PROPOSED

FAP ROUTE 322 (US 51) SECTION 12-1,BR **UNION COUNTY**

PROJECT: BHF-0322(085)

PROPOSED PROJECT BEGINS

STA. 798+50.0

C-99-048-08

SUPERSTRUCTURE REPLACEMENT ON US 51 OVER CACHE CREEK

HIGHWAY PLANS

ANNA TOWNSHIP

POSTED SPEED = 55 MPH

TRAFFIC DATA 2008 ADT = 6250

6.9% TRUCKS

FOR INDEX OF SHEETS, SEE SHEET NO. 2

FOR SUMMARY OF QUANTITIES, SEE SHEET NUMBERS 3 AND 4

PROPOSED SUPERSTRUCTURE REPLACEMENT OVER CACHE CREEK STRUCTURE NO. 091-0022
SINGLE SPAN STEEL W24 BRIDGE;
40-8" BK TO BK ABUTMENTS; 37°-30' SKEW

© STRUCTURE STA 801+60.00

PROPOSED PROJECT ENDS STA. 804+50.0

SN 091-0022

ROADWAY LENGTH = 559'-4" BRIDGE LENGTH = 40'-8" NET LENGTH OF PROJECT = 600'-0"

322 (12-1) BR UNION ILLINOIS CONTRACT NO. 98886

D-99-029-04



STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS SUBMITTED June 18 20 08 Man C. Ramie
DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER august 15, 20 08 Eric E. Hary & Interior Engineer of Design and Environment august 15, 2008 Christine M. Reed 10 DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

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ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123 OR 811

CONTRACT NO. 98886

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HIGHWAY STANDARDS

HIGHWAY STANDARDS

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000001-05		STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS	
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420401-06		BRIDGE APPROACH PAVEMENT	
421001-02		BAR REINFORCEMENT FOR CRC PAVEMENT	
482001-02		HMA SHOULDER ADJACENT TO FLEXIBLE PAVEMENT	
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630001-07		STEEL PLATE BEAM GUARDRAIL	
630201-05		PCC/HMA STABILIZATION AT STEEL PLATE BEAM GUARDRA	īL.
630301-04		SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TE	RMINALS
631031-06		TRAFFIC BARRIER TERMINAL, TYPE 6	
635006-02		REFLECTOR AND TERMINAL MARKER PLACEMENT	
635011-01		REFLECTOR MARKER AND MOUNTING DETAILS	
701201-02		LANE CLOSURE, 2L, 2W, DAY ONLY, FOR SPEEDS > 45 MPH	
701301-02		LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS	
701321-09		LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER	
701326-02		LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING, FOR SPEEDS	> 45 MPH
701901-00		TRAFFIC CONTROL DEVICES	
704001-04		TEMPORARY CONCRETE BARRIER	
780001-01		TYPICAL PAVEMENT MARKINGS	
781001-02		TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MA	RKERS

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Examined By:	In another
	DISTRICT CONSTRUCTION ENGINEER
Examined By:	Bauce in Poplar
	DISTRICT MATERIALS ENGINEER
Examined By:	A LA
	DISTRICT PROJECT IMPLEMENTATION ENGINEER
Examined By:	b) I Ott
	ASSISTANT REGIONAL ENGINEER
A	P
Approved By:	May Chamij
	DEPUTY DIRECTOR OF HIGHWAYS, REGION 5 ENGINEER
	June 18 20 08
	DATE

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	PLCT DATE = 6/5/2008	DATE	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

INDEX OF SHEETS, HIGHWAY STANDARDS AND SIGNATURE SHEET

SCALE: ____ SHEET NO. 2 OF 48 SHEETS STA. ___

GENERAL NOTES

FACTORS USED FOR ESTIMATING PLAN QUANTITIES ARE AS FOLLOWS AND SHALL NOT BE USED FOR THE BASIS OF FINAL QUANTITIES:

ALL HOT-MIX ASPHALT 2.016 TONS/CU.YD.

BITUMINOUS MATERIALS:

ON PAVEMENT O.09 GAL./SQ.YD.

INTERMEDIATE.LIFTS(FOG COAT)
0.04 GAL./SQ.YD.

ON AGGREGATE SURFACE O. 32 GAL./SQ. YD.

AGGREGATE (PRIME COAT)
0.0015 TONS/SQ.YD.

ALL AGGREGATE 2.05 TONS/CU.YD.

RIPRAP 1.50 TONS/CU.YD.

THE QUANTITY OF SHORT TERM PAVEMENT MARKING SHOWN IN THE PLANS IS BASED ON ONE APPLICATION FOR HOT MIX ASPHALT SURFACE COURSE.

ALL TEMPORARY EROSION CONTROL MEASURES SHALL BE LEFT IN PLACE UNTIL REMOVAL IS REQUIRED TO CONSTRUCT FINAL GRADE LINES.

THE CONTRACTOR SHALL STAMP STATIONING IN THE PROPOSED HOT MIX ASPHALT SURFACE AT 300 FT. INTERVALS ON ALTERNATING SIDES OF THE PAVEMENT AND AS DIRECTED BY THE ENGINEER. THE STATION SYMBOL STAMPS USED SHALL BE FURNISHED BY THE CONTRACTOR. THEY SHALL BE 5 1/2 IN. TALL, OF A DESIGN APPROVED BY THE ENGINEER, AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR. STATIONING FOR THIS PROJECT IS INCREASING TO THE SOUTH.

PRIOR TO PLACEMENT OF THE FINAL PAVEMENT MARKINGS THE RESIDENT ENGINEER SHALL CONTACT THE BUREAU OF OPERATIONS AND ARRANGE FOR INSPECTION AND APPROVAL OF THE PAVEMENT MARKING LAYOUT.

AREAS OF EXISTING AGGREGATE SHOULDERS THAT SHALL BE REMOVED FOR CONSTRUCTION OF FINAL SHOULDER SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE PER CUBIC YARD FOR EARTH EXCAVATION.

IT WILL BE THE CONTRACTORS RESPONSIBILITY TO REMOVE ANY DEBRIS OR DIRT CAUSED BY CONSTRUCTION ACTIVITY THAT COVERS THE NEW RIPRAP. NO EXTRA CONPENSATION SHALL BE ALLOWED FOR THIS WORK.

PROTECTIVE COAT SHALL BE APPLIED TO THE NEW BRIDGE DECK, THE BRIDGE APPROACH PAVEMENTS, AND THE BRIDGE APPROACH PAVEMENT CONNECTORS(PCC) SPECIAL. IN ACCORDANCE WITH ARTICLE 503. 19 OF THE STATE STANDARD SPECIFICATIONS. THE SEASONAL EXCEPTION SHALL NOT APPLY. THE PROTECTIVE COAT SHALL BE APPLIED REGARDLESS OF THE CURING METHOD USED. THE RATE OF APPLICATION FOR EACH COAT ON CUT GROOVED AREAS SHALL BE 25 SQUARE YARDS PER GALLON OF MIXTURE.

REMOVAL OF THE ENTIRE EXISTING BRIDGE APPROACH PAVEMENTS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE PER SQUARE YARD FOR PAVEMENT REMOVAL.

AGGREGATE FOR TEMPORARY ACCESS SHALL BE USED AS DIRECTED BY THE ENGINEER FOR MAINTENANCE PURPOSES. THE GRADATION SHALL BE CA-6 OR CA-10 AS DIRECTED BY THE ENGINEER. A QUANTITY OF 35 TONS HAS BEEN ESTIMATED FOR THIS WORK.

PLAN DIMENSIONS AND DETAILS RELATIVE TO THE EXISTING STRUCTURE HAVE BEEN TAKEN FROM EXISTING PLANS AND ARE SUBJECT TO NOMINAL CONSTRUCTION VARIATIONS. IT SHALL BE THE CONTRACTOR'S RESOPONSIBILITY TO VERIFY SUCH DIMENSIONS AND DETAILS IN THE FIELD AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OR ORDERING OF MATERIALS. SUCH VARIATIONS SHALL NOT BE A CAUSE FOR ADDITIONAL COMPENSATION OR A CHANGE IN THE SCOPE OF THE WORK. THE CONTRACTOR, HOWEVER, WILL BE PAID FOR THE ACTUAL QUANTITY FURNISHED AT THE UNIT PRICE BID FOR THE WORK. CONSTRUCTION PLANS ARE AVAILABLE FOR REVIEW AT THE DISTRICT 9 HEADQUATERS IN CARBONDALE, ILLINOIS.

IN ADDITION TO THE REQUIREMENTS OF ARTICLE 107.16, THE CONTRACTOR SHALL PROTECT THE SURFACE OF ALL BRIDGE DECK AND BRIDGE APPROACH PAVEMENTS IN A MANNER SATISFACTORY TO THE ENGINEER BEFORE ANY EQUIPMENT IS ALLOWED TO CROSS THE STRUCTURE. PROTECTION SHALL BE PROVIDED FOR ALLL EQUIPMENT AS DEFINED IN ARTICLE 101.17 REGARDLESS OF TRACK MOUNTED OR WHEELED.

AT ALL LOCATIONS WHERE ASPHALT OR CONCRETE PAVEMENT JOINS AN EXISTING HOT MIX ASPHALT OR CONCRETE PAVEMENT, A SAWED JOINT SHALL BE CONSTRUCTED. THE COST OF THIS JOINT SHALL BE INCLUDED IN THE TYPE OF PAVEMENT BEING CONSTRUCTED.

QUANTITIES SHOWN IN THE PLANS FOR BRIDGE DECK GROOVING AND PROTECTIVE COAT INCLUDE THE BRIDGE, THE BRIDGE APPROACH PAVEMENTS, AND THE BRIDGE APPROACH PAVEMENT CONNECTORS (PCC) SPECIAL.

THE ADVANCE DETECTOR LOOPS ARE TYPICALLY LOCATED 300 FEET IN ADVANCE OF THE STOP BAR. THE BUREAU OF OPERATIONS SHOULD APPROVE THE LOOP LOCATIONS PRIOR TO INSTALLATION.

VERTICAL PANELS SHOWN ON STANDARD 701321 WILL NOT BE REQUIRED ON THE STAGE II NEW BRIDGE PARAPET. THE BARRIER WALL REFLECTORS SHALL BE INSTALLED PRIOR TO OPENING STAGE II TRAFFIC.

ANY TIME THE CONCRETE BARRIER IS NOT IN THE PROPER POSITION, FLAGGERS SHALL BE IN PLACE TO CONTROL TRAFFIC. THE TEMPRARY TRAFFIC SIGNALS SHALL BE COVERED OR TURNED OFF.

THE CENTERLINE PAVEMENT MARKING SHALL BE REMOVED FROM THE STOP BAR TO THE SAND ATTENUATORS OR DRUMS. EDGE LINE PAVEMENT MARKING SHALL BE REMOVED IF A 10 FOOT LANE WIDTH CANNOT BE MAINTAINED. TEMPORARY EDGE LINES SHOULD BE INSTALLED WHEN THE EDGE LINES ARE REMOVED.

SCALE:

COMMITMENTS

NONE AS OF JUNE 27, 2008. REFER TO COMMITMENT FILE FOR ANY COMMITMENTS AFTER THIS DATE.

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	PLOT DATE = 6/18/2008	DATE -	REVISED -	ı

	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
GENERAL NOTES & COMMITMENTS	FAP 322	12-1,BR	UNION	48	3
			CONTRACT	NO. 9	98886
SHEET NO. 3 OF 48 SHEETS STA. TO STA.	FFD. ROAD (IST. NO. THEINOIS FED. 4	ID PROJECT		

SUMMARY OF QUANTITIES

RURAL-UNION COUNTY

			NDING 80% FED. 20% STATE		
		X071-2A SN 091-0022			
CODE NUMBER	ITEM DESCRIPTION	UNIT	TOTAL QUANTITIES		
20200100	EARTH EXCAVATION	CU YD	65		
20300100	CHANNEL EXCAVATION	CU YD	291		
20400800	FURNISHED EXCAVATION	CU YD	23		
20700400	POROUS GRANULAR EMBANKMENT, SPECIAL	CU YD	97		
25000350	SEEDING, CLASS 7	ACRE	0.50		
25000400	NITROGEN FERTILIZER NUTRIENT	POUND	64		
25000500	PHOSPHORUS FERTILIZER NUTRIENT	POUND	48		
25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	48		
25000700	AGRICULTURAL GROUND LIMESTONE	TON	1.0		
25001010	SEEDING, CLASS 2 (MODIFIED)	ACRE	0.50		
25100115	MULCH, METHOD 2	ACRE	1.00		
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	80		
28100107	STONE RIPRAP, CLASS A4	SQ YD	416		
28200200	FILTER FABRIC	SQ YD	416		
40201000	AGGREGATE FOR TEMPORARY ACCESS	TON	35		
40600100	BITUMINOUS MATERIALS (PRIME COAT)	GALLON	160		
40600300	AGGREGATE (PRIME COAT)	TON	2		
40600990	TEMPORARY RAMP	SQ YD	27		
40603320	HOT MIX ASPHALT SURFACE COURSE, MIX "C", N90	TON	102		
42001165	BRIDGE APPROACH PAVEMENT	SQ YD	275		
44000100	PAVEMENT REMOVAL	SQ YD	495		
44000155	HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2"	SQ YD	1195		
44004250	PAVED SHOULDER REMOVAL	SQ YD	785		
42001300	PROTECTIVE COAT	SQ YD	517		

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- 1		PLOT DATE = 7/1/2008	DATE	REVISED		SCALE: SHEET NO. 4 OF 48 SHEETS STA TO STA	FED. ROAD DIS	. NO ILLINOIS FE	D. AID PROJECT	

SUMMARY OF QUANTITIES (CONTINUED)

			RURAL-UNION COUNTY NDING 80% FED. 20% STATE X071-2A SN 091-0022
CODE NUMBER	ITEM DESCRIPTION	UNIT	TOTAL QUANTITIES
48203037	HOT-MIX ASPHALT SHOULDERS, 10"	SQ YD	933
50101500	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	1
50102400	CONCRETE REMOVAL	CU YD	18,8
50200100	STRUCTURE EXCAVATION	CU YD	97
50300100	FLOOR DRAINS	EACH	4
50300225	CONCRETE STRUCTURES	CU YD	22. 1
50300255	CONCRETE SUPERSTRUCTURE	CU YD	68. 9
50300260	BRIDGE DECK GROOVING	SQ YD	682.0
50300300	PROTECTIVE COAT	SQ YD	208
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1
50500405	FURNISHING AND ERECTING STRUCTURAL STEEL	POUND	25, 750
50500505	STUD SHEAR CONNECTORS	EACH	1, 116
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	17, 440
50800515	BAR SPLICERS	EACH	245
51205200	TEMPORARY SHEET PILING	SQ FT	195
51500100	NAME PLATES	EACH	1
52100520	ANCHOR BOLTS, 1"	EACH	24
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	31
60109580	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	152
63000000	STEEL PLATE BEAM GUARD RAIL, TYPE A	FOOT	388
63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4
63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	4
63200310	GUARDRAIL REMOVAL	FOOT	465
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	14
67100100	MOBILIZATION	L SUM	1

* Specialty Hems

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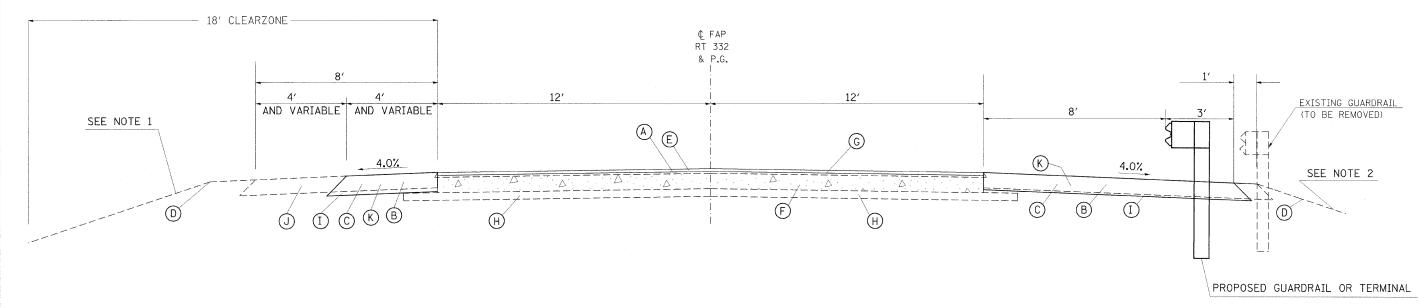
SUMMARY OF QUANTITIES (CONTINUED)

		HBP F	RURAL-UNION COUNTY UNDING 80% FED. 20% STATE X071-2A SN 091-0022
CODE NUMBER	ITEM DESCRIPTION	UNIT	TOTAL QUANTITIES
70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321	EACH	1
70100450	TRAFFIC CONTROL AND PROTECTION, STANDARD 701201	L SUM	1
70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD 701326	L SUM	1
70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	5
70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1
70106800	CHANGEABLE MESSAGE SIGN	CAL MO	3
70300100	SHORT-TERM PAVEMENT MARKING	FOOT	103
70300220	TEMPORARY PAVEMENT MARKING - LINE 4"	FOOT	1, 350
70301000	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	34
70400100	TEMPORARY CONCRETE BARRIER	FOOT	450
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	400
* 78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	1, 350
* 78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	8
* 78200405	GUARDRAIL MARKERS	EACH	10
* 78200500	BARRIER WALL MARKERS	EACH	4
* 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4
78300100	PAVEMENT MARKING REMOVAL	SQ FT	850
78300200	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	EACH	8
86200300	UNINTERRUPTIBLE POWER SUPPLY, EXTENDED	EACH	1
X0321430	BRIDGE APPROACH PAVEMENT CONNECTOR (PCC) SPECIAL	SQ YD	242
X0325305	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	311
Z0030250	IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2
Z0030350	IMPACT ATTENUATORS, RELOCATE (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2
Z0073700	TEMPORARY WALL BRACING SYSTEM	L SUM	1

* Specialty Items

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	PLOT DATE ≈ 7/1/2008	DATE	REVISED		SCALE: SHEET NO. 6 OF 48 SHEETS STA TO STA	FED. ROAD DIST. NO ILLINOIS FED. AID	

TYPICAL SECTION



TYPICAL SECTION OUTSIDE PAVEMENT REMOVAL

SECTION OUTSIDE GUARDRAIL LIMITS

SECTION WITHIN GUARDRAIL OR TRAFFIC BARRIER TERMINAL LIMITS

TO BE USED:

799+70 LT TO 800+84 LT 802+36 LT TO 804+50 LT

798+50 RT TO 800+84 RT

802+36 RT TO 803+58 RT

PROPOSED HOT MIX ASPHALT SURFACE COURSE, MIX "C", N90, 1 1/2" TON

PROPOSED HOT-MIX ASPHALT SHOULDERS, 10" SQ YD

EXISTING HOT-MIX ASPHALT SHOULDER (8" TYP.)

EXISTING GROUND LINE

PROPOSED HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2" SQ YD

EXISTING PCC PAVEMENT, 8"

EXISTING HOT-MIX ASPHALT SURFACE, 2 1/2"

EXISTING SUBBASE GRANULAR MATERIAL, 4"

PROPOSED EARTH EXCAVATION

EXISTING AGGREGATE SHOULDER

PROPOSED PAVED SHOULDER REMOVAL, SQ YD

NOTE 1: PROPOSED SLOPE SHALL BE 4:1 OR GREATER TO CLEAR ZONE AND 3:1 OR GREATER THEREAFTER.

NOTE 2: PROPOSED SLOPE 2.5:1 OR GREATER.

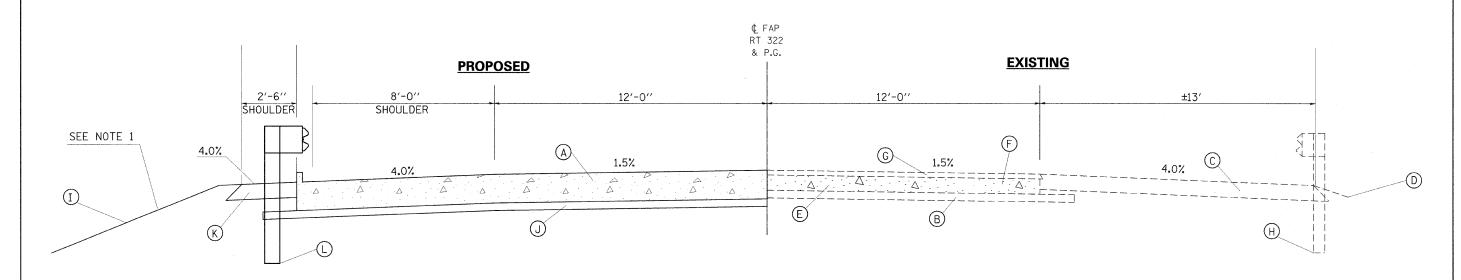
MIXTURE REQUIREMENTS

Location(s):	Hot-Mix Asphalt Surface Course
Mixture Use(s):	Hot-Mix Asphalt Surface Course, Mix C, N90
AC/PG:	PG64-22
RAP % (Max):	10
Design Air Voids:	4.0%, 90 Gyration Design
Mixture Composition:	IL-9.5 mm or IL12.5 mm
(Gradation Mixture)	
Friction Aggregate:	C Surface

Location(s):	Hot-Mix Asphalt Shoulders
Mixture Use(s):	Hot-Mix Asphalt Shoulders
AC/PG:	PG58-22
RAP % (Max):	50
Design Air Voids:	2.0%, 30 Gyration Design
Mixture Composition:	HMA Shoulders
(Gradation Mixture)	IMMA STIDUT Get S
Friction Aggregate:	None

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TYPICAL SECTION



TYPICAL SECTION WITHIN PAVEMENT REMOVAL

STA. 800+84 TO STA. 802+36

MIXTURE REQUIREMENTS

A	PROPOSED APPROACH OR CONNECTOR PAVEMENT
B) EXISTING SUBBASE GRANULAR MATERIAL, 4"
(C)) EXISTING HOT-MIX ASPHALT SHOULDER (8" TYP)
(D)) EXISTING GROUND LINE
Œ)) PROPOSED PAVEMENT REMOVAL SQYD
Ē) EXISTING PCC PAVEMENT, 8"
G) EXISTING HOT-MIX ASPHALT SURFACE, 2 1/2"
H) EXISTING GUARDRAIL
(I)) PROPOSED GROUND LINE
Ū) PROPOSED SUBBASE GRANULAR MATERIAL, 4" TON
(K)) PROPOSED HOT-MIX ASPHALT SHOULDER, 10" SQYD
(L)) PROPOSED GUARDRAIL
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USER NAME - shepardgd

PLOT DATE = 6/18/2008

PLOT SCALE = 50,0000 1/ IN.

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Mix Asphalt Surface Course, Mix C, N90 -22
-22
(, 90 Gyration Design
.5 mm or IL12.5 mm
face

Location(s):	Hot-Mix Asphalt Shoulders
Mixture Use(s):	Hot-Mix Asphalt Shoulders
AC/PG:	PG58-22
RAP % (Max):	50
Design Air Voids:	2.0%, 30 Gyration Design
Mixture Composition:	HMA Shoulders
(Gradation Mixture)	IUMA 211001 del 2
Friction Aggregate:	None

NOTE 1: PROPOSED SLOPE 2.5:1 OR GREATER OR MATCH EXISTING.

07177 OF HINDS				F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		TYPICAL SEC	TION	FAP 322	12-1,BR	UNION	48	8
DEFANIMENT OF INANSPORTATION	SCALE:	SHEET NO. 8 OF 48 SHEETS	STATO STA	FED. ROAD I	DIST. NO. ILLINOIS FED. A	CONTRACT ID PROJECT	T NO. 9	8886

PAVEMENT AND RESURFACING SCHEDULE

	HOT-MIX	PRIME	COAT	HOT-MIX	HOT-MIX ASPHALT	PAVED SHOULDER	TEMPORARY	PAVEMENT
LOCATION STATION TO STATION	ASPHALT SURFACE COURSE	BITUMINOUS MATERIALS	AGGREGATE	ASPHALT SHOULDERS, 10″	SURFACE REMOVAL 1 1/2"	REMOVAL	RAMP	REMOVAL
	TON	GAL	TON	SQ YD	SQ YD	SQ YD	SQ YD	SQ YD
FAP RT. 322(US 51)								
FAI 1(1: 322(US 317								
798+50 TO 799+40 RT				A 24 - 4 - 4 - 4 - 4 - 4 - 4 - 4 - 4 - 4		40.0		
799+40 TO 800+00 RT						57.0		
800+00 TO 800+84 RT						122.3		
800+84 TO 801+20 RT						14.0		
801+62 TO 802+36 RT						34.1		
802+36 TO 802+76 RT						63.6		
802+76 TO 803+29 RT						57.4		
803+29 TO 803+58 RT						14.5		
799+70 TO 800+00 LT						14.3		
800+00 TO 800+50 LT						52. 5		
800+50 TO 800+90 LT						59.6		
800+90 TO 800+90 LT						26.1		
801+90 TO 801+31 LT						17. 9		
						33. 3		
802+36 TO 802+60 LT								
802+60 TO 803+00 LT			N-84-8-14			58.9		
803+00 TO 803+96 LT						96.0		
803+96 TO 804+50 LT						24.0		
799+70 TO 799+90 LT				20.0				
799+90 TO 800+50 LT				83. 3				
800+50 TO 800+84 LT				41.6				
• 800+84 TO 801+57 LT				20.3				247.6
801+98 TO 802+36 LT				10.6				
802+36 TO 803+00 LT	14.3	22.4	0.3	78. 2	170.7			
803+00 TO 804+30 LT	29.1	47.5	0.5	180.6	346. 7			
804+30 TO 804+50 LT	4.5	6. 6	0.1	20.0	53. 3		13.5	
798+50 TO 798+73 RT	5. 2	7.6	0.1	23.0	61.3	and the subsection with the contract of the subsection of the subs	13.5	
798+73 TO 799+43 RT	15. 7	25.6	0.3	97. 2	186. 7			
799+43 TO 800+84 RT	32. 9	50.8	0.6	172.3	376.0			
800+84 TO 801+21 RT				10.3				
* 801+62 TO 802+36 RT				20.6				247.6
802+36 TO 802+67 RT				37.9				
802+67 TO 803+34 RT				93. 1				
803+34 TO 803+58 RT				24.0	THE STATE CONTROL OF THE STATE	CONTROL OF THE STATE OF THE STA		
TOTALS	102	160	2	933	1195 TO 801+39. 7 AND STATIO	785	27	495

FILE NAME =	USER NAME = shepardgd	DESIGNED	REVISED			F.A.P.	SECTION	COUNTY TO	OTAL SHEET
c:\projects\d902904\d902904ms.dgn		DRAWN	REVISED	STATE OF ILLINOIS	PAVEMENT AND RESURFACING SCHEDULE	FAP 322	12-1.BR	UNION 4	48 9
	PLUT SCALE = 50.0000 // IN.	CHECKED	REVISED	DEPARTMENT OF TRANSPORTATION				CONTRACT NO	NO. 98886
	PLOT DATE = 7/1/2008	REVISED		SCALE: SHEET NO. 9 OF 48 SHEETS STA TO STA	FED. ROAD DIST. N	AID PROJECT			

GUARDRAIL SCHEDULE

LOCATION STATION TO STATION	LENGTH (FOR INFORMATION ONLY)	GUARDRAIL REMOVAL	STEEL PLATE BEAM GUARDRAIL, TYPE A	TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (TANGENT)	TRAFFIC BARRIER TERMINAL, TYPE 6	TERMINAL MARKER DIRECT APPLIED
	FEET	FOOT	FOOT	EACH	EACH	EACH
FAP RT. 322(US 51)						
798+93.75 TO 799+43.75 RT	50.00			1		1
799+43.75 TO 800+81.25 RT	137.50		137.5			
800+06.00 TO 801+25.00 RT	119.00	119				
800+81.25 TO 801+25.00 RT	43.75				1	
801+63.00 TO 802+06.75 RT	43. 75				1	
801+63.00 TO 802+77.00 RT	114.00	114		N-900		
802+06.75 TO 802+69.25 RT	62, 50		62.5			
802+69.25 TO 803+19.25 RT	50.00			1		1
800+00.75 TO 800+50.75 LT	50,00			1		1
800+50.75 TO 801+13.25 LT	62, 50		62.5			
800+43.00 TO 801+57.00 LT	114.00	114				
801+13.25 TO 801+57.00 LT	43.75				1	
801+95.00 TO 802+38.75 LT	43. 75				1	2 10
801+95.00 TO 803+13.00 LT	118.00	118				
802+38.75 TO 803+63.75 LT	125.00		125.0			
803+63.75 TO 804+13.75 LT	50.00	***************************************		1		1

TOTALS		465	387.5	4	4	4

EARTHWORK SCHEDULE

LOCATION STAGE OF CONSTRUCTION	EARTH EXCAVATION	EMBANKMENT	CHANNEL EXCAVATION (UNSUITABLE)	SHRINKAGE FACTOR	EXCAVATION ADJUSTED FOR SHRINKAGE	EXCAVATION REQUIRED TO COMPLETE	BORROW EXCAVATION*
	CU YD	CU YD	CU YD		CU YD	CU YD	CU YD
FAP RT. 322(US 51)							
STAGE I	65	72	291	0. 24	49	23	23
TOTALS	65		291				23

• A SWELL FACTOR OF 1.18 WAS USED TO CALCULATE BORROW EXCAVATION.

SWELL FACTORS ARE FOR INFORMATION ONLY.

FILE NAME =	USER NAME = shepardgd	DESIGNED	REVISED			F.A.P. SECTION	COUNTY TOTAL SHEET
c:\projects\d902904\d902904ms.dgn		DRAWN	REVISED	STATE OF ILLINOIS	GUARDRAIL AND EARTHWORK SCHEDULES	FAP 322 12-1,BR	UNION 48 10
	PLOT SCALE = 50.0000 '/ IN.	CHECKED	REVISED -	DEPARTMENT OF TRANSPORTATION			CONTRACT NO. 98886
	PLOT DATE = 6/18/2008	DATE -	REVISED		SCALE: SHEET NO. 10 OF 48 SHEETS STA TO STA.	FED. ROAD DIST. NO ILLINOIS FED.	AID PROJECT

SEEDING AND EROSION CONTROL SCHEDULE

LOCATION	SEEDING CLASS 2	SEEDING CLASS 7	NITRO FERTILIZER	OGEN NUTRIENTS	PHOSPHORUS FERTILIZER	POTASSIUM FERTILIZER	AGRICULTURAL GROUND	MULCH METHOD 2	MULCH METHOD 2	TEMPORARY EROSION CONTROL
STATION TO STATION	(MODIFIED)	(TEMPORARY)	CLASS 2	CLASS 7	NUTRIENTS	NUTRIENTS	LIMESTONE	FOR CLASS 2	FOR CLASS 7	SEEDING
	ACRE	ACRE	POUND	POUND	POUND	POUND	TON	ACRE	ACRE	POUND
FAP RT. 322(US 51)										
798+55.00 TO 800+84.00 RT	0.1	0.1	12	4	12	12	0. 2	0.1	0, 1	20
801+66.00 TO 803+40.00 RT	0.1	0.1	12	4	12	12	0.2	0.1	0.1	20
798+69.00 TO 801+53.00 LT	0.1	0. 1	12	4	12	12	0. 2	0.1	0.1	20
802+33.00 TO 804+43.00 LT	0.1	0. 1	12	4	12	12	0.2	0.1	0.1	20
	0.50	0.50	48	16	48	48	1	0.50	0.50	80
			6	4		,		1.	00	

PAVEMENT MARKING SCHEDULE

LOCATION	LENGTH (FOR INFORMATION	PAVEMENT MARKING	TEMPORARY PAVEMENT MARKING LINE - 4"		PAINT PAVEMENT MARKING - LINE 4"		SHORT TERM PAVMENT	WORK ZONE PAVEMENT MARKING	RAISED REFLECTIVE PAVEMENT MARKERS	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	
STATION TO STATION	ONLY)	REMOVAL	WHITE	YELLOW	WHITE	YELLOW	MARKING	REMOVAL	TAVENENT MARKERS	TAVEMENT MARKER REMOVAL	
	FEET	SQ FT	FOOT	FOOT	FOOT	FOOT	FOOT	SQ FT	EACH	EACH	
FAP RT. 322(US 51)											
798+50 TO 804+50	600.0	850.0	1200.0	150.0	1200.0	150.0	102.5	34. 2	8	8	
TOTA	LS	850	1200	150	1200	150	103	34	8	8	
			13	350	1	350					

FILE NAME =	USER NAME = shepardgd	DESIGNED	REVISED
c:\projects\d902904\d902904ms.dgn		DRAWN	REVISED
	PLOT SCALE = 50.0000 '/ IN.	CHECKED	REVISED
·	PLOT DATE: = 5/18/2008	DATE	REVISED

						F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PAVEMENT MAR	KING AND	SEEDING AND	EROSION	CONTROL SCHED	ULES	FAP 322	12-1 , BR	UNION	48	11
								CONTRAC	T NO. 9	98886
SCALE:	SHEET NO. 11	OF 48 SHEETS	STA	TO STA		FED. ROAD I	DIST. NO ILLINOIS FED. AI	D PROJECT		

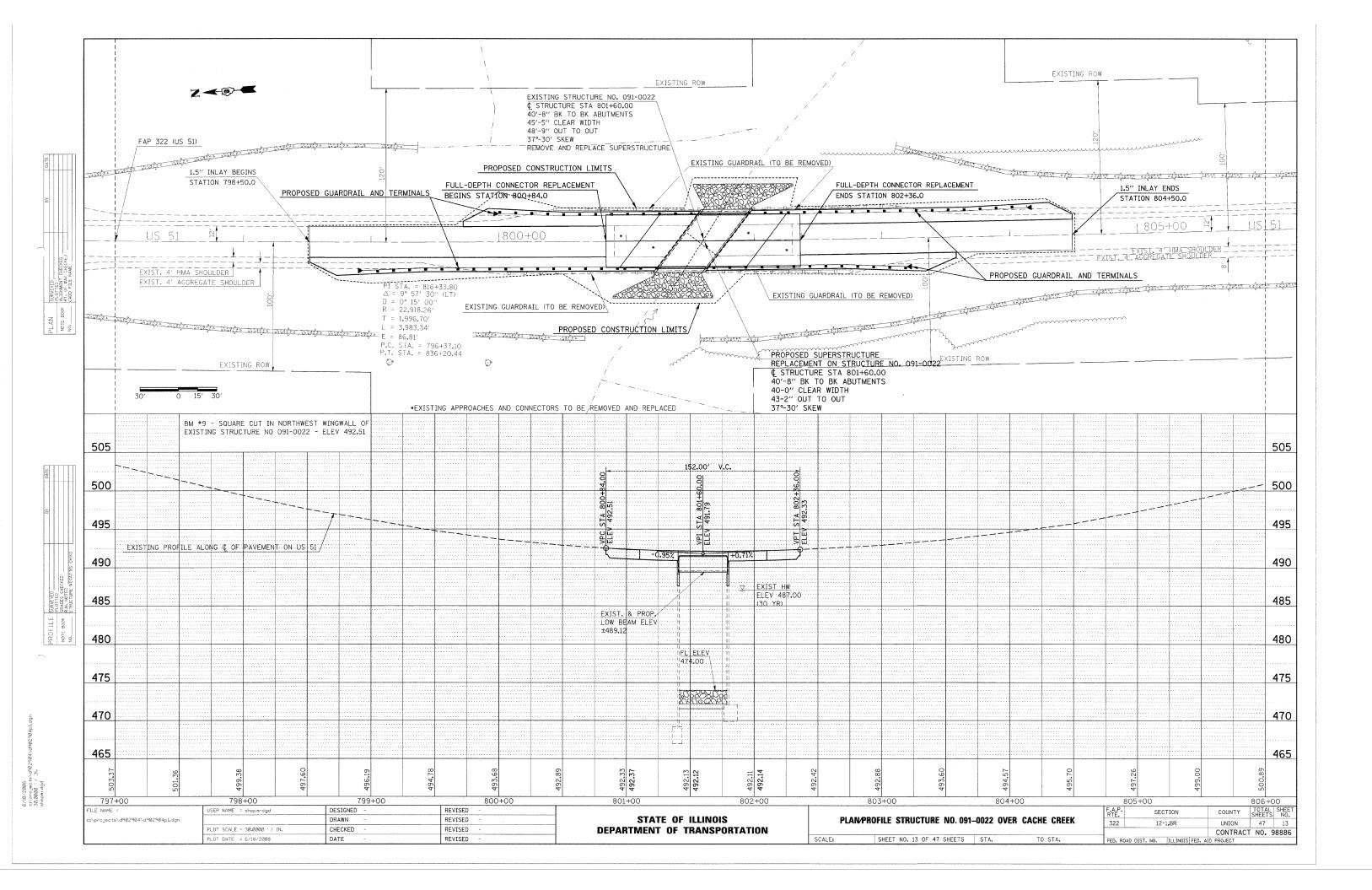
TEMPORARY CONCRETE BARRIER SCHEDULE

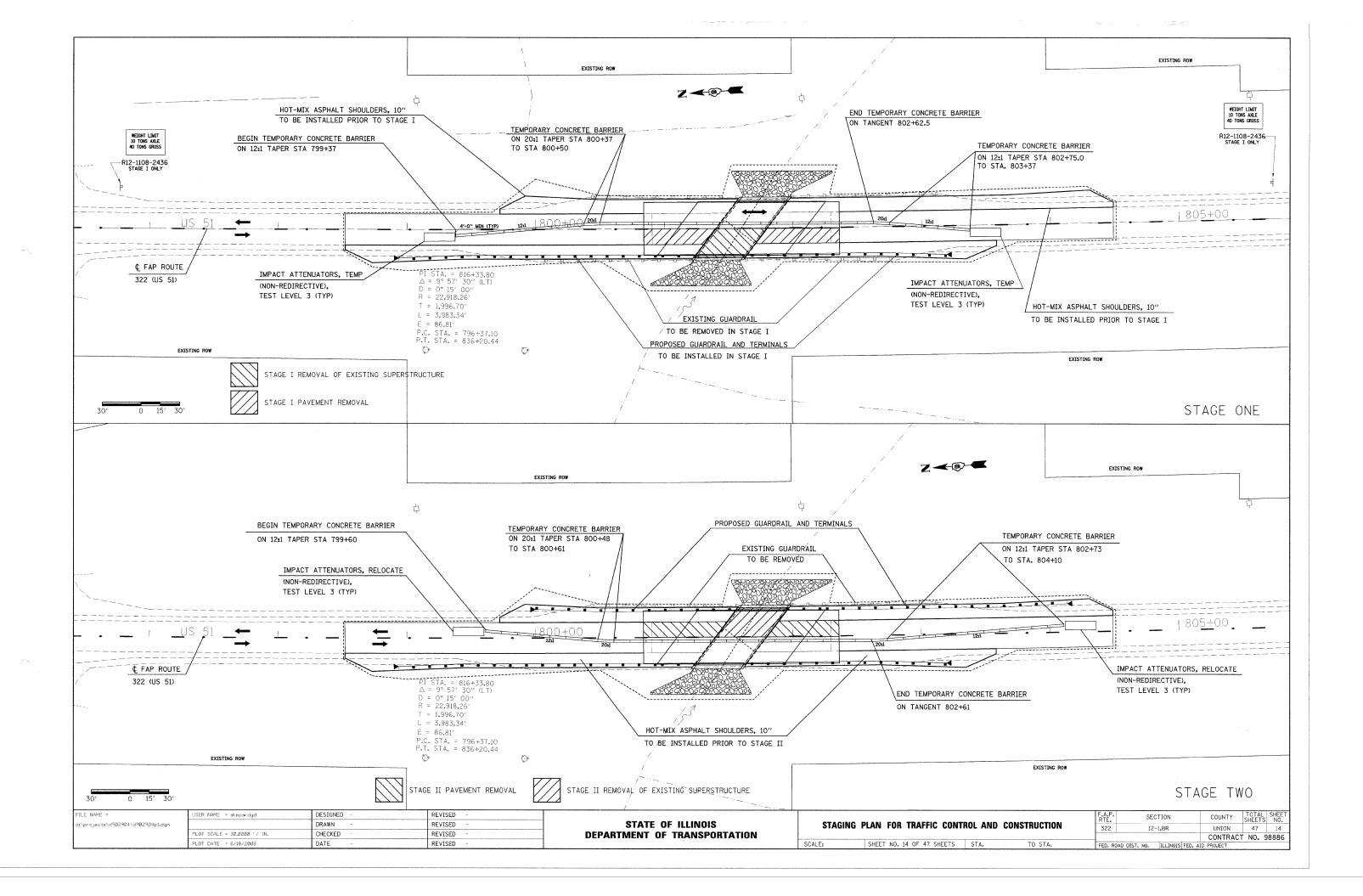
LOCATION STATION TO STATION	NUMBER OF BARRIER (FOR INFORMATION ONLY)	TEMPORARY CONCRETE BARRIER	RELOCATE TEMPORARY CONCRETE BARRIER	COMMENTS
	EACH	FOOT	F00T	
FAP RT. 322(US 51)				
	VIII.A.V. 11.3.V. A. V. V. T. T. V. V. V. T. V.			
STAGE I				
799+37 TO 800+37	8	100.0		12:1 TAPER
800+37 TO 800+50	1	12.5		20: 1 TAPER
800+50 TO 802+63	17	212.5		TANGENT
802+63 TO 802+75	1	12.5		20:1 TAPER
801+62 TO 803+37	5	62.5		12:1 TAPER
STAGE II				
799+60 TO 800+48	7		87.5	12:1 TAPER
800+48 TO 800+61	1		12.5	20: 1 TAPER
800+61 TO 802+61	16		200.0	TANGENT
801+98 TO 802+73	1		12.5	20:1 TAPER
802+73 TO 804+10	11	50.0	87. 5	12:1 TAPER
TO	TALS	450	400	

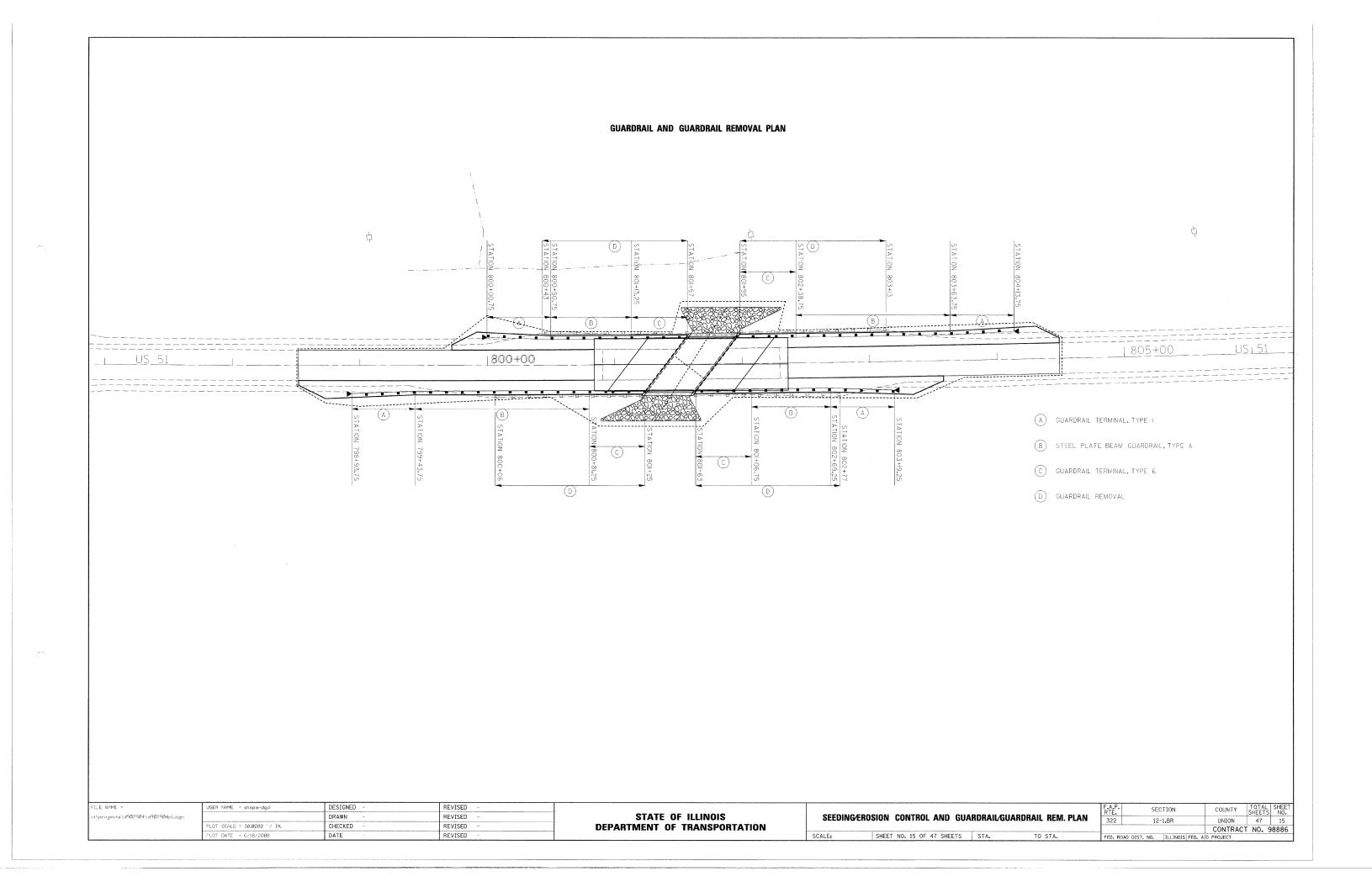
FILE NAME =	USER NAME = shepardgd	DESIGNED	REVISED
ci\projects\d902904\d902904ms.dgn		DRAWN	REVISED
	PLOT SCALE = 50.0000 '/ IN.	CHECKED	REVISED
	PLOT DATE = 6/18/2008	DATE	REVISED

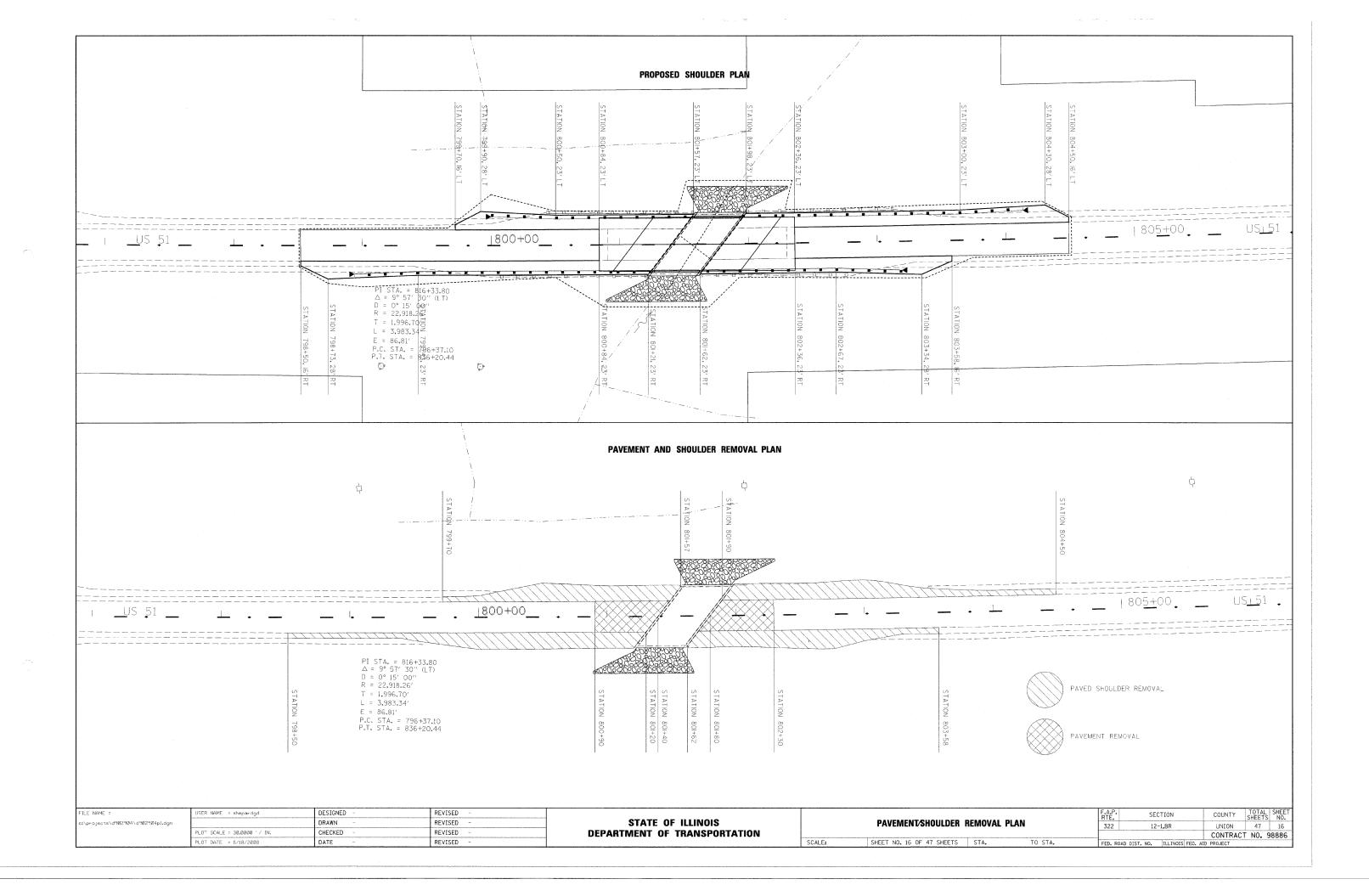
-	TEMPORARY CONCRETE BARRIER SCHEDULE							F.A.	TÈ.			
		TEMPURANT CUNGRETE BARKIER SCHEDULE								FAP	322	
	SCALE:	SHEET	NO. 1	2 OF	48	SHEETS	STA.	TO	STA.		FED.	ROA

F.A.P.	E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FAP 3	322	12-1 , BR	UNION	48	12
_			CONTRACT	NO. S	8886
		TOT US IT THE STORE A	CONTRACT	NO. S	36









Bench Mark: B.M. 9 is chiseled square on the N.W. wingwall of S.N. 091-0022 at Sta. 801+19.2, 26.5 feet right of U.S. 51. Elev. 492.51

Elev. 486.38

Temporary Wall Bracing System

(to be designed by the Contractor).

**Since radius is so large

build bridge straight.

Curve Data for

information only

PASSED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

Existing Structure: S.N. 091-0022 Built 1962 as F.A. Route 2, Section 12-1B at Station 801+60 as a simple span reinforced concrete channel Nelson beams 40'-4" Bk.-to-Bk. abutments 48'-9" 0.-0. deck. Closed abutments supported on spread footings. The existing superstructure is to be removed and replaced. Traffic to be maintained utilizing stage construction.

INDEX OF SHEETS

- 1. General Plan and Elevation
- 2. General Details
- 3. Temporary Concrete Barrier
- 4. Stage Construction Details 5. Top of Slab Elevations
- 6. Top of Slab Elevations
- 7. North Approach Elevations
- 8. South Approach Elevations
- 9. Superstructure 10. Superstructure Details
- 11. Diaphragm Details
- 12. Structural Steel
- 13. Structural Steel Details 14. Concrete Removal N. Abutment
- 15. Concrete Removal S. Abutment
- 16. North Abutment
- 17. South Abutment
- 18. Temporary Support System
- 19. Bar Splicer Assembly Details
- 20. Cantilever Formina Bracket

ELEVATION Stone Riprap Class A4 *** 30 Year H.W. Elev. = 487.0 from 1961 design plans. Limits of Existing -Structure Bk. of North Bk. of South Exist. Abut. Exist. Abut. Temporary Sta. 801+39.83 Sta. 801+80.17 Elev. 492.15 Elev. 492.10 Pilng typ. € U.S. 51 & -Stage Const. Line Sta. 801+60.00 💯 30' Bridge Appr. Pav't. Std. 420401 typ. 3 spa. @ ±13'-5" = 40'-4" 6" & Floor **EXISTING CURVE DATA drain spacing PI Sta. = 816+33.80 Δ = 9°-57'-30" (LT) 40'-4" Existing

Rk. to Rk. Abutments

+0.71%

V.P.I. Elev.

v.c. = 152'

PROFILE GRADE

(alona @ U.S. Rte. 51)

PLAN

EXPIRES 11-30-2008

Streambed

Elev. 474.00

Traffic Barrier Terminal

Std. 631031, Type 6 typ.

Elev. 486.37

STATION 801+60.00 REBUILT 20 BY STATE OF ILLINOIS F.A.P. RTE, 322 SEC, 12-1, BR LOADING HL93 STRUCTURE NO. 091-0022

NAME PLATE

Existing nameplate shall be cleaned and incorporated next to new name plate. Cost included with Name Plates.

DESIGN STRESSES

FIELD UNITS (New Construction)

- 3,500 psi
- 60,000 psi (reinforcement)
- 50,000 psi (AASHTO M270 Gr. 50) = 36,000 psi (AASHTO M270 Gr. 36)

LOADING HL-93

(Superstructure only)

Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

2007 AASHTO LRFD Bridge Design Specifications

U.S. 4th. Edition

- FIELD UNITS (Existing Construction)
- 1,400 psi (super.)
- 1,000 psi (sub.)

$f_s = 20,000 \text{ psi (reinforcement)}$ SEIS

MIC	DATA	
	- 7 (CD7)	

Seismic Performance Zone (SPZ) = 2 Bedrock Acceleration Coefficient (A) = 0.14g Site Coefficient (S) = 1.2

	ROUTE NO.	SECTION	cou	INTY	TOTAL SHEETS		SHEET NO.
	F.A.P. 322	12-1, BR	UN	ION	36	17	20 SHEETS
1	FED. ROAD DIST. NO. 7		ILLINOIS FED. AID PR		DJECT-		

Contract #98886

GENERAL NOTES

Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts. Bolts $\frac{3}{4}$ in. ϕ , holes $\frac{15}{16}$ in. ϕ , unless otherwise noted. Calculated weight of Structural Steel = 2,090 (AASHTO M270 Gr. 36)

= 20,330 (AASHTO M270 Gr. 50)

HEET NO.

No field welding is permitted except as specified in the contract documents. Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished based upon the unit price bid for the work.

Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

The concrete for bridge decks finished according to Article 503.16(a) of the Standard Specifications shall be placed and compacted parallel to the skew in uniform increments along centerline of bridge. The machine used for finishing shall be set parallel to the skew for striking off and screeding the concrete.

The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Pilina.

Slip forming of the parapets is not allowed on this contract.

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60 (IL Modified). See Special Provisions

Reinforcement bars designated (E) shall be epoxy coated.

The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Interstate Green, Munsell No. 7.5G 4/8. See Special Provision for "Cleaning and Painting New Metal Structures".

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment (Special)	Cu. Yd.		97	97
Stone Riprap, Class A4	Sq. Yd.		416	416
Filter Fabric	Sq. Yd.		416	416
Protective Coat	Sq. Yd.		208	208
Removal of Existing Superstructures	Each	1		1
Concrete Removal	Cu. Yd.	18.8		18.8
Structure Excavation	Cu. Yd.		97	97
Floor Drains	Each	4		4
Concrete Structures	Cu. Yd.		22.1	22.1
Concrete Superstructure	Cu. Yd.	68.9		68.9
Bridge Deck Grooving	Sq. Yd.	164.8		164.8
Furnishing and Erecting Structural Steel	L. Sum	1 .		1
Stud Shear Connectors	Each	1,116		1,116
Reinforcement Bars, Epoxy Coated	Pound	13,500	3,940	17,440
Bar Splicers	Each	229	16	245
Temporary Sheet Piling	Sq. Ft.		<i>1</i> 95	<i>1</i> 95
Name Plates	Each	1		1
Anchor Bolts, 1'' ø	Each	24		24
Geocomposite Wall Drain	Sq. Yd.		31	31
Pipe Underdrains for Structures, 4''	Foot		152	152
Structural Repair of Concrete < 5''	Sq. Ft.		311	311
Temporary Wall Bracing System	L. Sum		1	1
Furnishing and Erecting Structural Steel	Pound		25,750	25,750



GENERAL PLAN & ELEVATION U.S. ROUTE 51 OVER CACHE CREEK F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

D = 0°-15'-00"

R = 22,918.26'

T = 1,996.70'

L = 3,983.34'

P.C. Sta. = 796+37.10

P.T. Sta. = 836+20.44

DESIGNED POTULA METAL

CHECKED Jay D. Edward

CHECKED PMP JOE

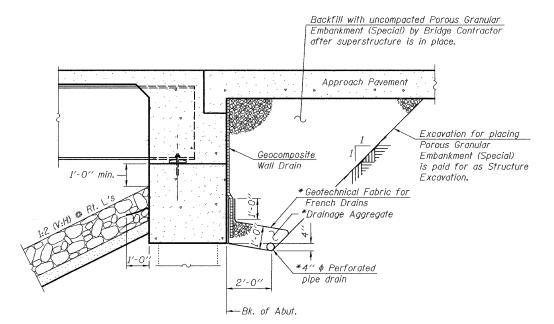
WOCUGOF DRAWN BECKY M. LEACH

E = 86.81'

No salvage

ROUTE NO.	SECTION	cor	JNTY	TOTAL SHEETS	SMEET NO.	SHEET	NO.	2
F.A.P. 322	12-1, BR	UNION		36	18	20 sнв	ETS	
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PR	OJECT-				

Contract #98886



SECTION THRU INTEGRAL ABUTMENT (Horiz. dim. @ Rt. L's)

* Included in the cost of Pipe Underdrains for Structures.

Note.

See sheets 16 and 17 of 20 for Drainage System Detail at outside edge of abutments.

Stone Riprap, 3'-0"
Class A4

Bedding 4'-0"
Filter fabric

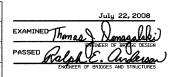
SECTION A-A

DESIGNED Patrick M. Petrone
CHECKED Jay D. Edwards

DRAWN BECKY M. LEACH

CHECKED PMP/JDE

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GENERAL DETAILS

F.A.P. ROUTE 322 - SECTION 12-1, BR

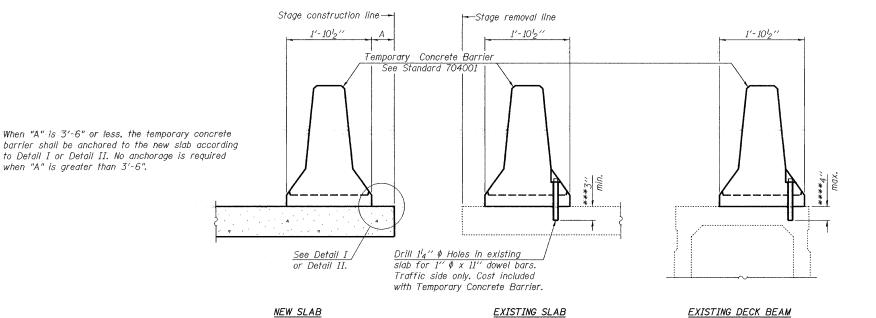
UNION CO.

STATION 801+60.00

STRUCTURE NO. 091-0022

ROUTE NO.	SECTION	cou	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 3
F.A.P. 322	12-1. BR	UN	ION	36	19	20sheets
FED. ROAD DIST	F. NO. 7	ILLINOIS	FEO. AID PR	OJECT-		

Contract #98886



NOTES

Detail I - With Bar Splicer or Couplers: Connect one (1) 1"x7"x10" steel £ to the top layer of couplers with 2-58" \$\phi\$ bolts

screwed to coupler at approximate & of each barrier panel.

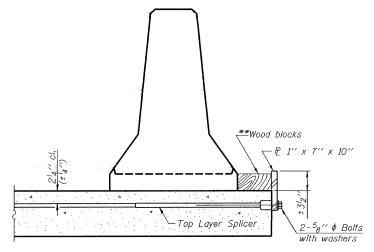
Detail II - With Extended Reinforcement Bars: Connect one (1) 1"x7"x10" steel 12 to the concrete slab or concrete wearing surface with 2-5₈"\$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate © of each barrier panel.

Cost of anchorage is included with Temporary Concrete Barrier. The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

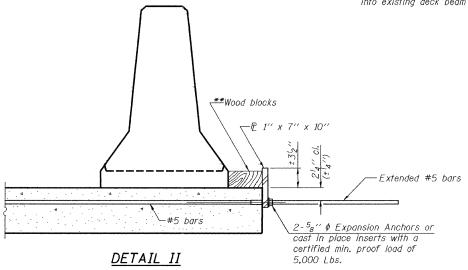
***Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.

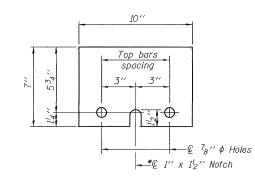


DETAIL I

when "A" is greater than 3'-6".



**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



STEEL RETAINER & 1" x 7" x 10"

* Required only with Detail II

TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

DESIGNED Patrick M. Petrone CHECKED Jay D. Edwards DRAWN BECKY M. LEACH CHECKED PMP/JDE

9-3-07

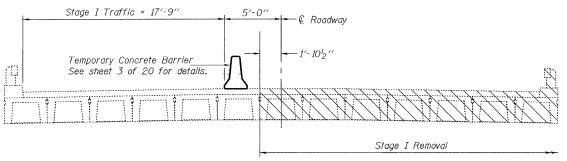
...\projects\bml00029\0910022.dgn 07/22/2008 11:20:54 AM

R-27

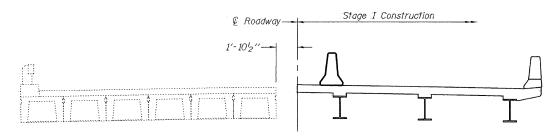


20 SHEETS

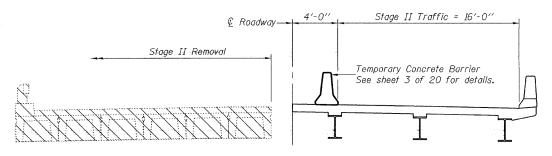
Contract #98886



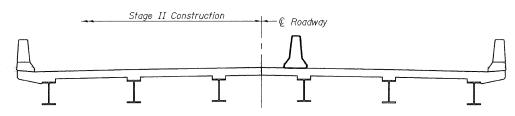
STAGE I REMOVAL



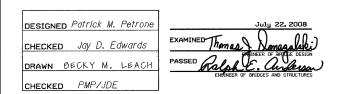
STAGE I CONSTRUCTION

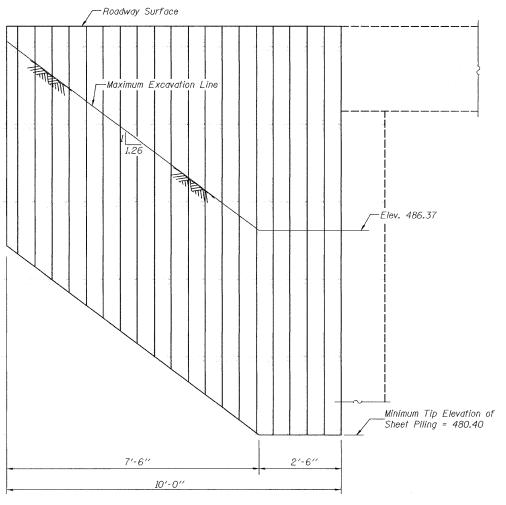


STAGE II REMOVAL



STAGE II CONSTRUCTION





TEMPORARY SHEET PILING

Dimensions parallel to & of Roadway

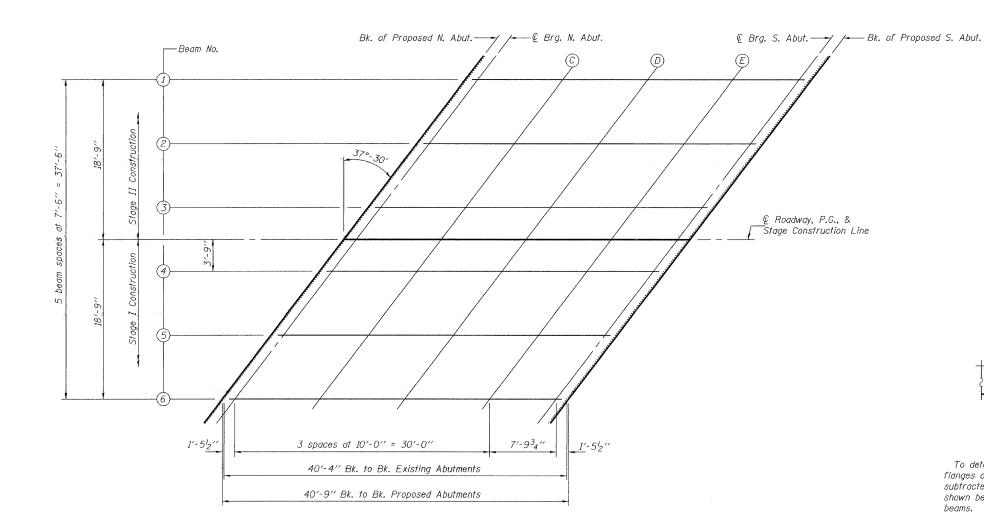
If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer.

Notes:

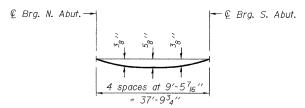
All cross sections are looking South. The removal of the existing biluminous wearing surface and the bridge rail is included in the Removal of Existing Superstructures.

STAGE CONSTRUCTION DETAILS F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

ROUTE NO.	SECTION	cou	NTY	SHEETS	SHEET NO.	SHEET NO.
F.A.P. 322	12-1, BR	UNI	ON	36	21	20 SHEETS



PLAN

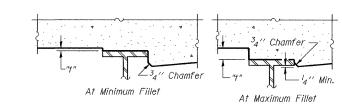


Contract #98886

DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on sheet 6 of 20.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown on sheet 6 of 20. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "t" above top flange of

FILLET HEIGHTS

DESIGNED Patrick M. Petrone CHECKED Jay D. Edwards DRAWN BECKY M. LEACH

CHECKED PMP/JDE

TOP OF SLAB ELEVATIONS F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

ROUTE NO.	950710N	co	INTY	TOTAL SHEETS	SHEET NO.	SHEE
F.A.P. 322	12-1, BR	UNION		36	22	20:
FEO. ROAD DIST	. NO. 7	ILLINOTS	PED, AID PR	DJECT-		1

20 SHEETS

Contract #98886

<u>BEAM 1</u>

In the second	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
	Bk. of Exist. N. Abut.	80154.22	-18.75	491.79	491.79
	₡ Brg. N. Abut.	80155.48	-18.75	491.78	491.78
	C D E	80165.48 80175.48 80185.48	-18.75 -18.75 -18.75	491.77 491.77 491.78	491.81 491.82 491.81
	₡ Brg. S. Abut.	80193.29	-18.75	491.80	491.80
	Bk. of Exist. S. Abut.	80194.55	-18.75	491.80	491.80

BE	A	М	2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of Exist. N. Abu	80148.47	-11.25	491.95	491.95
€ Brg. N. Abu	80149.73	-11.25	491.95	491.95
	C 80159.73 D 80169.73 E 80179.73	-11.25 -11.25 -11.25	491.93 491.92 491.93	491.96 491.97 491.95
© Brg. S. Abu	80187.54	-11.25	491.94	491.94
Bk. of Exist. S. Abu	80188.80	-11.25	491.94	491.94

BEAM 3

<u> </u>								
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade = Elevations Adjusted For Dead Load Deflection				
Bk. of Exist. N. Abut.	80142.71	-3.75	492.08	492.08				
€ Brg. N. Abut.	80143.97	-3.75	492.08	492.08				
C D E	80153.97 80163.97 80173.97	-3.75 -3.75 -3.75	492.06 492.04 492.04	492.09 492.09 492.07				
© Brg. S. Abut.	80181.78	-3.75	492.05	492.05				
Bk. of Exist. S. Abut.	80183.04	-3.75	492.05	492.05				

€ ROADWAY, PROFILE GRADE, & STAGE CONST. JOINT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of Exist. N. Abut.	80139.83	0.00	492.15	492.15
© Brg. N. Abut.	80141.09	0.00	492.15	492.15
C D E	80151.09 80161.09 80171.09	0.00 0.00 0.00	492.12 492.10 492.10	492.15 492.15 492.13
© Brg. S. Abut.	80178.91	0.00	492.10	492.10
Bk. of Exist. S. Abut.	80180.17	0.00	492.10	492.10

BEAM 4

	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
150	Bk. of Exist. N. Abut.	80136.96	3.75	492.10	492.10
	Brg. N. Abut.	80138.22	3.75	492.10	492.10
	C D E	80148.22 80158.22 80168.22	3.75 3.75 3.75	492.07 492.05 492.04	492.10 492.09 492.07
	₡ Brg. S. Abut.	80176.03	3.75	492.04	492.04
	Bk. of Exist. S. Abut.	80177.29	3.75	492.04	492.04

BEAM 5

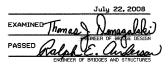
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of Exist. N. Abut.	80131.20	11.25	492.01	492.01
€ Brg. N. Abut.	80132.46	11.25	492.00	492.00
C D E	80142.46 80152.46 80162.46	11.25 11.25 11.25	491.97 491.94 491.93	492.00 491.99 491.95
€ Brg. S. Abut.	80170.27	11.25	491.92	491.92
Bk. of Exist. S. Abut.	80171.53	11.25	491.92	491.92

BEAM 6

	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk.	of Exist. N. Abut.	80125.45	18.75	491.88	491.88
	Brg. N. Abut.	80126.71	18.75	491.88	491.88
	C D E.	80136.71 80146.71 80156.71	18.75 18.75 18.75	491.84 491.80 491.78	491.87 491.85 491.81
	Brg. S. Abut.	80164.52	18.75	491.77	491.77
Bk.	of Exist. S. Abut.	80165.78	18.75	491.77	491.77

DESIGNED Patrick M. Petrone
CHECKED Jay D. Edwards

DRAWN BECKY M. LEACH
CHECKED PMP/JDE



TOP OF SLAB ELEVATIONS

F.A.P. ROUTE 322 - SECTION 12-1, BR

UNION CO.

STATION 801+60.00

STRUCTURE NO. 091-0022

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHE
F.A.P. 322	12-1. BR	UNION		36	23	20
FED. ROAD DIST	. NO. 7	R.LINOIS	FED. AID PR	OJECT-		

20 SHEETS

Contract #98886

EAST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
End of N. Appr, Pav't	80125.60	-20.00	491.86	
A B	80135.60 80145.60		491.81 491.78	
Bk. of Exist. N. Abut.	80155.60	-20.00	491.76	

EAST EDGE OF PAVEMENT

. 1	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
	End of N. Appr. Pav't	80119.46	-12.00	492.06	
		80129.46 80139.46		492.01 491.97	
	Bk. of Exist. N. Abut.	80149.04	-12.00	491.94	

@ ROADWAY, P.G. & STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
End of N. Appr. Pav't	80110.25	0.00	492.30	
A ² B	80120.25 80130.25	0.00	492.24 492.19	
Bk. of Exist. N. Abut.	80139.83	0.00	492.15	

WEST EDGE OF PAVEMENT

	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
, c.	End of N. Appr. Pav't	80101.04	12.00	492.18	
	A B	80111.04 80121.04		492.11 492.05	
	Bk. of Exist. N. Abut.	80130.63	12.00	492.00	

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
End of N. Appr. Pav't	80094.90	20.00	492.06	
A B	80104.90 80114.90		491.99 491.92	
Bk. of Exist. N. Abut.	80124.49	20.00	491.86	

NORTH APPROACH
TOP OF SLAB ELEVATIONS

F.A.P. ROUTE 322 - SECTION 12-1, BR
UNION CO.

<u>STATION 801+60.00</u> <u>STRUCTURE NO. 091-0022</u>

	End of North approach pavement (A) (B) (B)
	East edge for pavement
7007	Back of Existing North Abut. © Roadway, P.G. & Stage Const. Line
70-07	West edge of pavement
	2 Spaces at 10'-0" = 20'-0" 9'-7"
	PLAN

DESIGNED Patrick M. Petrone
CHECKED Jay D. Edwards
DRAWN BECKY M. LEACH

CHECKED PMP/JDE

EXAMINED Thomas Lamagaski

PASSED Rules OF BRIDGES AND STRUCTURES

ROUTE NO.	SECTION	cou	NTY	TOTAL SHEETS	SHEET NO.	SHE
F.A.P. 322	12-1, BR	UN	ION	36	24	20
FED. ROAD DIST	. NO. 7	BLLINOIS	FED. AID PR	OJECT-		

SHEET NO. 8 20 SHEETS

Contract #98886

EAST CURB LINE

No. 19 - 19 - 19 - 19 - 19 - 19 - 19 - 19					
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding	
Bk. of Exist. S. Abut.	80195.51	-20.00	491.78		
F G	80205.51 80215.51		491.81 491.85	-	
End of S. Appr. Pav't	80225.10	-20.00	491.91		

EAST EDGE OF PAVEMENT

 Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. of Exist. S. Abut.	80189.37	-12.00	491.93	
F	80199.37 80209.37	-12.00 -12.00	491.96 491.99	-
End of S. Appr. Pav't	80218.96	-12.00	492.04	

@ ROADWAY, P.G. & STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. of Exist. S. Abut.	80180.17	0.00	492.10	
 F G	80190.17 80200.17		492.12 492.15	
End of S. Appr. Pav't	80209.75	0.00	492.18	

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. of Exist. S. Abut.	80170.96	12.00	491.91	
F G	80180.96 80190.96		491.92 491.93	
End of S. Appr. Pav't	80200.54	12.00	491.96	

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. of Exist. S. Abut.	80164.82	20.00	491.75	
F G	80174.82 80184.82		491.75 491.76	
End of S. Appr. Pav't	80194.40	20.00	491.78	

SOUTH APPROACH

TOP OF SLAB ELEVATIONS

F.A.P. ROUTE 322 - SECTION 12-1, BR

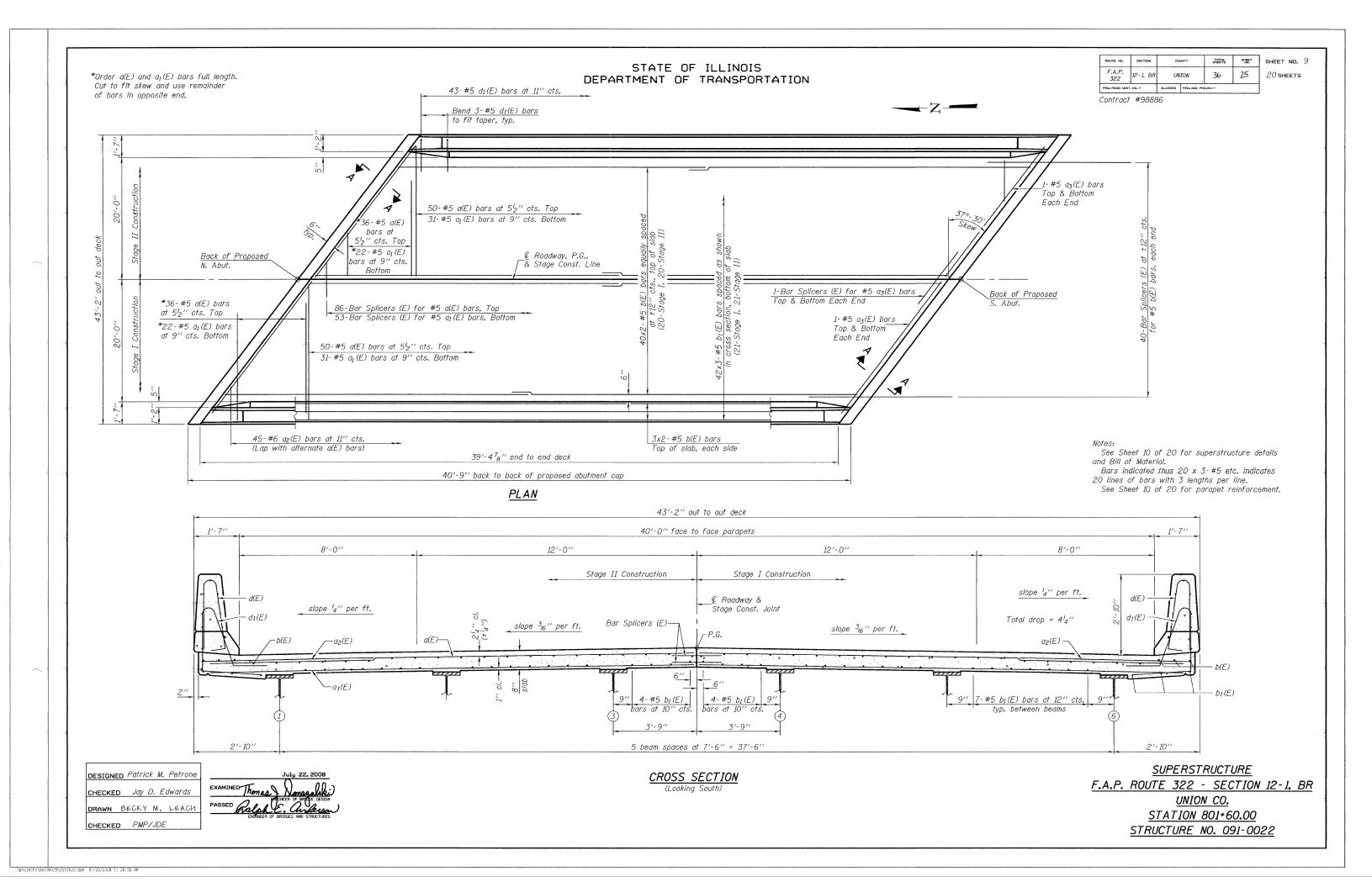
<u>UNION CO.</u> STATION 801+60.00 STRUCTURE NO. 091-0022

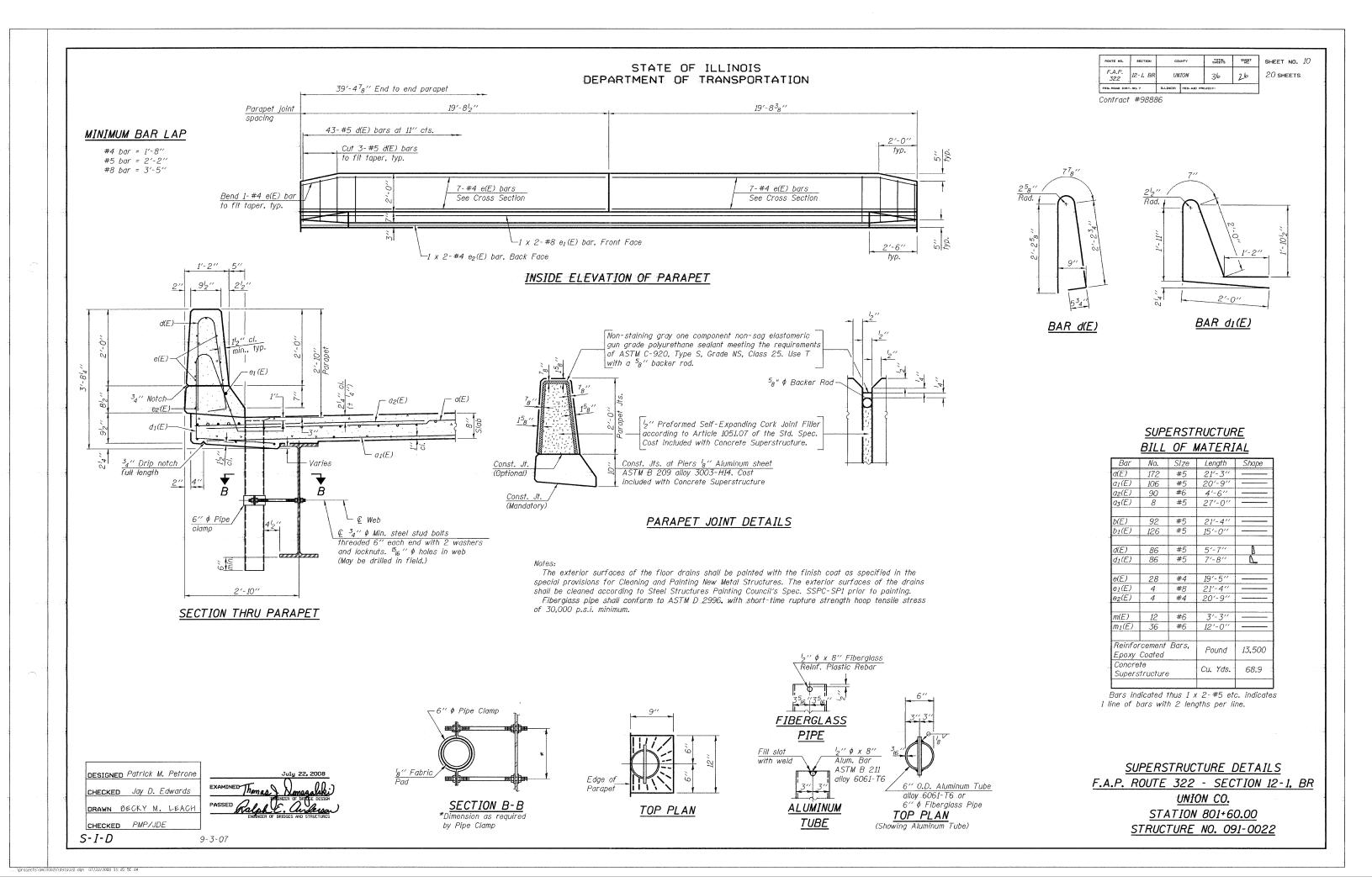
	End of South approach pavement
8′-0″	(E) (G)East curb line
12'-0"	Back of Existing South Abut. © Roadway, P.G. — & Stage Const. Line
12-0"	West edge of pavement
,,0-,8	
	PLAN

DESIGNED Patrick M. Petrone
CHECKED Jay D. Edwards
DRAWN ØECKY M. LEACH

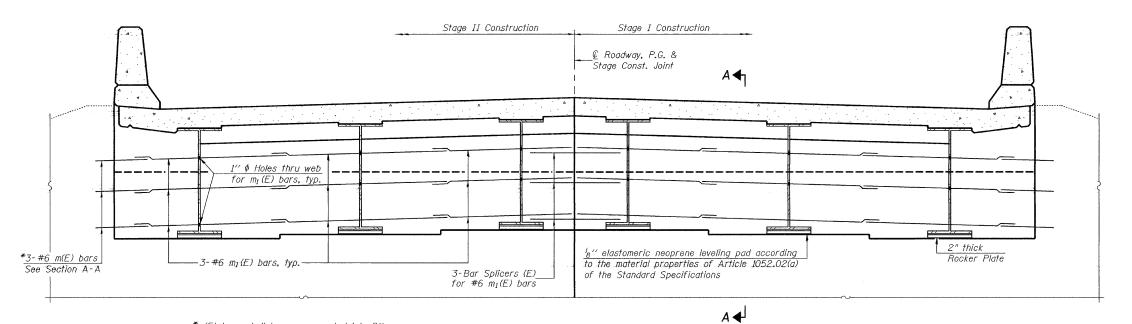
CHECKED PMP/JDE

PASSED Ralph C. Children
ENTINEER OF BRIDGES AND STRUCTURES





Contract #98886



*m(E) bars shall be epoxy grouted into 9" deep holes as per Section 584 of the Standard Specifications.

Reinforcement bars in diaphragm are billed with

Concrete in diaphragm is included with Concrete

MIN. BAR LAP #6 bar = 2'-6"

superstructure on sheet 10 of 20.

Superstructure on sheet 10 of 20.

DIAPHRAGM ELEVATION AT ABUTMENT

4'-0" 6'-0" skew Bar Splicers (E) for #5 b(E) bars -b(E) a3(E)-Approach Pavement $-a_I(E)$ -Const. Joints $\vdash_{b_1(E)}$ Elev. 490.46 (N. Abut.) Elev. 490.41 (S. Abut.) Const. Joint 2" thick Rocker Plate-Back of Proposed #5 v(E) bars Fill Plate at 12" cts. ^l₈" elastomeric neoprene leveling pad according to the material properties of Art. 1052.02(a) of the Standard Specifications. Cost included with Structural Steel. € Abut. — • Anchor bolt -Bk. of Exist. Abut.

CHECKED Jay D. Edwards

DRAWN BECKY M. LEACH

CHECKED PMP/JDE

9-3-07

<u>SECTION A-A</u>
Dimensions at right angles to abutment, except as shown.

DIAPHRAGM DETAILS

F.A.P. ROUTE 322 - SECTION 12-1, BR

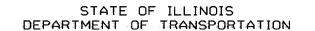
UNION CO.

STATION 801+60.00

STRUCTURE NO. 091-0022

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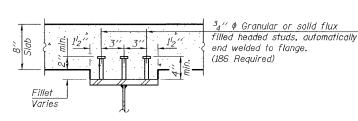
SI-DS2



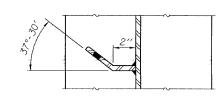
_ € Roadway, P.G., & Stage Const. Line



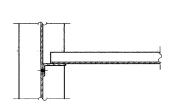
Contract #98886



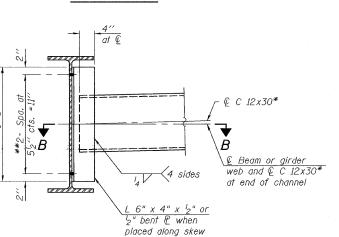
SECTION A-A



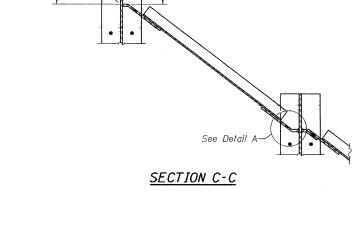
<u>DETAIL A</u>

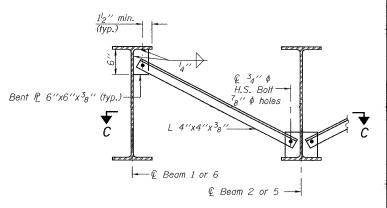


SECTION B-B



<u>DIAPHRAGM D</u> (5 required)





DIAPHRAGM D1

(8 required)

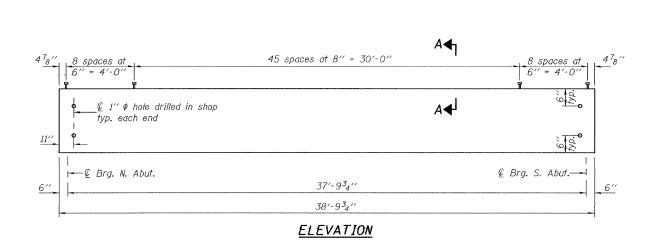
STRUCTURAL STEEL

F.A.P. ROUTE 322 - SECTION 12-1, BR

UNION CO.

STATION 801+60.00

STRUCTURE NO. 091-0022



16'-0³8''

| © Brg. | S. Abut.

5′-9′′ typ.

37′-9³4″

FRAMING PLAN

All beams are W24x84 (NTR) M270 Gr. 50.

21'-938"

Note:
Two hardened washers required for each set of oversized holes.

* Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section.

NTR denotes Notch Toughenss Requirements.

** ³4" \$ HS bolts, ¹⁵16" \$ holes

CHECKED PMP/JDE

DESIGNED Patrick M. Petrone

CHECKED Jay D. Edwards

—Beam No.

N. Abut.

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				,		
ROUTE NO.	SECTION	cou	INTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 13
F.A.P. 322	12-1, BR	UN	ION	36	29	20 SHEETS
FED. ROAD DIST	D DIST. NO. 7 ILLINOIS FED. AID PR		OJECT-			

Contract #98886

INTERIOR G	IRDER	MOMENT TABLE
		0.5 Sp. 1
I_s	(in ⁴)	2370
$I_{c}(n)$	(in4)	7966
$I_c(3n)$	(in4)	6027
Ss	(in ³)	196
So(n)	(in ³)	323
So(3n)	(in ³)	292
DC1	(k/')	0.897
M DC1	('k)	<i>1</i> 65.0
DC2	(k/')	0.152
M DC2	('k)	27.9
DW	(k/')	0.375
Mow	('k)	68.9
M& + Imp	('k)	423.9
Mu(Strength I)	('k)	1086.1
$\phi_f M_0$	('k)	1759.0
fs DC1	(ksi)	10.1
f _s DC2	(ksi)	1.2
f _s DW	(ksi)	2.8
fs 1.3(4+I)	(ksi)	20.7
fs (Service II)	(ksi)	34.8
fs (Total)(Strength I)		-
Vf	(k)	21.4

ſ	INTERIOR GIRDER REACTION TABLE				
ı	HL93 Loading Service I				
I			Abutment		
I	Roci	(k)	17.2		
1	R _{DC2}	(k)	2.9		
I	Row	(k)	7.2		
l	Rt + Imp	(k)	76.4		
	RTotal	(k)	103.7		

 $I_{\mathcal{S}}$, $S_{\mathcal{S}}$: Non-composite moment of inertia and section modulus of the steel section used for computing fs (Total-Strength I, and Service II) due to non-composite dead loads (in.4 and in.3). $I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing fs (Total-Strength I, and Service II) due to short-term composite live loads (in.4 and in.3). $I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in.4 and in.3). DC1: Un-factored non-composite dead load (kips/ft.). MDC1: Un-factored moment due to non-composite dead load (kip-ft.). DC2: Un-factored long-term composite (superimposed excluding

future wearing surface) dead load (kips/ft.).

MDC2: Un-factored moment due to long-term composite (superimposed

excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

Mt + Imp: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

My (Strength I): Factored design moment (kip-ft.).

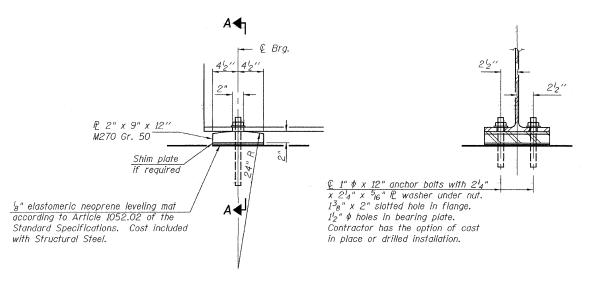
1.25 (MpcI + Mpc2) + 1.5 Mpw + 1.75 M $\frac{1}{2}$ + $\frac{1}{2}$ Compact composite positive moment capacity computed

according to Article 6.10.7.1 (kip-ft.).

fs (Service II): Sum of stresses as computed from the moments below (ksi). MDC1 + MDC2 + MDW + 1.3 M& + Imp

fs (Total)(Strength I): Sum of stresses as computed from the moments below on non-compact section (ksi).

1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M& + Imp V_f: Factored shear range computed according to Article 6.10.10.



ELEVATION AT ABUTMENT

SECTION A-A

FIXED BEARING

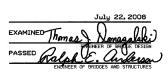
A l_8 " x 9" x 12" Fill P is required at Beam 4 North Abutment and Beam 3 South Abutment.

BILL OF MATERIAL

Item	Unit	Total
Anchor Bolts 1″ ∮	Each	24

STRUCTURAL STEEL DETAILS F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

DESIGNED Patrick M. Petrone CHECKED Jay D. Edwards DRAWN BECKY M. LEACH CHECKED PMP/JDE

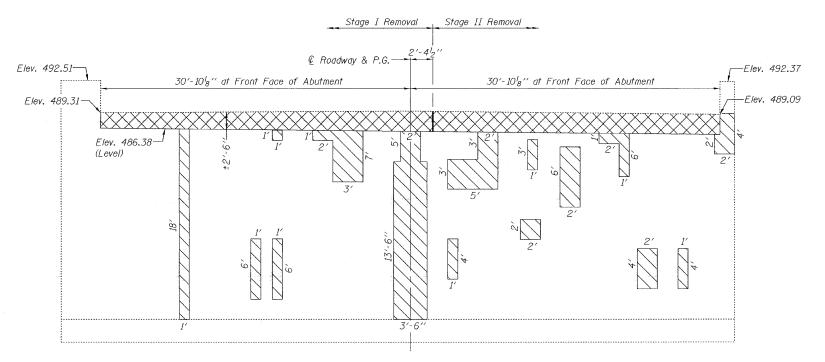


PROJECT NO. SECTION COUNTY 1707AL SHEET NO. I

F.A.P. 322 12-1. BR UNION 36 30

FED. ROND DIST. NO. 7 ILLINOIS FED. AID PROJECT-

Contract #98886



ELEVATION NORTH ABUTMENT

(Looking North)

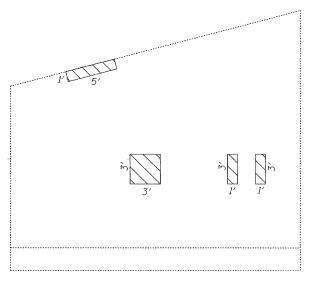
<u>LEGEND</u>



Structural Repair of Concrete < 5"

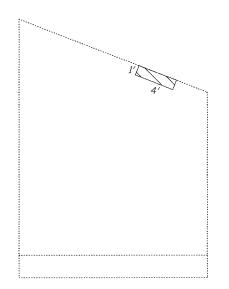


Concrete Removal



ELEVATION NORTHWEST WINGWALL

(Looking North)



ELEVATION NORTHEAST WINGWALL

(Lookina North)

BILL OF MATERIAL

Structural Repair of Concrete < 5″	Sq. Ft.	207
 Concrete Removal	Cu. Yd.	9.4

CONCRETE REMOVAL & REPAIR

AT NORTH ABUTMENT

F.A.P. ROUTE 322 - SECTION 12-1, BR

UNION CO.

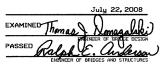
STATION 801+60.00

STRUCTURE NO. 091-0022

DESIGNED Patrick M. Petrone
CHECKED Jay D. Edwards

DRAWN BECKY M. LEACH

CHECKED PMP/JDE



ROUTE NO. SECTION TOTAL SHEET NO. 15 31 20 SHEETS UNION 36

Contract #98886

Stage II Removal Stage I Removal ?'-4^l2'' • Roadway & P.G. Elev. 492.35-—Elev. 492.35 30'-10'8'' at Front Face of Abutment 30'-10'8" at Front Face of Abutment -Elev. 489.15 Elev. 489.15 -(Level)

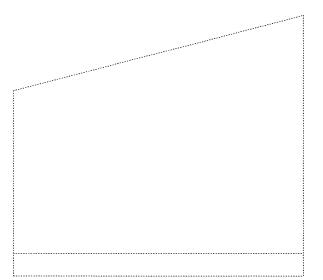
ELEVATION SOUTH ABUTMENT

(Looking South)

<u>LEGEND</u>

Structural Repair of Concrete < 5"

Concrete Removal



ELEVATION SOUTHEAST WINGWALL (Looking South)

ELEVATION SOUTHWEST WINGWALL (Looking South)

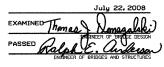
Structural Repair Concrete < 5" Cu. Yd. 9.4 Concrete Removal

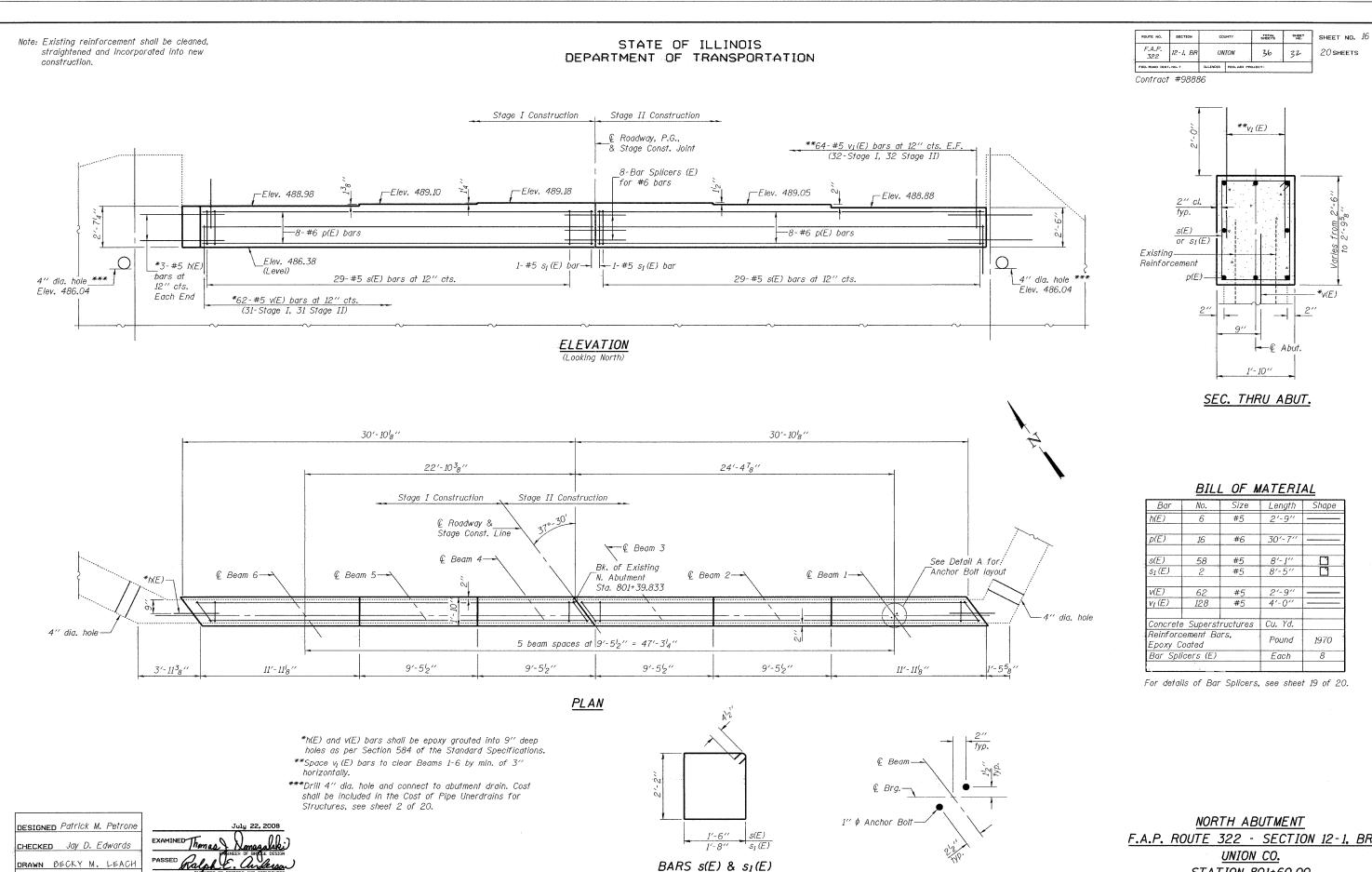
BILL OF MATERIAL

CONCRETE REMOVAL & REPAIR AT SOUTH ABUTMENT F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

DESIGNED Patrick M. Petrone CHECKED Jay D. Edwards DRAWN BECKY M. LEACH

CHECKED PMP/JDE





F.A.P. ROUTE 322 - SECTION 12-1, BR STATION 801+60.00 STRUCTURE NO. 091-0022

DETAIL A

1970

20 SHEETS

CHECKED PMP/JDE

ROUTE NO. Note: Existing reinforcement shall be cleaned, STATE OF ILLINOIS F.A.P. 322 straightened and incorporated into new 12-1. BR UNION DEPARTMENT OF TRANSPORTATION construction. Contract #98886 Stage II Construction Stage I Construction **64-#5 v1(E) bars at 12" cts. E.F. _€ Roadway, P.G., (32-Stage I, 32 Stage II) ~ Stage Const. Joint 8-Bar Splicers (E) for #6 bars Elev. 489.14 -Elev. 489.04 -Elev. 489.02 -Elev. 488.90 -Elev. 488.87 typ. -8-#6 p(E) bars -8-#6 p(E) bars or s1(E) Existing-Elev. 486.37 1-#5 $s_1(E)$ bar 1-#5 $s_1(E)$ bar *3-#5 h(E) Reinforcement (Level) bars at 29-#5 s(E) bars at 12" cts. 29-#5 s(E) bars at 12" cts. _4'' dia. hole *** 4" dia. hole *** 12" cts. Elev. 486.03 Elev. 486.03 Each End *62-#5 v(E) bars at 12" cts. (31-Stage I, 31 Stage II) **ELEVATION** (Looking South) SEC. THRU ABUT. 30'-1018" 30'-1018'' 24'-478' 22'-1038' BILL OF MATERIAL Stage II Construction Stage I Construction € Roadway & Stage Const. Line —**©** Beam 4 o(E) ⊈ Beam 3— See Detail A for: .58 Bk. of Existing Anchor Bolt layout 51(E) € Beam 6 € Beam 2-€ Beam 5-*h(E)-S. Abutment Sta. 801+80.167 v₁ (E) 128 Concrete Superstructures 4" dia. hole-Reinforcement Bars, 5 beam spaces at $9'-5^{l_2}$ " = $47'-3^{l_4}$ " FDOXV Coated Bar Splicers (E) 9'-52" 9'-512" 9'-512" 9'-512" 3'-1138' 11'-1118' 11'-1118' For details of Bar Splicers, see sheet 19 of 20. PLAN *h(E) and v(E) bars shall be epoxy grouted into 9" deep © Beamholes as per Section 584 of the Standard Specifications. **Space v_l (E) bars to clear Beams 1-6 by min. of $3^{\prime\prime}$ horizontally. € Brg.—

1" \$ Anchor Bolt

DETAIL A

1'-6'' s(E) 1'-8'' s₁(E)

BARS s(E) & s1(E)

***Drill 4" dia. hole and connect to abutment drain. Cost shall be included in the Cost of Pipe Unerdrains for

Structures, see sheet 2 of 20.

SOUTH ABUTMENT F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

SHEET NO. 17

20 SHEETS

- *v(E)

TOTAL SHEETS

36

**v1(E)

1'-10''

Size Length

#6

#5

#5

#5

#5

30'-7''

8'-1"

21-911

4'-0"

Cu. Yd.

Pound

Each

1970

SHEET NO.

33

DESIGNED Patrick M. Petrone

CHECKED Jay D. Edwards

CHECKED PMP/JDE

DRAWN BECKY M. LEACH

ROUTE NO. **Contractor is to verify beam length prior to ordering *© Transverse tie Æ's (2 per span). Place STATE OF ILLINOIS F.A.P. 322 material. Other sections meeting the section modulus additional shims at these points on top of tie P's. DEPARTMENT OF TRANSPORTATION requirements shown may be allowed subject to approval Securely weld shims to top flange of support beam. Minimum shim size is 6" x flange width. by the Bureau of Bridges and Structures. Maximum Girder Contract #98886 depth = 27", No additional payment will be allowed if the contractor chooses a heavier steel section than the one specified in the plans € Brg. — N. Abut. --- € Brg. S. Abut. - <u>€</u> Epoxy grouted ³₄"¢ threaded rods (Typ.) **W24x104 x 36'-5" (Cut as Shown) (Min. $Sx = 258 \text{ in }^3$) 36'-6'2" Face to Face of Abutments 40'-4" Bk. to Bk. of Existing Abutments ELEVATION - Tack weld nut to 31/2"repair P2 (Typ.) 40'-4" Bk. to Bk. of Existing Abutments Precast beams. V Section support beams TYPICAL DECK CROSS SECTION and HP Support columns 9'-32" 12'-2" 15'-1" (Looking South) 1" x 1'-1" x 1'-1" *****€ Concrete **NOTES** (12 Req'd) Diaphraams All structural steel shall conform to AASHTO Classification M-270 Gr. 36, Plan dimensions and details relative to existing plans are subject to routine $\bigcirc_{\overline{3}_{\mathcal{B}}}$ $\langle Typ.$ SECTION A-A variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished based upon the unit price bid for the work. 1"\$\phi\$ holes for epoxy grouted The contractor is advised that the existing Nelson Beams are in a $\frac{3}{4}$ # H.S. threaded rods. deteriorated condition with reduced load carrying capacity. It is the contractors responsibility to account for the condition of the beams when developing construction procedures. HP Support-See Section 584 of the Standard Specifications for Epoxy Grouting of Columns Threaded Rods: Minimum embedment 9". If the contractor's procedure for placement of beams involves placement of 1"Ø holes for cranes or other heavy equipment on the bridge, a detailed procedure shall be 34"\$ H.S. Bolts submitted to the Engineer for approval. The procedure shall include calculation prepared and sealed by an Illinois Licensed Structural Engineer, verifying that the equipment and procedure used will not overstress the existing beams. To -HP12x53 x 3'-0" distribute load to multiple beams and protect the existing surface, in all cases (12 Req'd) double layer mat of heavy timbers shall be used at all times under crane tracks and W Section or wheels and any outriggers in the down position. If necessary, shims shall be Support Beams used under the crane mat to ensure uniform contact with the underlying beams. The cost of epoxy grouting threaded rods on the pier cap, abutments and beams shall be included with Furnishing and Erecting Structural Steel. 36'-6'2" Face to Face of Abutments The Contractor has the option of using used steel. See Special Provisions. € 1"¢ holes for — -Bk. of Bk. of The Contractor shall remove the temporary support during Staged Removal of epoxy grouted N. Abut. S. Abut. TYPICAL SECTION TYPICAL SUPPORT BEAM the Superstructure. The Contractor shall burn off the threaded rods flush and 3₄ "\$ H.S. <u>PLAN</u> cover with epoxy. Cost included with Removal of Existing Superstructures. threaded rods. AT ABUTMENT END PLAN 2'-6" TEMPORARY SUPPORT FOR NELSON BEAMS DESIGNED Patrick M. Petrone F.A.P. ROUTE 322 - SECTION 12-1, BR BILL OF MATERIAL CHECKED Jay D. Edwards UNION CO. 13₁₆"\$ holes for DRAWN BECKY M. LEACH UNIT QUANTITY ITEM 3₄"¢ threaded rods STATION 801+60.00 Pound 25,750 urnishing and Erecting Structural Steel CHECKED PMP/JDE TRANSVERSE TIE P'S STRUCTURE NO. 091-0022

P 2" x 2'-9" x 6" (12 Req'd)

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TOTAL

36

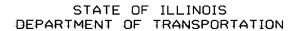
SHEET NO.

SHEET NO. 18

20 SHEETS

Stage I Removal

Shim as req' for tight fit (Typ.)





20 SHEETS

Contract #98886

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

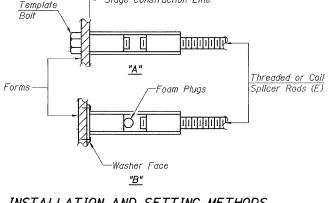
Minimum Capacity (Tension in kips) = 1.25 x fy x A_t

Minimum *Pull-out Strength = 0.66 x fy x A₁ (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

reinforcement bars.



-Stage Construction Line

BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

ROLLED THREAD DOWEL BAR

** ONE PIECE

-Wire Connector

lilililili

The diameter of this part is

equal or larger than the

diameter of bar spliced.

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

The diameter of this part

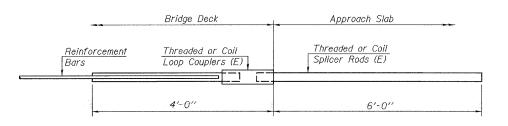
of the bar spliced.

is the same as the diameter

INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.

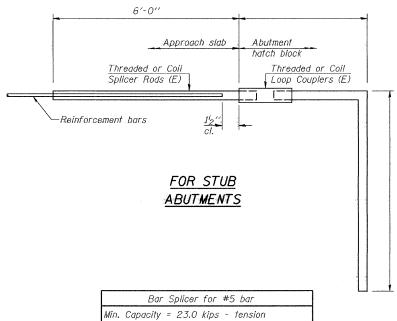
	BAR SPLICER ASSEMBLIES				
			Strength Requirements		
	Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension	
- 1	#4	1'-8''	14.7	7.9	
	#5	2'-0''	23.0	12.3	
	#6	2'-7''	33.1	17.4	
	#7	3′-5′′	45.1	23 . 8	
	#8	4'-6''	58.9	31.3	
-	#9	:5'-9"	75.0	39.6	
	#10	7′-3′′	95.0	50.3	
	#11	9'-0"	117.4	61.8	



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

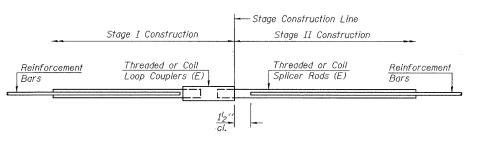
Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required = 80

DESIGNED Patrick M. Petrone CHECKED Jay D. Edwards DRAWN BECKY M. LEACH CHECKED PMP/JDE BSD-1 11-1-06



Min. Pull-out Strength = 12.3 kips - tension

No. Required =



STANDARD

 Bar Size	No. Assemblies Required	Location
#5	143	Superstructure
#6	6	Diaphragm
#6	8	N. Abutment
#6	8	S. Abutment

BAR SPLICER ASSEMBLY DETAILS F.A.P. ROUTE 322 - SECTION 12-1, BR UNION CO. STATION 801+60.00 STRUCTURE NO. 091-0022

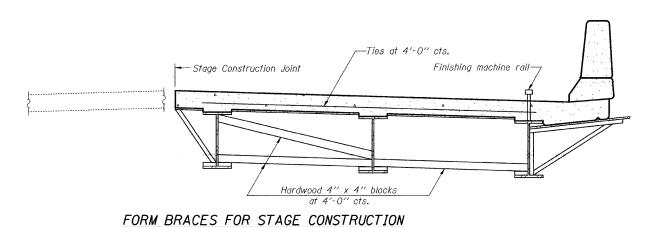
Contract #98886

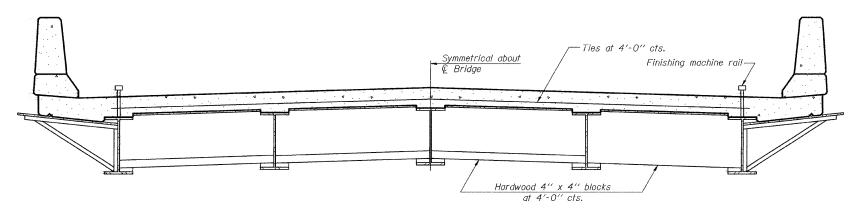
When cantilever forming brackets are used, the work shall be done according to Article 503.06(b) of the Standard Specifications, except as modified below and in the details shown on this sheet.

The finishing machine rails shall be placed on the top flange of the exterior beams.

The beams or girders, supporting cantilever forming brackets, shall be tied together at 4 foot intervals.

For Standard construction, or Stage Construction the Hardwood bracing materials shall be placed as shown between webs of beams in each bay.





FORM BRACES FOR STANDARD CONSTRUCTION

DESIGNED Patrick M. Petrone

CHECKED Jay D. Edwards

DRAWN BECKY M. LEACH

CHECKED PMP/JDE

SB-1

5-16-08

CANTILEVER FORMING BRACKETS FOR SUPERSTRUCTURES

WITH w27 BEAMS AND SMALLER

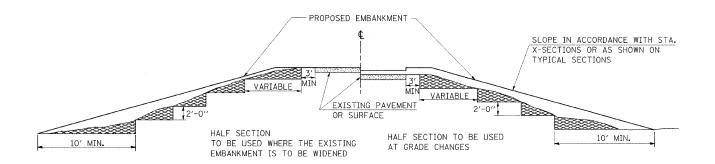
F.A.P. ROUTE 322 - SECTION 12-1, BR

UNION CO.

STATION 801+60.00

STRUCTURE NO. 091-0022

TYPICAL CROSS SECTION SHOWING STEP CONSTRUCTION ON EXISTING FILL



MATERIAL TO BE REMOVED AND REPLACED IN THE EMBANKMENT IN ACCORDANCE WITH ART. 205.04 OF THE STANDARD SPECIFICATION. COST TO BE INCLUDED IN THE VARIOUS ITEMS OF EXCAVATION AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED BECAUSE OF THIS WORK.

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	PLOT SCALE = 50.0000 // IN.	CHECKED	REVISED
	PLOT DATE = 6/18/2008	DATE	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

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CROSS	SECTIO	ON S	HOWING	s S	TEP	CONSTR	UCTION	ON	EXIST	ING	FILL

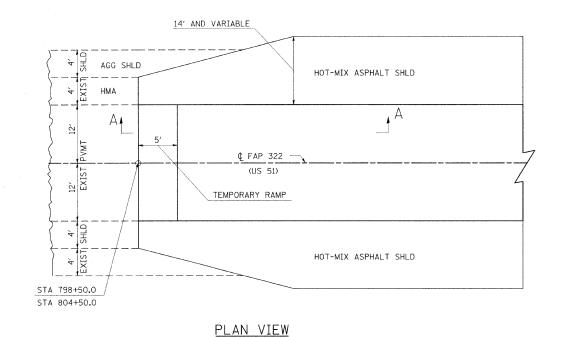
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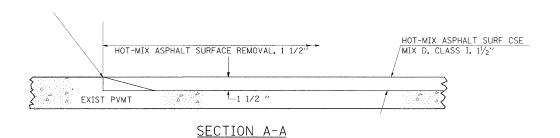
SHEET NO. 37 OF 48 SHEETS STA. __

SCALE:

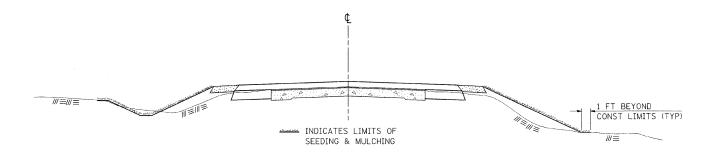
FAF JZZ	12-1,DR	CONTRACT		8886
FAP 322	12-1.BR	UNION	48	37
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE

BUTT JOINT DETAIL





SEEDING & MULCHING DETAIL



GENERAL NOTES

IN GENERAL, ALL EARTH SURFACES DISTURBED DURING CON-STRUCTION OPERATIONS SHALL BE SEEDED AND MULCHED UPON COMPLETION OF ALL GRADING OPERATIONS.

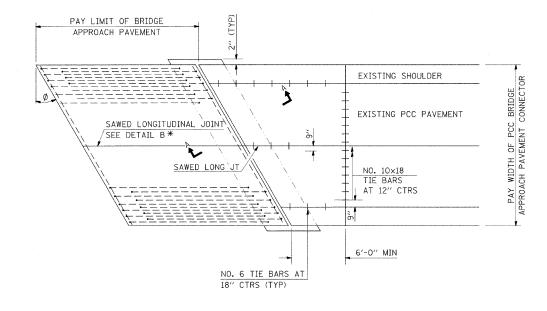
FERTILIZER NUTRIENTS AND LIMESTONE SHALL BE APPLIED TO ALL SEEDED AREAS.

THE RATES OF APPLICATION OF FERTILIZER, MULCH AND LIMESTONE SHALL BE AS SPECIFIED IN THE SPECIAL PROVISIONS.

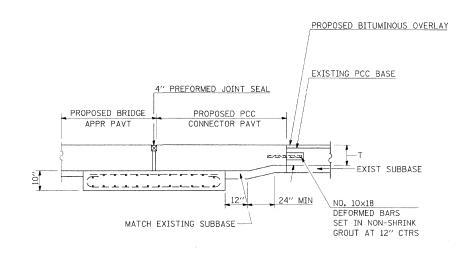
SECTIONS 250 AND 251 OF THE STANDARD SPECIFICATIONS SHALL GOVERN THIS WORK EXCEPT AS SPECIFIED HEREIN OR AS NOTED IN THE SPECIAL PROVISIONS.

FILE NAME =	USER NAME ≈ shepardgd	DESIGNED	REVISED			F.A.P. SECTION	COUNTY TOTAL SHEET NO.
c:\projects\d902904\d902904ms.dgn		DRAWN	REVISED	STATE OF ILLINOIS	BUTT JOINT AND SEEDING MULCHING DETAILS	FAP 322 12-1.BR	UNION 48 38
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PCC PAVEMENT CONNECTOR EXISTING CONSTRUCTION



BRIDGE APPROACH PAVEMENT CONNECTOR (PCC) SPECIAL (MODIFICATION TO STD 420401



SECTION A-A - RIGID PAVEMENT

REVISIONS
REDRAWN	10-11-01				
REVISEO	REVISEO	REVISEO			
REVISEO	REVISEO	REVISEO			
REVISEO	REVISEO	REVISEO	REVISEO		
REVISEO	REVISEO	REVISEO	REVISEO	REVISEO	
REVISEO	REVISEO	REVISEO	REVISEO	REVISEO	REVISEO
REVISEO	REVISEO	REVISEO	REVISEO	REVISEO	REVISEO
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REVISEO	REVISEO				

FILE	E NAME =	USER NAME = shepardgd	DESIGNED	REVISED			F.A.P. SECTI	ON COUNTY	TOTAL SHEET
c1/t	orojects\d902904\d902904ms.dgn		DRAWN	REVISED	STATE OF ILLINOIS	BRIDGE APPROACH PAVEMENT CONNECTOR (PCC) SPECIAL DETAIL	FAP 322 12-1.E	BR UNIÓN	48 39
		PLOT SCALE = 50.0000 '/ IN.	CHECKED	REVISED	DEPARTMENT OF TRANSPORTATION			CONTRAC	CT NO. 98886
		PLOT DATE = 6/18/2008	DATE	REVISED		SCALE: SHEET NO. 39 OF 48 SHEETS STA TO STA	_ FED. ROAD DIST. NO IL	LINOIS FED. AID PROJECT	

