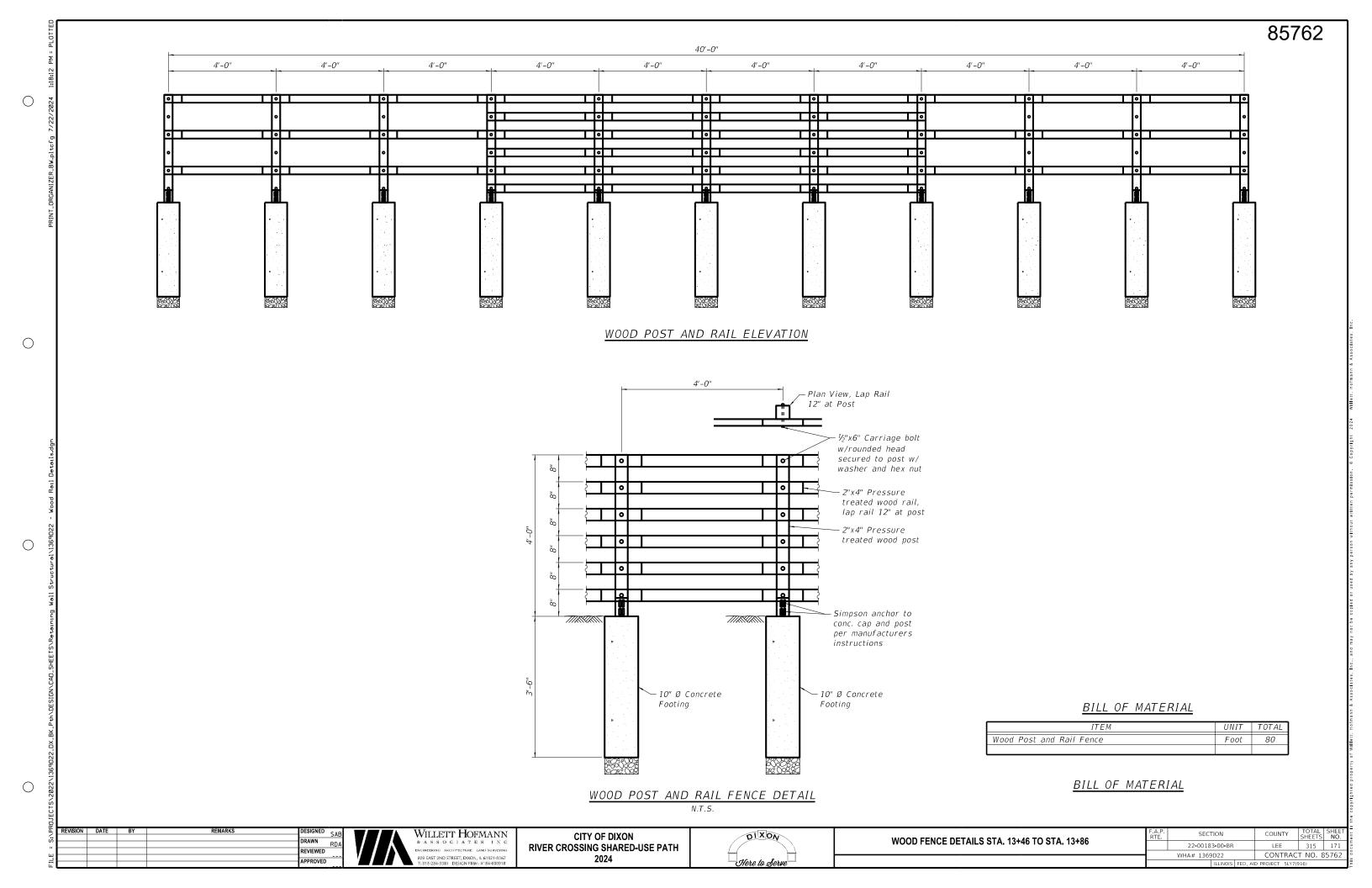
PIPE HANDRAIL DETAILS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	ent
PIPE HANDRAIL DETAILS		22 - 00183 - 00-BR	LEE	315	169	m no
	WHA# 1369D22 CONTRACT N		T NO. 8	IO. 85762		
		ILLINOIS FED A	D PROJECT 5LY70	916)		Ē



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WILLETT HOFMANN
A & S & O ESIGNED SAB
ORAWN RDA
COUNTY SHORT SHOULD SHOU



GENERAL NOTES:

- 1. ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST CODES. STANDARDS, AND THE IDOT STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION AND SUPPLEMENT SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS LATEST EDITION.
- 2. THE CONTRACTOR MUST VERIFY ALL OF THE INFORMATION SHOWN ON THE CONTRACT PLANS WHICH COULD AFFECT HIS WORK UNDER THIS CONTRACT FOR OPERATION OF THE EXISTING ROADWAY LIGHTING SYSTEM.
- 3. NO MATERIAL OR EQUIPMENT SHALL BE DELIVERED TO THE JOB SITE WITHOUT PRIOR INSPECTION AND APPROVAL BY THE ENGINEER. ANY MATERIAL AND EQUIPMENT NOT APPROVED BY THE ENGINEER MUST BE REMOVED FROM JOB SITE AT THE CONTRACTOR'S
- 4. ALL UNDERGROUND UNIT DUCT SHALL BE 30 INCHES MINIMUM BELOW GRADE PER IDOT SECTION 810. UNIT DUCT MUST BE POSITIONED IN THE FIELD TO AVOID CONFLICT WITH UNDER DRAINS, AND UNDERGROUND UTILITIES.
- 5. ALL SPLICING MUST BE IN POLE BASES WITH WATERPROOF SEALANT AND HEAT SHRINKABLE PLASTIC CAPS. UNLESS NOTED OTHERWISE.
- 6. NO LIGHTING CIRCUIT OR PORTION THEREOF SHALL BE REMOVED FROM NIGHT TIME OPERATION OF EXISTING LIGHTING WITHOUT THE APPROVAL OF THE ENGINEER. ALL EXISTING LIGHTING SHALL OPERATE FROM DUSK TO DAWN DAILY FOR DURATION OF THE PROJECT TO MAINTAIN ILLUMINATION OF TRAVELED ROADWAYS.
- 7. THE CONTRACTOR IS ADVISED THAT IN THE EVENT OF SNOW, HE SHALL BE RESPONSIBLE FOR THE IMMEDIATE REMOVAL OF ANY MAINTENANCE OF TRAFFIC PROTECTIVE DEVICE REQUIRED FOR THE TRAFFIC OPERATIONS THAT WOULD INTERFERE WITH SNOW REMOVAL OPERATION PREFORMED BY THE STATE IN ACCORDANCE WITH THE IDOT STANDARD SPECIFICATIONS.
- 8. REMOVAL AND DISPOSAL OF SURPLUS, UNSTABLE, UNSUITABLE, AND ORGANIC MATERIALS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 202 OF THE STANDARD
- 9. CONTRACTOR SHALL INSTALL FLEXIBLE CONDUIT AS PER DETAIL BE-902, WHEN TRANSISTION BETWEEN EXPANSION JOINT

SUMMARY OF QUANTITIES

PAY ITEM NO.	DESCRIPTION	UNIT	QTY
80400100	400100 ELECTRIC SERVICE INSTALLATION		1
81028200	UNDERGROUND CONDUIT, GALVANIZED STEEL, 2" CONDUIT	FOOT	50
81028730	UNDERGROUND CONDUIT, COILABLE NONMETTALIC CONDUIT, 1 1/4" DIA.	FOOT	13,410
81028750	UNDERGROUND CONDUIT, COILABLE NONMETTALIC CONDUIT, 2" DIA.	FOOT	350
81100100	CONDUIT ATTACHED TO STRUCTURE, 1/2" DIA., GALVANIZED STEEL	FOOT	830
81100300	CONDUIT ATTACHED TO STRUCTURE, 1" DIA., GALVANIZED STEEL	FOOT	40
81100600	CONDUIT ATTACHED TO STRUCTURE, 2" DIA., GALVANIZED STEEL	FOOT	1,120
81200200	CONDUIT EMEBEDDED IN STRUCTURE 3/4" DIA., PVC	FOOT	300
81300220	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4"	EACH	18
81300530	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 12" X 10" X 6"	EACH	24
81300830	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 18" X 18" X 8"	EACH	1
81400100	HANDHOLE	EACH	1
81702110	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1-1/C NO. 10	FOOT	16,530
81702120	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1-1/C NO. 8	FOOT	40,830
81702130	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1-1/C NO. 6	FOOT	44,670
81702190	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1-1/C NO. 4/0	FOOT	250
82500370	LIGHTING CONTROLLER, BASE MOUNTED, 240VOLT, 200 AMP	EACH	1
X1400210	LIGHT POLE, SPECIAL, 12'	EACH	100
X81212034	LUMINAIRE, TYPE D (SPECIAL)	EACH	28
X8212031	LUMINAIRE, TYPE A (SPECIAL)	EACH	72
X8212032	LUMINAIRE, TYPE B (SPECIAL)	EACH	5
X8212033	LUMINAIRE, TYPE C (SPECIAL)	EACH	90
X8360120	LIGHT POLE FOUNDATION SPECIAL	EACH	86

CONTROLLER NOTES:

- 1. CONTROL CABINET SHALL BE U.L. LISTED "INDUSTRIAL CONTROL PANEL" PER U.L. 508.
- 2. CONSTRUCTION SHALL BE NEMA 4X.
- VOLTAGE RATINGS OF SERVICE EQUIPMENT SHALL CONFORM TO THE SERVICE VOLTAGES INDICATED ON THE PLANS. 120/240V, 10, 3W.
- SERVICE EQUIPMENT ENCLOSURE AND METERING EQUIPMENT SHALL MEET THE REQUIREMENTS OF THE SERVING UTILITY.
- 5. SERVICE EQUIPMENT SHALL BE FACTORY WIRED AND CONFORM TO NEMA STANDARDS.
- THE EXTERIOR DOOR SHALL HAVE PROVISIONS FOR PADLOCKING. THE PADLOCK HOLE SHALL BE MINIMUM DIAMETER OF 11MM.
- 7. ALL TERMINALS FOR INCOMING SERVICE CONDUCTORS SHALL BE COMPATIBLE WITH EITHER COPPER SIZED TO SUIT THE CONDUCTORS SHOWN ON THE PLAN. TERMINALS LUGS SHALL BE COPPER SOLID NEUTRAL TERMINAL STRIP SHALL BE RATED 200A UNLESS OTHERWISE SPECIFIED AND FOR USE WITH COPPER OR ALUMINUM CONDUCTORS. THE TERMINAL SHOULD INCLUDE BUT NOT BE LIMITED TO:
 - A) INCOMING TERMINALS (LANDING LUGS)
 - B) NEUTRAL LUGS
 - C) SOLID NEUTRAL TERMINAL STRIP
 - D) TERMINAL STRIPS FOR CONDUCTORS WITHIN THE ENCLOSURE
- 8. AT LEAST 6 STANDARD SINGLE POLE CIRCUIT BREAKER SPACES (20MM NOMINAL) SHALL BE PROVIDED FOR BRANCH CIRCUITS. INTERIORS SHALL ACCEPT BOLT- ON OR CABLE-IN/CABLE- OUT CIRCUIT BREAKERS.
- BOLT- ON CIRCUIT BREAKERS MAY BE MOUNTED IN THE VERTICAL OR HORIZONTAL POSITION, CABLE- IN/ CABLE- OUT CIRCUIT BREAKERS SHALL BE MOUNTED IN VERTICAL
- 10. FASTENERS ON THE EXTERIOR OF THE ENCLOSURE SHALL BE VANDAL RESISTANT AND SHALL NOT BE REMOVABLE FROM THE EXTERIOR. ALL NUTS, BOLTS, SCREWS, WASHERS, AND HINGES SHALL BE STAINLESS STEEL.
- 11. PHENOLIC NAME PLATES SHALL BE PROVIDED AS REQUIRED.
- 12. A PLASTIC COVERED WIRING DIAGRAM SHALL BE ATTACHED TO THE INSIDE OF THE
- 13. FOUNDATION SHALL EXTEND 3" MINIMUM BEYOND EDGE OF ENCLOSURE.
- 14. FIRST TOP 6 CIRCUIT, OF PANEL SHALL BE NON- SWITCHED. BOTTOM SECOND 12 CIRCUIT, SHALL BE CONTACTOR CONTROL WITH PHOTOCELL, AND UNIVERSAL TIMER.

CONTACT NOTES:

CALL JULIE OR DIGGER FOR CABLE LOCATES, CALL IDOT EMC AT (708) 524-2145 FOR IDOT MAINTAINED CABLE LOCATES.

SYMBOL LEGEND:

85762

PROPOSED LIGHTING CONTROLLER

▣ TYPE A PROPOSED LIGHTING UNIT, SPECIAL

TYPE B PROPOSED CANOPY LIGHT FOR PAVILION.

TYPE C PROPOSED LIGHT, MARINE GRADE FOR PIER LIGHTING.

PROPOSED JUNCTION BOX SHALL BE STAINLESS STEEL, ATTACHED TO 0 STRUCTURE, SIZE AS NOTED ON PLANS

PROPOSED HANDHOLE

TYPE D PROPOSED LIGHTING UNIT, SPECIAL

PROPOSED UNIT DUCT UNIT DUCT, 600V, 3-1C NO.1, 1/C NO.8 GROUND, (XLP-TYPE USE), 2" DIA. POLYETHYLENE

PROPOSED ELECTRIC CABLE IN CONDUIT, 2" CNC

ABBREVIATIONS:

EXISTING TO REMAIN EDGE OF PAVEMENT E.O.P.

ELECTRICAL MAINTENANCE CONTRACT EMC

FEET OR FOOT GND GROUND JUNCTION BOX JB MAST ARM NUMBER NOT TO SCALE PROPOSED POLYVINYL CHLORIDE RGS RIGID GALVANISED STEEL

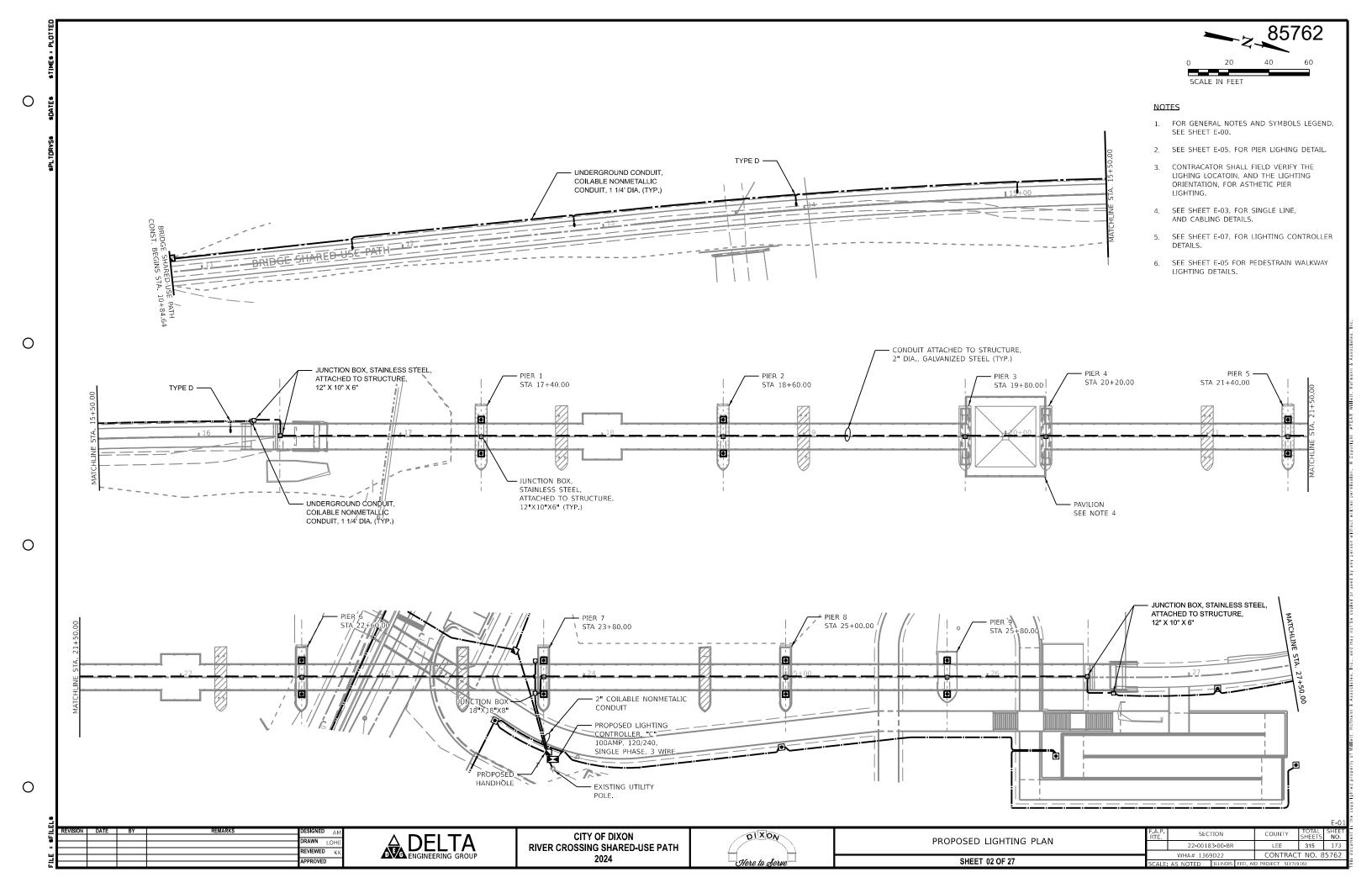
REMOVE $ST\Delta$ STATION

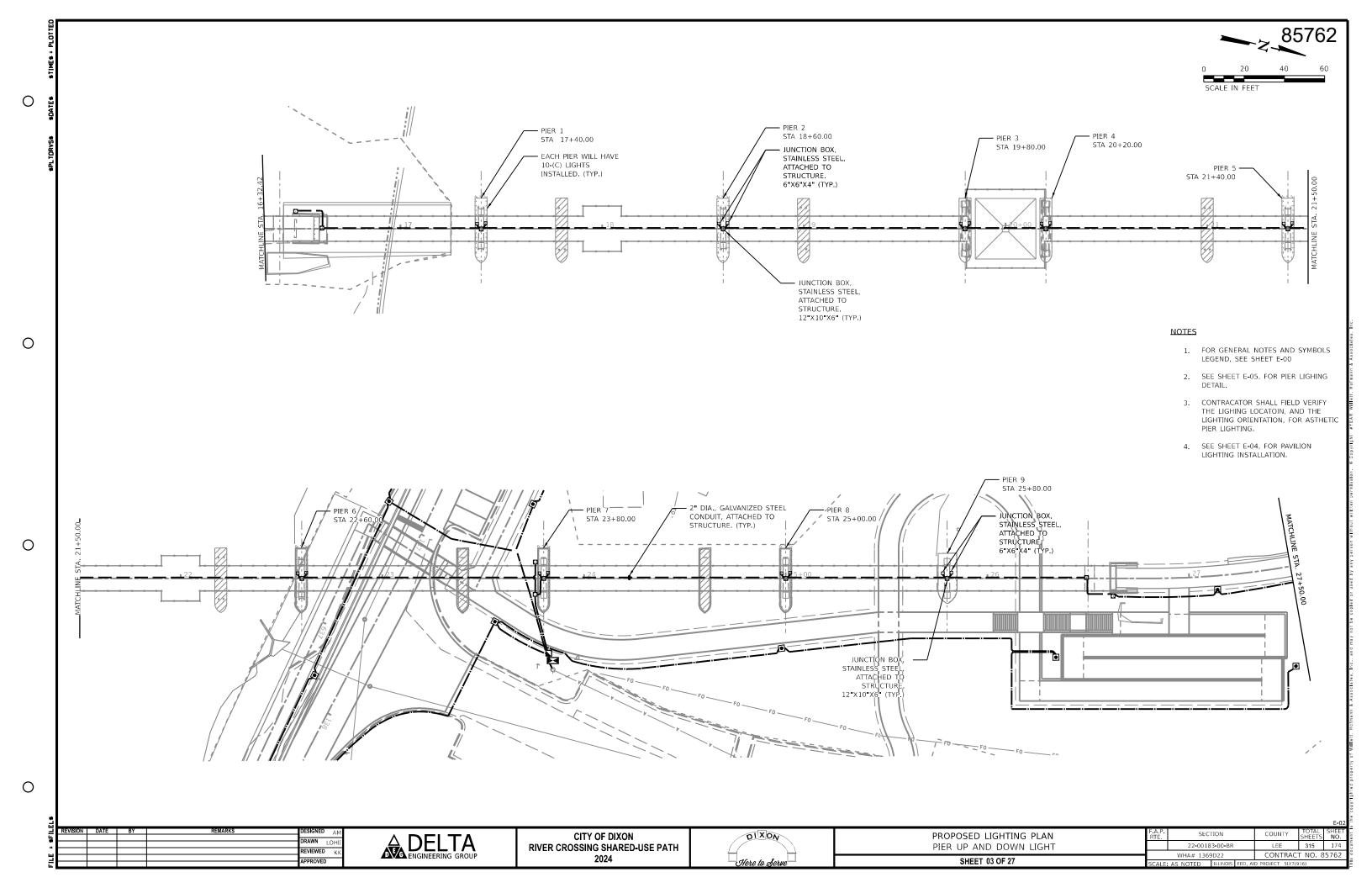
UNLESS NOTED OTHERWISE U.N.O. HPS HIGH PRESSURE SODIUM

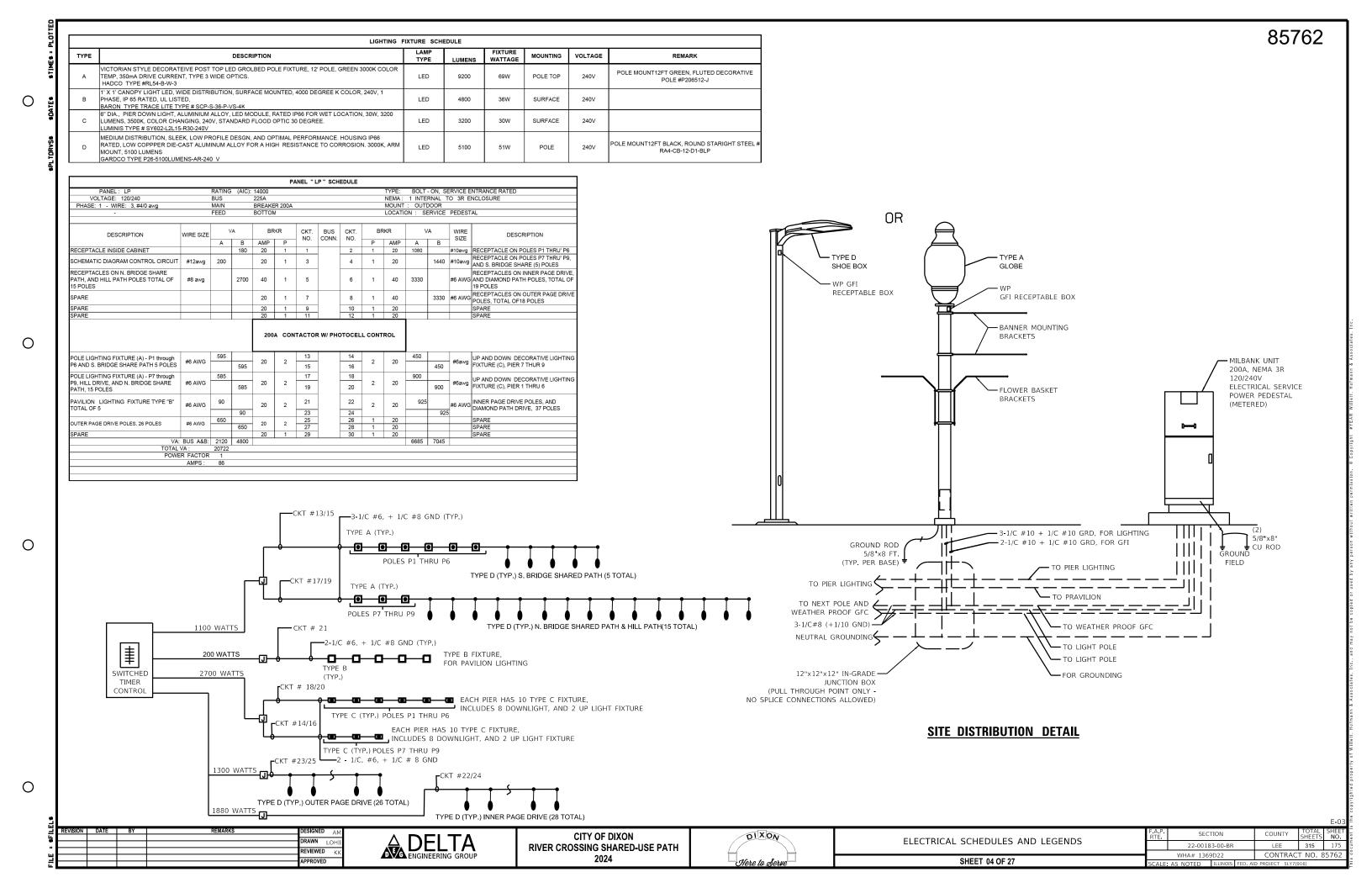


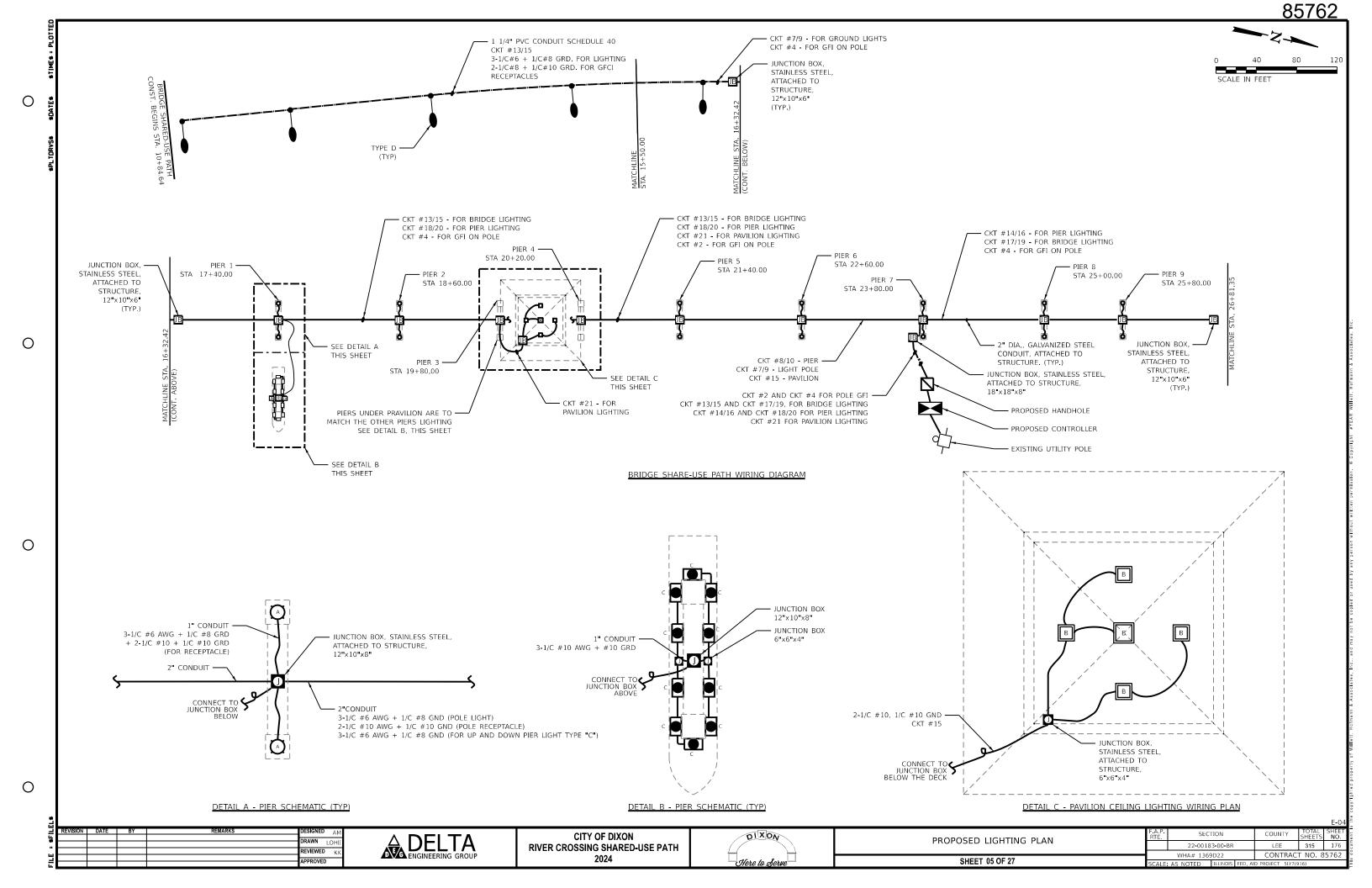


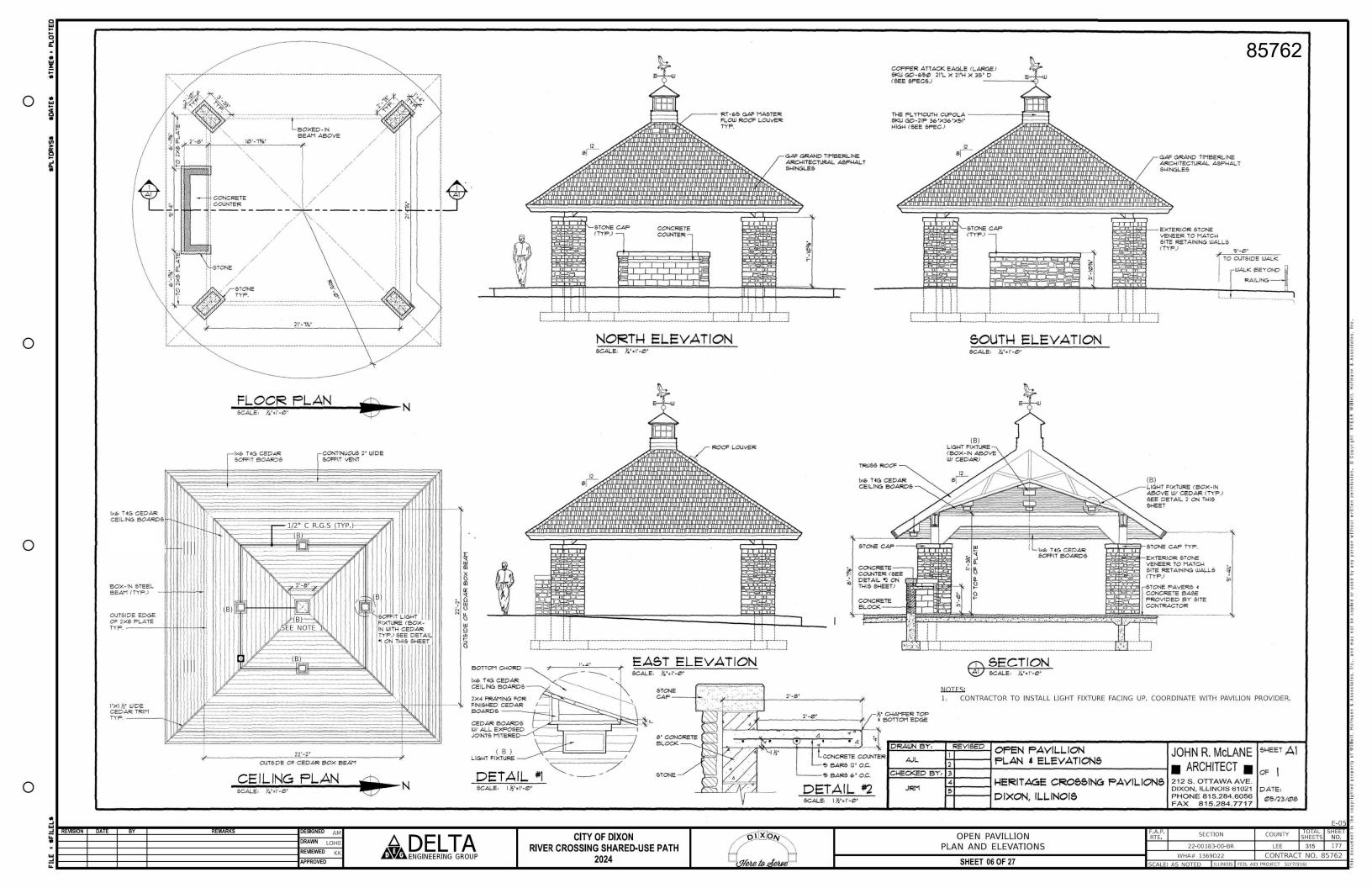
DIXON

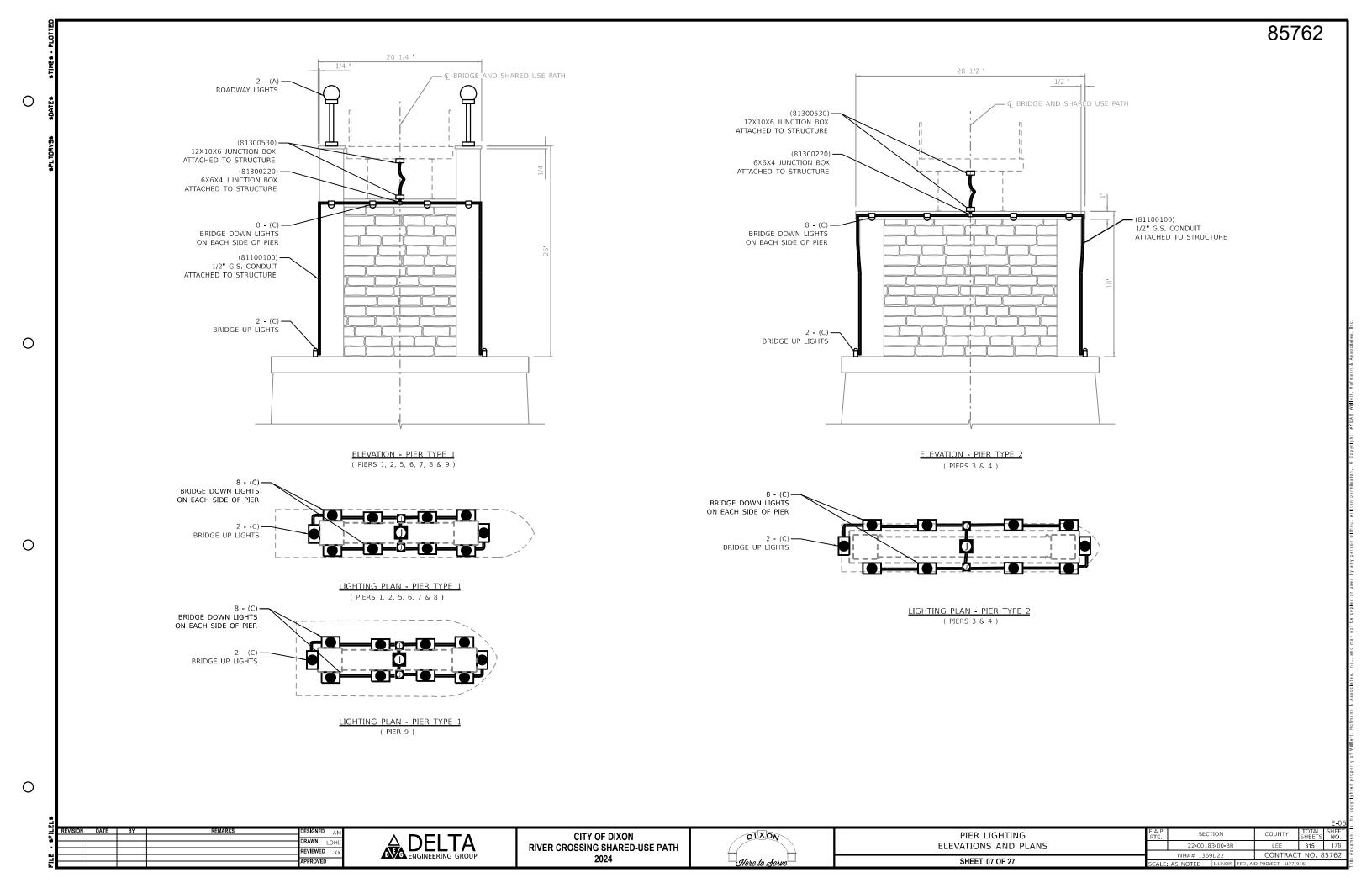




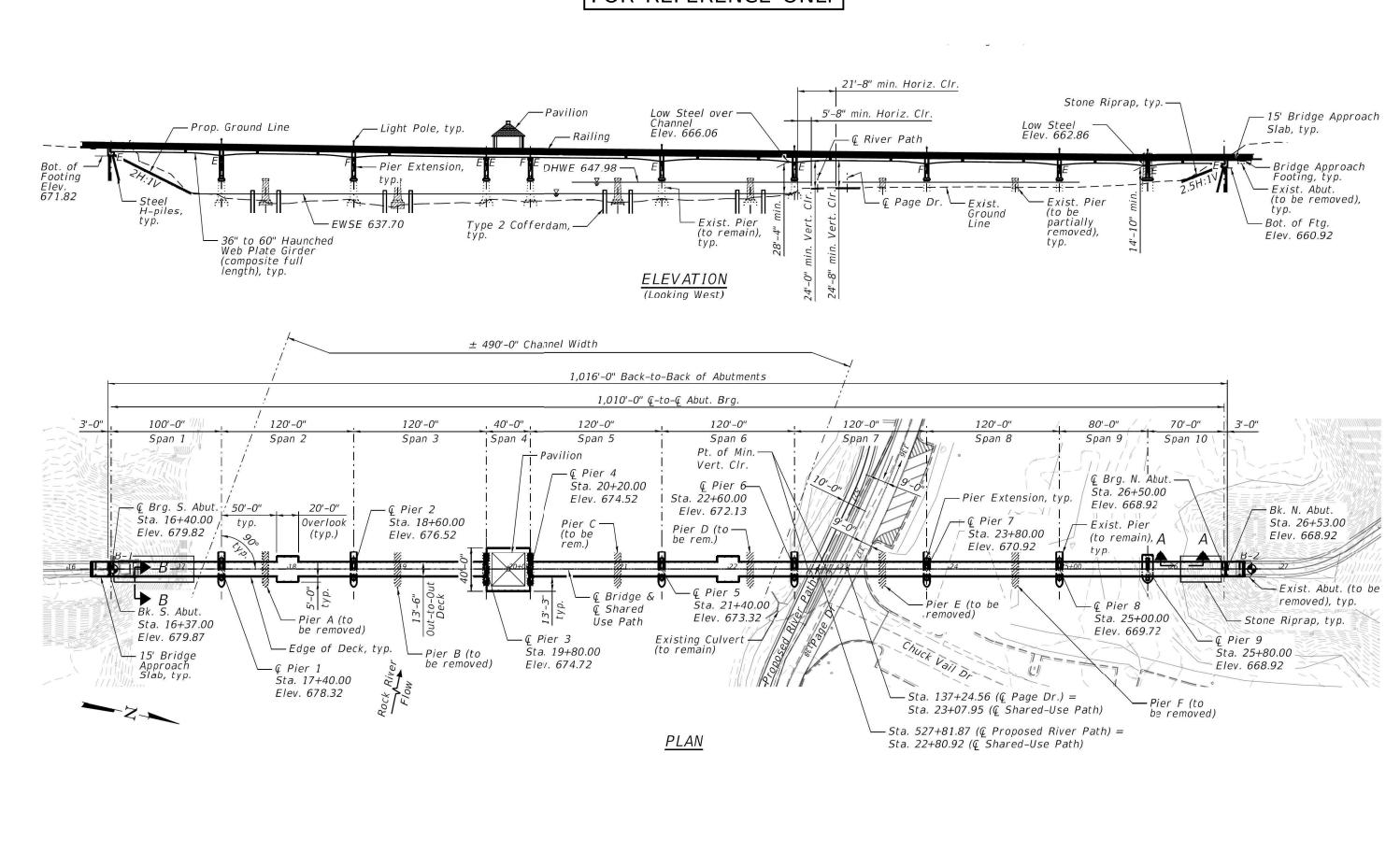








FOR REFERENCE ONLY



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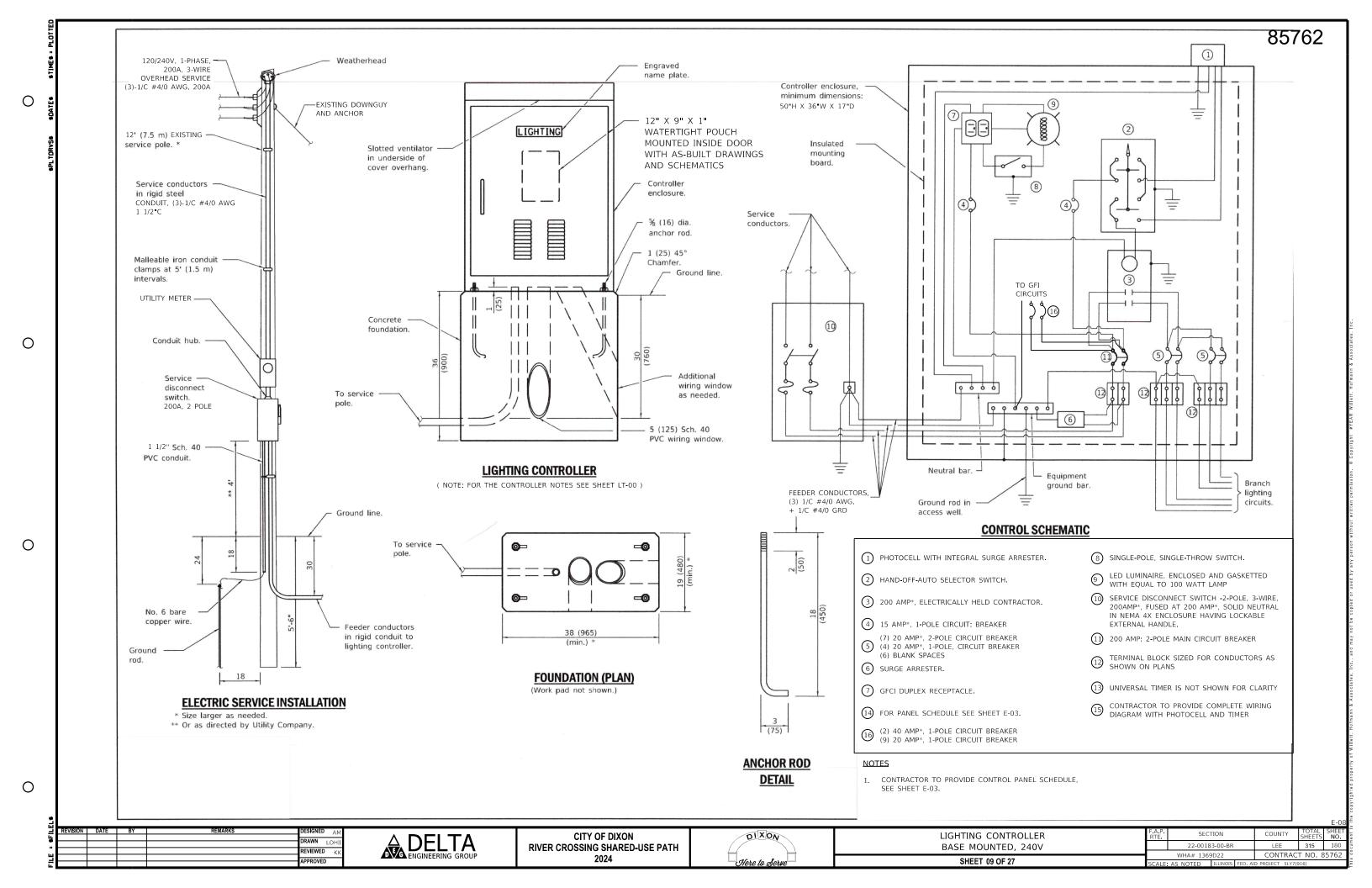
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DELTA ENGINEERING GROUP

CITY OF DIXON RIVER CROSSING SHARED-USE PATH 2024



SITE PLAN AND ELEVATION REFERENCE ONLY SHEET 08 OF 27



Urban

Refractive globe with Lumiloc LED engine GX4



Whether you are looking to beautify or add a sense of security and well-being to your outdoor space, the highly configurable Hadco LED refractive post tops paired with the latest LumiLock light engine GX4 will definitely help you achieve your goals. A multitude of exterior luminaire styles allow you to create promenades and areas exuding timeless, historical charm both day and night. The configurable LED light engine GX4 is an ideal alternative to HID sources, providing you with significant energy savings, and more choices for light levels, optics and controls. Includes Service Tag, Hadco's innovative way to provide assistance throughout the life of the product.

Qty

Ordering guide

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Example: RL34 A A B A 1 A S N R7 W A 3 N N N N SP1

Series RL54	Pod B	Roof A	Cage N	Finial N	Fastener	Finish J	Optic S	Pod Photo Control (location inside of pod) N
RL34 Wide Body Type 3 RL54 Wide Body Type 5	A Octagonal style B Round fitter with scalloped petals H Round contemporary L Round fluted long T Decorative leaf with scalloped petals	A Victorian B Acorn C Tall D Short G Adams	B¹ Cage for wide body globe E Band for wide body globe F Band for wide body globe G¹ Cage for wide body globe I¹ Case for wide body globe J¹ Cage for wide body globe J¹ Cage for wide body globe N None	A B C ² D ² E ² F G H N None	1 Hexhead 2 Allen head	A Black B White G Verde H Bronze J Green	S Short W Wide	E 120 VAC Button Eye H 208/240/277 Button Eye R³ 3 Pin Receptac N None

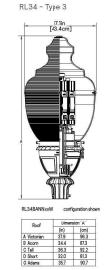
Anguar Anguar						T T	Ī	
Future Proof Photo Control N	Color Temp W	Voltage	Drive Current 3	Integral Control Options	Option 1	Option 2	Option 3	Surge Protection
RS ^{2,4} 5 pin receptacle on the engine R7 ^{2,4} 7 pin receptacle on the engine N None	W 3000K N 4000K	A 120- 277 VAC B 347- 480 VAC	2° 200mA 3 350mA 4 450mA 5° 530mA	Dynadinmer ** DA 4 Hrs 25% Reduction DB 4 Hrs 25% Reduction DC 4 Hrs 75% Reduction DC 5 Hrs 75% Reduction DC 5 Hrs 25% Reduction DC 6 Hrs 75% Reduction DF 6 Hrs 75% Reduction DF 6 Hrs 75% Reduction DF 8 Hrs 25% Reduction DH 8 Hrs 50% Reduction DH 8 Hrs 50% Reduction DJ 8 Hrs 75% None	AST * Adjustable start up time N None	CLO ⁶ Constant light output N None	OTL *Over the life N None	SPI 10kV/10kA Surge Protector SP2* 20kV/10kA Surge Protector

When SP2 option is selected, luminaire will be fitted with SP2 instead of SP1.
 Not available with B 347-480 voltage.
 Not available with R5 or R7.
 FAWS not available with CLO.

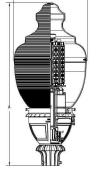
- Not available with A pod.
 Not available with B Roof.
 Not available with B Roof.
 Not available with B Roof.
 Use of photoelectric cell (pod photo control (R) only) or shorting cap is required to ensure proper illumination. When R, RS, R7 options are selected, product will ship with a horting cappl installed.
 Only available with A & B clear roof options. Not available with drive currents 4 or 5. R. 3 do r 64 with 5 option any available with A roof.
 Optional Dynadimner dimming schedules, DALI. AST, CLO, and OTL not evaluative with a 747-40 VAC.

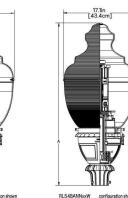
RL34-RL54_Spec www.hadco.com 03/23 page 1 of 8





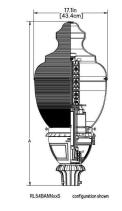
Dimensiors





RL54 - Type 5

RL34/RL54 Refractive Globe with Lumilock LED engine GX4



asions will vary when other pod, cage and brim options are specified.

Housing Cptions



H Round L Round Fluted
Contemporary Long



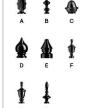












RL34-RL54_Spec www.hadco.com 03/23 page 4 of 8

TYPE A

HADCO

Poles & Brackets

by (Signify

Urban Decorative

Hadco P2000 Series decorative aluminum poles provide a wide range of options in a timeless aesthetic. All poles are made in the USA and always open to a wide range of add-ons and customizations to meet any project need.



P2065

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Ordering guide

Series P2065	Pole height	Finish	Accessory Location (factory installed)	Pole Accessories	Tenon Options
P2025 P2060 P2061 P2063 P2065 P2071	8 8ft ¹ 10 10ft 12 12ft 14 14ft 16 16ft 18 18ft 20 20ft ²	A Black B White G Verde H Bronze I Gray J Green Z Custom 3	N No Option T 12"Down from Top - Aligned with House Side B 4" lip from Top of Base - Aligned with House Side Z Custom	N No Option D Standard Duplex G GFI Duplex M Motion Control ³	N Standard Tenon - 3" OD x 3" ISF Internal Silp Fitter (for HFP Brackets) 4" OD Tenon or 4" pole w/o standard 3" tenon (consult factory for 3" OD poles)

Note: Top outlets not available with the HFP arms. Consult factory for HFP arm outlet mounting.

Footnotes:

1. Not available on P2065 models
2. Only available on P2071 models
3. Consult factory for quotation

Anchor Bolts & Templates (ordered separately)

12NC	Description	
912400110297	ANCHOR BOLT, 3/4-10x19x3, 4/PK	
912400128329	AB TEMPLATE, P2000 (excl. P2060)	
912400128328	AB TEMPLATE, P2060	

Banner Arm Bracket

example:	BA31A18B-A	

Pro	duct Code	Pole Dia.	# of Arms	Materials	Length	Finial	Finish
ВА		5"	2	Α		В	J
ВА	Banner Arm Bracket	3" 4" 5"	1 One 2 Two at 180°	MruimulA A	18" 24" 30"	B Ball	A Black B White G Verde H Bronze I Gray J Green Z Custom



Tie Down Bracket

example: TD32-H

Product Code	Pole Dia.	# of Arms	Finish	
TD	5" 2		J	
TD Tie Down Bracket	3" 4" 5"	1 One 2 Two at 180°	A Black B White G Verde H Bronze I Gray	



Flower Pot Bracket

example: FPB4212-B

Product Code	Pole Dia.	# of Arms	Length	Finish
FPB Flower Pot Bracket	5" 4" 5"	1 One 2 Two at 180°	12"	A Black B White
				G Verde H Bronze I Gray J Green Z Custom



P2065	P2065-10	5	Straight	Fluted, 12 Flat	10	0.188 - 0.267	3 x 3	73	8 to 12	16 x 21	5 x 8	3/4 x 19 x 3
P2065	P2065-12	5	Straight	Fluted, 12 Flat	12	0.188 - 0.267	3 x 3	82	8 to 12	16 x 21	5 x 8	3/4 x 19 x 3
							-					

REVIEWED K APPROVED



CITY OF DIXON RIVER CROSSING SHARED-USE PATH



LED POST TOP
DETAILS AND SCHEDULES

SHEET 10 OF 27

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F.A.P. RTE	SEC	ΠON	COUNTY	TOTAL SHEETS	SHE	
	22-0018	3-00-BR	LEE	315	18	
	WHA# 1369	D22	CONTRACT NO. 8576			
SCALE:	: AS NOTED	ILLINOIS	FED, AI	D PROJECT 5LY7(9	16)	

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DESCRIPTION

The SCP-S is a low-profile 12" surface mount square canopy with a variety of precision engineered optics for application flexibility. This canopy optimizes optical performance and long-life with superior thermal management in an attractive and durable die-formed aluminum enclosure with a premium PMMA lens that does not yellow over time. This product can be easily surface or pendant mounted and is the ideal energy saving solution for applications including, but not limited to, parking garages, schools, office complexes, light commercial development, apartments, walkways, entryways and stairwells.

CONSTRUCTION

- Precision die-formed aluminum enclosure and backplate with stainless steel hardware
- · White powder coat finish, custom colors available upon request
- IP65 rated light engine compartment
- Single 3/4" side knockout

OPTICS/LEDS

- · UV-stabilized polymethyl methacrylate (PMMA) optics that will not yellow
- Garage optics provides a type V short symmetric square distribution with light focused in the 60° to 80° zones to optimize spacing with even light distribution
- Performance optic provides a type VS (square) very short distribution and offers more light in the 30° to 60° zones, ideal for higher mounting heights
- · Low glare optic provides excellent Type VS (square) short distribution with exceptional glare control
- From 20W to 67W with up to 9016 lumens for maximum project flexibility
- Efficacies up to 134 LPW maximize energy savings and utility rebates
- 4000K CCT and 5000K CCT
- L70 of 190,000 hours
- CRI ≥71

ELECTRICAL

- 120-277VAC, 50/60Hz
- 0-10V Dimming driver

INSTALLATION

- Fixture enclosure is attached to backplate by four white fasteners
- · Backplate easily attaches to a recessed 3" or 4" J-box
- Pendant mounted using standard 1/2" downrod and hardware (supplied by others)

OPTION

• Integral battery backup (BB) offers over 900 lumens and 90 minutes of runtime for path of egress. Rated for ambient temperatures between 0°C to 40°C (32°F to 104°F). Battery backup option available on the SCP-S-20-P-VS-4K-WH only.

TESTING & COMPLIANCE

- cETLus Listed to UL1598 for Wet Locations for covered canopy applications
- Operating temperatures: -40°C to 40°C (-40°F to 104°F)

• Five year warranty (terms and conditions apply)

SCP-S Series

Surface Mount Performance LED Canopy



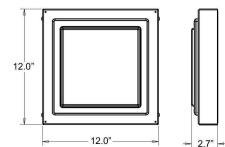




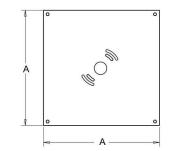


Specs At A Glance* Wattage (W) 20 36 50 67 Lumens (Im) 2670 4815 6586 8940 130 133 Efficacy (LPW) 133 131 70W 100W 150W 250W Equivalency (HID) Garage (G) - Type VS Square Short Low Glare (LG) - Type VS Square Short Distribution Performance (P) - Type VS Square Very Short CCT 4000K, CRI 120-277VAC, 50/60Hz, 0-10V Dimmable Input Voltage -40°C to 40°C (-40°F to 104°F) Operating Temp cETLus Listed, Wet Locations Certifications Covered Canopy Warranty 5 Years 7.0 lbs Weight

DIMENSIONS



TRANSITION PLATE DETAILS



Accessory	Α
MC-TR16	16"
MC-TR24	24"

85762

TYPE B

10'

20'

30'

30'

SAMPLE PHOTOMETRICS

SCP-S-36-P-VS-4K

30'

20'

IES: Type VS Square Very Short

10'

0

10'

20'

IORIZONTAL S	PACING CRI	ΓΕRIA: 1.78	
	.1fc		
	1fc 3fc		
	5fc 10f		
-) 	

Specifications are subject to change without notice. Installation must be performed in accordance with Barron Lighting Group installation instructions. 10810374 Rev 6

Page 1 of 3



Specifications are subject to change without notice. Installation must be performed in accordance with Barron Lighting Group installation instructions. 10810374 Rev 6

Page 3 of 3



REVISION	DATE	BY	REMARKS	DESIGNED
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SURFACE MO	UNT
LED DETAILS AND S	CHEDULES
SHEET 11 OF 27	7

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SECT	ΠON		COUNTY	TOTAL SHEETS	SHEET NO.
22-00183-00-BR			LEE	315	182
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	22-0018 WHA# 1369	WHA# 1369D22	22-00183-00-BR WHA# 1369D22	22-00183-00-BR LEE WHA# 1369D22 CONTRACT	SHEETS COUNTY SHEETS

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HADCO

by (s) ignify

also has the ability to switch light output in 4 steps.

Landscape

BL9 FlexScape LED





Project: Location Cat.No: Type: Lamps: Qtv:

example: BL9DW-AS7

Ordering guide

Series BL9	Lamping D	сст	Finish	Nount S7
BL9 Low Voltage 9W Bullet Lighting	D LED	W Warm (3000K) C Cool (4000K)	A Black H Bronze	\$7 Stake

Features

- 1. Housing/Construction: A360° die-cast aluminum, tool-less twist off/on shroud. Teeth to lock aim the knuckle arm for accurate aiming secured by black oxide Phillips-head stainless steel screw and metal locking nut to provide durable mounting of the accent. A 360° die-cast aluminum housing for driver with thermal management creates continuity with housing by repeating similar shape. All gaskets are 100% molded
- 2. Electrical: 10W (on high setting) Input voltage range (VAC): 10 - 14. Pre-wired with a 3-ft. pigtail for easy hookup to the low voltage supply cable. Driver housed in injected molded case with electronics encapsulated. 12V class 2 driver with integral switch for 4 preset light levels.
- 3. LED Board and Array: Single Luxeon M LED.
- 4. Controls: 12 Volt Class 2 driver with integral switch provides simple customer access to the adjustment between 4 present light
- 5. Optical Systems: Flat glass, low iron tempered clear glass, c-channel gasket

slips onto lens without tools or RTV. Zoomable optic / Injection molded acrylic (PMMA) clear, highly polished molded with select surfaces textured. Zoomable lens provides Narrow 15° to Medium 35° beam pattern depending on the position, Interchangeable lens provides Wide 60 flood

6. Mounting: 1/2"-14 NPSM male threads to screw onto mounting stake, or other mounting accessory, sold separately.

Finish

Breaking new ground with optimal versatility, the FlexScape BL9 has zoomable

optics (15° to 35°) & 60° by switching optic element. This adjustable luminaire

Thermoset polyester powder coat is electrostatically applied after a five-stage conversion cleaning process and bonded by heat fusion thermosetting.

Lamps

Integral LED module

Watts Consumed /Needed to Drive	2.0 W	4.5 W	8.6 W	10.7W
mA	230	600	1100	1500
3K 15°	113	241	∠29	567
3K 35°	132	284	505	669
3K 60°	111	241	428	561
4K 15°	131	269	477	602
4K 35°	155	316	562	710

Power Supply

Fixtures can be used with the HADCO Low Voltage Transformers series TC152, TSS, TC. Power supplies are available in 150W / 300W / 600W and 900W.

Ask your Philips representative for a full list of options.

IP66 Rating

Dust tight and sealed against direct jets of

Labels

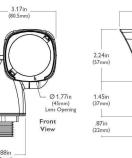
ETL Listed to U.S. safety standards for wet locations. cETL listed to Canadian safety standards for wet locations. Manufactured to ISO 9001:2008 Standards. 5-year limited

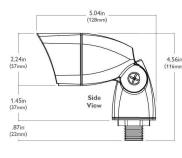
BL9 FlexScape LED

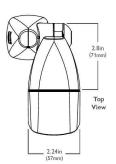
Accent aluminum

Dimensions





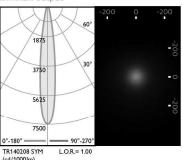




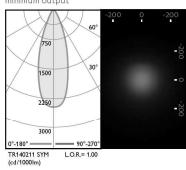
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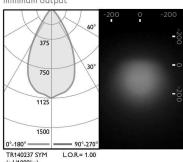
TYPE C

BL9 Warm 3000K



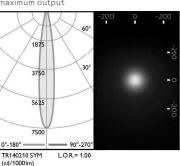
Narrow Flood



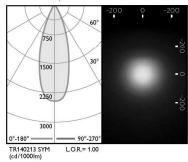


BL9 Cool 4000K

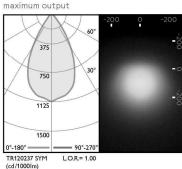
Spot



Narrow Flood



Flood



(s) ignify

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Signify North America Corporatio 200 Franklin Square Drive, Somerset, NJ 08873 Telephone 855-486-2216

Landscape_Spec Sheet_BL9.pdf 06/20 page 1 of 3

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CITY OF DIXON RIVER CROSSING SHARED-USE PATH



LED UP/DOWN PIER LIGHT	
DETAILS AND SCHEDULES	

F.A.P. RTE	SEC ⁻	ΠON		COUN	TY	TOTAL SHEETS	SHEET NO.
	22-0018	3-00-BR		LEE		315	183
	WHA# 1369	D22		CONT	RAC	T NO. 8	35762
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SHEET 12 OF 27



Gardco PureForm LED area medium comfort P26 features a sleek, low profile design. Comfort optics are designed to enhance visual comfort by reducing glare. Type 2, 3, and 5 optical distributions are availble. A full range of control options provides additional energy savings.

Project:		
Location:		
Cat.No:		
Type:		
Lumens:	Qty:	

Ordering guide

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example: P26-196L-650-NW-G2-AR-5-120-MGY

Prefix P26	Number of LEDs	Drive Current	LED Color - Generation WW-G2	Mounting	Distribution 2	Voltage 240
P26 PureForm area medium, 26*	196L 196 LEDs (comfort engine)	450 450 mA 650 650 mA 1150 1150 mA 1675 1675 mA 2100 2100 mA	WW-G2 Warm White 3000K, 80CRI Generation 2 NW-G2 Neutral White 4000K, 80CRI Generation 2 CW-G2 Cool White 5000K, 70CRI Generation 2 WY-G2 Warm Yellow 2700K, 80 CRI Generation 2! BW-G2 Balanced White 3500K (80 CRI) Generation 2	AR Arm Mount (standard) ² The following mounting kits must be ordered separately (See accessories) SF Slip Fitter Mount ² (fits to 2 ² / ₈ * 0.D. tenon) WS Wall mount with surface conduit rear entry permitted	Type 2 2 Comfort Type 2 Type 3 3 Comfort Type 3 Type 5 Comfort Type 5	240 240V 277 277V 347 347V 480 480V
	140L 140 LEDs (comfort engine)		AM-G2 Amber Generation 2 1,13	RAM Retrofit arm mount kit ²		

Dimming controls	Motion sensing lens	Photo-sensing	Electrical	Luminaire	Finish	
DD 0-10V External dimming (controls yo thera)* FAWS 7-Field Adjustable Wattage Selector 4* LC Integral wireless module 4.6.3.8 Bi-level functionality* 48 Dyna/Dimmer: Automatic Profile Dimming 0550 Security 50% Dimming, 1 hours 47 OKNO Media Rob Dimming, 50 to 35 Security 50% Dimming, 1 hours 47 CM30 Median 30% Dimming, 8 hours 47 CM30 Median 30% Dimming, 8 hours 47		PCB Photocontrol Button ¹⁹ TLR05 Twist Lock Receptacle 5 Pin ² TLR07 Twist Lock Receptacle 7 Pin ² TLR07 Twist Lock Receptacle W/Photocell ⁴ 20	Fusing FI Single (120, 277, 347VAC)* FI Souble (208, 240, 480VAC)* F3 Canadian Double Pull (208, 240, 480VAC)* Pole Mount Fusing FPI Single (120, 277, 347VAC)* FP2 Double (208, 240, 480VAC)* FP3 Canadian Double Pull (208, 240, 480VAC)* Surge Protection (10Ka standard) SP2 Increased 20kA	Square Pole Adapter included as standard TB Terminal Block RPA Round Pole Adapter (fits to 3"- 3.9" O.D. pole) "	Textured BK Black WH White BZ Bronze DGY Dark Gray MGY Medium Gray Customer specified RAL Specify option color or RAL (e RAL 7024) CC Custom color (Must supply color chip for required facto quote)	

Extended lead times apply. Contact factory for details.
 Mounts to a 4-5" OF round pole with adapter included for square poles.
 Limited to a maximum of 45 degrees aiming above horizontal.
 Not available with other control options.
 Not available with motion sensor.
 Not available with motion sensor.
 Not available with motion sensor.

P26_PureForm_area_medium_comfort 03/22 page 1 of 6

- 7. Not available in 347 or 480V.
 8. Must specify input voltage.
 9. Dimming will not be connected to NEMA receptacle if ordering with other control options.
 10. Not sealable in 480V. Order photocell separately with TLRDS

LI INNOVATION AWARDS CUL US







P26 PureForm LED medium

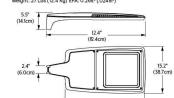
Area light with comfort optics

TYPE D

Slip fitter (SF) Weight: 32 Lbs (14.6 Kg) EPA: 0.38ft² (.035m²)

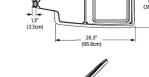
5.0" 3.6" (12.7cm) (9.0cm)

Weight: 27 Lbs (12.4 Kg) EPA: 0.26t² (.024m²)

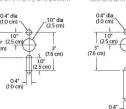


Weight: 28 Lbs (12.7 Kg) EPA: 0.28t² (.026m²)











P26_PureForm_area_medium_comfort 03/22 page 4 of 6

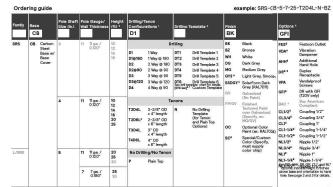
G GARDCO

Poles & Brackets



85762

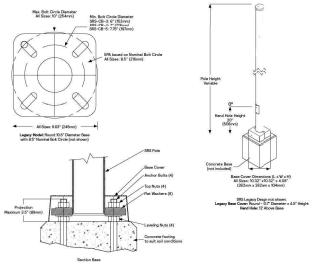
The Gardoo SRS Straight Round Steel pole consists of a one-piece high tensile carbon steel tibs ewolded and secured to the carbon steel base plate providing excellent strangth and integrity. The poles are finished with an electrostatically applied, thermally cured polyester powdercoat. All poles include base cover, hand hole, ground lug and top cap. Anchor boits and templates are ordered as a separate accessory.





Poles Straight Round Steel

Family	Base	Pole Shaft Stze (In.)	Pole Gauge/ Wall Thickness	Height (ft.) ⁵	Drilling Configuration	Drilling Template	Finish	Options
SRS Straight Round Steel	CB Carbon Steel Base w/ Cover	4	11 11 ga. / 0.120**	20 25	D1 1 Way D1@180 1 Way @ 180 D2 2 Way @ 180 D2@90 2 Way @ 90 D3 3 Way @ 90 D3@120 3 Way @ 120 D4 4 Way @ 90	DTS Drill Template 5	BZ Bronz, MG Mediun Gres BK Black	RS RapidShip*



NOTE: Factory supplied template must be used when setting anchor boits. Gardoo will not honor any claim for incorrect anchorage placement from failure to use factory supplied template SRS_Spec_Sheet_US 07/24 page 2 of 5

LIGHTING

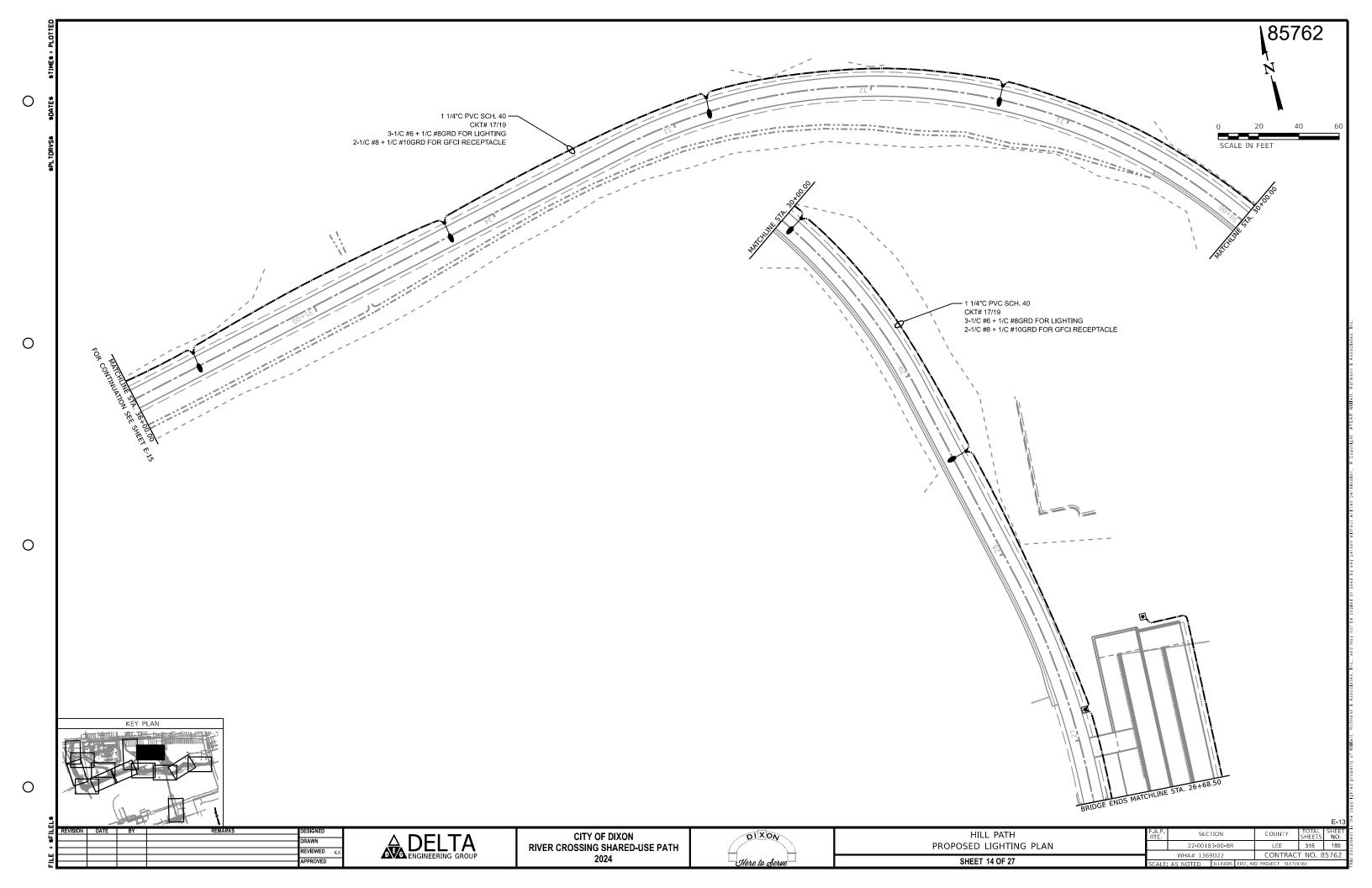
REVIEWED K APPROVED

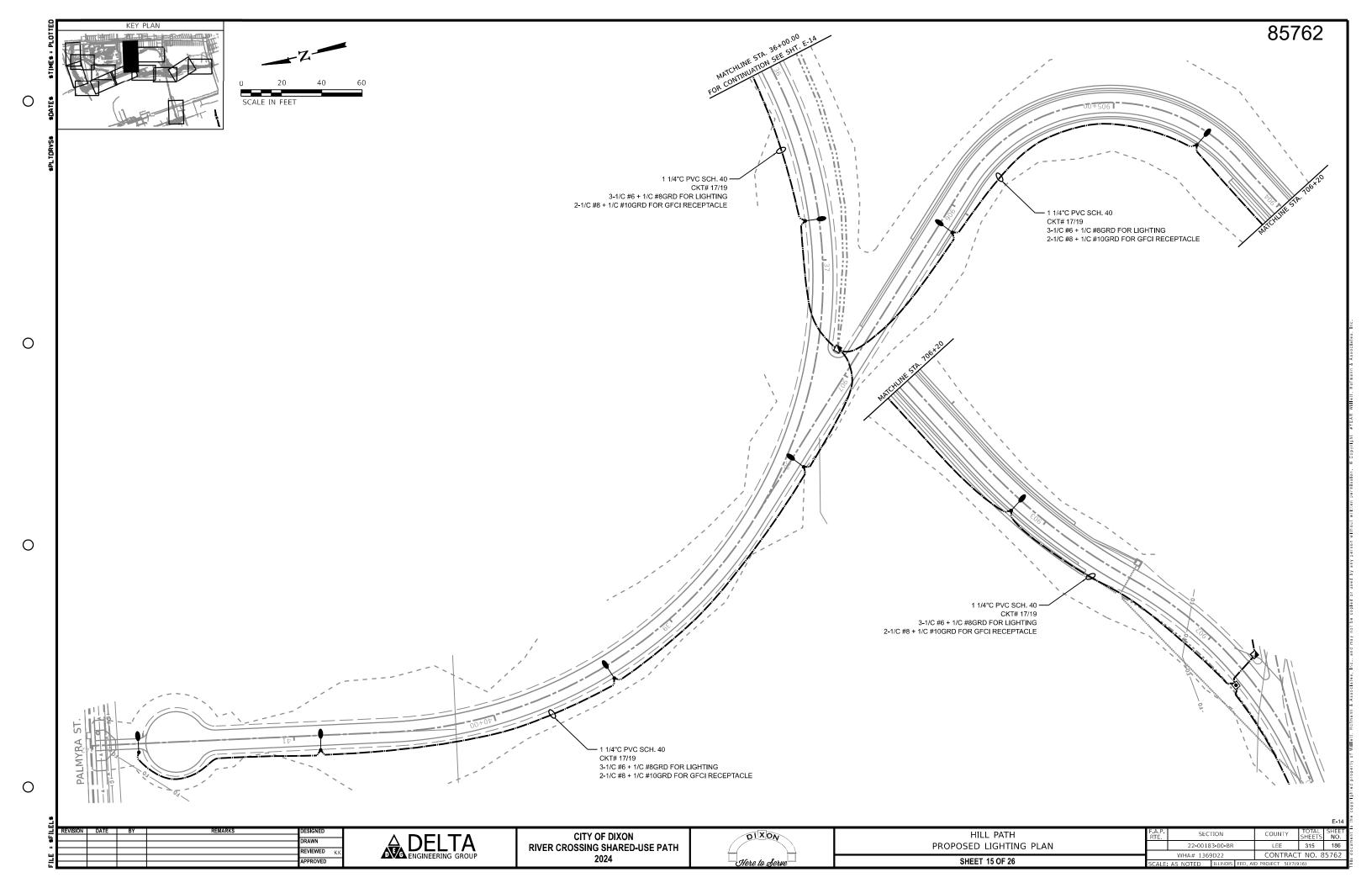


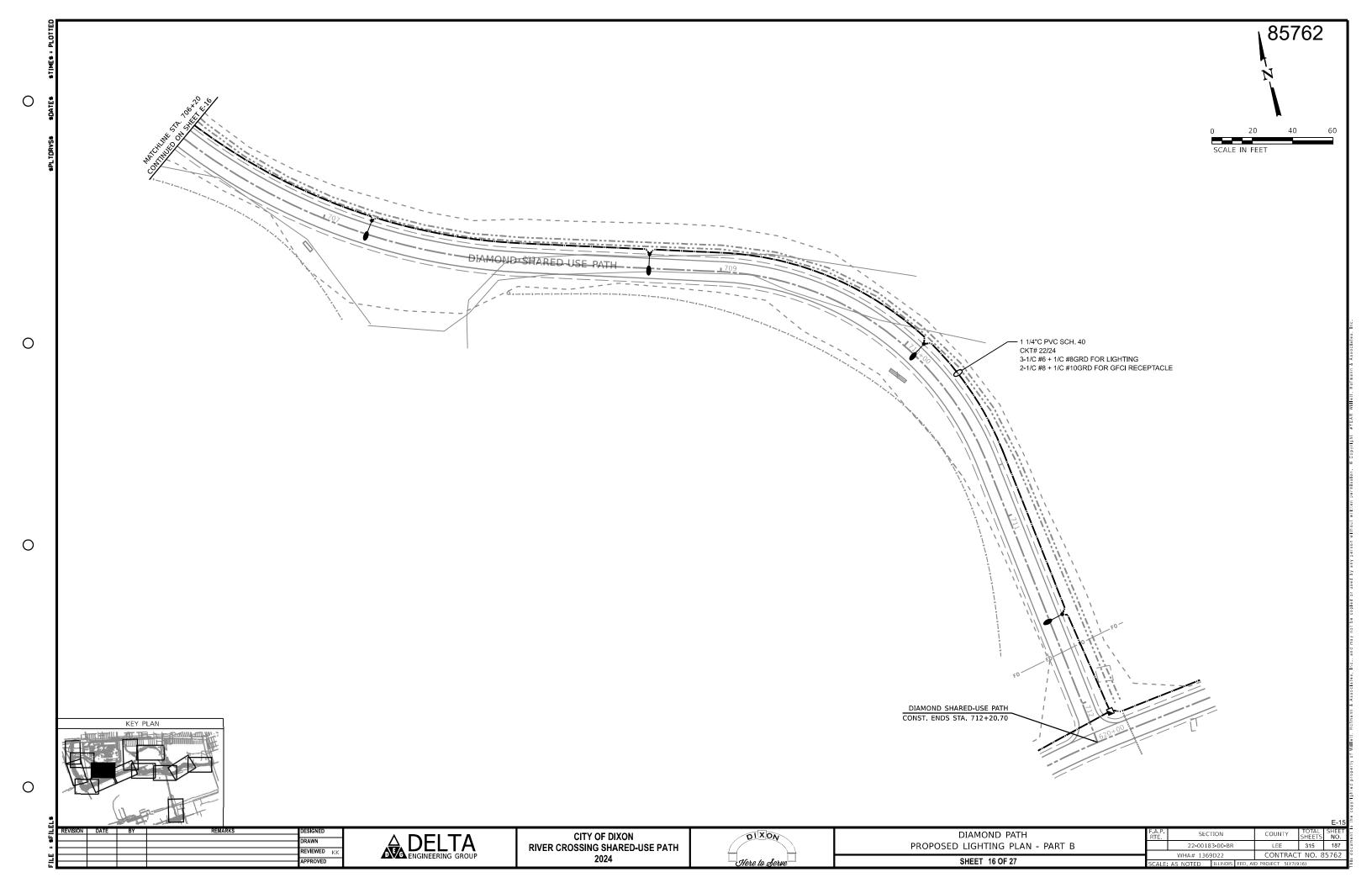
CITY OF DIXON RIVER CROSSING SHARED-USE PATH

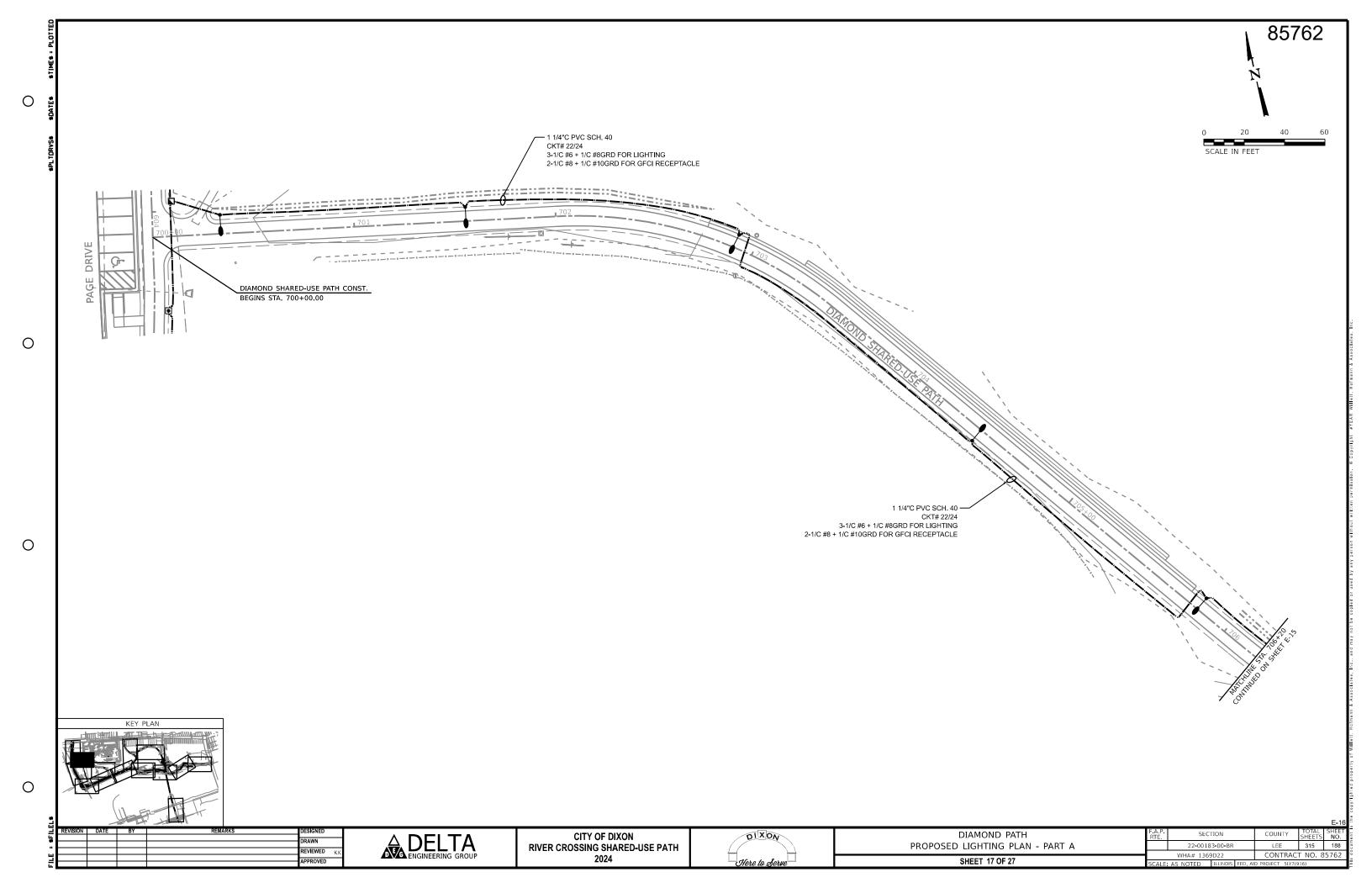


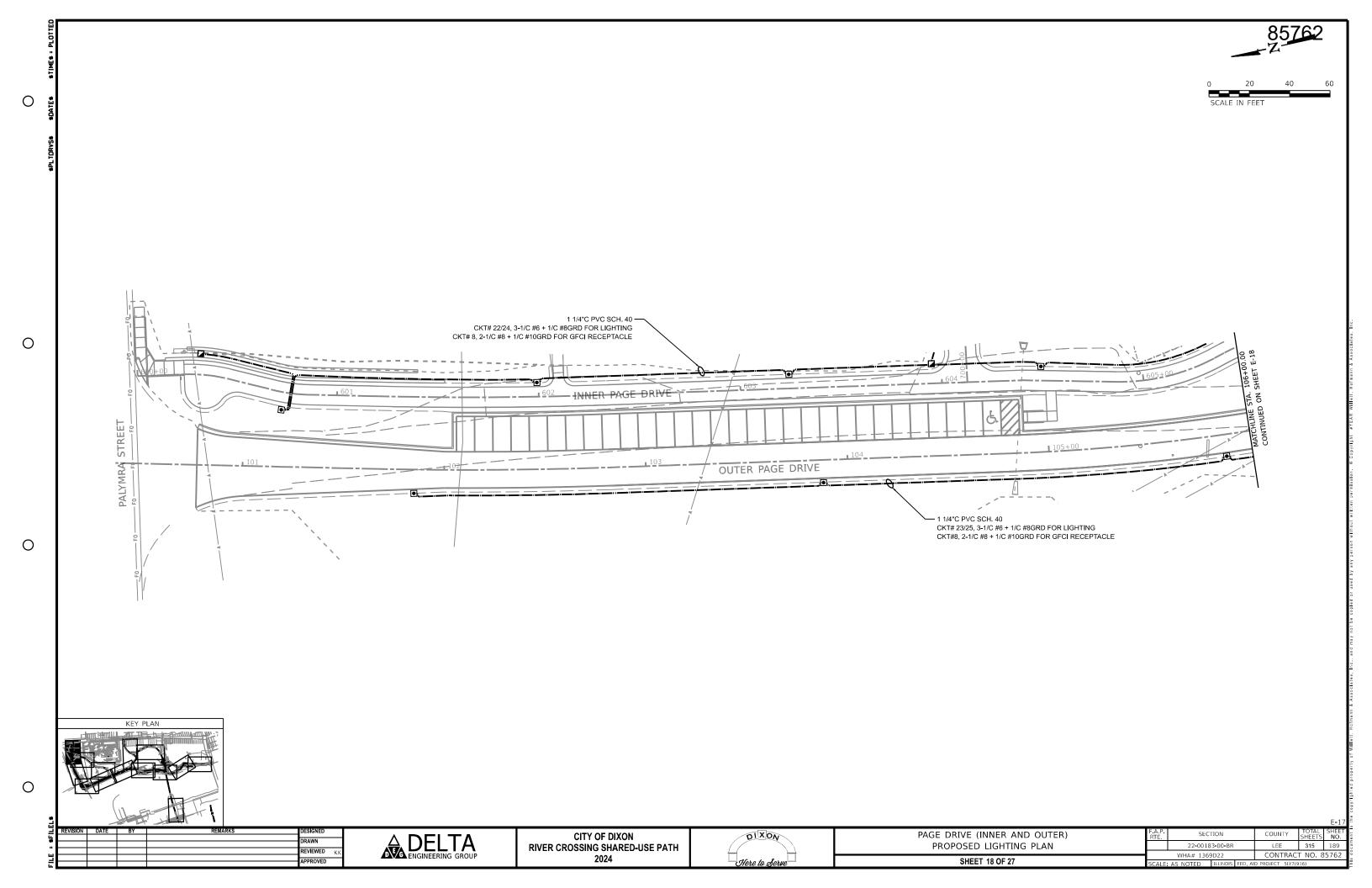
						_ 12
ITING FIVELING DETAIL	F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
ITING FIXTURE DETAIL		22-00183-00-BR		LEE	315	184
011557 40 05 07	WHA# 1369D22			CONTRACT NO. 85762		
SHEET 13 OF 27	SCALE:	AS NOTED	ILLINOIS FED. AI	D PROJECT 5LY7(9	16)	

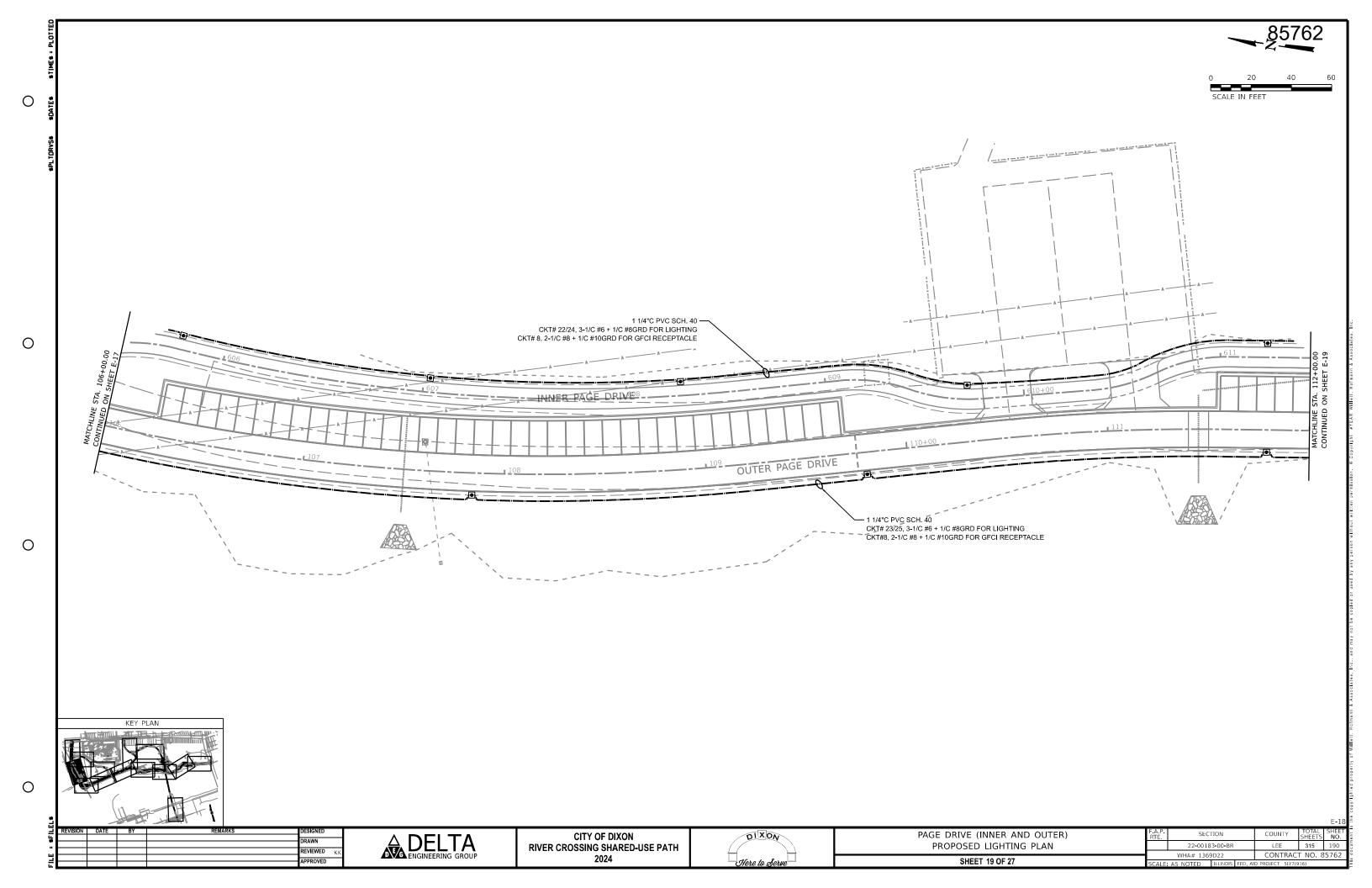


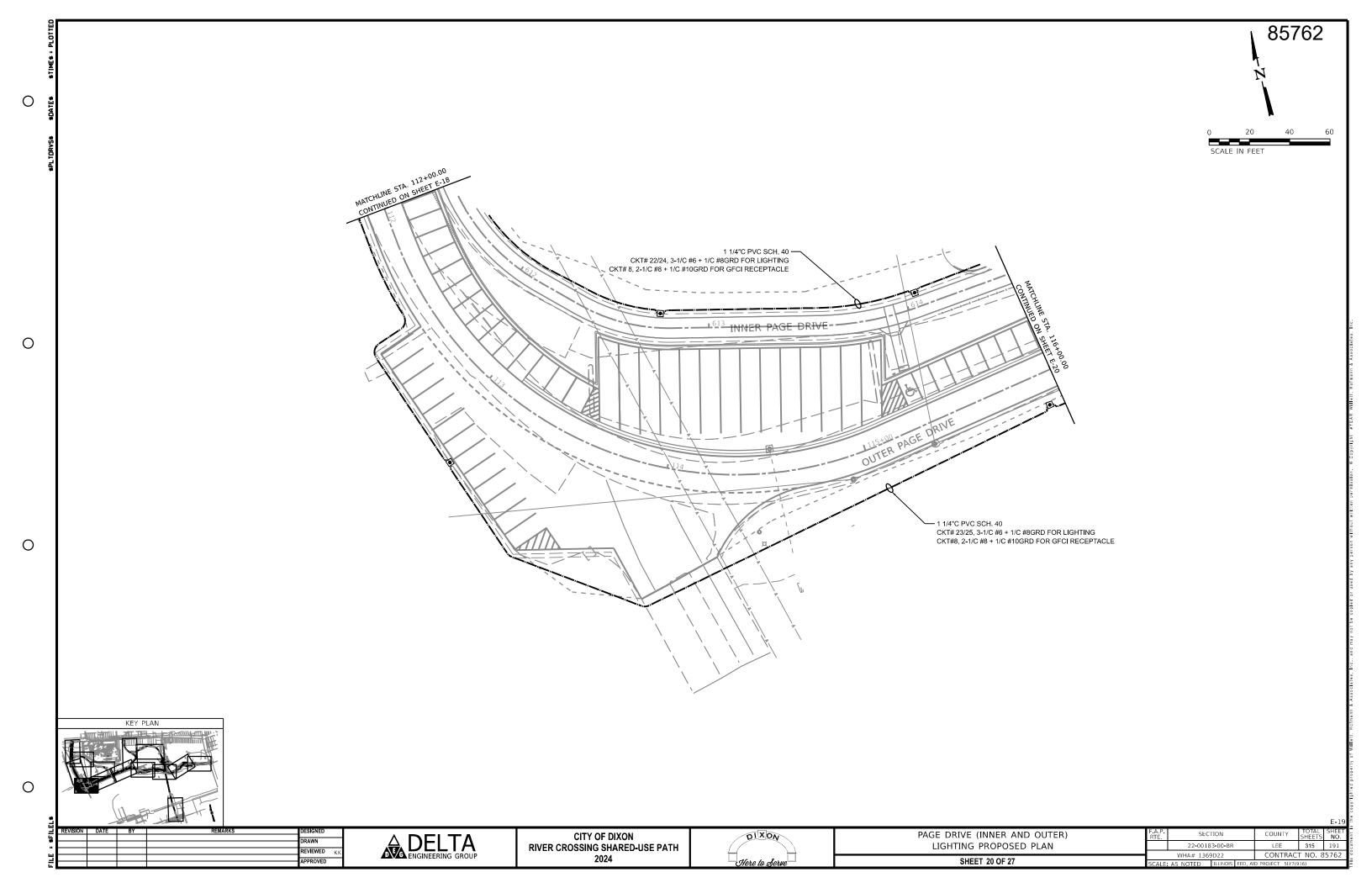


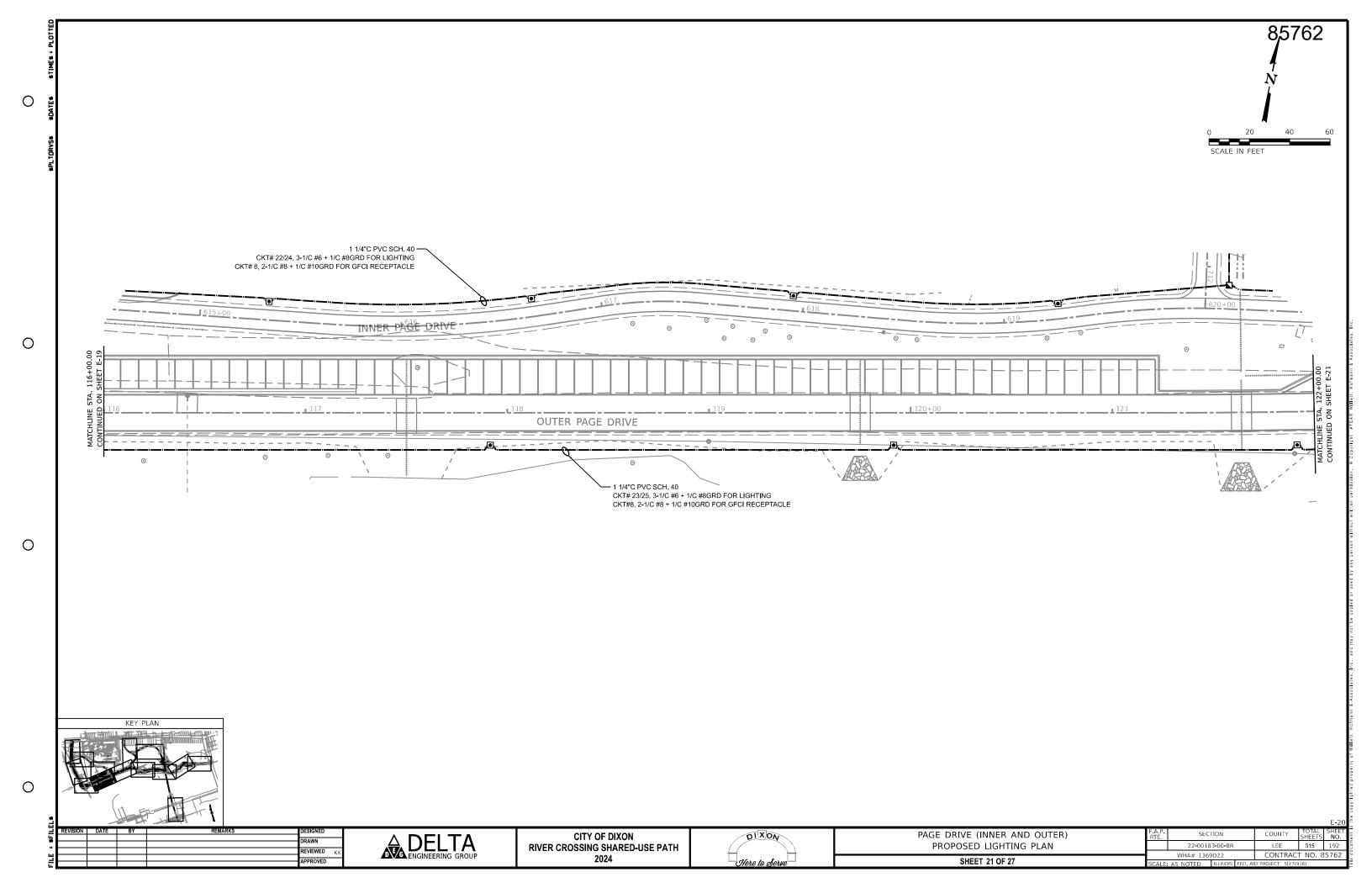


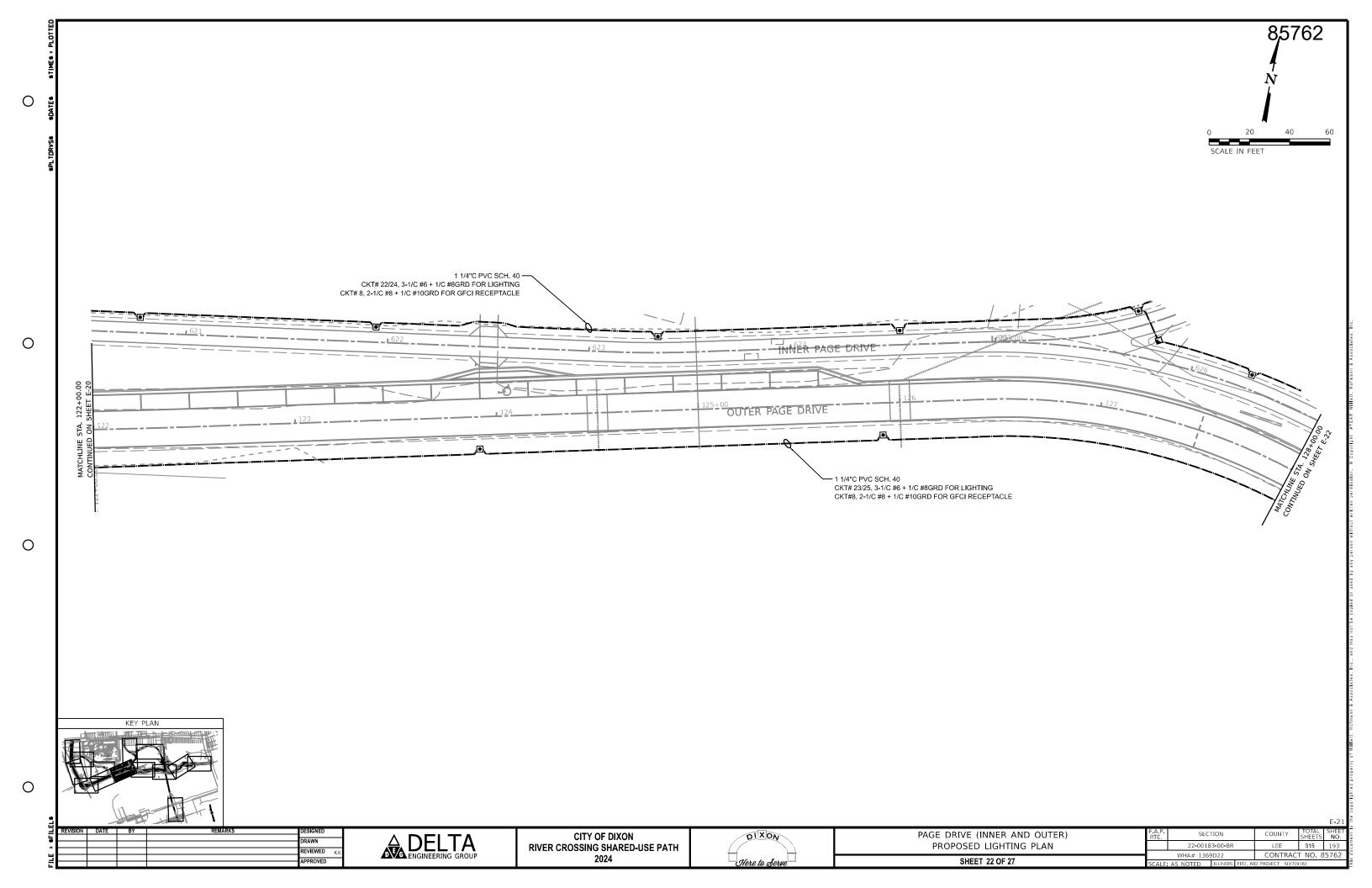


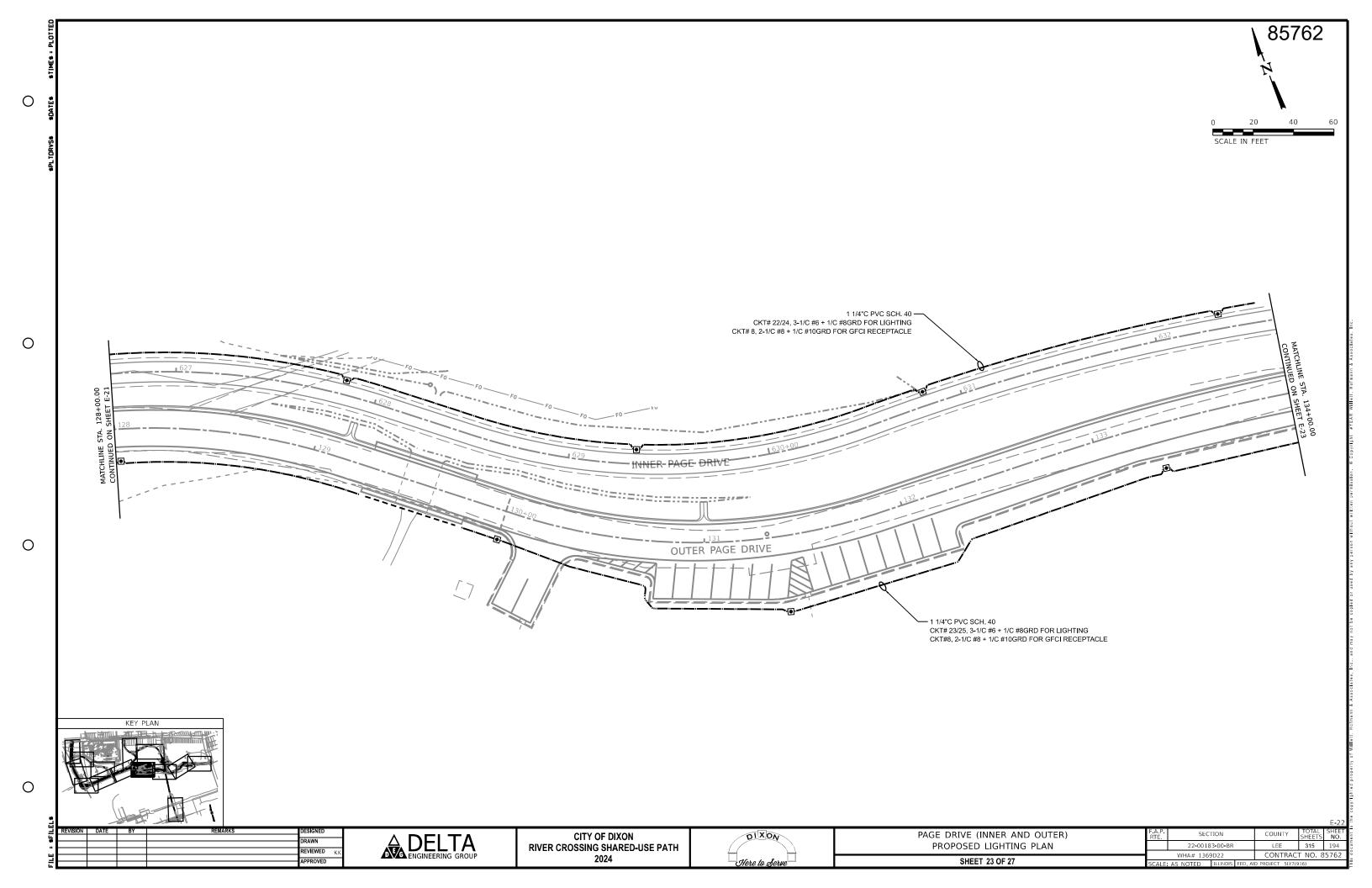


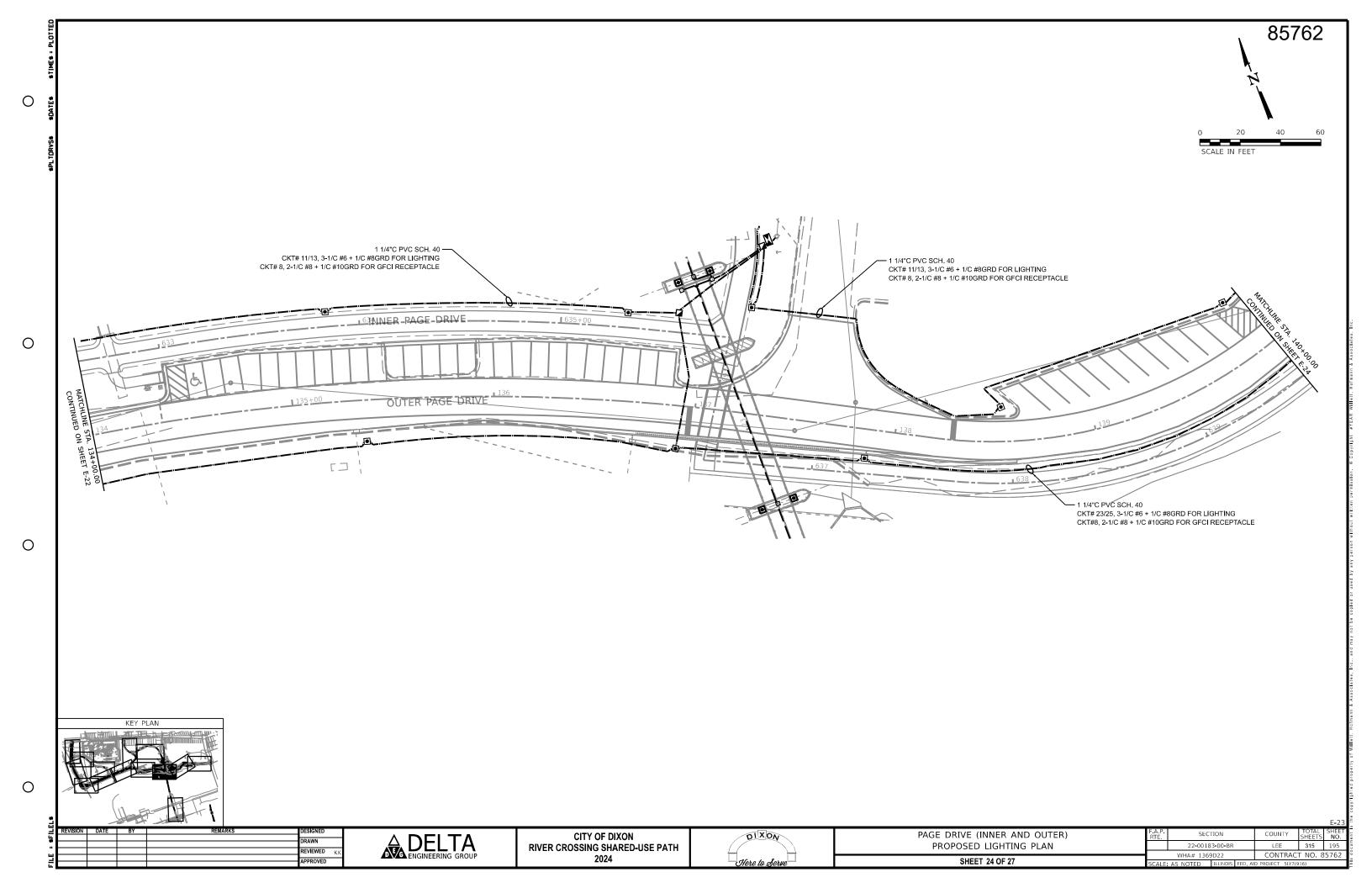


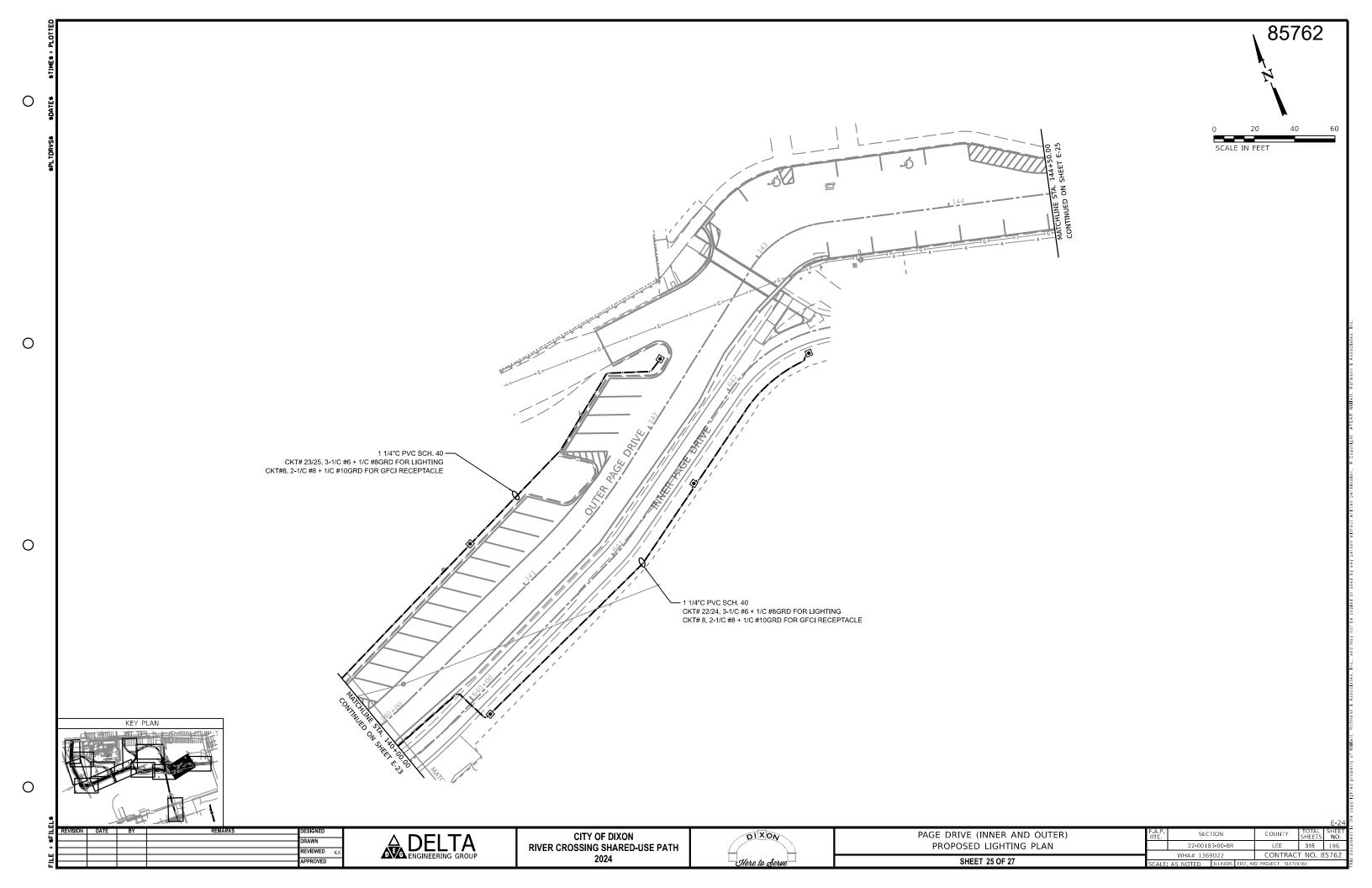


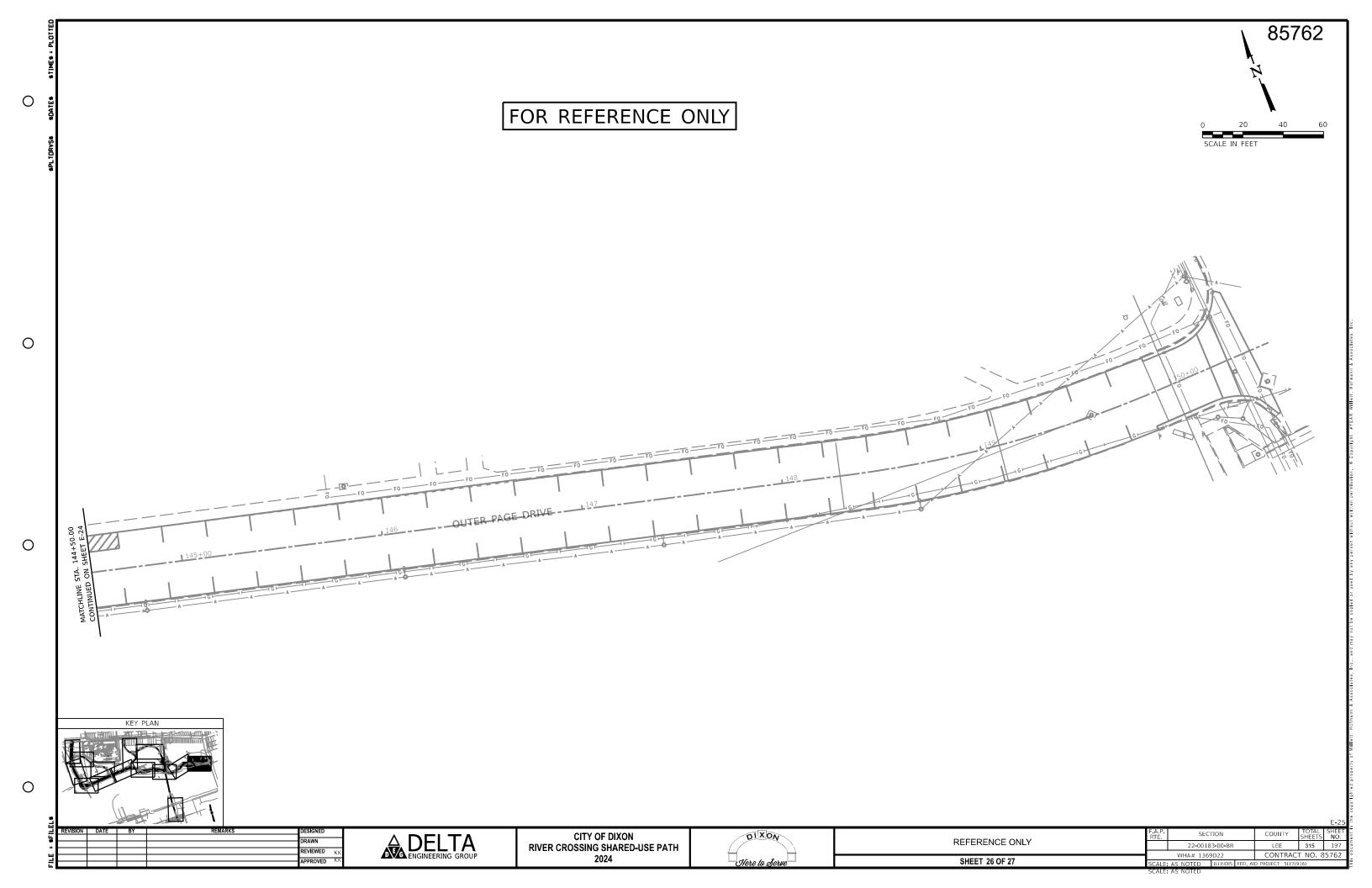


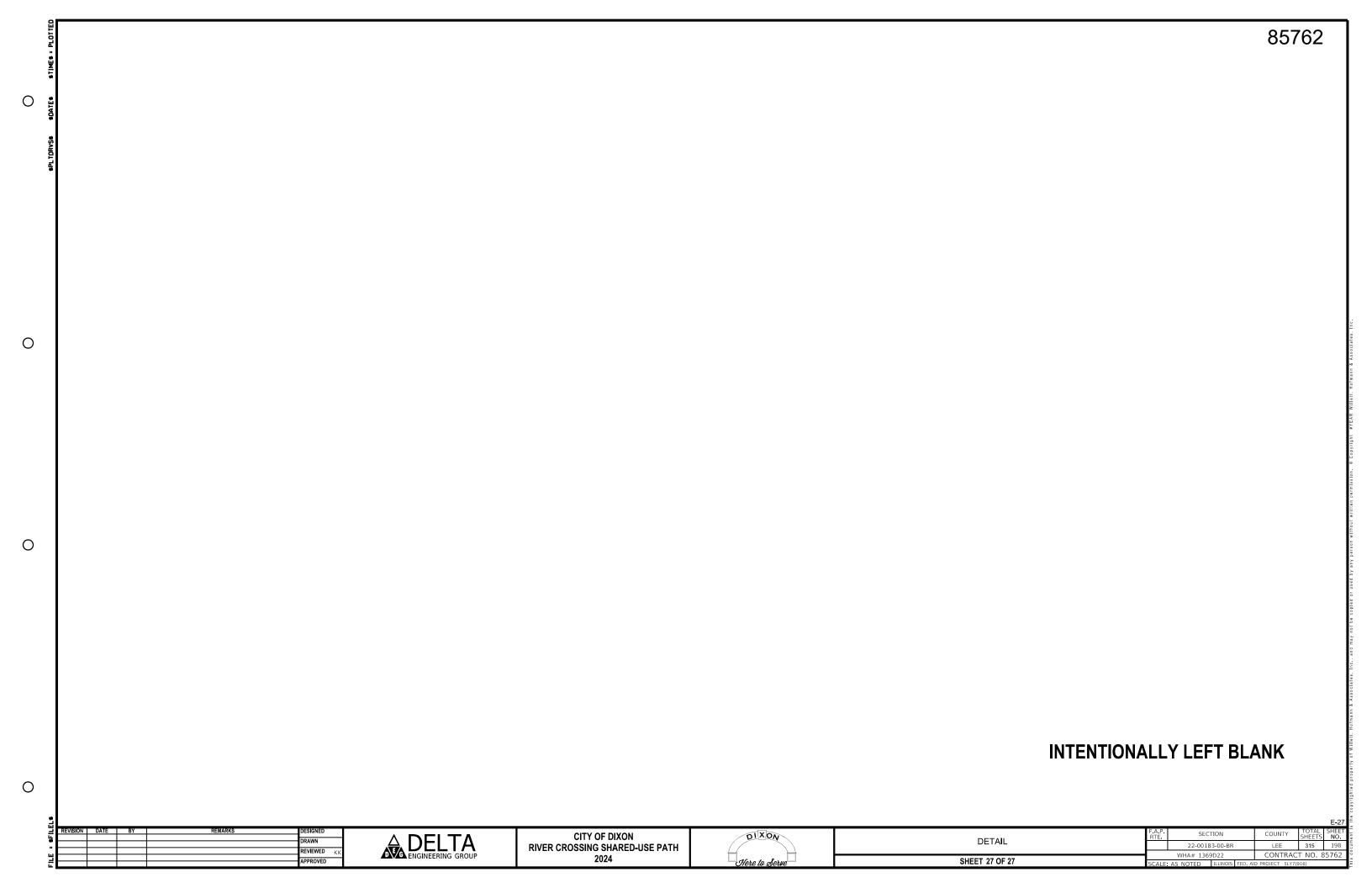


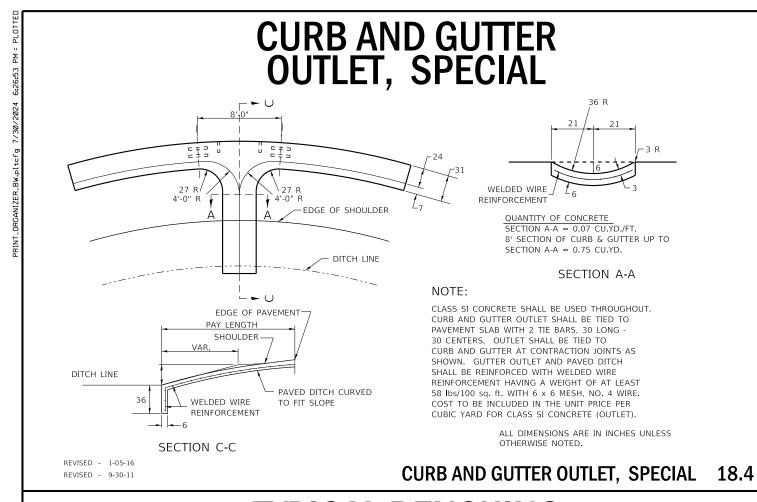




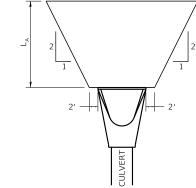




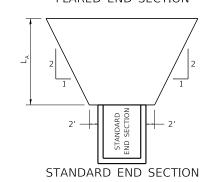




RIPRAP AT END SECTIONS



FLARED END SECTION



REVISED - 7-13-16

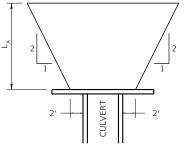
REVISED - 11-12-14

REVISED - 2-10-14

 $L_A = APRON LENGTH (ft)$

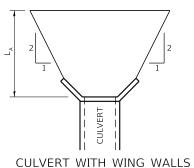
IF THE CULVERT OUTLETS INTO A DEFINED CHANNEL, RIPRAP BANK TO BANK FOR LENGTH (L_{Δ}) .

STANDARD END SECTION: 542001 (PIPE), 542011 (ELLIPTICAL) DISTRICT STANDARD 10.1 (BOX).



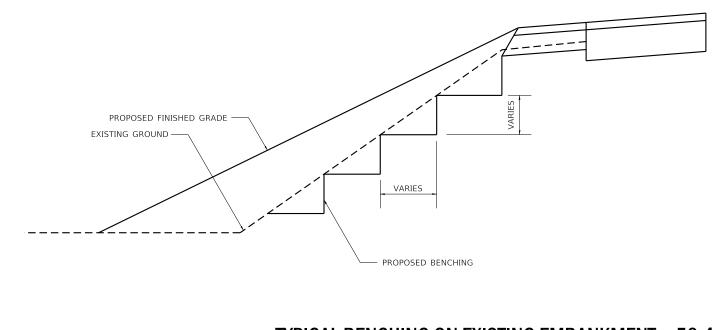
85762

CULVERT WITH HEADWALL



RIPRAP AT END SECTIONS 19.4

TYPICAL BENCHING ON EXISTING EMBANKMENT



TYPICAL BENCHING ON EXISTING EMBANKMENT 50.4 CITY OF DIXON RIVER CROSSING SHARED-USE PATH

DIXON

REGION 2 / DISTRICT 2 STANDARDS 18.4, 19.4, AND 50.4 SHEET 1 OF 11

22-00183-00-BR LEE 315 199 WHA# 1369D22 CONTRACT NO. 85762

REVISED - 2-22-06

 \bigcirc

Willett Hofmann

