346-3209 PROJECT ENGINEER: PATTI LeBEAU (618) 346-3179 (618)CONTACT:

STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

PROPOSED HIGHWAY PLANS FAP ROUTE 42 (IL 127) SECTION 2BR

PROJECT: F-0042(097)

WASHINGTON COUNTY

C-98-140-05 STRUCTURE REPLACEMENT

STA 487+25 PROPOSED STRUCTURE NO. 095-0076 ONE SPAN WF BEAM BRIDGE WITH INTEGRAL ABUTMENTS, 85'-6" BK TO BK ABUTMENTS,

LATITUDE X: 38.4853°

LONGITUDE Y: 89.3586°

ILLINOIS PROFESSIONAL NO. 43244

GROSS LENGTH = 796.00 FEET 0.151 MILES NET LENGTH = 796.00 FEET 0.151 MILES

CUMMINS ENGINEERING CORPORATION SPRINGFIELD, ILLINOIS

CONTRACT NO. 76389

D-98-119-00



STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

INDEX OF SHEETS

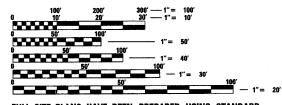
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- 4-5 TYPICAL SECTIONS
 - SCHEDULES
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 - PLAN & PROFILE
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- 15-30 BRIDGE PLANS SN 095-0076
- 31-33 CROSS SECTIONS

HIGHWAY STANDARDS

280001-02	635011-01
420001-06	701006-02
420401-05	701011-01
515001-02	701201-02
542401	701311-02
601101	701321-08
609006-02	701326-02
630001-06	702001-06
630301-03	704001-02
631031-05	780001-01
631046-02	781001-02
635006_02	

MICROFILMED **REEL NUMBER AWARDED** RESIDENT ENGINEER AS BUILT CHANGES WERE MADE

ON THE FOLLOWING SHEETS



ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES, IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123

CONTRACT NO. 76389

WASHINGTON COUNTY

FAP ROUTE 42 (IL 127)

BEGIN IMPROVEMENT

END IMPROVEMENT

STA 491 + 23

40'-2" O TO O DECK, SKEW 0°

FAP 42 (IL 127)

SU= 5.6% MU= 11.1%

ADT = 3500 (2004)

ADT = 5200 (2024)

STA 483 + 27

FAP ROUTE	SECTION	COUNTY	TOTAL SMEETS	SHEET NO
42	2BR	WASHINGTON	33	2 .
STA.		TO STA.		
CVICTI	NO CONDITIO			

CONTRACT NO. 76389

GENERAL NOTES

- 1. THE STANDARDS AND REVISION NUMBERS LISTED SHALL APPLY TO THIS CONTRACT.
- 2. "ROAD CONSTRUCTION AHEAD" SIGNS SHALL BE PLACED AT THE BEGINNING AND END OF THE PROJECT LIMITS AND THE INTERSECTING SIDE ROADS. COST IS INCLUDED IN THE TRAFFIC CONTROL PAY ITEMS.
- 3. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING MATERIALS.
- 4. THE THICKNESS OF THE BITUMINOIUS MIXTURE SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE BITUMINOUS MIXTURE IS PLACED.
- 5. FLAGMEN SHALL BE PRESENT DURING ALL CLOSURE HOURS, INCLUDING LUNCH HOURS, AND NO ADDITIONAL COMPENSATION SHALL BE ALLOWED.
- ANY TRAFFIC CONTROL PAY ITEM NOT USED SHALL BE DELETED FROM THE CONTRACT AND NO ADDITIONAL COMPENSATION SHALL BE ALLOWED.
- 7. ILLINOIS STATE LAW REQUIRES A 48-HOUR NOTICE BE GIVEN TO ALL UTILITIES BEFORE DIGGING. FIELD MARKING OF FACILITIES MAY ALSO BE OBTAINED BY CALLING J.U.L.I.E. AND FOR NON-J.U.L.I.E. MEMBERS, THE UTILITY COMPANY DIRECTLY. AGENCIES KNOWN TO HAVE FACILITIES WITHIN THE PROJECT AREA ARE AS FOLLOWS;
 - *FRONTIER COMMUNICATIONS
 - *AMEREN IP
 - *CONSOLIDATED WATER SERVICES

(MEMBERS OF J.U.L.I.E. (800) 892-0123 INDICATED BY *. NON-J.U.L.I.E. MEMBERS MUST BE NOTIFIED INDIVIDUALLY.)

- 8. WHERE SECTION OF SUB-SECTION MONUMENTS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE RESURFACED OVER OR REMOVED. THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL PROPERTY MARKERS AND MONUMENTS UNTIL THE OWNER, AN AUTHORIZED SURVEYOR, OR AGENT HAS WITNESSED OR OTHERWISE REFERENCED THE LOCATION.
- 9. THE CONTRACTOR SHALL FURNISH AND INSTALL WOOD SIGN SUPPORTS IN ACCORDANCE WITH SECTION 730 OF THE STANDARD SPECIFICATIONS, HOWEVER, INSTALLATION BY METHOD "A" (ARTICLE 730.04(a)) SHALL BE THE ONLY METHOD PERMITTED.
- 10. SAW CUTTING ON ALL EDGES FOR REMOVAL ITEMS SHALL BE INCLUDED IN THE COST OF THE REMOVAL ITEM AS INDICATED AND ACCORDING TO SECTION 440 OF THE STANDARD SPECIFICATIONS. (APPROX. 900')
- 11. ALL EXCAVATED AREAS DUE TO THE WIDENING OPERATIONS SHALL BE PROTECTED WITH EXTENDED LEG BARRICADES AND THE APPROPRIATE LIGHTS.
- 12. THE TRAFFIC CONTROL MEASURES SHALL SUPPLEMENT AND BE IN ACCORDANCE WITH TRAFFIC CONTROL STANDARD 701321.

MIXTURE REQUIREMENTS

	BITUMINOUS CONC. SURFACE COURSE, SUPERPAVE, MIX "C" N70	LÉVELING BINDER (MACHINE METHOD), SUPERPAVE N70	BITUMINOUS BASE COURSE, SUPERPAVE	BITUMINOUS SHOULDERS, SUPERPAVE	BRIDGE APPROACH PAVEMENT CONNECTOR FLEXIBLE
MIXTURE USE	SURFACE	LEVEL BINDER	BASE COURSE	TOP LIFT SHOULDERS	BINDER
AC/PG	PG 64-22	PG 64-22	PG 58-22	PG 58-22	PG 64-22
RAP % (MAX)	10%	10%	30%	30%	15%
DESIGN AIR VOIDS	4.0% • Ndes=70	4.0% @ Ndes=70	2.0% o Ndes=50	**2.0% • Ndes=30	4.0% • Ndes=70
MIX COMPOSITION					
(GRADATION MIXTURE)					
FRICTION AGG	MIXTURE C	MIXTURE C	BASE COURSE	BAM	MIXTURE B

PLAN QUANTITIES FOR BITUMINOUS CONCRETE SURFACE COURSE ITEMS ARE CALCULATED USING A UNIT WEIGHT OF 112 LB/SQ YD/1" THICKNESS

EXISTING BITUMINOUS SURFACES SERVING AS A BASE FOR LEVELING BINDER AND/OR SURFACE COURSE REQUIRE PRIME COAT APPLIED AT A RATE OF .08 GAL/SQ YD AS SPECIFIED IN ARTICLE 406.06 OF THE STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.

** TOP LIFT SHOULDERS - DESIGN THIS MIX AT 2.0% VOIDS AND ADD ASPHALT TO REDUCE VOIDS TO 1.5%

EROSION CONTROL PLAN

ALL EROSION CONTROL PRODUCTS FURNISHED SHALL BE SPECIFICALLY RECOMMENDED BY THE MANUFACTURER FOR THE USE SPECIFIED IN THE EROSION CONTROL PLAN. PRIOR TO APPROVAL AND USE OF THE PRODUCT THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER A NOTARIZED CERTIFICATION BY THE PRODUCER STATING THE INTENDED USE OF THE PRODUCT AND THAT THE PHYSICAL PROPERTIES REQUIRED FOR THIS APPLICATION ARE MET OR EXCEEDED. THE CONTRACTOR SHALL PROVIDE MANUFACTURER RECOMMENDED INSTALLATION PROCEDURES TO FACILITATE THE ENGINEER IN CONSTRUCTION INSPECTION.

TEMPORARY SEEDING AND MULCH SHALL BE COMPLETED ON A WEEKLY BASIS ON EXPOSED GROUND AND SHALL BE IN ACCORDANCE WITH SECTION 280 OF THE STANDARD SPECIFICATIONS EXCEPT THAT THE MULCH AND TEMPORARY SEEDING SHALL BE PAID FOR AS TEMPORARY EROSION CONTROL SEEDING AND NO OTHER PAYMENT WILL BE PERMITTED.

ALL AREAS DISTURBED FOR ANY REASON SHALL BE SEEDED WITH CLASS 2 SEEDING AS DIRECTED BY THE ENGINEER. NUTRIENTS SHALL CONFORM TO ARTICLE 250.04 EXCEPT FERTILIZER NUTRIENTS WILL NOT BE PAID FOR SEPARATELY BUT CONSIDERED AS INCLUDED IN THE COST OF CLASS 2 SEEDING.

CLASS 2 SEEDING TO BE PLACED AS SOON AS EARTHWORK IS COMPLETED.

COMMITMENTS

NONE

GENERAL NOTES, COMMITMENTS

FAP ROUTE 42 (IL 127) SECTION 2BR WASHINGTON COUNTY ILLINOIS DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES

F.A.P. SECTION
42 2BR COUNTY TOTAL SHEET NO. WASHINGTON STA. TO STA.
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

		CUMMARY OF CHANTITIES		80%.FED.	CONS	TRUCTION TYPE (ODE
		SUMMARY OF QUANTITIES		<i>201.57ATE</i> TOTAL	X0 7 1-2A	SFTY-3N	
	CODE NO	ITEM	UNIT	QUANTITIES			
	20200100	EARTH EXCAVATION	CU YD	255	255		
	20300100	CHANNEL EXCAVATION	CU YD	1040	1040		
	20700400	POROUS GRANULAR EMBANKMENT (SPECIAL)	CU YD	215	215		
	25000200	SEEDING, CLASS 2	ACRE	0.5	0.5		
	25100115	MULCH, METHOD 2	ACRE	0.5	0.5		
	28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	150	150		
	28000300	TEMPORARY DITCH CHECKS	EACH	4	4		
	28000400	PERIMETER EROSION BARRIER	FOOT	1056	1056		
	281,00109	STONE RIPRAP, CLASS A5	SQ YD	970	970		
	28200200	FILTER FABRIC	SQ YD	970	970		
	40600200	BITUMINOUS MATERIALS (PRIME COAT)	TON	0.2	0.2		
	40600980	BITUMINOUS SURFACE REMOVAL - BUTT JOINT	SQ YD	494	494		
	42001165	BRIDGE APPROACH PAVEMENT	SQ YD	254	254		
	42001300	PROTECTIVE COAT	SQ YD	254	254		·
	42001430	BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)	SQ YD	72	72		
	44000100	PAVEMENT REMOVAL	SQ YD	504	504	,	
	48202000	BITUMINOUS SHOULDERS SUPERPAVE	TON	32	32		
	50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1	1		
	50200100	STRUCTURE EXCAVATION	CU YD	230	230		
	50300225	CONCRETE STRUCTURES	CU YD	40.2	40.2		
	50300255	CONCRETE SUPERSTRUCTURE	CU YD	132.4	132.4		
	50300260	BRIDGE DECK GROOVING	SQ YD	559	559		
	50300300	PROTECTIVE COAT	SQ YD	423	423		
	50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1,	· 1		
	50500505	STUD SHEAR CONNECTORS	EACH	1350	1350		
	50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	31490	31490		
	51201600	FURNISHING STEEL PILES HP12X53	FOOT	335	335		
	51202700	DRIVING STEEL PILES	FOOT	335	335		
	51203600	TEST PILE STEEL HP12X53	EACH	2	2		
	51205200	TEMPORARY SHEET PILING	SQ FT	1365	1365		
	51500100	NAME PLATES	EACH	1	1		
	54213447 59/00/00 60100945 60/09580 60900315	END SECTIONS 12" GEOCOMPOSITE WALL ORAIN PIPE DRAINS 12" PIPE UNDER ORAINS FOR STRUCTURES 4" TYPE D INLET BOX, STANDARD 609006	EACH SQ.YO. FOOT FOOT EACH	1 83 16 /50	1 83 16 /50 1		
	60900515	CONCRETE THRUST BLOCKS	EACH	1	1		
*	63000000	STEEL PLATE BEAM GUARD RAIL, TYPE A	FOOT	538	538	,	

		SUMMARY OF QUANTITIES	;	80!.FED. 20%.STATE	CONSTRUCTION TYPE CODE		
-				TOTAL	X071-2A	SFTY-3N	
L	CODE NO	ITEM	UNIT	QUANTITIES			
	63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4	4		
	63100167	TRAFFIC BARRIER TERMINAL TYPE 1, SPECIAL (TANGENT)	EACH	4 :	4		
	63200310	GUARDRAIL REMOVAL	FOOT	163	163		
	63301990	REMOVE AND RE-ERECT TRAFFIC BARRIER TERMINAL, TYPE 1	EACH	2	2		
	67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	12	12		
l	67100100	MOBILIZATION	L SUM	1	1		
	70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321	EACH	1	y 1		
	70100450	TRAFFIC CONTROL AND PROTECTION, STANDARD 701201	L SUM	1	1		
	70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD 701326	L SUM	1	1		
	70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	60	60		
	70106600	TEMPORARY BRIDGE TRAFFIC SIGNALS (STATE FURNISHED CONTROLLER)	EACH	1	1		
	70106700	TEMPORARY RUMBLE STRIP	EACH	6	6		
	70300100	SHORT-TERM PAVEMENT MARKING	FOOT	164	164		
١	70301000	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	55	55		
ļ	70400100	TEMPORARY CONCRETE BARRIER	FOOT	550	550		
	70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	510	510		
	70500100	TEMPORARY STEEL PLATE BEAM GUARD RAIL, TYPE A	FOOT	325	325		
	70500685	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 10	EACH	2	2		
ř	78000200	THERMOPLASTIC PAVEMENT MARKING - LINE 4"	FOOT	3082	3082		
	78008210	POLYUREA PAVEMENT MARKING TYPE I - LINE 4"	FOOT	582	582		
-	78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	10	10		
	78100105	RAISED REFLECTIVE PAVEMENT MARKER (BRIDGE)	EACH	2	2		
4	78200420	GUARDRAIL MARKERS, TYPE B	EACH	10	10		
4	78200520	BARRIER WALL MARKERS, TYPE B	EACH	4	4		
+	78200530	BARRIER WALL MARKERS, TYPE C	EACH	4	4		
+	78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4	. 4		1
	78300100	PAVEMENT MARKING REMOVAL	SQ FT	864	864		
	78300200	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	EACH	12	12		
	X0321475	PIPE ELBOW, 12"	EACH	2	2		
	X3550500	BITUMINOUS BASE COURSE SUPERPAVE 8"	SQ YD	1121	1121		
	X4066416	BITUMINOUS CONCRETE SURFACE COURSE, SUPERPAVE, MIX "C", N70	TON	28	28		

* SPECIALTY ITEMS

ILLINOIS DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES

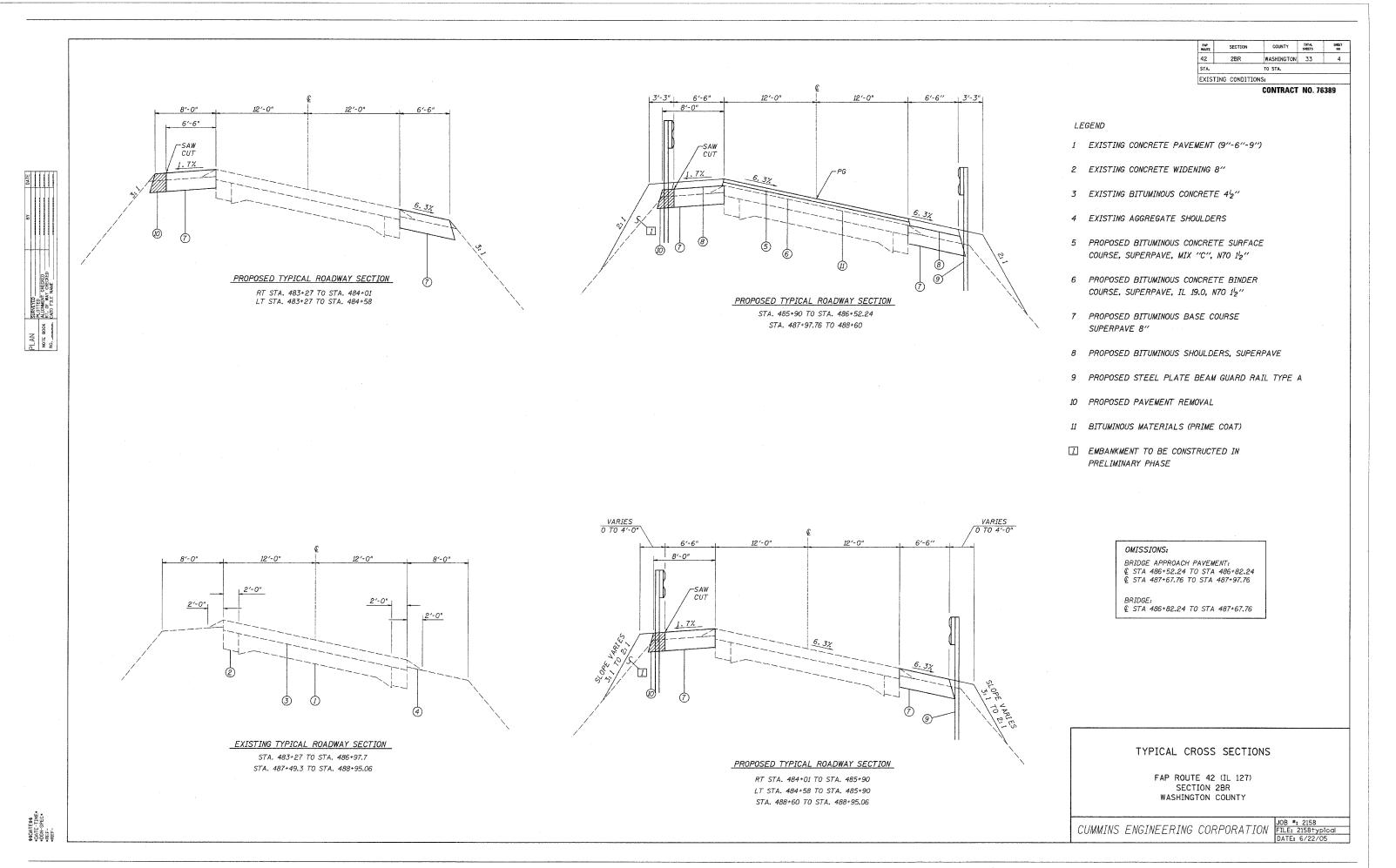
	CONTRACT NO.								
F.A.P. RTE.	SECTION	С	OUNTY	TOTAL SHEETS	SHEET NO.				
42	2BR	W	ASHINGTON	33	3A				
STA. TO STA.									
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT									

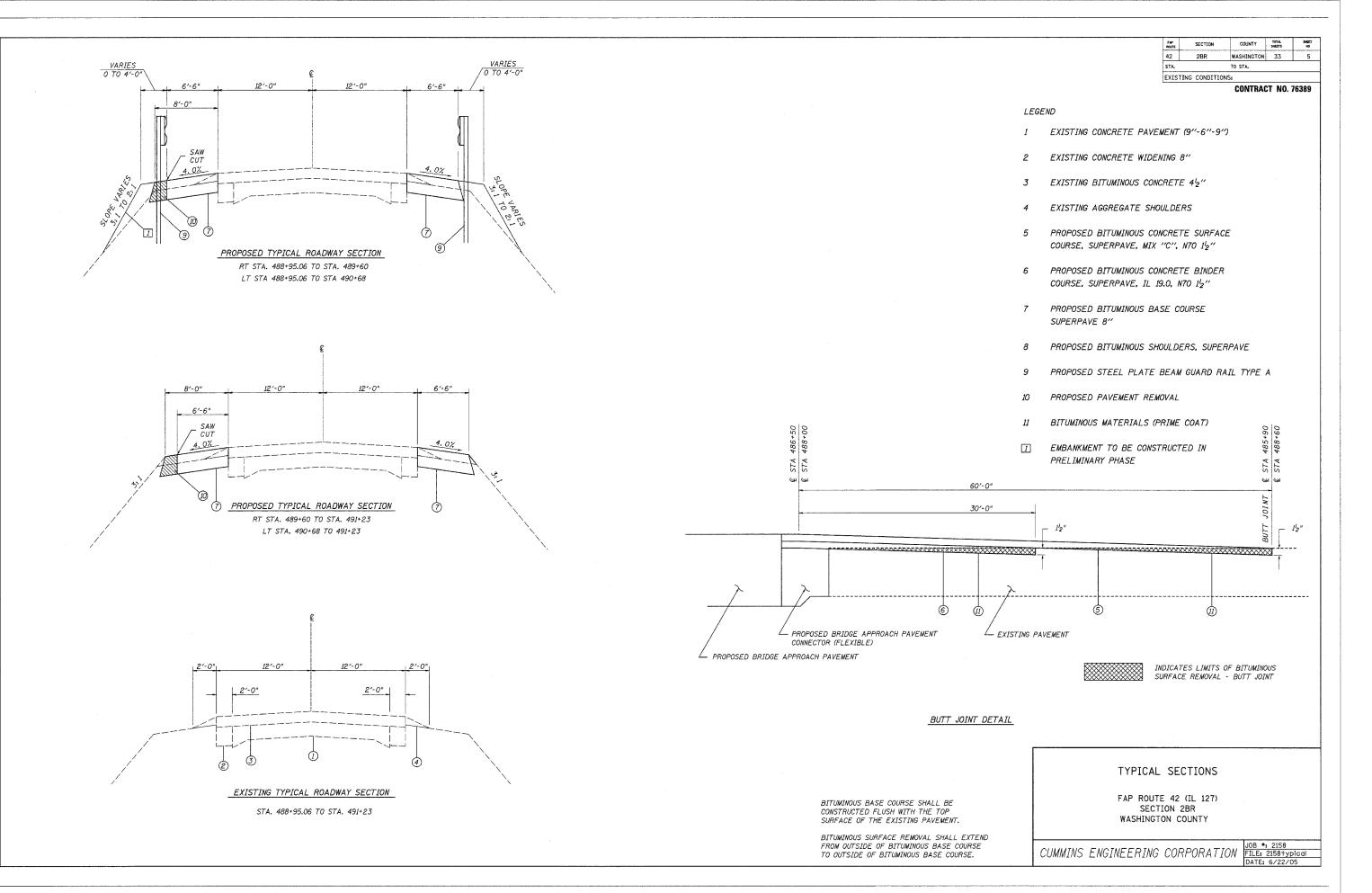
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	CLIMATON OF CHANTITIES			80/, FED. CONSTRUCTION TYPE (
	SUMMARY OF QUANTITIES	I	TOTAL	X0 7 1-2A	SFTY-3N	,	
CODE NO	ITEM	UNIT	QUANTITIES				
(4066770	LEVELING BINDER (MACHINE METHOD), SUPERPAVE N70	TON	% 16	16			
0002600	BAR SPLICERS	EACH	374	374	·		
0030250	IMPACT ATTENUATORS, TEMPORARY (NON- REDIRECTIVE), TEST LEVEL 3	EACH	2		2		
20030350	IMPACT ATTENUATORS, RELOCATE (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2		2		
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	SUMMARY	0F	QUANTITIES			CONS	TRUCTION TYPE	CODE	
CODE NO	JOIMINIATO	ITEM		UNIT	TOTAL QUANTITIES	garante de terre de la company			
CODE NO									
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NOTE BOOK A	No.

LC	CATIO)N	EARTH	EARTH EXCAVATION	EMBANKMENT	EARTHWORK
			EXCAVATION	ADJUSTED FOR		BALANCE WASTE (+)
CTATIO	y TO	STATION		25% SHRINKAGE		OR SHORTAGE (-)
3/A/10/	V 10 3	STATION	CU YD	CU YD	CU YD	CU YD
483+27	TO	484+00	27.0	20.3	6.8	<i>13</i> ,5
484+00	TO	485+00	<i>33.3</i>	25.0	24.1	0.9
485+00	TO	486+00	33.3	25.0	40.7	- 15.7
486+00	ΤO	486+50	21.3	16.0	18.5	-2 . 5
486+50	ΤO	486+75	11.1	8.3	7.4	0.9
486+75	TO	486+82	2.7	2.0	2.1	-0.1
487+68	TO	488+00	16.7	12.5	10.7	1.8
488+00	ΤO	488+25	10.6	8.0	7.4	0.6
488+25	ΤO	488+50	8.3	6.2	6.9	-0.7
488+50	TO	489+00	16.7	12.5	12.0	0.5
489+00	TO	490+00	<i>33.3</i>	25.0	11.1	13.9
490+00	TO	490+50	15.7	11.8	3.7	8.1
490+50	ΤO	491+00	13.9	10.4	8.3	2.1
491+00	TO	491+23	7.7	5.8	3.4	2.4
TOTAL			251.6	188.8	163.1	25.7
ROUNDED	TOTA	L	255	190	<i>1</i> 65	30

EARTHWORK REQUIRED FOR WIDENING IS INCLUDED IN THE EARTH EXCAVATION QUANTITY.

LOCATION	V	WIDTH	SQ YD
LT	STA 483+27.00	TO STA 486+97.70 B	330
LT	STA 487+49.00	TO STA 491+23.00 8	333
RT	STA 483+27.00	TO STA 486+42.07 6.5	228
RT	STA 488+04.94	TO STA 491+23.00 6.5	230
TOTAL			1,121

BITUMINOUS	CONCRETE SURFACE	COURSE, SUPERPA	VE, MIX	"C", N70	
LOCATION			WIDTH	THICKNESS	TON
LT & RT	STA 485+90.00	TO STA 486+52.24	24	1.5	14
LT & RT	STA 487+97.76	TO STA 488+60.00	24	1.5	14
TOTAL					28

BITUMINOUS	CONCRETE BINDER	COURSE,	SUPERPAVE	E, IL 19.	O, N7O	
LOCATION				WIDTH	THICKNESS	TON
LT & RT	STA 486+20.00	TO STA	486+52.24	24	1.5	8
LT & RT	STA 487+97.76	TO STA	488+30.00	24	1.5	8
TOTAL	•					16

BITUMINOUS	SHOULDERS SUPERI	PAVE				
LOCATION				WIDTH	THICKNESS	TON
LT & RT	STA 485+90.00	TO STA	486+52.24	6.5	3	16
LT & RT	STA 487+97.76	TO STA	488+60.00	6.5	3	<i>1</i> 6
ΤΟΤΔΙ						32

BITUMINOUS CONCRETE QUANTITIES CALCULATED BASED ON 112 POUNDS PER SQUARE YARD PER 1 INCH OF THICKNESS.

<i>BITUMINOUS</i>	SURFACE REMOVAL	- BUTT JOINT		
LOCATION			WIDTH	SQ YD
LT & RT	STA 485+90.00	TO STA 486+50.00	37	247
LT & RT	STA 488+00.00	TO STA 488+60.00	37	247
TOTAL				494

BITUMINOUS	MATERIALS (PRIME	COAT)				
LOCATION				WIDTH	GAL	TON
LT & RT	STA 485+90.00	TO STA	486+52.24	37	21	0.1
LT & RT	STA 487+97.76	TO STA	488+60.00	37	21	0.1
TOTAL					42	0.2

THE QUANTITY FOR BITUMINOUS MATERIALS PRIME COAT REFLECTS TWO APPLICATIONS, ONE PRIOR TO PLACEMENT OF BINDER COURSE AND ONE PRIOR TO PLACEMENT OF BITUMINOUS SHOULDERS.

BITUMINOUS MATERIALS PRIME COAT QUANTITY CALCULATED BASED ON AN APPLICATION RATE OF 0.08 GALLON PER SQUARE YARD (7.9 POUNDS PER GALLON)

BRIDGE APPROACH PAVEMENT	
LOCATION	WIDTH
STA 486+52.24 TO STA 486+82.24	38.333
STA 487+67.76 TO STA 487+97.76	38.333
TOTAL	
BRIDGE APPROACH PAVEMENT CONNECTOR	(FLEXIBLE
LOCATION CONNECTOR	(FLEXIBLE WIDTH
LOCATION STA 486+42 TO STA 486+52.24	
LOCATION	WIDTH

APPROACH PAVEMENT INLET RT STA 487+8	32.5	
ITEM	QTY	UNIT
TYPE D INLET STANDARD 609006	1	EACH
PIPE DRAINS 12"	16	FOOT
END SECTIONS 12"	1	EACH
PIPE ELBOW 12"	2	EACH
CONCRETE THRUST BLOCK	1	EACH

THE MEASURED LENGTH OF PIPE DRAINS SHALL EXCLUDE THE LENGTH OF ELBOWS.

LOCATION	ON					WIDTH	SQ YD
LT & R	TSTA	486+42.00	TO	STA	486+97.70	24	149
LT & R	TSTA	487+49.30	ΤO	STA	488+04.94	24	149
LT	STA	483+27.00	TO	STA	486+42.00	1.5	53
LT	STA	486+42.00	ΤO	STA	486+97.70	8	50
LT	STA	487+49.30	T0	STA	488+04.94	8	50
LT	STA	488+04.94	T0	STA	491+23.00	1.5	53
TOTAL							504

STEEL PLA	TE BEAM	GUARD R	AIL, TYPE A	
LOCATION				F00T
LT STA 48.	5+41.73	TO STA	486+52.65	112.5
LT STA 48	7+97.35	TO STA	489+83.45	187.5
RT STA 48	4+85.68	TO STA	486+50.52	162.5
RT STA 48	7+99.48	TO STA	488+75.56	75.0
TOTAL				537.5

TRAFFIC BARRIER	TERMINAL,	TYPE 6	
LOCATION			EACH
LT STA 486+52.65	TO STA	486+82.87	1
LT STA 487+67.13	TO STA	487+97.35	1
RT STA 486+50.52	TO STA	486+81.61	1
RT STA 487+68.39	TO STA	487+99.48	1
TOTAL			4

TRAFFIC BARRIER	TERMINAL	TYPE 1, SPECIAL	(TANGENT)
LOCATION			EACH
LT STA 484+87.00	TO STA	485+36.25	1
LT STA 489+11.30	TO STA	489+61,30	1
RT STA 484+34.96	TO STA	484+85.68	1
RT STA 488+75.56	TO STA	489+25.56	1
TOTAL			4

GUARDRAIL REMOVAL	
LOCATION	FOOT
LT STA 486+41.50 TO STA 486+95.50	54
LT STA 487+52.00 TO STA 487+80.00	28
RT STA 486+69.00 TO STA 486+97.00	28
RT STA 487+50.00 TO STA 488+03.00	53
TOTAL	163

REMOVE AND RE-ERE	CT TRAFFIC BARRIER	TERMINAL.	TYPE 1
LOCATION			EACH
LT STA 484+92.43	TO STA 485+41.73		1
LT STA 489+83.45	TO STA 490+33.45		1
TOTAL			2

GUARDRAIL MARKERS TYPE B				
LOCATION	EACH			
LT STA 485+41.73	TO STA 486+82.87	2		
LT STA 487+67.13	TO STA 489+83.45	3		
RT STA 484+85.68	TO STA 486+81.61	3		
RT STA 487+68.39	TO STA 488+75.56	2		
TOTAL		10		

BARRIER WALL MARKERS TYPE B & TYPE C						
LOCATIO)N		EACH	EACH		
LT STA	486+82.87	TO STA 487+67.13	2	2		
RT STA	486+81.61	TO STA 487+68.39	2	2		
TOTAL			4	4		

TERMINAL MARKERS - DIRECT APPLIED	
LOCATION	EACH
LT STA 484+92.43	1
LT STA 490+33.45	1
RT STA 484+34.96	1
RT STA 489+25.56	1
TOTAL	4

LOCATION		FOOT
STA 484+51.00	TO STA 485+70.60	120
STA 485+70.60	TO STA 488+79.40	310
STA 488+79.40	TO STA 489+99.00	120
TOTAL		550

LOCATION	EACH
STA 484+50.00	1
STA 490+00.00	1
TOTAL	2

LOCATION		F00T
STA 484+70.00	TO STA 485+69.50	100
STA 485+69.50	TO STA 488+80.60	310
STA 488+80.60	TO STA 489+80.30	100
TOTAL		510

IMPACT ATTENUATORS RELOCATE, (NON-REDIRECTIVE) TEST	LEVEL 3
LOCATION	EACH
STA 484+69.00	1
STA 489+81.00	1
TOTAL	2

TEMPORARY STEEL	PLATE BEAM GUARD RAIL, TYPE A	
LOCATION		F00T
LT STA 485+36.25	TO STA 486+95.65	162.5
LT STA 487+51.05	TO STA 489+11.30	162.5
TOTAL		325.0

TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 10	
LOCATION	EACH
LT STA 486+95.65 TO STA 486+98.50	1
LT STA 487+48.20 TO STA 487+51.05	1
TOTAL	2

	PAVEMENI MA	ARKING REMOVAL		
		LOCATION	TYPE	SQ FT
1)	STA 482+72	TO STA 485+90	DOUBLE C.L.	210
1)	STA 488+60	TO STA 491+88	DOUBLE C.L.	217
D	STA 483+27	TO STA 491+23	LT EDGE LINE	263
2	STA 483+27	TO STA 485+90	RT EDGE LINE	87
2	STA 488+60	TO STA 491+23	RT EDGE LINE	87
	TOTAL			864

SEE SHEET 14 FOR PAVEMENT MARKING AND RAISED REFLECTIVE MARKER SCHEDULES.

1) REMOVE PRIOR TO STAGE 1 2) REMOVE PRIOR TO STAGE 2

FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET
42	2BR	WASHINGTON	33	6
STA.		TD STA.		
EXISTING CONDITIONS:				

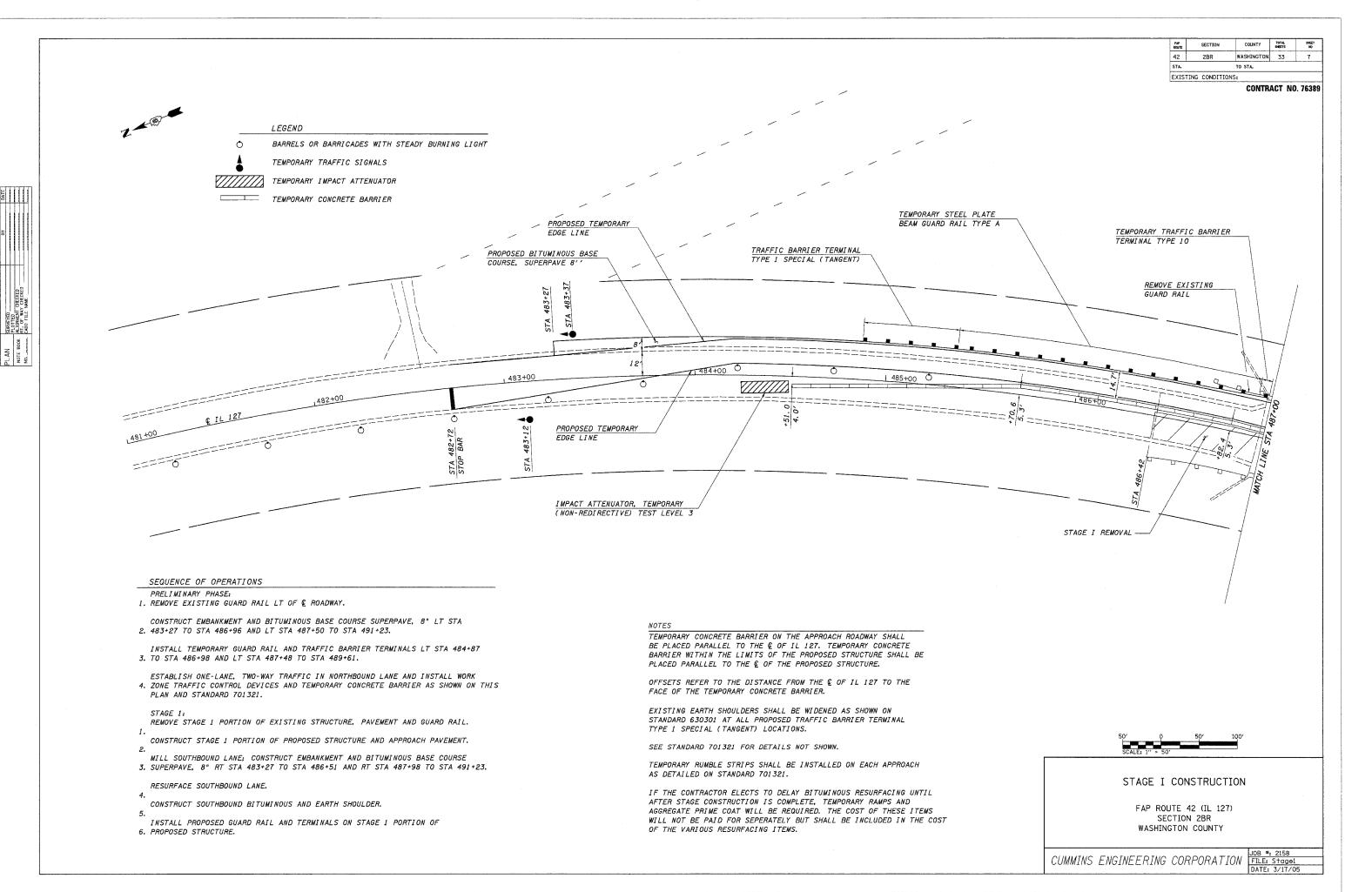
CONTRACT NO. 76389

TEMPORARY DITCH CHECKS	
LOCATION	EACH
STA 484+00	1
STA 485+00	1
STA 486+00	1
STA. 486+80	1
TOTAL	4
PERIMETER EROSION BARRIER	
LOCATION	FOOT
LT STA 483+27 TO STA 486+74	356
LT STA 487+75 TO STA 491+23	350
RT STA 487+69 TO STA 491+23	350
TOTAL	1,056

SCHEDULE OF QUANTITIES

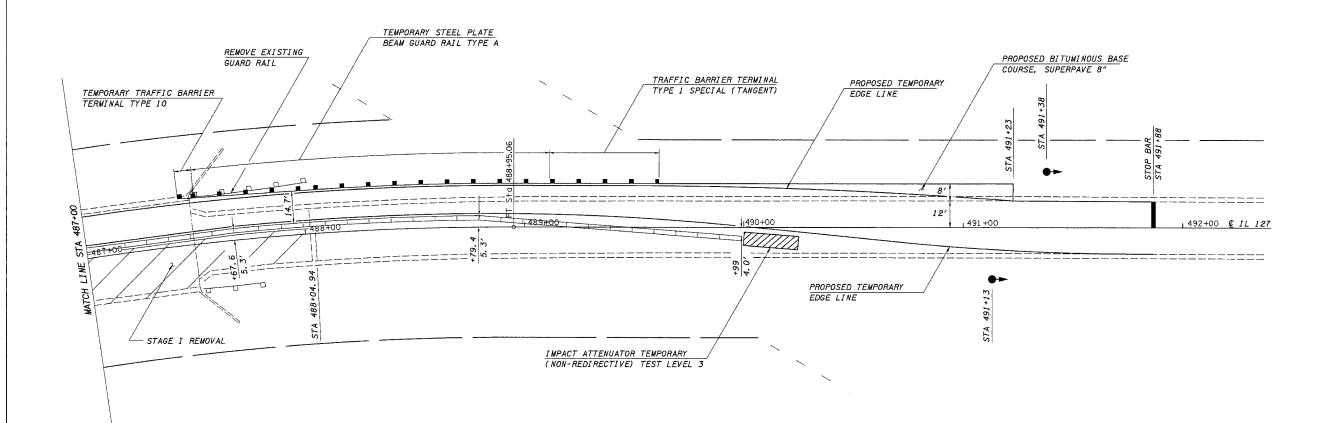
FAP ROUTE 42 SECTION 2BR WASHINGTON COUNTY

CUMMINS ENGINEERING CORPORATION



004~SPEC *EFF-TOPO086.00N: LV= 2~3, 5-19, 21, 23-25, 27-36, 38-56, 58-*PECF-DET F DAN: 10-2-62 TEMPORARY CONCRETE BARRIER

CONTRACT NO. 76389



TEMPORARY	PAVEMENT	MARKINGS

LOCATION		LINE TYPE & COLOR	PAVEMENT MARKING TAPE, TYPE III 4"	TEMPORARY PAVEMENT MARKING - LINE 4"	WORKZONE PAVEMENT MARKING REMOVAL
			F00T	FOOT	SQ FT
STAGE 1					
12' LT STA 482+72	TO 12' LT STA 483+27	WHITE EDGE LINE	56		19
12' LT STA 483+27	TO 20' LT STA 484+20	WHITE EDGE LINE	94		32
20' LT STA 484+20	TO 20' LT STA 485+90	WHITE EDGE LINE	170		57
20' LT STA 485+90	TO 20' LT STA 488+60	WHITE EDGE LINE		270	
20' LT STA 488+60	TO 20' LT STA 490+30	WHITE EDGE LINE	170		57
20' LT STA 490+30	TO 12' LT STA 491+23	WHITE EDGE LINE	94		32
12' LT STA 491+23	TO 12'LT STA 491+88	WHITE EDGE LINE	66		22
12' RT STA 482+72	TO 5.3' LT STA 484+20	WHITE EDGE LINE	150		50
5.3' LT STA 484+20	TO 5.3' LT STA 485+90	WHITE EDGE LINE	170		57
5.3' LT STA 485+90	TO 5.3' LT STA 488+60	WHITE EDGE LINE		270	
5.3' LT STA 488+60	TO 5.3' LT STA 490+30	WHITE EDGE LINE	170		57
5.3' LT STA 490+30	TO 12' RT STA 491+88	WHITE EDGE LINE	160		53
ΤΟΤΔΙ			1 300	540	436

QUANTITIES FOR PLACEMENT AND REMOVAL OF PAVEMENT MARKING TAPE TYPE III, 4" AND TEMPORARY PAVEMENT MARKING LINE 4" ARE INCLUDED FOR INFORMATION ONLY. PLACING AND REMOVING THESE ITEMS WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST FOR TRAFFIC CONTROL AND PROTECTION STANDARD 701321.



STAGE I CONSTRUCTION

FAP ROUTE 42 (IL 127) SECTION 2BR WASHINGTON COUNTY

CUMMINS ENGINEERING CORPORATION FILE: ST

LE: Stage1 TE: 3/17/05

COUNTY TOTAL SMEETS SECTION 42 2BR STA. EXISTING CONDITIONS: CONTRACT NO. 76389

LEGEND

TEMPORARY TRAFFIC SIGNALS TEMPORARY IMPACT ATTENUATOR

TEMPORARY CONCRETE BARRIER

PROPOSED TEMPORARY EDGE LINE 483+00

SEQUENCE OF OPERATIONS

- 1. SHIFT TRAFFIC CONTROL DEVICES FOR STAGE 2 CONSTRUCTION. ESTABLISH ONE-LANE, TWO-WAY TRAFFIC IN SOUTHBOUND LANE AND RELOCATE WORK ZONE TRAFFIC CONTROL DEVICES, TEMPORARY CONCRETE BARRIER AND IMPACT ATTENUATORS AS SHOWN ON THIS PLAN AND STANDARD 701321.
- 2. REMOVE TEMPORARY GUARD RAIL AND TERMINALS.
- 3. REMOVE STAGE 2 PORTION OF PAVEMENT AND STRUCTURE INCLUDING PARTIAL REMOVAL OF BITUMINOUS BASE COURSE CONSTRUCTED IN THE PRELIMINARY PHASE AS SHOWN ON
- 4. CONSTRUCT STAGE 2 PORTIONS OF PROPOSED STRUCTURE AND APPROACH PAVEMENTS.
- 5. MILL NORTHBOUND LANE AND RESURFACE.
- 6. CONSTRUCT BITUMINOUS SHOULDERS AND EARTH SHOULDERS.
- 7. INSTALL TRAFFIC BARRIER TERMINAL TYPE 6 AND STEEL PLATE BEAM GUARD RAIL; RE-ERECT TRAFFIC BARRIER TERMINAL TYPE 1 SPECIAL (TANGENT).
- 8. REMOVE TRAFFIC CONTROL DEVICES, TEMPORARY PAVEMENT MARKINGS, TEMPORARY CONCRETE BARRIER, AND IMPACT ATTENUATORS; INSTALL SHORT-TERM PAVEMENT MARKINGS AND OPEN ALL LANES TO TRAFFIC.

- 1. REMOVE SHORT-TERM PAVEMENT MARKINGS; INSTALL PAVEMENT MARKINGS AND RAISED REFLECTIVE MARKERS.
- 2. SEEDING AND FINAL CLEAN UP.

NOTES

IMPACT ATTENUATOR, RELOCATE (NON-REDIRECTIVE) TEST LEVEL 3

> PAVEMENT REMOVAL-(PARTIAL REMOVAL OF BIT. BASE CSE.)

¢ FAP RTE. 42 (IL 127) 1 484+00

12'

SEE STANDARD 701321 FOR DETAILS NOT SHOWN.

PROPOSED TEMPORARY EDGE LINE

TEMPORARY CONCRETE BARRIER ON THE APPROACH ROADWAY SHALL BE PLACED PARALLEL TO THE & OF IL 127. TEMPORARY CONCRETE BARRIER WITHIN THE LIMITS OF THE PROPOSED STRUCTURE SHALL BE PLACED PARALLEL TO THE & OF THE PROPOSED STRUCTURE.

REMOVE TEMPORARY GUARD RAIL

STAGE 2 REMOVAL

AND TERMINALS

OFFSETS REFER TO THE DISTANCE FROM THE & OF IL 127 TO THE FACE OF THE TEMPORARY CONCRETE BARRIER.

IF THE CONTRACTOR ELECTS TO DELAY BITUMINOUS RESURFACING UNTIL AFTER STAGE CONSTRUCTION IS COMPLETE, TEMPORARY RAMPS AND AGGREGATE PRIME COAT WILL BE REQUIRED. THE COST OF THESE ITEMS WILL NOT BE PAID FOR SEPERATELY BUT SHALL BE INCLUDED IN THE COST OF THE VARIOUS RESURFACING ITEMS.



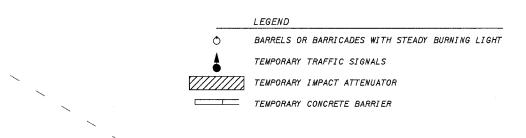
STAGE II CONSTRUCTION

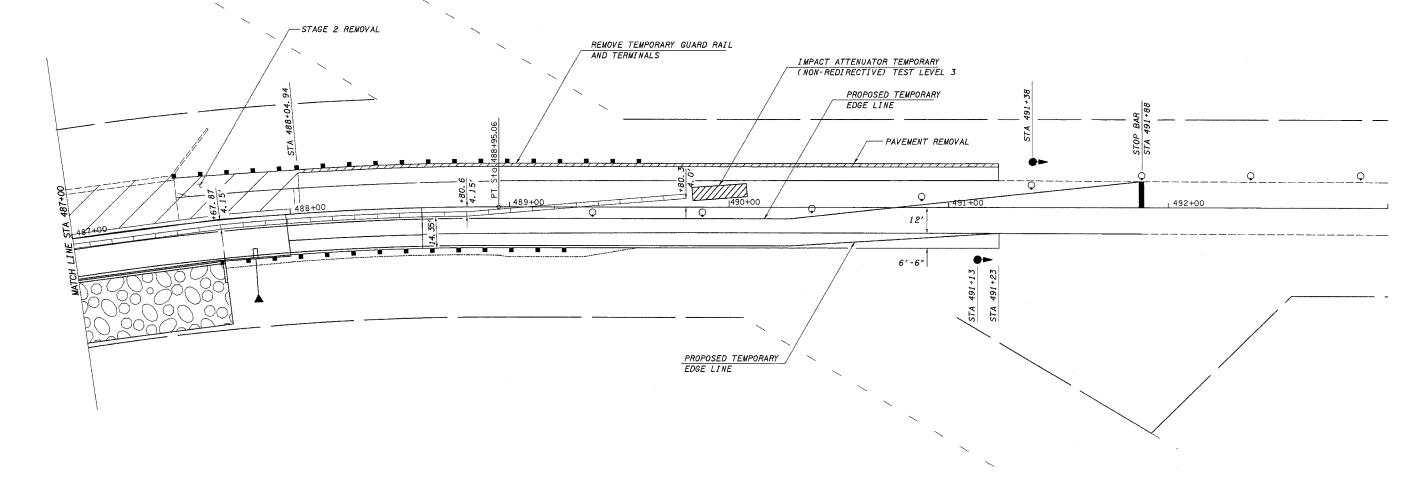
FAP ROUTE 42 (IL 127) SECTION 2BR WASHINGTON COUNTY

CUMMINS ENGINEERING CORPORATION



CONTRACT NO. 76389





TEMPORA	RY PAVEMENT	MARKINGS				
LOCATION	1			LINE TYPE & COLOR	PAVEMENT MARKING TAPE, TYPE III 4" FOOT	WORKZONE PAVEMENT MARKING REMOVAL SQ FT
STAGE 2						
12' RT	STA 482+72	TO 12' RT	STA 483+27	WHITE EDGE LINE	56	19
12' RT	STA 483+27	TO 18.5' RT	STA 485+70	WHITE EDGE LINE	243	81
18.5′ RT	STA 485+70	TO 18.5' RT	STA 490+28	WHITE EDGE LINE	<i>458</i>	<i>152</i>
18.5' RT	STA 490+28	TO 12' RT	STA 491+23	WHITE EDGE LINE	96	<i>32</i>
12.0' RT	STA 491+23	TO 12' RT	STA 491+88	WHITE EDGE LINE	66	22
12' LT	STA 482+72	TO 4.15' RT	STA 485+70	WHITE EDGE LINE	299	99
4.15' RT	STA 485+70	TO 4.15' RT	STA 490+28	WHITE EDGE LINE	<i>458</i>	<i>152</i>
4.15' RT	STA 490+28	TO 12' LT	STA 491+88	WHITE EDGE LINE	161	54
TOTAL					1,837	611

QUANTITIES FOR PLACEMENT AND REMOVAL OF PAVEMENT MARKING TAPE TYPE III, 4" ARE INCLUDED FOR INFORMATION ONLY. PLACING AND REMOVING THESE ITEMS WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST FOR TRAFFIC CONTROL AND PROTECTION STANDARD 701321.

SHORT-	TERM	PAVEMENT	MARKING

LOCATION	LINE TYPE & COLOR	SHORT-TERM PAVEMENT MARKING FOOT	WORKZONE PAVEMENT MARKING REMOVAL SQ FT
STA 482+72 TO STA 491+88 STA 483+27 TO STA 491+23	YELLOW CENTERLINE WHITE EDGE LINE	100 64	33 22
TOTAL		164	55

SHORT-TERM PAVEMENT MARKING SHALL BE INSTALLED PRIOR TO REMOVAL OF TEMPORARY CONCRETE BARRIER AND TRAFFIC CONTROL DEVICES USED FOR STAGE 2 CONSTRUCTION.

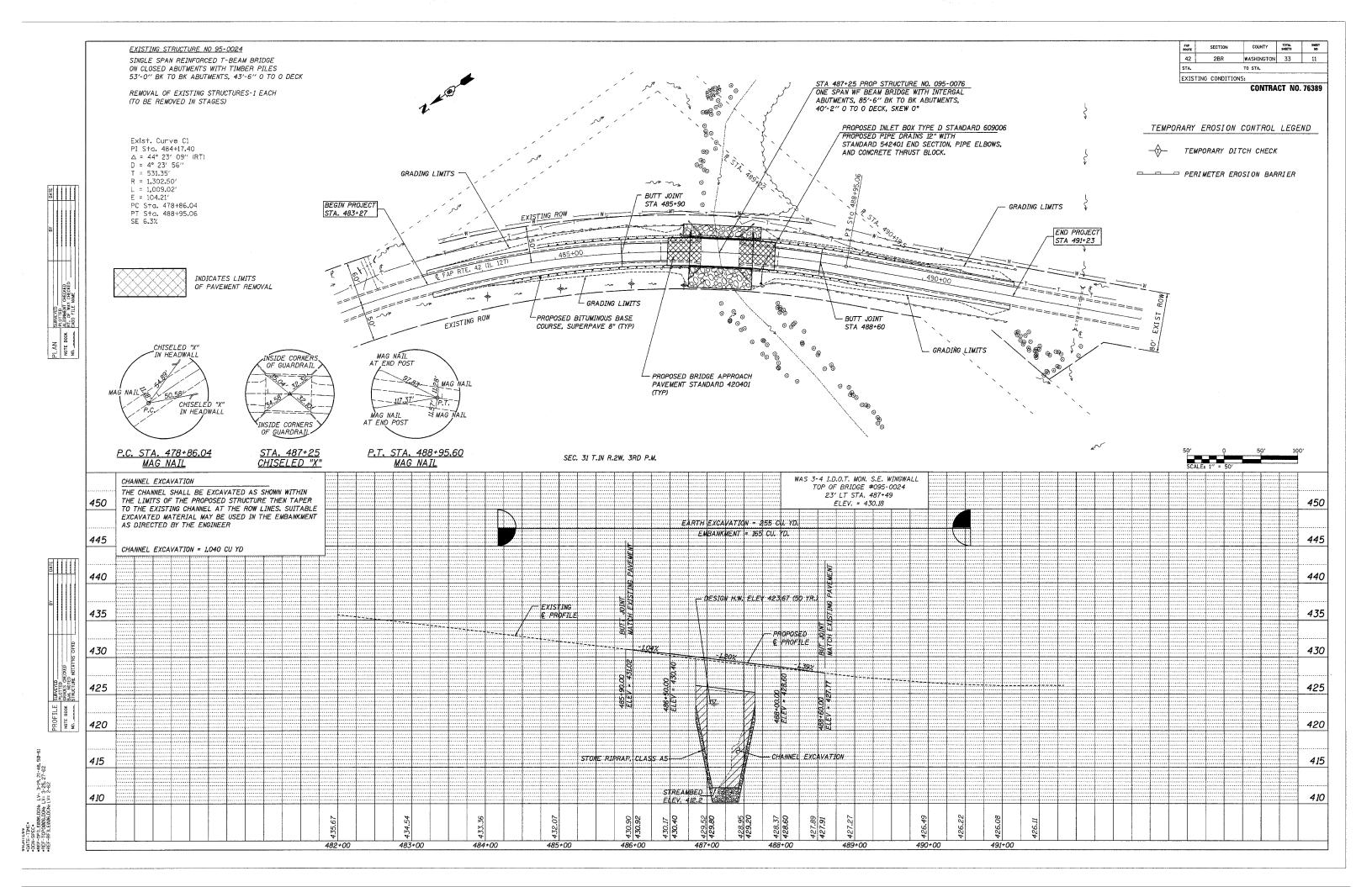


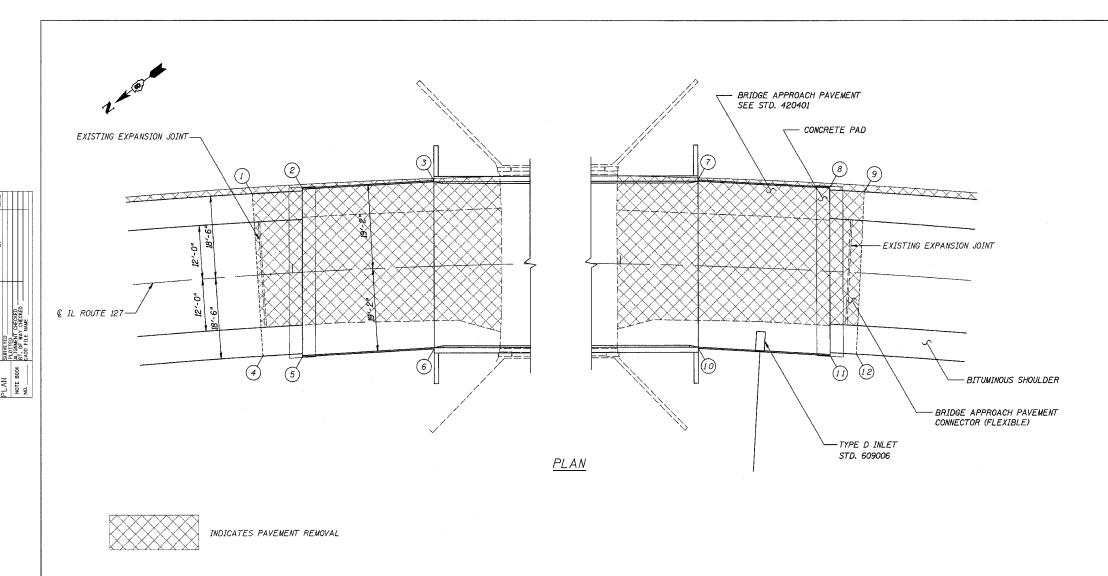
STAGE II CONSTRUCTION

FAP ROUTE 42 (IL 127) SECTION 2BR WASHINGTON COUNTY

CUMMINS ENGINEERING CORPORATION

FILE: Stage2
DATE: 3/17/05





PROPOSED BITUMINOUS BINDER AND SURFACE BRIDGE APPROACH PAVEMENT BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE) EXISTING 9" P.C.C. PAVEMENT--CONCRETE PAD EXISTING 212" BIT. BINDER & SURF.

SECTION THROUGH APPROACH

NOTE: SEE STANDARD 420401 FOR DETAILS NOT SHOWN.

PROPOSED BRIDGE APPROACH PAVEMENT SHALL BE GROOVED AS SPECIFIED IN SECTION 503 OF THE STANDARD SPECIFICATIONS.

BRIDGE DECK GROOVING

228 SQ YD

SECTION COUNTY TOTAL SHEETS 42 2BR STA. TO STA. EXISTING CONDITIONS:

CONTRACT NO. 76389

TABLE OF OFFSETS								
BRIDGE APPROACH PAVEMENT								
NO.	STA.	OFFSET *						
1	SEE CON	NECTORS						
2	486+53.26	19.17 LT						
3	486+82.87	19.51 LT						
4	SEE CON	NECTORS						
5	486+51.12	19.17 RT						
6	486+81.61	18.81 RT						
7	487+67.13	19.51 LT						
8	487+96.74	19.17 LT						
9		NECTORS						
10	487+68.39	18.81 RT						
11	487+98.88	19.17 RT						
12	SEE CONNECTORS							
	IECTOR PAVE	MENT						
NO.	STA.	OFFSET						
1	486+42.00	18.5 LT						
4	486+42,00	18.5 RT						
9	488+04.94	18.5 LT						
12	488+04.94	18.5 RT						

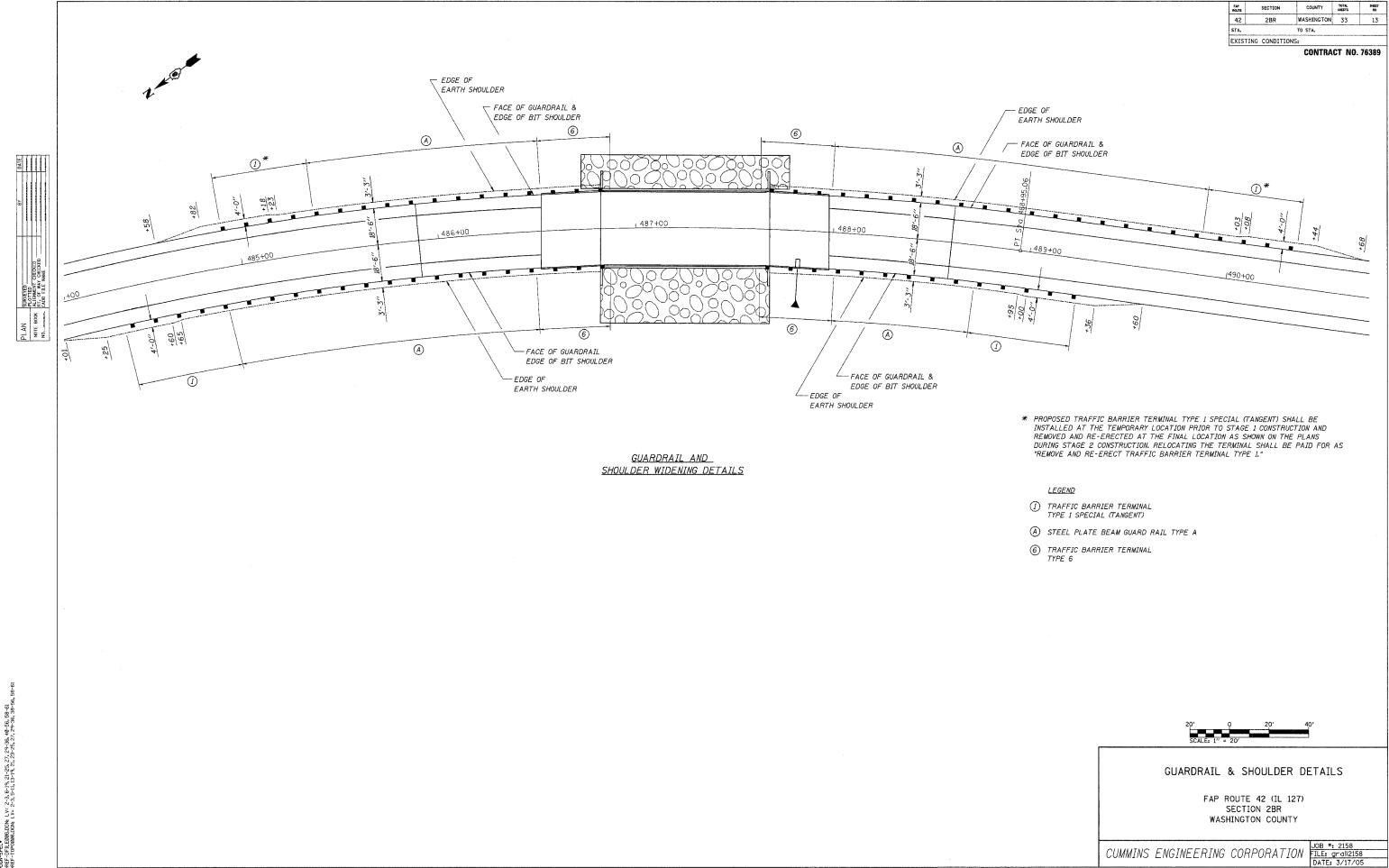
* OFFSETS REFER TO THE DISTANCE FROM © TO THE BACK OF CURB.



BRIDGE APPROACH PAVEMENT DETAILS

FAP ROUTE 42 (IL 127)
SECTION 2BR
ILLINOIS DEPT. OF TRANSPORTATION
DISTRICT 8 WASHINGTON COUNTY

CUMMINS ENGINEERING CORPORATION JOB *: 2158
FILE: APPRPVT
DATE: 3/17/05



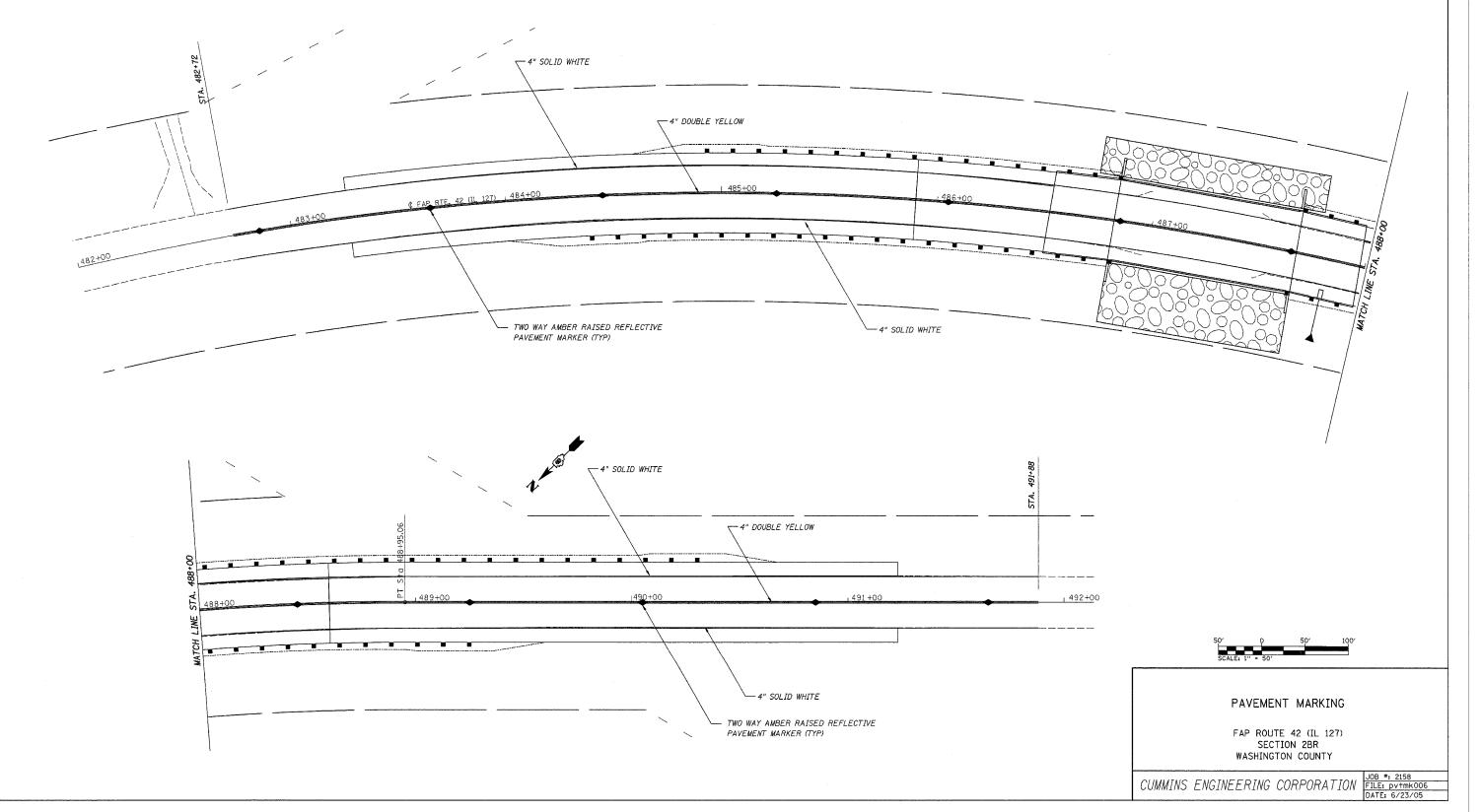
LOCATION	THERMOPLASTIC PAVEMENT MARKING - LINE 4"			POLYUREA PAVEMENT MARKING - LINE 4"			RAISED REFLECTIVE	PAVEMENT MARKERS
CTATION TO CTATION	WHITE EDGE LINE (LEFT)	YELLOW DOUBLE (CENTERLINE)	WHITE EDGE LINE (RIGHT)	WHITE EDGE LINE (LEFT)	YELLOW DOUBLE (CENTERLINE)	WHITE EDGE LINE (RIGHT)	REMOVE	PROPOSED (PAVEMENT)
STATION TO STATION	F00T	F00T	FOOT	FOOT	FOOT	F00T	EACH	EACH
STA. 482+72 TO STA. 486+52.24	381	761	381	0	О	0	5	5
STA. 486+52.24 TO STA. 487+97.76	0	0	0	146	291	146	2	2
STA. 487+97.76 TO STA. 491+88	391	781	391	0	0	0	5	5
TOTAL	772	1,542	772	146	291	146	12	12

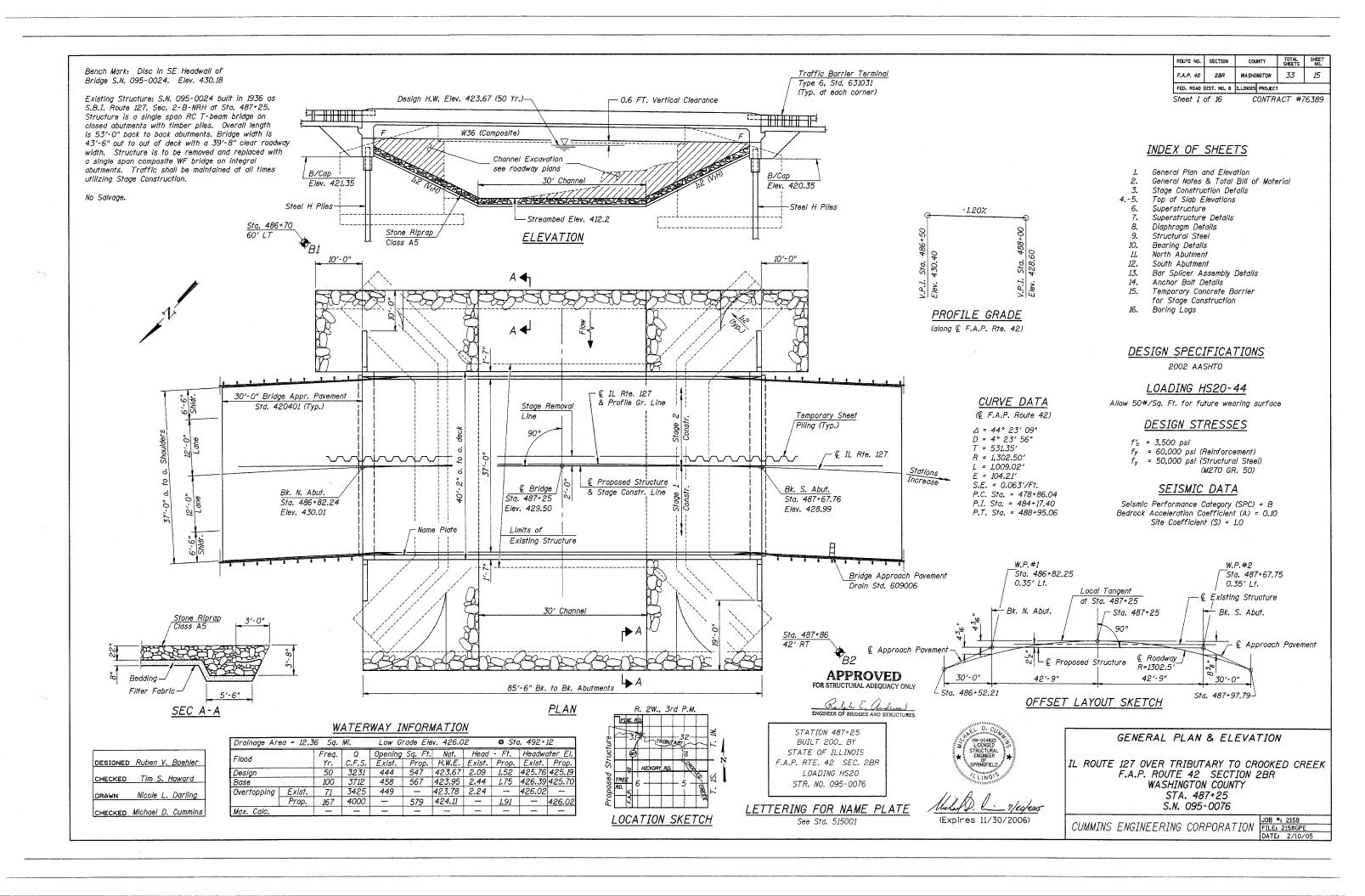
FAP ROUTE	SECTION	COUNTY	TOTAL SNEETS	SHEET NO
42	2BR	WASHINGTON	33	14
STA.		TO STA.		

CONTRACT NO. 76389

EXISTING CONDITIONS:

1





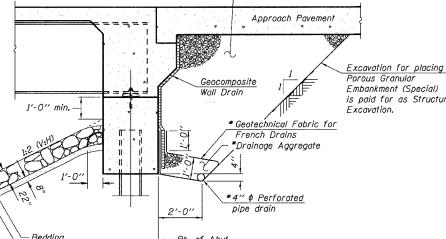
GENERAL NOTES

- Fasteners shall be high strength bolts. Bolts 34 " ϕ , open holes ${}^{15}6$ " ϕ , unless otherwise noted.
- Calculated weight of Structural Steel = 91,130 pounds.
- Field welding of construction accessories will not be permitted to beams or girders. .3.
- The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These components are the wide flange beams.
- Reinforcement bars shall conform to the requirements of AASHTO M31 or M322 Grade 60.
- Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.
- The contractor shall drive two HP12x53 test piles in permanent locations, one at each abutment as directed by the Engineer before ordering the remainder of piles.
- In addition to all other requirements of Section 512 of the Standard Specifications, splices for steel H-piles shall develop the full capacity of the steet's cross sectional area of the pile for tension, shear and bending forces. One approved method of achieving this requirement is full penetration butt welding of the entire cross section. Other types of splices meeting the full capacity requirement may be allowed subject to the approval of the Engineer. Any proposal by the Contractor to use an alternate splice method must include adequate documentation demonstrating that the full tension, shear and bending capacities will be met. Appropriate welder qualifications will be required for the positions and processes used in splicing all piles. Nondestructive testing of completed welds will be limited to visual inspection.
- All construction joints shall be bonded.

CHECKED Michael D. Cummins

- 10. Excavation behind existing closed abutment walls shall be done before removing the existing superstructure. The Contractor shall sawcut the existing abutments at the stage removal line before Stage I removal.
- The inorganic zinc rich primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be gray, Munsell No 5B 7/1. See special provision for Cleaning and Painting New Metal Structures.

Backfill with uncompacted Porous Granular Embankment (Special) by Bridge Contractor after superstructure is in place.

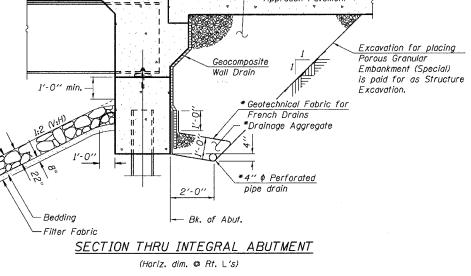


*Included in the cost of Pipe Underdrains for Structures.

Stone Riprap

Class A:

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).



Porous Granular Embankment (Special) Cu. Yd. Sq. Yd. 970 Stone Riprap, Class A5 Sq. Yd. 970 970 ilter Fabri Removal of Existing Structures Each 1 230 Structure Excavation Cu. Yd. Concrete Structures Cu. Yd. 40.2 40.2 Concrete Superstructure Cu. Yd. 132.4 132.4 Bridge Deck Grooving Sq. Yd. 331 331 Sq. Yd. 423 423 Protective Coat urnishing and Erecting Structural Steel L. Sum 1350 Stud Shear Connectors Each Reinforcement Bars, Epoxy Coated Pound 25550 5940 31490 335 335 Furnishing Steel Piles HP12x53 Foot 335 335 Driving Steel Piles Foot Test Pile Steel HP12x53 Each Temporary Sheet Piling Sq. Ft. 1365 1365 Name Plates Each Bar Splicers Each 360 374 Geocomposite Wall Drain Sq. Yd. 83 150 150 Pipe Underdrains for Structures 4' Foot

ROUTE NO. SECTION

F.A.P. 42 2BR

Sheet 2 of 16

TOTAL BILL OF MATERIAL

ITFM

FED. ROAD DIST. NO. 8 ILLINOIS PROJECT

UNIT SUPER SUB TOTAL

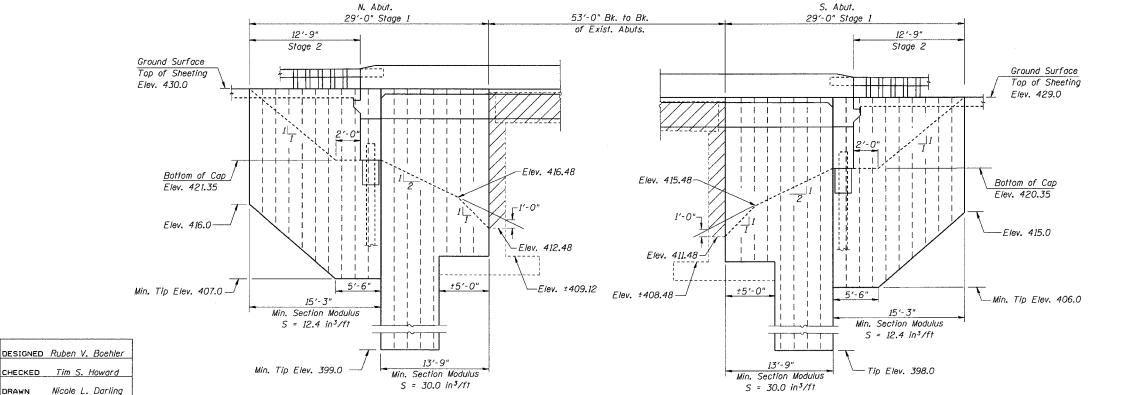
COUNTY

WASHINGTON

TOTAL SHEET SHEETS NO.

33

CONTRACT #76389



TEMPORARY SHEET PILING DETAIL

(Looking East)

Notes:

Hatched area indicates limits of Removal of Existing Structures.

If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer.

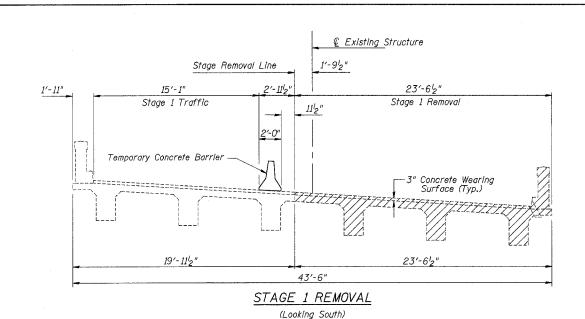
The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling.

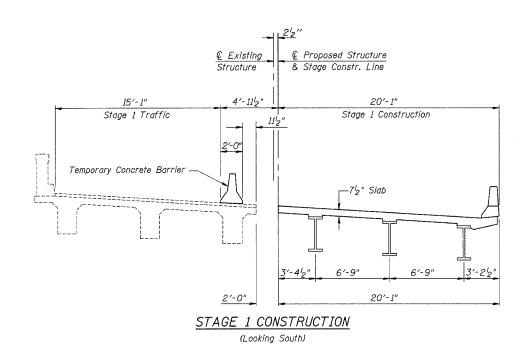
Hard driving may be encountered during the sheet piling installation. The Contractor shall provide the appropriate driving equipment for the soil conditions indicated on the boring logs.

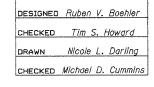
GENERAL NOTES & TOTAL BILL OF MATERIAL

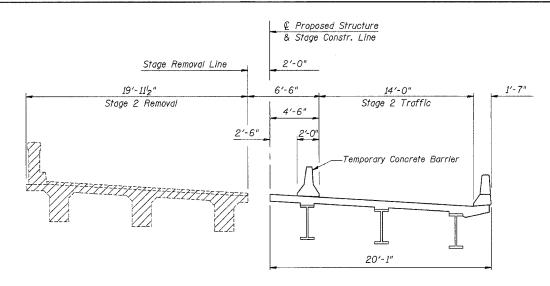
IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION JOB *: 2158
FILE: 2158BILLMAT
DATE: 5/30/06



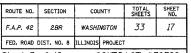




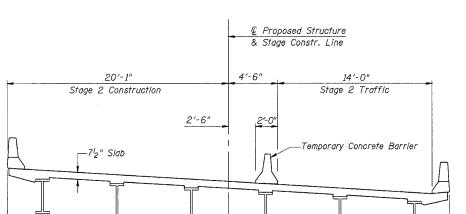


STAGE 2 REMOVAL

(Looking South)



Sheet 3 of 16 CONTRACT #76389



STAGE 2 CONSTRUCTION

40'-2" Out to Out Deck

(Looking South)

Notes:

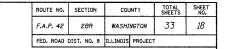
20'-1"

Hatched areas indicate Removal of Existing Structures. For details of Temporary Concrete Barrier, see sheet 15 of 16. See Roadway Plans for quantity of Temporary Concrete Barrier.

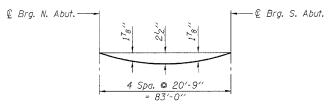
STAGE CONSTRUCTION DETAILS

IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION JOB *: 2158
FILE: 2158STAGE
DATE: 3/14/05



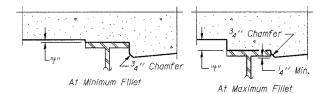
CONTRACT #76389 Sheet 4 of 16



DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

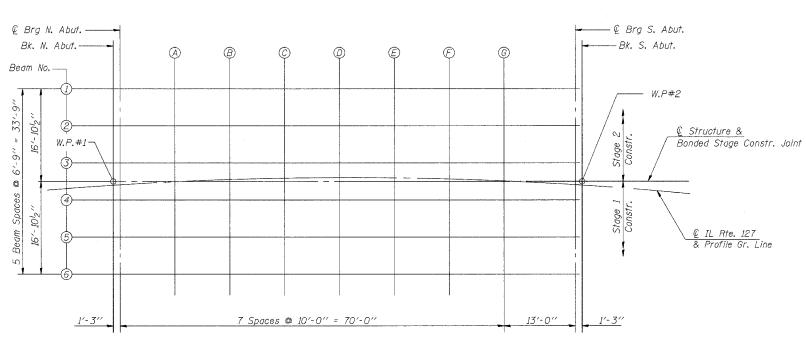
Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown on sheet 5 of 16.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheet 5 of 16, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS





<u>PLAN</u>

Work this sheet with sheet 5 of 16.

TOP OF SLAB ELEVATIONS

IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION

JOB *: 2158
FILE: 2158SLAB
DATE: 2/10/05

DESIGNED Ruben V. Boehler CHECKED Tim S. Howard DRAWN Nicole L. Darling CHECKED Michael D. Cummins

E-S 4-30-97

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.P. 42	2BR	WASHINGTON	<i>3</i> 3	19
FED. ROAD	DIST. NO. 8	ILLINOIS PROJECT	-	***************************************

Sheet 5 of 16

CONTRACT #76389

<u>BEAM 1</u>

BEAM 2

BEAM 3

© STRUCTURE & BONDED STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. N. Abut.	48682.800	- 17.219	431.091	431.091
₡ Brg. N. Abut.	48684.033	- 17.179	431.074	431.074
A B C D E F G	48693.901 48703.771 48713.644 48723.519 48733.393 48743.267 48753.139	- 16.902 - 16.701 - 16.576 - 16.527 - 16.553 - 16.656 - 16.834	430.938 430.807 430.681 430.559 430.442 430.330 430.223	431.011 430.953 430.859 430.767 430.630 430.488 430.318
₡ Brg. S. Abut.	48765.967	- 17.179	430.091	430.091
Bk. S. Abut.	48767.200	-17.219	430.078	430.078

L	Location			Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
	Bk.	N.	Abut.	48682.583	- 10.472	430.669	430.669		
Œ.	Brg.	Ν.	Abut.	48683.823	-10.432	430.651	430.651		
			A B C D E F G	48693.741 48703.662 48713.586 48723.511 48733.437 48743.361 48753.283	-10.154 -9.952 -9.826 -9.777 -9.804 -9.906 -10.085	430.515 430.383 430.256 430.134 430.016 429.904 429.796	430.588 430.529 430.435 430.342 430.204 430.062 429.891		
©	Brg.	s.	Abut.	48766.177	-10,432	429.663	429,663		
	Bk.	s.	Abut.	48767.417	-10.472	429.651	429.651		

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection				
Bk. N. Abut.	48682.364	- 3.726	430.246	430.246				
© Brg. N. Abut.	48683.610	- 3.685	430.229	430.229				
A B C D E F G	48693.579 48703.552 48713.527 48723.503 48733.480 48743.456 48753.429	-3.406 -3.203 -3.077 -3.027 -3.054 -3.157 -3.337	430.092 429.959 429.832 429.709 429.591 429.477 429.369	430.165 430.105 430.010 429.917 429.778 429.636 429.464				
© Brg. S. Abut.	48766.390	- 3.685	429.236	429.236				
Bk. S. Abut.	48767.636	-3.726	429,223	429.223				

	3 377102	. 00/10/	710071071	
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjuste For Dead Load Deflection
Bk. N. Abut.	48682.254	-0.353	430.035	430.035
& Brg. N. Abut.	48683.503	-0.312	430.018	430.018
A	48693.498	-0.032	429.880	429.953
В	48703.496	0.172	429.747	429.893
С	48713.497	0.298	429.619	429.798
D	48723.500	0.348	429.496	429.704
E	48733.502	0.321	429.378	429.565
F	48743.504	0.218	429.264	429.423
G	48753.503	0.037	429.156	429.251
© Brg. S. Abut.	48766.497	-0.312	429.022	429.022
Bk. S. Abut.	48767.746	-0.353	429.009	429,009

BEAM 4

BEAM 5

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. N. Abut.	48682.143	3.021	429.824	429.824
© Brg. N. Abut.	48683.395	3.061	429.806	429,806
A B C D E F G	48693.416 48703.440 48713.467 48723.496 48733.524 48743.552 48753.577	3.342 3.546 3.673 3.723 3.696 3.592 3.411	429.668 429.535 429.407 429.283 429.165 429.051 428.942	429.742 429.681 429.586 429.492 429.352 429.209 429.037
© Brg. S. Abut.	48766.605	3.061	428.808	428.808
Bk. S. Abut.	48767.857	3.021	428,795	428.795

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. N. Abut.	48681.919	9.767	429.402	429.402
© Brg. N. Abut.	48683.178	9.808	429.384	429.384
Α	48693.251	10.090	429,245	429.318
В	48703.328	10.295	429.111	429.258
С	48713.407	10.423	428.982	429.161
D	48723,488	10.473	428.858	429.066
Ε	48733.569	10.446	428.739	428.927
F	48743.649	10.342	428.625	428.783
· G	48753.726	10.160	428 . 515	428.610
© Brg. S. Abut.	48766.822	9.808	428.380	428 . 380
Bk. S. Abut.	48768.081	9.767	428.368	428.368

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. N. Abut.	48681.693	16.513	428.979	428.979
© Brg. N. Abut.	48682.958	16.554	428.962	428.962
A	48693.084	16.838	428.822	428.895
В	48703.214	17.044	428.688	428.834
С	48713.346	17.173	428.558	428.737
D	48723.480	17.223	428.433	428.641
Ε	48733.614	17.196	428.313	428.501
F	48743.747	17.091	428.198	428.357
G	48753.877	16.908	428.088	428.183
€ Brg. S. Abut.	48767.042	16.554	427.953	427.953
Bk. S. Abut.	48768.307	16.513	427.940	427.940

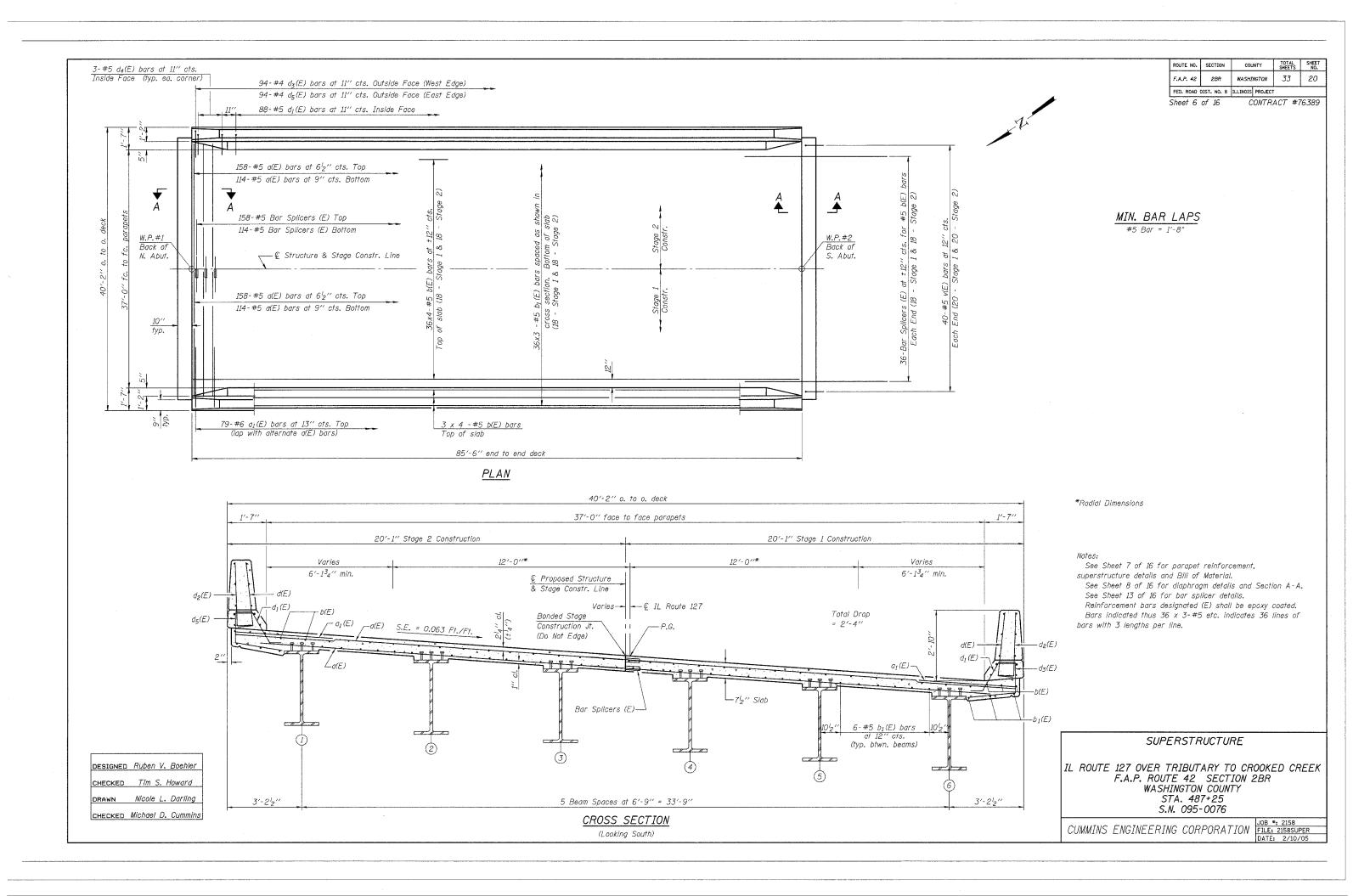
Work this sheet with sheet 4 of 16.

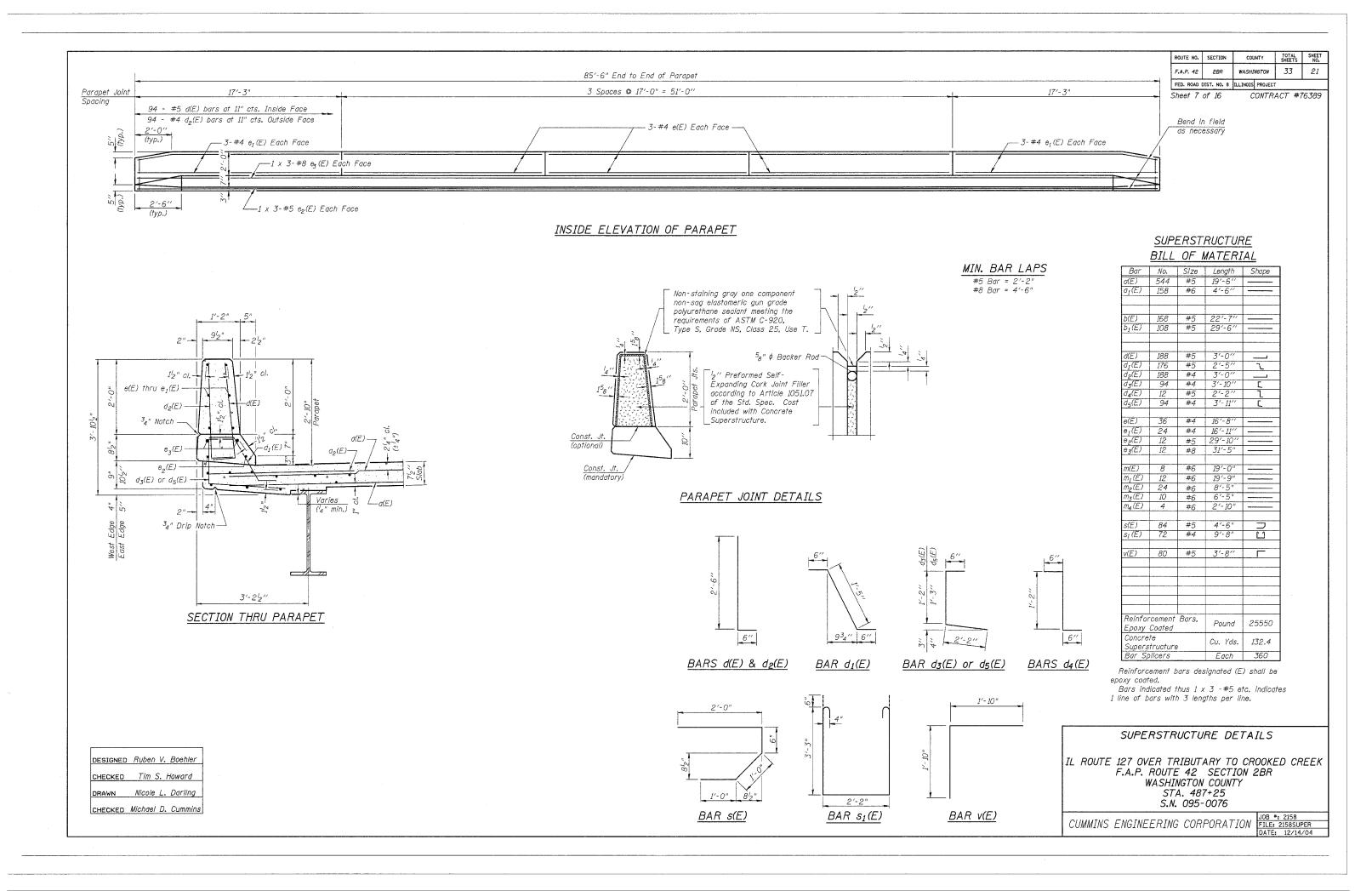
TOP OF SLAB ELEVATIONS

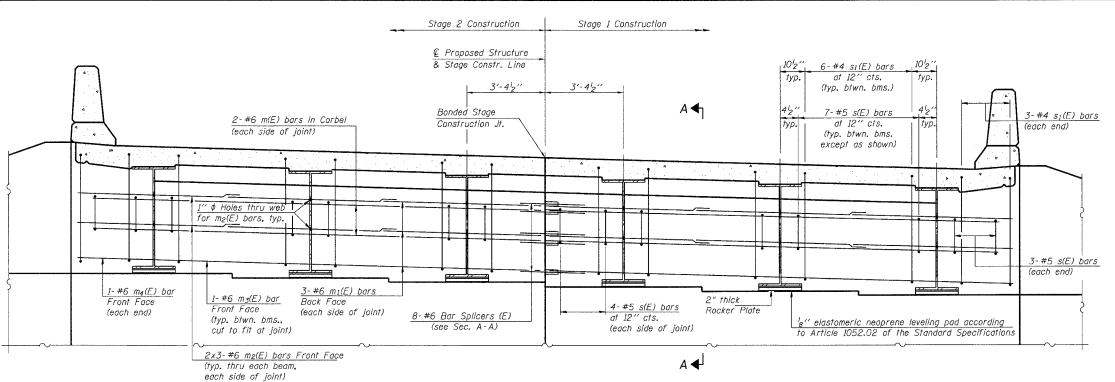
IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION

DESIGNED Ruben V. Boehler CHECKED Tim S. Howard DRAWN Nicole L. Darling CHECKED Michael D. Cummins







DIAPHRAGM ELEVATION AT SOUTH ABUTMENT

(Looking South) (North Abutment Similar)

Notes:

DESIGNED Ruben V. Boehler

CHECKED Tim S. Howard

DRAWN Nicole L. Darling

CHECKED Michael D. Cummins

Reinforcement bars in diaphragm are billed with superstructure on sheet 7 of 16.

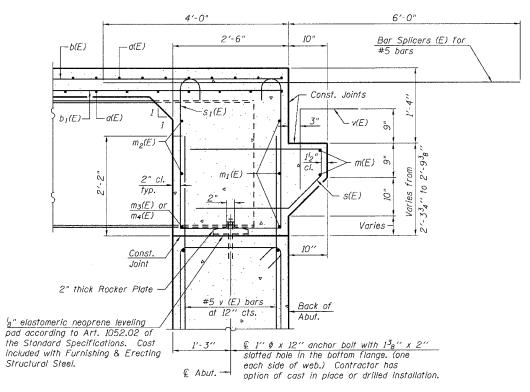
Concrete in diaphragm is included with Concrete Superstructure on sheet 7 of 16.

For details of bars s(E) & $s_1(E)$ see sheet 7 of 16. The s(E) and $s_1(E)$ bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.

For anchor bolt details see sheet 14 of 16.

Bars indicated thus 2x3 - #6 etc. indicates 2 lines of bars with 3 lengths per line.

> MIN. BAR LAP #6 bar = 2'-9"



SECTION A-A

*Cost included with Concrete Superstructure.

DIAPHRAGM DETAILS

ROUTE NO. SECTION

Sheet 8 of 16

2BR

FED. ROAD DIST. NO. 8 ILLINOIS PROJECT

F.A.P. 42

COUNTY

WASHINGTON

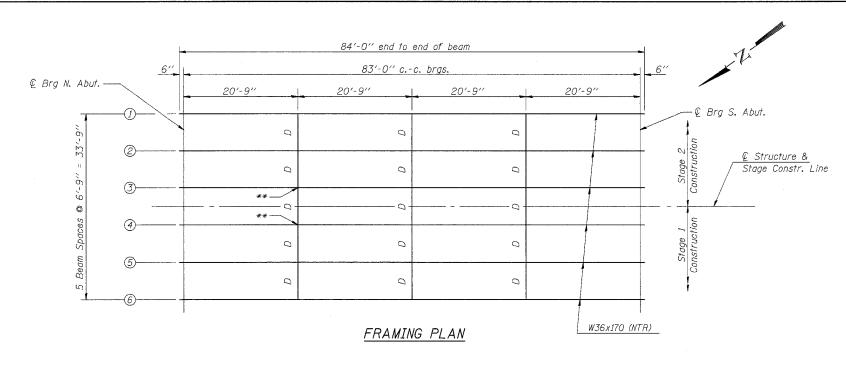
TOTAL SHEETS

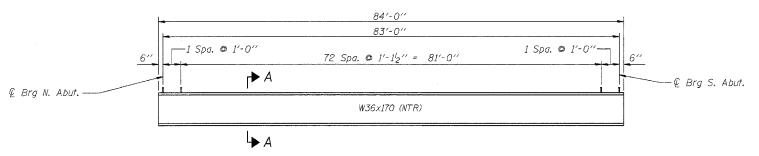
33

CONTRACT #76389

IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

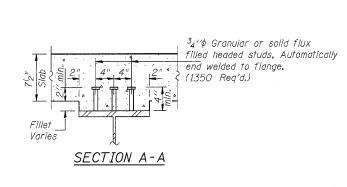
CUMMINS ENGINEERING CORPORATION JOB *: 2158
FILE: 2158SUPER
DATE: 5/30/06



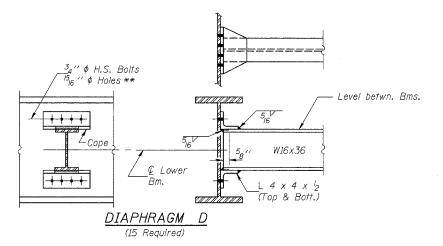


BEAM ELEVATION

(Showing Shear Connector Spacing)



DESIGNED Ruben V. Boehler CHECKED Tim S. Howard DRAWN Nicole L. Darling CHECKED Michael D. Cummins



** Use $^{13}_{16}$ " wide x $^{12}_{2}$ " long slotted holes in L's for diaphragm connections between beams 3 & 4. Bolts shall be finger tightened prior to deck pour for stage 2 construction and then fully tightened after completion of deck pour for stage 2 construcion.

ROUTE NO.	SECTION	COL	INTY	TOTAL SHEETS	SHEET NO.
F.A.P. 42	2BR	WASH!	NGTON	33	23
FED. ROAD	DIST. NO. 8	ILLINOIS	PROJECT		

Sheet 9 of 16 CONTRACT #76389

Is and Ss are the moment of inertia and section modulus of the steel section used in computing fs (Overload). Ic(n) and Sc(n) are the moment of inertia and section modulus of the composite section used in computing stresses due to Live Load.

Ic(3n) and Sc(3n) are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead loads. (see AASHTO 10.38) VR is the maximum Live Load + Impact shear range in span.

The Plastic Moment capacity (Mu) is computed according to AASHTO 10.48.1 and 10.50.1.1.

fs (Overload) is the sum of the stresses due to $MQ + MsQ + {}^{5}_{3}(M + M(Imp)).$

MP - Moment due to dead loads on non-composite section. Ms₽ - Moment due to dead loads on composite section.

Mt - Moment due to live loads on composite section. M (Imp) - Moment due to live load impact on composite section. Ma (Applied Moment)=1.3[M ℓ + Ms ℓ + δ_3 (M ℓ + M(Imp))].

INTERIOR	GIRDER I	REACTION	TABLE
		Abut.	5.
R₽	(k)	<i>54.</i> 3	3
R4	(k)	39.3	3
Imp.	(k)	9.4	
R (Total)	(k)	103.0)

INTERIOR GIRDE	R MOMENT TABLE
	0.5.0
	0.5 Span
Is (in	
Ic (n) (ir	n ⁴) 24511
	14) 18004
Ss (ii	³) 580
Sc (n) (li	n ³) 800
	n ³) 724
Q (k/f	7.) 0.850
	(k) 732
se (k/f	
Ms₽ ((k) 394
MŁ ((k) 748
M(Imp) ((k) 180
53[M \ + M(Imp)] (′k) 1547
Ma ((k) 3474
Mu ((k) 3846
fs@ non-comp(k.s.	.i.) 15.1
fsℓ(comp) (k.s.	
$fs_{3}(4+Imp)$ (k.s.	i.) 23.2
fs (Overload) (k.s.	
VR ((k) 48.7

*Compact, Braced Section

All steel for beams, diaphragms and connection L's shall be

Two hardened washers shall be required over all 1516 " \$\phi\$ holes and two $1^l{}_2{}^{\prime\prime}$ x $1^l{}_2{}^{\prime\prime}$ x $^5{}_{l6}{}^{\prime\prime}$ R washers shall be required over all

"NTR" denotes members to which Notch Toughness Requirements,

AASHTO M270, Grade 50.

slotted holes for diaphragms.

Zone 2 are applicable.

— € Abut. for m₂(e) bars 1'-0" | 6" -End of Beam

TYP. END OF BEAM ELEVATION

TOP OF BEAM ELEVATIONS

(For Fabrication Only)

		,
Location	€ Brg. N. Abut.	€ Brg. S. Abut.
Beam 1	430.39	429.39
Beam 2	429.97	428.97
Beam 3	429.55	428,55
Beam 4	429.13	428.13
Beam 5	428.71	427.71
Beam 6	428.29	427.29

STRUCTURAL STEEL

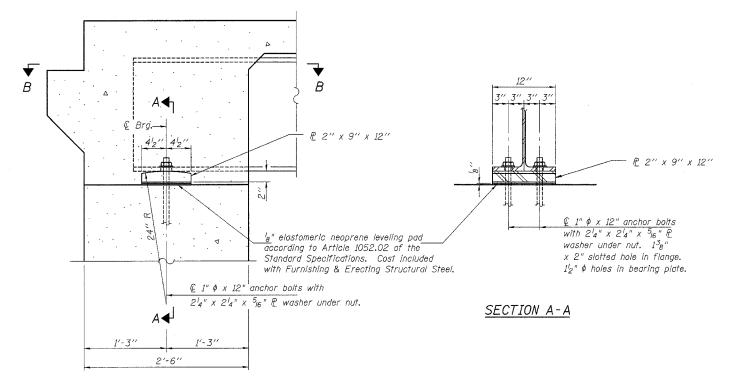
IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION JUBE #: 2158S | FILE: 2158SS | DATE: 2/10/0

COUNTY TOTAL SHEET SHEETS NO. ROUTE NO. SECTION F.A.P. 42 2BR WASHINGTON 33 24 FED. ROAD DIST. NO. 8 ILLINOIS PROJECT

Sheet 10 of 16

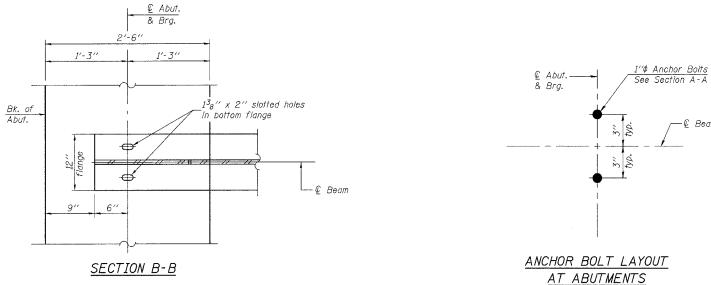
CONTRACT #76389



ELEVATION AT ABUTMENT

ABUTMENT BEARING

(12 Required)



DRAWN Nicole L. Darling CHECKED Michael D. Cummins

DESIGNED Ruben V. Boehler CHECKED Tim S. Howard

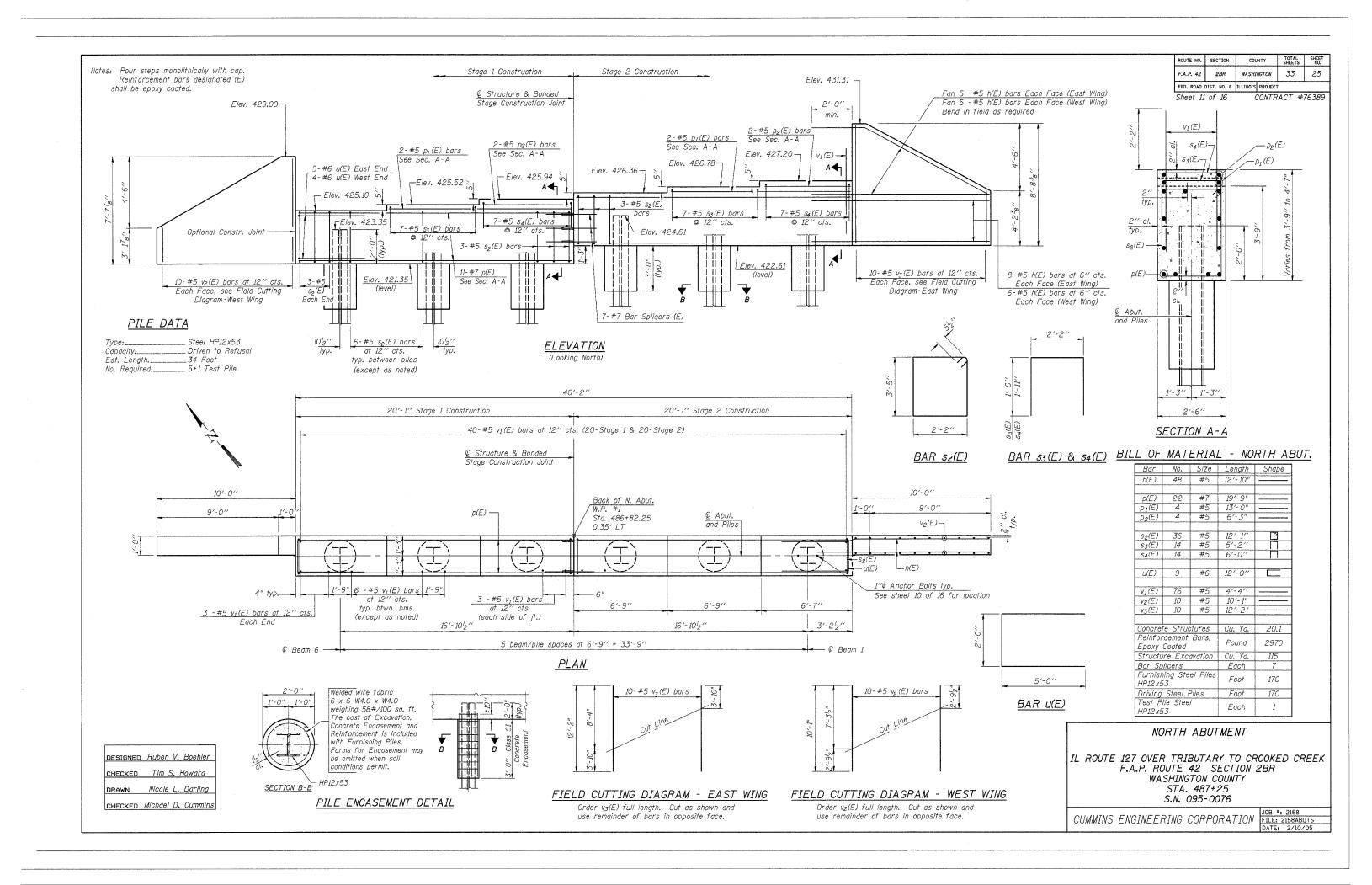
Steel bearing plates shall be AASHTO M270, Grade 50.

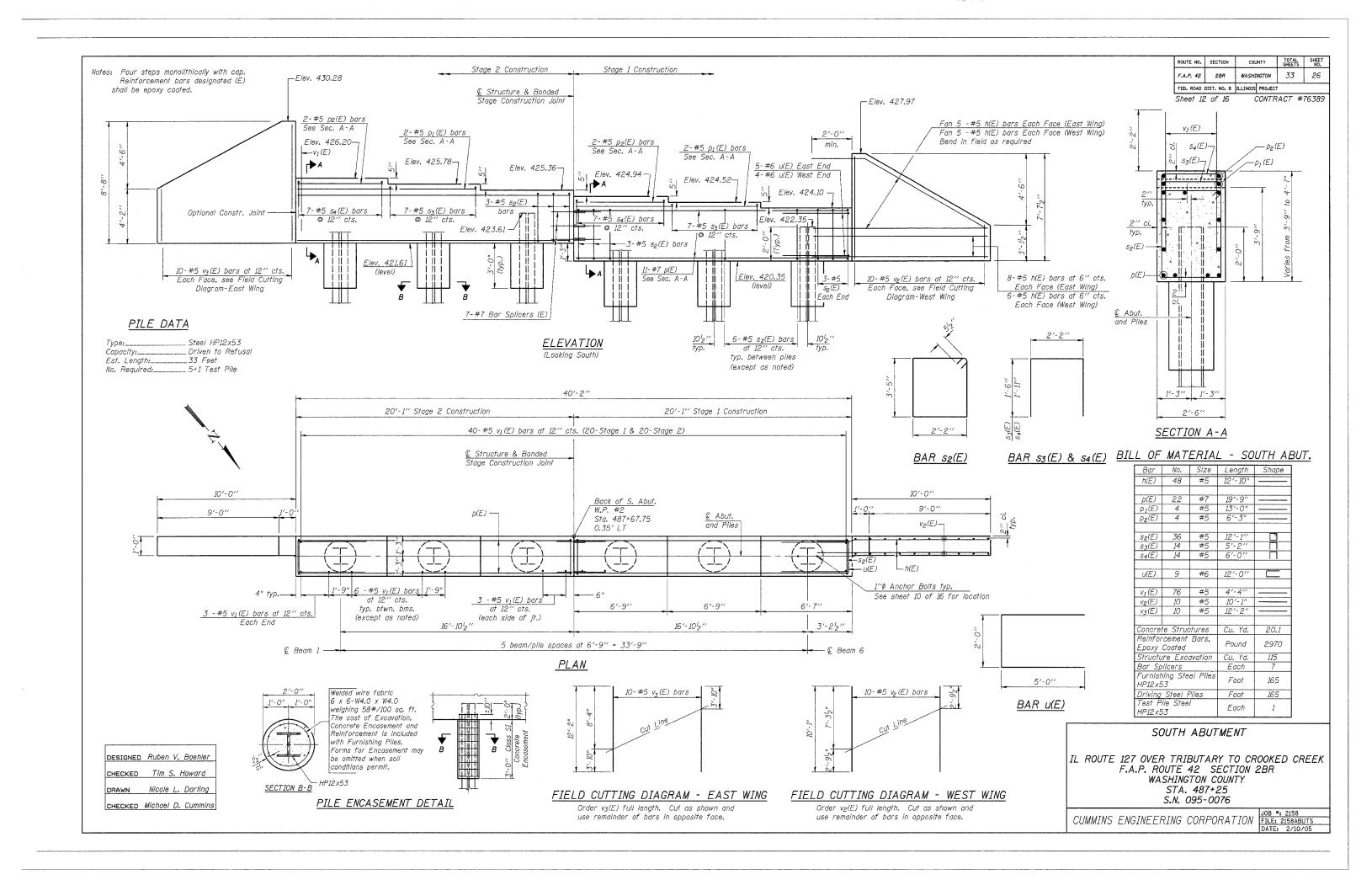
BEARING DETAILS

IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION

JOB *: 2158
FILE: 2158BRG
DATE: 11/12/04







NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity (Tension in kips) = $1.25 \times fy \times A_t$

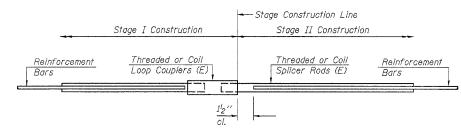
Minimum *Pull-out Strength = 1.25 x fs_{allow} x A_t (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

fs_{allow}= Allowable tensile stress in lapped reinforcement bars in ksi (Service Load) A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

BAR SPLICER ASSEMBLIES					
D		Strengt	Strength Requirements		
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension		
#4	1'-8''	14.7	5.9		
#5	2'-0"	23.0	9.2		
#6	2'-7"	33.1	13.3		
#7	3′-5′′	45.1	18.0		
#8	4′-6′′	58.9	23.6		
#9	5′-9′′	75.0	30.0		
#10	7′-3′′	95.0	38.0		
#11	9'-0"	117.4	46.8		

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



STANDARD

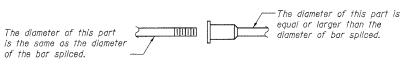
Bar Size	No. Assemblies Required	Location
#5	272	Deck
#6	16	Diaphragms
#7	14	Abutments

BAR SPLICER ASSEMBLY DETAILS

IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION

JOB *: 2158 FILE: 2158BARSPL DATE: 11/16/04



ROLLED THREAD DOWEL BAR

** ONE PIECE - Wire Connector

BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

Bridge Deck

Threaded or Coil

Loop Couplers (E)

Reinforcement

Bars



<u>"A"</u>

- Stage Construction Line

Foam Plugs

Threaded or Coll

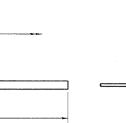
Splicer Rods (E)

<u>Template</u>

Forms-

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



Approach Slab

Threaded or Coil Splicer Rods (E)

6'-0" Approach slab Abutment hatch block Threaded or Coil Threaded or Coil Splicer Rods (E Loop Couplers (E) Reinforcement bars

FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tension No. Required = 72

DESIGNED Ruben V. Boehler CHECKED Tim S. Howard Nicole L. Darling CHECKED Michael D. Cummins

BSD-1 9-01-03 Bar Splicer for #5 bar

FOR PILE BENT ABUTMENTS

No. Required =

Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tension

COUNTY TOTAL SHEET NO. ROUTE NO. SECTION 33 F.A.P. 42 2BR WASHINGTON 28 FED. ROAD DIST. NO. 8 ILLINOIS PROJECT

Sheet 14 of 16

CONTRACT #76389

MATERIALS FOR ILLINOIS COIL-LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold drawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 519, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coll wire shall be made of any suitable soft steel wire. The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed. The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type I, Grade 1 and of a Class suitable for the temperature at installation.

INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

1. With the coil wire in place, the bolt shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete

2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

1. A threaded rod stud with nut and washer of the type specified.

2. A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical.

Location	Type
Abutments	A307

ASTM F 1554 Grade 105, ASTM A 449 and AASHTO M 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above.

GENERAL NOTES

plates to the diameter and depth shown or according to the manufacturer's recommendation after beams or girders have been erected and adjusted. Prior to setting the bolts, the holes shall be dry and all dust and loose particles shall be removed by the use of compressed air or vacuuming. The anchor bolts, furnished and installed and including the epoxy grout or capsules shall not be paid for separately but shall be included in the unit bid price for Furnishing and Erecting Structural Steel.

Holes in the masonry for anchor bolts shall be drilled through the base

ILLINOIS COIL-LOCK ANCHOR BOLT DESIGNED Ruben V. Boehler

End of

″Ε ′′φ

CHECKED Tim S. Howard Nicole L. Darling CHECKED Michael D. Cummins

16" at Bottom_

PLAN-COIL WIRE

The Illinois Coil-Lock Anchor Bolt is a proprietary

item which is the property of the Illinois Department of

Transportation. Use, reproduction or disclosure without express written permission is prohibited and protected

the fabrication of this bolt for use on highway projects

in the State of Illinois shall be permitted and there shall be no incurred charges or fees to the manufacturer or the fabricator for producing or fabricating this bolt.

134"

2"

218'

11316 " 278"

18"

138"

158" 15/6

28"

22"

1/6"

258" 256" 338"

′′d′′¢ Holes with zerk

Anchor Bolt (See Bearing Details

Top of base plate

for number, size and length.)

for epoxy grout

End of aroove

 $\frac{5}{32}$ wide x $\frac{3}{32}$ deep groove in anchor bolt with 8" \$

"D"\$

under Federal copyright laws. The production and

ABB-1 4-30-99 ANCHOR BOLT DETAILS FOR BEARINGS

IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION

JOB *: 2158 FILE: 2158ANCHOR

ROUTE NO. SECTION COUNTY F.A.P. 42 2BR WASHINGTON 33 FED, ROAD DIST, NO. 8 ILLINOIS PROJECT

Sheet 15 of 16

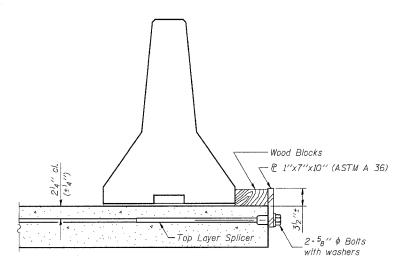
CONTRACT #76389

Temporary Concrete Barrier See Standard 704001 Stage Construction Line — --- Stage Removal Line NJ shape F shape 2'-0"/ NJ shape 1'-10'2 When "A" is 3'-6" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required / See Detail I or Detail II. Drill 14" \$\phi\$ Holes in existing slab for 1" \$\phi\$ x 10" dowel bars. Traffic side only. Cost included [/] Styrofoam Pads (NJ Shape only) See Standard 704001 with Temporary Concrete Barrier.

NEW SLAB

EXISTING SLAB

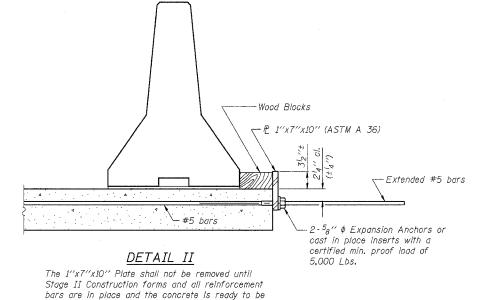
SECTIONS THRU SLAB

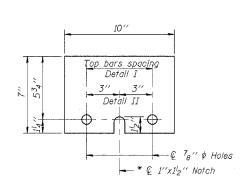


DETAIL I

when "A" is greater than 3'-6".

The 1"x7"x10" Plate shall not be removed until Stage II Construction forms and reinforcement bars are in place.





NOTES

Connect one (1) 1"x7"x10" steel £ to the top layer of couplers with 2-58" \$\phi\$ bolts screwed to coupler at approximate & of

Connect one (1) 1"x7"x10" steel $\mathbb R$ to the concrete slab with 2- 58 " ϕ Expansion Anchors or cast in place inserts spaced between the

top layer of reinforcement at approximate € of

Detail I - With Bar Splicer or Couplers:

each barrier panel. Detail II - With Extended Reinforcement Bars:

each barrier panel.

Cost of anchorage is included with Temporary Concrete Barrier.

P 1"x7"x10"

* Required only with Detail II

DESIGNED Ruben V. Boehler CHECKED Tim S. Howard DRAWN Nicole L. Darling CHECKED Michael D. Cummins

R-27 9-01-03

FOR STAGE CONSTRUCTION IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

TEMPORARY CONCRETE BARRIER

CUMMINS ENGINEERING CORPORATION FILE: 2158BARRIER

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date 10/15/03

ROUTE FAP 42 DESCR	IPTION			-	IL 127	over Coon Creek	LOGGE) BI	<u>Mar</u>	k Schr	eade
SECTION2BR	_ LOC	CATIO	N	NW 1	⁄4, SE	1/4, SEC. 31, TWP. 1N, RNG. 2W, 3 P	M				
COUNTY Washington DF	RILLING	MET	HOD		Holle	ow Stem Auger HAMMER	TYPE _	14	10# <i>F</i>	utoma	tic
STRUCT. NO. 095–0024 Station 1 BORING NO. 1 Station 486+70 Offset 60.00ft Left		P T H	B L O W S	U C S Qu	M 0 1 S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter 410.6 Upon Completion	_ft _ft <u>▼</u>		L 0 W S	၁၀၈ ချ	M O I S T
Ground Surface Elev. 423.1	ft	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(/6")	(tsf)	(%)
Brown Silty CLAY						Gray Silty CLAY (continued)	404.0	_	8 10	2.6 S⁄20	24
			1			Gray Silty CLAY	401.6		4		
	,		2 3	0.6 S∕20	24				6 9	2.3 S⁄20	24
		– 5	2					-25	2		
			5 6	1.6 S⁄20	27				3 4	1.0 B⁄20	25
			2						4		
			3 3	2.2 S⁄20	29				7 11	3.1 S⁄20	20
	413.6	-10	1				393.6	-30	10		
Brown Clay LOAM			2 3	0.8 S⁄20	22	Gray Fine to Medium SAND See Gradation @ 30 ft			15 46	NC	15
	410.6		2				391.1				
Brown Silty Clay LOAM	410.0	<u> </u>	3	1.2 S⁄20	21	Auger Refusal on Limestone – En of Boring	d				
	408.6	 15	4								
Brown Fine to Coarse SAND with some Gravel See Gradation @ 15 ft	h		10 14	NC	18						
	406.1		4								
Gray Silty CLAY			6 10	2.7 \$⁄20	24						
		-20	4					-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

W	of Trai	Departn Isportation
ROUTE	FAP 42	DESCRIPTION
OF OTHER		

ment on

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date ____10/16/03__

LOGGED BY Mark Schreader

IL 127 over Coon Creek LOCATION NW 1/4, SE 1/4, SEC. 31, TWP. 1N, RNG. 2W, 3 PM COUNTY Washington DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic STRUCT. NO. Surface Water Elev. C S C S Stream Bed Elev. Ō W S BORING NO. _ Groundwater Elev.: Qu S Qu Т 487 + 86 First Encounter _419.6 ft Ψ H Station Offset 42.00ft Right **Upon Completion** (ft) (/6") (tsf) (%) (ft) (/6") (tsf) (%) Ground Surface Elev. 430.1 After Hrs. Gray Silty CLAY (continued) 8 4.0 16 S/20 Brown Silty LOAM 9 3.8 22 0.5 22 2 S/20 13 S/20 9 2.3 25 1 0.4 27 2 S/20 13 S/20 4 2.2 23 2 0.6 28 Brown Silty Clay LOAM 2 S/20 5 S/20 420.6 10 4.6 13 Gray Fine to Medium SAND See Gradation @ 10 ft NC 18 S/20 4 Brown Sandy LOAM See Gradation @ 12.5 ft 7 Brown Sandy Clay LOAM See Gradation @ 15 ft 1.5 18 LIMESTONE 50/2" 28 10 S/10 394.1 End of Boring 4 NC 5 2.0 19 Gray Silty CLAY S/20

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

BORING LOGS

ROUTE NO. SECTION

Sheet 16 of 16

2BR

FED. ROAD DIST. NO. 8 ILLINOIS PROJECT

F.A.P. 42

COUNTY

WASHINGTON

33

CONTRACT #76389

30

IL ROUTE 127 OVER TRIBUTARY TO CROOKED CREEK F.A.P. ROUTE 42 SECTION 2BR WASHINGTON COUNTY STA. 487+25 S.N. 095-0076

CUMMINS ENGINEERING CORPORATION JOB #: 2158
FILE: 2158BORING
DATE: 11/16/04

DESIGNED	Ruben V. Boehler
CHECKED	Tim S. Howard
DRAWN	Nicole L. Darling
	Michael D. Cummin

