

4/30/2024 10:22:13 AM



4/30/2024 10:22:19 AM SHEET S03B-26 OF S

S AT R18.1 (SHT. 1 OF 2) 6-0133 (REV)		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		2020-005-BR	соок	908	502	
10-0135 (IXEV)				CONTRA	ACT NO.	62K73
03B-40 SHEETS		ILLINOIS	FED. A	D PROJECT		



4/30/2024 10:22:23 AM

S AT R18.1 (SHT. 2 OF 2)	F.A.I. RTE	RTE. SECTION		COUNTY	SHEETS	NO.	
6-0133 (REV)		2020-005-BR		COOK	908	503	
					CONTRA	ACT NO.	62K73
03B-40 SHEETS			ILLINOIS	FED. A	PROJECT		



4/30/2024 10:22:29 AM



4/30/2024 10:22:32 AM







4/30/2024 10:22:39 AM SHEET S03B-30 OF S

S AT R18.6 (SHT. 1 OF 2) L6-0133 (REV)		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		2020-005-BR	соок	908	506	
10-0133 (IXEV)				CONTRA	ACT NO.	62K73
03B-40 SHEETS		ILLINOIS	FED. A	D PROJECT		



4/30/2024 10:22:42 AM



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ENGINEERING GROUP LLC

PLOT DATE =

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- 4/29/2024 REVISED SHEET S03B-32 OF S0

	INTE:					OTILLIO	110.
-6-0133 (REV)	90/94	2020-0	05 <b>-</b> BR		соок	908	508
0-0133 (I(EV)					CONTRA	ACT NO.	62K73
03B-40 SHEETS			ILLINOIS	FED. A	D PROJECT		



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- steel using existing steel

19.6 SOUTH END (SHT. 2 OF 4)	F.A.I. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
-6-0133 (REV)		2020-005-BR			COOK	908	509
					CONTR	ACT NO.	62K73
03B-40 SHEETS			ILLINOIS	FED. A	AID PROJECT		



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19.6 SOUTH END (SHT. 3 OF 4)		SECTION		COUNTY	SHEETS	SHEET NO.
L6-0133 (REV)		2020-005-BR		COOK	908	510
				CONTR/	ACT NO.	62K73
603B-40 SHEETS		ILLINOIS	FED. A	D PROJECT		



4/30/2024 10:22:56 AM



4/30/2024 10:23:02 AM

ENGINEERING GROUP, LLC

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PLOT DATE =

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4/29/2024

DATE

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**DEPARTMENT OF TRANSPORTATION** 

SHEET S03B-36 OF S

STEEL REPAIRS AT R19.6 NORTH END (SHT. 1 OF 2)	F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-0133 (REV)	90/94	2020-005-BR	соок	908	512	
				CONTR	ACT NO.	62K73
SHEET S03B-36 OF S03B-40 SHEETS		ILLINOIS	FED. A	D PROJECT		



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**DEPARTMENT OF TRANSPORTATION** 

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ENGINEERING GROUP, LLC

.OT SCALE =

PLOT DATE =

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DATE - 4/29/2024

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REVISED -

SHEET S03B-38 OF S03

AL STEEL REPAIRS AT SWAY FRAMES (PIER 19)	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-0133 (REV)	90/94	2020-005-BR	соок	908	514
			CONTR	ACT NO.	62K73
SHEET S03B-38 OF S03B-40 SHEETS		ILLINOIS FED. 4	D PROJECT		



10:23:19 AM

				Location			
	R17.1	R17.4	R17.5	R17.6	R18.1	R18.4	R18.6
Α	7 <sup>1</sup> / <sub>16</sub> "	8½"	8½"	$8^{1/3}$ "	711/16"	7 <i>11/<sub>16</sub>"</i>	7 <sup>11</sup> ⁄16"
В	9"	10"	10"	9"	9"	9"	9"
С	15/16"	$2^{1}/2^{"}$	$2\frac{1}{4}$ "	15%"	13⁄4"	2"	2"
D	-	-	-	-	13/4"	2"	2"
Ε	-	-	-	-	$6\frac{1}{2}''$	6"	6"
F	12"	14"	14"	12"	12"	12"	12"
G							
Н	10"	11"	11"	10"	10"	10"	10"
J	$2\frac{3}{4}$ "	25%"	25⁄%"	2¾"	$2^{1}/_{2}^{"}$	$2\frac{1}{2}$ "	2½"
K	3/"	7 <sub>16</sub> "	716"	3/11	3/3"	3/8"	3/8"
L	15/3"	2"	2"	15⁄;"	15//	2"	2"

ETAILS (SHT. 1 OF 2)	F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
6-0133 (REV)		2020-005-BR	соок	908	515	
0-0133 (ILEV)				CONTR	ACT NO.	62K73
03B-40 SHEETS	ILLINOIS FED. AID PROJECT					



4 22										
E E		USER NAME =	DESIGNED - JJS	REVISED -		ELASTOMERIC BEARING DETAILS (SHT. 2 OF 2)	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEET
D M M			CHECKED - MI	REVISED -	STATE OF ILLINOIS		90/94	2020-005-BR	соок	908 516
N DEL		PLOT SCALE =	DRAWN - JJS	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 016-0133 (REV)				RACT NO. 62K73
MO	ENGINEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -		SHEET S03B-40 OF S03B-40 SHEETS		ILLINOIS FED. A	ID PROJECT	
4/	20/2024 10:22:21 AM									

4/30/2024 10:23:21 AM





\$DATE\$

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ND ELEVATION 33 (SB)		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
		2020-005-BR			соок	908	518
		CONTRAC					2K73
3C-21 SHEETS			ILLINOIS	FED. A	D PROJECT		



PLOT DATE =

K.G.W.

REVISED

CHECKED -

ND ELEVATION		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
33 (SB)	90/94	2020-005-BR		COOK 908		519	
55 (56)				CONTRAC	T NO. 62	2K73	
03C-21 SHEETS	ILLINOIS FED. A			D PROJECT			



\$DATE\$ \$TIME\$



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### GENERAL NOTES

- Fasteners shall be ASTM F 3125 Grade A325 Type 1, hot-dip galvanized bolts. Bolts %" dia., 1. holes 15/6" dia. unless otherwise noted.
- 2. No field welding is permitted except as specified in the contract documents.
- 3. Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity furnished at the unit price bid for the work.
- 5. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- Existing structural steel that will be in contact with new structural steel shall be cleaned and 6. painted prior to erection as required by the Special Provisions "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".
- All new structural steel, connection bolts, nuts and washers shall be hot-dip galvanized. See 7. Special Provisions for "Hot Dip Galvanized for Structural Steel".
- 8. The Contractor shall take the necessary precautions for the protection of passing vehicles, bicycles and pedestrians from falling objects and/or materials until completion of work.
- 9. The Contractor is responsible to remove, support and reinstall all existing electrical conduits interfering with the work. See special provision "Protection and Maintenance of Existing Underpass Luminaires".
- 10. The Contractor shall contact Chandra Libby, the Director of City of Chicago Department of Family Support Services (DFSS) at 312-746-5443 or Chandra.Libby@cityofchicago.org to coordinate the relocation of persons and their personal belongings under the bridges.
- 11. Where existing structural steel is shown to be removed, the Contractor shall exercise care during removal of existing structural steel to ensure that the adjacent structural steel will not be detrimentally impacted. The Contractor shall repair any damage to the existing structural steel caused by his operation as directed by the Engineer at no additional cost to the Department.
- 12. Where removal of existing rivets is required to facilitate the work, remove rivets and replace with the same diameter high strength bolts, unless noted otherwise.
- 13. Burning of existing rivets or bolts will not be permitted when the subject fasteners are installed in existing structural steel to remain in place. Such rivets and bolts shall have the head of the fastener sheared off and the shank driven or drilled out. Care shall be exercised when removing such fasteners so as not to damage the existing structural steel to remain. Removal of such fasteners will not be measured for payment but shall be included in the cost of the associated work. Any damage to the existing structural steel to remain in place due to the removal of such fasteners shall be repaired or replaced as directed by the Engineer at no additional cost to the Department
- 14. All structural steel shall conform to AASHTO Classification M-270 Gr. 36. unless otherwise noted.
- 15. Ends of stiffener angle fill plates shall be clipped and/or ground to match top and bottom flange angle fillets. Cost included in Structural Steel Repair.
- 16. Anchor rods shall be ASTM F1554, Grade 55.
- 17. Anchor rods shall be installed according to Article 521.06 of the Standard Specifications.
- 21. Repairs and replacements shown are based on field inspection. Conditions in the field may have changed. Contractor to verify all components for repair or replacement as directed by the Engineer.
- 22. Contractor shall field verify the required bolt length of thread necessary to install all bolts in accordance with the Standard Specifications and Section 8.2.1 of the 2004 RCSC "Specification for Structural Joints using ASTM A325 or A490 Bolts."
- 23. Prior to ordering any material, the Contractor shall verify in the field all proposed steel dimensions, rivet and bolt spacing, bearing heights and shim plate thickness dimensions.

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2.	The Ioac Bea pro exp
3.	Clea

an exterior faces of existing steel (angles, plates and gusset plates) according to the special provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

4. Construct template for location of existing rivets/bolts and fabrication of fill and repair plates and angles.

5. Place proposed bearing/intermediate stiffener angles with holes centered with centerline of existingrivets/bolts.

6. Field drill holes as required and secure proposed stiffener angles to stringer using erection pins/rods.

7. Starting at one end, remove one rivet/bolt/erection pin/rod at a time and replace with a 7/8" diameter H.S. bolt. Proceed until all rivets/bolts/erection pins (or rods) are replaced.

ITEM	UNIT	SUPER	SUB	TOTAL
Elastomeric Bearing Assembly, Type I	Each	7		7
Elastomeric Bearing Assembly, Type III	Each	1		1
Anchor Bolts, 1¼"	Each	4		4
Jack and Remove Existing Bearings	Each	8		8
Structural Steel Repair	Pound	5,430		5,430
Protect and Maintain Existing Underpass Luminaire	L Sum	0.08		0.08
Temporary Shoring and Cribbing	Each	13		13

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AODEL: \$MODE	TLE NAME: X:\OF

E: X:		USER NAME =	DESIGNED -	J.T.B.	REVISED		GENERAL DATA	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEET SHEETS NO.
- MA	GR@EF		CHECKED -	H.A.	REVISED	STATE OF ILLINOIS	SN 016-0133 (SB)	90/94	2020-005-BR	соок	908 522
E Z	8501 W. Higgins Road; Suite 280	PLOT SCALE =	DRAWN -	D.C.P.	REVISED	DEPARTMENT OF TRANSPORTATION	3N 010-0133 (3B)			CONTRA	CT NO. 62K73
FILE	Chicago, Illinois 60631; (773) 399-0112			K.G.W.	REVISED		SHEET S03C-06 OF S03C-21 SHEETS		ILLINOIS F	ED. AID PROJECT	

### INDEX OF SHEETS

503C-01-503C-05 503C-06 503C-07 503C-09 503C-10 503C-11 503C-12 503C-13 503C-13 503C-14 503C-15 503C-16 503C-17 503C-18 503C-19	General Plan & Elevation I thru V General Data Partial Framing Plan I Partial Framing Plan II Structural Steel Repairs I Structural Steel Repairs II Structural Steel Repairs IV Structural Steel Repairs V Structural Steel Repairs VI Structural Steel Repairs IX Structural Steel Repairs X Structural Steel Repairs X
S03C-18 S03C-19 S03C-20 S03C-21	Structural Steel Repairs X Structural Steel Repairs XI Elastomeric Bearing Details I Elastomeric Bearing Details II

### D CONSTRUCTION SEQUENCE FOR STRINGER REPAIRS

Prior to removal of bearing and intermediate stiffeners, the existing inger shall be temporary supported. See Special Provision for mporary Shoring and Cribbing.

jack capacities for lifting should be based on maximum expected d present during the lift derived from the reactions shown in the am Reaction Table included with the plans. The jack capacity wided should be between 50% and 100% greater than the maximum pected load.

### TOTAL BILL OF MATERIAL



### NOTES:

- 1. For Steel Repair 1, see Sheet S03C-09 and S03C-13.
- 2. For Steel Repair 2A, see Sheet S03C-09.
- 3. For Steel Repair 2B, see Sheet S03C-11.
- 4. For Steel Repair 3, see Sheet S03C-10.
- 5. For Steel Repair 4, see Sheet S03C-11.
- 6. For Steel Repair 5, see Sheet S03C-12.
- 7. For Steel Repair 6, see Sheet S03C-12.
- 8. For Steel Repair 7, see Sheet S03C-12.
- 9. For Steel Repair 16, see Sheet S03C-13.

## BEAM REACTION TABLE

	STRI	NGER	GIRDER
LOADS	514.4	<i>S15.4</i>	G3
RDL (k)	22.5	28.2	165.2
RLL (k)	35.7	39.1	185.1
RIM (k)	9.3	10.1	44.8
R Total (k)	67.5	77.4	395.1

10 W II		USER NAME =	DESIGNED -	J.T.B.	REVISED		PARTIAL FRAMING PLAN I	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEET SHEETS NO.
5 ₽ME	GR@EF		CHECKED -	H.A.	REVISED	STATE OF ILLINOIS		90/94	2020-005-BR	соок	908 523
	8501 W. Higgins Road; Suite 280	PLOT SCALE =	DRAWN -	D.C.P.	REVISED	DEPARTMENT OF TRANSPORTATION	SN 016-0133 (SB)			CONTRAC	CT NO. 62K73
MO	Chicago, Illinois 60631; (773) 399-0112	PLOT DATE =	CHECKED -	K.G.W.	REVISED		SHEET S03C-07 OF S03C-21 SHEETS		ILLINOIS FED.	AID PROJECT	

*LEGEND* 

	Existing PPC I-Beam
	Existing steel beam, cross frame & lateral bracing
	Existing steel girder
SR xx	Steel Repair
$\otimes$	Temporary Shoring and Cribbing



PARTIAL FRAMING PLAN

## NOTES:

- 1. For Steel Repair 1, see Sheets S03C-13, S03C-18 and S03C-19.
- 2. For Steel Repair 2C, see Sheet S03C-17.

- 3. For Steel Repair 8 and 8A, see Sheet S03C-15.
- 4. For Steel Repair 9, see Sheet S03C-15.
- 5. For Steel Repair 10, see Sheet S03C-15.
- 6. For Steel Repair 11, see Sheet S03C-15.
- 7. For Steel Repair 12, see Sheet S03C-15.
- 8. For Steel Repair 13, see Sheet S03C-16.
- 9. For Steel Repair 14, see Sheet S03C-16.
- 10. For Steel Repair 15, see Sheet S03C-19.

### BEAM REACTION TABLE

STRINGER				STRI	NGER NU	MBER		
REACTION TABLE	V	<i>S17.5</i>	<i>S17.6</i>	S17.7	<i>S17.8</i>	517.9	S17.10 S17.11 S17.12	517.13
RDL (k,	)	32.6	36.5	40.9	32.9	31.1	37.8	33.3
RLL (k)		43.4	46.3	49.1	39.4	38.1	45.9	35.9
RIM (k)	)	11.1	11.8	12.5	10.0	9.7	11.7	9.1
R Total (k)	)	87.1	94.6	102.5	82.3	78.9	95.4	78.3

	USER NAME =	DESIGNED -	J.T.B.	REVISED		PARTIAL FRAMING PLAN II	F.A.I. RTE	SECTION	COUNTY TOTAL SHEETS	SHEET NO.
GRAEF		CHECKED -	H.A.	REVISED	STATE OF ILLINOIS	SN 016-0133 (SB)	90/94	2020-005-BR	COOK 908	524
8501 W. Higgins Road; Suite 280	PLOT SCALE =	DRAWN -	D.C.P.	REVISED	DEPARTMENT OF TRANSPORTATION	5N 010-0133 (3B)			CONTRACT NO. 62	K73
♀ ☐ Chicago, Illinois 60631; (773) 399-0112	PLOT DATE =	CHECKED -	K.G.W.	REVISED		SHEET S03C-08 OF S03C-21 SHEETS		ILLINOIS FED	AID PROJECT	

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Brg. Pier 18		-	5	//		
	519.1				<u>  </u>   —	
	519.2			 	 	
	519.3			  .		
	519.4				1	
	519.5					
	519.6			ا ب	  -	ns
	519.7			ا با		-Bear
	519.8			ا إ		I Jdc
	519.9			   		48"
				+	i⊢ —   	Existing 48" PPC I-Beams
	519.11					Exi
	519.12			·····		
	519.13				<u>  </u>	
	519.14					
	519.15				       	

70'-4<sup>7</sup>/8'' Span 19

*LEGEND* 

	Existing PPC I-Beam
	Existing steel beam, cross frame & lateral bracing
	Existing steel girder
SR xx	Steel Repair
$\otimes$	Temporary Shoring and Cribbing



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2'-7 <i>3</i> 4" f web)	NOT	TES							
"x 2'-11" of web)	1. F	or Steel Rep	air 2B see Sheet S	03C-11.					
s, typ.	<u>LEG</u>	END							
-7 <sup>3</sup> /4 <sup>1111</sup>	SR	Steel Repai	r						
web)	0	Existing Bolt/Rivet to Remain							
	0	Remove Exi	sting Bolt/Rivet an	d Install I	Vew Bo	o/t			
	•	Drill New E	Bolt hole and Instal	I New Boli	t				
REPAIRS I		F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.			

EL REPAIRS I	F.A.I. RTE	SECT	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
33 (SB)	90/94	2020-005-BR			соок	908	525
33 (3D)					CONTRAC	T NO. 62	2K73
03C-21 SHEETS	ILLINOIS FED. AID PROJECT						



	USER NAME =	DESIGNED -	J.T.B.	REVISED		STRUCTURAL STEEL REPAIRS II	F.A.I. RTE	SECTION	COUNTY	SHEETS	SHEET NO.
GRaef	CHECKED - H.A. REVISED		STATE OF ILLINOIS	SN 016-0133 (SB)	90/94	2020-005-BR	соок	908	526		
8501 W. Higgins Road; Suite 280 Chicago, Illinois 60631; (773) 399-0112	PLOT SCALE = PLOT DATE =	DRAWN - CHECKED -	K.G.W.	REVISED REVISED	DEPARTMENT OF TRANSPORTATION	SHEET S03C-10 OF S03C-21 SHEETS		ILLINOIS FED. A	CONTRAC	CT NO. 6	K73

$\langle \rangle$	Existing Bolt/Rivet to Remain
0	Remove Existing Bolt/Rivet and Install New Bolt
•	Drill New Bolt hole and Install New Bolt
F.S.	Indicates Far Side
N.S.	Indicates Near Side
LL	Indicates Long Leg
LLH	Indicates Long Leg Horizontal
SL	Indicates Short Leg



## STRINGER SCHEDULE

Stringer No.	Top Flange Angle sizes	Bottom Flange Angle sizes	Fill Plate Thickness
S14.4	$L4x4x^{1}/_{2}$	L6x4x <sup>1</sup> / <sub>2</sub>	1/2"
S15.4	L4x4x5‰	L6x4x5%	5/8"
S17.4 - S17.8	L4x4x5‰	L6x4x5%	5/8"
517.9 - 517.12	$L4x4x^{1/2}$	L6x4x1/2	1/2"
S17.13	L4x4x5‰	L6x4x5%	5/8"
518.9	$L4x4x^{1/2}$	L6x4x <sup>1</sup> / <sub>2</sub>	1/2"

GROEF 8501 W. Higgins Roads Suite 280	SEER NAME     =     DESIGNED -     J.T.B.     REVISED       CHECKED -     H.A.     REVISED       OT SCALE     E     DRAWN -     D.C.P.     REVISED		REVISED	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	STRUCTURAL STEEL REPAIRS III SN 016-0133 (SB)	F.A.I. RTE 90/94	SECTION 2020-005-BR	COUNTY TOTAL SHI SHEETS N COOK 908 52 CONTRACT NO. 62K73	
See 1 Chicago, Illinois 60631; (773) 399-0112	PLOT DATE = CHECKED -		K.G.W.	REVISED		SHEET S03C-11 OF S03C-21 SHEETS		ILLINOIS FE	D. AID PROJECT
♀     □     Chicago, Illinois 60631; (773) 399-0112       5/3/2024     12:52:30 PM	PLOT DATE =	CHECKED -	K.G.W.	REVISED		SHEET \$03C-11 OF \$03C-21 SHEETS		ILLINOIS FE	D. AID PROJECT



SECTION B-B

## NOTES

- 1. The contractor shall submit, for approval by the Engineer, plans for jacking existing beams and removing the existing bearings prior to commencing any related work.
- 2. Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I or Type III.
- 3. The cost of temporary shoring and cribbing of the existing beam at the bearing replacment shall be included in the cost of Jack and Remove Existing Bearings.
- 4. The cost of existing bearing removal shall be included in the cost of Jack and Remove Existing Bearings.
- 5. Two  $\frac{1}{8}$  in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

### LEGEND

- Existing Bolt/Rivet to Remain
- Ο Remove Existing Bolt/Rivet and Install New Bolt
- Drill New Bolt hole and Install New Bolt
- N.S. Indicates Near Side
- Indicates Long Leg LL



STEEL REPAIR 5 (Girder G1A, Looking Southeast) – Tighten Bearing Anchor Rod Nut



— Tighten Bearing Anchor Rod Nut

<u>STEEL REPAIR 5</u> (Girder G1B, Looking South)



(Girder G1 at Pier 16, Looking Northwest)

– Tighten Bearing Anchor Rod Nut



–Install Missing Bottom Flange  $1^{l}_{\mathcal{A}''}$  Ø Bolt



STEEL REPAIR 7 (Girder G1 at Pier 13, Looking South) -Replace Loose Bearing Sole Plate 1% Ø Bolt and Nut to Bottom Flange





-Tighten Bearing Anchor Rod Nut

STE<u>EL REPAIR 5</u> (Girder G3 at Pier 14, Looking South)

MOM ::	USER NAME =		BEGIGNED - C.N.D. REVISED			STRUCTURAL STEEL REPAIRS IV	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEET	
* : \$	GR@EF		CHECKED -	H.A.	REVISED	STATE OF ILLINOIS		90/94	2020-005-BR	соок	908 528
≝ ≧ 8501 w.	8501 W Higgins Road: Suite 280 PLOT SCALE =	PLOT SCALE =	DRAWN - J.T.B. REVISED		REVISED	DEPARTMENT OF TRANSPORTATION	SN 016-0133 (SB)			CONTRACT NO. 62K7	
Chicago,	, Illinois 60631; (773) 399-0112	PLOT DATE = CHECKED -		K.G.W.	REVISED		SHEET S03C-12 OF S03C-21 SHEETS		ILLINOIS FED.	FED. AID PROJECT	
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STEEL REPAIR 5 (Girder G4 at Pier 15, Looking North)



STEEL REPAIR 6 (Girder G5, Looking Northwest)

# NOTES

The cost of Steel Repair 5, Steel Repair 6 and Steel Repair 7 will not be paid separately and shall be included with Structural Steel Repair.



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EL REPAIRS V	F.A.I. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.	
33 (SB)	90/94	2020-005-BR			соок	908	529	
55 (5b)					CONTRAC	T NO. 62	2K73	
03C-21 SHEETS			ILLINOIS	FED. A	D PROJECT			



JSER NAME = DESIGNED -REVISED J.T.B. STRUCTURAL STEEL GRaEF STATE OF ILLINOIS CHECKED H.A. REVISED SN 016-0133 LOT SCALE = DRAWN D.C.P. REVISED **DEPARTMENT OF TRANSPORTATION** 8501 W. Higgins Road; Suite 280 Chicago, Illinois 60631; (773) 399-0112 PLOT DATE = SHEET S03C-14 OF S03C CHECKED -K.G.W. REVISED

## NOTES:

- 1. Cross frames between S17.4-S17.8 and S17.9-S17.13 not shown for clarity.
- 2. For Steel Repair 8, Steel Repair 8A, Steel Repair 9, Steel Repair 10, Steel Repair 11 and Steel Repair 12, see Sheet S03C-15.
- 3. The contractor shall submit, for approval by the Engineer, plans for jacking existing beams and removing the existing bearings prior to commmencing any related work.
- 4. The cost of existing bearing removal shall be included in the cost of Jack and Remove Existing Bearings.
- 5. Ends of bearing seat stiffener angles shall be crimped and/or ground to match top and bottom flange angle fillets. Cost included in Structural Steel Repair.
- 6. Two  $\frac{1}{8}$  in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

# <u>LEGEND</u>

SR XX Steel Repair

- Existing Bolt/Rivet to Remain
- O Remove Existing Bolt/Rivet and Install New Bolt
- Drill New Bolt hole and Install New Bolt

L REPAIRS VI		SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
33 (SB)	90/94	2020-005-BR		соок	908	530	
55 (5D)					CONTRAC	T NO. 62	2K73
3C-21 SHEETS	ILLINOIS FED. AID PROJECT						



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соок 908 531 CONTRACT NO. 62K73 ILLINOIS FED. AID PROJECT



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8501 W. Higgins Road: S	Suite 280 PLOT SCALE =	DRAWN -	D.C.P.	REVISED	DEPARTMENT OF TRANSPORTATION	SN 016-0133 (SB)			CONTRACT NO. 62K73
♀ ┤ Chicago, Illinois 60631; (7	773) 399-0112 PLOT DATE =	CHECKED -	K.G.W.	REVISED		SHEET S03C-16 OF S03C-21 SHEETS	]	AID PROJECT	
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# <u>LEGEND</u>

$\langle \rangle$	Existing Bolt/Rivet to Remain
0	Remove Existing Bolt/Rivet and Install New Bolt
•	Drill New Bolt hole and Install New Bolt
F.S.	Indicates Far Side
N.S.	Indicates Near Side
LL	Indicates Long Leg
LLH	Indicates Long Leg Horizontal
SL	Indicates Short Leg



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- Remove Existing Bolt/Rivet and Install New Bolt
- Drill New Bolt hole and Install New Bolt

L REPAIRS IX	F.A.I. SECTION			COUNTY	TOTAL SHEETS	SHEET NO.	
33 (SB)	90/94 2020-005-BR		соок	908	533		
55 (5B)					CONTRACT NO. 62K73		
03C-21 SHEETS			ILLINOIS	FED. A	D PROJECT		



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- Remove Existing Bolt/Rivet and Install New Bolt
- Drill New Bolt hole and Install New Bolt

EL REPAIRS X	F.A.I. SECTION		COUNTY	TOTAL SHEETS	SHEET NO.		
33 (SB)	90/94 2020-005-BR		соок	908	534		
55 (5B)					CONTRAC	T NO. 62	2K73
03C-21 SHEETS			ILLINOIS	FED. A	D PROJECT		



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	DESIGNED -	J.T.B.	REVISED		ELASTOMERIC BEARING DETAILS I	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEET SHEETS NO.	
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8501 W. Higgins Road; Suite 280	PLOT SCALE =	DRAWN - D.C.P.	D.C.P. REVISED DEPARTMENT OF TRANSPORTATION		SN 010-0135 (5D)	CONTF			T NO. 62K73
Chicago, Illinois 60631; (773) 399-0112	PLOT DATE =	CHECKED - K.G.W.	REVISED		SHEET S03C-21 OF S03C-21 SHEETS	ILLINOIS FED. AID PROJ			
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#### GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Calculated weight of Structural Steel = 10,220 lb (M270 Grade 36)  $= 160 \ lb \ (M270 \ Grade \ 50)$
- 3. Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 4. Bars noted thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bars per line.
- 5. All exposed concrete edges shall have a  $\frac{3}{4}$ "x45° chamfer except where shown otherwise.
- 6. Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.
- 7. For SMA overlay on Approach Slab, see Civil Sheets.
- 8. Protective Coat shall be applied to the top of reconstructed transverse joint areas, top and inside faces of parapets, and top of Latex Concrete Overlay.
- 9. Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.
- 10. Prior to pouring the new concrete deck for expansion joint reconstruction and deck slab repairs, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by gualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding  $\frac{1}{4}$  deep shall be identified and reported to the Bureau of Bridges and Structures for further dispositions. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 11. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 12. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provisions "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".
- 13. All new structural steel shall be hot-dip galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".
- 14. Fasteners shall be ASTM F 3125 Grade A325 Type 1. Fasteners shall be hot dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel." Bolts  $\frac{3}{4}$  in. diameter, holes  $\frac{12}{16}$  in. diameter, unless otherwise noted.
- 15. No field welding is permitted expected as specified in the contract documents.
- 16. Adjacent I-90/94 reversible bridge is not shown throughout the plans for clarity.
- 17. The Contractor shall take the necessary precautions for the protection of passing vehicles, bicycles and pedestrians from falling objects and/or materials until completion of work.
- 18. The Contractor is responsible to remove, support and reinstall all existing electrical conduits interfering with the work. See special provision "Protection and Maintenance of Existing Underpass Luminaires".
- 19. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 20. The Contractor is responsible to protect the existing conduit and junction box embedded in the parapet during removal and construction. Any damage to the existing conduit and junction box shall be repaired by the Contractor at his or her expense at no charge to IDOT.
- 21. Where underpass lighting is present on the structure, the Contractor shall adjust the Protective Shielding to ride above the existing lighting fixtures in order to maintain the existing level of lighting on the roadway underneath. Details shall be approved by the Engineer before installation.
- 22. Any adjustment done to the Protective Shield System must not change the load carrying capacity (or containment specifications) as indicated in the Standard Specifications. Cost of adjusting shielding is including in the cost of Protective Shield.



### INDEX OF SHEETS

504-01	General Plan and Elevation
504-02	General Notes, Index of Sheets & TBOM
504-03	Stage Construction (Sheet 1 of 2)
504-04	Stage Construction (Sheet 2 of 2)
504-05	Temporary Concrete Barrier
504-06	Deck Repair Plan
504-07	S. Abut. Joint Removal & Reconstruction (Sht. 1 of 3)
504-08	S. Abut. Joint Removal & Reconstruction (Sht. 2 of 3)
504-09	S. Abut. Joint Removal & Reconstruction (Sht. 3 of 3)
504-10	N. Abut. Joint Removal & Reconstruction (Sht. 1 of 3)
504-11	N. Abut. Joint Removal & Reconstruction (Sht. 2 of 3)
504-12	N. Abut. Joint Removal & Reconstruction (Sht. 3 of 3)
504-13	Preformed Joint Strip Seal
504-14	Framing Plan
S04-15	Structural Steel Repair Details (Sheet 1 of 6)
S04-16	Structural Steel Repair Details (Sheet 2 of 6)
<i>S04-17</i>	Structural Steel Repair Details (Sheet 3 of 6)
504-18	Structural Steel Repair Details (Sheet 4 of 6)
504-19	Structural Steel Repair Details (Sheet 5 of 6)
504-20	Structural Steel Repair Details (Sheet 6 of 6)
504-21	South Abutment Repairs
504-22	North Abutment Repairs
504-23	Pier 1 Repairs
504-24	Pier 2 Repairs
	•

- S04-25 Slope Wall Repairs
- *S04–26* Bar Splicer Assembly and Mechanical Splicer Details

#### SCOPE OF WORK

- 1. Provide Protective shield within limits indicated on the plans.
- 2. Scarify  $\frac{3}{4}$ " from the bridge deck slab.
- 3. Perform Deck Slab Repairs.
- 4. Reconstruct Expansion Joints at the South and North abutments and install new preformed joint strip seals.
- 5. Apply 3" Bridge Deck Latex Concrete Overlay on Bridge Deck.
- 6. Perform  $\frac{1}{4}$ " Diamond Grinding to top of bridge deck and abutment hatched block.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay on the Approach Slabs, see Roadway Plans.
- 8. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- 9. Apply protective coat to the top of reconstructed transverse joint areas, top and inside faces of parapets, and top of Latex Concrete Overlay.
- 10. Repair diaphragm as shown on the plans.
- 11. Perform structural concrete repairs and epoxy crack injection for the abutments and piers as noted on the plans.
- 12. Perform Slope Wall repairs.

### GENERAL NOTES (CONT.)

- 23. The Contractor shall contact Chandra Libby, the Director of City of Chicago Department of Family Support Services (DFSS) at 312-746-5443 or Chandra Libby@cityofchicago.org to coordinate the relocation of persons and their personal belongings under the bridges within the areas bounded by the temporary chain-link-fence.
- 24. The intent of the temporary fence is to deny access of any unauthorized personnel under the bridge during construction. Actual fence installations may vary from what is shown on the plans. All fence installations must be approved by the Engineer.
- 25. Concrete Sealer shall be applied to the designated areas of the abutments.
- 26. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See special provision for Debris Removal.

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ENGINEERING GROUP, LLC	PLOT DATE = DATE - 4/29/2024 REVISED -		REVISED -		SHEET S04-02 OF S04-26 SHEETS	SHEET S04-02 OF S04-26 SHEETS ILLINOIS		
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TOTAL	BILL	0F	MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	38	38
Concrete Removal	Cu Yd	27.9	-	27.9
Slope Wall Removal	Sq Yd	-	113	113
Protective Shield	Sq Yd	912	-	912
Concrete Superstructure	Cu Yd	31.3	-	31.3
Protective Coat	Sq Yd	2,225	-	2,225
Furnishing And Erecting Structural Steel	Pound	10,220	-	10,220
Reinforcement Bars, Epoxy Coated	Pound	5,700	-	5,700
Bar Splicers	Each	32	-	32
Slope Wall 4 Inch	Sq Yd	-	113	113
Preformed Joint Seal 2 1/2"	Foot	261	-	261
Preformed Joint Strip Seal	Foot	196	-	196
Concrete Sealer	Sq Ft	-	778	778
Epoxy Crack Injection	Foot	-	60	60
Slope Wall Crack Sealing	Foot	-	25	25
Bridge Deck Grooving (Longitudinal)	Sq Yd	1,851	-	1,851
Protect And Maintain Existing Underpass	L Sum	-	0.04	0.04
Approach Slab Repair (Full Depth)	Sq Yd	48	-	48
Approach Slab Repair (Partial Depth)	Sq Yd	48	-	48
Structural Steel Removal	Pound	8,220	-	8,220
Structural Steel Repair	Pound	160	-	160
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	1,891	-	1,891
Bridge Deck Scarification 3/4"	Sq Yd	1,891	-	1,891
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	-	79	79
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	-	6	6
Deck Slab Repair (Full Depth, Type II)	Sq Yd	34	-	34
Diamond Grinding (Bridge Section)	Sq Yd	1,949	-	1,949
Temporary Construction Fence	Foot	-	403	403
Temporary Shoring And Cribbing	Each	-	2	2
Locks For Gates	Each	-	4	4



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# STAGE I REMOVAL

- 1. Install temporary concrete barrier as shown to locate traffic on the west side of the existing structure.
- 2. Perform  $\frac{3}{4}$ " bridge deck scarification.
- 3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the North and South Abutments.
- 5. Perform temporary shoring and cribbing at locations shown on the plans within the limits of Stage I removal.
- 6. Remove existing longitudinal preformed joint seal between west parapet and reversible lane parapet.

### STAGE I CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct expansion joints and install new preformed joint strip seals within the limits of Stage I Construction.
- 3. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
- 4. Apply 3" Bridge Deck Latex Concrete Overlay.
- Perform  $\frac{1}{4}$ " Diamond Grinding to bridge deck and 5. abutment hatched block.
- 6. Perform Bridge Deck Grooving (Longitudinal) for the 3" Bridge Deck Latex Concrete Overlay and reconstructed abutment expansion joint areas.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 8. Apply protective coat to top and inside faces of west parapet, reconstructed abutment expansion joints and to the surfaces of the new overlay.
- 9. Perform slope wall repairs as shown on the plans.
- 10. Replace existing longitudinal preformed joint seal between west parapet and reversible lane parapet.

## STAGE II REMOVAL

- 1. Install temporary concrete barrier as shown to locate traffic on the east side of the existing structure.
- 2. Perform 3/" bridge deck scarification.
- 3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the North and South Abutments.
- 5. Perform temporary shoring and cribbing at locations shown on the plans within the limits of Stage II removal.

\*Match existing cross slopes

N (SHEET 1 OF 2)		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.		
.6-0132 (NB)	90/94	2020-005-BR			COOK	908	540	
.0-0132 (NB)					CONTRACT NO. 62K7			
4-26 SHEETS	ILLINOIS FED. AID PROJECT							



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# STAGE II CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct expansion joints and install new preformed joint strip seals within the limits of Stage II Construction.
- 3. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
- 4. Apply 3" Bridge Deck Latex Concrete Overlay.
- 5. Perform  $\frac{1}{4}$ " Diamond Grinding to bridge deck and abutment hatched block.
- 6. Perform Bridge Deck Grooving (Longitudinal) for the 3" Bridge Deck Latex Concrete Overlay and reconstructed abutment expansion joint areas.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 8. Apply protective coat to top and inside faces of east parapet, reconstructed abutment expansion joints and to the surfaces of the new overlay.
- 9. Repair diaphragm as shown on the plans.
- 10. Perform slope wall repairs as shown on the plans.

\*Match existing cross slopes



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beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart,

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BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Protective Coat	Sq Yd	2155
Preformed Joint Seal 2 1/2"	Foot	261
Bridge Deck Grooving (Longitudinal)	Sq Yd	1851
Approach Slab Repair (Full Depth)	Sq Yd	48
Approach Slab Repair (Partial Depth)	Sq Yd	48
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	1891
Bridge Deck Scarification 3/4"	Sq Yd	1891
Deck Slab Repair (Full Depth, Type II)	Sq Yd	34
Diamond Grinding (Bridge Section)	Sq Yd	1949

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BAR d3(E)



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N,

BAR d9(E)

Bar	No.	Size	Length	Shape
a(E)	20	#5	24'-0"	
a1(E)	20	#5	26'-7"	
a2(E)	6	#6	6'-6"	
d(E)	6	#4	4'-2"	Ĺ
d1(E)	6	#5	2'-7"	/
d2(E)	3	#4	4'-11"	L
d3(E)	3 3 2 3 5	#5	4'-8''	Ĵ
d4(E)	2	#5	2'-9"	
d5(E)	3	#5	5'-4"	L
d6(E)	5	#4	3'-8''	L
d7(E)	5	#5	3'-8"	L
d8(E)	3	#5	6'-1"	
d9(E)	4	#5	6'-2"	/
d10(E)	6	#6	2'-0"	
h(E)	12	#6	23'-3"	
h1(E)	12	#6	25'-10"	
s(E)	12	#5	3'-8''	
s1(E)	18	#5	4'-0''	
s2(E)	42	#5	3'-4"	
u(E)	39	#5	3'-10"	Π
u1(E)	38	#5	3'-4"	Π
u2(E)	16	#5	4'-0"	
Concrete			Cu Yd	13.6
Concrete		ructure	Cu Yd	15.1
Protectiv			Sq Yd	34
Reinforce Epoxy Co		nrs,	Pound	2,790

BILL OF MATERIAL

#5	3'-6"
#6	4'-0''

# NOTES:

- 1. For legend, see Sheet S04-07.
- 2. For preformed joint strip seal details, see Sheet S04-13.
- 3. For bar splicer assembly details, see Sheet S04-26.
- 4. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 5. Epoxy grout d4(E), d8(E) and d9(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.



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**DEPARTMENT OF TRANSPORTATION** 

STRUCTURE NO. 01 SHEET S04-12 OF S04

# BILL OF MATERIAL

6''

BAR d1(E)

9½"

1½"

	3"
<u>آ</u> ر ا	T

7″ BAR d9(E)

> MIN BAR LAPS #5 3'-6"

> > 4'-0'' #6

# NOTES:

- 1. For legend, see Sheet S04-07.
- 2. For preformed joint strip seal details, see Sheet S04-10.
- 3. For bar splicer assembly details, see Sheet S04-26.
- 4. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 5. Epoxy grout d4(E), d8(E), and d9(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.

ONSTRUCTION (SHT. 3 OF 3)	F.A.I. RTE	SECT	TION		COUNTY	TOTAL SHEETS	SHEET NO.	
L6-0132 (NB)	90/94	90/94 2020-005-BR		соок	908	549		
LO-0132 (NB)					CONTRACT NO. 62K7			
04-26 SHEETS	ILLINOIS FED. A			D PROJECT				

				-
Bar	No.	Size	Length	Shape
a2(E)	6	#6	6'-6"	
a3(E)	20	#5	25'-6"	
a4(E)	20	#5	28'-2''	
d(E)	6	#4	4'-2" 2'-7"	C
d1(E)	6	#5	2'-7"	$\sim$
d2(E)	5	#4	4'-11''	L
d3(E)	5 2 3	#5	4'-8''	<u> </u>
d4(E)	2	#5	2'-9"	
d5(E)	3	#5	4'-8''	L
d6(E)	3 3	#4	3'-8''	
d7(E)	3	#5	3'-8"	L
d8(E)	4	#5	6'-1"	
d9(E)	3	#5	6'-2"	$\overline{\ }$
d10(E)	7	#6	2'-0''	
h2(E)	12	#6	24'-9"	
h3(E)	12	#6	27'-5"	
s(E)	30	#5	3'-8"	
s2(E)	42	#5	3'-4''	
u(E)	8	#5	3'-10''	П
u3(E)	41	#5	3'-1"	Π
u4(E)	42	#5	3'-7"	П
u5(E)	8	#5	4'-3''	
Concrete	Removal	1	Cu Yd	14.3
Concrete			Cu Yd	16.2
Protectiv	e Coat		Sq Yd	36
Reinforce	ement Ba	nrs,	Pound	2,910
Ероху Сс	pated		Found	2,910



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SHEET S04-13 OF S0

The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4<sup>1</sup>/<sub>2</sub>" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{6}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



## LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	196

T STRIP SEAL 16-0132 (NB)		SECTION			COUNTY	TOTAL SHEETS	SHEET NO.	
		2020-005-BR			соок	908	550	
16-0132 (NB)					CONTRACT NO. 62K			
04-26 SHEETS	ILLINOIS FED. AID PROJECT			D PROJECT				

7*2'-6%*'' 72'-65/8'' 113'-0<sup>7</sup>/<sub>8</sub>'' Span 1 Span 2 Span 3 - Exist. W36x245 Beam, typ. .5-21/2 4 508- 235-418 32"-8¾" 29'-1¼" Stage II Construction Stage I Construction Remove and Replace Exist. W36x245 Typ. Diaphragm 45°2'3" Typ. 220+00 Stations Increase 221+00 ⊊ Brg. S. Abut. Sta. 220+04.36 - © Pier 1 Sta. 220+74.16 A 508. - @ Pier 2 Sta. 221+85.43 Remove and Replace Exist.— Diaphragm Remove and Replace Exist. Diaphragm… 0, -W36x135 -W36x245 W36x194 -W36x245 -W36x135 FRAMING PLAN Ø

## NOTES:

1. All work is to be performed utilizing staged construction. See Sheets 504-03 and 504-04 for details.

2. For Diaphragms Replacement Details, see Sheets S04-15 thru S04-20.

Defau		USER NAME =	DESIGNED - LAB, AMS	REVISED -		FRAMING PLAN	F.A.I. RTE	SECTION	COUNTY TOTAL SHEET SHEETS NO.
AME			CHECKED - MI	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 016-0132 (NB)	90/94	2020-005-BR	СООК 908 551
		DIV PLOT SCALE = DRAWN - LAB, AMS	DRAWN - LAB, AMS	REVISED -					CONTRACT NO. 62K73
A MO	ENGINEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -		SHEET S04-14 OF S04-26 SHEETS		ILLINOIS FED.	AID PROJECT

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BILL OF MATERIAL							
ITEM	UNIT	QUANTITY					
Furnishing And Erecting Structural Steel	Pound	10,220					
Structural Steel Removal	Pound	8,220					
Structural Steel Repair	Pound	160					





Remove and Replace Exist. Diaphragm



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ENGINEERING GROUP, LLC

LOT DATE =

DATE

- 4/29/2024

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ILLINOIS FED AID PROJECT

SHEET S04-15 OF S04-26 SHEETS



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SHEET S04-16 OF S04-26 SHEETS



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DETAILS (SHEET 3 OF 6) 16-0132 (NB)		SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
		2020-005-BR			соок	908	554
					CONTRACT NO. 62K		
04-26 SHEETS			ILLINOIS	FED. A	D PROJECT		



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	ENGINEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -		SHEET S04-18
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CONTRACT NO. 62K73 4-18 OF S04-26 SHEETS ILLINOIS FED. AID PROJECT



ENGINEERING GROUP, LLC

PLOT DATE =

DATE

- 4/29/2024

REVISED -

- 1. For Notes, see Sheet S04-15.
- 2. For Sections A-A, DD-DD, and GG-GG, see Sheet S04-16.
- 3. For Section BB-BB and Detail 4, see

Structural Steel Removal

- Shop drill holes in new steel. Use new steel as template to field drill holes in existing steel.
- Field drill holes in new steel using existing steel as template

DETAILS (SHEET 5 OF 6)	F.A.I. RTE	SECTION			COUNTY TOTAL SHEETS		SHEET NO.
16-0132 (NB)	90/94	2020-0	05 <b>-</b> BR		соок	908	556
10-0132 (NB)					CONTR	ACT NO.	62K73
04-26 SHEETS			ILLINOIS	FED. A	D PROJECT		



**DEPARTMENT OF TRANSPORTATION** 

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PLOT DATE =

DRAWN - LAB, AMS

- 4/29/2024

DATE

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REVISED -

### NOTES:

- 1. For Notes, see Sheet S04-15.
- 2. For Sections A-A and DD-DD, see Sheet S04-16.

- 3. For Section FF-FF, see Sheet S04-17.
- 4. For Sections AA-AA, BB-BB and Detail 4, see Sheet S04-18.

### LEGEND:



Structural Steel Removal

- Shop drill holes in new steel. Use new steel as template to field drill holes in existing steel.
- о
- Field drill holes in new steel using existing steel as template.

DETAILS (SHEET 6 OF 6)	F.A.I. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
L6-0132 (NB)	90/94	2020-0	05 <b>-</b> BR		соок	908	557
					CONTR/	ACT NO.	62K73
04-26 SHEETS			ILLINOIS	FED. A	D PROJECT		



(4)

3

(5)

(6)

# SOUTH ABUTMENT ELEVATION

(Looking South)

SUMMARY SOUTH B		MENT
R DL	(k)	29.1
R LL	(k)	39.8
R IM	(k)	10.1
R Total	(k)	79.0

(s)

(9)

(7`

6 SF

2								
<u> </u>		USER NAME =	DESIGNED - HMI	REVISED -		SOUTH ABUTMENT REPAIRS	F.A.I. SECTION	COUNTY TOTAL SHEET
WE.			CHECKED - MI	REVISED -	STATE OF ILLINOIS		90/94 2020-005-BR	COOK 908 558
ž		PLOT SCALE =	DRAWN - HMI	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 016-0132 (NB)		CONTRACT NO. 62K73
FILE	ENGINEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -		SHEET S04-21 OF S04-26 SHEETS	ILLINOIS FE	D. AID PROJECT

NOTES:

repair of concrete.

abutment backwall.

4. For Slope Wall repairs, see Sheet S04-25.

(1B)

(1A)

4 SF

1 LF

1 LF -

1. Quantities and limits shown are estimated for bidding purposes only. The actual

2. Temporary shoring and cribbing shall be installed prior to the start of the

the Engineer in the field at the time of construction.

areas to be repaired, and the type(s) of repairs to be used, will be determined by

structural repair of concrete and shall be removed after completing the structural

3. Concrete Sealer is to be applied to the abutment seats and the bottom 2 feet of the

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	368
Epoxy Crack Injection	Foot	26
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	4
Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq Ft	6
Temporary Shoring And Cribbing	Each	1



# LEGE<u>ND</u>

/	/	/

SF

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

- Square Foot

LF – Linear Foot



# NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Concrete Sealer is to be applied to the abutment seats and the bottom 2 feet of the abutment backwall.
- 3. For Slope Wall repairs, see Sheet S04-25.

Defau	TTDA	USER NAME =	DESIGNED -	HMI	REVISED -		NORTH ABUTMENT REPAIRS	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEET SHEETS NO.
AM L			CHECKED -	MI	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 016-0132 (NB)	90/94	2020-005-BR	соок	908 559
		PLOT SCALE =	DRAWN -	HMI	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 010-0132 (NB)			CONTR	ACT NO. 62K73
EILE	ENGINEERING GROUP, LLC	PLOT DATE =	DATE -	4/29/2024	REVISED -		SHEET S04-22 OF S04-26 SHEETS		ILLINOIS FED. AI	D PROJECT	

NORTH ABUTMENT ELEVATION (Looking North)

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BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	410
Epoxy Crack Injection	Foot	34
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	21





SF

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Epoxy Crack Injection (Width > 0.06")

- Square Foot

LF – Linear Foot



# NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

efau		USER NAME =	DESIGNED -	НМІ	REVISED -		PIER 1 REPAIRS	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEET SHEETS NO.
AMI D	HRM		CHECKED -	MI	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 016-0132 (NB)	90/94	2020-005-BR	соок	908 560
		PLOT SCALE =	DRAWN -	HMI	REVISED -	DEPARTMENT OF TRANSPORTATION				CONTR	RACT NO. 62K73
N N N	ENGINEERING GROUP, LLC	PLOT DATE =	DATE -	4/29/2024	REVISED -		SHEET S04-23 OF S04-26 SHEETS		ILLINOIS	FED. AID PROJECT	
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R DL

R LL

r im

R Total

(k) 131.5

(k) 58.4

(k) 13.5

(k) 203.4

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	BILL OF MATERIA	<u>AL</u> UNIT	QUANTITY
Structural Repair of	Concrete (Depth Equal ches)	Sq Ft	43
To or Less Than 5 Ir Temporary Shoring a	nches) And Cribbing	Each	1
	Existing Electric Conduit to remain		
I-90/94 Rev Lanes	Image: marked state sta	SHT INC Southeas	<b>6: PIER</b> 1 st)
<u>L</u> [	EGEND Structural R Equal to or	epair of Less tha	Concrete (Dept n 5 inches)

SF – Square Foot



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#### INSTALLATION AND SETTING METHODS

- "A" : Set bar splicer assembly by means of a template bolt.
- "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.

				- 1		
Threaded splice	r bar leng	th = min	lap length	+ 1½" +	+ thread length	

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Location	Bar size	No. assemblies required	Minimum Iap length
South Abutment	#5	10	3'-6"
Exp. Jt.	#6	6	4'-0''
North Abutment	#5	10	3'-6"
Exp. Jt.	#6	6	4'-0''

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200 4	BSD-I	1-1-2020									
Defau		USER NAME =	DESIGNED - LAB, HMI	REVISED -		BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS	F.A.I. RTE	SECTION	COUNTY	TOTAL S SHEETS	HEET NO.
AMI C			CHECKED - MI	REVISED -	STATE OF ILLINOIS	STRUCTURE NO. 016-0132 (NB)	90/94	2020-005-BR	соок	908	563
U N D		PLOT SCALE =	DRAWN - LAB, HMI	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 010-0132 (ND)			CONTR	RACT NO. 6	32K73
EILE MOI	ENGINEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -		SHEET S04-26 OF S04-26 SHEETS		ILLINOIS	FED. AID PROJECT		
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# STANDARD MECHANICAL SPLICER

Location	Bar	No. assemblies
Location	size	required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.



### GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 3. Bars noted thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bars per line.
- 4. All exposed concrete edges shall have a  $\frac{3}{4}$ "x45° chamfer except where shown otherwise.
- 5. Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.
- 6. For SMA overlay on Approach Slab, see Civil Sheets.
- 7. Protective Coat shall be applied to the top of reconstructed transverse joint areas, top and inside faces of parapets and top of Latex Concrete Overlay.
- 8. Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.
- 9. Prior to pouring the new concrete deck for expansion joint reconstruction and deck slab repairs, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding  $\frac{1}{4}$ " deep shall be identified and reported to the Bureau of Bridges and Structures for further dispositions. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 10. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 11 Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".
- 12. All new structural steel shall be hot-dip galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".
- 13. Fasteners shall be ASTM F 3125 Grade A325 Type 1. Fasteners shall be hot dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel." Bolts  $\frac{3}{4}$  in. diameter, holes  $\frac{13}{16}$  in. diameter, unless otherwise noted.
- 14. No field welding is permitted except as specified in the contract documents.
- 15. Adjacent I-90/94 reversible bridge is not shown throughout the plans for clarity.
- 16. The Contractor shall take the necessary precautions for the protection of passing vehicles, bicycles and pedestrians from falling objects and/or materials until completion of work.
- 17. The Contractor is responsible to remove, support and reinstall all existing electrical conduits interfering with the work. See special provision "Protection and Maintenance of Existing Underpass Luminaires".
- 18. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 19. The Contractor is responsible to protect the existing conduit embedded in the parapet during concrete removal and construction. Any damage to the existing conduit shall be repaired by the Contractor at no additional cost to the Department.
- 20. Where underpass lighting is present on the structure, the Contractor shall adjust the Protective Shielding to ride above the existing lighting fixtures in order to maintain the existing level of lighting on the roadway underneath. Details shall be approved by the Engineer before installation.
- 21. Any adjustment done to the Protective Shield System must not change the load carrying capacity (or containment specifications) as indicated in the Standard Specifications. Cost of adjusting shielding is including in the cost of Protective Shield.
- 22. The Contractor shall contact Chandra Libby, the Director of City of Chicago Department of Family Support Services (DFSS) at 312-746-5443 or Chandra.Libby@cityofchicago.org to coordinate the relocation of persons and their personal belongings under the bridges within the areas bounded by the temporary chain-link-fence.

### INDEX OF SHEETS

S05-01 General Plan And Elevation S05-02 General Notes, Index of Sheets & TBOM S05-03 Stage Construction (Sheet 1 of 2) S05-04 Stage Construction (Sheet 2 of 2) S05-05 Temporary Concrete Barrier S05-06 Deck Repair Plan S05-07 Drainage Scupper Type A Adjustment Details S05-08 S. Abut. Joint Removal & Reconstruction (Sht. 1 of 3) S05-09 S. Abut. Joint Removal & Reconstruction (Sht. 2 of 3) S05-10 S. Abut. Joint Removal & Reconstruction (Sht. 3 of 3) S05-11 N. Abut. Joint Removal & Reconstruction (Sht. 1 of 3) S05–12 N. Abut. Joint Removal & Reconstruction (Sht. 2 of 3) S05-13 N. Abut. Joint Removal & Reconstruction (Sht. 3 of 3) S05-14 Preformed Joint Strip Seal S05-15 Framing Plan Steel Repairs S05-16 Structural Steel Repair Details S05-17 South Abutment Repairs S05-18 North Abutment Repairs S05-19 Pier 1 Repairs S05-20 Pier 2 Repairs 505-21 Slope Wall Repairs S05-22 Bar Splicer Assembly Details

#### SCOPE OF WORK

- 1. Provide Protective Shield within limits indicated on the plans.
- 2. Scarify  $\frac{3}{4}$ " from the bridge deck slab.
- 3. Perform Deck Slab Repairs and adjust existing scuppers and inlets as required.
- 4. Reconstruct Expansion Joints at the South and North abutments and install new preformed joint strip seals.
- 5. Apply 3" Bridge Deck Latex Concrete Overlay on Bridge Deck.
- 6. Perform <sup>1</sup>/<sub>4</sub>" Diamond Grinding to top of bridge deck and abutment hatched block.
- Apply 2" Stone-Mix Asphalt (SMA) Overlay on the 7. Approach Slabs, see Roadway Plans.
- 8. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- Apply protective coat to the top of reconstructed 9 transverse joint areas and top and inside faces of parapets and top of Latex Overlay.
- 10. Perform structural concrete repairs and epoxy crack injection for the abutments and piers as noted on the plans.
- 11. Perform Slope Wall repairs.
- 12. Repair the existing drainage system, as shown on the plans.

#### GENERAL NOTES (CONT.):

23. The intent of the temporary fence is to deny access of any unauthorized personnel under the bridge during construction. Actual fence installations may vary from what is shown on the plans. All fence installations must be approved by the Engineer.

24. Concrete Sealer shall be applied to the designated areas of the abutments.

- 25. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See special provision for Debris Removal.
- 26. Calculated weight of Structural Steel = 330 lb (M270 Grade 36)

TUTAL DILL OF MA		<u>4</u> L		
ITEM	UNIT	SUPER	SUB	TOTAL
Granular Embankment	Cu Yd	-	4	4
te Removal	Cu Yd	40.7	-	40.7
Vall Removal	Sq Yd	-	11	11
ive Shield	Sq Yd	1149	-	1149
te Superstructure	Cu Yd	46.7	-	46.7
ive Coat	Sq Yd	2466	-	2466
ning And Erecting Structural Steel	Pound	330	-	330
cement Bars, Epoxy Coated	Pound	7630	-	7630
licers	Each	32	-	32
Vall 4 Inch	Sq Yd	-	11	11
med Joint Seal 2 1/2"	Foot	237	-	237
ned Joint Strip Seal	Foot	289	-	289
te Sealer	Sq Ft	-	1131	1131
Crack Injection	Foot	-	33	33
Vall Crack Sealing	Foot	-	163	163
Drainage System Repair	Foot	-	22	22
Deck Grooving (Longitudinal)	Sq Yd	1653	-	1653
And Maintain Existing Underpass	L Sum	-	0.04	0.04
ch Slab Repair (Full Depth)	Sq Yd	52	-	52
ch Slab Repair (Partial Depth)	Sq Yd	52	-	52
iral Steel Removal	Pound	330	-	330
Deck Latex Concrete Overlay, 3 Inches	Sq Yd	2163	-	2163
g Drainage System	L Sum	0.063	-	0.063
Deck Scarification 3/4"	Sq Yd	2163	-	2163
ral Repair Of Concrete (Depth Equal ess Than 5 Inches)	Sq Ft	-	117	117
ral Repair Of Concrete (Depth Greater Inches)	Sq Ft	-	14	14
lab Repair (Full Depth, Type II)	Sq Yd	2 2	-	2
ge Scuppers To Be Adjusted	Each	2	-	2
d Grinding (Bridge Section)	Sq Yd	2163	-	2163
ary Construction Fence	Foot	-	381	381
ary Shoring And Cribbing	Each	-	1	1
For Gates	Each	-	4	4

# TOTAL BILL OF MATERIAL

Porous

Concret

Slope W

Protecti

Concret

Protect.

Furnish

Reinfor

Bar Spi

Slope W

Preform

Preforn

Concret

Epoxy

Slope W

Bridge

Bridge

Protect

Approad

Approad

Structu

Bridge

Cleanin Bridge

Structu

To Or Le

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Than 5

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Locks F

OF SHEETS & TBOM	F.A.I. RTE. SECTION			COUNTY	TOTAL SHEETS	SHEET NO.	
16-0131 (NB)	90/94	2020-005-BR			соок	908	565
					CONTR	ACT NO.	62K73
05-22 SHEETS			ILLINOIS	FED. A	D PROJECT		



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### STAGE I REMOVAL

- 1. Install Temporary Concrete Barrier as shown to locate traffic on the east side of the existing structure.
- 2. Perform  $\frac{3}{4}$ " Bridge Deck Scarification.
- 3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the North and South Abutments.
- 5. Remove existing longitudinal preformed joint seal between north parapet and reversible lane parapet.

### STAGE I CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct expansion joints and install new preformed joint strip seals within the limits of Stage I Construction.
- *3. Perform Structural Repair of Concrete and epoxy crack injection for the abutments and piers.*
- 4. Apply 3" Bridge Deck Latex Concrete Overlay.
- 5. Perform ¼" diamond grinding to bridge deck and abutment hatched block.
- 6. Perform Bridge Deck Grooving (Longitudinal) for the 3" Bridge Deck Latex Concrete Overlay and reconstructed abutment expansion joint areas.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 8. Apply Protective Coat to top and inside faces of parapets, reconstructed abutment expansion joint areas, and to the surfaces of the new overlay.
- 9. Perform Slope wall repairs as shown on the plans.
- 10.Replace existing longitudinal preformed joint seal between north parapet and reversible lane parapet.

### STAGE II REMOVAL

- 1. Install Temporary Concrete Barrier as shown to locate traffic on the west side of the existing structure.
- 2. Perform <sup>3</sup>/<sub>4</sub>" Bridge Deck Scarification.
- *3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.*
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the North and South Abutments.
- 5. Perform temporary shoring and cribbing at locations shown on the plans within the limits of Stage II Removal.

#### Match Existing Cross-Slopes.

N (SHEET 1 OF 2)	F.A.I. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
16-0131 (NB)	90/94	2020-005-BR			соок	908	566
10-0131 (NB)					CONTR	ACT NO.	62K73
05-22 SHEETS			ILLINOIS	FED. A	D PROJECT		



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ENGINEERING GROUP, LLC

PLOT DATE =

DATE - 4/29/2024

REVISED -

# STAGE II CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct expansion joints and install new preformed joint strip seals within the limits of Stage II Construction.
- 3. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
- 4. Apply 3" Bridge Deck Latex Concrete Overlay.
- 5. Perform  $\frac{1}{4}$ " Diamond Grinding to bridge deck and abutment hatched block.
- 6. Perform Bridge Deck Grooving (Longitudinal) for the 3" Bridge Deck Latex Concrete Overlay and reconstructed abutment expansion joint areas.
- 7. Adjust drainage scuppers.
- 8. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the Approach slab and taper into existing roadway. See Roadway Plans.
- 9. Apply Protective Coat to top and inside faces of parapets and reconstructed abutment expansion joint areas, and to the surfaces of the new overlay.
- 10. Perform Slope wall repairs as shown on the plans.

Match Existing Cross-Slopes.

N (SHEET 2 OF 2)	F.A.I. SECTION RTE.			COUNTY	TOTAL SHEETS	SHEET NO.	
16-0131 (NB)	90/94 2020-005-BR			соок	908	567	
10-0131 (NB)					CONTR	ACT NO.	62K73
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SHEET S05-05 OF S0

beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart,

F.A.I. RTE	I. SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
90/94	90/94 2020-005-BR			соок	908	568
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S. Approach Slab 232'-41/2" End-to-End Prop. Deck 1'-8¼" 68'-0'' 93'-0'' Span 3 Span 2 33'-5½" I Construction Preformed Joint Seal 21/2". -0.3 SY See Detail A Q Brg. Pier 2 I-90/94 NB Roadway & Brg. Pier 1 Stage Sta. 241+65.13 & Stage Const. Line Deck \_ <sup>©</sup>' 41+00 242+00 -2 SY Lanes 6 Structure Brg. S. Abut. 0ut-to-0ut Sta. 242+11.63 ⅆ Sta. 240+97.13 6 4 struction -0.4.50.4 SY -0.3 SY гſ Perform Bridge Deck Limits of  $\frac{3}{4}$ " Bridge Deck -31/4" Grooving (Longitudinal) Reconstruct Expansion Scarification and 3" Bridge on traffic lanes <u>46'-</u> 11 ( Deck Latex Concrete Overlay, (both ends) 1/4" Diamond Grinding Stage 8 Drainage scupper, ∠Perform ¾" Bridge Deck to be adjusted, typ. Scarification, Apply 3" Bridge See Sheet S05-07 Deck Latex Concrete Overlay, Perform 1/4" Diamond Grinding and Apply Protective Coat 6'-Ra Exist. Light Pole to remain -NOTES: 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of  $\checkmark$ deck repairs at the time of construction. € Webster Ave. 2. For bridge deck final cross section, see Sheet S05-04. 3. For South and North transverse joint removal and reconstruction, see Sheets S05-08 thru S05-13. I-90/94 Apply 2" Stone-Matrix Asphalt (SMA) Rev Lanes 4. Perform  $\frac{1}{4}$ " Diamond Grinding to top of bridge deck and abutment hatched block overlay on the North and South DECK PLAN 8%" Approach Slabs 5. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes. (see Roadway Plans) 2 6. Protective Coat shall be applied to the top of reconstructed transverse joints, top and inside face of parapets, and top of latex concrete overlay. NOTES (CONT.): 7. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost incidental to 10. Approach Slab Repair (Full Depth) and Approach Slab Repair (Partial Depth) Concrete Removal. quantities have been estimated (based on a nominal 3% of bridge approach 2" area) for bidding purposes only. The actual areas to be repaired, and the 8. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging type(s) of repairs to be used, will determined by the Engineer in the field the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or at the time of construction. 1/2" diaphragms to remain caused by the Contractor in the performance of his/her work shall 1" be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department. DETAIL A 9. Removal of the existing preformed joint seal is included in the cost of Preformed Joint (Reinforcement not shown for clarity) (Looking Upstation) Seal 21/3

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 STRUCTURE NO. 016-0131 (NB)
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 SHEET \$05-06
 OF \$05-22
 SHEETS
 ILLINOIS
 FED. AID PROJECT
 CONTRACT NO. 62K73





# SECTION B-B

STRUCTURE NO. 01 SHEET S05-07 OF S05-

DRAINAGE SCUPPER TYPE A

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## NOTES

- 1. The Contractor shall field verify Existing Dimensions and Details of the Existing Scuppers and make necessary adjustments prior to construction of New Adjusting Rings or ordering of material for Adjusting Drainage Scuppers.
- 2. All Cast Iron Parts shall be Grey Iron conforming to the requirements of AASHTO M 105, Class 35B.
- 3. Cast Iron Parts shall be unfinished.
- 4. The Contractor shall take appropriate measures to ensure that Protective Coat is not applied to the scuppers.
- 5. Adjusting Rings shall be from Neenah or approved equal. Structural steel weldments or equal section and of the same configuration may be submitted in place of Cast Iron. Fillet or full penetration welds may be used for weldments. Details shall be submitted to the Engineer for approval.
- 6. Provide a  $\frac{1}{6}$ " Fillet Weld around perimeter of new Adjusting Rings to secure to existing Scupper.
- 7. Cost of all labor and materials necessary to clean all existing floor drains and scuppers, install adjusting scupper rings, remove and reinstall grates is included in the cost for Drainage Scupper to be Adjusted.

13⁄4''

Prop. Adjusting Ring B

- Prop. Adjusting Ring A

## BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scuppers To Be Adjusted	Each	2

ADJUSTMENT DETAILS	F.A.I. RTE	A.I. SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
6-0131 (NB)	90/94	90/94 2020-005-BR			соок	908	570
0-0131 (NB)					CONTR/	ACT NO.	62K73
-22 SHEETS			ILLINOIS	FED. A	D PROJECT		



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E	ENGINEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -		SHEET S05-10 OF S05-22
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SHEET S05-13 OF S05-22 SHEETS

ILLINOIS FED. AID PROJECT



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SHEET S05-14 OF S0

The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the  $4 \ensuremath{\mathscr{V}}_2$  " maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

> weld a Omit seal

flush

# LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	289

IT STRIP SEAL 16-0131 (NB)		SECT	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
		90/94 2020-005-BR		соок	908	577	
10-0131 (ND)					CONTR/	ACT NO.	62K73
05-22 SHEETS	ILLINOIS FED. AID PROJECT			D PROJECT			



- 4. Diaphragm connection holes shall be  $1_{16}^{\prime\prime}$ " for  $7_8^{\prime\prime}$ " bolts. Two hardened washers shall be required at diaphragm connections. Fasteners shall be high strength bolts.
- 5. Cost of field drilling, bolts, nuts and washers shall be included in the cost of Furnishing and Erecting Structural Steel.

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E Z		PLOT SCALE =	DRAWN - LAB	REVISED -	DEPARTMENT OF TRANSPORTATION		-		CONTR/	ACT NO. 62K73
	ENGINEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -		SHEET S05-15 OF S05-22 SHEETS	ILLINOIS FED. AID PROJECT			

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<u>BILL OF MATERIAL</u>						
UNIT	QUANTITY					
Pound	330					
Pound	330					
	UNIT Pound					

# <u>LEGEND</u>



Remove and Replace Exist. Diaphragm



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 Image: Property of the property

STRUCTURE NO. 016-0131 (NB) SHEET S05-17 OF S05-22 SHEETS

# BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	640
Epoxy Crack Injection	Foot	6
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	36
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	5

CONTRACT NO. 62K73

ILLINOIS FED. AID PROJECT



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BILL OF MATERIAL	L
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ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	491
Epoxy Crack Injection	Foot	24
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	26
Temporary Shoring And Cribbing	Each	1



STRUCTURE NO. 016-0131 (NB) **DEPARTMENT OF TRANSPORTATION** OT SCALE = DRAWN - FL REVISED DATE - 4/29/2024 SHEET S05-19 OF S05-22 SHEETS PLOT DATE = REVISED -ENGINEERING GROUP LLC 2:43:47 PM

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ILLINOIS FED. AID PROJECT

CONTRACT NO. 62K73





### EXISTING LIGHTING: PIER 2 (Looking North)



EXISTING LIGHTING: PIER 2 (Looking South)



EXIST. DRAINAGE SYSTEM REPAIR: LOCATION 2 - PIER 2 (Looking North)

	USER NAME =	DESIGNED - FL	REVISED -		PIER 2 REPAIRS	F.A.I. RTE	SECTION	COUNTY TOTAL SHEET SHEETS NO.	
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ITEM	UNIT	QUANTITY
Bridge Drainage System Repair	Foot	5
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	26
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	8

## <u>LEGEND</u>



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Structural Repair of Concrete (Depth

SF

Square Foot

Greater than 5 inches)



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SHEET S05-21 OF S05-2





### INSTALLATION AND SETTING METHODS

- "A" : Set bar splicer assembly by means of a template bolt.
- "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

(All components shall be provided from one supplier)

Location	Bar	No. assemblies	Minimum
	size	required	Iap length
South Abutment	#5	10	3'-6"
	#6	6	4'-0"
North Abutment	#5	10	3'-6"
North Abutment	#6	6	4'-0"

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# STANDARD MECHANICAL SPLICER

Location	Bar	No. assemblies
	size	required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements

for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.



### GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 3. Bars noted thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bars per line.
- 4. All exposed concrete edges shall have a  $\frac{3}{4}$ "x45° chamfer except where shown otherwise.
- 5. Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.
- 6. For SMA overlay on Approach Slab, see Civil Sheets.
- 7. Protective Coat shall be applied to the top of reconstructed transverse joint areas, top and inside faces of parapets, and top of Latex Concrete Overlay.
- 8. Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F
- 9. Prior to pouring the new concrete deck for expansion joint reconstruction and deck slab repairs, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding  $\frac{1}{4}$ " deep shall be identified and reported to the Bureau of Bridges and Structures for further dispositions. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 10. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 11. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures"
- 12. All new structural steel shall be hot-dip galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel"
- 13. Fasteners shall be ASTM F 3125 Grade A325 Type 1. Fasteners shall be hot dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel." Bolts  $\frac{3}{4}$  in. diameter, holes  $^{13}$ /<sub>16</sub> in. diameter, unless otherwise noted.
- 14. No field welding is permitted except as specified in the contract documents
- 15. Adjacent I-90/94 reversible bridge is not shown throughout the plans for clarity.
- 16. The Contractor shall take the necessary precautions for the protection of passing vehicles, bicycles and pedestrians from falling objects and/or materials until completion of work.
- 17. The Contractor is responsible to remove, support and reinstall all existing electrical conduits interfering with the work. See special provision "Protection and Maintenance of Existing Underpass Luminaires".
- 18. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 19. The Contractor is responsible to protect the existing conduit embedded in the parapet during concrete removal and construction. Any damage to the existing conduit shall be repaired by the Contractor at no additional cost to the Department.
- 20. Where underpass lighting is present on the structure, the Contractor shall adjust the Protective Shielding to ride above the existing lighting fixtures in order to maintain the existing level of lighting on the roadway underneath. Details shall be approved by the Engineer before installation.
- 21. Any adjustment done to the Protective Shield System must not change the load carrying capacity (or containment specifications) as indicated in the Standard Specifications. Cost of adjusting shielding is including in the cost of Protective Shield.
- 22. Calculated weight of Structural Steel = 2,990 lb (M270 Grade 36)

## INDEX OF SHEETS

- S06-01 General Plan and Elevation General Notes, Index of Sheets & TBOM 506-02 506-03 Stage Construction (Sheet 1 of 2) 506-04 Stage Construction (Sheet 2 of 2) 506-05 Temporary Concrete Barrier S06-06 Deck Repair Plan S06-07 E. Abut. Joint Removal & Reconstruction (Sht. 1 of 3) S06-08 E. Abut. Joint Removal & Reconstruction (Sht. 2 of 3) S06-09 E. Abut. Joint Removal & Reconstruction (Sht. 3 of 3) S06-10 W. Abut. Joint Removal & Reconstruction (Sht. 1 of 3) S06-11 W. Abut. Joint Removal & Reconstruction (Sht. 2 of 3) S06-12 W. Abut. Joint Removal & Reconstruction (Sht. 3 of 3) S06-13 Preformed Joint Strip Seal Framing Plan 506-14 S06-15 Structural Steel Repair Details (Sheet 1 of 3) Structural Steel Repair Details (Sheet 2 of 3) 506-16 Structural Steel Repair Details (Sheet 3 of 3) 506-17 S06-18 East Abutment Repairs S06-19 West Abutment Repairs
- Pier 1 Repairs 506-20
- S06-21 Pier 2 Repairs
- S06-22 Slope Wall Repairs
- S06-23 Bar Splicer Assembly and Mechanical Splicer Details

# SCOPE OF WORK

- 1. Provide Protective shield within limits indicated on the plans.
- 2. Scarify  $\frac{3}{4}$ " from the bridge deck slab.
- 3. Perform Deck Slab Repairs.
- 4. Reconstruct Expansion Joints at the East and West abutments and install new preformed joint strip seals.
- 5. Apply a 3" Bridge Deck Latex Concrete Overlay on Bridge Deck.
- 6. Perform  $\frac{1}{4}$ " Diamond Grinding to top of bridge deck and abutment hatched block.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay on the approach Slabs, see Roadway Plans,
- 8. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- 9. Apply protective coat to the top of reconstructed transverse joint areas, top and inside faces of parapets, and top of Latex Overlay.
- 10. Replace diaphragms as shown on the plans.
- 11. Perform structural concrete repairs and epoxy crack injection for the abutments and piers as noted on the plans.
- 12. Perform Slope Wall repairs.

### GENERAL NOTES (CONT.):

- 23. The Contractor shall contact Chandra Libby, the Director of City of Chicago Department of Family Support Services (DFSS) at 312-746-5443 or Chandra.Libby@cityofchicago.org to coordinate the relocation of persons and their personal belongings under the bridges within the areas bounded by the temporary chain-link-fence.
- 24. The intent of the temporary fence is to deny access of any unauthorized personnel under the bridge during construction. Actual fence installations may vary from what is shown on the plans. All fence installations must be approved by the Engineer.
- 25. Concrete Sealer shall be applied to the designated areas of the abutments.
- 26. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See special provision for Debris Removal.

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Porous Gran

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Slope Wall F

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Furnishing ,

Reinforceme

Bar Splicer:

Preformed .

Concrete Se

Epoxy Cracl

Slope Wall

Bridge Deck

Protect And

Luminaire

Approach Si

Approach Si

Structural S Bridge Deck

Bridge Deck

Structural H

Eaual To Or

Structural R

Greater Tha

Deck Slab R

Deck Slab R

Diamond Gr

*Temporary* 

Locks For G

emporary (

Inches

Preformed

Slope Wall

ITEM	UNIT	SUPER	SUB	TOTAL
nular Embankment	Cu Yd	-	2	2
emoval	Cu Yd	32.4	-	32.4
Removal	Sq Yd	-	4	4
Shield	Sq Yd	755	-	755
iperstructure	Cu Yd	35.8	-	35.8
Coat	Sq Yd	1,690	-	1,690
And Erecting Structural Steel	Pound	2,990	-	2,990
ent Bars, Epoxy Coated	Pound	5,240	-	5,240
5	Each	32	-	32
4 Inch	Sq Yd	-	4	4
Joint Seal 2 1/2"	Foot	196	-	196
Joint Strip Seal	Foot	183	-	183
ealer	Sq Ft	-	833	833
k Injection	Foot	-	85	85
Crack Sealing	Foot	-	63	63
k Grooving (Longitudinal)	Sq Yd	1,042	-	1,042
Maintain Existing Underpass	L Sum	-	0.04	0.04
lab Repair (Full Depth)	Sq Yd	49	-	49
lab Repair (Partial Depth)	Sq Yd	49	-	49
Steel Removal	Pound	2,990	-	2,990
k Latex Concrete Overlay, 3	Sq Yd	1,436	-	1,436
k Scarification 3/4"	Sq Yd	1,436	-	1,436
Repair Of Concrete (Depth <sup>-</sup> Less Than 5 Inches)	Sq Ft	-	561	561
Repair Of Concrete (Depth an 5 Inches)	Sq Ft	-	14	14
Repair (Full Depth, Type I)	Sq Yd	0.4	-	0.4
Repair (Full Depth, Type II)	SqYd	53	-	53
inding (Bridge Section)	Sq Yd	1,499	-	1,499
Construction Fence	Foot	-	255	255
Shoring And Cribbing	Each	-	3	3
ates	Each	-	4	4



Parapet



- 1. Install temporary concrete barrier as shown to locate traffic on the north side of the existing structure.
- 2. Perform  $\frac{3}{4}$ " bridge deck scarification.
- 3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the abutments.
- 5. Perform temporary shoring and cribbing at locations shown on the plans with the limits of stage I removal.
- 6. Remove existing longitudinal preformed joint seal between north parapet and reversible lane parapet.

### STAGE I CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct expansion joints and install new preformed joint strip seals within the limits of Stage I Construction.
- $\frac{1'-7''}{3}$  3. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
  - 4. Apply 3" Bridge Deck Latex Concrete Overlay.
  - 5. Perform  $\frac{1}{4}$ " Diamond Grinding to bridge deck and abutment hatched block.
  - 6. Perform Bridge Deck Grooving (Longitudinal) for the 3" Bridge Deck Latex Concrete Overlay and reconstructed abutment expansion joint areas.
  - 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
  - 8. Apply protective coat to top and inside faces of south parapet, reconstructed abutment expansion joints and to the surfaces of the new overlay.
  - 9. Perform slope wall repairs as shown on the plans.
  - 10. Replace existing longitudinal preformed joint seal between north parapet and reversible lane parapet.

## STAGE II REMOVAL

- 1. Install temporary concrete barrier as shown to locate traffic on the south side of the existing structure.
- 2. Perform  $\frac{3}{4}$ " bridge deck scarification.
- 3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the abutments.
- 5. Perform temporary shoring and cribbing at locations shown on the plans with the limits of stage II removal.

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F.A.I. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2020-005-BR			соок	908	588
			CONTRACT NO. 62K73			
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ENGINEERING GROUP, LLC

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# STAGE II CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct expansion joints and install new preformed joint strip seals within the limits of Stage II Construction.
- 3. Perform structural repair of Concrete and epoxy crack injection for the abutments and piers.
- 4. Apply 3" Bridge Deck Latex Concrete Overlay.
- 5. Perform  $\frac{1}{4}$ " Diamond Grinding to bridge deck and abutment hatched block.
- 6. Perform bridge deck grooving (longitudinal) for the 3" bridge deck latex concrete overlay and reconstructed abutment expansion joint areas.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 8. Apply protective coat to top and inside faces of north parapet, reconstructed abutment expansion joints and to the surfaces of the new overlay.
- Parapet 9. Perform slope wall repairs as shown on the plans.

\*Match existing cross slopes

N (SHEET 2 OF 2) 16-0130 (NB)		F.A.I. SECTION RTE			COUNTY	TOTAL SHEETS	SHEET NO.
		2020-005-BR			соок	908	589
16-0130 (NB)	CONTRACT N					ACT NO.	62K73
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SHEET S06-05 OF S06

beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart,

RETE BARRIER		SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
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3-23 SHEETS			ILLINOIS	FED. A	D PROJECT		

E. Approach 195'-4¾" End-to-End Deck 1'-41/4" 54'-6" 83'-8'' 54'-6'' Span 1 Span 2 Span 3 Preformed Joint Seal € Damen Ave. 2<sup>1</sup>/<sub>2</sub>". See Detail 1 Exist. Light Pole to remain u0 Shldr 33'-5<sup>1</sup>/<sub>2</sub>" Stage I Constr Perform Bridge Deck - 12'-Grooving (Longitudinal)  $7' - 10^{1/_{2}''}$ on traffic lanes Lanes at = 24'-0 · Þ 0.4 SY & Brg. E. Abut. - Limits of ¾" Bridge Deck Sta. 246+42.43 Scarification and 3" Bridge *Q* Pier 2 ¢ I-90/94 NB Roadway Deck Latex Concrete Overlay, -0.2 SY 247+00 & Stage Const. Line 1/4" Diamond Grinding 248+00 🧯 Structure Pier 1 □**--** 0.5 SY Out `-Şta. 247+00.78 0<sup>1/2</sup>" 0.6 SY anes at  $\Box$ 0.8 SY 1.1 SY 1.1 SY <u>37'-</u> 11 Cr 214 Exist. Light Pole to remain Perform 3/4" Bridge Deck Apply 2" stone Matrix Asphalt Par (SMA) overlay on the East and Scarification, Apply 3" Bridge Deck Latex Concrete Overlay, West Approach Slabs. See least End Perform 1/4" Diamond Grinding and Roadway Plans NOTES Apply Protective Coat I-90/94 Rev Lanes PLAN 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction. 8<sup>7</sup>/8'' 2. For bridge deck final cross section, see Sheet S06-04. 7/8' 3. For East and West transverse joint removal and reconstruction, see Sheets S06-07 thru S06-12. NOTES (CONT.): 4. Perform  $\frac{1}{2}$ " Diamond Grinding to top of bridge deck and abutment hatched block. 10. Approach Slab Repair (Full Depth) and Approach 5. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes. Slab Repair (Partial Depth) quantities have been estimated (based on a nominal 3% of bridge 6. Protective coat shall be applied to the top and inside face of parapets and top of latex Concrete Overlay. approach area) for bidding purposes only. The actual areas to be repaired, and the type(s) of 7. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging to steel beams and diaphragms to repairs to be used, will determined by the 1/2" remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of Engineer in the field at the time of construction. his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department. \_\_\_\_\_\_\_1" 8. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved DETAIL 1 bar splicer or anchorage system. Cost incidental to Concrete Removal. (Reinforcement not shown for clarity) 9. Removal of the existing preformed joint seal is included in the cost of Preformed Joint Seal 21/2"



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 DECK REPAIR PLAN
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 STRUCTURE NO. 016-0130 (NB)
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 SHEET \$06-06 OF \$06-23 SHEETS
 CONTRACT NO. 62K73





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DATE

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SHEET S06-09 OF S06-

# BILL OF MATERIAL



Bar	No.	Size	Length	Shape	
a(E)	20	#5	22'-11"		
a1(E)	20	#5	25'-9"		
a2(E)	6	#6	6'-6"		
d(E)	8	#4	3'-11"	ر ا	
d1(E)	8	#5	2'-7"	~	
d2(E)	11	#4	3'-8''	L	
d3(E)	11	#5	3'-8''		
d4(E)	3	#5	2'-9"		
d5(E)	3 3	#5	4'-8''	L	
d6(E)	3	#5	5'-5"		
d7(E)	3	#5	5'-7"		
d8(E)	3	#5	2'-0"	4	
h(E)	12	#6	22'-2"		
h1(E)	12	#6	24'-10"		
s(E)	48	#6	3'-1"		
u(E)	89	#5	3'-2"		
Concrete	Removal		Cu Yd	16.0	
Concrete	Superst	ructure	Cu Yd	17.8	
Protectiv	e Coat		Sq Yd	36	
Reinforco Epoxy Co		nrs,	Pound	2,610	

4. Removal and disposal of the existing expansion joints is included with

5. Epoxy grout d4(E), d6(E) and d7(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost

### MIN BAR LAPS

3'-6'' #5

#6

4'-0''

ONSTRUCTION (SHT. 3 OF 3)	F.A.I. RTE	SECT	TION		COUNTY	TOTAL SHEETS	SHEET NO.
16-0130 (NB)		2020-005-BR			соок	908	594
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## BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	20	#5	22'-11"	
a2(E)	6	#6	6'-6''	
a3(E)	20	#5	26'-1"	
d(E)	7	#4	3'-11"	Ę
d1(E)	7	#5	2'-7"	$\sim$
d2(E)	11	#4	3'-8''	
d3(E)	11	#5	3'-8''	
d4(E)	3	#5	2'-9''	
d5(E)	3 3	#5	4'-8''	
d6(E)	3	#5	5'-5"	
d7(E)	3	#5	5'-7"	
d8(E)	3	#5	2'-9''	Ζ
h(E)	12	#6	22'-2"	
h2(E)	12	#6	25'-5"	
s(E)	48	#6	3'-1''	
u(E)	91	#5	3'-2''	П
Concrete	Removal	i	Cu Yd	16.4
Concrete	Superst	ructure	Cu Yd	18.0
Protectiv	e Coat		Sq Yd	36
Reinforce Epoxy Co		ars,	Pound	2,630

MIN BAR LAPS 3'-6" #5

#6 4'-0"

W. ABUT. JOINT REMOVAL & RECONSTRUCTION (SHT. 3 OF 3) STRUCTURE NO. 016-0130 (NB)		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		2020-005-BR	соок	908	597
STRUCTURE NO. 010-0130 (NB)		•	CONTR	ACT NO.	62K73
SHEET S06-12 OF S06-23 SHEETS		ILLINOIS F	ED. AID PROJECT		



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The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4<sup>1</sup>/<sub>2</sub>" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{6}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



# LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	183

T STRIP SEAL		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
16-0130 (NB)	90/94	2020-005-BR	соок	908	598	
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### NOTES:

- 1. All work is to be performed utilizing staged construction. See Sheets S06-03 and S06-04 for details.
- 2. For Diaphragms Removal and Replacement Details, see Sheets S06-15thru S06-17.
- З. All proposed diaphragms and associated connection plates and angles shall conform to the requirements of AASHTO M270 Grade 36.
- 4. Diaphragm connection holes shall be  $1_{16}^{\prime\prime\prime}$  for  $7_{6}^{\prime\prime\prime}$  bolts. Two hardened washers shall be required at diaphragm connections, fastners shall be high strength bolts.
- 5. Cost of field drilling, bolts, nuts and washers shall be included in the cost of Furnishing And Erecting Structural Steel.

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<u>BILL OF MATERIA</u>	<u>\L</u>	
ITEM	UNIT	QUANTITY
Furnishing And Erecting Structural Steel	Pound	2,990
Structural Steel Removal	Pound	2,990



Remove and Replace Exist. Diaphragm



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SHEET S06-15 OF S06-

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90/94	94 2020-005-BR			соок	908	600
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