

GENERAL NOTES

- 1. ELECTRICAL CONDUIT SHALL BE INSTALLED A MINIMUM OF 30" BELOW GRADE.
- 2. FIBER OPTIC CONDUIT SHALL BE INSTALLED A MINIMUM OF 42" BELOW GRADE.
- 3. CONDUIT CROSSING OVER/UNDER OTHER UTILITIES SHALL MAINTAIN A SEPARATION OF AT LEAST 12 INCHES OR AS SPECIFIED BY OWNING UTILITY.
- PROPOSED CONDUIT ROUTES SHOWN ON THE PLANS ARE SCHEMATIC ONLY. CONTRACTOR TO VERIFY EXACT ROUTE WITH ENGINEER.
- 5. ALL ABANDONED CABLES AND CONDUIT SHALL COMPLY WITH NEC REQUIREMENTS.

CONDUIT COUPLER

- THE CONTRACTOR SHALL INSTALL A CONDUIT COUPLER AT THE JOINT LOCATIONS IN
 THE CONCRETE PARAPET ON THE BRIDGE CAPABLE OF ACCEPTING THE LONGITUDINAL
 MOVEMENT. ALL METALLIC PARTS OF THE COUPLING SHALL BE MADE OF STAINLESS
 STEEL OR AS APPROVED BY THE ENGINERR. ANY NON-STAINLESS METAL SHALL BE HOT
 DIP GALVANIZED AND COATED TO PREVENT REACTION WITH THE CONCRETE.
- 2. THE CONTRACTOR SHALL INSTALL COUPLINGS AT ALL BRIDGE EXPANSION JOINTS AND SHALL BE RESPONSIBLE TO DETERMINE THE PROPER NUMBER OF COUPLINGS REQUIRED. SEE STRUCTURAL DRAWINGS FOR THE EXPANSION JOINT LOCATIONS.

ABBREVIATIONS AND ACRONYMS

NOTATION	DESCRIPTION
ATS	ATTACHED TO STRUCTURE
СНН	COMMUNICATION HANDHOLE
CNC	COILABLE NONMETALLIC CABLE
DMS	DYNAMIC MESSAGE SIGN
EB	EASTBOUND
ECC	ELECTRICAL CABLE IN CONDUIT
EHH	ELECTRICAL HANDHOLE
ELEC	ELECTRICAL
EOP	EDGE OF PAVEMENT
EOS	EDGE OF SHOULDER
EOT	EDGE OF TRAVEL WAY
(E)	EXISTING
FO	FIBER OPTIC
FT.	FEET
GND	GROUND
(GRN)	GREEN
GS	GALVANIZED STEEL
GSC	GALVANIZED STEEL CONDUIT (RIGID)
HDHH	HEAVY-DUTY HANDHOLE
НН	HANDHOLE
(1)	INSTALL
IN.	INCHES
ITS	INTELLIGENT TRANSPORTATION SYSTEMS
MM	MULTIMODE
NB	NORTHBOUND
PGSC	PVC COATED GALVANIZED STEEL CONDUIT
(R)	REMOVE
ROW	RIGHT-OF-WAY
SM	SINGLE MODE
SB	SOUTHBOUND
STA.	STATION
TEMP	TEMPORARY
WB	WESTBOUND

ITS PLAN LEGEND

EXISTING	PROPOSED	DESCRIPTION
		CONDUIT
— F0 ——	— F0 ——	FIBER OPTIC CABLE IN CONDUIT
Ţ	0	BARRIER WALL EMBEDDED JUNCTION BOX
—E—	—E—	ELECTRICAL CABLE IN CONDUIT
		STRUCTURE-MOUNTED JUNCTION BOX
	С	HEAVY-DUTY HANDHOLE (COMMUNICATIONS)
E	E	HEAVY-DUTY HANDHOLE (ELECTRICAL)
H		HEAVY-DUTY HANDHOLE (MIXED USE)
		INDUCTION LOOP
0-1>		RAMP METER SIGNAL ASSEMBLY OR FLASHER ASSEMBLY
\bowtie		GROUND MOUNTED CABINET
		LIGHTING CONTROLLER CABINET
$\dot{\Box}$	•	UTILITY SERVICE POLE

ve	USER NAME = \$USER\$	DESIGNED	-	НН	REVISED	-
ve 7		DRAWN	-	IG	REVISED	-
"	PLOT SCALE = \$SCALE\$	CHECKED	-	нн	REVISED	-
	PLOT DATE = 5/1/2024	DATE	-	04/29/2024	REVISED	-

SCALE: NTS

	PROPOSED	ITS	PLAN –	GENERAI	. NOTES	F.A.I. RTE	SEC	TION		COUNTY	TOTAL SHEETS
	I-90/94					90/94	4 2020-005-BR		COOK	908	
1-30/34				_				CONTRACT	F NO. 62		
	SHEET 1	OF 1	SHEETS	STA.	TO STA.			ILLINOIS	FED Δ	ID PROJECT	

	ITS SCHEDULE 62K73										
LOCATION	FIBER OPTIC CABLE IN CONDUIT 96 FIBERS SINGLE MODE	FIBER OPTIC CABLE IN CONDUIT 12 FIBERS SINGLE MODE	FIBER OPTIC SPLICE-LATERAL	REMOVAL OF FIBER OPTIC CABLE							
	FOOT	FOOT	EACH	FOOT							
STA 465+00 TO STA 480+00	1510			1500							
STA 525+00 TO STA 540+00	1510			1500							
STA 540+00 TO STA 555+00	1510			1500							
STA 615+00 TO STA 630+00	1510			1500							
TOTAL	6040			6000							
TOTAL +5%	6340			6300							

CONCRETE BA	RRIER SCHEDULE	62K73
LOCATION	CONCRETE BARRIER REMOVAL	CONCRETE BARRIER TRANSITION
	FOOT	FOOT
STA 337+57.11 TO STA 337+72.79	15.7	15.7
TOTAL	15.7	15.7

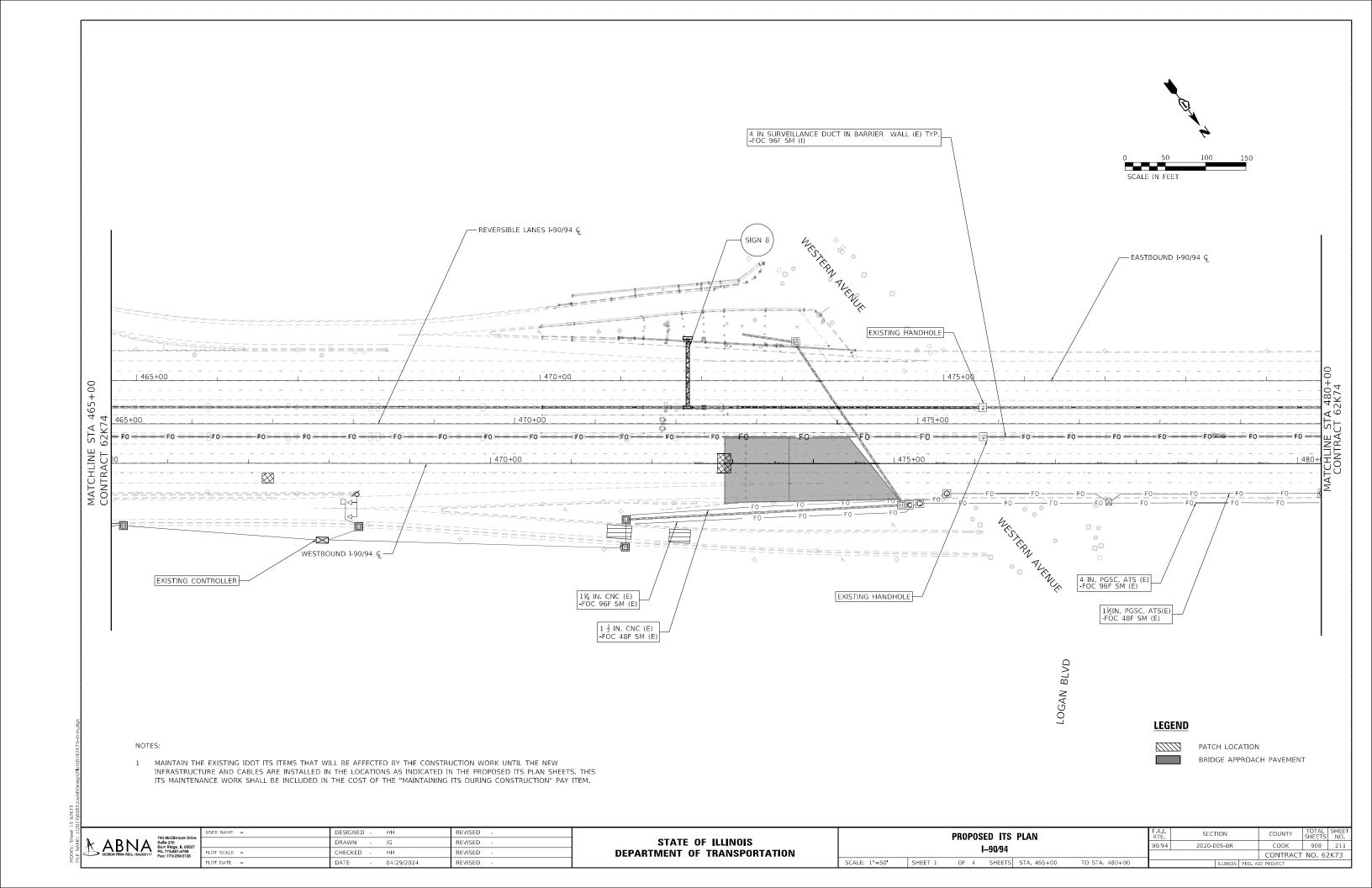
MAINTAINING ITS D	DURING CONSTRUCTION 62K73
LOCATION	MAINTAINING ITS DURING CONSTRUCTION
	Cal Mo
ALL THE PROJECT	12
TOTAL	12

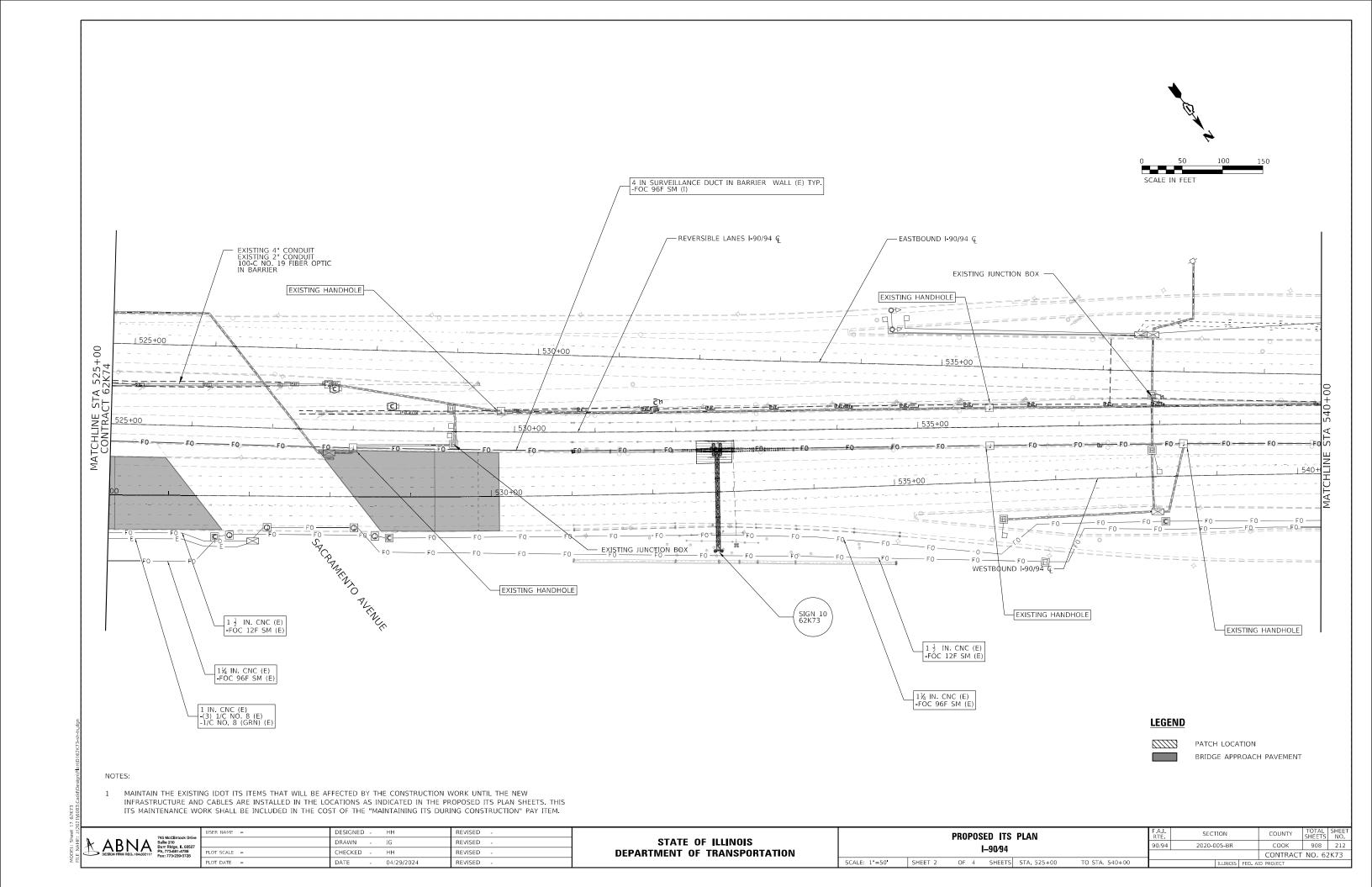
	ELECTRIC SCHEDULE 62K73									
LOCATION	UNDERGROUND CONDUIT GALVANIZED STEEL 4" DIA.	JUNCTION BOX TYPE J 41" X 12" X 9"	ATTACHED TO	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 12" X 12" X 4"	ELECTRIC CABLE IN CONDUIT 600 V (XLP-TYPE USE) 1/C NO.8	ELECTRIC CABLE IN CONDUIT 600 V (XLP-TYPE USE) 1/C NO.6				
	FOOT	EACH	EACH	EACH	FOOT	FOOT				
STA 337+67.47		1								
STA 337+67.47			1							
STA 337+75.94 TO STA 337+99.44	55.6									
STA 337+67.47				1						
STA 337+75.94 TO STA 337+99.44										
SUBTOTAL	55.6	1		1	80	240				
TOTAL +5%	60				85	252				
TOTAL	60	1	1	1	85	252				

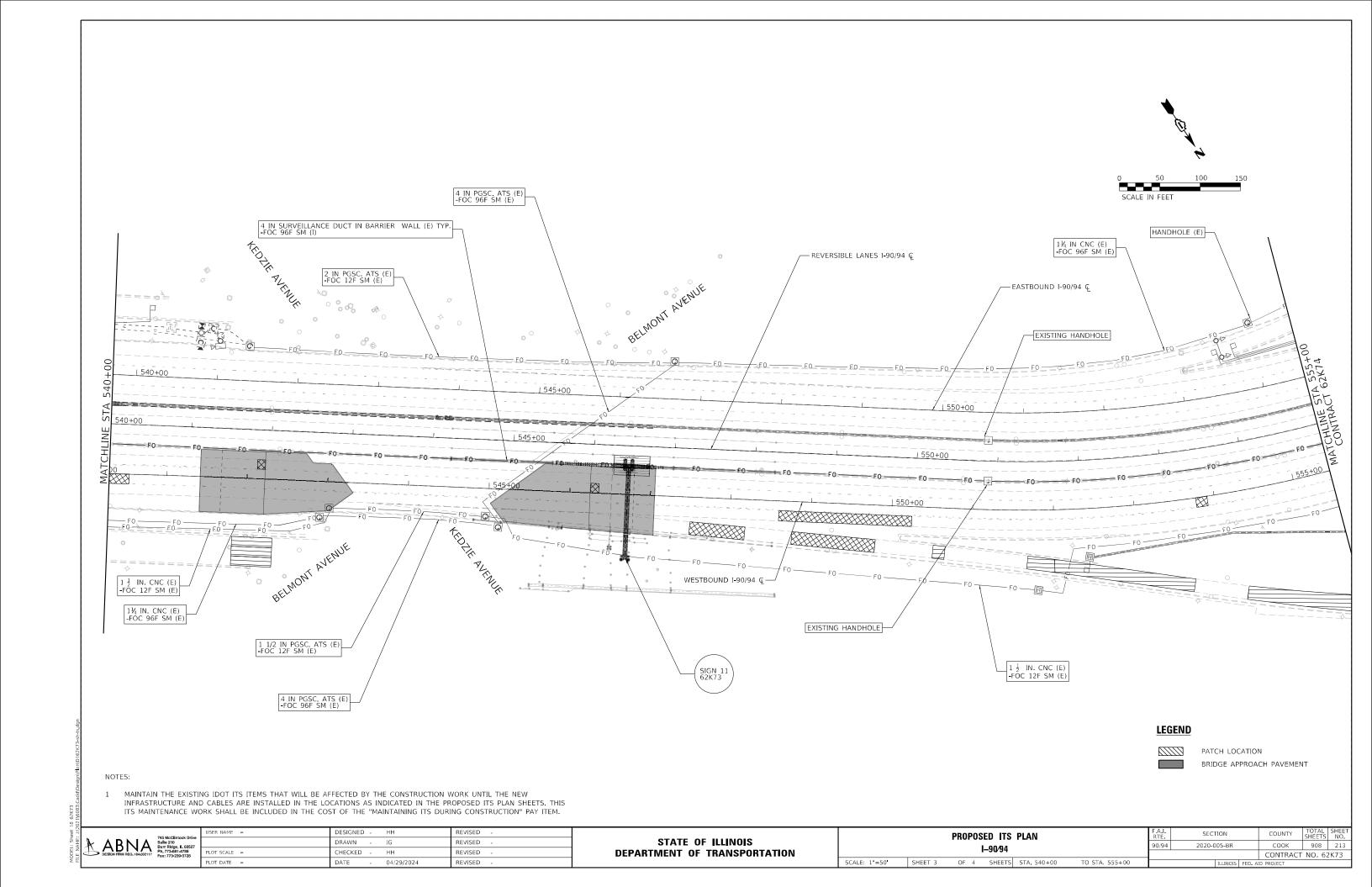
CONDUIT COUPLE	R 62K73
LOCATION	CONDUIT COUPLER
	EACH
EVERY JOINT ON BRIDGE DECKS	50
TOTAL	50

	USER NAME = \$USER\$	DESIGNED	-	НН	REVISED -	
ve		DRAWN	-	IG	REVISED -	
	PLOT SCALE = \$SCALE\$	CHECKED	-	нн	REVISED -	
	PLOT DATE = 5/1/2024	DATE	-	04/29/2024	REVISED -	

1								F.A.I. RTE. SECTION		TOTAL	SHEET NO.
ı				I-90⁄94	90/94	2020-005-BR	соок	908	210		
ļ	I-3U34								CONTRACT	NO. 62	2K 7 3
	SCALE: NTS	SHEET 1	ET 1 OF 1 SHEETS) STA. TO STA.					ILLINOIS FED. A	ID PROJECT		







4 IN SURVEILLANCE DUCT IN BARRIER WALL (E) TYP. -FOC 96F SM (I) 615+ 2K74 J 615+00

LEGEND

PATCH LOCATION

BRIDGE APPROACH PAVEMENT

NOTES:

MAINTAIN THE EXISTING IDOT ITS ITEMS THAT WILL BE AFFECTED BY THE CONSTRUCTION WORK UNTIL THE NEW INFRASTRUCTURE AND CABLES ARE INSTALLED IN THE LOCATIONS AS INDICATED IN THE PROPOSED ITS PLAN SHEETS. THIS ITS MAINTENANCE WORK SHALL BE INCLUDED IN THE COST OF THE "MAINTAINING ITS DURING CONSTRUCTION" PAY ITEM.

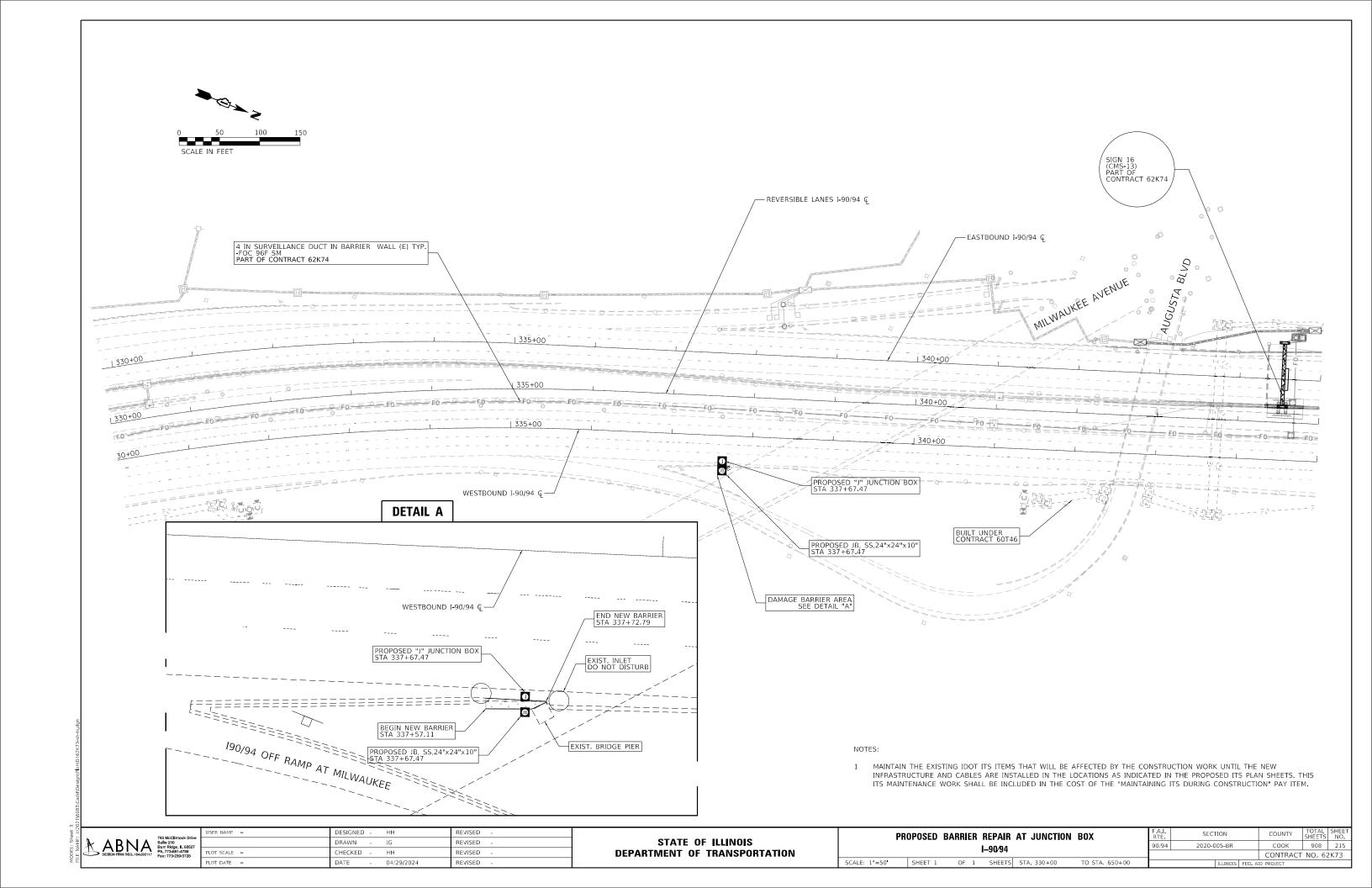
SIGN 13 62K73

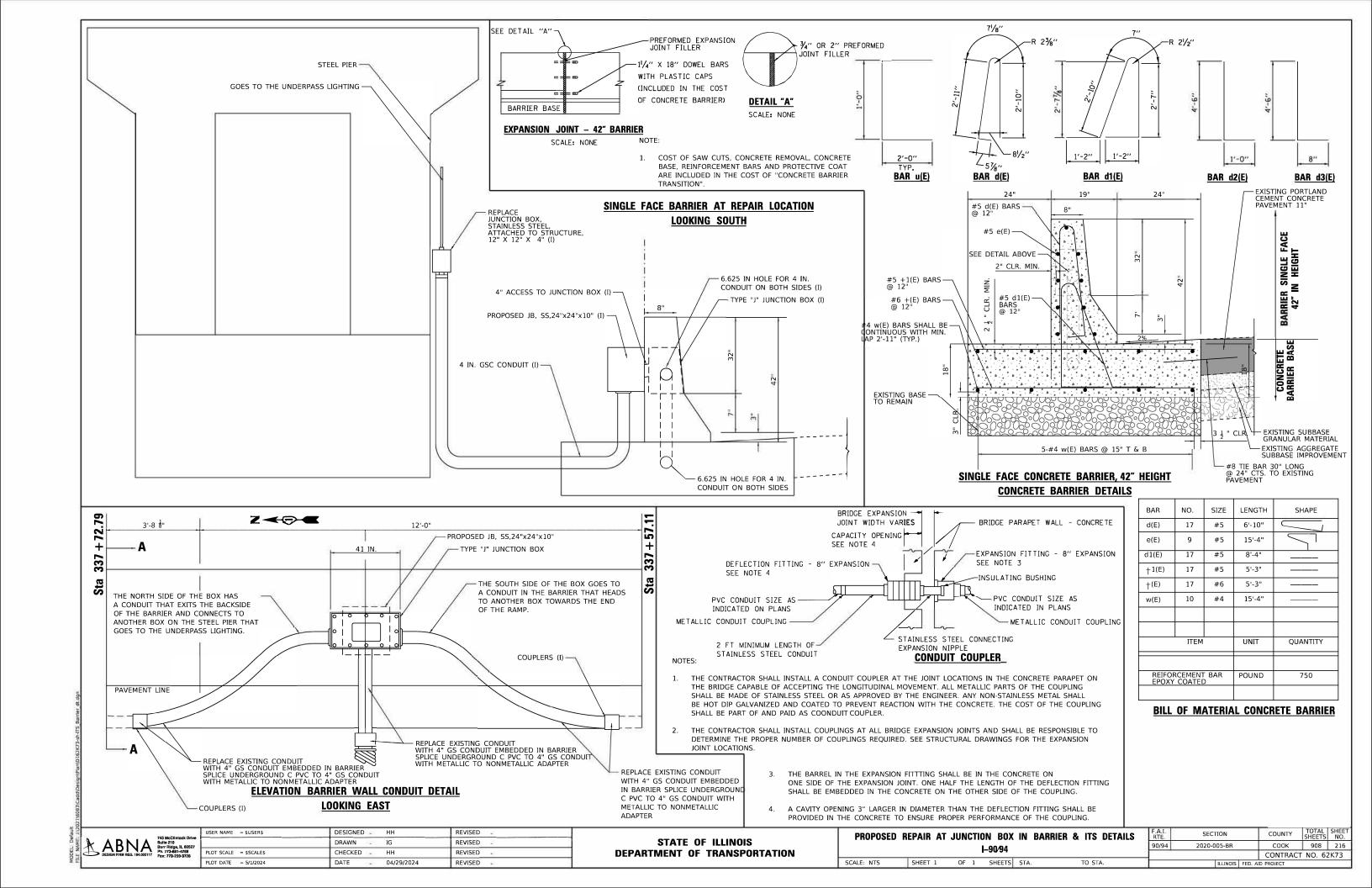
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k Drive 60527 '88 728		DRAWN	-	IG	REVISED -
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	PLOT DATE =	DATE	-	04/29/2024	REVISED -

STATI	E 01	FILLINOIS
DEPARTMENT	OF	TRANSPORTATION

EXISTING HANDHOLE

PROPOSED ITS PLAN							SECT	ION	COUNTY	SHEETS	
I–90∕94							2020-0	05-BR	COOK	908	214
				CONTRACT	NO. 62	2K73					
SCALE: 1"=50'	SHEET 4	OF 4	SHEETS	STA. 615+00	TO STA. 630+00	ILLINOIS FED. AID PROJECT					





ABBNA 745 McClintock Drive Sulte 210 Sulte 210

EXISTING

PROPOSED

 \boxtimes

 \boxtimes

FIBER OPTIC CABLE

HANDHOLE

POLE MOUNTED CABINET

GROUND MOUNTED CABINET

DESIGNED -REVISED DRAWN -IG REVISED CHECKED -REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SCALE: NTS

ITS PLANS FIBER OPTIC CABLE DIAGRAM I-90⁄94 SHEET 1 OF 3 SHEETS STA. 465+00 TO STA. 480+00

COUNTY TOTAL SHEET NO.

COOK 908 217 SECTION 90/94 2020-005-BR CONTRACT NO. 62K73

ITS 1

	WESTI		
MATCHLINE STA 465+00 CONTRACT 62K73	I-90/I-94 FOC 96F SM	(KENNEDY EXPY)	MATCHLINE STA 480+00 CONTRACT 62K73
	FOC 48F SM		
	FOC 96F SM		
		I	

EXISTING PROPOSED FIBER OPTIC CABLE \boxtimes POLE MOUNTED CABINET \boxtimes GROUND MOUNTED CABINET HANDHOLE

525+00 2K73	JUNCTION BOX JUNCTION BOX	FOC 96F SM
RACT (I-90/I-94 (KENNEDY EXPY)	
MATCHLINE STA 525+00 CONTRACT 62K73	FOC 96F SM	FOC 9
	FOC 12F SM	
		JUNCTION BOX
	FOC 96F SM	

ITS 2

COUNTY TOTAL SHEET NO.
COOK 908 218

ABNA DESIGN FIRM REG. 184,002117	745 McClintock Drive Sulte 210 Burr Ridge, IL 60527 Ph. 773-881-4788 Fax: 773-239-3728

	USER NAME =	DESIGNED	-	HH	REVISED	-
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"	PLOT SCALE =	CHECKED	-	НН	REVISED	-
	PLOT DATE =	DATE	-	04/29/2024	REVISED	-

STATE OF I	LLINOIS
DEPARTMENT OF THE	RANSPORTATION

ITS PLANS FIBER OPTIC CABLE DIAGRAM						F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	
			I-90⁄94			90/94	2020-005-BR	COOK	908	218
1-30-34								CONTRACT	NO. 62	K73
SCALE: NTS	SHEET 2	OF 3	SHEETS	STA. 525+00	TO STA. 555+00	ILLINOIS FED. AID PROJECT				

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745 McClintock Drive Sulks 210 BUT Ridge, IL 60527 Ph. 773-881-1788 Psx: 773-239-3728 Pp.

PROPOSED

 \boxtimes

 \boxtimes

FIBER OPTIC CABLE

HANDHOLE

POLE MOUNTED CABINET

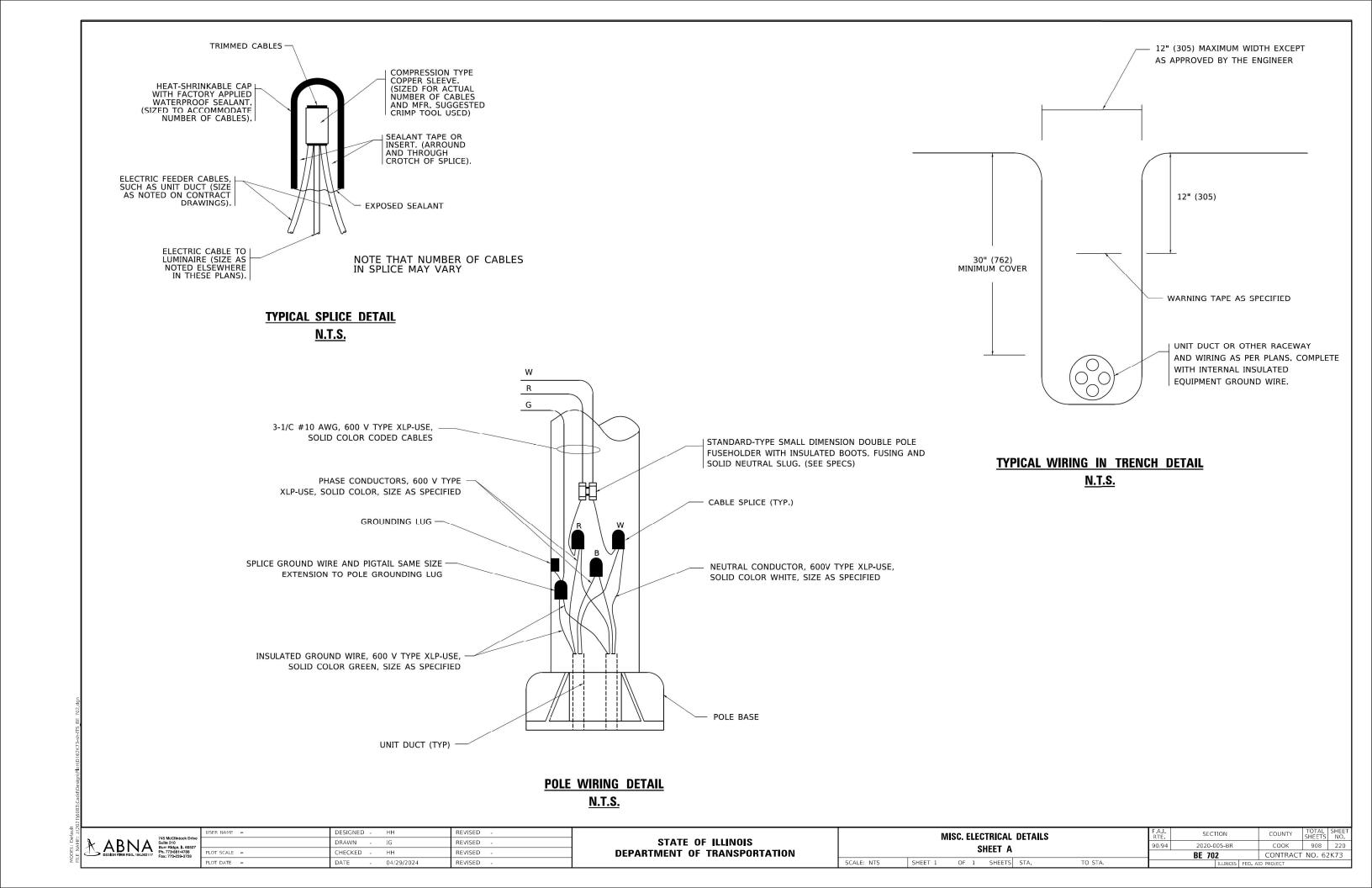
GROUND MOUNTED CABINET

EXISTING

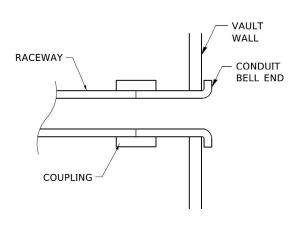
ive	USER NAME =	DESIGNED	-	НН	REVISED	-
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	PLOT DATE =	DATE	-	04/29/2024	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

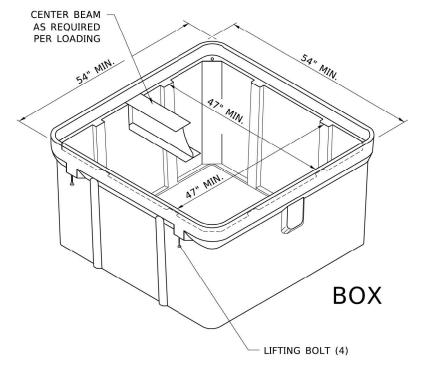
MATCHLINE STA 615+00 CONTRACT 62K73		630+00 2K73
NE S- RACT	I-90/I-94 (KENNEDY EXPY)	STA ACT 6
MATCH_I CONT	FOC 96F SM	MATCHLINE STA 630+00 CONTRACT 62K73



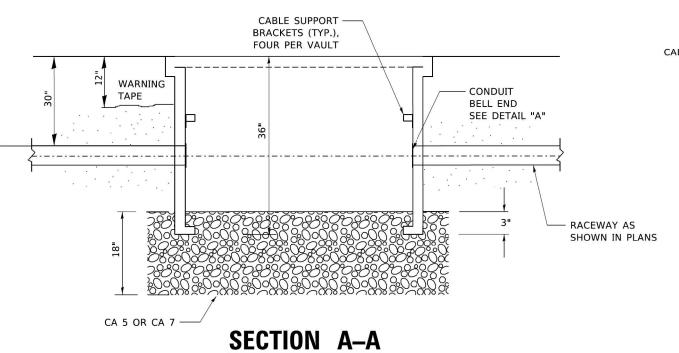
COMMUNICATIONS VAULT LOAD RATINGS							
COMPONENT	ANSI	LOA	.OADING				
COMPONENT	TIER	DESIGN	TEST				
вох	22	22,500 lbs.	37,750 lbs.				
COVER	22	22,500 lbs.	37,750 lbs.				

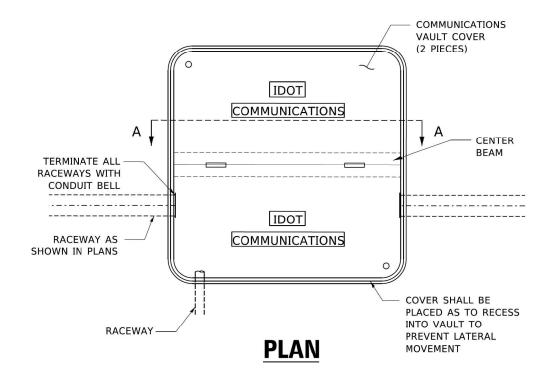


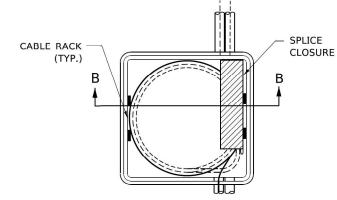
DETAIL A



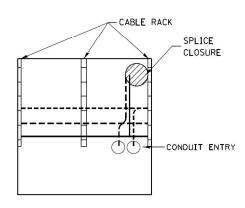
ISOMETRIC







TOP VIEW



SECTION B-B

NOTES:

- 1. BOX SHALL HAVE AN OPEN BASE.
- ALL OPENINGS IN STRUCTURE MUST BE MACHINED AT TIME OF FABRICATION OR PUNCH DRIVEN AT TIME OF PLACEMENT. IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS.
- 3. FIELD PLACEMENT OF COMMUNICATIONS VAULT SHALL BE AS DIRECTED BY THE ENGINEER.
- ALL DIMENSIONS ARE MINIMUM AND A LARGER SIZE HANDHOLE MAY BE USED, WITH THE APPROVAL OF THE ENGINEER, TO FACILITATE USING A MANUFACTURER'S STANDARD PRODUCT.

		U
» ΔRNIΔ	745 McClintock Drive Suite 210 Burr Ridge, IL 60527	
DESIGN FIRM REG. 184,002117	Ph. 773-881-4788 Fax: 773-239-3728	PI
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	USER NAME =	DESIGNED	-	НН	REVISED	-
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	PLOT SCALE =	CHECKED	-	нн	REVISED	-
	PLOT DATE =	DATE	-	04/29/2024	REVISED	-

CO	MMUNICATI	ON	VAI	JLT, CO	MPOSITE	CONCRETE
SCALE: NTS	SHEET 1	OF	1	SHEETS	STA.	TO STA.

	F.A.I. RTE	SECT	COUNTY	TOTAL SHEETS	SHEET NO.	
	90/94	2020-0	COOK	908	221	
_		BE 705		CONTRACT	NO. 62	2K73
			ILLINOIS	D PROJECT		

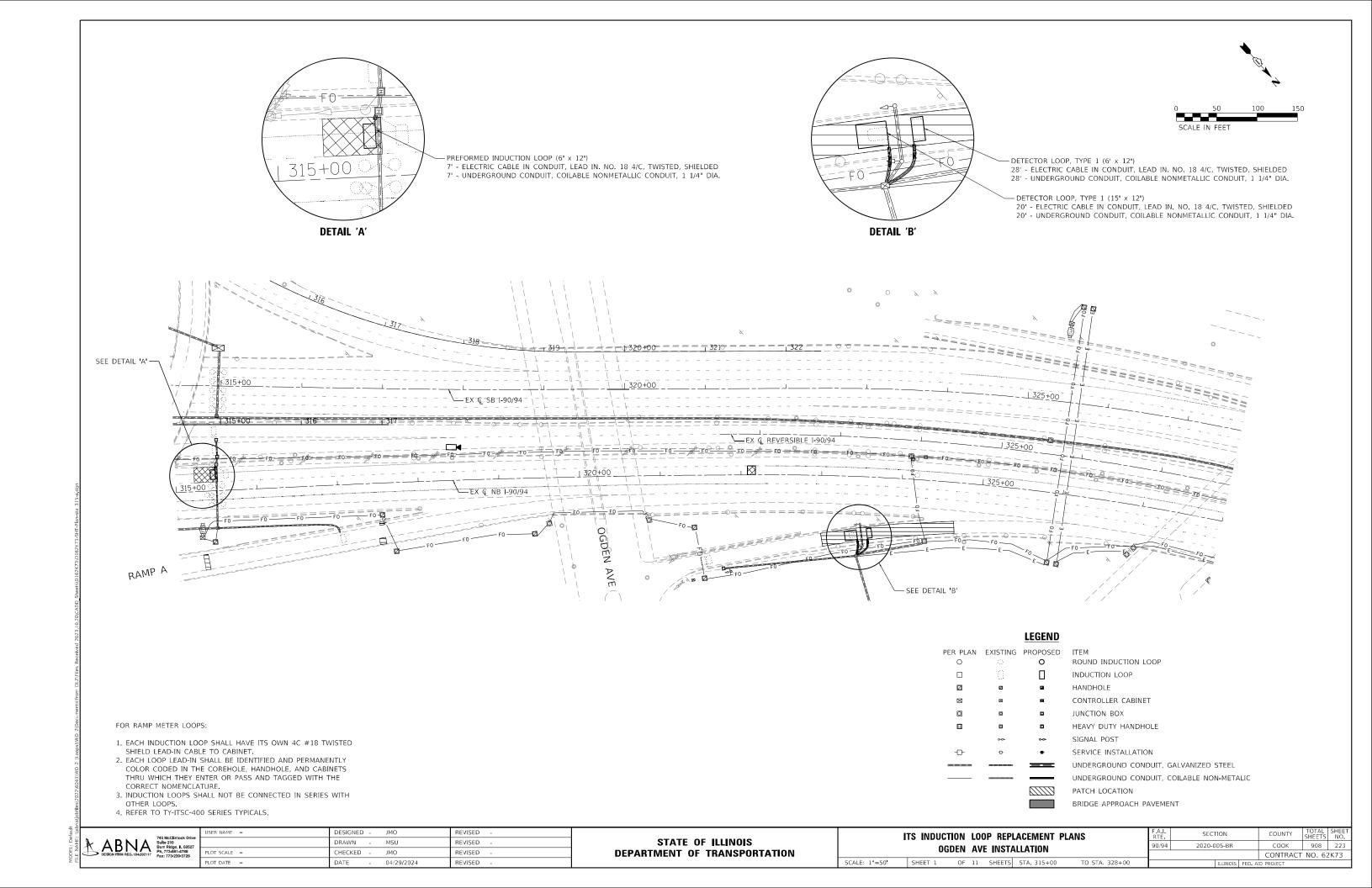
ITS INDUCTION LOOP REPLACEMENT SCHEDULE

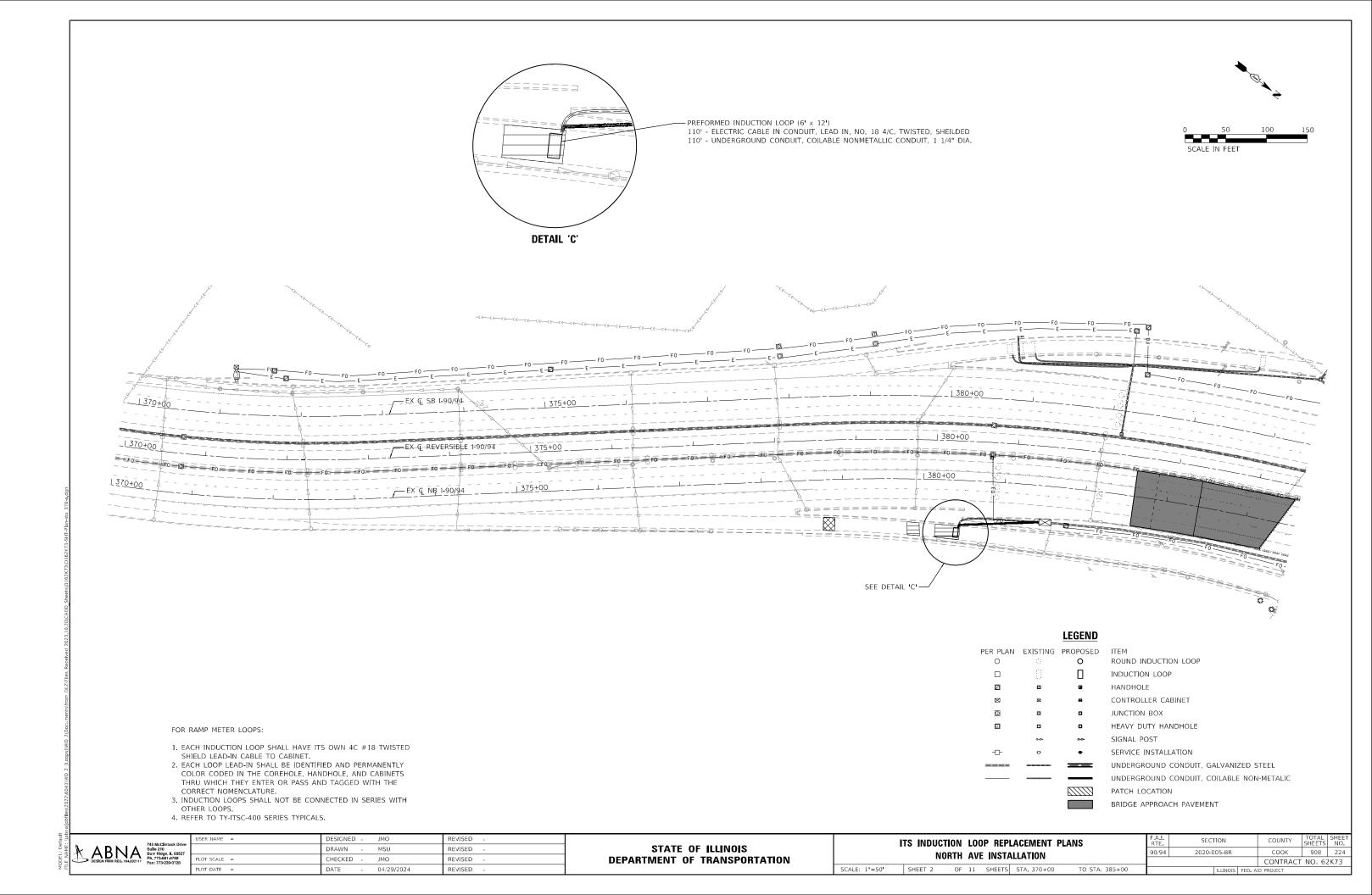
LOCATION	STA	OFFSET	UNDRGRD C GALVS 2	UNDRGRD C CNC 1 1/4	DET LOOP T1	EC C LEAD 18 4C TW SH	PREF INDUCTION LOOP
			FOOT	FOOT	FOOT	FOOT	FOOT
OGDEN AVENUE	315+49.16 323+47.32 323+59.94	26' L 78' R 76' R	0	55	102	55	42
E/O NORTH AVENUE	380+45.18 381+45.88	59' R 45' R	0	110	0	110	42
NORTH AVENUE	389+54.50 389+81.54 389+94.45	47' R 57' R 56' R	0	86	102	86	0
NORTH ARMITAGE AVENUE	418+51.18 420+06.89 420+19.64	53' R 51' R 50' R	0	338	60	338	42
WEBSTER AVE	438+69.05 438+74.37	119' R 20' R	23	12	0	12	67
NORTH WESTERN AVENUE	471+13.25 471+27.01 471+36.87	139' R 140' R 149' R	0	42	42	42	60
NORTH CALIFORNIA AVENUE	511+41.35 516+73.97 516+76.08	160' R 63' R 139' R	66	9	54	9	99
SACRAMENTO AVENUE	535+80.73 536+04.49 536+08.57	127' R 115' R 130' R	0	46	0	46	102
KIMBALL AVENUE	552+05.50 552+07.51 561+35.41 561+63.52 561+64.66 563+61.36 563+63.82 563+75.49	131' R 119' R 106' R 57' R 33' R 133' R 126' R 131' R	115	35	144	35	25
ADDISON STREET	583+86.31 583+98.15 584+37.71 586+75.85 587+69.15	160' R 162' R 133' R 150' R 144' R	125	112	92	112	50
KEELER AVENUE	639+53.13 639+64.12 639+65.97	170' R 174' R 169' R	22	0	102	0	0
TOTAL			351	845	698	845	529

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	USER NAME =	DESIGNED	-	JMO	REVISED	-
ive		DRAWN	-	MSU	REVISED	-
"	PLOT SCALE =	CHECKED	-	JMO	REVISED	-
	PLOT DATE =	DATE	-	04/29/2024	REVISED	-

						F.A.I. RTE	SECTIO
SCHEDULE OF QUANTITIES						90/94	2020-005
SCALE: NTS	SHEET 1	OF 1	SHEETS	STA.	TO STA.		ILL





DEPARTMENT OF TRANSPORTATION

REVISED

2020-005-BR

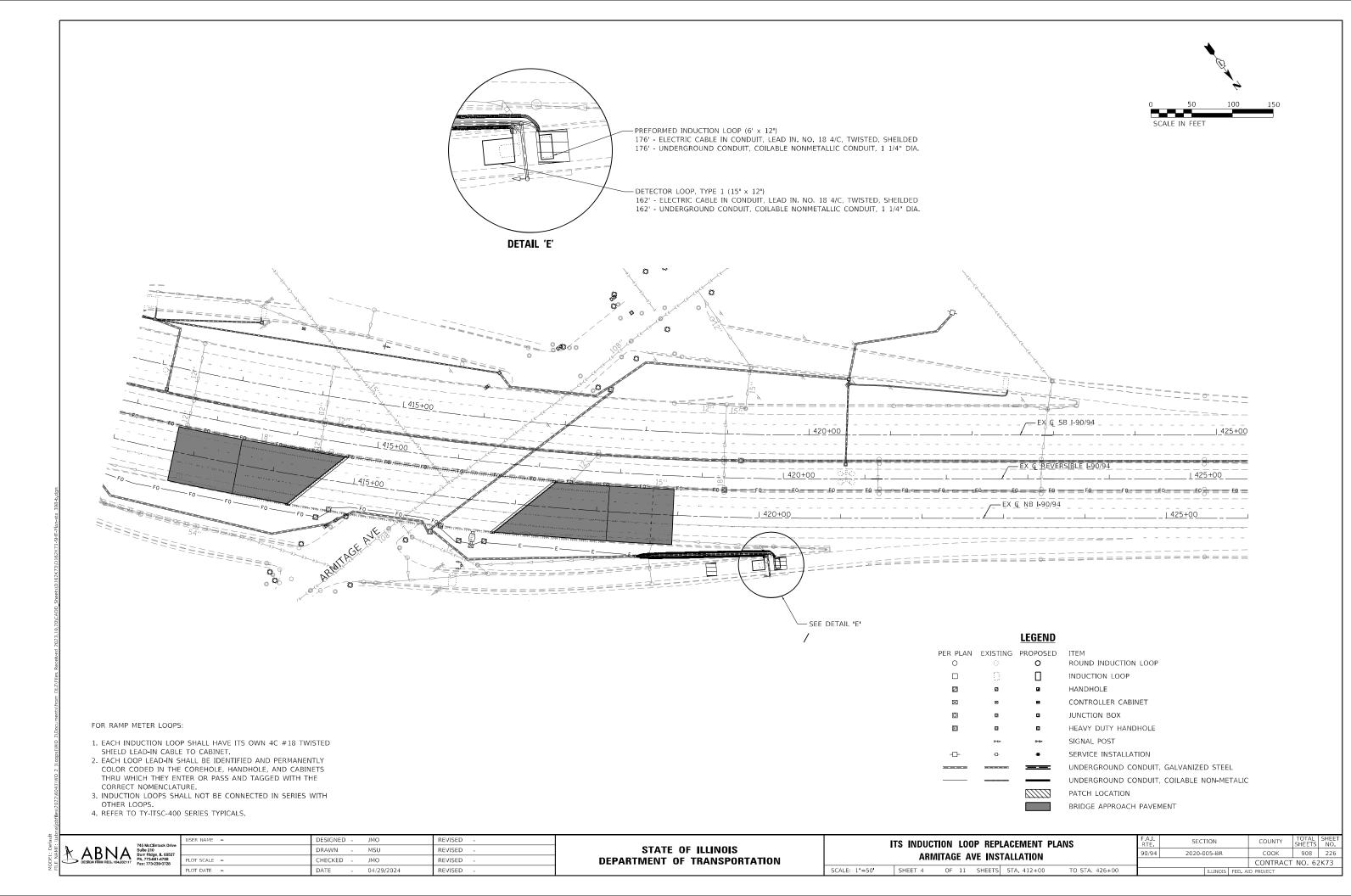
TO STA. 398+00

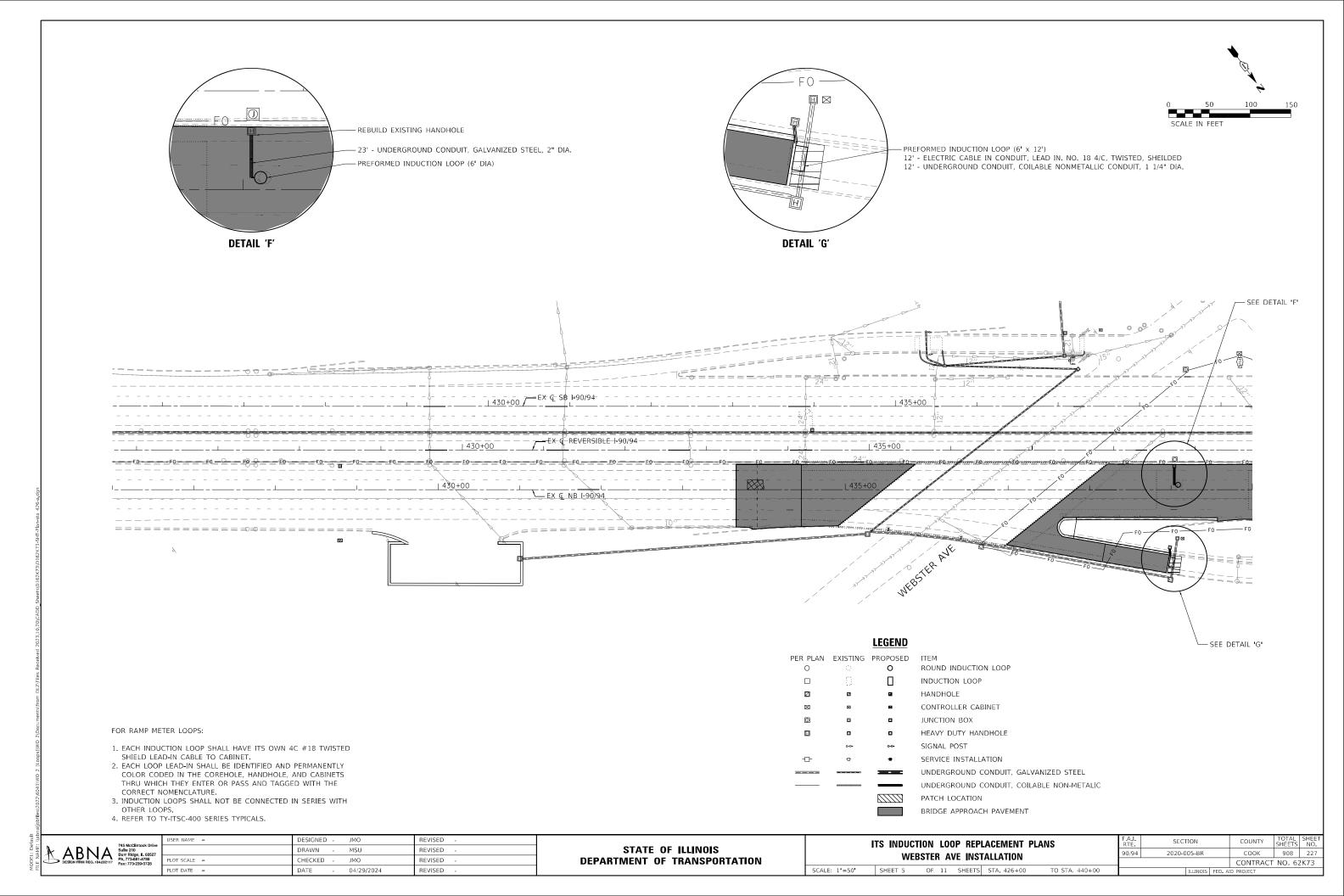
NORTH AVE INSTALLATION

SCALE: 1"=50" SHEET 3 OF 11 SHEETS STA 385+00

COOK 908 225

CONTRACT NO. 62K73





ABNA Sule 210 Sur Ridge, IL 60527 Ph, 773-681-4788 Fax: 773-239-3728

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

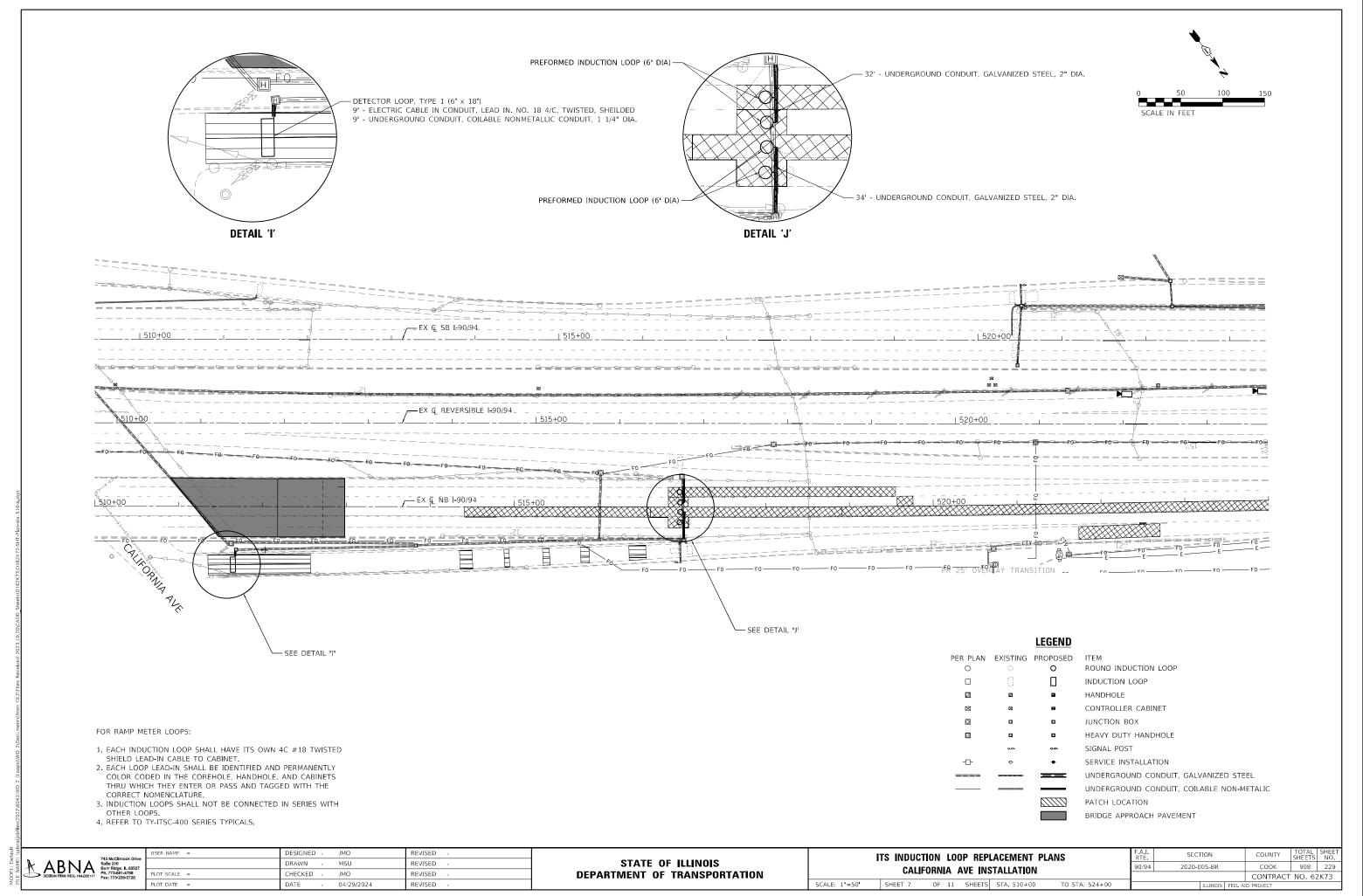
ITS INDUCTION LOOP REPLACEMENT PLANS WESTERN AVE/LOGAN BLVD INSTALLATION

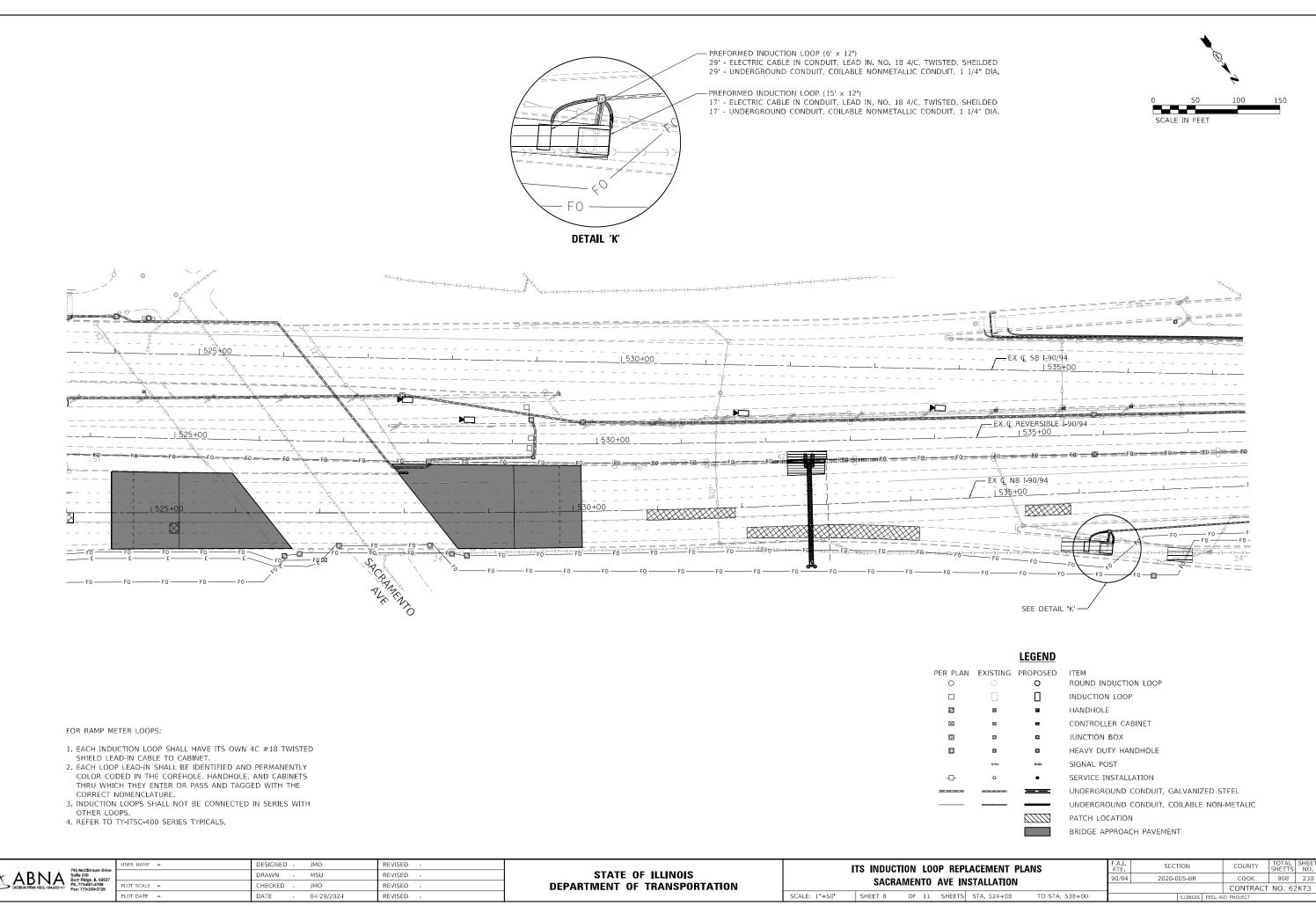
SCALE: 1"=50" SHEET 6 OF 11 SHEETS STA. 468+00 TO STA

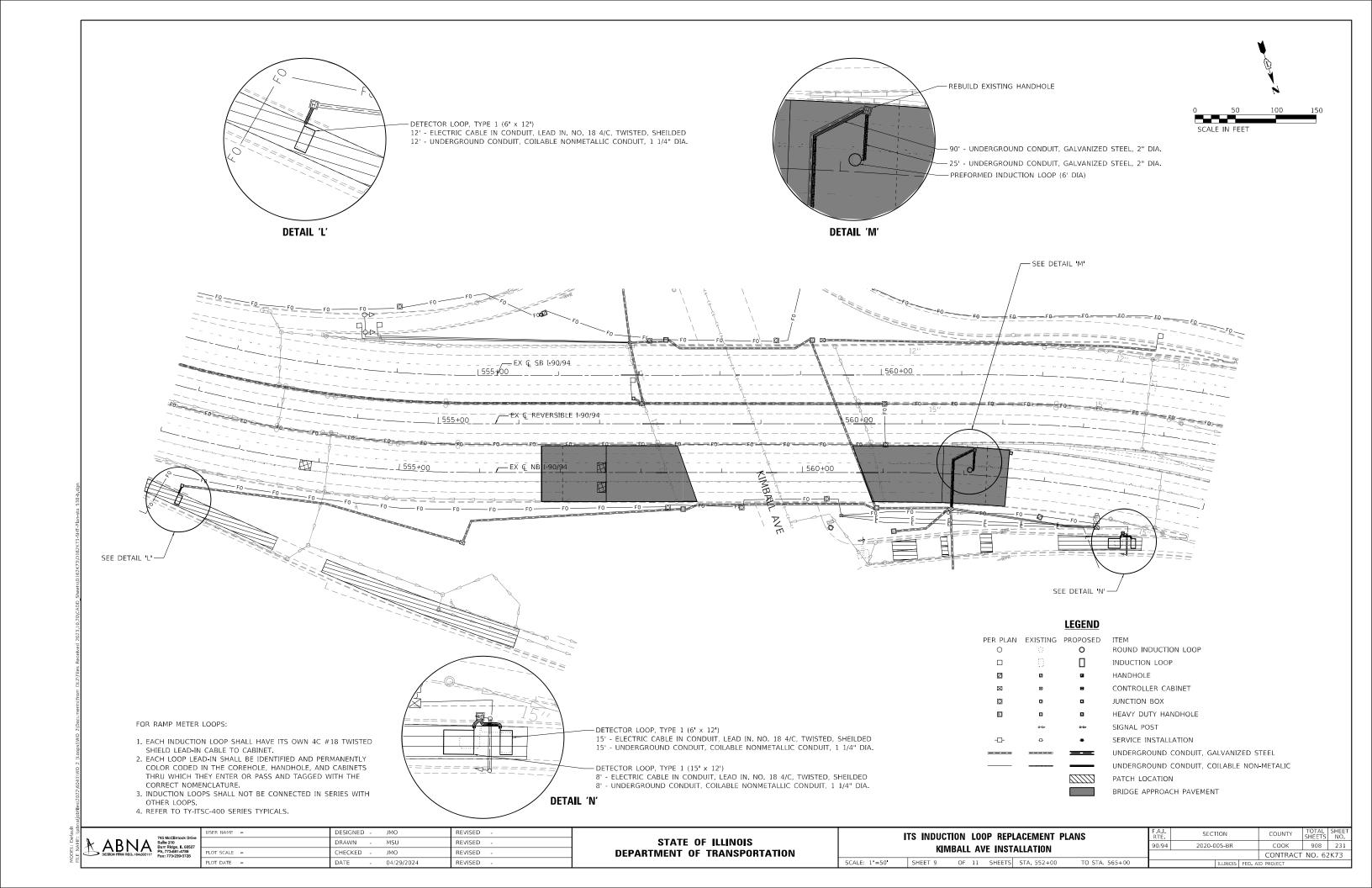
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

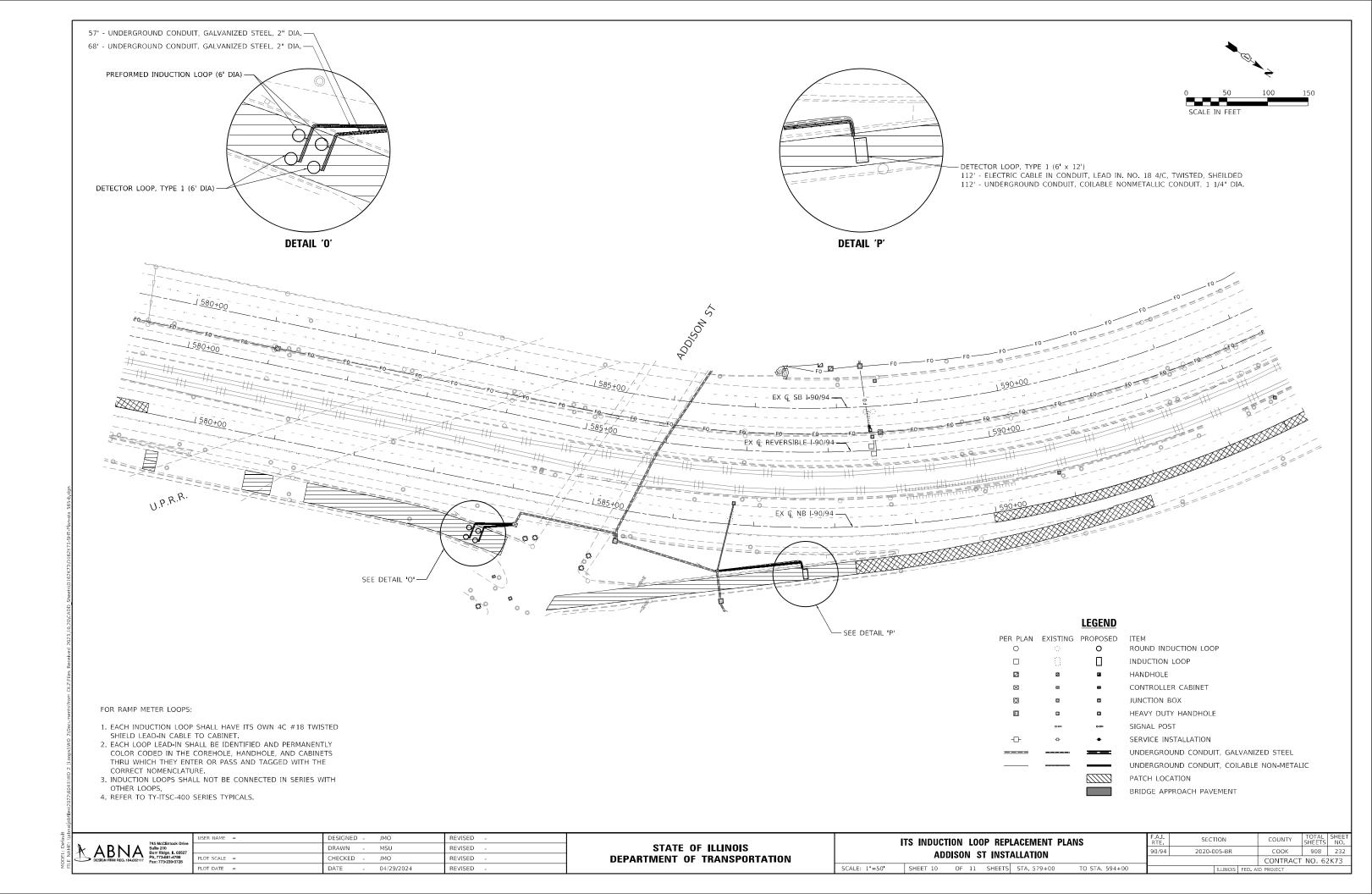
 90/94
 2020-005-BR
 COOK
 908
 228

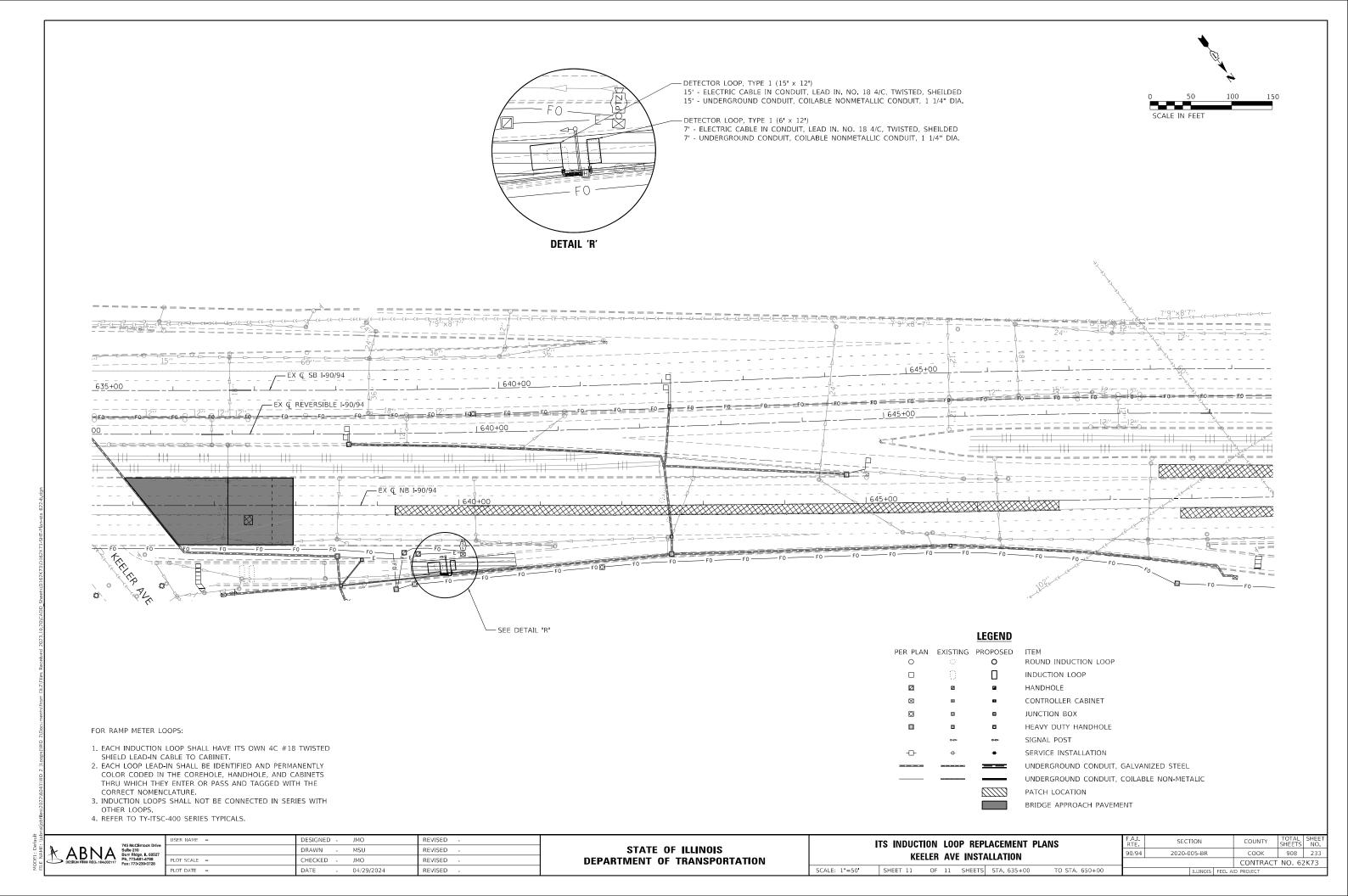
 CONTRACT NO. 62 K73

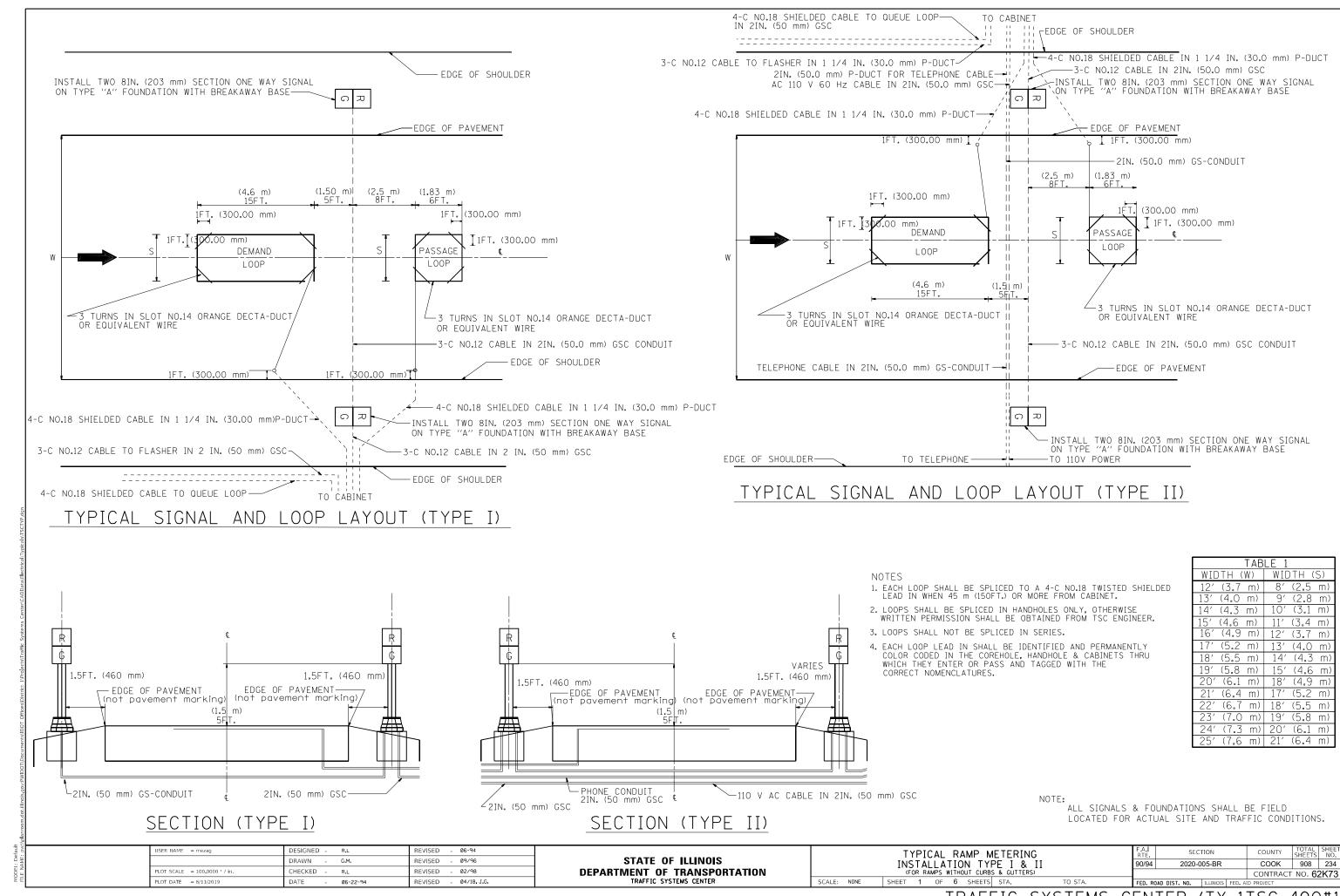


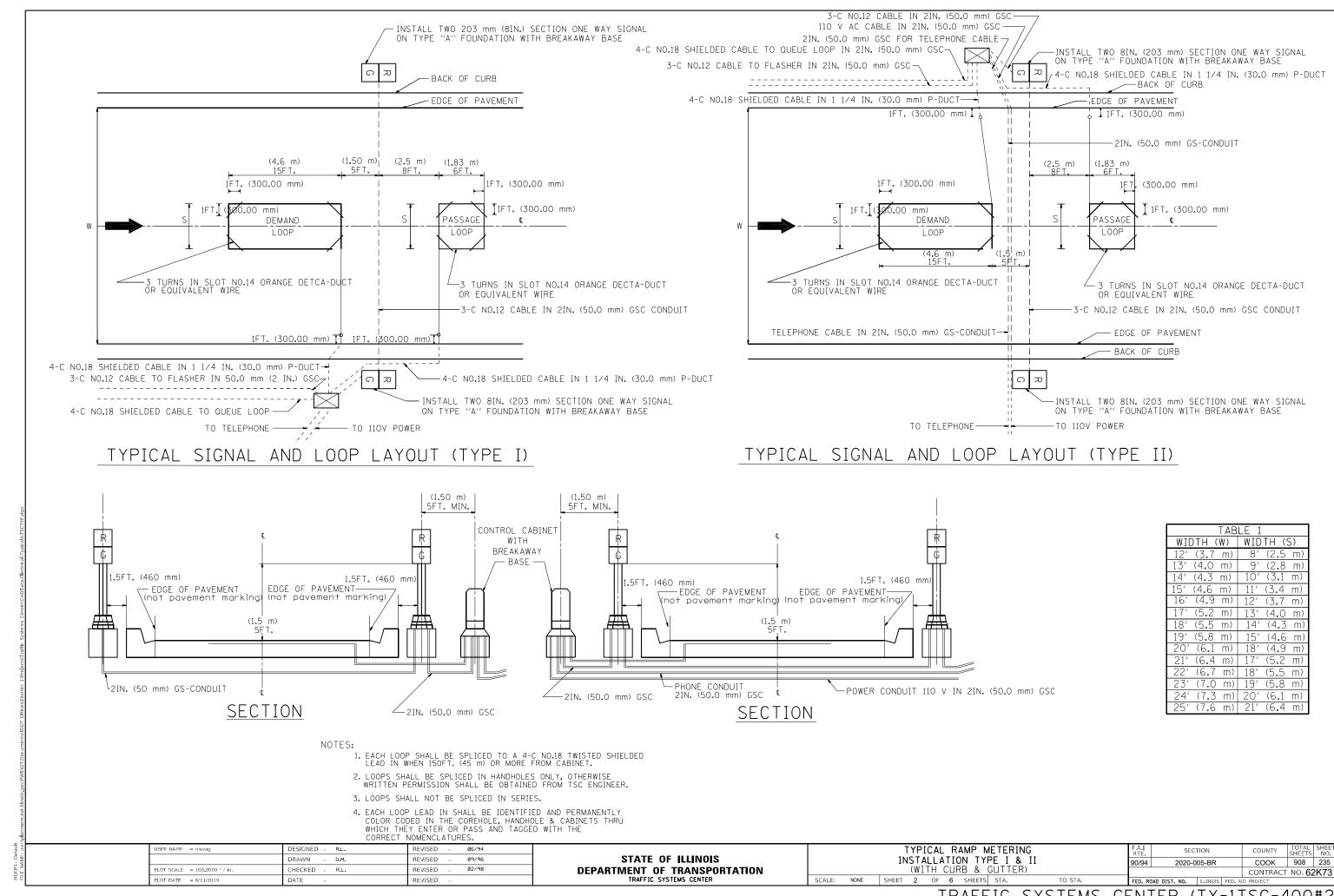


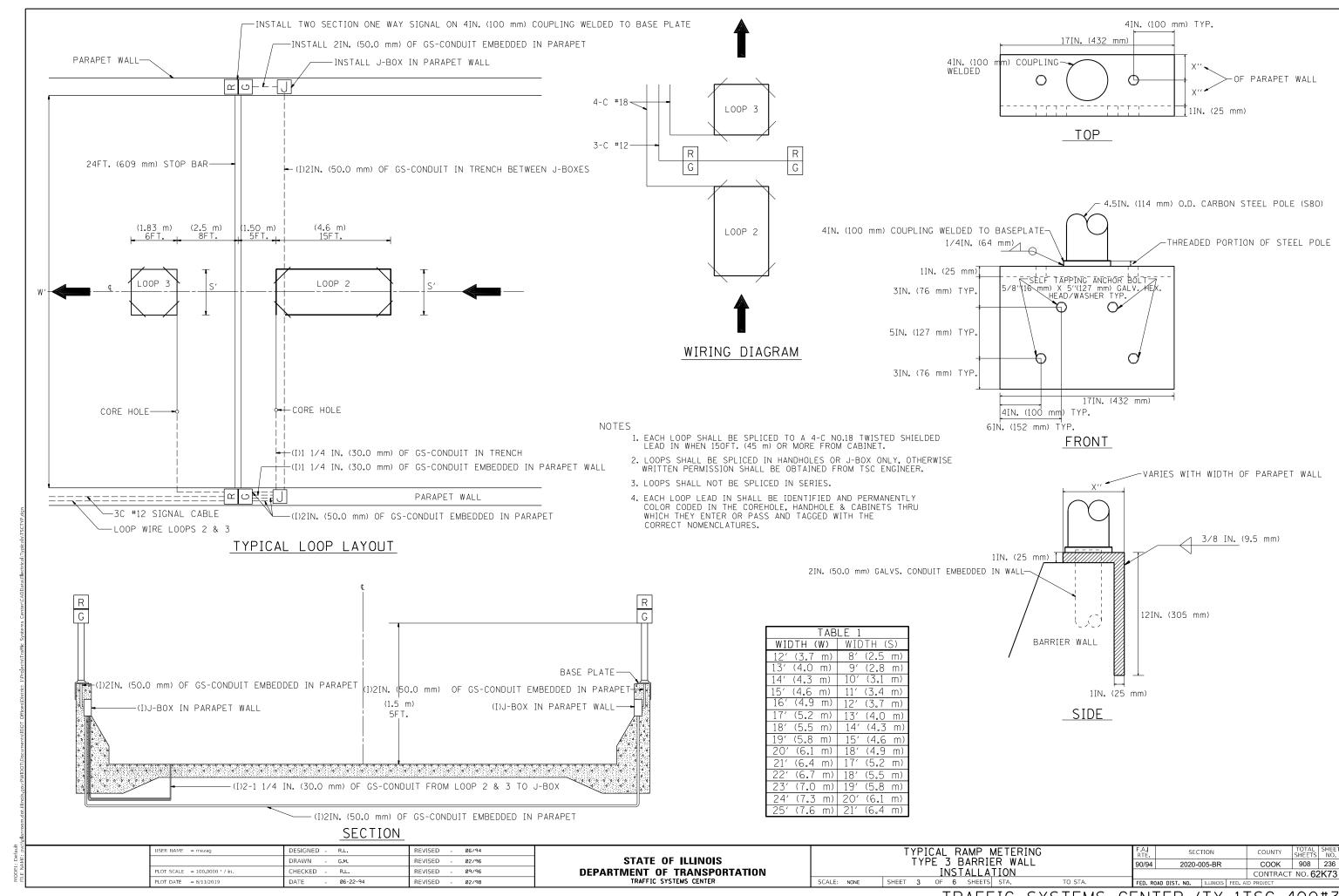


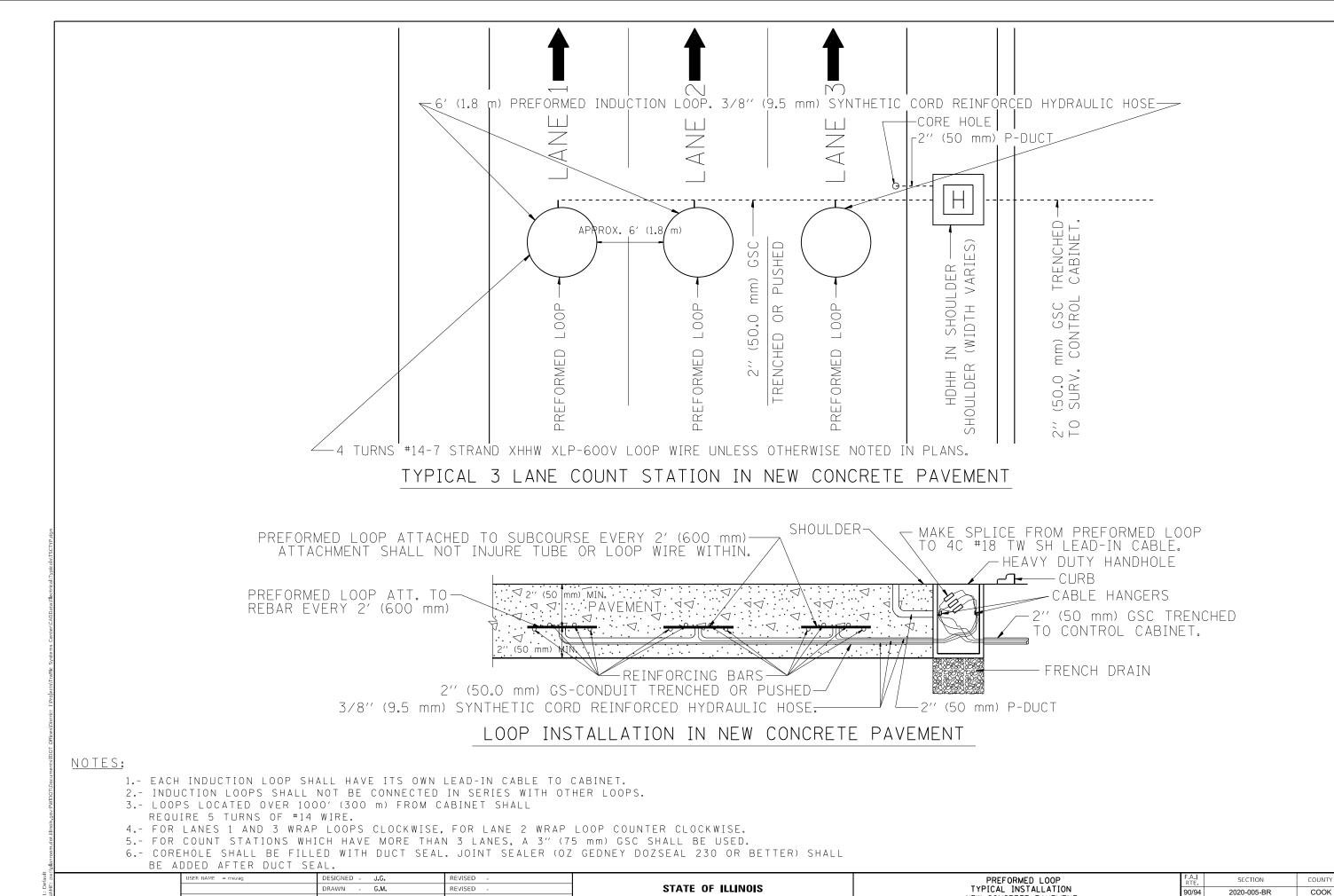












DEPARTMENT OF TRANSPORTATION

TRAFFIC SYSTEMS CENTER

PLOT SCALE = 100.0000 / in.

PLOT DATE = 1/24/2020

HECKED -

DATE

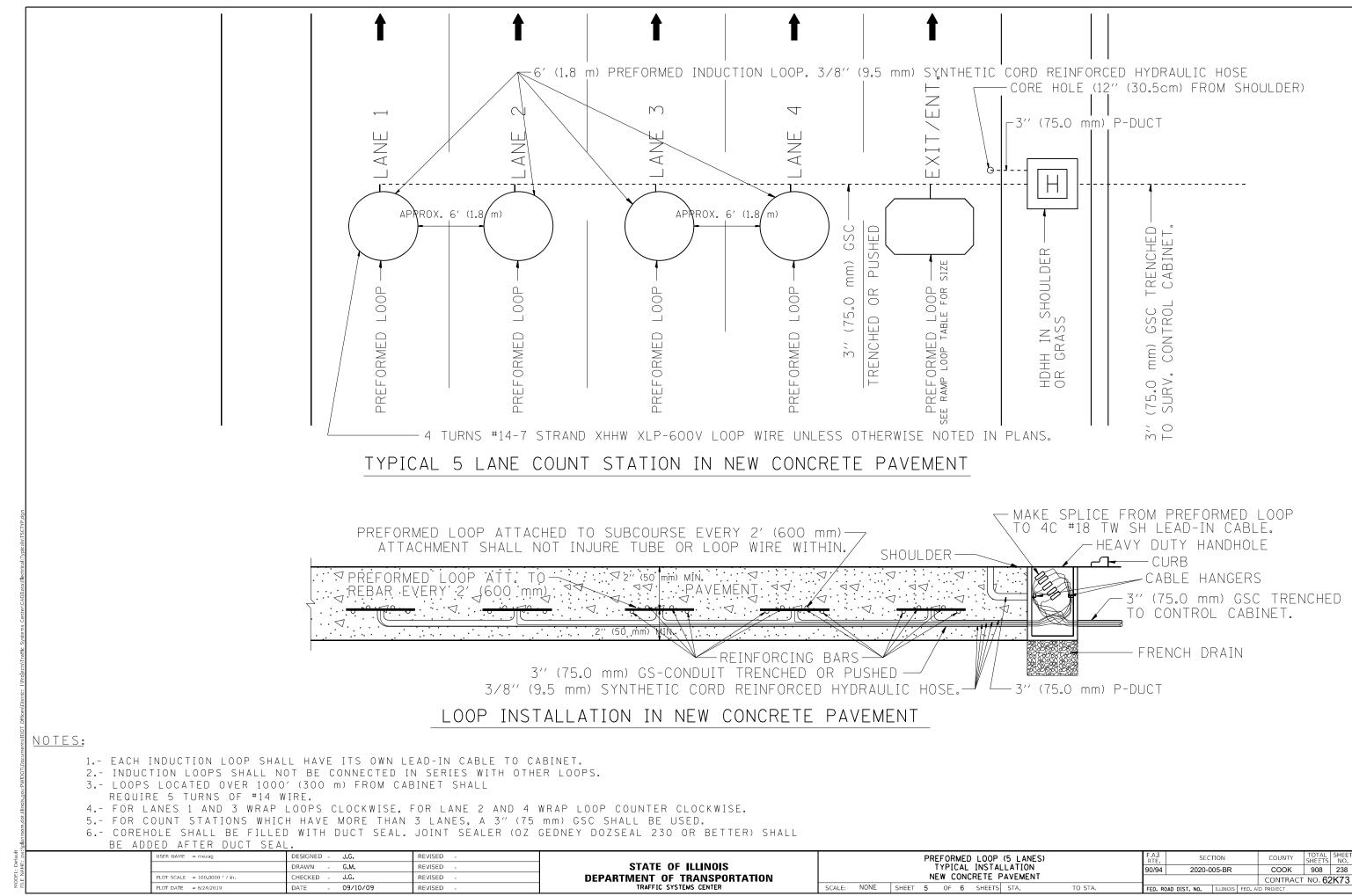
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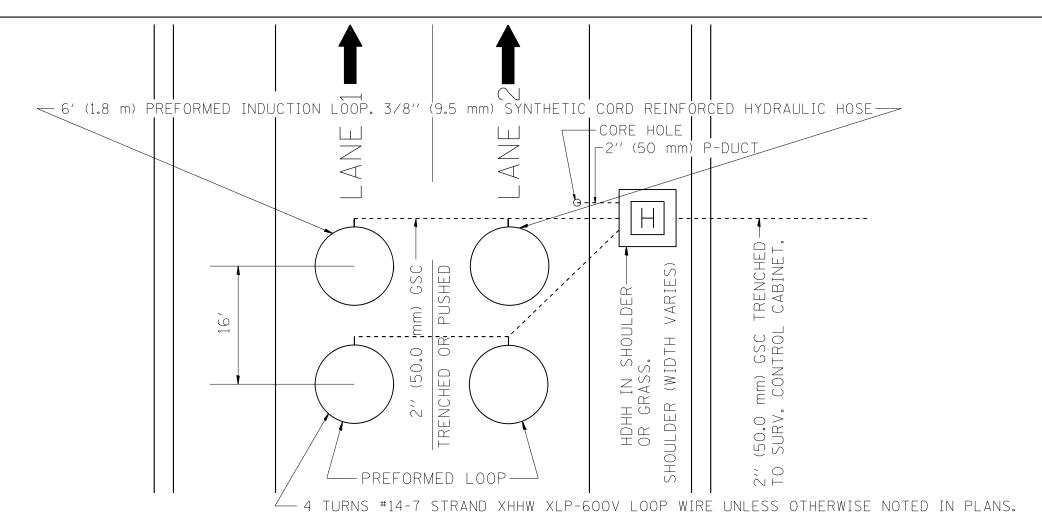
06/22/04

REVISED

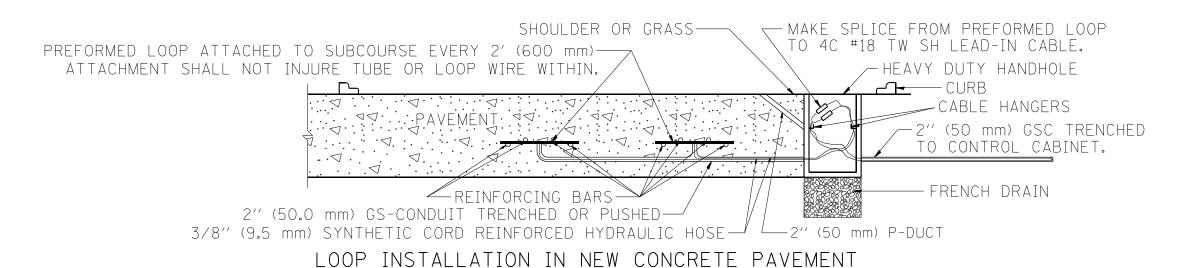
REVISED

2020-005-BR COOK 908 237 90/94 NEW CONCRETE PAVEMENT CONTRACT NO. 62K73 SCALE: NONE SHEET 4 OF 6 SHEETS STA.





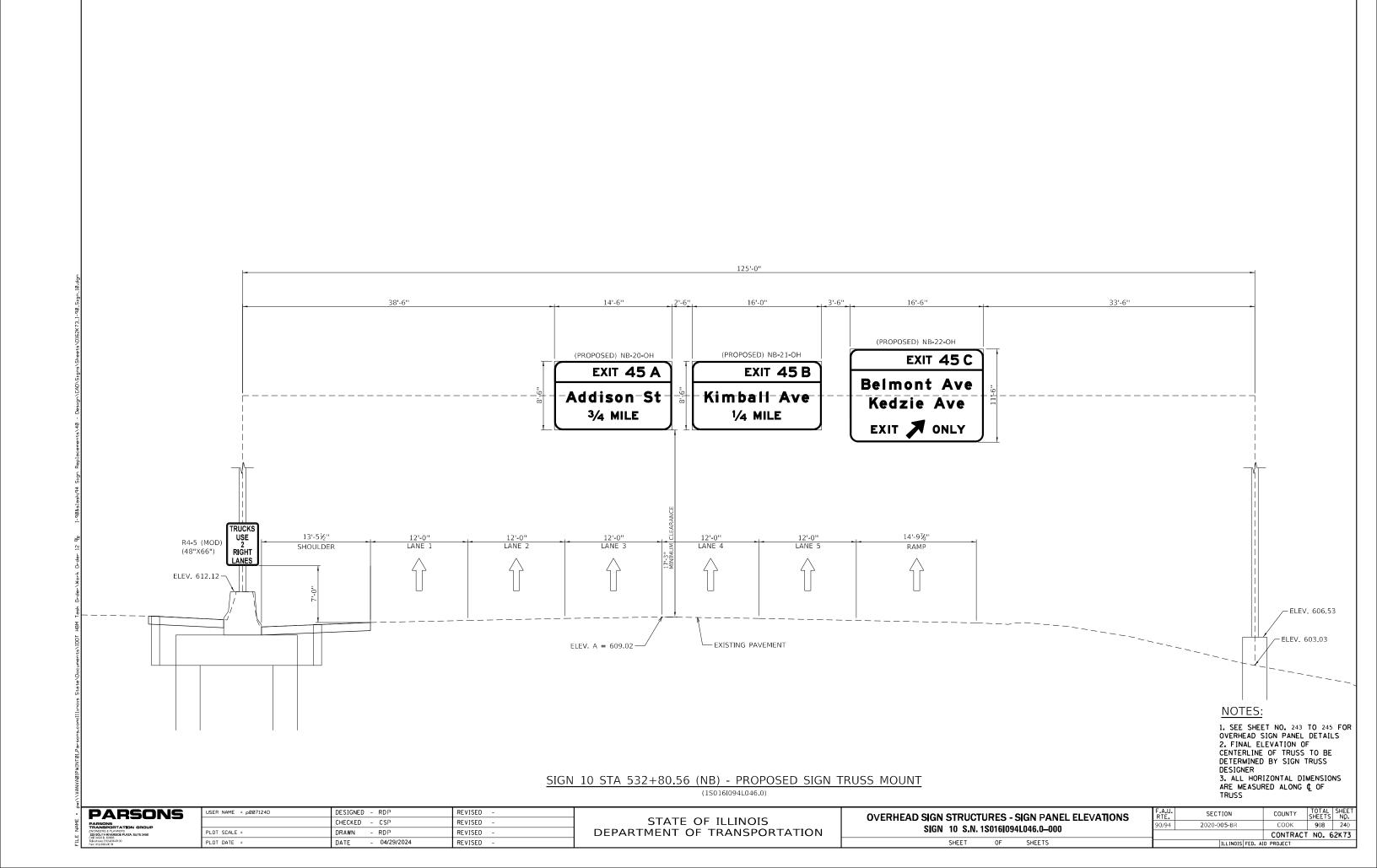
TYPICAL 2 LANE SPEED COUNT, CLASSIFICATION STATION IN NEW CONCRETE PAVEMENT

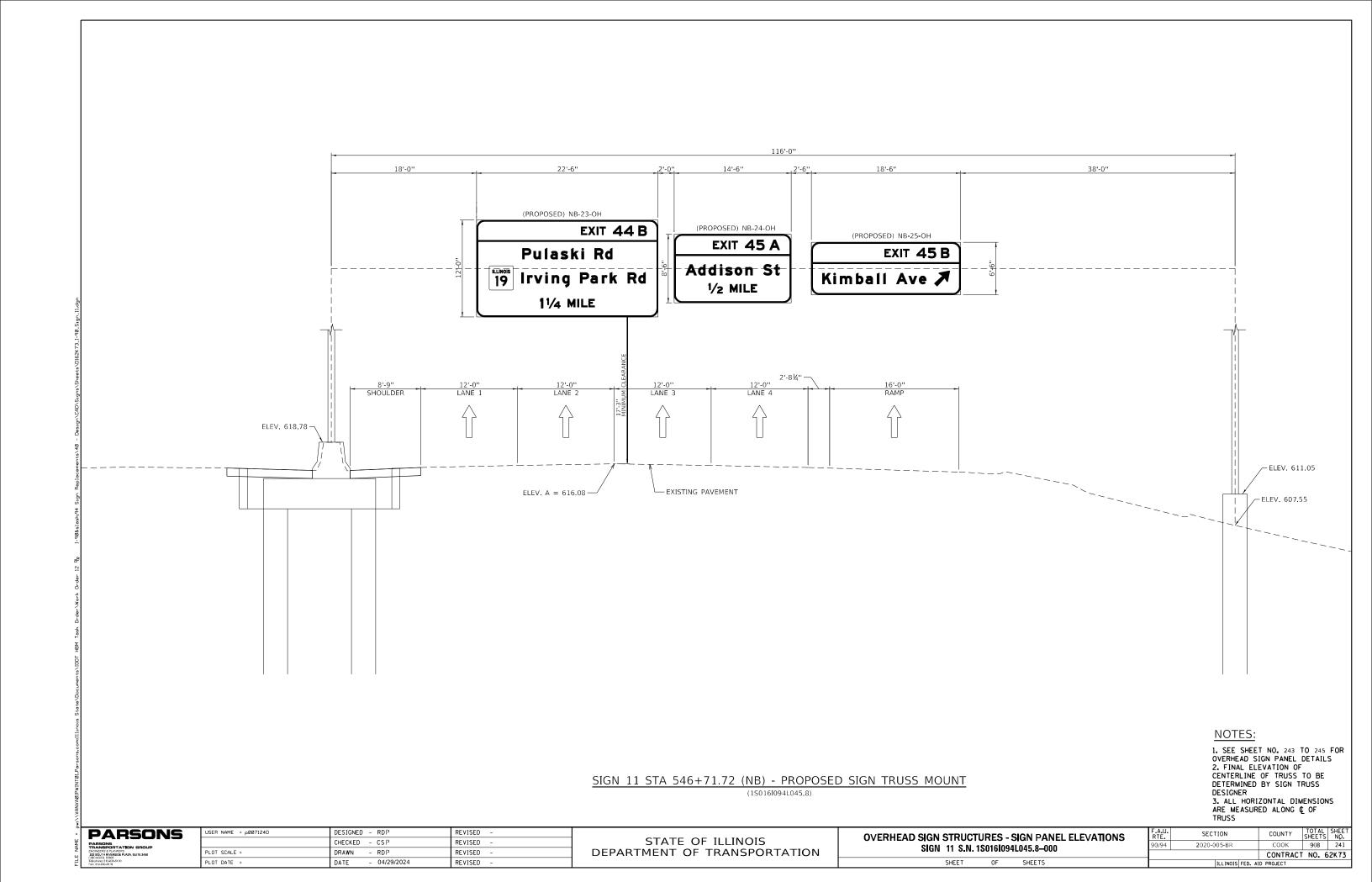


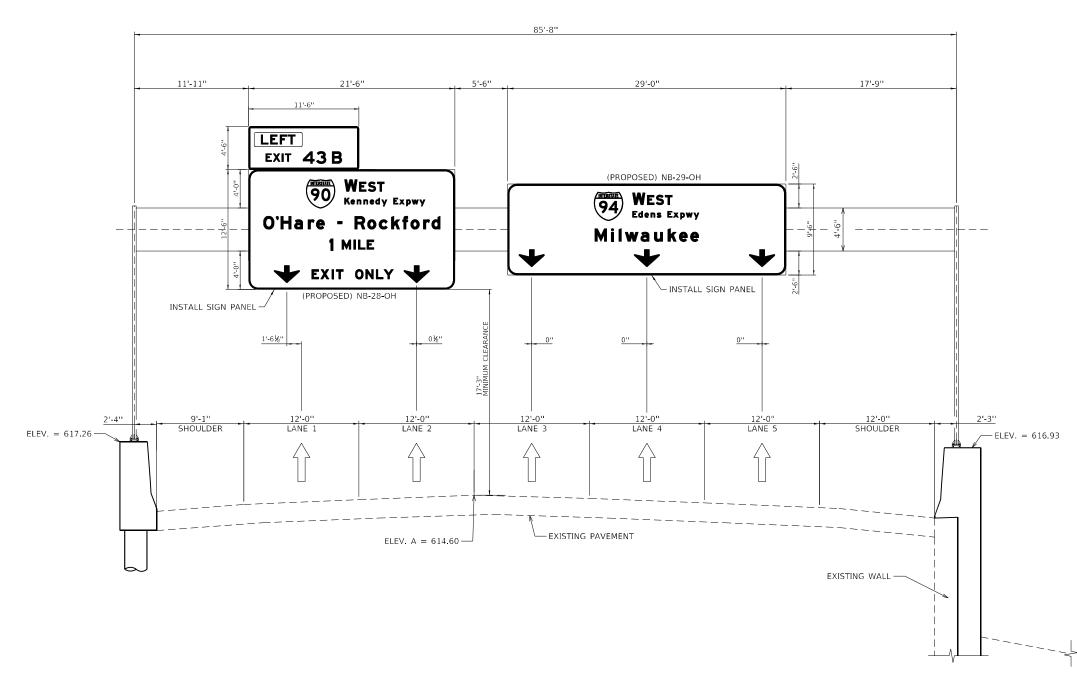
NOTES:

- 1.- EACH INDUCTION LOOP SHALL HAVE ITS OWN 4C #18 TWISTED SHIELD LEAD-IN CABLE TO CABINET.
- 2.- INDUCTION LOOPS SHALL NOT BE CONNECTED IN SERIES WITH OTHER LOOPS.
- 3.- LOOPS LOCATED OVER 1000' (300 m) FROM CABINET SHALL REQUIRE 5 TURNS OF #14 WIRE.
- 4.- FOR LANE 1 WRAP LOOPS CLOCKWISE, FOR LANE 2 WRAP LOOP COUNTER CLOCKWISE.
- 5.- COREHOLE SHALL BE FILLED WITH DUCT SEAL. JOINT SEALER (OZ GEDNEY DOZSEAL 230 OR BETTER) SHALL BE ADDED AFTER DUCT SEAL.

USER NAME = mezag	DESIGNED -	J.G.	REVISED - 07/01/2010		PREFORMED LOOP	F.A.I BTE	SECTION	COUNTY	TOTAL	SHEET
	DRAWN -	G.M.	REVISED -	STATE OF ILLINOIS	TYPICAL INSTALLATION	90/94	2020-005-BR	COOK	908	239
PLOT SCALE = 100.0000 / in	CHECKED -	J.G.	REVISED -	DEPARTMENT OF TRANSPORTATION	NEW CONCRETE PAVEMENT			CONTRACT		2K73
PLOT DATE = 6/25/2019	DATE -	06/25/2010	REVISED -	TRAFFIC SYSTEMS CENTER	SCALE: NONE SHEET 6 OF 6 SHEETS STA. TO STA.	FED. ROAD DIS	T. NO. ILLINOIS FED. AI	D PROJECT		







SIGN 13 STA 615+28.04 (NB) - PROPOSED SIGN TRUSS MOUNT (150161094L044.6)

NOTES:

1. SEE SHEET NO. 243 TO 245 FOR OVERHEAD SIGN PANEL DETAILS 2. ALL HORIZONTAL DIMENSIONS ARE MEASURED ALONG ¢ OF TRUSS

PARSONS

FARAISPORTATION GROUP

CHICAGOS FLANACIS

GILLOGOS FLANACIS

USER NAME = p007124D	DESIGNED - RDP	REVISED -
	CHECKED - CSP	REVISED -
PLOT SCALE =	DRAWN - RDP	REVISED -
PLOT DATE =	DATE - 04/29/2024	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION
 OVERHEAD SIGN STRUCTURES - SIGN PANEL ELEVATIONS

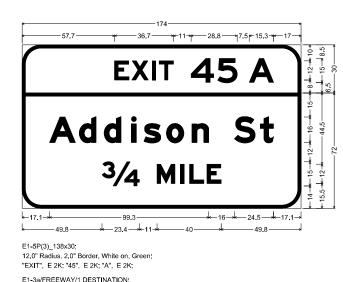
 SIGN
 13
 S.N. 1S016I094L044.6—000

SHEET OF SHEETS

 F,A,L. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL NO.
 SHEETS NO.

 90/94
 2020-005-BR
 COOK
 908
 242

 CONTRACT
 NO.
 62K73



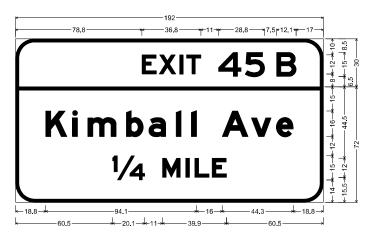
34 M I L E 49.8 84.2 98.7 104.5 115.3

1S016I094L046.0-000
14'-6" × 8'-6"
2"
12"
OVERHEAD
TYPE: REFLECTIVE - ZZ
COLOR: GREEN
TYPE: REFLECTIVE - ZZ
COLOR: WHITE

	,	
- ZZ		
- ZZ		
	J	

SYMBOL	ROT	Х	Υ	WID	HT
AR_TYPE A	315	-	-	22.3	35.6

NOTE: ALL ARROWS (DOWN OR 45°) USED ON OVERHEAD SIGNS SHALL BE DEMOUNTABLE AND INCLUDED IN THE COST OF THE SIGN PANEL.

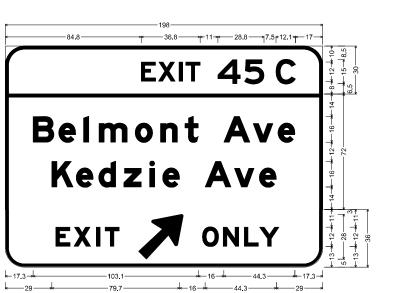


E1-5P(3)_138x30; 12.0" Radius, 2.0" Border, White on, Green; "EXIT", E 2K; "45", E 2K; "B", E 2K;

E1-3a/FREEWAY/1 DESTINATION; 12.0" Radius, 2.0" Border, White on, Green; "Kimball Ave", E Mod 2K; "%", E Mod 2K; "MILE", E Mod 2K; Table of letter and object lefts

			•								
E 70 0	X	I 102.3	T	4	66	5	122	B			
70.0	09.5	102.3	100.0	0 12	0.0	1-	· J. Z	10	12.5		
K 18.8	i 35.4	m 45.0	b 69.0	a 83.1	I 100	0.1	I 109	.7	A 128.9	v 147.1	e 162.6
¼ 60.5	M 91.6	I 106.1	L 111.8	E 3 12:	2.6						

STRUCTURE NUMBER	1S016I094L046.0-000
WIDTH x HEIGHT	16'-0" x 8'-6"
BORDER WIDTH	2"
CORNER RADIUS	12"
MOUNTING	OVERHEAD
BACKGROUND	TYPE: REFLECTIVE - ZZ
	COLOR: GREEN
LEGEND/BORDER	TYPE: REFLECTIVE - ZZ
	COLOR: WHITE



= 31.5 = 39.5 = +12 = 28 = +12 = 47.7 = 27.3 = 11.5 = 12.0" Radius, 2.0" Border, White on, Green; "EXIT", E 2K; "45", E 2K; "C", E 2K;

E1-3a/FREEWAY/1 DESTINATION; 12.0" Radius, 2.0" Border, White on, Green; "Belmont Ave", E Mod 2K; "Kedzie Ave", E Mod 2K;

84.8	95.5	108.3	112	.6 13	2.6 14	49.2 1	68.9		
В	е	ı	m	0	n	t	Α	v 154.6	е
17.3	33.3	48.8	58.4	81.0	96.8	112.0	136.4	154.6	170.
K 29.0	e 44.2	d 58.3	z 74.0	i 90.0	e 98.1	A 124.7	v 142.9	e 158.4	
								Y 158.6	

1S016I094L046.0-000
16'-6" × 11'-6"
2"
12"
OVERHEAD
TYPE: REFLECTIVE - ZZ
COLOR: GREEN
TYPE: REFLECTIVE - ZZ
COLOR: WHITE

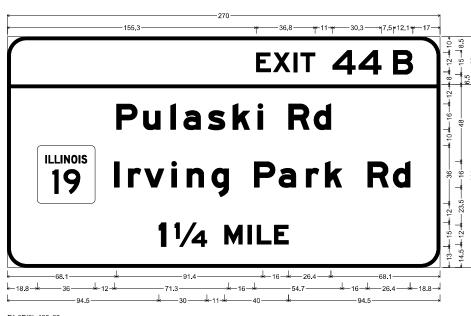


F	PARSONS
TR	RSONS ANSPORTAT I ON GROUP
	INEERS & PLANWERS
222 S	OUTH RIVERSIDE PLAZA, SUITE 2450 AGO IL 60806
	shoric 312-630-5100

USER NAME = 35361 DESIGNED - RDP	REVISED -
CHECKED - CSP	REVISED -
PLOT SCALE = DRAWN - RDP	REVISED -
PLOT DATE = DATE - 04/29/2	024 REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

U.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
4	2020-005-BR	соок	908	243
		CONTRACT	NO.62	2K73
	ILLINOIS FED. AI	D PROJECT		



12.0" Radius, 2.0" Border, White on, Green, "EXIT", E 2K, "44", E 2K, "B", E 2K,

E1-3a/FREEWAY/1 DESTINATION;

12.0" Radius, 2.0" Border, White on, Green;

1 ¼ M I L E 94.5 104.4 135.5 150.0 155.8 166.6

"Pulaski Rd", E Mod 2K; "Irving Park Rd", E Mod 2K; "11/4", E Mod 2K; "MILE", E Mod 2K;

Table of letter and object lefts

E X I T 4 4 B B 155.3 166.0 178.9 183.1 203.1 219.3 240.9 P u I a s k i R d 68.1 84.9 101.9 110.0 125.2 140.8 156.3 175.5 191.3 | No. | No.

_		
	STRUCTURE NUMBER	1S016I094L045.8-000
	WIDTH x HEIGHT	22'-6" x 12'-0"
	BORDER WIDTH	2"
	CORNER RADIUS	12"
	MOUNTING	OVERHEAD
	BACKGROUND	TYPE: REFLECTIVE - ZZ
		COLOR: GREEN
	LEGEND/BORDER	TYPE: REFLECTIVE - ZZ
		COLOR: WHITE



E 1-38/FREEWAY/ I DESTINATION;													
12.0" Radius, 2.0" Border, White on, Green,													
"Addison St", E Mod 2K, "1/2", E Mod 2K, "MILE", E Mod													
Table	of let	ter and	d obje	ct	lefts	3							
E	Х	ı	Т	4	ļ.		5		Α				
57.7	68.4	81.2	85.4	1	05.	4	122	2.1	141	.7			
Α	d	d	i	s		0		n		S		t 148.6	1
17.1	35.6	51.2	68.1	76	0.6	9	0.0	10	5.9	13	32.4	148.6	
1/2	М	I 97.5	L	Т	Е]						
51.1	83.0	97.5	103.	3	114	.1							

E1-3a/FREEWAY/1 DESTINATION;

1S016I094L045.8-000
14'-6" x 8'-6"
2"
12"
OVERHEAD
TYPE: REFLECTIVE - ZZ
COLOR: GREEN
TYPE: REFLECTIVE - ZZ
COLOR: WHITE



E1-5P(3)_138x30; 12.0" Radius, 2.0" Border, White on, Green, "EXIT", E 2K, "45", E 2K, "B", E 2K;

E1-3a/FREEWAY/1 DESTINATION; 12.0" Radius, 2.0" Border, White on, Green, "Kimball Ave", E Mod 2K; Arrow A-13.33UC - 29.3" 45';

Table of letter and object lefts

E X I T 4 5 B 108.8 119.5 132.4 136.6 156.6 173.2 192.9 K i m b a I I A v e ## 7 16.3 32.9 42.5 66.5 80.6 97.6 107.2 126.4 144.6 160.1 182.7

STRUCTURE NUMBER	1S016I094L045.8-000
WIDTH x HEIGHT	18'-6" x 6'-6"
BORDER WIDTH	2"
CORNER RADIUS	12"
MOUNTING	OVERHEAD
BACKGROUND	TYPE: REFLECTIVE - ZZ
	COLOR: GREEN
LEGEND/BORDER	TYPE: REFLECTIVE - ZZ

COLOR: WHITE

SYMBOL	ROT	Х	Υ	WID	HT
AR_TYPE A	315	-	-	22.3	35.6

NOTE: ALL ARROWS (DOWN OR 45°) USED ON OVERHEAD SIGNS SHALL BE DEMOUNTABLE AND INCLUDED IN THE COST OF THE SIGN PANEL.



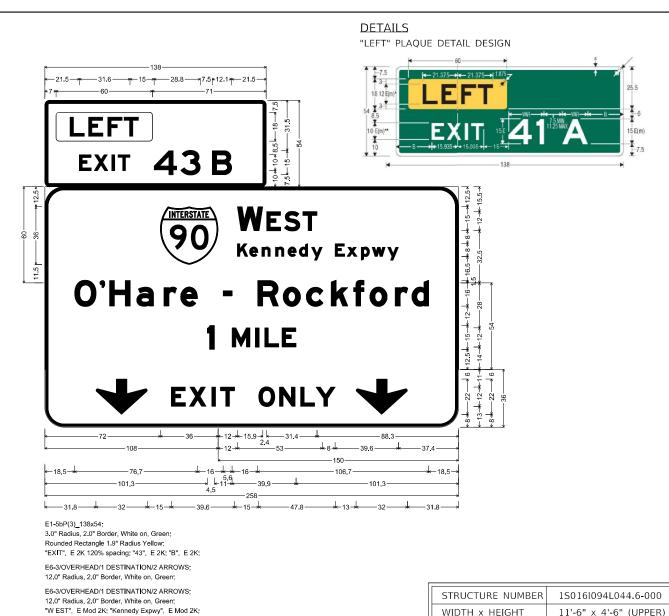
PARSONS

USER NAME = 35361	DESIGNED - RDP	REVISED -	
	CHECKED - CSP	REVISED -	
PLOT SCALE =	DRAWN - RDP	REVISED -	
PLOT DATE =	DATE - 04/29/2024	REVISED -	

STATE OF ILLINOIS	
DEPARTMENT OF TRANSPORTATION	

OVERHEAD SIGN STRUCTURES - SIGN PANEL DETAIL	
SIGN 11 S.N. 1S016I094L045.8-000	

.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE
0/94	2020-005-BR	COOK	908	24
		CONTRACT	NO.62	K73
	ILLINOIS FED. AI	ID PROJECT		



1S016I094L044.6-000
11'-6" x 4'-6" (UPPER)
2"
3"
OVERHEAD
TYPE: REFLECTIVE - ZZ
COLOR: GREEN
TYPE: REFLECTIVE - ZZ
COLOR: WHITE

STRUCTURE NUMBER	1S016I094L044.6-000
WIDTH x HEIGHT	21'-6" x 12'-6" (LOWER)
BORDER WIDTH	2"
CORNER RADIUS	12"
MOUNTING	OVERHEAD
BACKGROUND	TYPE: REFLECTIVE - ZZ
	COLOR: GREEN
LEGEND/BORDER	TYPE: REFLECTIVE - ZZ
	COLOR: WHITE



E6-3/OVERHEAD/1 DESTINATION/2 ARROWS; 12.0" Radius, 2.0" Border, White on, Green;

E6-3/OVERHEAD/1 DESTINATION/2 ARROWS; 12.0" Radius, 2.0" Border, White on, Green;

"W EST", E Mod 2K; "Edens Expwy", E Mod 2K;

E6-3/OVERHEAD/1 DESTINATION/2 ARROWS; 12.0" Radius, 2.0" Border, White on, Green;

"Milwaukee", E Mod 2K; Down Arrow 22.0" 270'; Down Arrow 22.0" 270'; Down Arrow 22.0" 270'; Table of letter and object lefts

STRUCTURE NUMBER	1S016I094L044.6-000
WIDTH x HEIGHT	29'-0" × 9'-6"
BORDER WIDTH	2"
CORNER RADIUS	12"
MOUNTING	OVERHEAD
BACKGROUND	TYPE: REFLECTIVE - ZZ
	COLOR: GREEN
LEGEND/BORDER	TYPE: REFLECTIVE - ZZ
	COLOR: WHITE

SYMBOL	ROT	Х	Υ	WID	HT
AR_DOWN	0	-	-	32	22

NOTE: ALL ARROWS (DOWN OR 45°) USED ON OVERHEAD SIGNS SHALL BE DEMOUNTABLE AND INCLUDED IN THE COST OF THE SIGN PANEL.



PARSONS
PARSONS
TRANSPORTATION GROUP
ETIGINEER'S FLANNERS
22 SOUTH REPRESIDER PLAZA SUITE 2450

USER NAME = 35361	DESIGNED - RDP	REVISED -
	CHECKED - CSP	REVISED -
PLOT SCALE =	DRAWN - RDP	REVISED -
PLOT DATE =	DATE - 04/29/2024	REVISED -

E6-3/OVERHEAD/1 DESTINATION/2 ARROWS;

Table of letter and object lefts

W E S T 12.0 30.3 41.3 52.9

E X I T 4 3 B 21.5 30.6 41.7 45.6 68.1 84.7 104.4

1 M I L E 101.3 116.8 131.3 137.0 147.8

12.0" Radius, 1.5" Border, 0.5" Indent, Black on, Yellow;

12.0" Radius, 2.0" Border, White on, Green;
"O'Hare - Rockford", E Mod 2K; "1", E Mod 2K; "MILE", E Mod 2K;

K e n n e d y E x p w y 12.0 19.6 27.4 35.8 43.6 50.6 58.2 73.0 80.4 89.3 96.2 105.8

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 31.8
 78.8
 90.1
 104.1
 109.5
 133.4
 146.3
 159.4
 169.0
 194.2

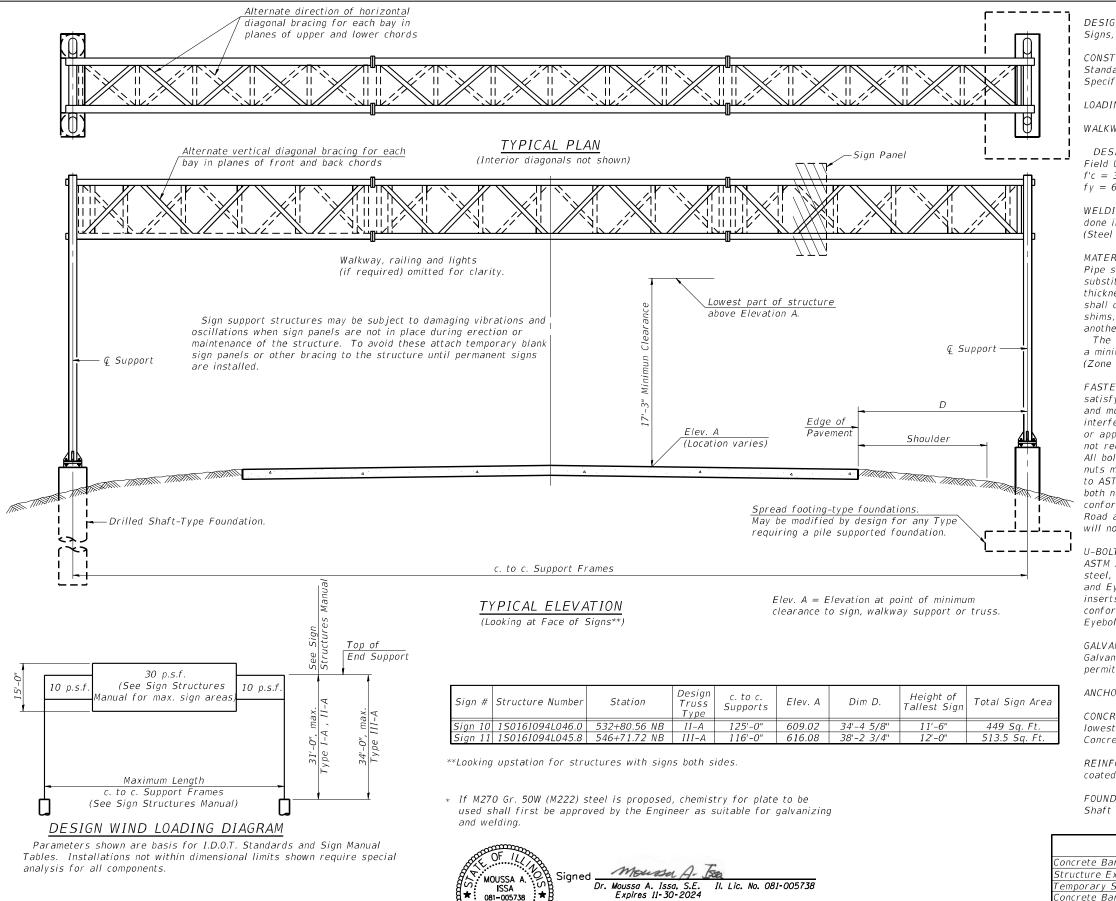
O | H | a | r | e | - R | o | c | k | f | o | r | d | d | 111.2 | 132.8 | 148.6 | 163.0 | 178.5 | 192.4 | 202.7 | 218.5 | 228.9 |

Down Arrow 22.0" 270'; "EXIT", E Mod 2K specified length; "ONLY", E Mod 2K specified length; Down Arrow 22.0" 270';

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN	I STRUCTUI 13 S.N. 180			DETAIL
	SHEET	0F	SHEETS	

.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
0/94	2020-005-BR	COOK	908	245
		CONTRACT	NO.62	2K73
	ILLINOIS FED. AI	ID PROJECT		



081-005738 CHICAGO, ILLINOIS GENERAL NOTES

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES: Field Units

f'c = 3,500 p.s.i.

fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer.

The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to ASTM F1554 Gr. 105.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Concrete Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

FOUNDATIONS: The contract unit price for Concrete Foundations and Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.

TOTAL BILL OF MATERIAL

TOTAL BILL OF THAT EXTAL							
ITEM	UNIT	TOTAL					
Concrete Barrier Removal	Foot	88.0					
Structure Excavation	Cu Yd	93.5					
Temporary Soil Retention System	Sq Ft	784					
Concrete Barrier Transition	Foot	66					
Overhead Sign Structure – Span, Type II–A (4'–6" X 5'–3")	Foot	125					
Overhead Sign Structure – Span, Type III-A (5'-0" X 7'-0")	Foot	116					
Drilled Shaft Concrete Foundations	Cu Yd	108.5					
Remove Overhead Sign Structure - Span	Each	2					
Lead							

05-A-1

HBM ENGINEERING GROUP LICE

 2-17-2017

 USER NAME =
 DESIGNED - KJD REVISED - CHECKED - MI REVISED - PLOT SCALE = DRAWN - KJD REVISED - PLOT DATE = DATE - 04/29/2024 REVISED - PLOT DATE = DATE - 04/29/2024 REVISED - PLOT DATE - DA

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FOR SHEETS OSGI-01 THRU OSGI-24

(TOTAL OF 24 SHEETS)

04/29/2024

OVERHEAD SIGN STRUCTURES - GENERAL PLAN & ELEVATION - ALUMINUM TRUSS & STEEL SUPPORTS

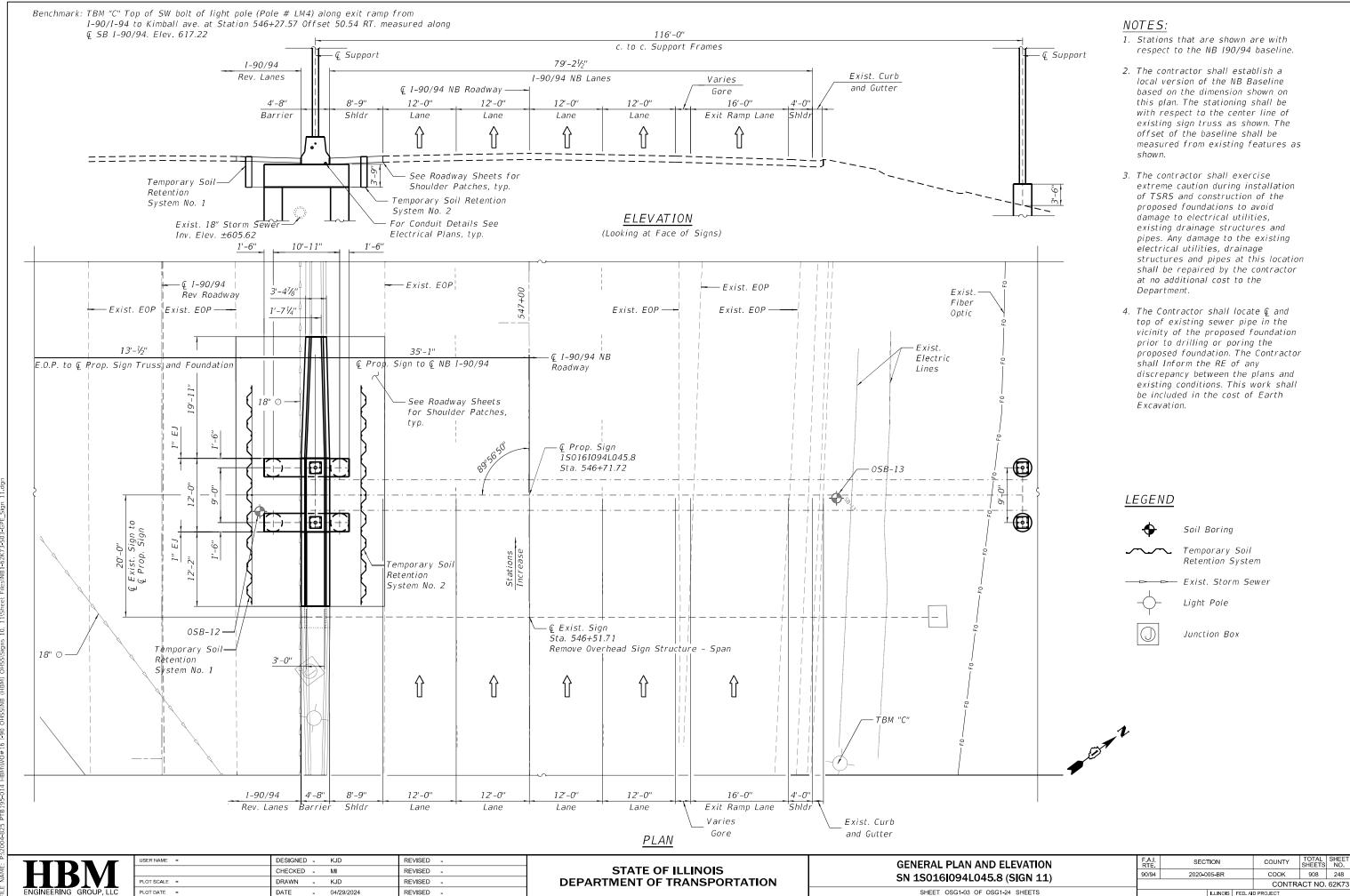
SHEET OSG1-01 OF OSG1-24 SHEETS

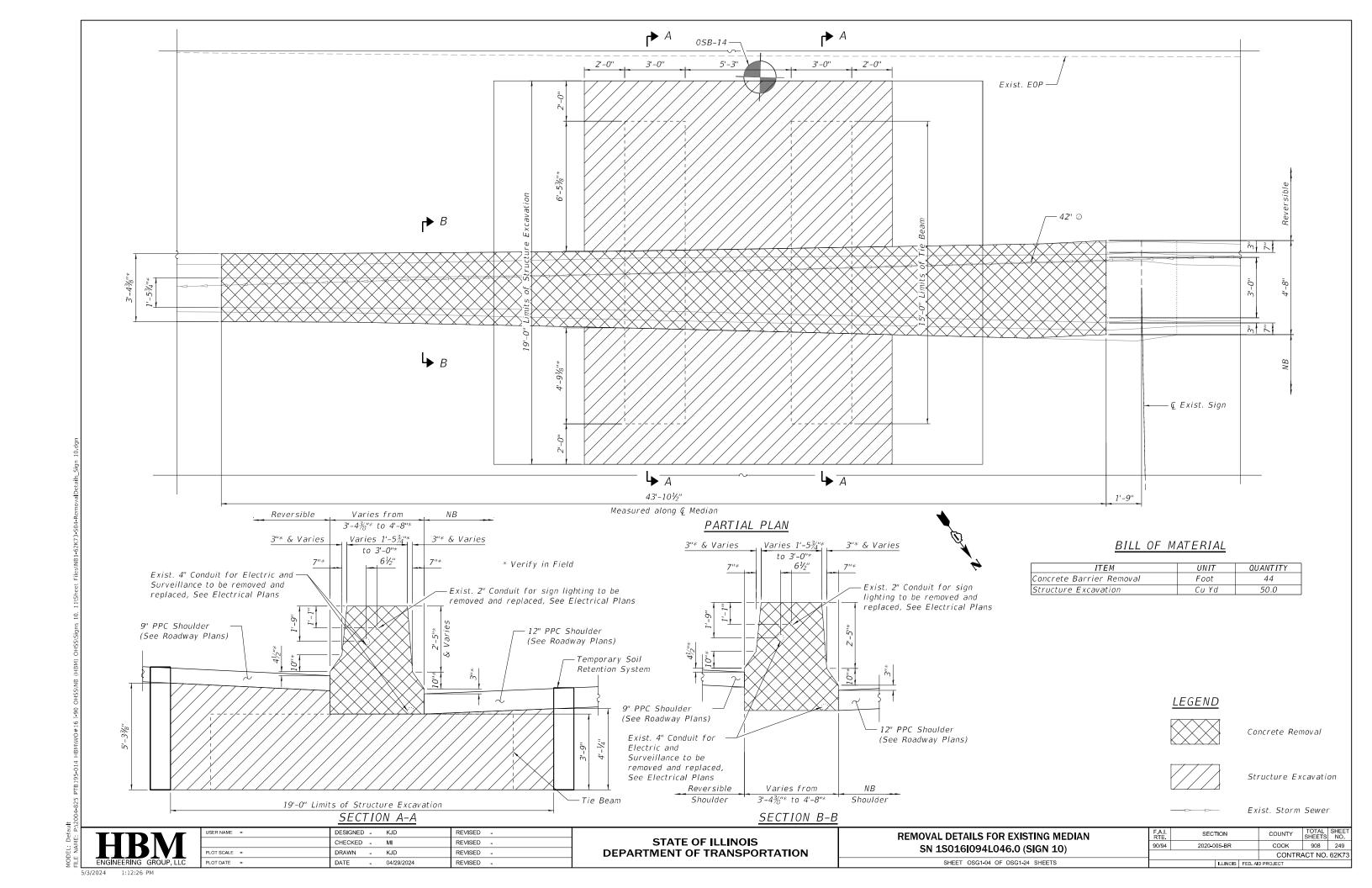
5/3/2024 1:11:48 PM

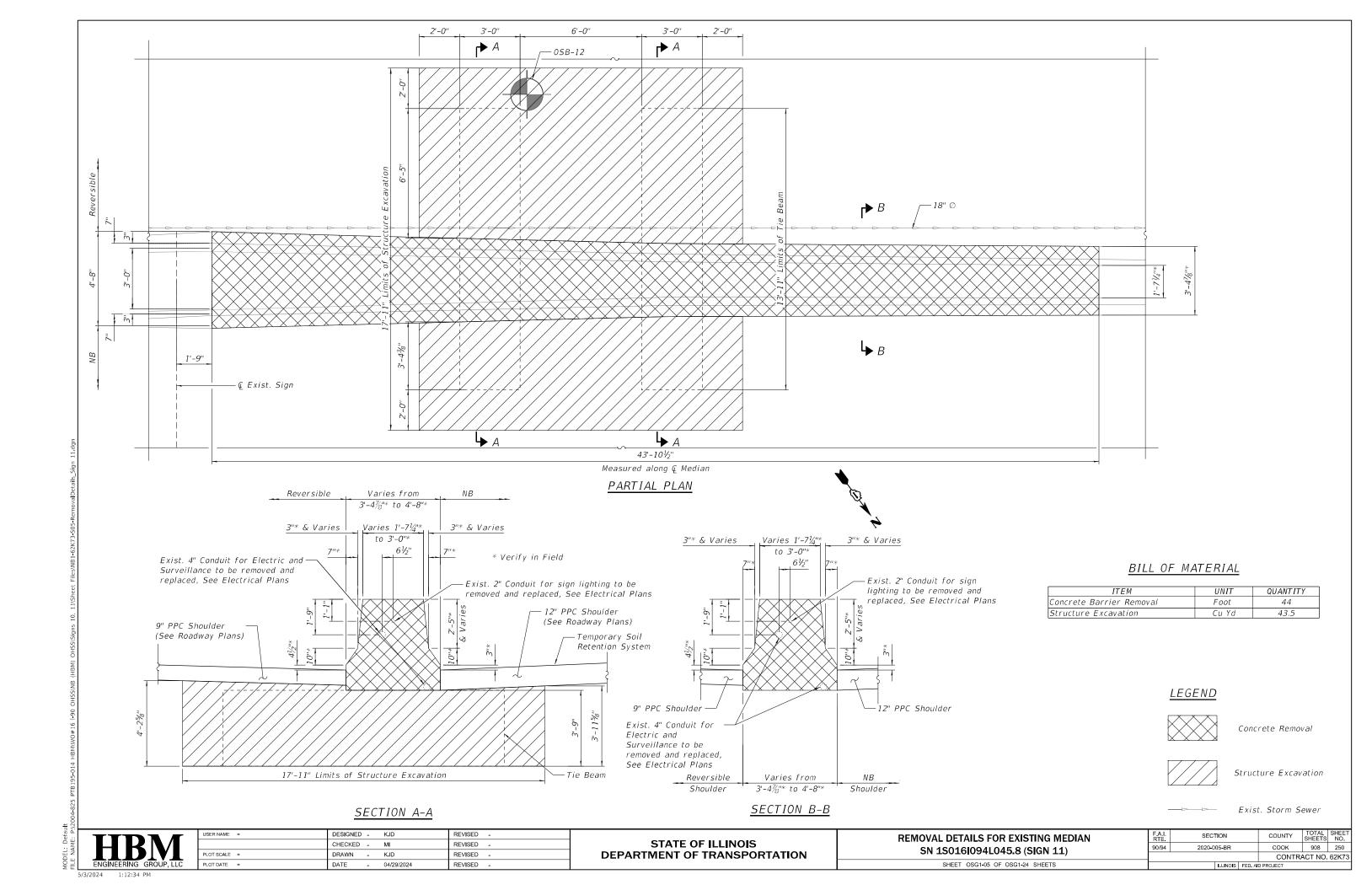
Benchmark: TBM "B" Square cut on top of SW edge of "Exit 45C" overhead sign along I-90/I-94 Belmont ave. exit ramp at Station 533+03.13 OFFSET 65.73 RT. measured along @ NB I-90/94. Elev. 607.49. NOTES: 1. Stations that are shown are with 125'-0" respect to the NB 190/94 baseline. c. to c. of Support Frames 2. The contractor shall establish a local version of the NB Baseline **G** Support - Ç Support based on the dimension shown on 92'-33/8" I-90/94 this plan. The stationing shall be Rev. Lanes I-90/94 NB Lanes Exist. Curb with respect to the center line of and Gutter © I-90/94 NB Roadway existing sign truss as shown. The offset of the baseline shall be 13'-53/8" 4'-8" 12'-0" 12'-0" 12'-0" 12'-0" 12'-0" 14'-10" measured from existing features as Barrier Lane Exit Ramp Lane Lane Lane Lane Lane Shldr shown. 3. The contractor shall exercise extreme caution during installation of TSRS and construction of the proposed foundations to avoid See Roadway Sheets for damage to electrical utilities, Temporary Soil Shoulder Patches, typ. existing drainage structures and Retention pipes. Any damage to the existing Temporary Soil Retention System No. 1 System No. 2 ELEVATION electrical utilities, drainage structures and pipes at this location For Conduit Details See (Looking at Face of Signs) shall be repaired by the contractor Electrical Plans, typ. Exist. 42" Storm Sewerat no additional cost to the Inv. Elev. ±597.32 Department. 1'-6" 12'-0" 4'-8"* 4. The Contractor shall locate & and top of existing sewer pipe in the 3'-0" vicinity of the proposed foundation prior to drilling or poring the proposed foundation. The Contractor 11'-5%" 51'-9¾'' shall Inform the RE of any - TBM "B" discrepancy between the plans and E.O.P. to @ Prop. Sign Truss and Foundation @ Prop. Sign to @ NB I-90/94 existing conditions. This work shall Exist. EOP -Exist. EOP be included in the cost of Earth Exist. EOP Exist. EOP 42" ∅ Exist. EOP Excavation. © I-90/94 Rev -Roadway *LEGEND* € Exist. Sign - See Roadway Sheets ¢ 1-90/94 NB -Sta. 533+01.55 Soil Boring for Shoulder Patches, Remove Overhead Sign Structure - Span Exist. Roadway Fiber Exist. Curb 12' Optic and Gutter Temporary Soil 150161094L046.0 Retention System Sta. 532+80.56 193 0|SB-15 0SB-14-Catch Basin • -| **)**--Light Pole Underground Combined Temporary Soil Storm Sewer Retention Temporary-System No. 2 Exist. (0 Exist. Storm Sewer Electric ~ Retention Line System No. 1 Exist. Exist. Fiber Electric Optic Line 1'-5¾"* * Verify in Field to Match Existing 3'-43/8"* 9'-35%''± 13'-5%' I-90/94 12'-0" 12'-0" 12'-0" 12'-0" 12'-0" 14'-10" 4'-0'' Rev. Lanes Shldr Lane Lane Lane Lane Lane Lane Barrier Shldr PLANDESIGNED - KJD REVISED -SECTION COUNTY **GENERAL PLAN AND ELEVATION** STATE OF ILLINOIS CHECKED - MI REVISED -90/94 2020-005-BR COOK 908 247 SN 1S016I094L046.0 (SIGN 10) REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 62K73 SHEET OSG1-02 OF OSG1-24 SHEETS

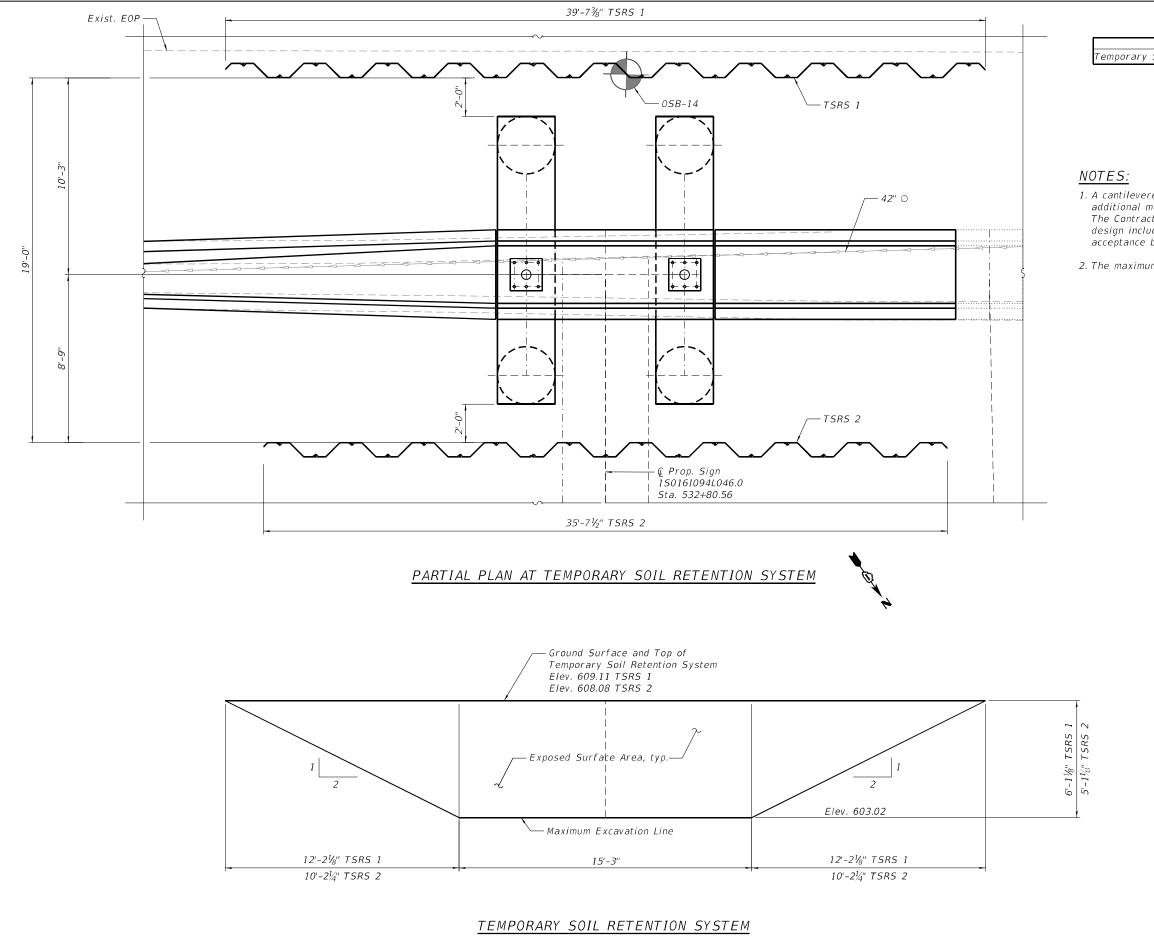
DATE - 04/29/2024

REVISED -









BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Temporary Soil Retention System	Sg Ft	423

- 1. A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
- 2. The maximum allowable excavation slope is 1:2 (V:H).

<u>LEGEND</u>



Soil Boring

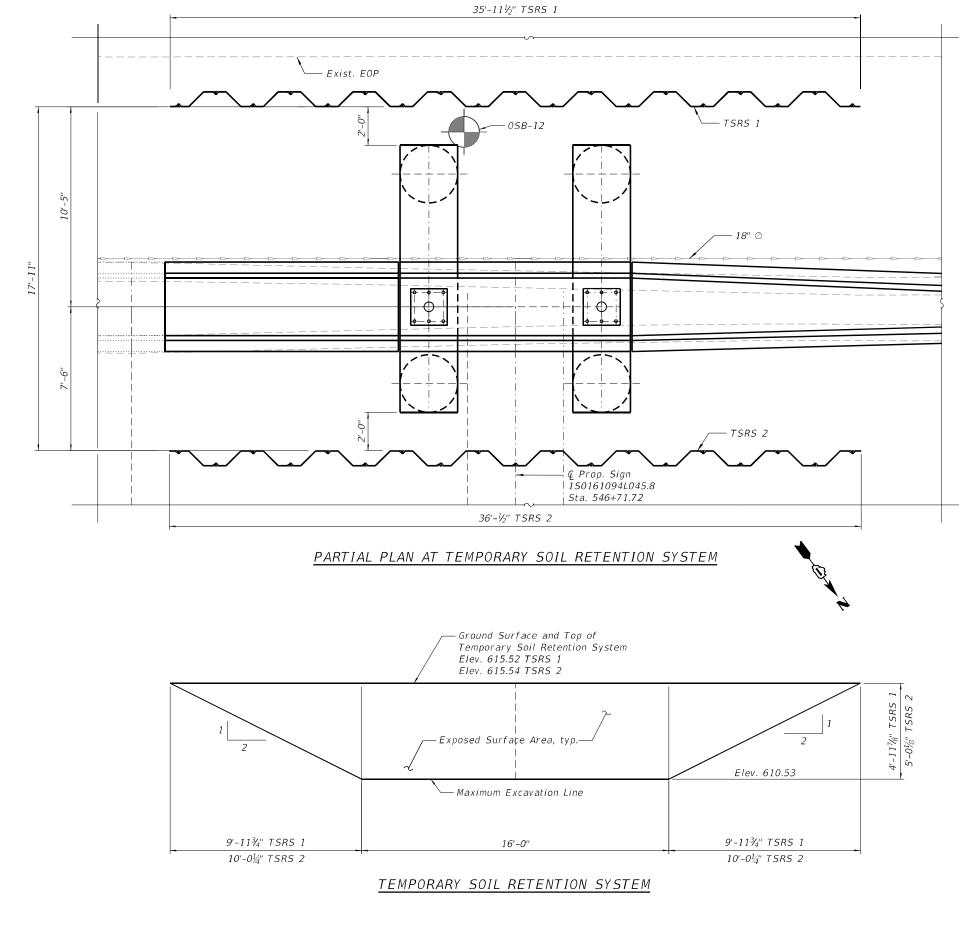
Temporary Soil Retention System

Exist. Storm Sewer



USER NAME =	DESIGNED	-	KJD	REVISED -
	CHECKED	-	MI	REVISED -
PLOT SCALE =	DRAWN	-	KJD	REVISED -
PLOT DATE =	DATE	-	04/29/2024	REVISED -

A.I. RTE	SEC ⁻	TION		COUNTY	TOTAL SHEETS	SHEET NO.	
0/94	4 2020-005-BR		соок	908	251		
·				CONTRA	ACT NO.	62K73	
	Lu puoto Leep aug paga rege						



BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Temporary Soil Retention System	Sq Ft	361

N<u>OTES:</u>

- 1. A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
- 2. The maximum allowable excavation slope is 1:2 (V:H).

<u>LEGEND</u>



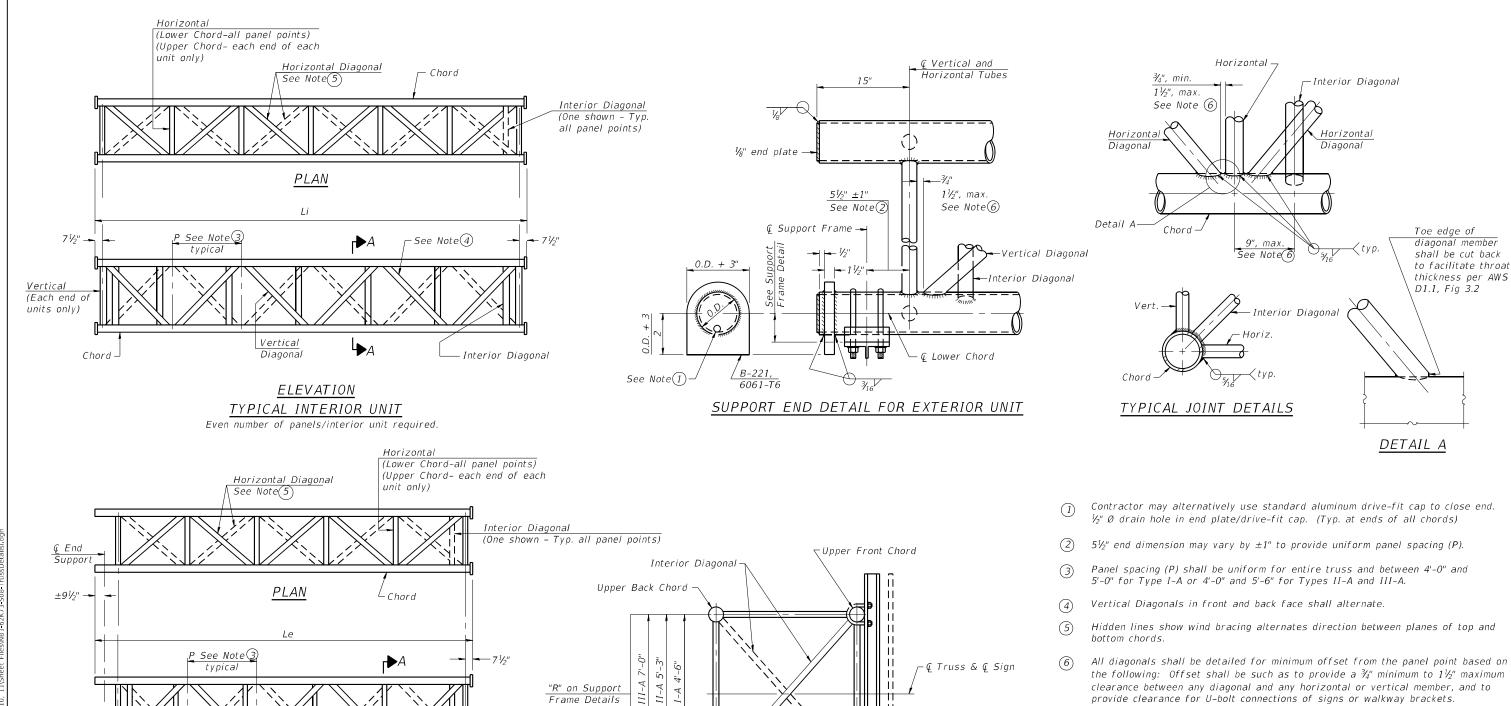
Soil Boring

Temporary Soil Retention System

Exist. Storm Sewer

USER NAME =	DESIGNED	-	KJD	REVISED -
	CHECKED	-	MI	REVISED -
PLOT SCALE =	DRAWN	-	KJD	REVISED -
PLOT DATE =	DATE	-	04/29/2024	REVISED -

A.I. TE	SECT	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.
)/94	2020-0	05 - BR		соок	908	252
•				CONTRA	ACT NO.	62K73
ILLINOIS FED. AID PROJECT						



ELEVATION TYPICAL EXTERIOR UNIT

V<u>ertical Diagon</u>al

See Note 4

Even or odd number of panels/exterior units allowed.

the following: Offset shall be such as to provide a $\frac{3}{4}$ " minimum to $1\frac{1}{2}$ " maximum

SECTION A-A

└"T" on Support Frame Details

Type I-A 4'-0"

Type II-A 4'-6"

Type III-A 5'-0"

Vertical

(Each end of units only)

See Support

End Detail

2-17-2017

USER NAME =	DESIGNED	-	KJD	REVISED	-
	CHECKED	-	MI	REVISED	-
PLOT SCALE =	DRAWN	-	KJD	REVISED	-
PLOT DATE =	DATE	-	04/29/2024	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

Lower Back Chord -

OVERHEAD SIGN STRUCTURES - ALUMINUM TRUSS DETAILS FOR TRUSS TYPES II-A AND III-A SHEET OSG1-08 OF OSG1-24 SHEETS

Reverse direction of interior

II_ Sign Panel - See sign panel

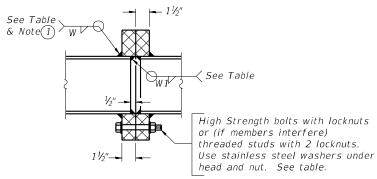
sheet for details.

- Lower Front Chord

diagonals at alternate panels.

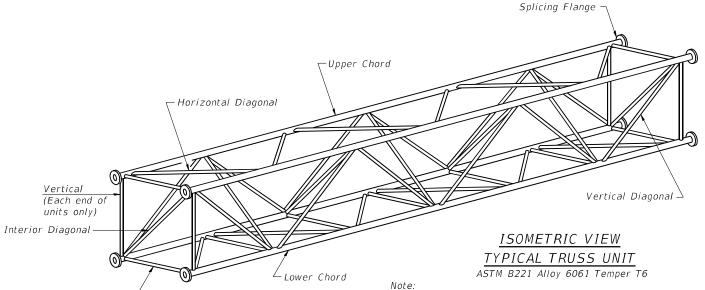
SECTION COUNTY 90/94 2020-005-BR COOK 908 253 CONTRACT NO. 62K73

Sign #	Structure Number	Station	Truss Design	Exterior Units (2)			Interior Unit			Upper and Horiz		Horiza	Verticals; Horizontals; Vertical,		Splicing Flange						
			Type	No. Panels	Unit Lgth.(Le)	Panel Lgth.(P)	No. Req'd	No. Panels	Unit Lgth. (Li)	Panel Lgth.(P)	0.D.	Wall	0.D.	Wall	Midspan	Bol No./Splice		Weld W	Size W1	А	В
Sign 10	1S016I094L046.0	532+80.56 NB	II-A	6	32'-0"	5'-0 1/4"	2	6	31'-4 1/2"	5'-0 1/4"	7"	3/8"	3"	5/16"	4 1/2"	8	1"	7/16"	5/16"	11 1/2"	15"
Sign 11	15016I094L045.8	546+71.72 NB	III-A	8	39'-4 1/2"	4'-8 1/4"	1	8	38'-9"	4'-8 1/4"	8 1/2"	1/2"	3 1/2"	5/16"	3 1/8"	8	1 1/4"	9/16"	7/16"	13"	16 1/2"



SECTION B-B

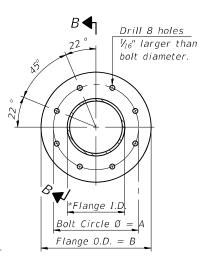
1) Splicing Flanges shall be attached to each truss unit with the truss shop assembled to camber shown. Truss units shall be in proper alignment and flange surfaces shall be shop bolted into full contact before welding. Sufficient external welds or tacks shall be made to secure flanges until remaining welds are made after disassembly. Adjacent flanges shall be "match marked" to insure proper field assembly.



Units shall be shipped individually with adequate provision to prevent detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The Contractor is responsible for maintaining the configuration and protection of the units.

Drill 6 holes $\frac{1}{1_{16}}$ " larger than bolt diameter. *Flange I.D.

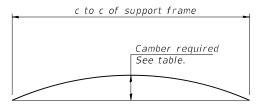
TRUSS TYPES I-A, II-A, & III-A



TRUSS TYPES II-A & III-A

SPLICING FLANGES

ASTM B221, Alloy 6061-T6 or ASTM B209, Alloy 6061-T651 *To fit O.D. of Chord with maximum gap of V_{16} ".



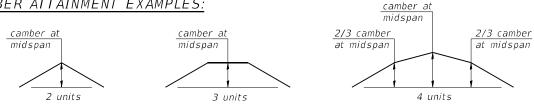
(Upper Chord - each end of each unit only)

(Lower Chord - all panel points)

CAMBER DIAGRAM

Camber curve shown is theoretical. Actual camber attained by slope changes at splices between units.

CAMBER ATTAINMENT EXAMPLES:



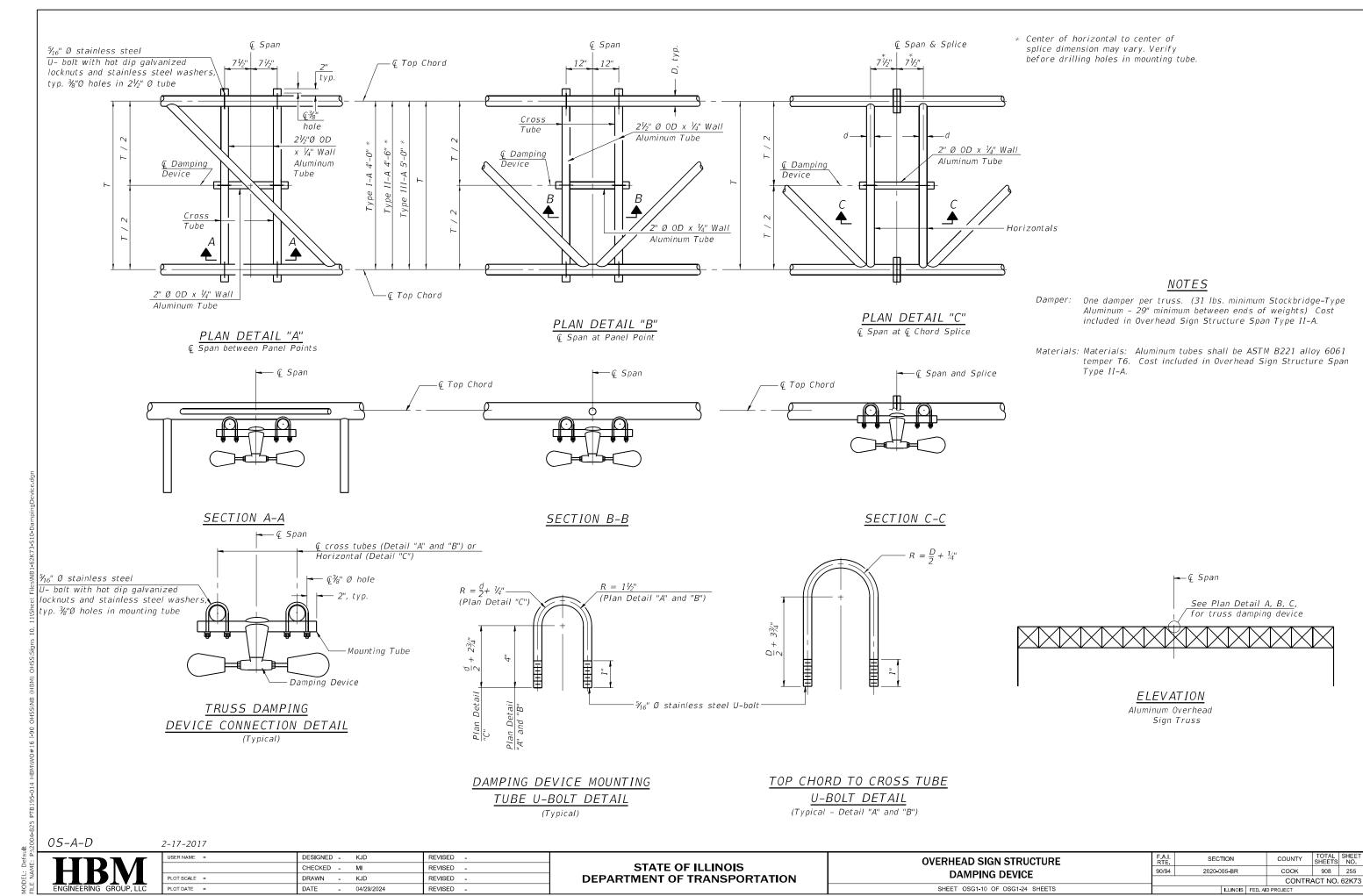
Camber shown is for fabrication only, measured with truss fully supported. (No-load condition)

2-17-2017

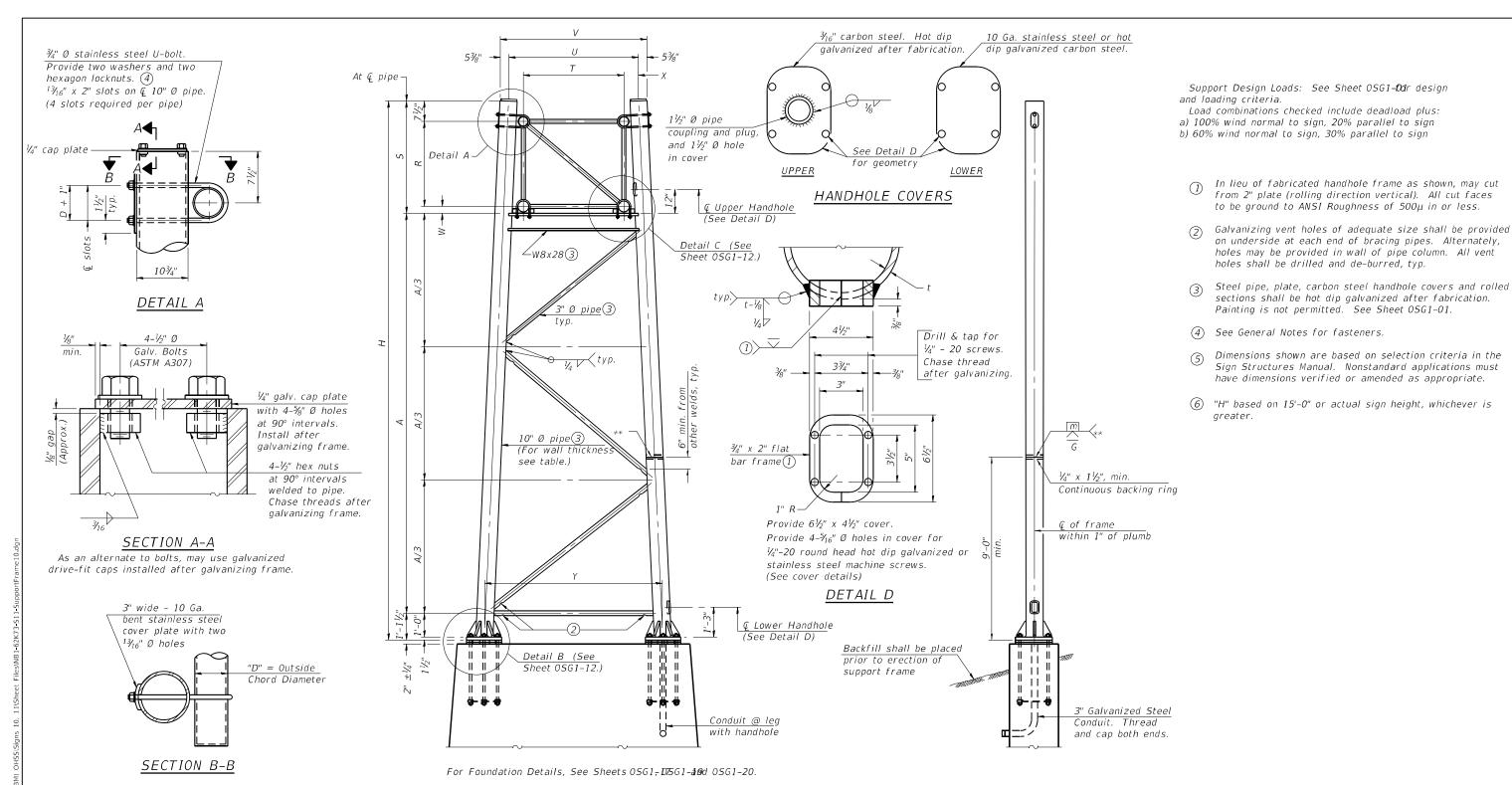
USER NAME =	DESIGNED	-	KJD	REVISED -	
	CHECKED	-	MI	REVISED -	
PLOT SCALE =	DRAWN	-	KJD	REVISED -	
PLOT DATE =	DATE	-	04/29/2024	REVISED -	

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** **OVERHEAD SIGN STRUCTURES - ALUMINUM TRUSS DETAILS** FOR TRUSS TYPES II-A AND III-A SHEET OSG1-09 OF OSG1-24 SHEETS

SECTION COUNTY 90/94 2020-005-BR COOK 908 254 CONTRACT NO. 62K73



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END ELEVATION

SIDE ELEVATION

8'-3"

8'-3"

10" Ø PIPE TRUSS SUPPORT FRAME

** One butt welded joint is allowed only on one post per support frame. If used, weld procedure must be preapproved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

	Sign #	Structure Number	Station	Supp	ort	Truss Type	Pipe Wall Thickness	H(6)	A
	, J			Left	Right	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Inickness		
	Sign 10	150161094L046.0	532+80.56 NB	Χ		II-A	0.365" (Std)	25'-2 3/4"	17'-10"
Ŀ					Χ		0.365" (Std)	30'-9 7/8"	23'-5 1/8"

areater.

from 2" plate (rolling direction vertical). All cut faces

on underside at each end of bracing pipes. Alternately,

holes may be provided in wall of pipe column. All vent

sections shall be hot dip galvanized after fabrication.

Dimensions shown are based on selection criteria in the

Sign Structures Manual. Nonstandard applications must

have dimensions verified or amended as appropriate.

Painting is not permitted. See Sheet OSG1-01.

holes shall be drilled and de-burred, typ.

to be ground to ANSI Roughness of 500µ in or less.

2-17-2017

Truss

Type

II-A (5)

 $I - \Delta$

R

4'-6"

5'-3"

S

4'-0"

4'-6"

5'-51/2"

6'-31/4"

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	CHECKED	-	MI	REVISED -
PLOT SCALE =	DRAWN	-	KJD	REVISED -
PLOT DATE =	DATE	-	04/29/2024	REVISED -

Dimensions

6'-43/1"

6'-113/4"

W

4"

43/4"

9"

91/2"

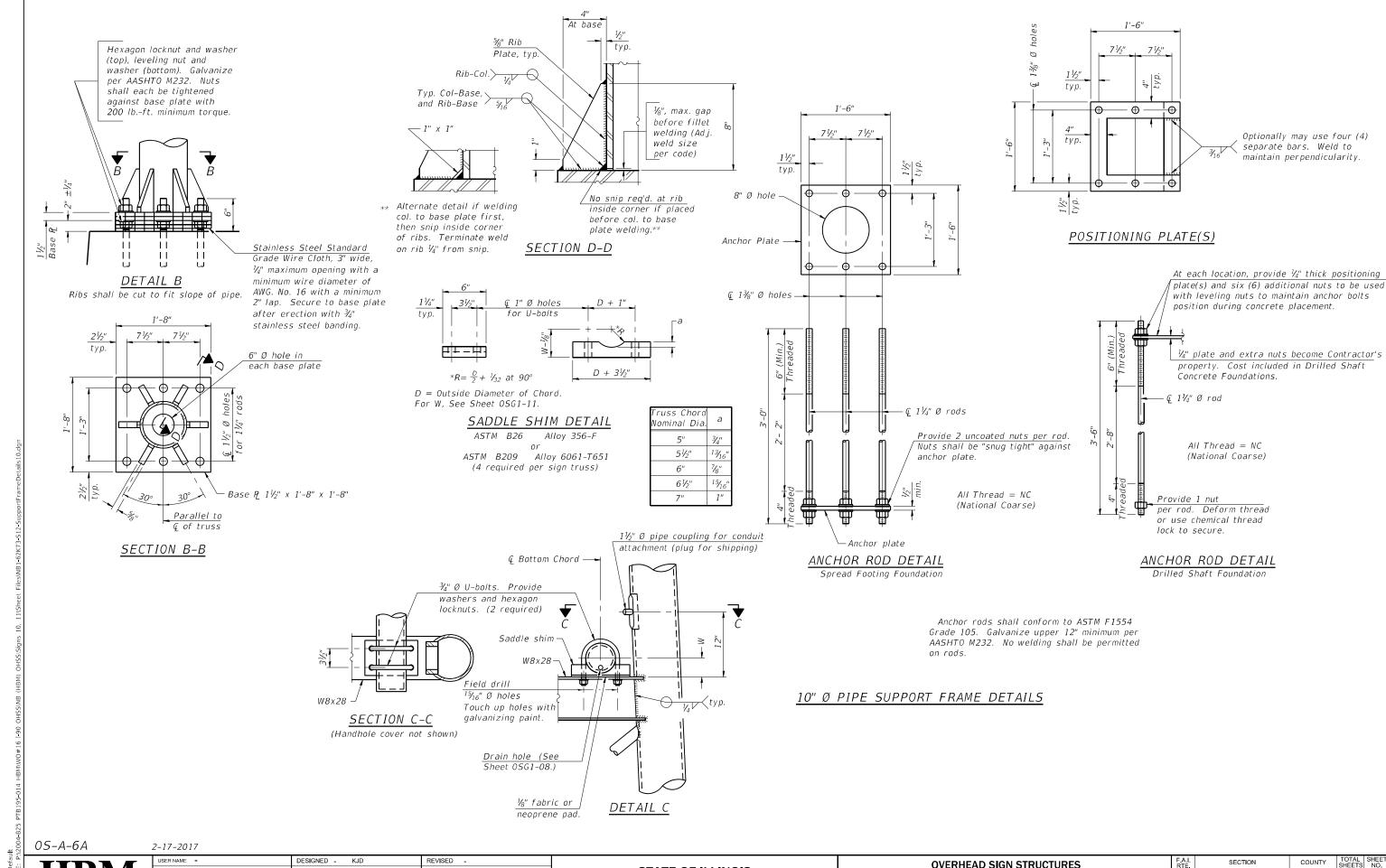
U

5'-6"

6'-1"

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES SUPPORT FRAME	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FOR TYPE II-A ALUMINUM TRUSS	90/94	2020-005-BR	соок	908	256
TOR THE IFA ALUMINUM TROOS			CONTI	RACT NO	62K73
SHEET OSCI-11 OF OSCI-24 SHEETS		LUNIOIO L	EED AID BROJECT		



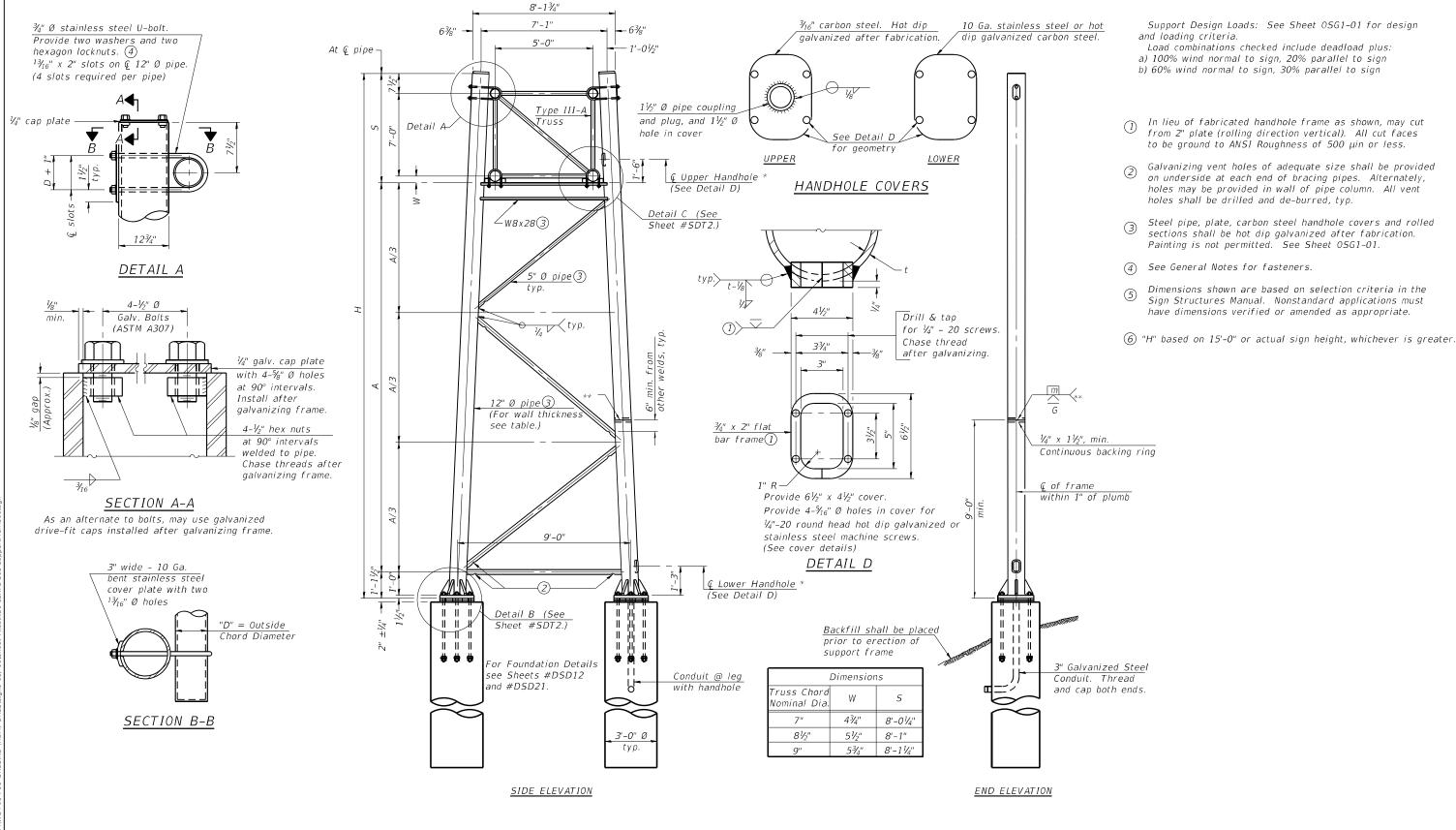
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES 90/94 2020-005-BR SUPPORT FRAME FOR TYPE II-A ALUMINUM TRUSS- DETAILS SHEET OSG1-12 OF OSG1-24 SHEETS

COOK 908 257

CONTRACT NO. 62K73



TRUSS SUPPORT DETAILS

(12" Ø Pipe-Type III-A Truss

** One butt welded joint is allowed only on one post per support frame. If used, weld procedure must be preapproved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

Sign #	Structure Number	Station	Supp	ort	Truss Type	Pipe Wall Thickness	H6	А
			Left	Right				
Cian 11	1S016I094L045.8	546+71.72 NB	Χ		III-A	0.5" (XS)	26'-6 1/8"	17'-3 5/8"
Sign 11				Χ		0.5" (XS)	34'-2 7/8"	25'-0 3/8"

054-A-8a

HBM

2-17-2017

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 PLOT DATE
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 DATE
 04/29/2024
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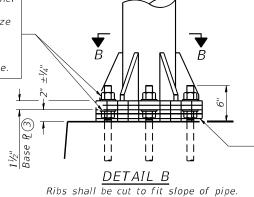
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 OVERHEAD SIGN STRUCTURES - SUPPORT FRAME
 FAI. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL SHEETS NO.

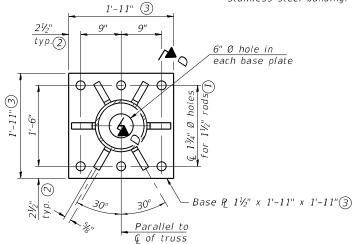
 FOR TYPE III-A ALUMINUM TRUSS
 90/94
 2020-005-BR
 COOK
 908
 258

 SHEET OSG1-13 OF OSG1-24 SHEETS
 SHEET OSG1-12 SHEETS
 ILLINOIS FED AND PROJECT

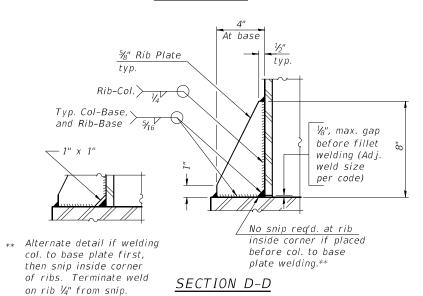
ENGINEERING G

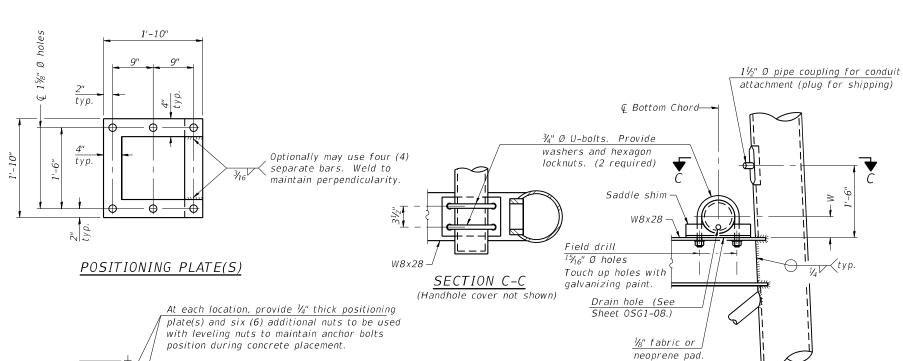


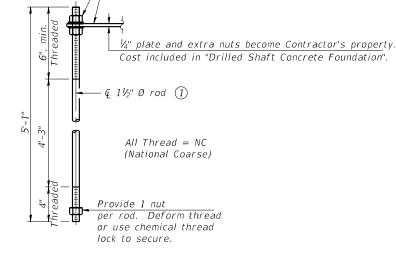
Stainless Steel Standard Grade Wire Cloth, 3" wide, 1/4" maximum opening with a minimum wire diameter of AWG. No. 16 with a minimum 2" lap. Secure to base plate after erection with ¾" stainless steel banding.



SECTION B-B







ANCHOR ROD DETAIL

Anchor rods shall conform to ASTM F1554 Grade 105 Galvanize upper 12" minimum per AASHTO M232. No welding shall be permitted on rods.

TYPE III-A TRUSS 12" Ø PIPE SUPPORT FRAME DETAILS

- 1 1 ¾ g rod, 2" Ø holes
- (2) 2¾" edge distance

For Type III-A Truss spans greater than 150 ft, and up to 160 ft.:

- ③ Base P₂ 15/8" x 1'-111/2" x 1'-111/2"

054-A-8aA

2-17-2017

HBM
ENGINEERING GROUP, LLC

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES SUPPORT FRAME FOR TYPE III-A ALUMINUM TRUSS SHEET OSG1-14 OF OSG1-24 SHEETS

SECTION COUNTY 2020-005-BR COOK 908 259 CONTRACT NO. 62K73

DETAIL C

 $D + 3\frac{1}{3}$

for U-bolts

SADDLE SHIM DETAIL

ASTM B209 Alloy 6061-T651

(4 required per sign truss)

Alloy 356-F

 $*R = \frac{\nu}{2} + \frac{1}{32}$ at 90° D = Outside Diameter of Chord.

ASTM B26

For W, see Sheet #SAT2.

typ.

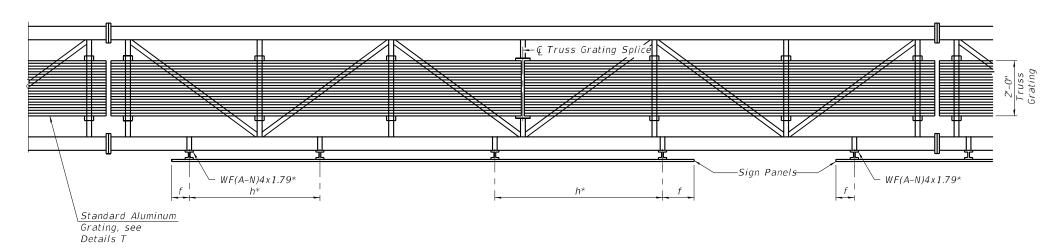
11/4"

russ Chora

81/2"

ominal Dia.

TYPICAL FRONT ELEVATION



BRACKET TABLE

	WF(A-N)4x1.79 ASTM B308, Alloy 606								
Sign V	Number								
Greater Than	Less Than or Equal To	Brackets Required							
	8'-0"	2							
8'-0"	14'-0"	3							
14'-0"	20'-0"	4							
20'-0"	5								
26'-0"	32'-0"	6							

SECTION A-A

Place all sign brackets as close to panel points as practical.

* Space sign brackets WF(A-N)4x1.79 for efficiency and within limits shown:

f=12" maximum, 4" minimum (End of sign to $\mathcal Q$ of nearest bracket) h=6'-0" maximum ($\mathcal Q$ to $\mathcal Q$ sign support brackets, WF(A-N)4x1.7

Notes:

For Detail T and Section B-B, see Sheet OSG1-16.
Truss grating to facilitate inspection shall run full length
(center to center of support frames) 12"± on overhead trusses.
Cost of truss grating is included in "Overhead Sign Structure".

Truss Costing width dimensions are positive and many variables.

Truss Grating width dimensions are nominal and may vary $\frac{V_2}{2}$ based on available standard widths.

 $05-\Delta-9-NW$

4-1-2020

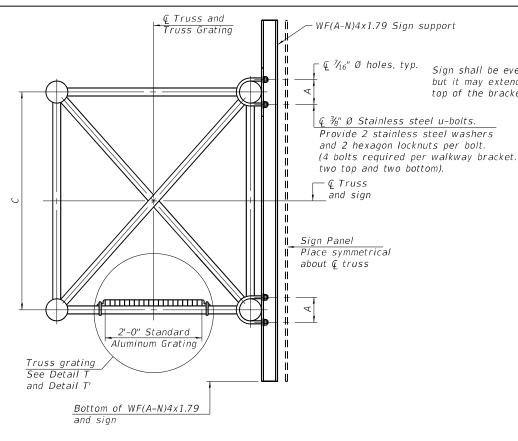


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PLOT DATE =	DATE	-	04/29/2024	REVISED -

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5 PTB195-014 HBM\WO#16 I-90 OHSS\NB (HBM) OHSS\Signs 10, 11\Sheet Files\NB1-62K73-S15-WalkwayDetailsI.dgr



SECTION B-B

SPECIFICATIONS FOR STANDARD ALUMINUM GRATING

Main Bearing Bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on $1\frac{3}{16}$ " centers and conform to ASTM B221 Alloy 6061–T6.

Cross bars shall be \aleph_{16} " x $1\frac{1}{2}$ " on 4" centers and conform to ASTM B221 Alloy 6063-T5 or 6061-T6.

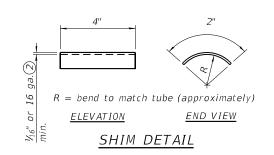
ΩR

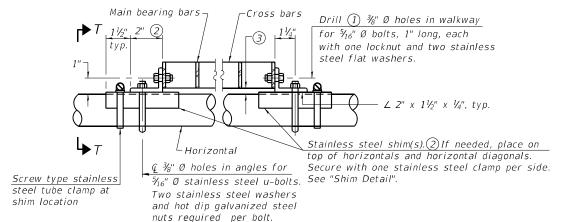
Aluminum Grating with modified "t" sections for main bearing bars shall meet the following requirements:

Main bars shall conform to ASTM B221 Alloy 6061-T6 and have a minimum section modulus equal to 0.0705 in.³ per bar, a depth of $1\frac{1}{2}$ ", spaced on $1\frac{3}{16}$ " centers.

Cross bars shall conform to ASTM B221 Alloy 6063-T5 or T-42 and spaced on 4" centers.

Sign #	Structure Number	Station	Α	С
Sign 10	150161094L046.0	532+80.56 NB	8"	5'-3"
Sian 11	1S016I094L045.8	546+71.72 NB	9 1/2"	7'-0"





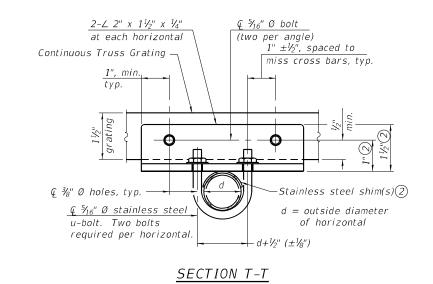
DETAIL T

U-bolt and angle connections

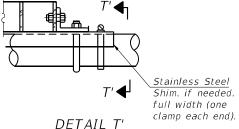
required at horizontals only.

Sign shall be even with the top of the bracket, but it may extend no more than 6" above the top of the bracket for field adjustments.

(Continuous Truss grating)

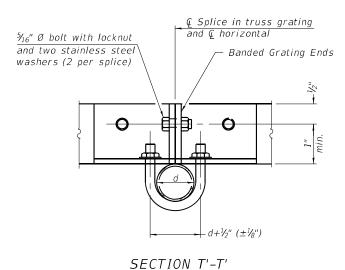


- Drilling holes in grating may be done in shop or field, based on Contractor's preference and subject to accurate alignment.
- 2 Stainless steel shims shall be placed as shown in Detail T if needed to compensate for alignment variations between horizontal and diagonal pipes beyond adjustment provided by angles. Thicker shims may be used subject to shims performing properly.
- 3 Tube to grating gap may vary from 0 to ½", max. to align walkway, allow for camber, etc.



DETAILT

(Truss grating splice)
Details not shown same as Detail T.
Alternate materials may be used subject to the
Engineer's review and approval.



05-A-10-NW

4-1-2020

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 KJD
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 PLOT DATE
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 DATE
 04/29/2024
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES
ALUMINUM WALKWAY DETAILS
SHEET OSG1-16 OF OSG1-24 SHEETS

 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 90/94
 2020-005-BR
 COOK
 908
 261

 CONTRACT NO. 62K73

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Bar	Number	Size	Length	Shape
v4(E,	24	#9	F less 5"	
#4 k	ar spiral	(E) - see	Side Eleva	tion

NOTES

3" Ø Galvanized Steel

Conduit. Thread

and cap both ends.

The foundation dimensions shown are based on the presence of mostly cohesive soils with an average Unconfined Compressive Strength (Qu) of at least 1.25 tsf, which must be determined by previous soil investigations at the jobsite. When other conditions are indicated, the boring data will be included in the plans and the foundation dimensions shown will be the result of site specific designs.

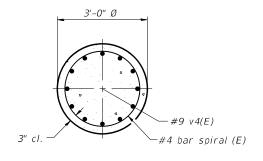
If the conditions encountered are different than those indicated, the Contractor shall notify the Engineer to determine if the foundation dimensions need to be modified. If dimensions "B" or "F" are revised by more than 12" by the Contractor, "as-built" plans shall be prepared and submitted to the District Bureau of Operations for future reference.

No sonotubes or decomposable forms shall be used below the lower conduit entrance. Permanent metal forms or other shielding may not be left in place below that elevation without the Engineer's written permission.

Concrete shall be placed monolithically, without construction joints.

Backfill shall be placed per Article 502 of Standard Specification and prior to erection of support column.

A normal surface finish followed by a Concrete Sealer application will be required on concrete surfaces above the lowest elevation 6" below finished ground line. Cost included in Drilled Shaft Concrete Foundation.



SECTION A-A

<u>DETAILS FOR 10" Ø SUPPORT FRAME</u> <u>TYPE I-A or II-A TRUSS</u>

	Structure Number	Station		Left Foun	dation			Class DS Concrete				
			Elevation Top	Elevation Bottom	В	F	Elevation Top	Elevation Bottom	А	В	F	(Cu. Yds.)
)	15016I094L046.0	532+80.56 NB	-	-	-	-	606.53	582.03	3'-6"	21'-0"	24'-6"	12.90

054-F3 054-F3

2-17-2017

LIB USER NAME =

PLOT SCALE =

PLOT DATE =

#4 bar spiral (E) at 6" pitch

12-#9 v4(E) bars-

3 hoops minimum top and bottom

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

Sign #

Sign 10

OVERHEAD SIGN STRUCTURES
DRILLED SHAFT DETAILS - SN 1S016I094L046.0 (SIGN 10)

SHEET OSG1-17 OF OSG1-24 SHEETS

7½"

7½"

7½"

7½"

7½"

7½"

8'-3"

PLAN

*\frac{\cappa_{4}" \(\Delta \) x 10'-0" copper weld ground rod driven into groun 9'-0". Cost of rod, cable, conduit, caps and clamps shall be included in Drilled Shaft Concrete Foundations.

SIDE ELEVATION

8'-3" (to (

₹

3'-0" Ø

Approved clamps for grounding*

> #6 copper wire or cable

> > _3'-0" Ø

For anchor rod size and placement, see Support Frame Detail Sheet.

END VIEW

Elevation (Top)

Elevation (Bottom)

> * Anchor rod shall be ground or filed to bright metal at clamp and cable connection location.

Bar	Number	Size	Length	Shape
v4(E)	24	#9	F less 5"	
#4 ba	ar spiral	(E) - see	Side Eleva	tion

The foundation dimensions shown are based on the presence of mostly cohesive soils with an average Unconfined Compressive Strength (Qu) of at least 1.25 tsf, which must be determined by previous soil investigations at the jobsite. When other conditions are indicated, the boring data will be included in the plans and the foundation dimensions shown will be the result of site specific designs.

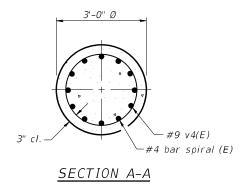
If the conditions encountered are different than those indicated, the Contractor shall notify the Engineer to determine if the foundation dimensions need to be modified. If dimensions "B" or "F" are revised by more than 12" by the Contractor, "as-built" plans shall be prepared and submitted to the District Bureau of Operations for future reference.

No sonotubes or decomposable forms shall be used below the lower conduit entrance. Permanent metal forms or other shielding may not be left in place below that elevation without the Engineer's written permission.

Concrete shall be placed monolithically, without construction joints.

Backfill shall be placed per Article 502 of Standard Specification and prior to erection of support column.

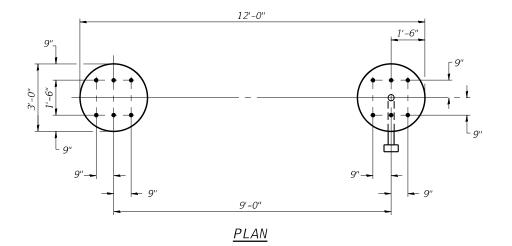
A normal surface finish followed by a Concrete Sealer application will be required on concrete surfaces above the lowest elevation 6" below finished ground line. Cost included in Drilled Shaft Concrete Foundation.



DETAILS FOR 12" Ø SUPPORT FRAME TYPE III-A TRUSS

Sign #	Structure Number	Station		Left Found	dation			Class DS Concrete				
			Elevation Top	Elevation Bottom	В	F	Elevation Top	Elevation Bottom	А	В	F	(Cu. Yds.)
Sign 11	150161094L045.8	546+71.72 NB	-	-	ı	-	611.05	582.55	3'-6"	25'-0"	28'-6"	15.00
•												

9'-0" & to & Elevation (Top) Approved clamps for grounding #4 bar spiral (E) at 6" pitch 3" Ø Galvanized Steel Conduit. Thread and cap both ends. #6 copper wire or cable ₹ 12-#9 v4(E) bars -³¾" Ø x 10'-0" copper weld ground rod driven into ground 9'-0". Cost of rod, cable, conduit, caps and clamps shall be included in Drilled Shaft Concrete Foundations. 3'-0" Ø 3'-0" Ø Elevation (Bottom) END VIEW SIDE ELEVATION 3 hoops minimum top and bottom



For anchor rod size and placement, see Support Frame Detail Sheet.

* Anchor rod shall be ground or filed to bright metal at clamp and cable connection location.

USER NAME =	DESIGNED	-	KJD	REVISED -
	CHECKED	-	MI	REVISED -
PLOT SCALE =	DRAWN	-	KJD	REVISED -
PLOT DATE =	DATE	-	04/29/2024	REVISED -

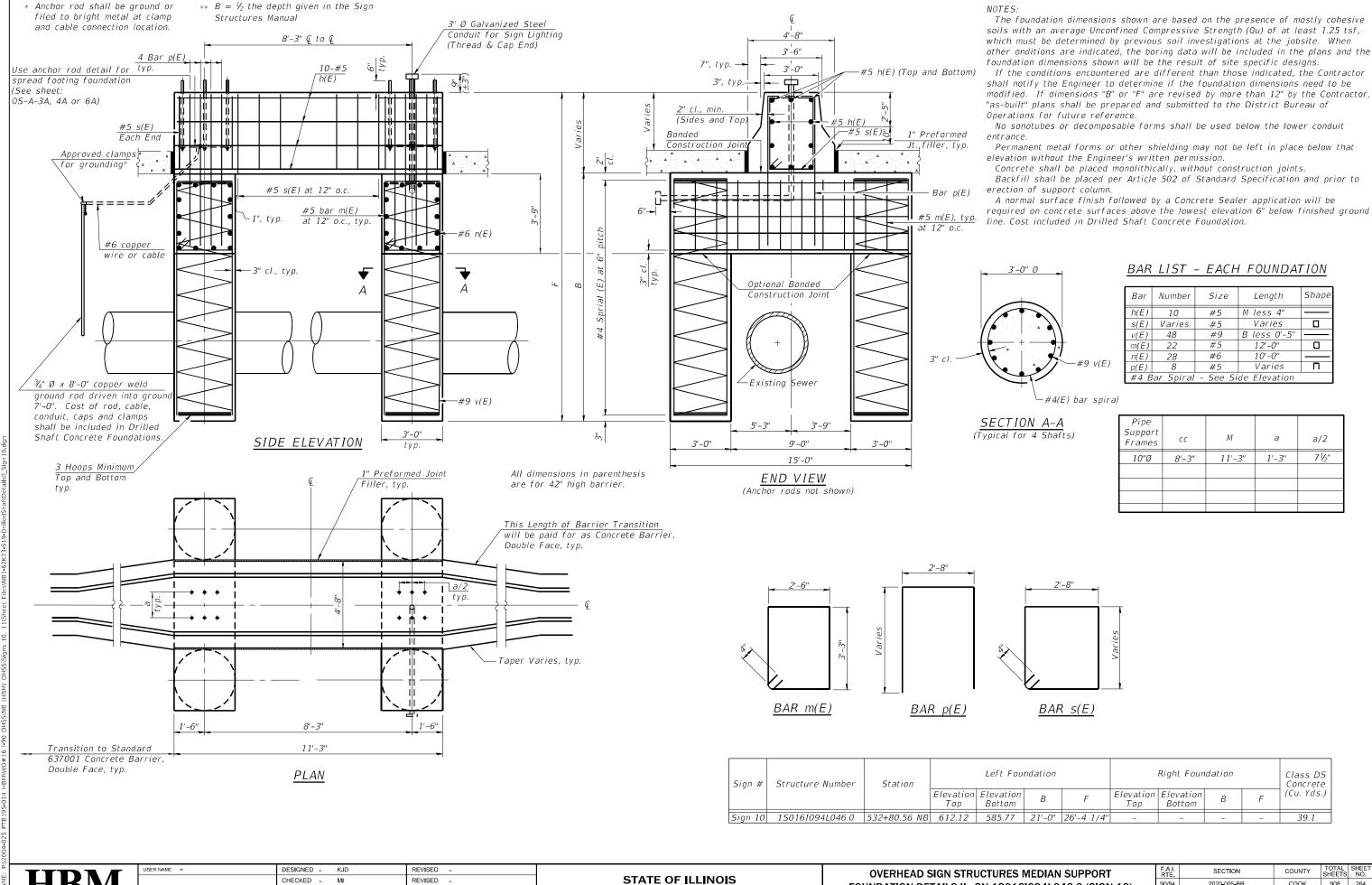
STATE OF ILLINOIS

OVERHEAD SIGN STRUCTURES DRILLED SHAFT DETAILS - SN 1S016I094L045.8 (SIGN 11) SHEET OSG1-18 OF OSG1-24 SHEETS

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
90/94	2020-005-BR		соок	908	263
			CONTRA	ACT NO.	62K7
	ILLINOIS	FED. Al	D PROJECT		

2-17-2017

DEPARTMENT OF TRANSPORTATION



DEPARTMENT OF TRANSPORTATION

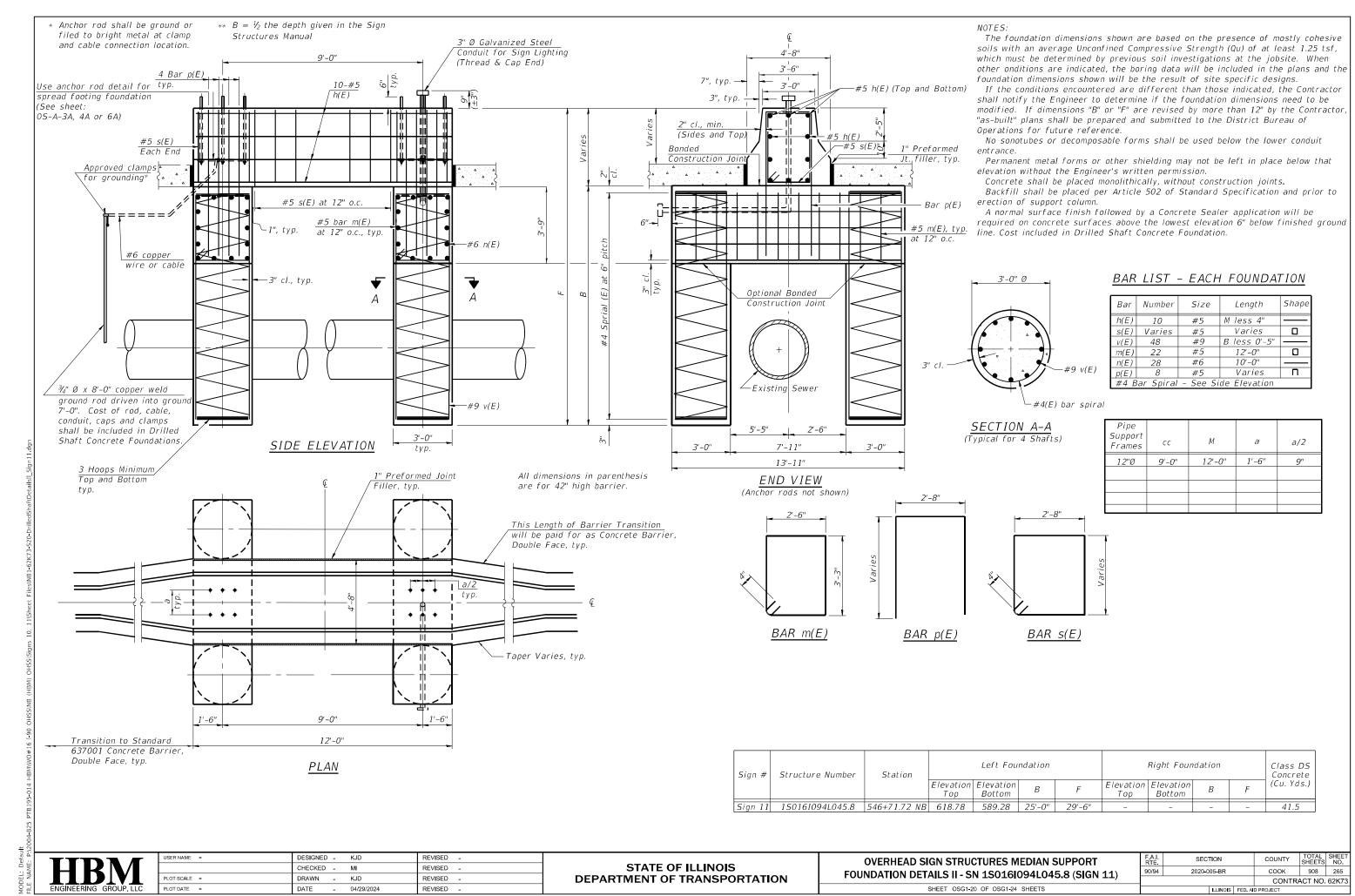
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REVISED .

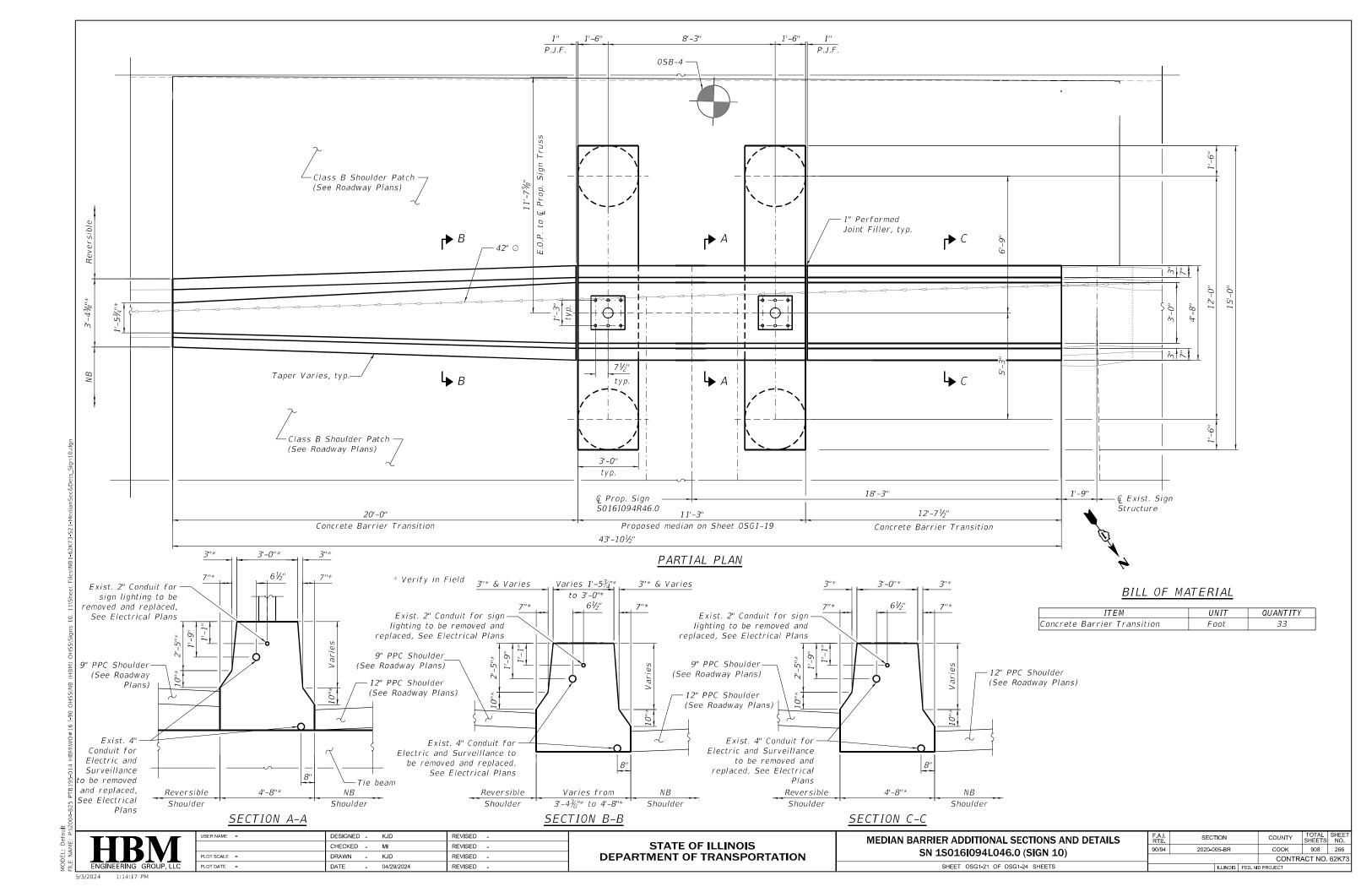
04/29/2024

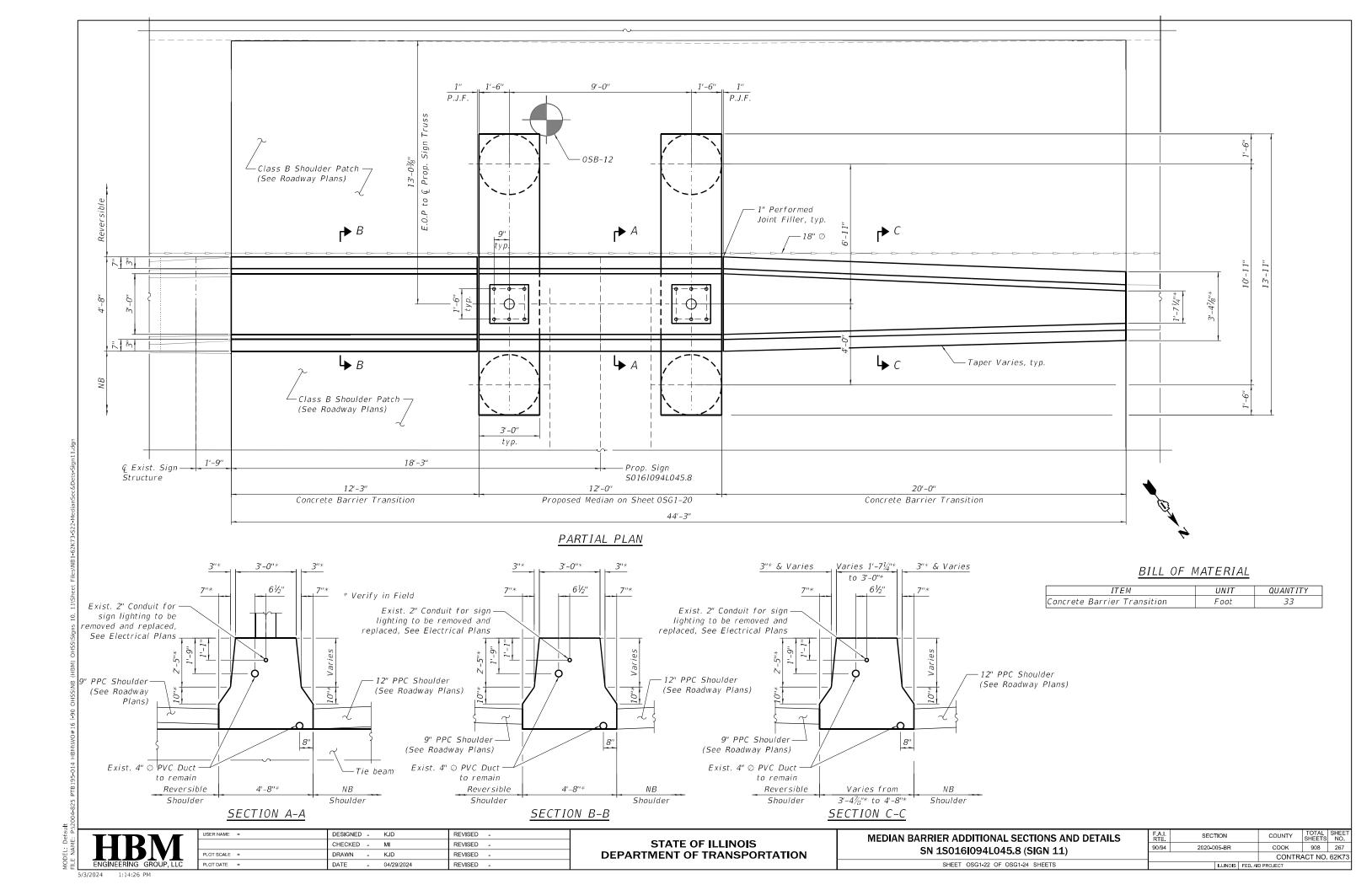
FOUNDATION DETAILS II - SN 1S016I094L046.0 (SIGN 10) SHEET OSG1-19 OF OSG1-24 SHEETS

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2020-005-BR	соок	908	264	
			CONTRA	ACT NO.	62K73
	ILLINOIS	FED. A	D PROJECT		



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GSI Job No. 19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date 10/19/21

PTB 185-012, WO #32 **PROJECT** I-90 & I-94 Tollway LOCATION COUNTY **DRILLING METHOD** Hollow Stem Auger **HAMMER TYPE** CME Automatic D B U M Surface Water Elev. CLIENT **HBM** E L CO C S Stream Bed Elev. 0 P O SI BORING NO. OSB-14 T W S T W Groundwater Elev.: 1920385 Northing H S Qu T H S Qu T First Encounter 1155368 **Easting Upon Completion** Dry ft Ground Surface Elev. 608.3 (ft) (/6") (tsf) (%) (pcf) Hrs. ft After (ft) (/6") (tsf) (%) (pcf) CLAY LOAM-brown & gray-stiff to 10.0" CONCRETE very stiff (continued) CRUSHED STONE-loose (Fill) 5 2.50 20 6 B CLAY LOAM-dark brown & gray spotted black-very stiff to hard 6 2.50 21 3 0.90 23 10 B 4 B CLAY LOAM-gray-soft to medium 3 2.00 24 2 0.50 24 Р becoming gray @ -8.0' 3 0.25 27 4.00 11 4 P Ρ End Of Boring @ -30.0'. Boring backfilled with cuttings. 4 3.10 17 5 В 5 2.60 14 4 B SILTY CLAY-black-stiff 1.40 29 В CLAY LOAM-brown & gray-stiff to very stiff 6 2.90 16 I B

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



GSI Job No. __19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date <u>11/1/21</u>

	PROJECT	PTB 185-012	, WO #32													
	LOCATION	I-90 & I-94 To	ollway													
	COUNTY	Cook	DRILLING	ME	тно	D _		Hol	low Stem Auger	HAMMER T	YPE .	(CME	Auto	mati	c
	CLIENT			D E P T	B L O W	U C S	M O I S	DRY DWZW-FY	Surface Water Elev Stream Bed Elev Groundwater Elev.:		_ ft _ ft	D E P T	B L O W	U C S	M O I S	→
	Northing Easting			Н	s	Qu		j T	First Encounter	Dry		Н	S	Qu	Τ	Ĭ Ţ
		ce Elev. 608	.0 ft	(ft)	(/6")	(tsf)	(%)	(pcf)	Upon Completion _ After Hrs.	Dry	ft	(ft)	(/6")	(tsf)	(%)	
	10.0" TOPSOIL-	-black	607.2	_			21		CLAY LOAM-brown & q	gray-very		2				
	CRUSHED ASP STONE-medium		007.2		8		21		Cum (comunaca)				4			
	310NL-median	r dense (r iii)			15 12		4					_	4 7	2.00 P	23	
8/21			605.0	_								-				
J 11/1	CLAY LOAM-bro spotted black-sti	own & gray iff to very stiff (F	ill)	-	5							_	2			
OG.GF				_	9	1.50	17					_	3	3.50	24	
79-B_L				5	11	Р					582.5	-25	0	В		-
35/190				_	2				CLAY-gray-medium sti	ff			2			
NG LOC				_	4	1.50	24					_	2	0.80	19	\dashv
BORIL				_	3	В							1	В		
9079-B												_	•			
2K74\1					3	1.50	21						3	0.50	24	
K73&6;				-10	10	В			End Of Boring @ -30.0	' Poring	578.0	-30	4	Р		
TS 62				_					backfilled with cuttings.	. Boring		0				
ROJEC				_	4 5	2.10	22					-				
EDY P					8	В										
KENN																
WO#1				_	3 5	1.90	21					_				
2-007-				<u> </u>	12	1.90 B	21					-35				
OOT 19				_								_				
EFK, IC				_	4							_				
Z.\PRO.ECTS\2019\19079-B EFK, IDOT 192-007-WO#15 KENNEDY PRO.ECTS 62K73&62K74\19079-B BORING LOGS\19079-B_LOG.GPJ 11/18\21					7	2.60 B	19									
019/19	CLAVICANI	0 ma	590.0	_												
ECTS/2	CLAY LOAM-bro	own & gray-very		_	3							-				
PROJE				_	4 6	2.70 B	19					_				
ίż				-20		ט						-40	L			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



USER NAME =	DESIGNED -	KJD	REVISED -	
	CHECKED -	MI	REVISED -	
PLOT SCALE =	DRAWN -	KJD	REVISED -	
PLOT DATE =	DATE -	04/29/2024	REVISED -	



GSI Job No. __19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date 10/19/21

PTB 185-012, WO #32 **PROJECT** I-90 & I-94 Tollway LOCATION COUNTY **DRILLING METHOD** Hollow Stem Auger **HAMMER TYPE** CME Automatic D B UM Surface Water Elev. CLIENT **HBM** Е CO Ε СО Stream Bed Elev. 0 SI S BORING NO. OSB-12 T W S T W Groundwater Elev.: Northing 1921207 H S Qu T First Encounter H S Qu T 1154249 **Easting Upon Completion** Dry ft Ground Surface Elev. 614.6 (ft) (/6") (tsf) (%) (pcf) Hrs. ft After (ft) (/6") (tsf) (%) (pcf) 10.0" CONCRETE CLAY LOAM-dark brown & gray-stiff to very stiff (Fill) CRUSHED STONE-dense (Fill) (continued) 6 1.20 22 13 В CLAY LOAM-dark brown & SILTY CLAY-black-stiff gray-stiff to very stiff (Fill) 4 2.00 18 4 1.00 27 6 B Р CLAY LOAM-brown & gray-stiff 6 3.00 16 1.10 26 В В 5 2.75 18 4 1.00 26 В End Of Boring @ -30.0'. Boring backfilled with cuttings. 6 2.30 19 В 6 2.50 18 10 B 5 3.00 18 В 5 2.00 18 50/3" B

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



GSI Job No. __19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date <u>11/1/21</u>

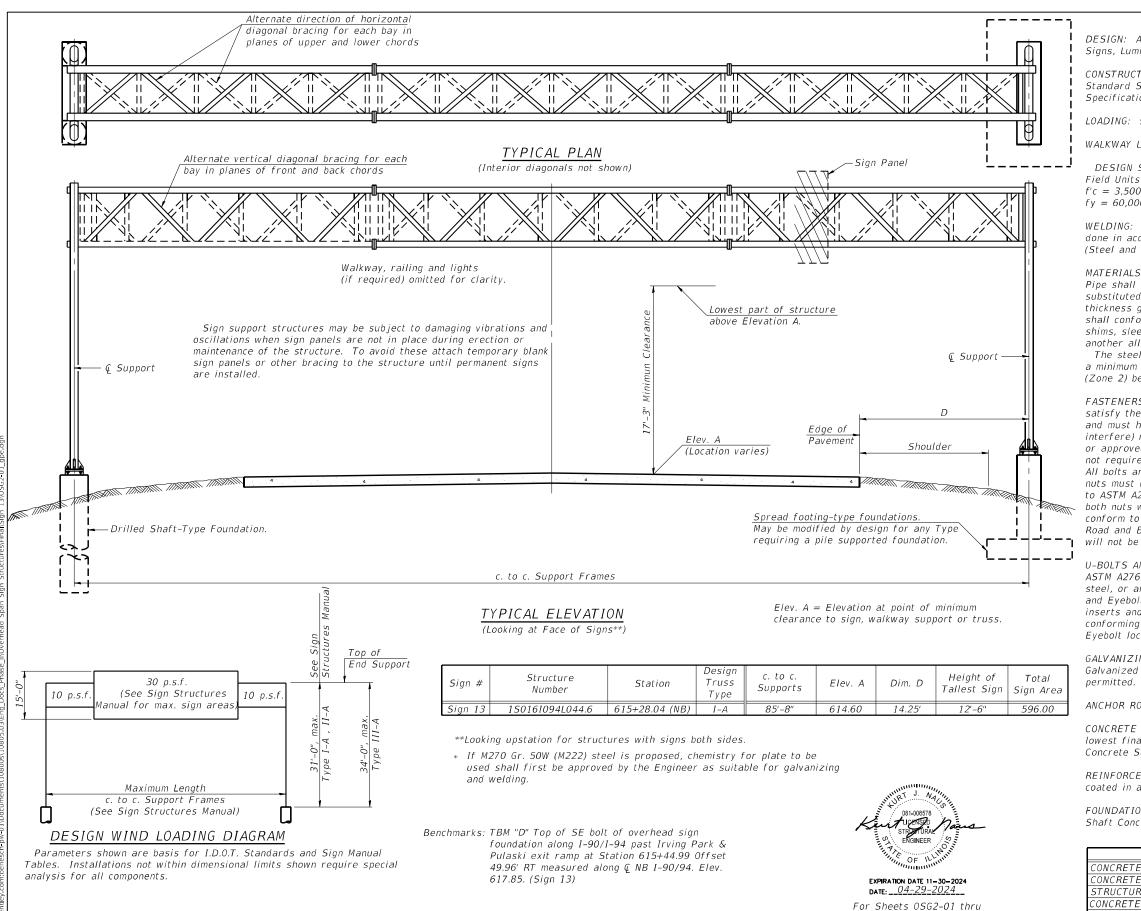
PROJECT	PTB 185-012	2, WO #32													_
LOCATION	I-90 & I-94 T	ollway													
COUNTY	Cook	DRILLING	ME	тно	D _		Hol	llow Stem Auger	HAMMER T	YPE	(CME	Auto	mati	С
CLIENT	НВМ		D E	B L	U	M O	PRY	Surface Water Elev. Stream Bed Elev.		_ ft _ ft	D E	B L	UC	M 0	DRY D
	1921284 1154304		P T H	O W S	S Qu	S T	DEZS-T-Y	Groundwater Elev.: First Encounter Upon Completion	Dry Dry		P T H	O W S	S Qu	I S T	DHZW-+Y
Ground Surfa 8.0" SANDY TO	ce Elev. 61	4.8 ft	(ft)	(/6")	(tsf)	(%)	(pcf)	_		ft	(ft)	(/6")	(tsf)	(%)	(pcf)
CLAY LOAM-br	rown & gray	614.1	_	6		19		CLAY LOAM-brown & spotted black-stiff to ha (continued)	gray ard (Fill)		_	4			
spotted black-s	tiff to hard (Fill)		_	2633	4.50 P	11						1.309	2.00 P	22	
/18/21											_		J.		
8.GPJ 11				7	4.50	15						7	3.30	21	
9-B LOO			-5	8	Р					589.3	-25	7	В		
GS/1907			_	3				SANDY CLAY LOAM (Stone-very stiff (Fill)	with	309.3		8			
RING LO				4 5	2.00 B	20					_	6 5	3.50 B	17	
O-B B-O		9						CLAY LOAM-brown &		586.8					
(74/1907			_	2	3.50	16		gray-medium stiff				3	0.60	2/	
(73&62)			-10	6	В	10		- LO(D : 0 00 f		584.8	-30	2	В	2-7	
ECTS 62k			_	4				End Of Boring @ -30.0 backfilled with cuttings	J'. Boring i.		_				
PROJE			_		2.20 B	18									
KENNED															
-WO#15				4	1.30	20									
192-007		·-	-15	7	В	20					-35				
, ibo1				4											
Z.PROJECTSV2019/19079-B EFK, IDOT 192-007-WO#15 KENNEDY PROJECTS 62K73&62K74/19079-B BDRING LOGS/19079-B_LOG.GPJ 11/18/21				4	3.50 B	21		•			_				
3/2019/15															
KOJECTS		*		4	3.00	19									
3.5.Z			-20	4	В						-40	-			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



USER NAME	=	DESIGNED	-	KJD	REVISED -
		CHECKED	-	MI	REVISED -
PLOT SCALE	=	DRAWN	-	KJD	REVISED -
PLOT DATE	=	DATE	-	04/29/2024	REVISED -

SOIL BORINGS II		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
	90/94	2020-005-BR		соок	908	269
				CONTRA	ACT NO.	62K73
SHEET OSG1-24 OF OSG1-24 SHEETS		ILLINOIS	FED. A	D PROJECT		



GENERAL NOTES

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES:

f'c = 3,500 p.s.i.

fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer.

The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel insert's and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not

ANCHOR RODS: Shall conform to ASTM F1554 Gr. 105.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Concrete Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

FOUNDATIONS: The contract unit price for Concrete Foundations and Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.

TOTAL BILL OF MATERIAL

TOTAL BILL OF MATERIAL		
ITEM	UNIT	TOTAL
CONCRETE BARRIER REMOVAL	Foot	21
CONCRETE REMOVAL	Cu. Yd.	3.2
STRUCTURE EXCAVATION	Cu. Yd.	7
CONCRETE BARRIER TRANSITION	Foot	9
OVERHEAD SIGN STRUCTURE SPAN TYPE I-A (4'-0" X 4'-6")	Foot	86
CONCRETE FOUNDATIONS	Cu. Yd.	18.1
DRILLED SHAFT CONCRETE FOUNDATIONS	Cu. Yd.	15.3
REMOVE OVERHEAD SIGN STRUCTURE - SPAN	Each	1
TEMPORARY SOIL RETENTION SYSTEM	Sq. Ft.	92
FAL		TOTAL SE

benesch 312-565-0450

Alfred Benesch & Compan 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 Job No. 10805.03

05-A-1

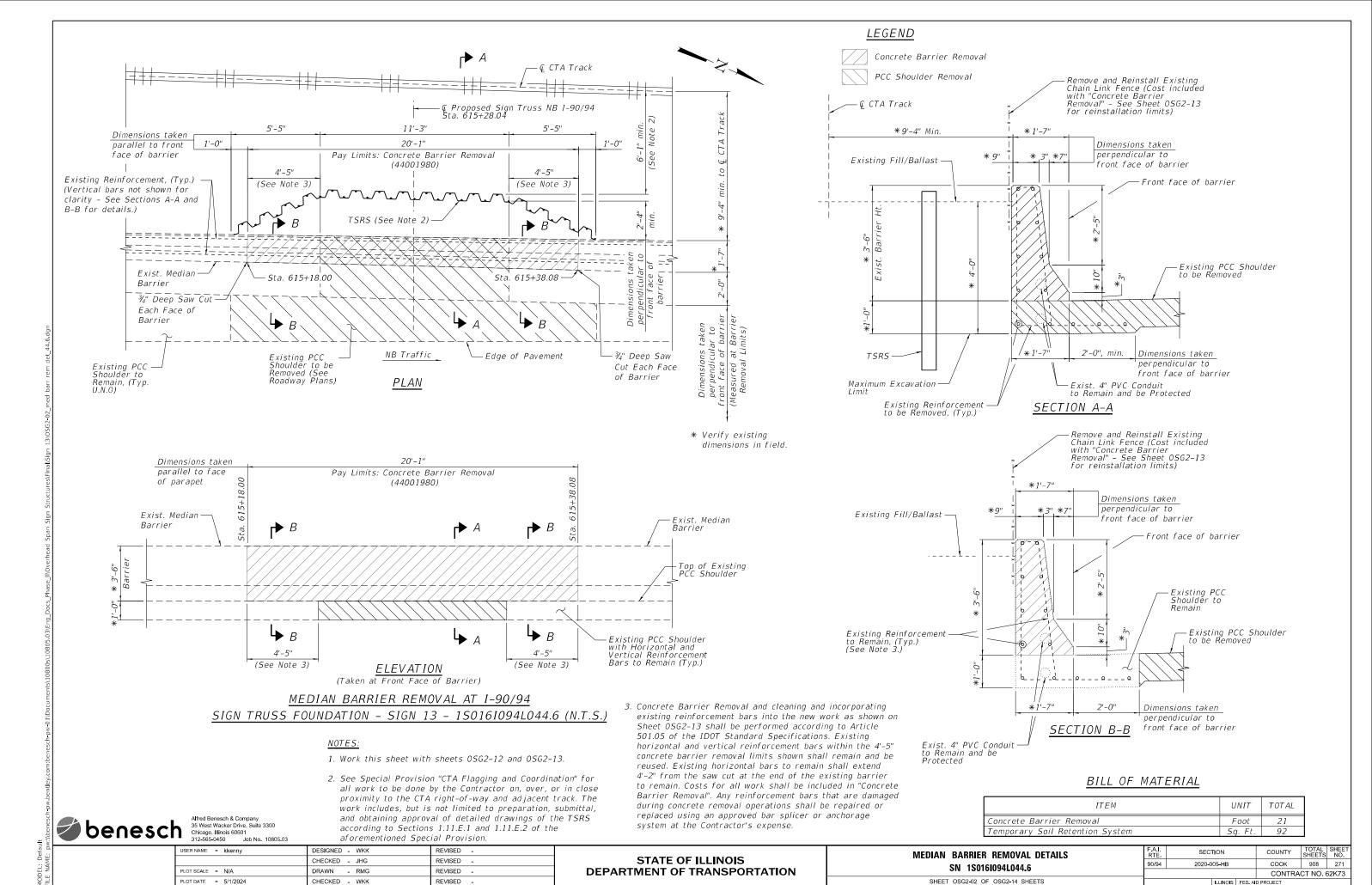
2-17-2023

DESIGNED - WKK REVISED SER NAME = kkenny CHECKED - JHG REVISED DRAWN - RMG REVISED PLOT DATE = 5/1/2024 CHECKED - WKK REVISED

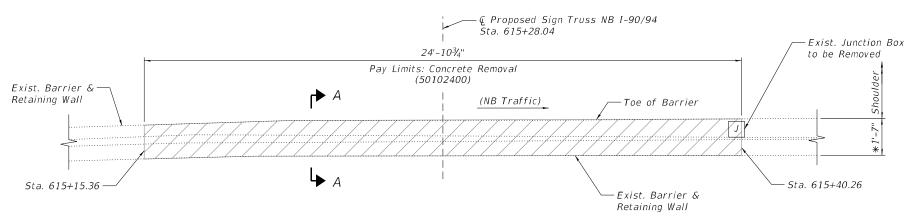
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

OSG2-14 (Total of 14 Sheets)

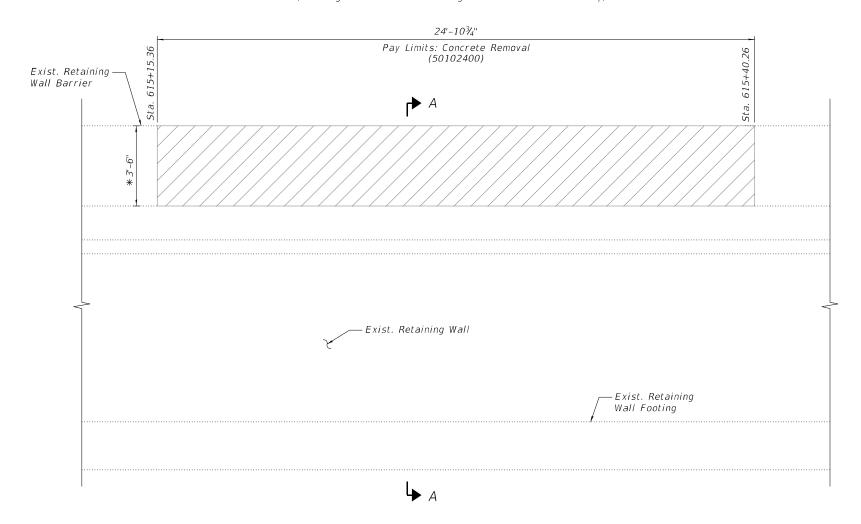
SHEETS NO. SECTION COUNTY GENERAL PLAN & ELEVATION - ALUMINUM TRUSS & STEEL SUPPORTS RTE. 2020-005-HB COOK 908 270 SN 1S016I094L44.6 CONTRACT NO. 62K73 SHEET OSG2-01 OF OSG2-14 SHEETS



5/1/2024 6:41:30 AM



(Existing Shoulder and Retaining Wall not shown for Clarity)



ELEVATION

RETAINING WALL REMOVAL AT 1-90/94 SIGN TRUSS FOUNDATION - SIGN 13 - 1S016I094L044.6

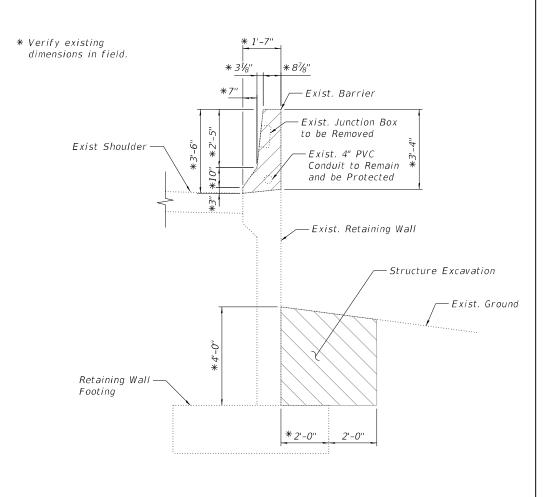
Note:

Work this Sheet with Sheet OSG2-11.

LEGEND

Concrete Removal

Structure Excavation



SECTION A-A

BILL OF MATERIAL

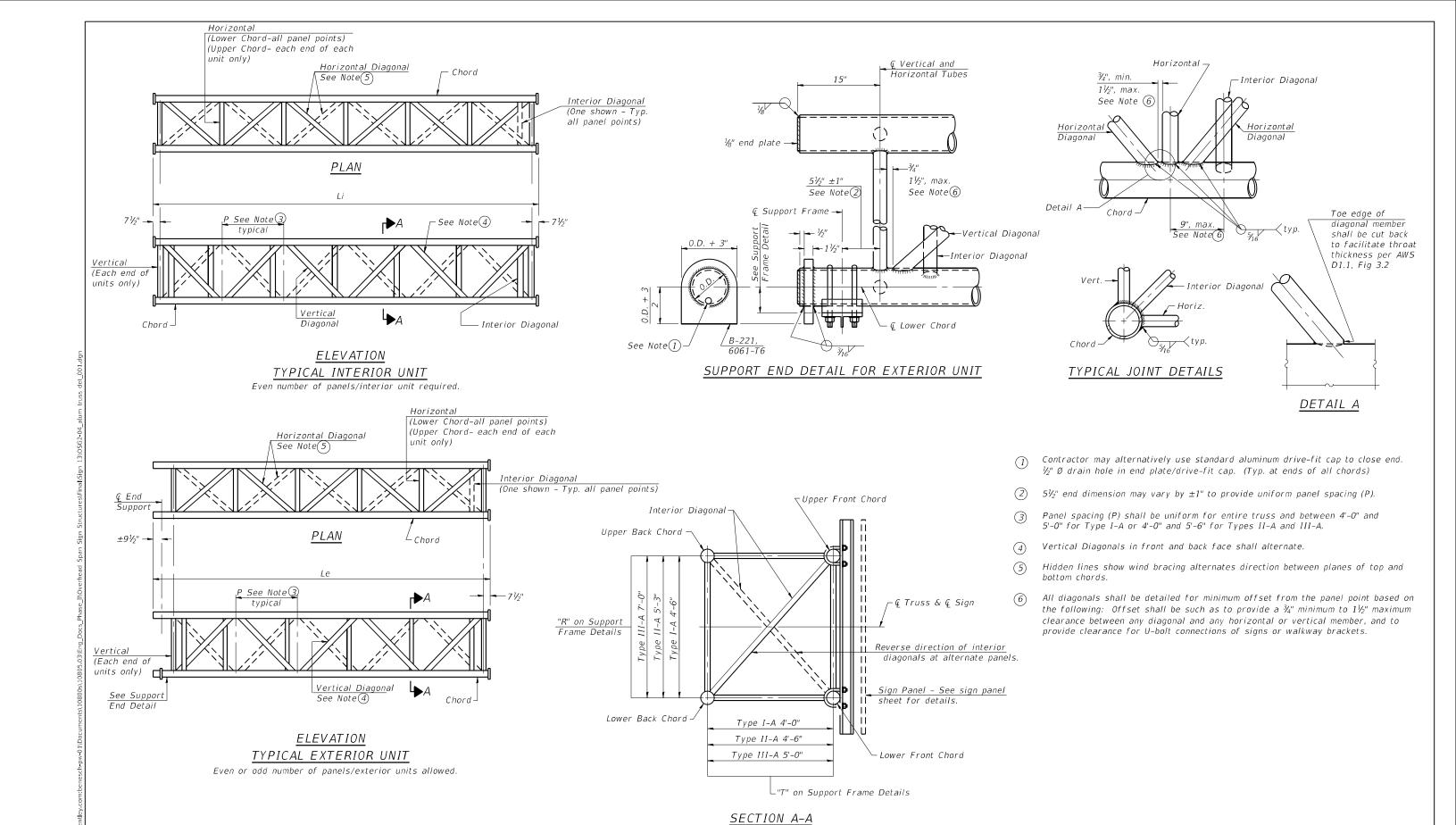
ITEM	UNIT	TOTAL
Concrete Removal	Cu. Yd.	3.2
Structure Excavation	Cu. Yd.	7



USER NAME = kkenny	DESIGNED - WKK	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE = N/A	DRAWN - RMG	REVISED -
PLOT DATE = 5/1/2024	CHECKED - WKK	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** **RETAINING WALL REMOVAL DETAILS** SN 1S016I094L044.6 SHEET OSG2-03 OF OSG2-14 SHEETS

SECTION COUNTY COOK 908 272 90/94 2020-005-HB CONTRACT NO. 62K73



DESIGNED - WKK REVISED CHECKED - JHG REVISED . DRAWN - RMG REVISED CHECKED - WKK PLOT DATE = 5/1/2024 REVISED .

2-17-2023

05-A-2

Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10808

Job No. 10805.03

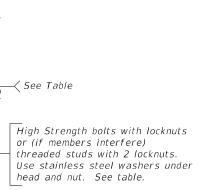
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ALUMINUM TRUSS DETAILS FOR TRUSS TYPES I-A, II-A AND III-A (1 OF 2) SHEET OSG2-04 OF OSG2-14 SHEETS

SECTION COUNTY 2020-005-HB COOK 908 273 CONTRACT NO. 62K73

benesch

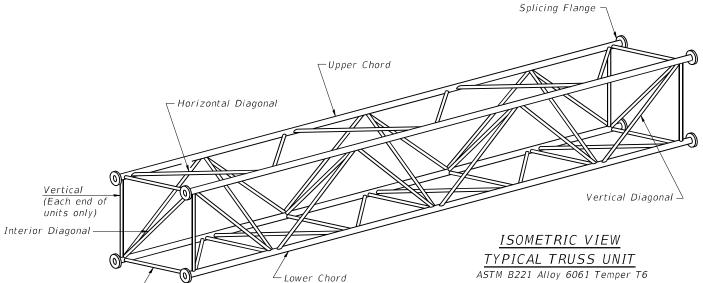
Sir	gn #	Structure	CI II	Design Truss	Exte	rior Units	(2)		Interio	or Unit		Upper & Cho		Vertical,	Horizontals; Horizontal,	Camber		Sį	olicing	Flange		
3/19	9" "	Number	Station	Tyna	No. Panels	Unit	Panel	No.	No. Panels	Unit	Panel	(),,		and Interio	or Diagonals	Midspan	∥ Bo	lts	Weld 3	Sizes	4	
				Type	per Unit	Lgth.(Le)	Lgth.(P)	Req'd.	per Unit	Lgth.(Li)	Lgth.(P)	0.D.	Wall	0.D.	Wall	Midspan	No./Splice	Dia.	W	W 1	Α	В
	13	15016I094L044.6	615+28.04 (NB)	I-A	6	29'-4½"	4'-7"	1	6	28'-9"	4'-7"	5"	⁵ / ₁₆ "	2½"	⁵ / ₁₆ "	21/2"	6	⁷ /8"	5/ ₁₆ "	1/4"	8¾"	11¾"



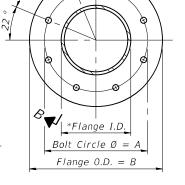
SECTION B-B

See Table & Note(1) WVQ

> 1) Splicing Flanges shall be attached to each truss unit with the truss shop assembled to camber shown. Truss units shall be in proper alignment and flange surfaces shall be shop bolted into full contact before welding. Sufficient external welds or tacks shall be made to secure flanges until remaining welds are made after disassembly. Adjacent flanges shall be "match marked" to insure proper field assembly.



Note: Units shall be shipped individually with adequate provision to prevent detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The Contractor is responsible for maintaining the configuration and protection of the units.



*Flange I.D.

TRUSS TYPES I-A, II-A, & III-A

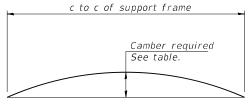
В◀ҵ

Drill 6 holes $\frac{1}{1_{16}}$ " larger than bolt diameter.

Drill 8 holes ⅓₆" larger than bolt diameter.

TRUSS TYPES II-A & III-A SPLICING FLANGES

ASTM B221, Alloy 6061-T6 or ASTM B209, Alloy 6061-T651 *To fit O.D. of Chord with maximum gap of $\frac{1}{16}$ ".



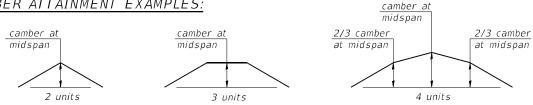
(Upper Chord - each end of each unit only)

(Lower Chord - all panel points)

CAMBER DIAGRAM

Camber curve shown is theoretical. Actual camber attained by slope changes at splices between units.

CAMBER ATTAINMENT EXAMPLES:



Camber shown is for fabrication only, measured with truss fully supported. (No-load condition)



Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10808

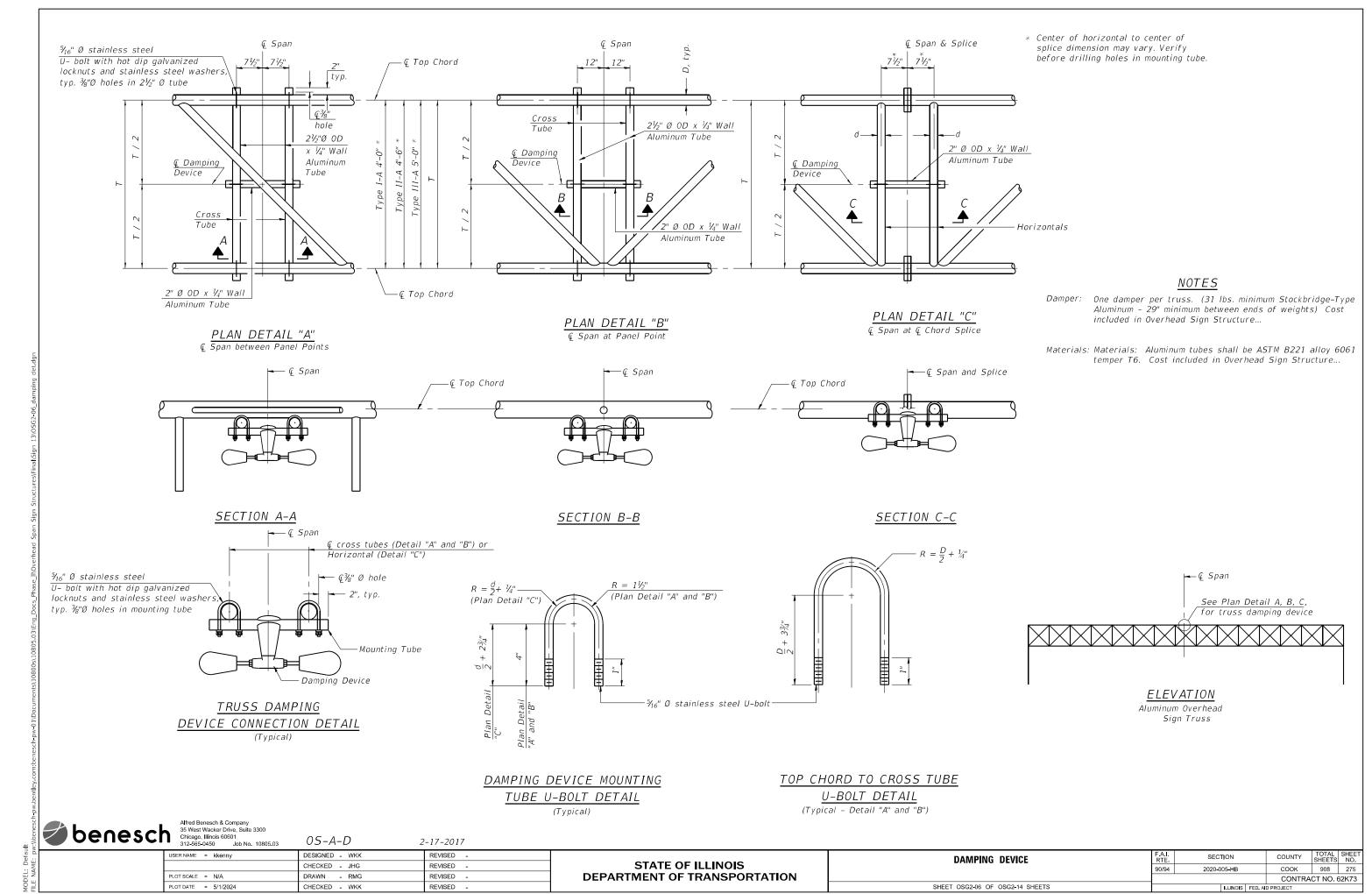
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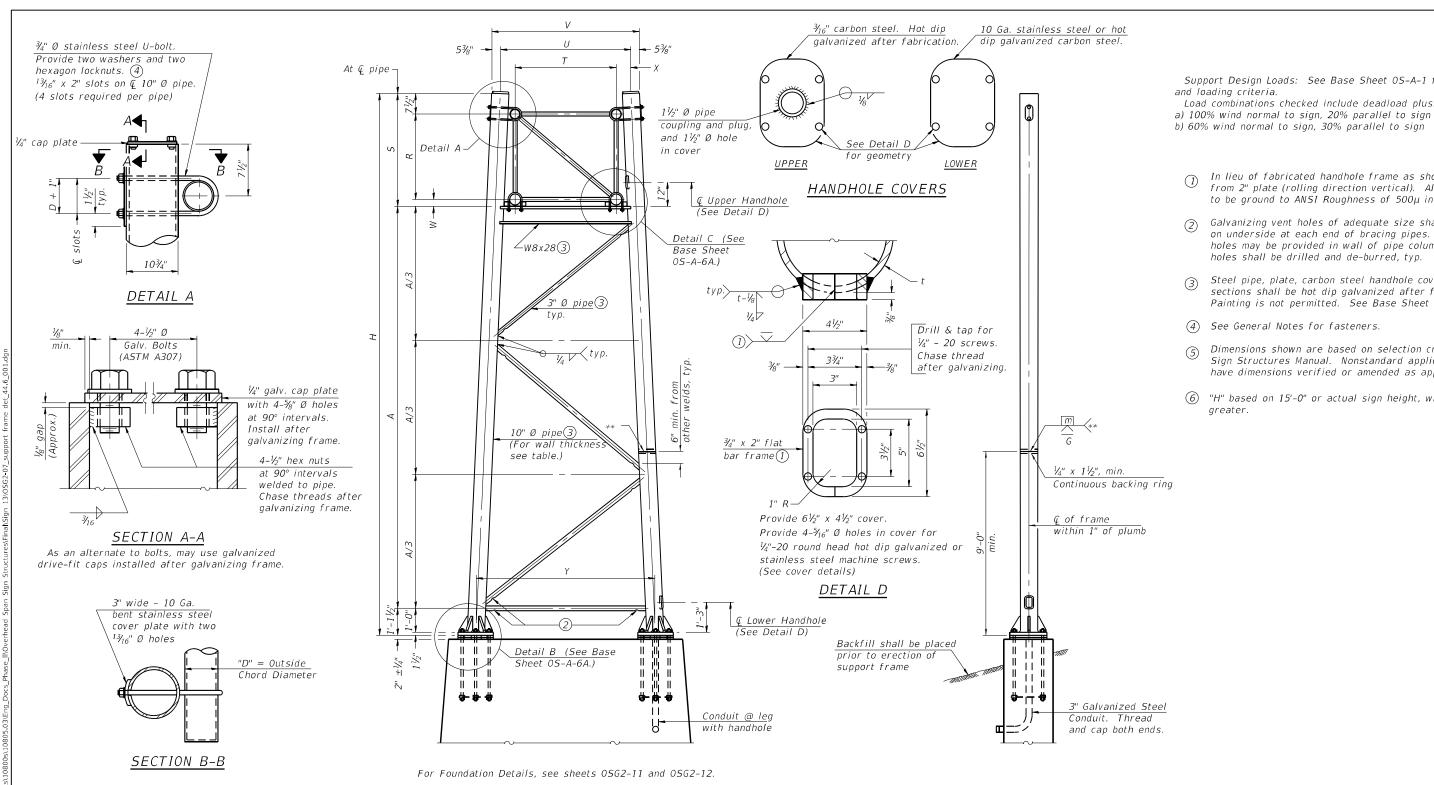
2-17-2017

312-303-0430 300 NO. 10003.03		. ==17
USER NAME = kkenny	DESIGNED - WKK	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE = N/A	DRAWN - RMG	REVISED -
PLOT DATE = 5/1/2024	CHECKED - WKK	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

ALUMINUM TRUSS DETAILS FOR F.A.I. SECTION COUNTY SHEETS NO. TRUSS TYPES I-A, II-A AND III-A (2 OF 2) 90/94 2020-005-HB COOK 908 274					
ALUMINUM TRUSS DETAILS FOR	F.A.I. RTE	SECTION	COUNTY		
TRUCS TYPES I_A II_A AND III_A (2 OF 2)	90/94	2020-005-HB	соок	908	274
111003 111 LO 1-A, 11-A AND 111-A (2 01 2)			CONTRA	CT NO.	NO. 274
SHEET OSG2-05 OF OSG2-14 SHEETS		ILLINOIS FED. AI	D PROJECT		





SIDE ELEVATION

Truss				Dimen	sions			
Туре	R	5	T	U	V	W	X	Υ
I-A	4'-6"	5'-5 ¹ / ₂ "	4'-0"	5'-6"	6'-43/4"	4"	9"	8'-3"
II-A (5)	5'-3"	6'-31/4"	4'-6"	6'-1"	6'-11¾"	4¾"	91/2"	8'-3"

10" Ø PIPE TRUSS SUPPORT FRAME

** One butt welded joint is allowed only on one post per support frame. If used, weld procedure must be preapproved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

Sign #	Structure	Station	Support		Truss	Pipe Wall	Н		
Jigii #	Number	Station	Left	Right	Type	Thickness	6	Α	
Sign 13	150161094L044.6	615+28.04 (NB)	1		I-A	0.279"	29.17'	22.59'	
Sign 13	15016I094L044.6	615+28.04 (NB)		1	I-A	0.279"	29.50'	22.92'	

Support Design Loads: See Base Sheet OS-A-1 for design

In lieu of fabricated handhole frame as shown, may cut from 2" plate (rolling direction vertical). All cut faces

to be ground to ANSI Roughness of 500µ in or less.

2) Galvanizing vent holes of adequate size shall be provided on underside at each end of bracing pipes. Alternately,

3 Steel pipe, plate, carbon steel handhole covers and rolled sections shall be hot dip galvanized after fabrication.

Painting is not permitted. See Base Sheet OS-A-1.

Dimensions shown are based on selection criteria in the

Sign Structures Manual. Nonstandard applications must

have dimensions verified or amended as appropriate.

(6) "H" based on 15'-0" or actual sign height, whichever is

holes shall be drilled and de-burred, typ.

(4) See General Notes for fasteners.

areater.

holes may be provided in wall of pipe column. All vent

Load combinations checked include deadload plus:



05-A-6

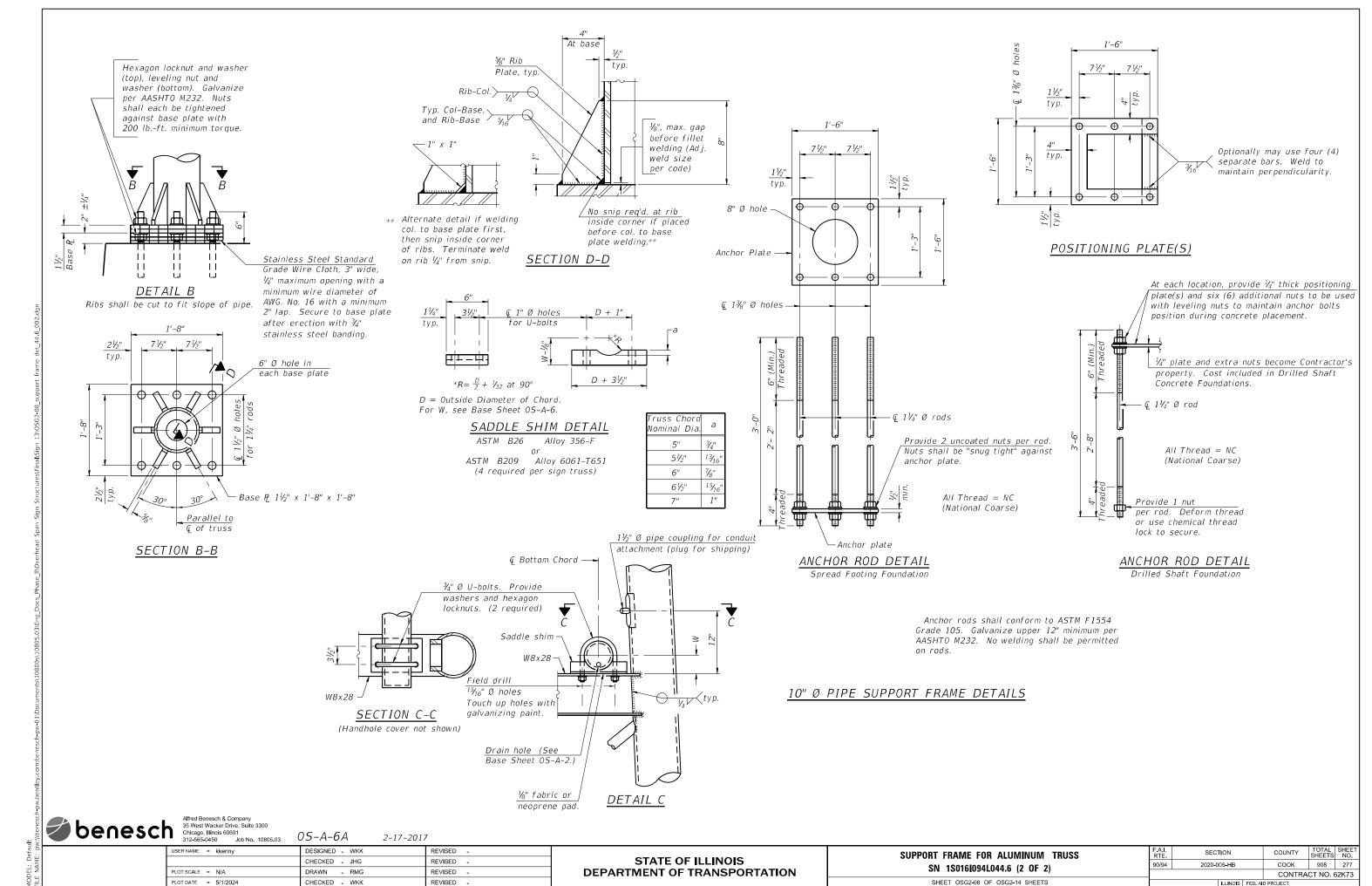
2-17-2017

USER NAME = kkenny	DESIGNED - WKK	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE = N/A	DRAWN - RMG	REVISED -
PLOT DATE = 5/1/2024	CHECKED - WKK	REVISED -

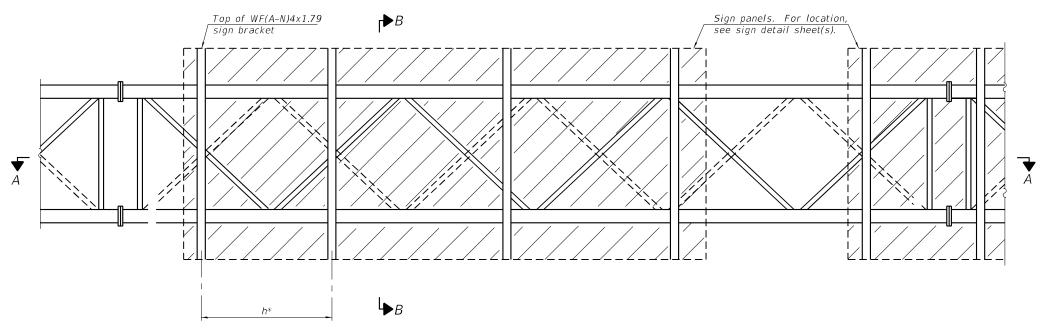
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SUPPORT FRAME FOR ALUMINUM TRUSS	F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
SN 1S016I094L044.6 (1 OF 2)	90/94	2020-005-HB		соок	908	276
3N 130101094L044.0 (1 OF 2)				CONTRA	CT NO. 6	32K73
SHEET OSG2-07 OF OSG2-14 SHEETS		ILLINOIS	FED AIR	PROJECT		

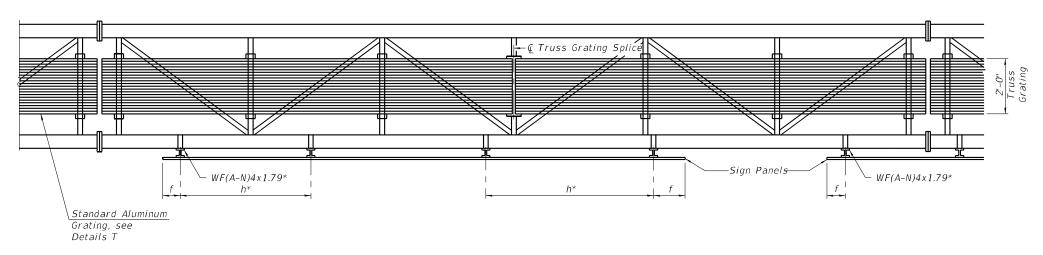
END ELEVATION



5/1/2024 6:41:59 AM



TYPICAL FRONT ELEVATION



SECTION A-A

Place all sign brackets as close to panel points as practical.

* Space sign brackets WF(A-N)4x1.79 for efficiency and within limits

BRACKET TABLE WF(A-N)4x1.79ASTM B308, Alloy 6061-T6

> Equal To 8'-0"

14'-0"

20'-0"

26'-0"

32'-0"

Number

Required

4

Less Than or Brackets

Sign Width

Greater Than

8'-0"

14'-0"

20'-0"

26'-0"

f=12" maximum, 4" minimum (End of sign to $\mathcal Q$ of nearest bracket) h=6'-0" maximum ($\mathcal Q$ to $\mathcal Q$ sign support brackets, WF(A-N)4x1.7

shown:

For Detail T and Section B-B, see Base Sheet OS-A-10-NW. Truss grating to facilitate inspection shall run full length (center to center of support frames) 12"± on overhead trusses. Cost of truss grating is included in "Overhead Sign Structure".

Truss Grating width dimensions are nominal and may vary ½"± based on available standard widths.

Hered Benesch & Company
35 West Wacker Drive, Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 10805.03 05-A-9-NW

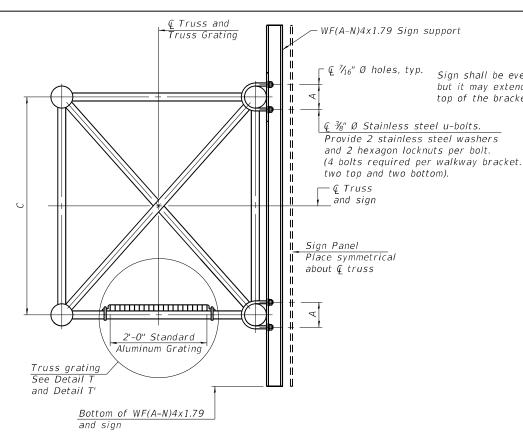
4-1-2020

USER NAME = kkenny	DESIGNED - WKK	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE = N/A	DRAWN - RMG	REVISED -
PLOT DATE = 5/1/2024	CHECKED - WKK	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** **ALUMINUM WALKWAY DETAILS** SN 1S016[094L044.6 (1 OF 2) SHEET OSG2-09 OF OSG2-14 SHEETS

SECTION COUNTY 2020-005-HB COOK 908 278 CONTRACT NO. 62K73

5/1/2024 6:42:03 AM



SECTION B-B

SPECIFICATIONS FOR STANDARD ALUMINUM GRATING

Main Bearing Bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on $1\frac{3}{16}$ " centers and conform to ASTM B221 Alloy 6061-T6.

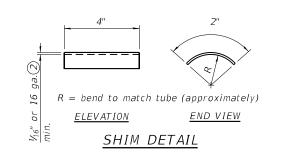
Cross bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on 4" centers and conform to ASTM B221 Alloy 6063-T5 or 6061-T6.

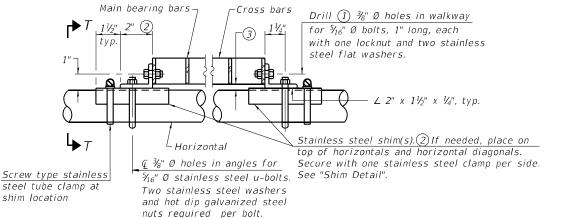
Aluminum Grating with modified "t" sections for main bearing bars shall meet the following requirements:

Main bars shall conform to ASTM B221 Alloy 6061-T6 and have a minimum section modulus equal to 0.0705 in.3 per bar, a depth of $1\frac{1}{2}$ ", spaced on $1\frac{3}{16}$ " centers.

Cross bars shall conform to ASTM B221 Alloy 6063-T5 or T-42 and spaced on 4" centers.

Sign #	Structure Number	Station	А	С
Sign 13	1S016I094L044.6	615+28.04 (NB)	5¾"	4'-6"





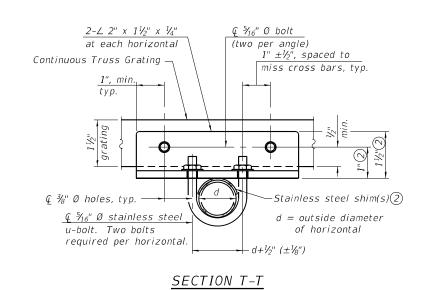
DETAIL T

U-bolt and angle connections

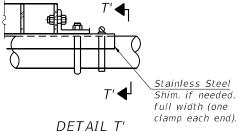
required at horizontals only.

Sign shall be even with the top of the bracket, but it may extend no more than 6" above the top of the bracket for field adjustments.

(Continuous Truss grating)

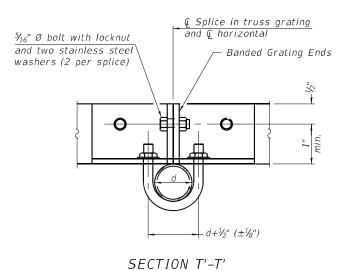


- 1) Drilling holes in grating may be done in shop or field, based on Contractor's preference and subject to accurate alignment.
- (2) Stainless steel shims shall be placed as shown in Detail T <u>if needed</u> to compensate for alignment variations between horizontal and diagonal pipes beyond adjustment provided by angles. Thicker shims may be used subject to shims performing properly.
- 3 Tube to grating gap may vary from 0 to ½", max. to align walkway, allow for camber, etc.



(Truss grating splice)

Details not shown same as Detail T. Alternate materials may be used subject to the Engineer's review and approval.



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Chicago, Illinois 60601

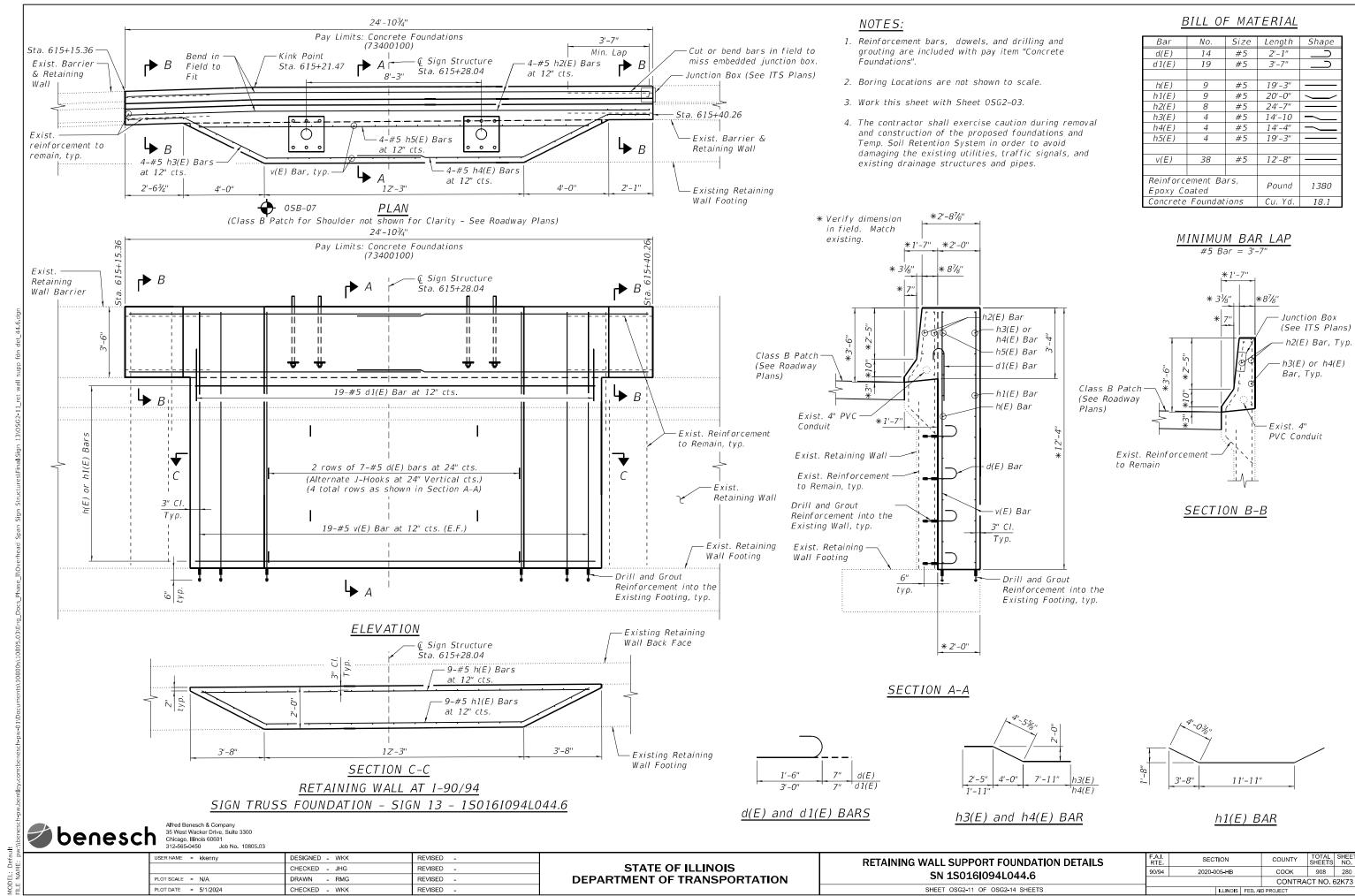
05 1 10 NW

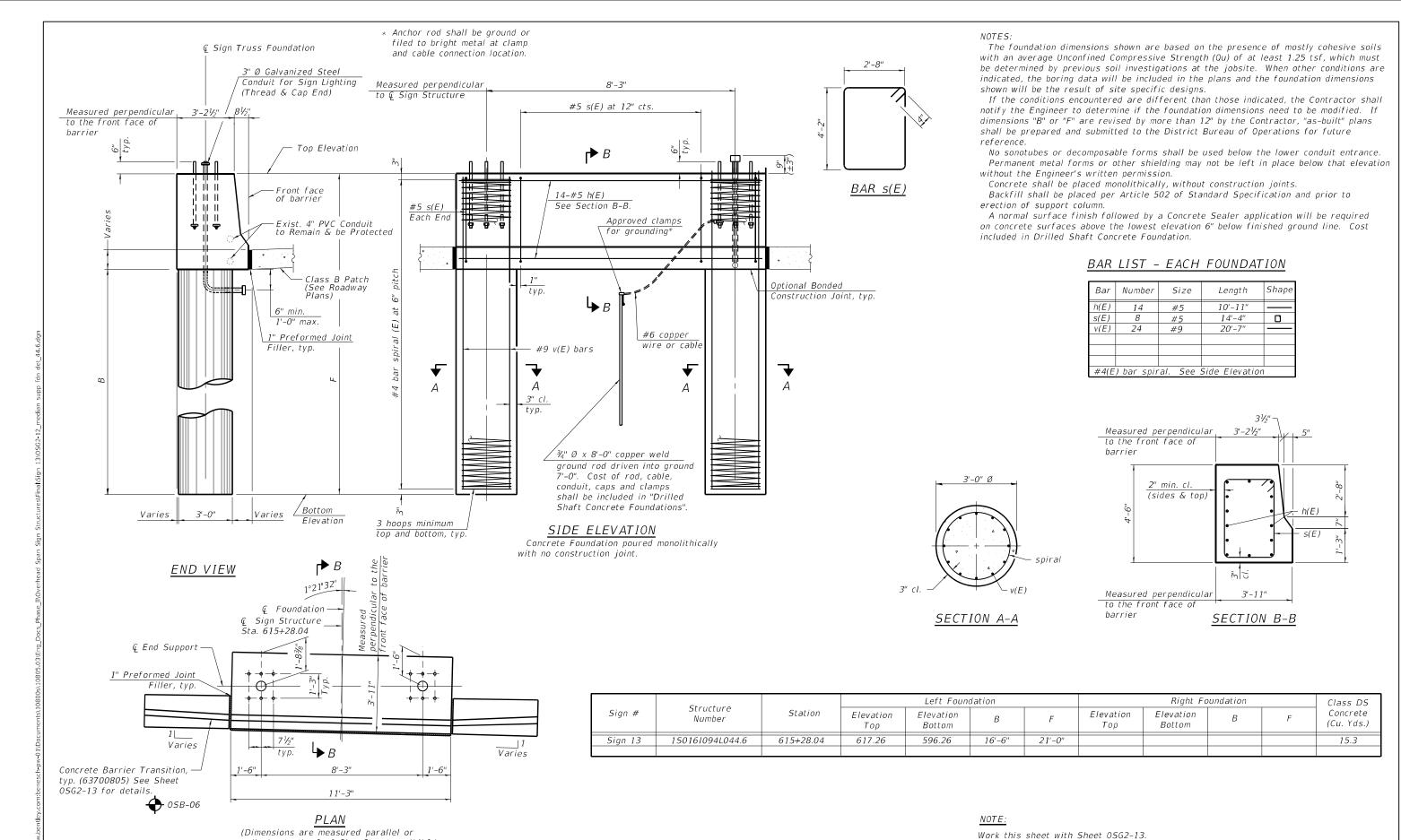
<u> </u>	312-565-0450 Job No. 10805.03	3-A-10-NW 4-1	-2020
	USER NAME = kkenny	DESIGNED - WKK	REVISED -
		CHECKED - JHG	REVISED -
	PLOT SCALE = N/A	DRAWN - RMG	REVISED -
	PLOT DATE = 5/1/2024	CHECKED - WKK	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ALUMINUM	WALKWAY DETAILS
SN 1S016[0)94L044.6 (2 OF 2)
SHEET OSG2-1	IO OF OSG2-14 SHEETS

SECTION COUNTY 90/94 2020-005-HB COOK 908 279 CONTRACT NO. 62K73





DEPARTMENT OF TRANSPORTATION

perpendicular to the Q of Sign Structure U.N.O.)
Alfred Benesch & Company
35 West Wacker Drive, Suite 3300
Chicago, Illinois 60601
312-865-0450 Job No. 10805.03

DRAWN

- RMG

CHECKED - WKK

SER NAME = kkenny

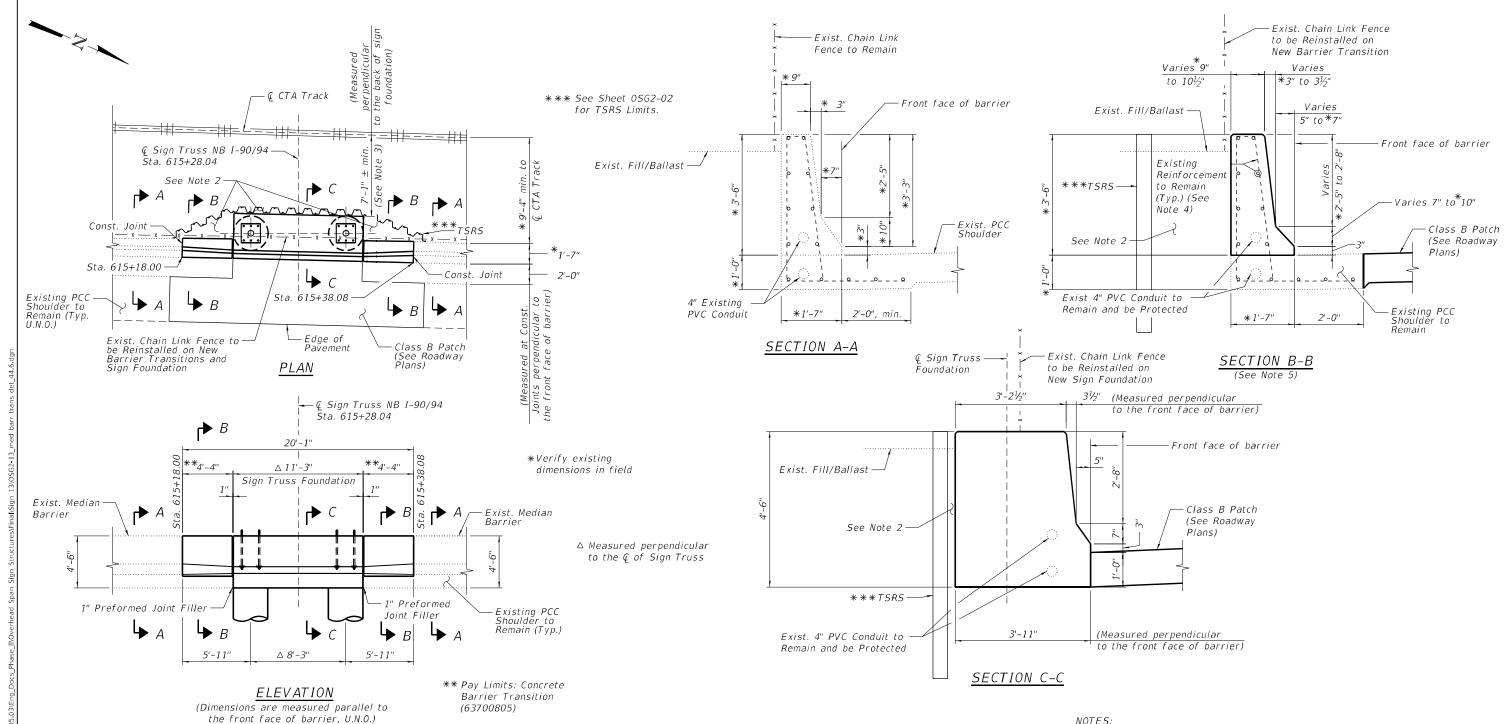
PLOT DATE = 5/1/2024

REVISED

REVISED

MEDIAN SUPPORT FOUNDATION DETAILS
SN 1S016I094L044.6
SHEET OSG2-12 OF OSG2-14 SHEETS

benesch



NOTES:

- 1. Work this sheet with Sheets OSG2-02 and OSG2-12.
- 2. Existing Fill/Ballast removed to construct median barrier transitions and sign foundation shall be placed and compacted behind the completed median barrier transitions and sign foundation to the original Fill/Ballast elevation. Cost included with "Concrete Barrier Transition".
- 3. See Special Provision "CTA Flagging and Coordination" for all work to be done by the Contractor on, over, or in close proximity to the CTA right-of-way and
- 4. Existing reinforcement bars shall be cleaned and incorporated into the proposed concrete barrier transitions. See Sheet OSG2-02 for additional information.
- 5. The Concrete Barrier Transitions shall be constructed according to Section 637 of the IDOT Standard Specifications. A Protective Coat shall be applied to the top and vertical surfaces of the Concrete Barrier Transitions exposed to traffic according to Article 637.10 of the IDOT Standard Specifications. Cost included with "Concrete Barrier Transition".

ITEM	UNIT	TOTAL
Concrete Barrier Transition	F00T	9

BILL OF MATERIAL

MEDIAN BARRIER TRANSITIONS AT

I-90/94 SIGN TRUSS FOUNDATION - SIGN 13 - 1S016I094L044.6 (N.T.S.)



Job No. 10805.03

USER NAME = kkenny	DESIGNED - WKK	REVISED -
	CHECKED - JHG	REVISED -
PLOT SCALE = N/A	DRAWN - RMG	REVISED -
PLOT DATE = 5/1/2024	CHECKED - WKK	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION MEDIAN BARRIER TRANSITION DETAILS 90/94 2020-005-HB SN 1S016I094L044.6 SHEET OSG2-13 OF OSG2-14 SHEETS

COUNTY COOK 908 282 CONTRACT NO. 62K73

GSI Job No. __19079-B

6 1.00 22

SOIL BORING LOG

End Of Boring @ -30.0'. Boring backfilled with cuttings.

Page <u>1</u> of <u>1</u> Date 10/23/21

PTB 185-012, WO #32 PROJECT I-90 & I-94 Tollway LOCATION

10.0" CONCRETE

DRILLING METHOD Hollow Stem Auger D B U M BY P O S I DEST

CLAY LOAM-brown & gray-medium stiff to stiff (Fill) CRUSHED ASPHALT & STONE-very dense (Fill) 3 1.20 21 CLAY LOAM-brown & gray-stiff to very stiff (Fill) becoming gray @ -23.0' 2 0.80 25 2 0.60 22 4 6 2.00 17 -10 5 B

4 3.50 19 7 P SILTY CLAY-brown & gray-stiff to very stiff (Fill)

The Unconfined Compressive Strength (UCS) Fallure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)

benesch

Alfred Benesch & Company 35 West Wacker Drive, Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 10805.03

SER NAME = kkenny DESIGNED - WKK REVISED -CHECKED - JHG REVISED -DRAWN - RMG REVISED -PLOT DATE = 5/1/2024 CHECKED - WKK REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SOIL BORING LOG SN 1S016I094L044.6 SHEET OSG2-14 OF OSG2-14 SHEETS

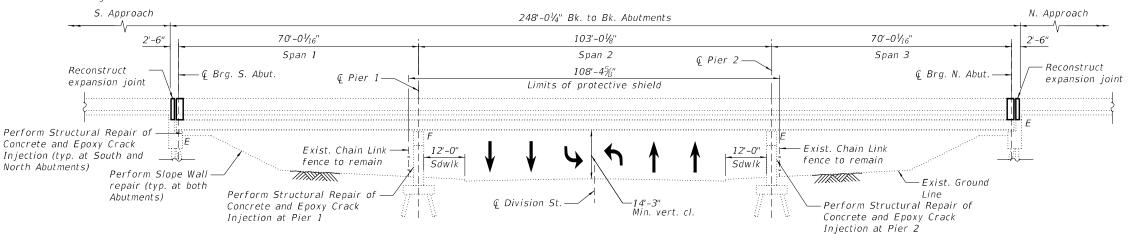
SECTION COUNTY COOK 908 283 90/94 2020-005-HB CONTRACT NO. 62K73

5/1/2024 6:42:38 AM

Existing Structure: S.N. 016-0135 was originally built in 1958. The bridge was widened between 1990 and 1993, and expansion joint repairs were performed in 2013. The structure has a back-to-back abutment length of 248'-0¼" and an out-to-out deck width that varies from 70'-9¾" to 74'-3½". The superstructure consists of a 7½" thick reinforced concrete deck supported on three span continuous steel beams of span lengths $70'-0\frac{1}{16}''$, $103'-0\frac{1}{16}''$, and $70'-0\frac{1}{16}''$. The substructure consists of reinforced concrete abutments and piers supported on reinforced concrete drilled shafts.

Traffic is to be maintained utilizing stage construction.

No salvage.



ELEVATION

DESIGN SPECIFICATIONS

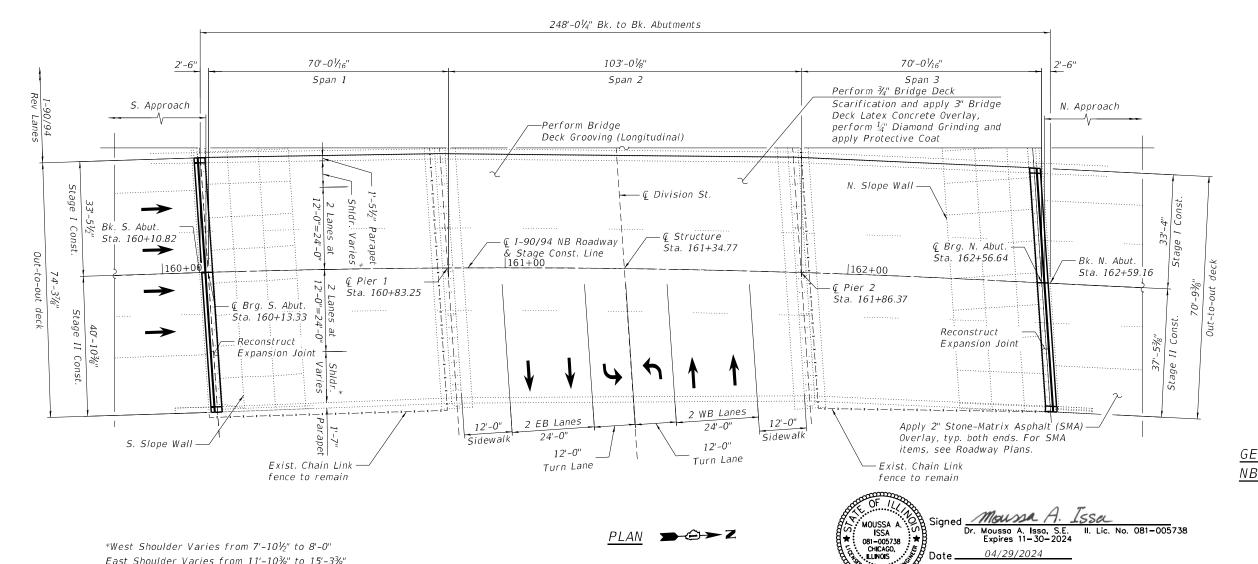
2002 AASHTO Standard Specification for Highway Bridges, 17th Edition

RECONSTRUCTION 1993

1989 AASHTO Standard Specifications with 1990 & 1991 Interim

NOTES:

- All stations are to the © I-90/94 NB Roadway and taken from existing plans.
- 2. No future wearing surface is allowed.



LOCATION SKETCH GENERAL PLAN AND ELEVATION NB I-90/94 OVER DIVISION ST. F.A.I. ROUTE 90/94

Range 14E, 3rd P.M.

Structure

Location

SECTION 2020-005-BR COOK COUNTY STATION 161+34.77

S.N. 016-0135 (NB)

DESIGNED - LAB, HMI REVISED . SER NAME = CHECKED - MI REVISED . LAB, HMI REVISED 4/29/2024 REVISED

East Shoulder Varies from 11'-10\%" to 15'-3\%"

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

STRUCTURE NO. 016-0135 (NB) SHEET S01-01 OF S01-22 SHEETS

For Sheets S01-01thru S01-22

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2020-005-BR		соок	908	284
			CONTRA	ACT NO.	62K73
	ILLINOIS	FED. A	D PROJECT		

GENERAL NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Calculated weight of Structural Steel = 1,000 lb (M270 Grade 36) 100 lb (M270 Grade 50)
- 3. Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 4. Bars noted thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bars per line.
- 5. All exposed concrete edges shall have a 3/4"x45° chamfer except where shown otherwise.
- 6. Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.
- 7. For SMA overlay on Approach Slab, see Roadway Sheets.
- 8. Protective Coat shall be applied to the top of reconstructed transverse joint areas, top and inside faces of parapets, and top of Latex Concrete Overlay.
- 9. Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.
- 10. Prior to pouring the new concrete deck for expansion joint reconstruction and deck slab repairs, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding ¼ deep shall be identified and reported to the Bureau of Bridges and Structures for further dispositions. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 11. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 12. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".
- 13. All new structural steel shall be hot-dip galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".
- 14. Fasteners shall be ASTM F 3125 Grade A325 Type 1. Fasteners shall be hot dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel." Bolts $\frac{3}{4}$ in. diameter, holes $\frac{13}{16}$ in. diameter, unless otherwise noted.
- 15. The Contractor is responsible to remove, support and reinstall all existing electrical conduits interfering with the work. See special provision "Protection and Maintenance of Existing Underpass Luminaires".
- 16. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 17. The Contractor is responsible to protect the existing conduit and junction box embedded in the parapet during concrete removal and construction. Any damage to the existing conduit and junction box shall be repaired by the Contractor at no additional cost to the Department.
- 18. Where underpass lighting is present on the structure, the Contractor shall adjust the Protective Shielding to ride above the existing lighting fixtures in order to maintain the existing level of lighting on the roadway underneath. Details shall be approved by the Engineer before installation.
- 19. Any adjustment done to the Protective Shield System must not change the load carrying capacity (or containment specifications) as indicated in the Standard Specifications. Cost of adjusting shielding is including in the cost of Protective Shield.or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 20. No field welding is permitted except as specified in the contract documents.
- 21. Adjacent I-90/94 reversible bridge is not shown throughout the plans for clarity.

INDEX OF SHEETS

1-01	General	Plan an	d Elevat	ion		
11 02	Canaral	Matac	Inday of	Chasta	_	TDO

501-02	General	Notes,	Index	of	Sh.	eets	&	TBC
CO1 03	C+			· /			21	

501-03 Stage Construction (Sheet 1 of 2) 501-04 Stage Construction (Sheet 2 of 2)

S01-05 Temporary Concrete Barrier

501-06 Deck Repair Plan

S01-07 S. Abut. Joint Removal & Reconstruction (Sht. 1 of 3)

501-08 S. Abut. Joint Removal & Reconstruction (Sht. 2 of 3) 501-09 S. Abut. Joint Removal & Reconstruction (Sht. 3 of 3)

501–09 S. Abut. Joint Removal & Reconstruction (Sht. 3 of 3) 501–10 N. Abut. Joint Removal & Reconstruction (Sht. 1 of 3)

S01-11 N. Abut. Joint Removal & Reconstruction (Sht. 2 of 3) S01-12 N. Abut. Joint Removal & Reconstruction (Sht. 3 of 3)

S01-13 Preformed Joint Strip Seal

S01-14 Framing Plan

S01-15 Structural Steel Repair Details (Sheet 1 of 2)

S01-16 Structural Steel Repair Details (Sheet 2 of 2)

S01-17 South Abutment Repairs

501-18 North Abutment Repairs

S01-19 Pier 1 Repairs S01-20 Pier 2 Repairs

501-21 Slope Wall Repairs

S01-22 Bar Splicer Assembly and Mechanical Splicer Details

SCOPE OF WORK

- 1. Provide Protective shield within limits indicated on the plans.
- 2. Scarify $\frac{3}{4}$ " from the bridge deck slab.
- 3. Perform Deck Slab Repairs.
- 4. Reconstruct Expansion Joints at the South and North abutments and install new preformed joint strip seals.
- 5. Apply a 3" Bridge Deck Latex Concrete Overlay on Bridge Deck
- 6. Perform ¼" Diamond Grinding to top of bridge deck and abutment hatched block.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay on the approach Slabs, see Roadway Plans.
- 8. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- 9. Apply protective coat to the top of reconstructed transverse joint areas, top and inside faces of parapets, and top of Latex Concrete Overlay.
- 10. Replace diaphragm as shown on the plans.
- 11. Perform structural concrete repairs and epoxy crack injection for the abutments and piers as noted on the plans.
- 12. Perform Slope Wall repairs.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	16	16
Concrete Removal	Cu Yd	29.6	-	29.6
Slope Wall Removal	Sq Yd	-	51	51
Protective Shield	Sq Yd	877	=-	877
Concrete Superstructure	Cu Yd	32.3	=	32.3
Protective Coat	Sq Yd	2,116	-	2,116
Furnishing And Erecting Structural Steel	Pound	1,000		1,000
Reinforcement Bars, Epoxy Coated	Pound	4,660	-	4,660
Bar Splicers	Each	36	-	36
Slope Wall 4 Inch	Sq Yd	-	51	51
Preformed Joint Seal 2 1/2"	Foot	245		245
Preformed Joint Strip Seal	Foot	146	-	146
Concrete Sealer	Sq Ft	-	605	605
Epoxy Crack Injection	Foot		27	27
Slope Wall Crack Sealing	Foot	-	49	49
Bridge Deck Grooving (Longitudinal)	Sq Yd	1,305	-	1,305
Protect And Maintain Existing Underpass Luminaire	L Sum	-	0.04	0.04
Approach Slab Repair (Full Depth)	Sq Yd	49	_	49
Approach Slab Repair (Partial Depth)	Sq Yd	49	-	49
Structural Steel Removal	Pound	1,000	_	1,000
Structural Steel Repair	Pound	100	-	100
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	1,844	-	1,844
Bridge Deck Scarification 3/4"	Sq Yd	1,844	-	1,844
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	-	118	118
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	-	50	50
Deck Slab Repair (Full Depth, Type I)	Sq Yd	0.6	-	0.6
Deck Slab Repair (Full Depth, Type II)	Sq Yd	23.6	-	23.6
Diamond Grinding (Bridge Section)	Sq Yd	1,891	-	1,891
Locks for Gates	Éach	-	4	4

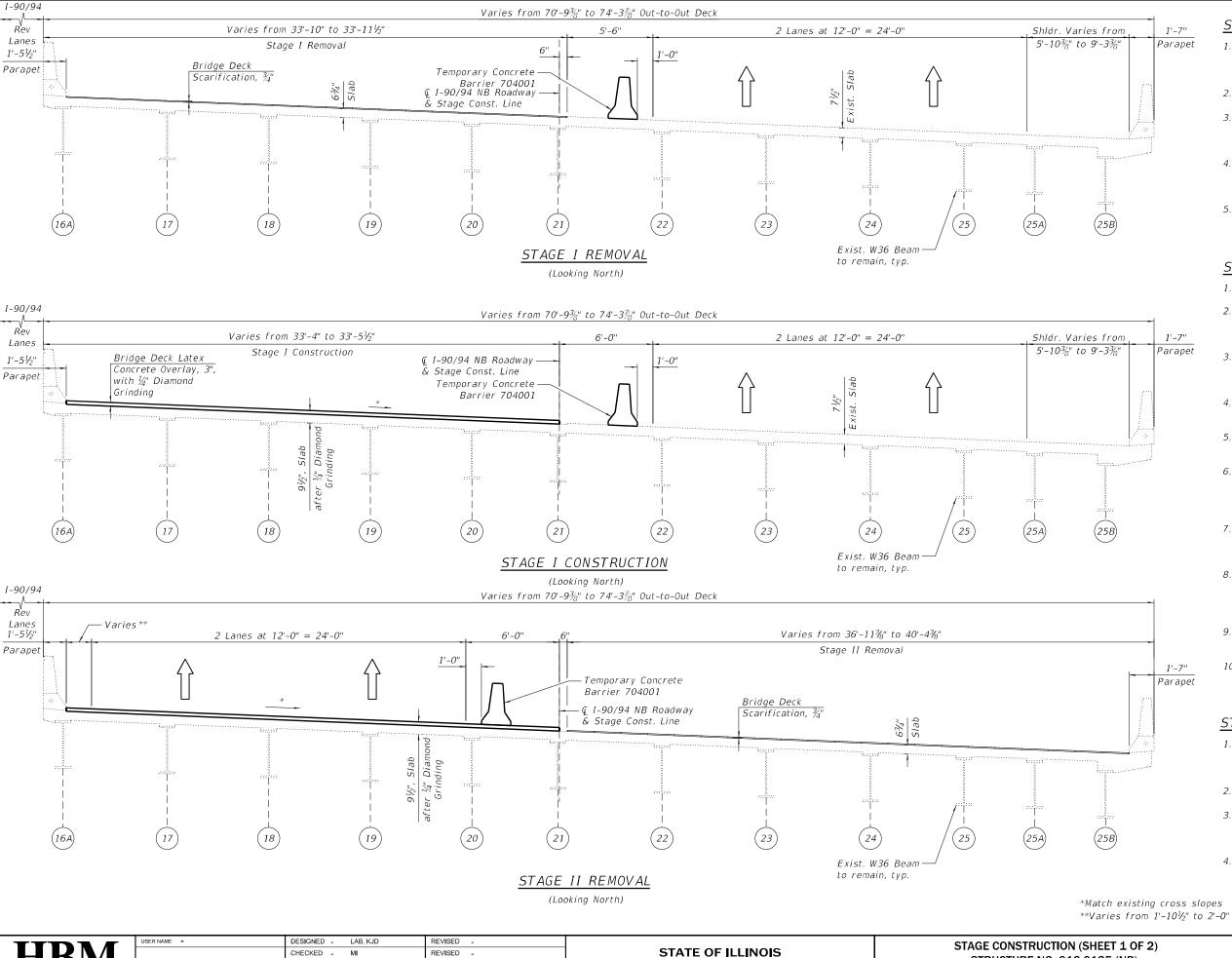
GENERAL NOTES (CONT.)

- 22. The Contractor shall contact Chandra Libby, the Director of City of Chicago Department of Family Support Services (DFSS) at 312-746-5443 or Chandra.Libby@cityofchicago.org to coordinate the relocation of persons and their personal belongings under the bridges.
- 23. The intent of the temporary fence is to deny access of any unauthorized personnel under the bridge during construction. Actual fence installations may vary from what is shown on the plans. All fence installations must be approved by the Engineer.
- 24. Concrete Sealer shall be applied to the designated areas of the abutments.
- 25. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See special provision for Debris Removal.



USER NAME =	DESIGNED	-	LAB, AMS	REVISED	-
	CHECKED	-	MI	REVISED	-
PLOT SCALE =	DRAWN	-	LAB, AMS	REVISED	-
PLOT DATE =	DATE	-	4/29/2024	REVISED	-

GENERAL NOTES, INDEX OF SHEETS & TBOM	F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-0135 (NB)	90/94	2020-005-BF	₹	соок	908	285
311(00101/L 1(0: 010-0133 (1(b)				CONTR	ACT NO.	62K73
SHEET S01-02 OF S01-22 SHEETS		ILLIN	OIS FED. A	ID PROJECT		



STAGE I REMOVAL

- Install temporary concrete barrier as shown to locate traffic on the east side of the existing structure.
 - 2. Perform 3/4" bridge deck scarification.
- 3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the abutments.
- 5. Remove existing longitudinal preformed joint seal between west parapet and reversible lane parapet.

STAGE I CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct expansion joints and install new preformed joint strip seals within the limits of Stage I Construction.
- 3. Perform Structural Repair of Concrete and Epoxy Crack Injection for abutments and Piers.
- 4. Apply 3" Bridge Deck Latex Concrete Overlay.
- 5. Perform 1/4" Diamond Grinding to bridge deck and abutment hatched block.
- 6. Perform Bridge Deck Grooving (Longitudinal) for the 3" Bridge Deck Latex Concrete Overlay and reconstructed abutment expansion joint areas.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 8. Apply protective coat to top and inside faces of west parapet, reconstructed abutment expansion joints and to the surfaces of the new overlay.
- 9. Perform slope wall repairs as shown on the plans.
- 10. Replace existing longitudinal preformed joint seal between west parapet and reversible lane parapet.

STAGE II REMOVAL

- 1. Install temporary concrete barrier as shown to locate traffic on the west side of the existing structure.
- 2. Perform ¾" bridge deck scarification.
- 3. Remove areas of existing deck for full-depth deck slab repairs at locations shown in the plans.
- 4. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the abutments.

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

LAB, KJD

4/29/2024

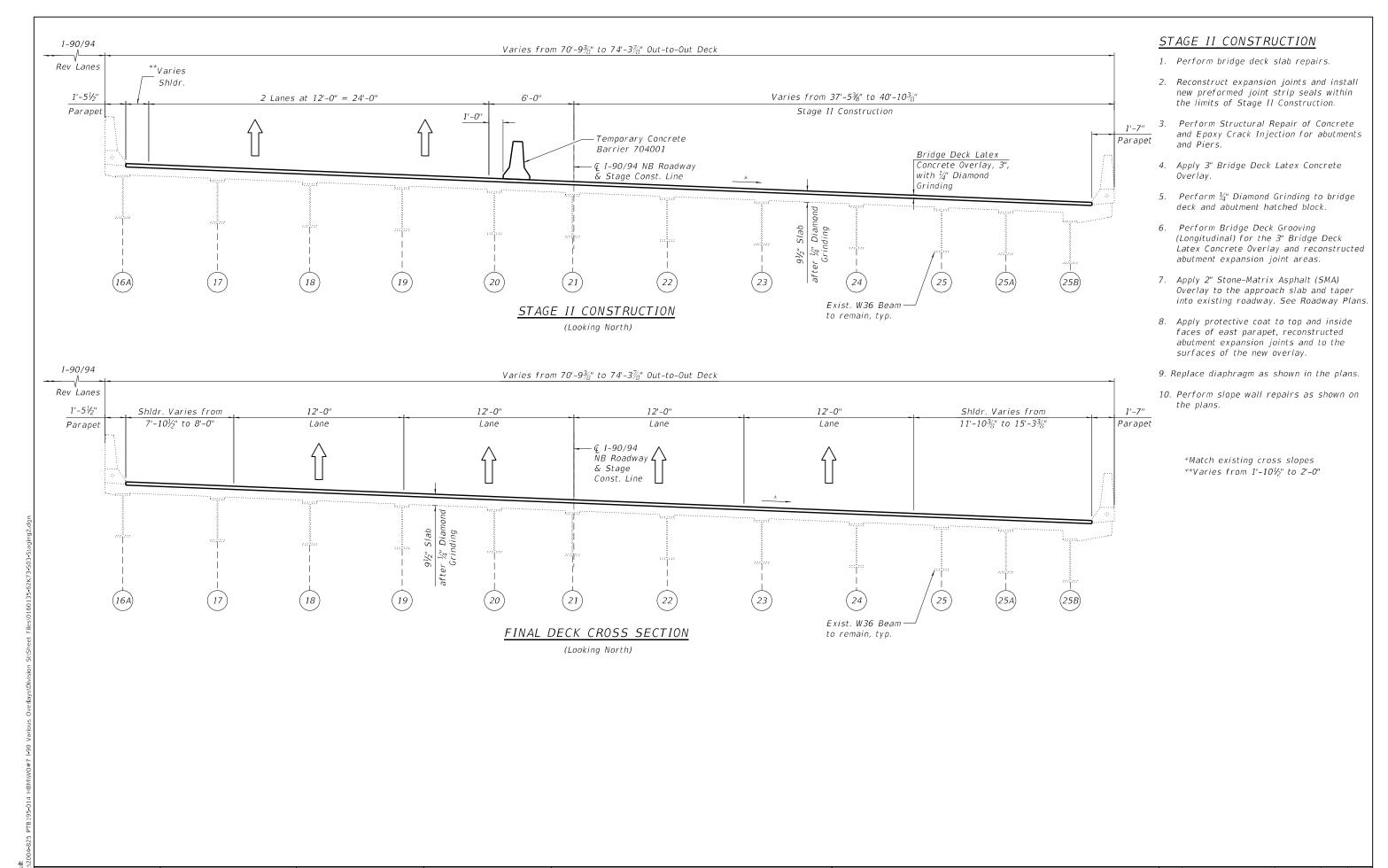
DATE

REVISED

REVISED .

STRUCTURE NO. 016-0135 (NB) SHEET S01-03 OF S01-22 SHEETS

SECTION COUNTY 90/94 2020-005-BR COOK 908 286 CONTRACT NO. 62K73



ENGINEERING GROUP, LLC

 USER NAME =
 DESIGNED - LAB, KJD
 REVISED

 CHECKED - MI
 REVISED

 PLOT SCALE =
 DRAWN - LAB, KJD
 REVISED

 PLOT DATE =
 DATE - 4/29/2024
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

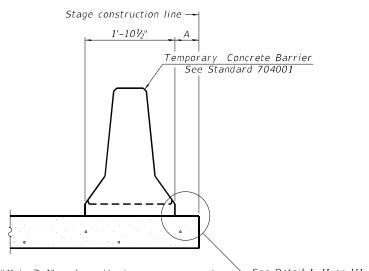
STAGE CONSTRUCTION (SHEET 2 OF 2)
STRUCTURE NO. 016-0135 (NB)

SHEET S01-04 OF S01-22 SHEETS

 F.A.I. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL NO. NO.
 SHEETS NO.

 90/94
 2020-005-BR
 COOK
 908
 287

 CONTRACT NO. 62K73



∽ See Detail I, II or III When "A" is 3'-1" or less, the temporary concrete barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

- Stage removal line ← Stage removal line 1'-101/2" 1'-101/2" Temporary Concrete Barrier See Standard 704001 6" min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint is required when "A" is greater than 3'-1".

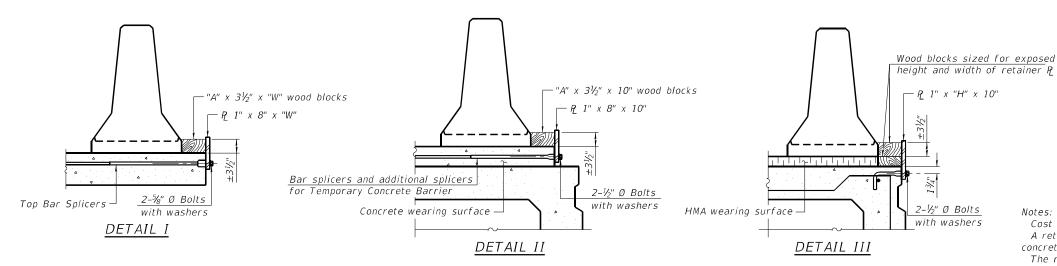
* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

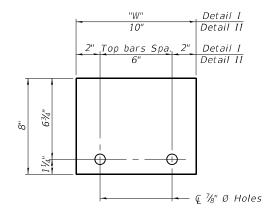
EXISTING DECK BEAM

NEW SLAB OR NEW DECK BEAM

SECTIONS THRU SLAB OR DECK BEAM

EXISTING SLAB

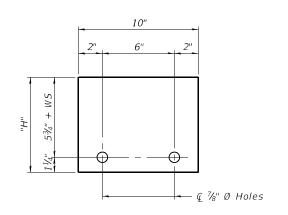




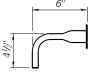
RAILING CRITERIA

NCHRP 350 Test Level	3
Railing Weight (plf)	440

10-12-2021



STEEL RETAINER R 1" x "H" x 10" (Detail III)



RESTRAINING PIN

BAR SPLICER FOR #4 BAR - DETAIL III

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate $\c c$ of each temporary concrete barrier.

1x8 UNC

1" Ø pin

US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 gauge thick washer

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

DESIGNED - JMI, AMS REVISED -CHECKED - MI REVISED -REVISED - 4/29/2024 REVISED -

STEEL RETAINER P 1" x 8" x "W"

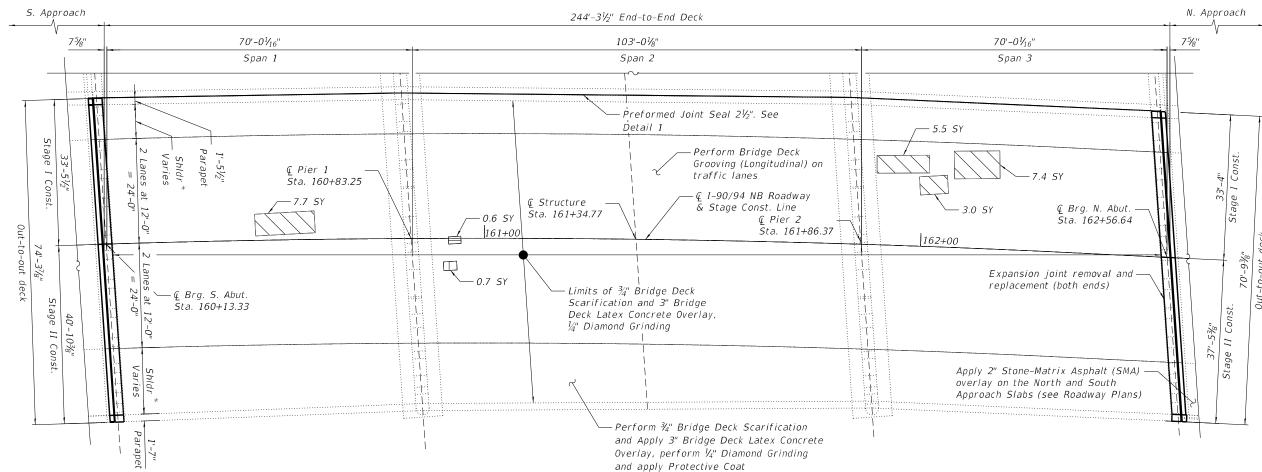
(Detail I and II)

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **TEMPORARY CONCRETE BARRIER** STRUCTURE NO. 016-0135 (NB) SHEET S01-05 OF S01-22 SHEETS

SECTION 90/94 2020-005-BR COOK 908 288 CONTRACT NO. 62K73

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Protective Coat	Sq Yd	2,063
Preformed Joint Seal 2 1/2"	Foot	245
Bridge Deck Grooving (Longitudinal)	Sq Yd	1,305
Approach Slab Repair (Full Depth)	Sq Yd	49
Approach Slab Repair (Partial Depth)	Sq Yd	49
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	1,844
Bridge Deck Scarification 3/4"	Sq Yd	1,844
Deck Slab Repair (Full Depth, Type I)	Sq Yd	0.6
Deck Slab Repair (Full Depth, Type II)	Sq Yd	23.6
Diamond Grinding (Bridge Section)	Sq Yd	1,891



NOTES:

- 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction.
- 2. For bridge deck final cross section, see Sheet S01-04.
- 3. For North and South transverse joint removal and reconstruction, see Sheets S01-07 thru S01-12.
- 4. Perform $\frac{1}{4}$ " Diamond Grinding to top of bridge deck and abutment hatched block.
- 5. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- 6. Protective Coat shall be applied to the top of reconstructed transverse joints, top and inside face of parapets and top of latex concrete overlay.
- 7. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.
- 8. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department
- 9. Removal of the existing preformed joint seal is included in the cost of Preformed Joint Seal $2\frac{1}{2}$ ".

NOTES (CONT):

DECK PLAN

10. Approach Slab Repair (Full Depth) and Approach Slab Repair (Partial Depth) quantities have been estimated (based on a nominal 3% of bridge approach area) for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will determined by the Engineer in the field at the time of construction.

(Reinforcement not shown for clarity)

*West Shoulder varies from 7'-10 $\frac{1}{2}$ " to 8'-0" East Shoulder varies from 11'-10 $\frac{3}{8}$ " to 15'-3 $\frac{3}{8}$ "

**Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"

<u>LEGEND</u>



**Deck Slab Repair (Partial Depth)

Deck Slab Repair
(Full Depth, Type I)

Deck Slab Repair (Full Depth, Type II)

Square Yard

HBM ENGINEERING GROUP, LLC

USER NAME =	DESIGNED	-	LAB	REVISED	-
	CHECKED	-	MI	REVISED	-
PLOT SCALE =	DRAWN	-	LAB	REVISED	-
PLOT DATE =	DATE	-	4/29/2024	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

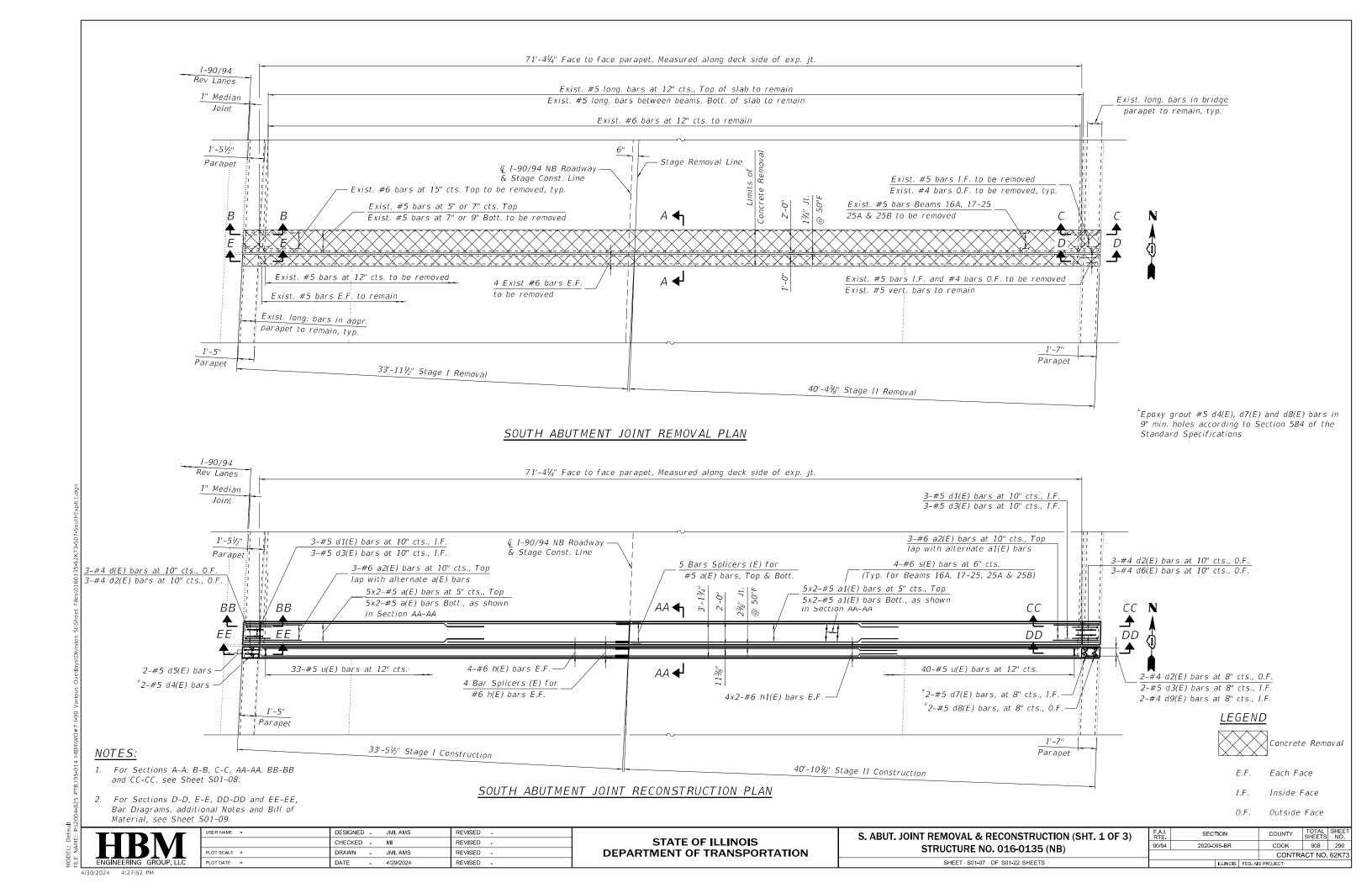
DECK REPAIR PLAN
STRUCTURE NO. 016-0135 (NB)
SHEET S01-06 OF S01-22 SHEETS

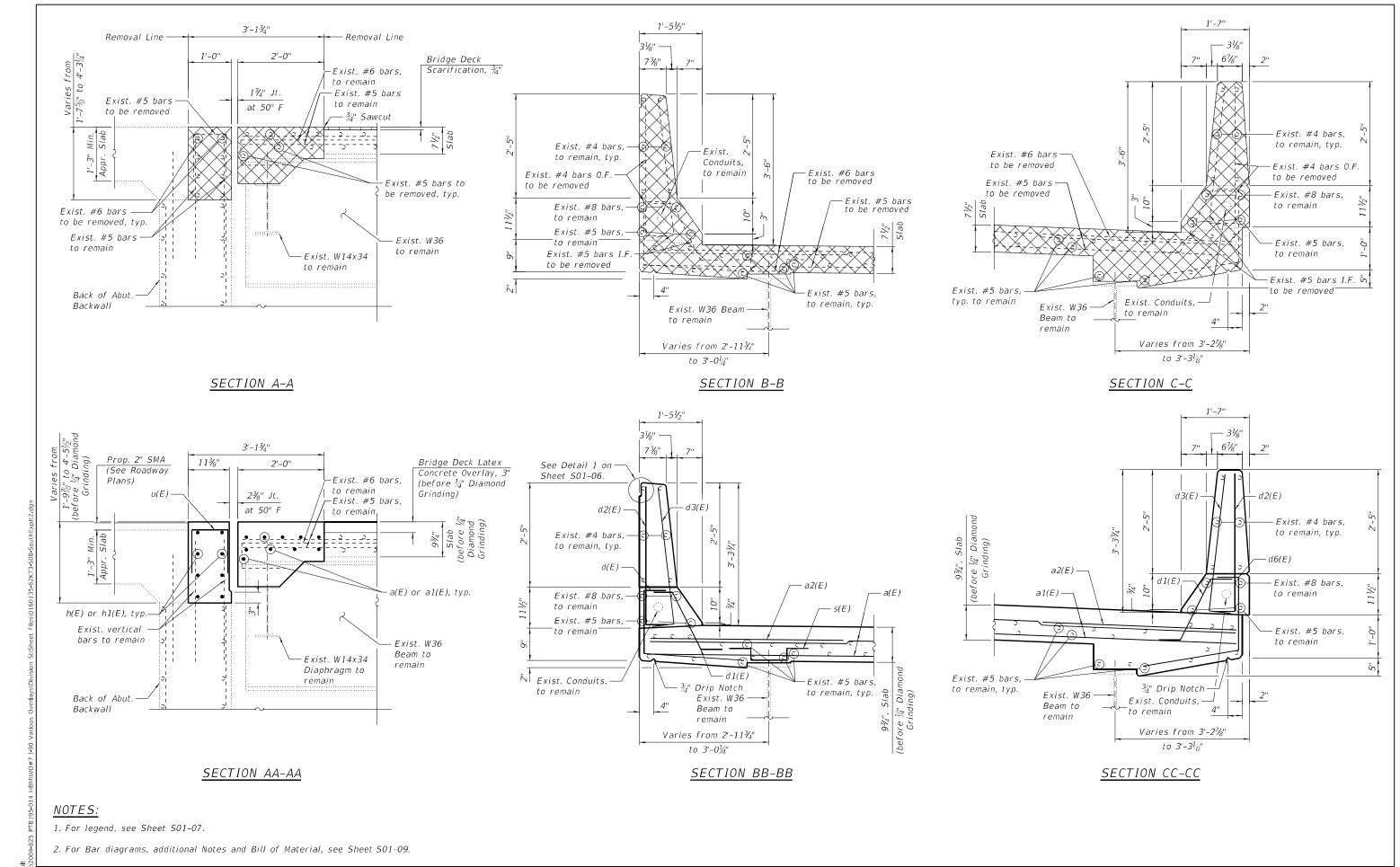
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 90/94
 2020-005-BR
 COOK
 908
 289

 CONTRACT NO. 62K73

ENGINEERING GRO





ENGINEERING GROUP, LLC

 USER NAME
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 DESIGNED
 JMI, AMS
 REVISED

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 REVISED

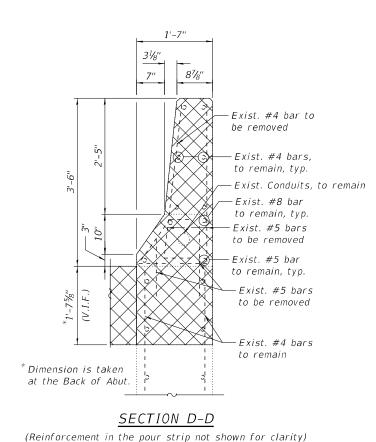
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 DRAWN
 JMI, AMS
 REVISED

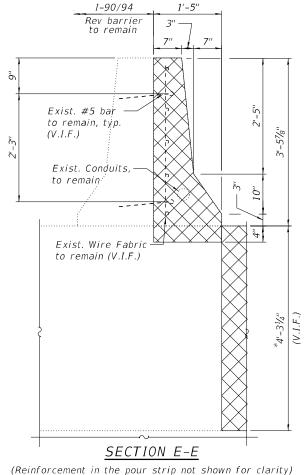
 PLOT DATE
 =
 DATE
 4/29/2024
 REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

S. ABUT. JOINT REMOVAL & RECONSTRUCTION (SHT. 2 OF 3)
STRUCTURE NO. 016-0135 (NB)

SHEET S01-08 OF S01-22 SHEETS



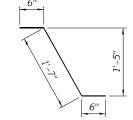


BARS d(E) & d6(E)

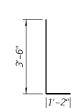
 BARS
 A
 B

 d(E)
 1'-5"
 1'-11"

 d6(E)
 1'-8"
 2'-1"



<u>BAR d1(E)</u>



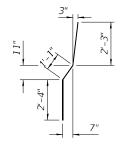
 $BAR \ d5(E)$

No. 20 20 Length 18'-5" 22'-1" #5 #5 a2(E) #6 6'-6" 3'-10" #4 2'-7" 3'-8" d1(E) #5 #4 3'-8" 2'-9" d3(E) #5 #5 d5(E) #5 4'-3" d6(E) #4 5'-8" d7(E) #5 5'-6" 2'-0" d8(E) #5 d9(E) #5 31'-9" 21'-4" #6 h1(É) 16 #6 48 ___ s(E) 3'-1" #6 u(E) #5 14.8 Concrete Removal Cu Yd Concrete Superstructure Cu Yd 16.1 Protective Coat Reinforcement Bars, Pound 2,390 Epoxy Coated

BILL OF MATERIAL

BARS d2(E) & d3(E)

6"



BAR d7(E)

 $BAR \ s(E)$



BAR u(E)

MIN.	BAR	LAPS

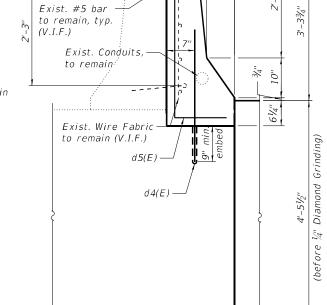
#5

#6 4'-0"

1'-7" -d2(E)d3(E) -– Exist. #4 bars, to remain, typ. — Exist. Conduits, to remain - d9(E) - Exist. #8 bars to remain -Exist. #5 bars to remain - d7(E) d8(E) - Exist. #4 bars to remain

SECTION DD-DD

(Reinforcement in the pour strip not shown for clarity)



I-90/94 Rev barrier

to remain

SECTION EE-EE (Reinforcement in the pour strip not shown for clarity)

NOTES:

- 1. For legend, see Sheet S01-07.
- 2. For preformed joint strip seal details, see Sheet S01-13.
- 3. For bar splicer assembly details, see Sheet S01-22.
- 4. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 5. Epoxy grout d4(E), d7(E) and d8(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.

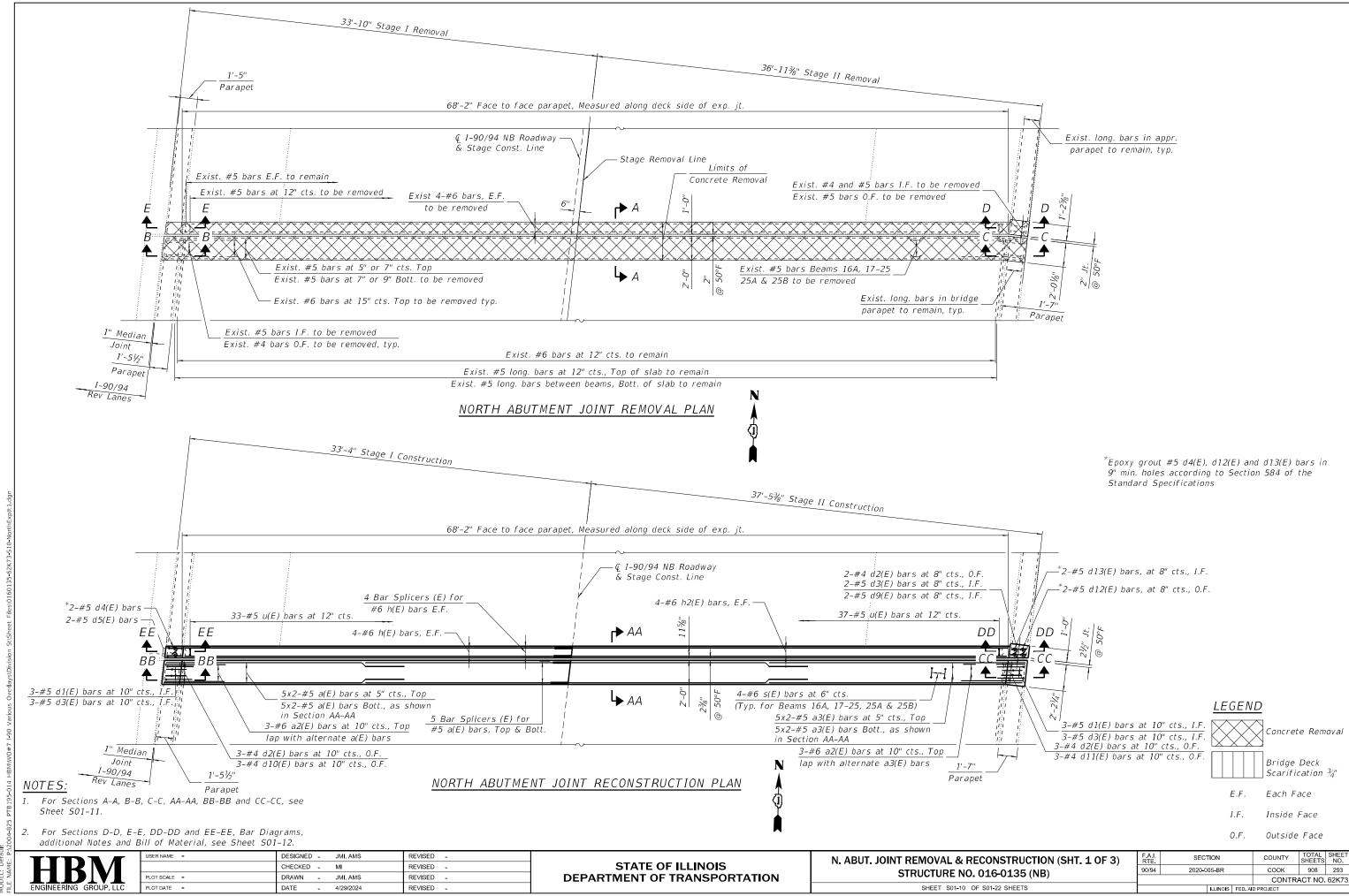


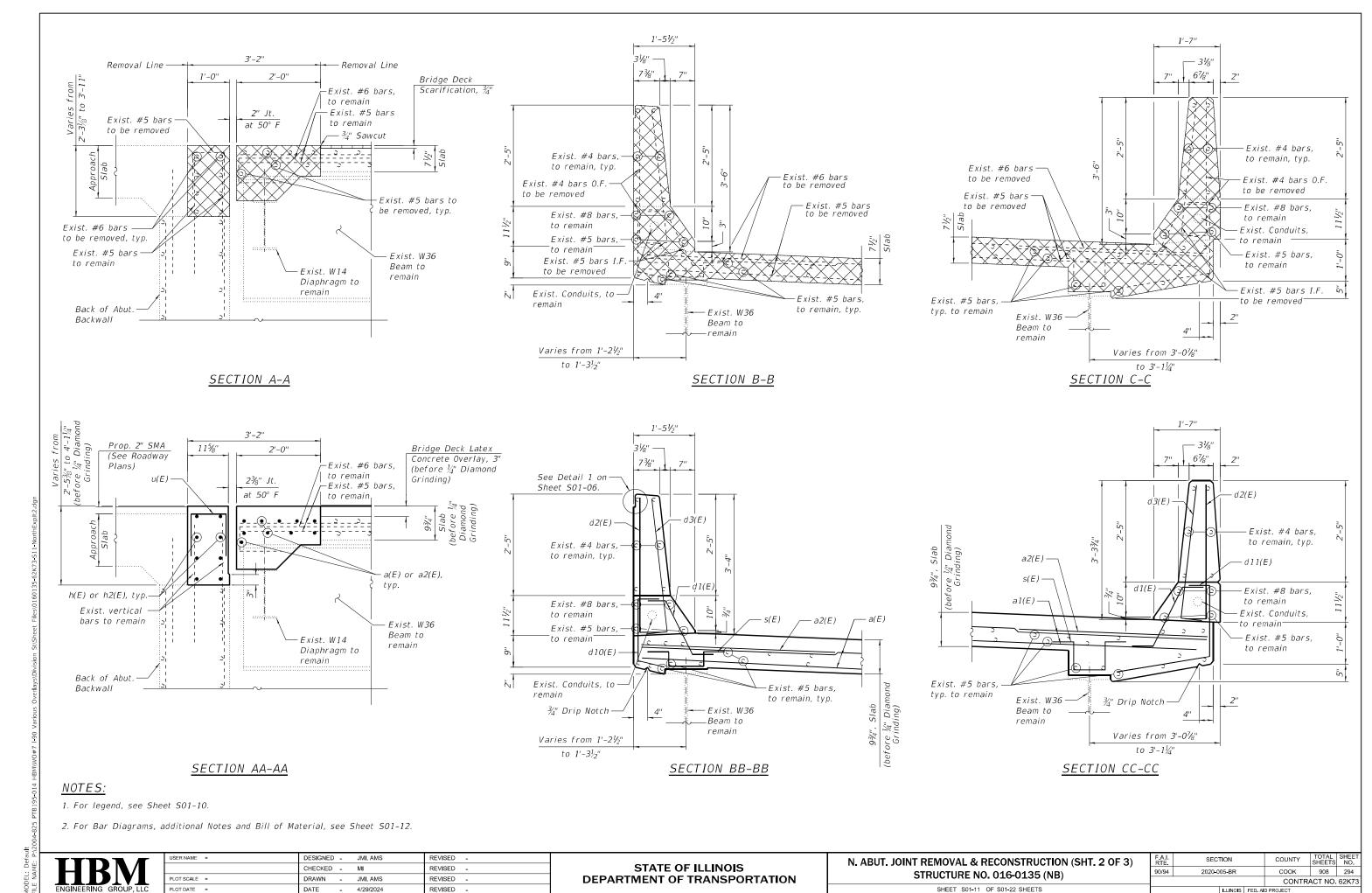
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		CHECKED	-	MI	REVISED -
	PLOT SCALE =	DRAWN	-	JMI, AMS	REVISED -
	PLOT DATE =	DATE	-	4/29/2024	REVISED -
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

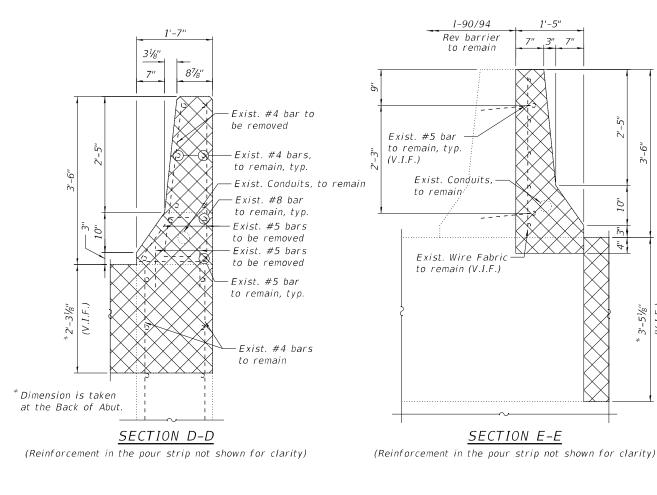
S. ABUT. JOINT REMOVAL & RECONSTRUCTION (SHT. 3 OF 3) STRUCTURE NO. 016-0135 (NB)
SHEET S01-09 OF S01-22 SHEETS

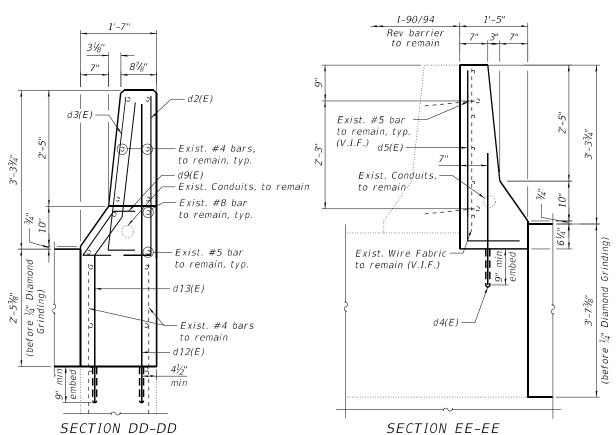
F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2020-005-BR	соок	908	292
		CONTRA	ACT NO.	62K73

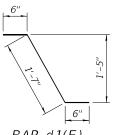




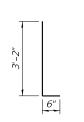
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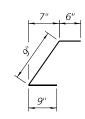




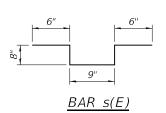
BAR d1(E)

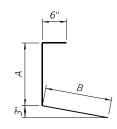


BARS d2(E) & d3(E)



BAR d9(E)

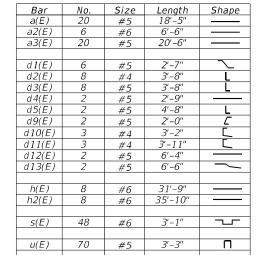




BARS d10(E) & d11(E)

u12(E)	1-5	1-5
d13(E)	1'-8"	1'-11"
1	_1	
3'-6"		
ξ		
	1'-2"	

 $BAR \ d5(E)$



Concrete Removal

Reinforcement Bars,

Protective Coat

Epoxy Coated

Concrete Superstructure

14.8

16.2

26

2,270

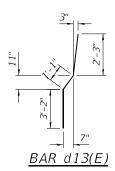
Cu Yd

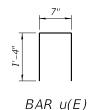
Cu Yd

Sq Yd

Pound

BILL OF MATERIAL





MIN. BAR LAPS

<i>#5</i>	3'-6"
#6	4'-0"

NOTES:

- 1. For legend, see Sheet S01-10.
- 2. For preformed joint strip seal details, see Sheet S01-13.
- 3. For bar splicer assembly details, see Sheet S01-22.
- 4. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 5. Epoxy grout d4(E), d12(E) and d13(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.



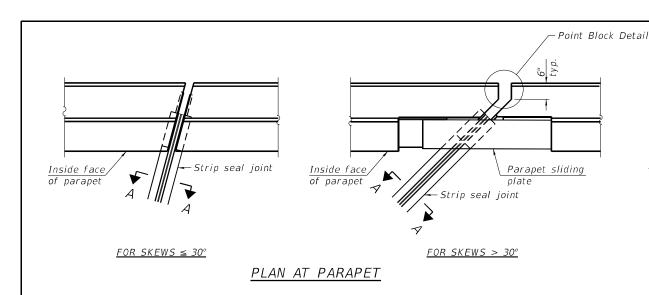
(Reinforcement in the pour strip not shown for clarity)

USER NAME =	DESIGNED	-	JMI, AMS	REVISED	-
	CHECKED	-	MI	REVISED	-
PLOT SCALE =	DRAWN	-	JMI, AMS	REVISED	-
PLOT DATE =	DATE	-	4/29/2024	REVISED	-

(Reinforcement in the pour strip not shown for clarity)

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION N. ABUT. JOINT REMOVAL & RECONSTRUCTION (SHT. 3 OF 3) **STRUCTURE NO. 016-0135 (NB)** SHEET S01-12 OF S01-22 SHEETS

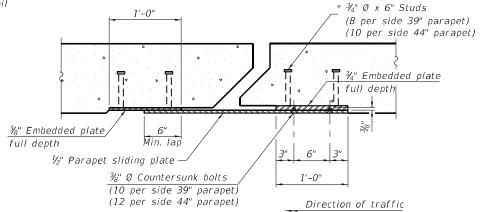
F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2020-005-BR	соок	908	295
		CONTRA	ACT NO.	62K73
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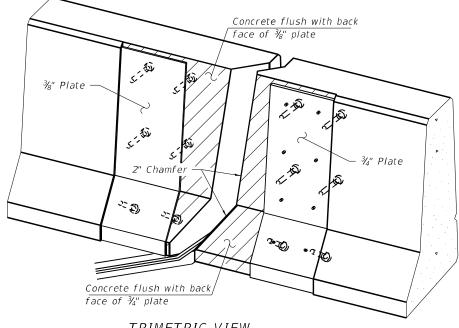
Top of locking

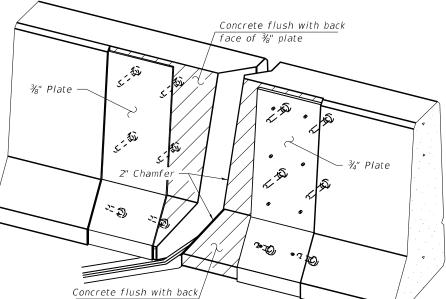
Top of deck

edge rail



SECTION B-B





The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

ELEVATION AT PARAPET

Parapet sliding

Inside Face

of Parapet

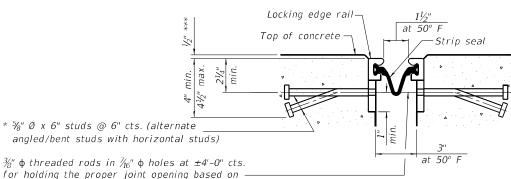
2" Max.

В

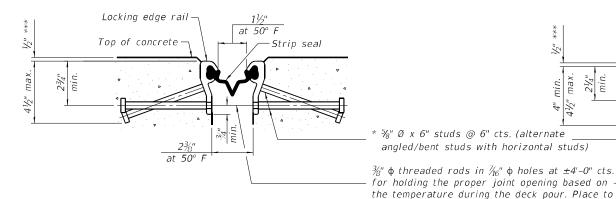
%" Ø x 6" Studs

Detail .

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



TRIMETRIC VIEW (Showing embedded plates only)



SHOWING ROLLED RAIL JOINT

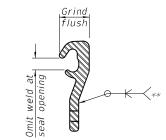
* $\frac{1}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs) $\frac{3}{6}$ " ϕ threaded rods in $\frac{1}{6}$ " ϕ holes at ± 4 '-0" cts.

SHOWING WELDED RAIL JOINT

<u>ROLLED</u> WELDED RAIL (EXTRUDED) RAIL

LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	146

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

miss studs. All rods shall be burned, or sawed

off flush with the plates after concrete is set.

DETAIL A

*** Before 1/4" Diamond Grinding

			1-7
	USER NAME =	DESIGNED - JMI, AMS	REVISED -
1 K V/I		CHECKED - MI	REVISED -
	PLOT SCALE =	DRAWN - JMI, AMS	REVISED -
NEERING GROUP, LLC	PLOT DATE =	DATE - 4/29/2024	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

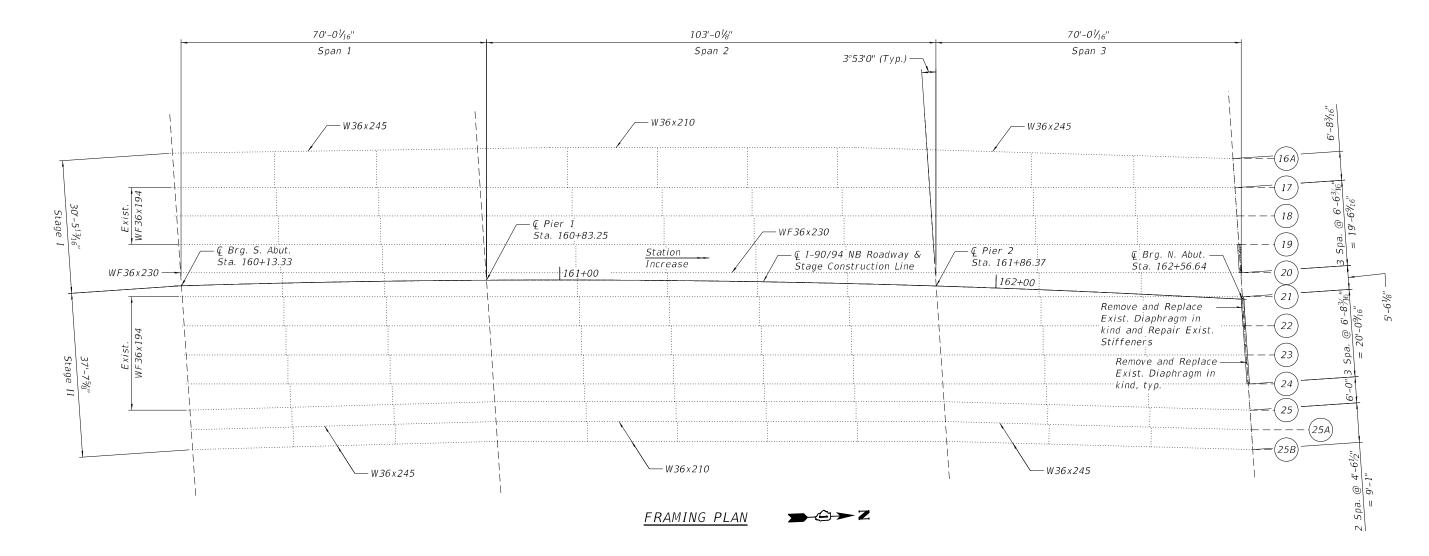
PREFORMED JOINT STRIP SEAL STRUCTURE NO. 016-0135 (NB) SHEET S01-13 OF S01-22 SHEETS

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
90/94	2020-005-BR		соок	908	296
			CONTRA	ACT NO.	62K7
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BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Furnishing And Erecting Structural Steel	Pound	1,000
Structural Steel Removal	Pound	1,000
Structural Steel Repair	Pound	100



NOTES:

1. All work is to be performed utilizing stage construction. See Sheets S01-03 and S01-04 for details.

2. For Diaphragm Removal and Replacement Details and Exist. Stiffeners Repair, see Sheets S01-15 and S01-16.

LEGEND



Remove and Replace Exist.
Diaphragm



USER NAME =	DESIGNED	-	LAB	REVISED -
	CHECKED	-	MI	REVISED -
PLOT SCALE =	DRAWN	-	LAB	REVISED -
PLOT DATE =	DATE	-	4/29/2024	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

FRAMING PLAN
STRUCTURE NO. 016-0135 (NB)

SHEET S01-14 OF S01-22 SHEETS

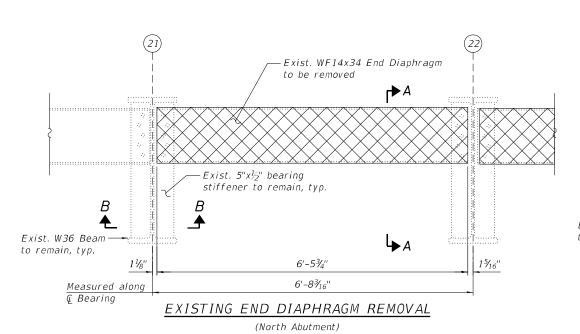
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 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

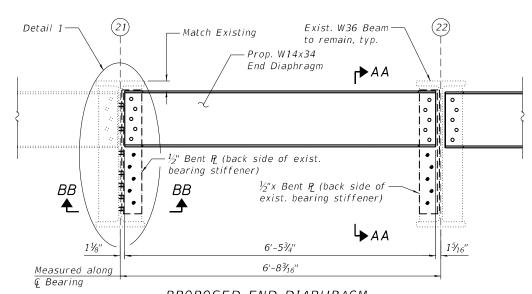
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 2020-005-BR
 COOK
 908
 297

 CONTRACT NO. 62K73

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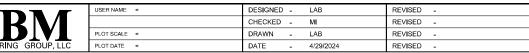




PROPOSED END DIAPHRAGM (North Abutment)

NOTES:

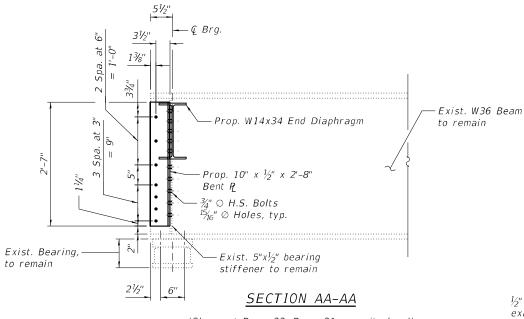
- 1. For location of Diaphragm Removal/Replacement and Bill of Material, see Sheet S01-14.
- All structural steel for the bearing stiffener repair plates shall be AASHTO M270 Grade 50. All structural steel for diaphragms and associated connection plates may be AASHTO M270 Grade 36.
- Diaphragm connection holes shall be $^{15}\!\!/_{6}$ " for $^{3}\!\!/_{4}$ " bolts. Two hardened washers shall be required at diaphragm connections. Fasteners shall be high strength bolts.
- All proposed bearing stiffener repair plates, bolts, nuts, washers and associated field-drilling shall be paid for as Structural Steel Repairs. The proposed diaphragm shall be paid for as Furnishing and Erecting Structural Steel.
- Holes in new steel shall be field drilled using existing steel as template.
- No field welding shall be permitted.



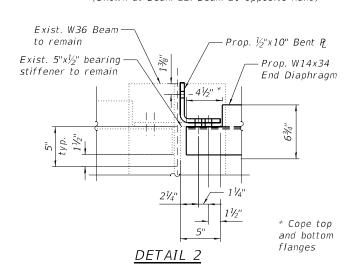
€ Brg. ---Exist. WF14x34 End Diaphragm to be removed — Exist. W36 Beam to remain Exist. 5"x1/3" bearing stiffener to remain Exist. Bearing, to remain

SECTION A-A

(Shown at Beam 22, Beam 21 opposite hand)



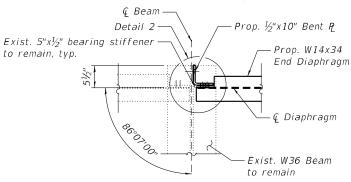
(Shown at Beam 22. Beam 21 opposite hand)



⊈ Beam — -Exist. WF14x34 End Exist. $5''x^{1/2}''$ bearing stiffener Diaphragm to be removed to remain, typ. € Diaphragm -Exist. W36 to remain, typ.

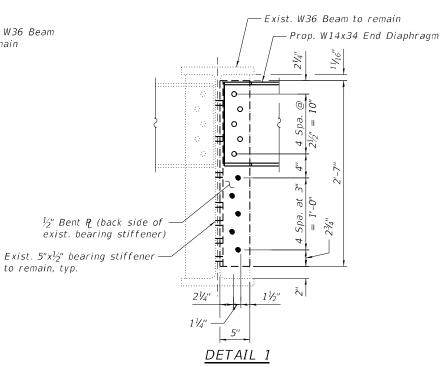
SECTION B-B

(Shown at Beam 21. Beam 22 opposite hand)



SECTION BB-BB

(Shown at Beam 21. Beam 22 opposite hand)



(Shown at Beam 21. Beam 22 opposite hand)

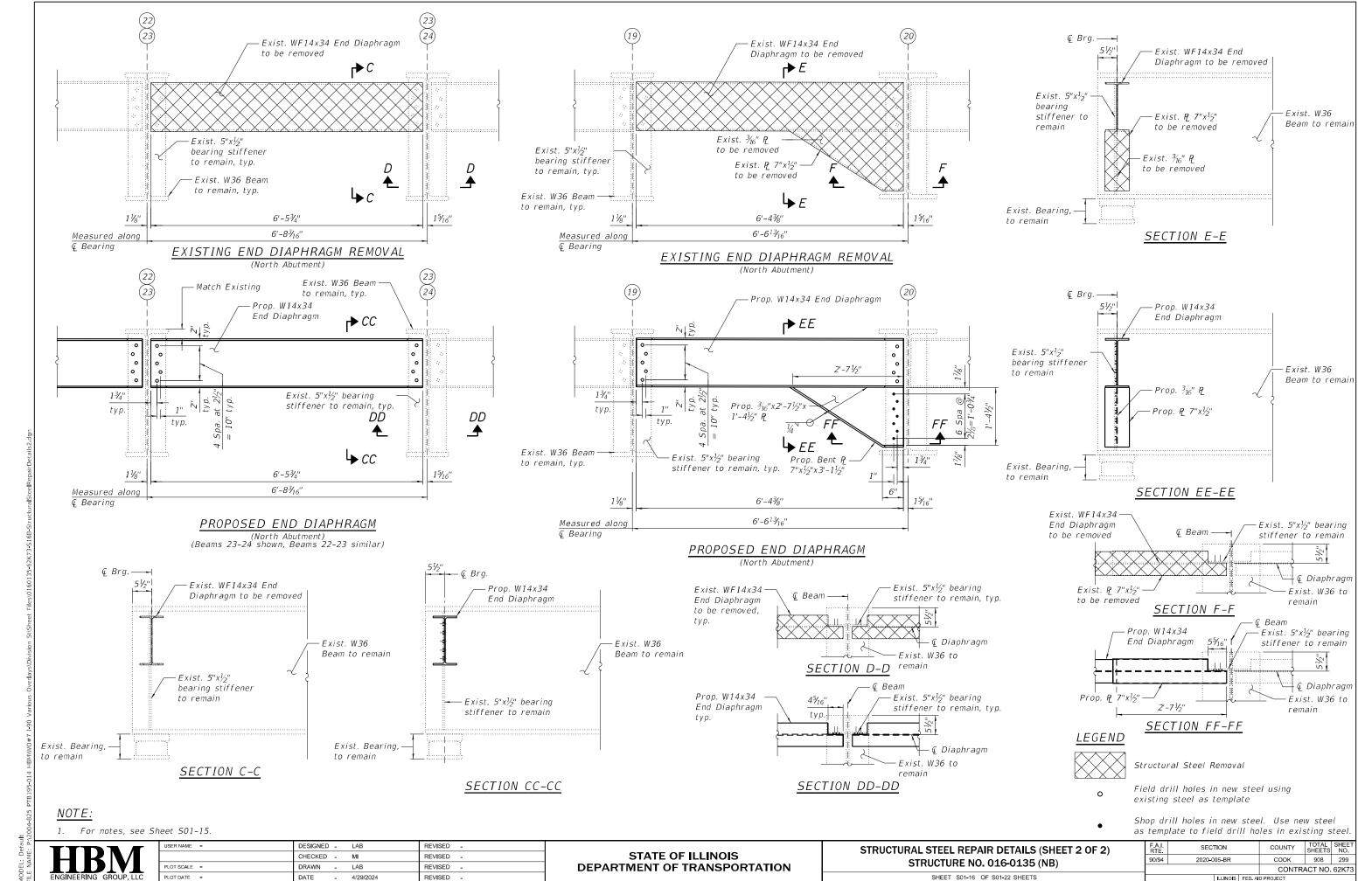
LEGEND

Structural Steel Removal

- Field drill holes in new steel using existing steel as template
- Shop drill holes in new steel. Use new steel as template to field drill holes in existing steel.

SECTION 90/94 2020-005-BR COOK 908 298 CONTRACT NO. 62K73

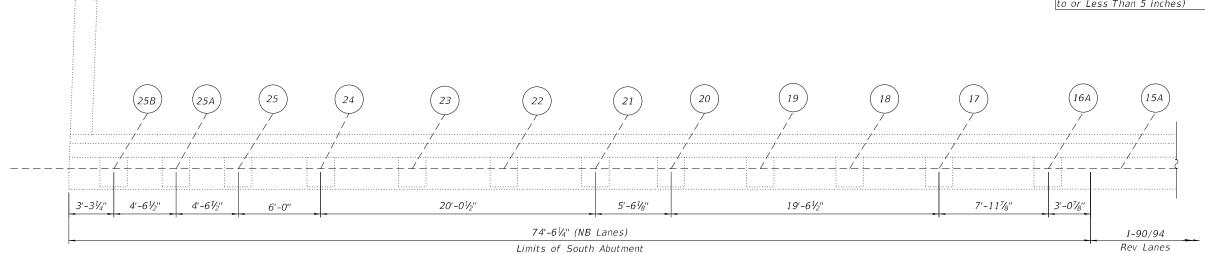
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** STRUCTURAL STEEL REPAIR DETAILS (SHEET 1 OF 2) **STRUCTURE NO. 016-0135 (NB)** SHEET S01-15 OF S01-22 SHEETS



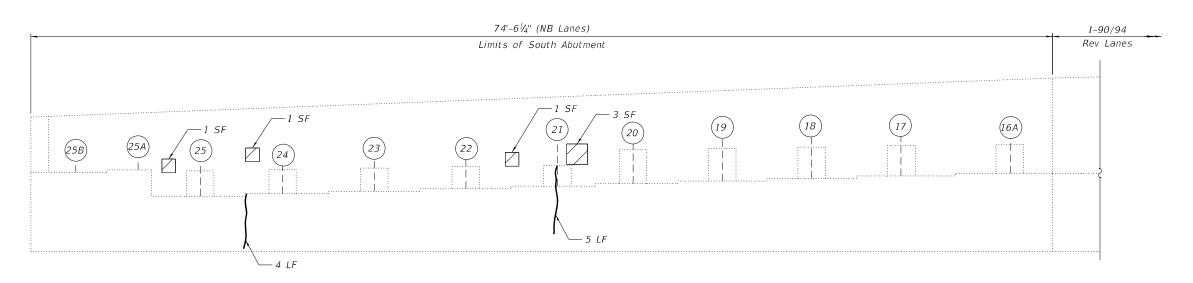
4/30/2024 4:30:53 PM

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	311
Epoxy Crack Injection	Foot	9
Structural Repair of Concrete (Depth Equal	Sq Ft	6







SOUTH ABUTMENT ELEVATION

(Looking South)

NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Concrete Sealer is to be applied to the abutment seats and the bottom 2 feet of the abutment backwall.
- 3. For Slope Wall repairs, see Sheet S01-21.

	USER NAME =	DESIGNED - AMS	REVISED -
NGINEERING GROUP, LLC		CHECKED - MI	REVISED -
	PLOT SCALE =	DRAWN - AMS	REVISED -
	PLOT DATE =	DATE - 4/29/2024	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOUTH ABUTMENT REPAIRS STRUCTURE NO. 016-0135 (NB)

SHEET S01-17 OF S01-22 SHEETS
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 90/94
 2020-005-BR
 COOK
 908
 300

 CONTRACT NO. 62K73

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Epoxy Crack Injection (Width > 0.06")

Square Foot Linear Foot

LEGEND

SF

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