

PLAN

INTERIOR GIRDER MOMENT TABLE	
	0.5 Span
I_s	(in ⁴) 4,760
$I_c (n)$	(in ⁴) 12,772
$I_c (3n)$	(in ⁴) 9,462
S_s	(in ³) 345
$S_c (n)$	(in ³) 503
$S_c (3n)$	(in ³) 456
\bar{Q}	(k'/') 0.92
M_Q	(k') 348
S_Q	(k'/') 0.54
M_{S_Q}	(k') 203
M_L	(k') 522
$M_{(Imp)}$	(k') 145
$5_3[M_L + M_{(Imp)}]$	(k) 1,111
M_a	(k) 2,161
M_u	(k) 2,497
$f_s \bar{Q}$ non-comp	(ksi) 12.1
$f_s \bar{Q}$ (comp)	(ksi) 5.3
$f_s 5_3[M_L + M_{(Imp)}]$	(ksi) 26.5
f_s (Overload)	(ksi) 44.0
f_s (Total)	(ksi) 62
VR	(k) 62

I_s and S_s are the moment of inertia and section modulus of the steel section used in computing f_s (Total & Overload).

$I_c(n)$ and $S_c(n)$ are the moment of inertia and section modulus of the composite section used in computing stresses due to live load.

$I_c(3n)$ and $S_c(3n)$ are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead loads.

VR is the maximum Live Load + Impact Shear range in span.

M_a (Applied Moment) = $1.3[M_L + 5/3(M_L + M_{(Imp)})]$.

The Plastic Moment capacity (M_u) is computed according to AASHTO 10.48.1 & 10.50.1.1.

f_s (Overload) is the sum of the stresses due to $M_Q + M_{S_Q} + 5/3(M_L + M_{(Imp)})$.

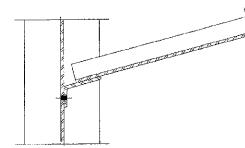
f_s (Total) is the sum of the stresses due to $1.3[M_L + m_S Q + 5/3(M_L + M_{(Imp)})]$.

M_Q - Moment due to dead loads on non-composite section.

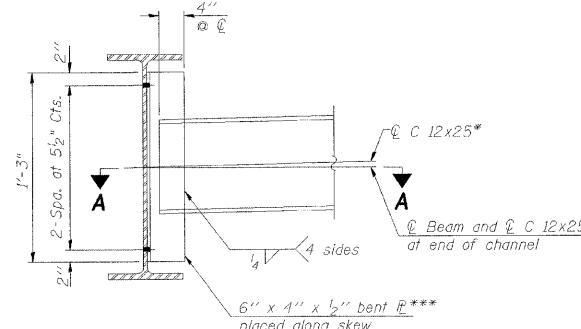
M_{S_Q} - Moment due to dead loads on composite section.

M_L - Moment due to live load on composite section.

$M_{(Imp)}$ - Moment due to live load impact on composite section.



SECTION A-A



*** 1 7/8" long-slotted vertical holes on connecting bent plates of adjacent beams along the stage construction line. See Notes 1 and 5.

DIAPHRAGM D

(20 Required)

INTERIOR GIRDER REACTION TABLE	
	Abutments
R_Q	(k) 73.1 **
R_L	(k) 48.6
M_p	(k) 13.5
R (Total)	(k) 135.3

** - Includes weight of concrete Diaphragms and Approach Slab

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Furnishing and Erecting Structural Steel	L. Sum	1

NOTES:

- Two hardened washers shall be required over all oversized holes for diaphragms.
- *Alternate channels C12x30 are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department. 3-3/4" HS bolts, 15/16" holes
- Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.
- All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as noted otherwise.
- Bolts for slotted holes shall only be finger tightened prior to pouring of deck slab and shall be fully tightened after completion of deck pour.

ILLINOIS DEPARTMENT OF TRANSPORTATION

FRAMING PLAN

ILLINOIS ROUTE 53 OVER

SPRING BROOK CREEK

FAU 2578 SECTION 532B-1

STRUCTURE NO. 022-0189

DUPAGE COUNTY STATION 166+46.79

SCALE: NONE DRAWN BY: E. MROCZEK

DATE: 6/12/09 CHECKED BY: G. HATLESTAD