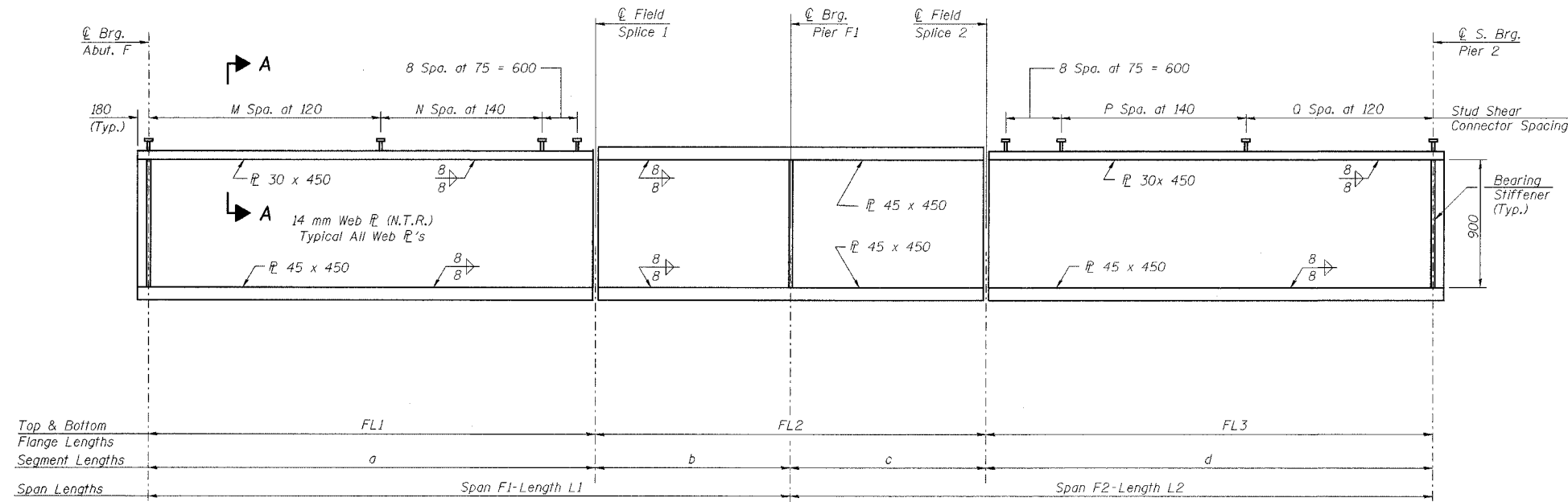


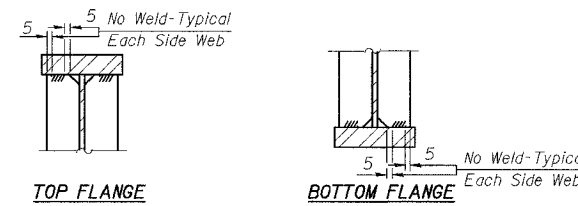
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 35
F. A. I. 80/94		COOK	90	50	42 SHEETS



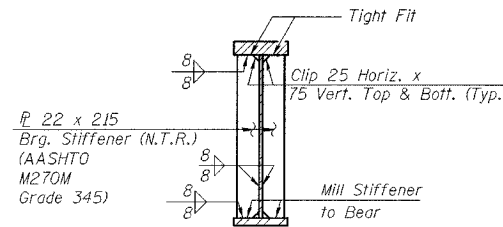
GIRDER ELEVATION

(All Plates shall be N.T.R.)
"N.T.R." denotes notch toughness requirements are applicable



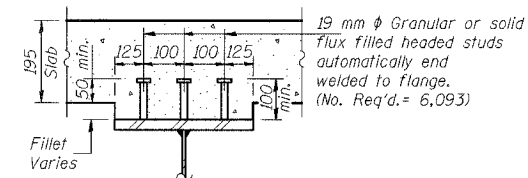
STIFFENER TO FLANGE WELD

Typical for Bearing Stiffeners



AT PIERS F1 & 2 & ABUTMENT F

BEARING STIFFENERS



SECTION A-A
"NOT IN CONTRACT"

Notes:

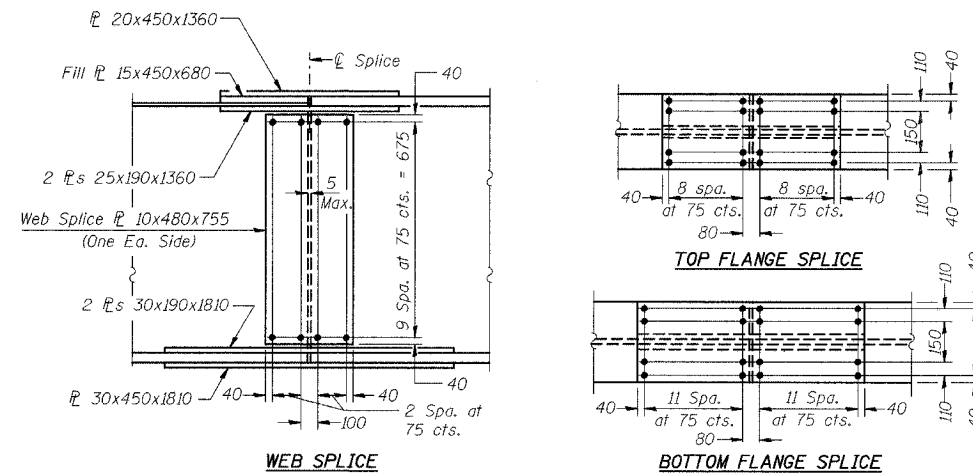
- For Span Lengths L1, L2 & Segment Lengths a thru d, see Sheet No. 34 of 42.
- All Flange Plates & Web Plates shall be AASHTO M270M Grade 345.
- All Field Splice Plates, except Fill Plates, shall be AASHTO M270M Grade 345 and shall meet the Notch Toughness Requirements (N.T.R.).
- All dimensions are in millimeters (mm) except noted otherwise.

SHEAR CONNECTOR NUMBERS
"NOT IN CONTRACT"

Girder	Span F1		Span F2	
	M	N	P	Q
1	64	93	100	69
2	65	94	98	68
3	65	94	95	66
4	66	95	93	65
5	67	97	90	64
6	67	97	88	63

GIRDER TOP AND BOTTOM FLANGE LENGTHS
(Meters)

Girder	FL1	FL2	FL3
1	22.431	8.072	23.945
2	22.590	8.129	23.475
3	22.749	8.186	23.006
4	22.908	8.243	22.538
5	23.067	8.301	22.068
6	23.226	8.358	21.599



FIELD SPLICE 1 & 2

DESIGNED	ML
CHECKED	MAS
DRAWN	LK
CHECKED	MAS

CONTRACT NO. 62898

INTERIOR GIRDER MOMENT TABLE

		0.4 Sp.F1	Pier	0.6 Sp.F2
Is	(10 ⁶ mm ⁴)	8,116	9,885	8,116
Ic (n)	(10 ⁶ mm ⁴)	19,602	-	19,602
Ic (3n)	(10 ⁶ mm ⁴)	13,621	-	13,621
Ss	(10 ³ mm ³)	19,075	19,976	19,075
Sc (n)	(10 ³ mm ³)	25,859	-	25,859
Sc (3n)	(10 ³ mm ³)	23,270	-	23,270
Sbr	(10 ³ mm ³)	1,516	-	1,516
W	(kN/m)	12.6	17.4	12.6
M _l	(kN·m)	701	1,751	730
s _l	(kN/m)	5.3	-	5.3
M _s	(kN·m)	309	-	346
M _t	(kN·m)	1,153	911	1,114
M (Imp)	(kN·m)	289	228	279
S _z (M _t +M (Imp))	(kN·m)	2,403	1,897	2,322
Ma	(kN·m)	4,436	4,742	4,417
Mbr	(kN·m)	3	-	-
f _s (non-comp)	(MPa)	37	88	38
f _s (comp)	(MPa)	13	-	15
f _s S _z (M _t +M (Imp))	(MPa)	93	95	90
f _t	(MPa)	1.8	-	-
f _s (Overload)	(MPa)	143	183	143
f _s (Total)	(MPa)	186	238	186
F _{cr} (Overload)	(MPa)	327	223	327
VR	(kN)	498	-	498
F _{cr}	(MPa)	345	305	345

INTERIOR GIRDER REACTION TABLE

	Ramp F Abut.	Pier F1	Pier 2	
R _l	(kN)	180	605	198
R _t	(kN)	225	285	208
Imp.	(kN)	67	86	62
R (Total)	(kN)	472	976	468

F_{cr} - Critical average flange stress (smaller of F_{cr1} or F_{cr2} for partially braced flanges and F_y for continuously braced flanges) computed according to the 2003 AASHTO Guide Specifications for Horizontally Curved Steel Girder Highway Bridges (Sections 5.2, 5.3 and 5.4).

F_{cr} (Overload) - Critical average flange stress at overload computed according to the 2003 AASHTO Guide Specifications for Horizontally Curved Steel Girder Highway Bridges Section 9.5.

I_s and S_s are the moment of inertia and section modulus of the steel section used in computing f_s (Total and Overload).

I_c(n) and S_c(n) are the moment of inertia and section modulus of the composite section used in computing stresses due to live load.

I_c(3n) and S_c(3n) are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead load (see AASHTO 10.38).

VR is the maximum $\epsilon +$ impact shear range in span.

Ma (Applied Moment) = 1.3 [M_l + M_s + 5/3 (M_t + M (Imp))]

f_s (Overload) is the sum of stresses due to M_l + M_s + 5/3 (M_t + M (Imp))

f_s (Total) is the sum of stresses due to 1.3 [M_l + M_s + 5/3 (M_t + M (Imp))]

S_{br} is the section modulus for one flange plate for lateral flange bending.

M_{br} is the lateral bending moment for flange plate (factored).

f_t is the calculated normal stress at the edge of flange due to lateral bending (factored).

M_t and R_t include the effects of centrifugal force and superelevation.

ILLINOIS DEPARTMENT OF TRANSPORTATION
I-94 EAST BOUND / IL 394 SOUTH BOUND
GIRDER ELEVATION & DETAILS
SPAN 1F & 2F - UNIT 3

RAMP F OVER THORN CREEK
F.A.P. 332 SECTION 2004-133F
COOK COUNTY
STA. 440+704.350 STRUCTURE NO. 016-2845
DATE 05/16/05
SCALE ---

HNTB

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