

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

GIRDER MOMENT TABLE - RAMP 3 FLARE

$I_s, S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Total and Overload) due to non-composite dead loads (in.<sup>4</sup> and in.<sup>3</sup>).  
 $I_c(n), S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "n", used for computing  $f_s$  (Total and Overload) due to short-term composite live loads (in.<sup>4</sup> and in.<sup>3</sup>).  
 $I_c(3n), S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total and Overload) due to long-term composite (superimposed) dead loads (in.<sup>4</sup> and in.<sup>3</sup>).  
 Z: Plastic Section Modulus of the steel section in non-composite areas (in.<sup>3</sup>).  
 $\phi$ : Un-factored non-composite dead load (kips/ft.).  
 $M\phi$ : Un-factored moment due to non-composite dead load (kip-ft.).  
 $s\phi$ : Un-factored long-term composite (superimposed) dead load (kips/ft.).  
 $M_s\phi$ : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).  
 $M_L$ : Un-factored live load moment (kip-ft.).  
 $M_I$ : Un-factored moment due to impact (kip-ft.).  
 $M_a$ : Factored design moment (kip-ft.).  
 $1.3 [M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)]$   
 $M_u$ : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).  
 $f_s$  (Overload): Sum of stresses as computed from the moments below (ksi).  
 $M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)$   
 $f_s$  (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).  
 $1.3 [M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)]$   
 $VR$ : Maximum  $\frac{1}{4}$  + impact shear range within the composite portion of the span for stud shear connector design (kips).  
 $S_I$ : Section modulus of one flange plate for lateral flange bending (in.<sup>3</sup>).  
 $M_{bl}$ : Factored lateral bending moment for flange plate (kip-ft.).

	Girder 3.1		Girder 3.2			Girder 3.3 to 3.8			Girder 3.9	Girder 3.11	Girder 3.17	Cross Head Girder	
	0.5 Sp. R3-1	0.5 Sp. R3-2	0.4 Sp. R3-1	R3 Pier	0.6 Sp. R3-2	0.4 Sp. R3-1	R3 Pier	0.6 Sp. R3-2	0.4 Sp. R3-1	0.5 Sp. R3-1	0.5 Sp. R3-1	0.4 Span	at G1
$I_s$ (in <sup>4</sup> )	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	49,795	15,000	9,040	131,400	131,400
$I_c$ (n) (in <sup>4</sup> )	31,876	31,876	31,876		31,876	31,876		31,876	88,605	29,661			
$I_c$ (3n) (in <sup>4</sup> )	23,429	23,429	23,429		23,429	23,429		23,429	67,636	21,883			
$S_s$ (in <sup>3</sup> )	837	837	837	837	837	837	837	837	1,747	837	504	4,074	4,074
$S_c$ (n) (in <sup>3</sup> )	1,096	1,096	1,096		1,096	1,096		1,096	2,110	1,073			
$S_c$ (3n) (in <sup>3</sup> )	995	995	995		995	995		995	1,948	971			
Z (in <sup>3</sup> )				943								4541	4541
$\phi$ (k/ft)	0.98	1.18	0.89	1.57	1.42	0.98	1.38	0.98	1.41	1.11	1.29		
$M\phi$ (k)	705.0	318.0	396.0	1,111.0	584.0	671.0	1243.0	340.0	1644.0	228.0	357.0	2758.0	5970.0
$s\phi$ (k/ft)	0.40	0.40	0.36		0.57	0.40		0.40	0.60	0.80			
$M_s\phi$ (k)	288.0	113.0	179.0		249.0	292.0		157.0	708.0	147.0			
$M_L$ (k)	640.0	329.0	565.0	348.0	560.0	703.0	413.0	560.0	857.0	270.0	334.0	1049.0	1336.0
$M$ (Imp) (k)	160.0	95.0	141.0	87.0	139.0	162.0	103.0	140.0	197.0	81.0	96.0	262.0	347.0
$5_3 [M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)]$ (k)	1,333.3	706.7	1,177.0	725.0	1,165.0	1,441.7	860.0	1,166.7	1,756.7	585.0	716.7	2,185.0	2,805.0
$M_a$ (k)	3,024.2	1,479.0	2,278.0	2,387.0	2,598.0	3,126.1	2,734.0	2,162.8	5,341.3	1,248.0	1,395.8	6,425.9	11,407.5
$M_u$ (k)	5,073.0	5,073.0	5,073.0	3,429.0	5,073.0	5,073.0		5,073.0	10,286.0	4,897.0	2,397.0	18,920.0	18,920.0
$f_s \phi$ (non-comp) (ksi)	10.1	4.6	5.7	16.0	8.4	9.6	17.8	4.9	11.3	3.3	8.5	8.1	17.6
$f_s \phi$ (comp) (ksi)	3.5	1.4	2.2	-	3.0	3.5	-	1.9	4.4	1.8	-	-	-
$f_s 5_3 (\frac{1}{4} + Imp)$ (ksi)	14.6	7.7	12.9	10.4	12.8	15.8	12.3	12.8	10.0	6.5	17.1	6.4	8.3
$f_s$ (Overload) (ksi)	28.2	13.7	20.8	26.4	24.2	28.9	30.1	19.5	25.6	11.6	25.6	14.6	25.8
$f_s$ (Total) (ksi)							39.2						
VR (k)	52.6	49.9	55.4		55.5	52.2		56.5	53.0	46.0			

\* Compact section  
 \*\* Braced non-compact and partially braced section

GIRDER REACTION TABLES - RAMP 3 FLARE

	Girder 3.1				Girder 3.2			Girder 3.3 to 3.8			Girder 3.9	Girder 3.11	Girder 3.17	Cross Head Girder			
	Carrier Girder	Pier R3 (West)	Pier R3 (East)	FB 3.4	Carrier Girder	Pier R3	Q Bearing Abutment	Carrier Girder	Pier R3	Q Bearing Abutment	Carrier Girder	Pier R3	Q N. Brg C. Abut 1	G3.9	Carrier Girder & FB3.6	South Bearing	North Bearing
$R_{DL}$ (k)	53.2	53.2	36.0	34.5	41.6	138.2	47.7	52.5	147.6	38.3	88.3	78.5	39.6	39.4	31.1	554.0	1034.0
$R_{LL}$ (k)	41.8	41.8	38.6	38.6	40.8	47.0	40.8	42.0	51.2	41.0	42.8	38.3	35.4	35.4	38.6	100.0	160.0
$R_I$ (k)	10.4	10.4	11.1	11.1	10.3	11.7	10.2	9.6	12.2	10.2	9.8	8.8	10.6	10.6	11.2	25.0	40.0
$R_{TOTAL}$ (k)	105.4	105.4	85.7	84.2	92.7	196.9	98.7	104.1	211.0	89.5	140.9	125.6	85.6	85.4	80.9	679.0	1234.0

Girder 3.12			
	0.4 Sp. R3-1	R3 Pier	0.6 Span R3-2
$I_s$ (in <sup>4</sup> )	78,863	78,863	78,863
$I_c$ (n) (in <sup>4</sup> )	127,048	85,300	127,048
$I_c$ (3n) (in <sup>4</sup> )	99,626	3,100	99,626
$S_s$ (in <sup>3</sup> )	2719	2719	2719
$S_c$ (n) (in <sup>3</sup> )	3154	85,300	3154
$S_c$ (3n) (in <sup>3</sup> )	2941	3,100	2941
$S_I$ (in <sup>3</sup> )	161	161	161
$\phi$ (k/ft)	3.1	2.1	1.6
$M\phi$ (k)	784	1358	765
$s\phi$ (k/ft)	1.3	0.73	0.5
$M_s\phi$ (k)	326	497	246
$M_L$ (k)	512.0	470.0	626.0
$M$ (Imp) (k)	133.0	117.0	150.0
$5_3 [M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)]$ (k)	1075.0	979.0	1294.0
$M_a$ (k)	2841.0	3685.0	2997.0
$M_{bl}$ (k)	26.0	73	42.0
$f_s \phi$ (non-comp) (ksi)	3.5	6.0	3.4
$f_s \phi$ (comp) (ksi)	1.4	2.0	1.0
$f_s 5_3 (\frac{1}{4} + Imp)$ (ksi)	4.1	3.8	5.0
$f_s$ (Overload) (ksi)	9.0	11.8	9.4
$f_s$ (Total) (ksi)	11.7	15.4	12.3
$F_{cr}$ (Overload) (ksi)	47.5	47.5	47.5
VR (k)	59.1		56.3
$F_{cr}$ (ksi)	49.3	32.3	44.3

Girder 3.12			
	G1	R3 Pier	R3 Abut
$R_{DL}$	71.0	254.0	59.0
$R_{LL}$	41.0	55.0	42.0
$R_I$	10.0	14.0	10.0
$R_{TOTAL}$	122.0	323.0	111.0

MOMENT TABLE  
 RAMP 3 FLARE  
 STRUCTURE NO. 016-0724

TYLIN INTERNATIONAL

DESIGNED - JPN	REVISIONS	
	NAME	DATE
CHECKED - AMD,		
DRAWN - MAU		
CHECKED - AMD,		
DATE - 08/02/10		

SHEET NO. 94	F.A.I RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	55	0711.2R & 1011.1BR	COOK	200	124
137 SHEETS	CONTRACT NO. 60L39				
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT					

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