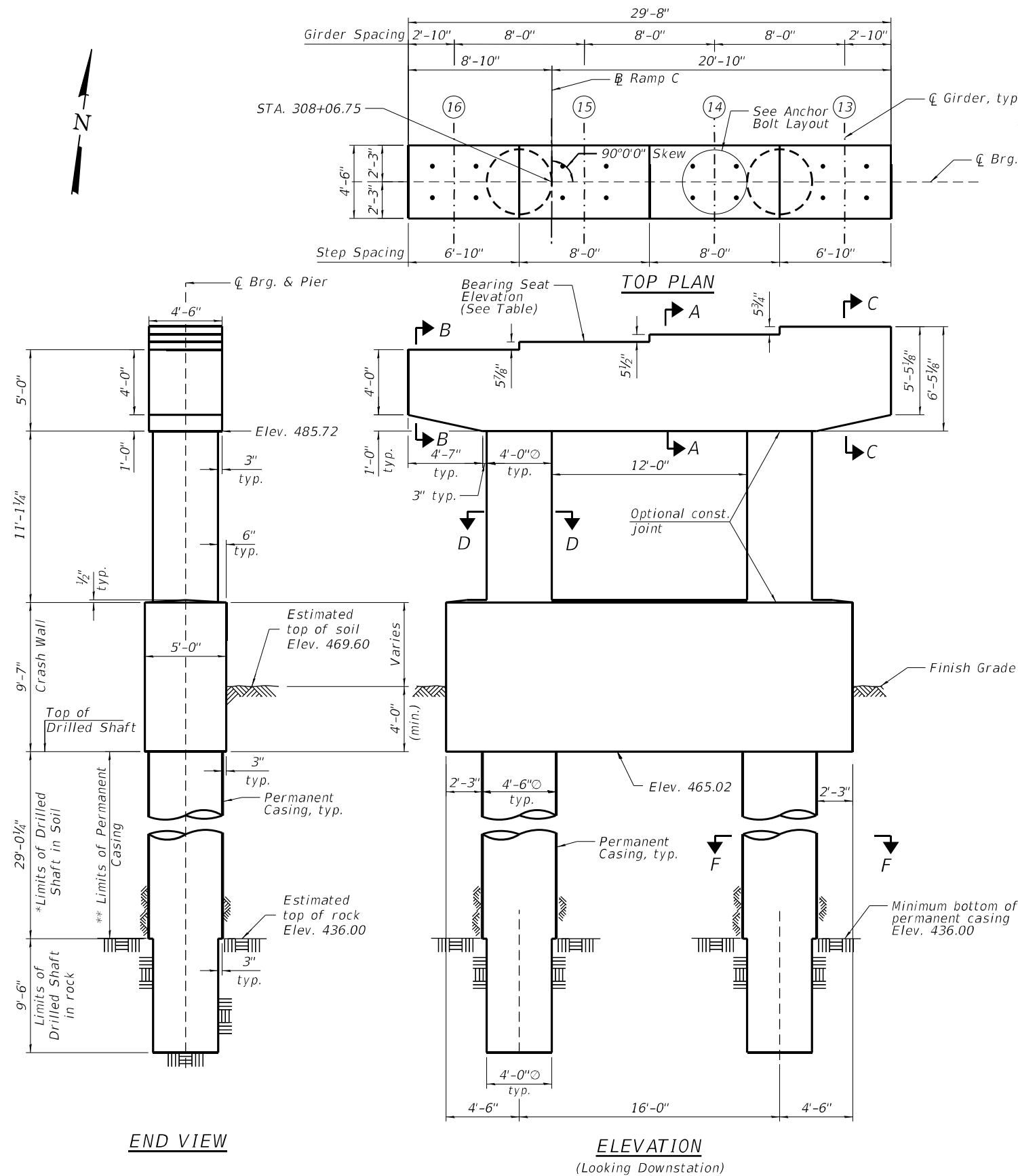
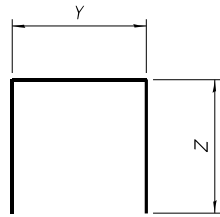
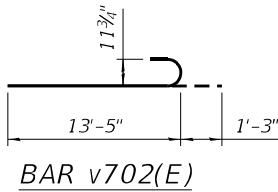
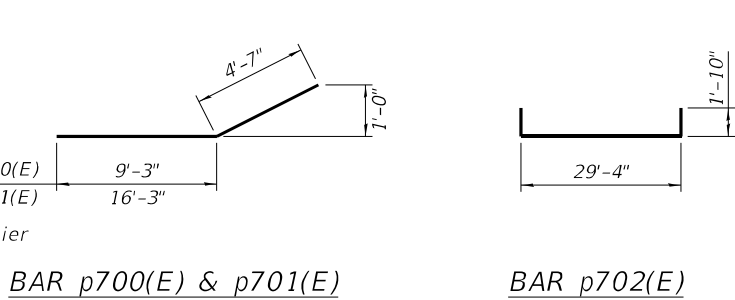


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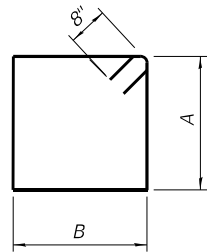
* The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.

** Contractor is responsible for determining the casing thickness and the actual tip elevation to be used. See Article 516.06(d) of the Standard Specifications. Pay limits for the Permanent Casing shall be based on the minimum length shown. Permanent Casing shall extend into rock as needed if the upper zone of rock is fractured and excessive water inflow is encountered.



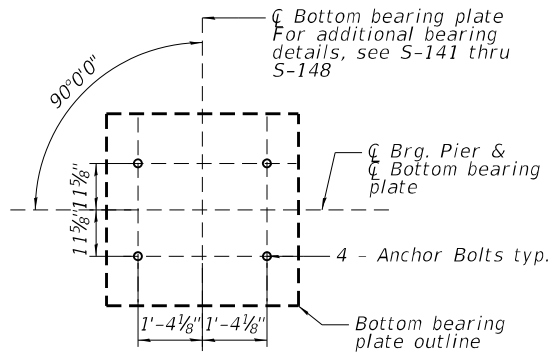
X	Y	Z
s700(E)	4'-8"	8'-3"
s701(E)	4'-8"	5'-5"
s705(E)	2'-9"	4'-4"
s706(E)	2'-9"	5'-1"
s707(E)	4'-2"	10"
u700(E)	4'-6"	3'-4"
u701(E)	4'-0"	2'-9"

BAR X



BAR	A	B
s702(E)	3'-9"	2'-9"
s703(E)	3'-11"	2'-9"
s704(E)	4'-2"	2'-9"

BARS s702(E) thru s704(E)



ANCHOR BOLT LAYOUT

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h700(E)	28	#6	24'-8"	—
h701(E)	2	#5	26'-11"	—
h702(E)	10	#5	29'-2"	—
h703(E)	2	#5	18'-1"	—
h704(E)	2	#5	7'-1"	—
h705(E)	6	#5	5'-7"	—
h706(E)	6	#5	8'-8"	—
h707(E)	6	#5	10'-7"	—
h708(E)	6	#5	6'-6"	—
p700(E)	8	#8	13'-10"	—
p701(E)	8	#8	20'-10"	—
p702(E)	7	#10	33'-0"	—
s700(E)	50	#7	21'-2"	U
s701(E)	50	#7	15'-6"	U
s702(E)	2	#6	14'-4"	□
s703(E)	2	#6	14'-8"	□
s704(E)	2	#6	15'-2"	□
s705(E)	20	#6	11'-5"	U
s706(E)	108	#6	12'-11"	U
s707(E)	10	#4	5'-10"	U
*** sp700	2	#5	38'-8"	W
*** sp701(E)	2	#5	11'-3"	W
u700(E)	20	#6	11'-2"	U
u701(E)	12	#5	9'-6"	U
v700	32	#10	38'-7"	—
v701(E)	32	#10	22'-0"	—
v702(E)	32	#9	14'-8"	—
Structure Excavation			Cu. Yd.	45
Concrete Structures			Cu. Yd.	82.3
Reinforcement Bars			Pound	7,180
Reinforcement Bars, Epoxy Coated			Pound	15,360
Permanent Casing			Foot	59
Drilled Shaft in Soil			Cu. Yd.	34.2
Drilled Shaft in Rock			Cu. Yd.	8.9

***Length is height of spiral

BEARING SEAT ELEVATION	
Girder No.	Elevation
13	492.15
14	491.68
15	491.22
16	490.72

NOTES:

- Space reinforcement to miss anchor bolts.
- Pour steps monolithically with cap.
- For Sections and Details see Sheet S-176.
- Permanent casing should be twised and pushed until top of bedrock is reached. Any other method should not be allowed. A minimum 3-foot thick soil plug should be maintained while pushing, twisting, and excavating within the casing until top of bedrock is reached.

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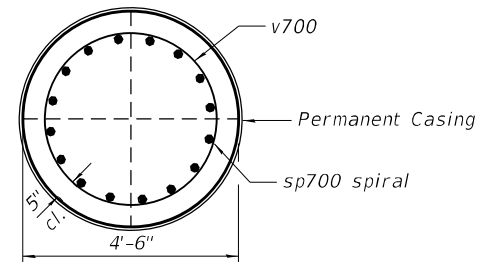
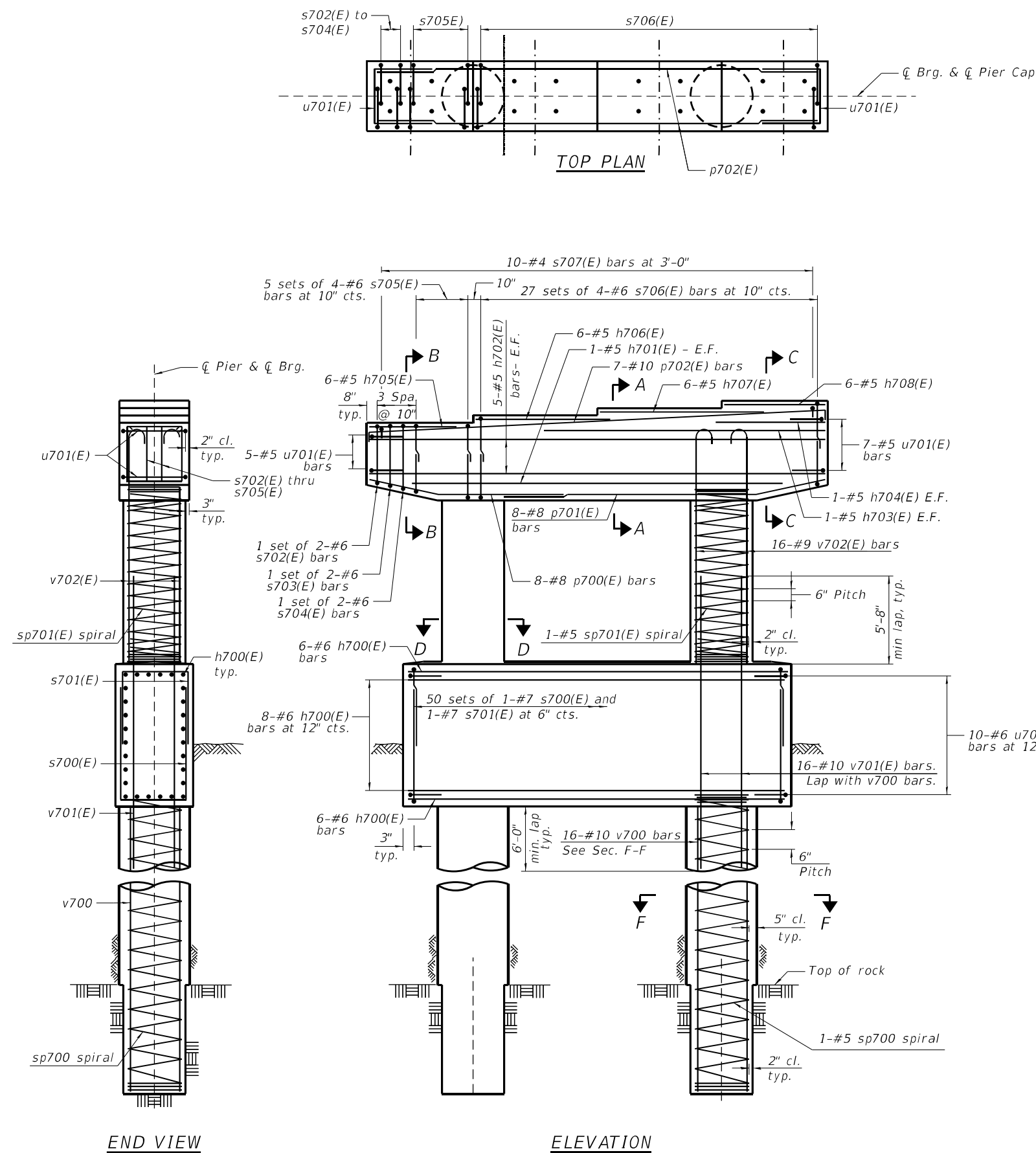
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	CHECKED - YC	REVISED -
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PLOT DATE = 6/6/2023	CHECKED - MLK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

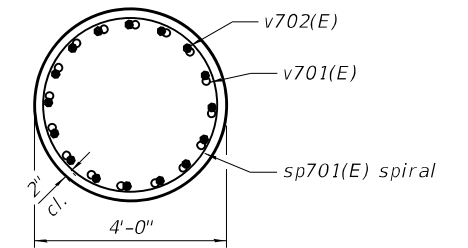
PIER 4C PLAN ELEVATION
STRUCTURE NO. 090-0182

SHEET S-175 OF S-194 SHEETS

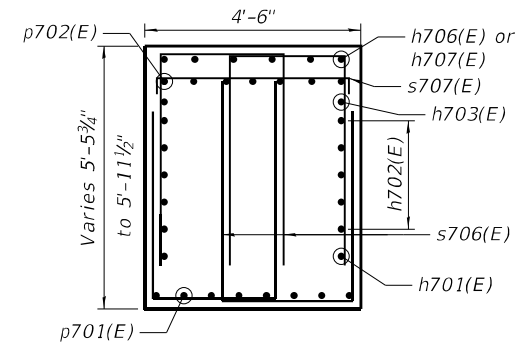
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	701
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



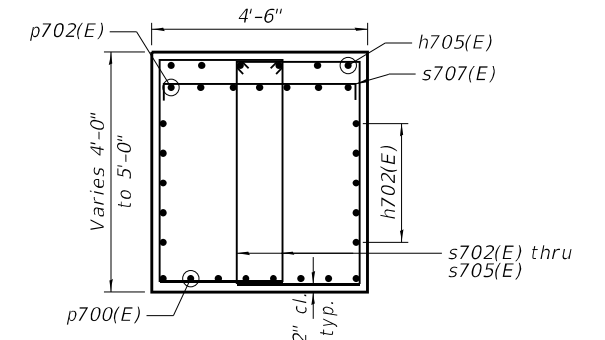
SECTION F-F



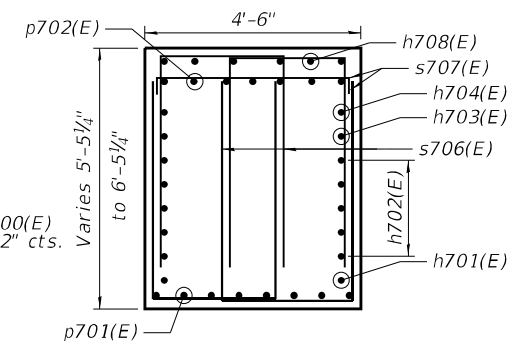
SECTION D-D



SECTION A-A



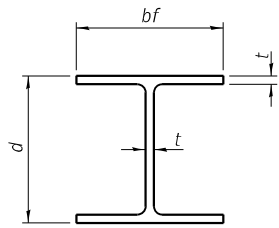
SECTION B-B



SECTION C-C

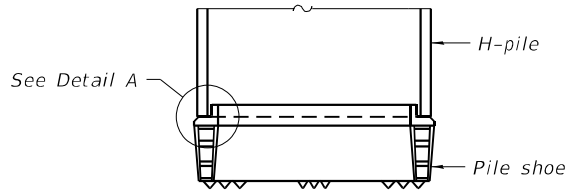
- NOTES:

1. #5 sp700 and #5 sp701(E) - Provide 1 1/2 extra turns, shop welded together per AWS D1.4 top and bottom. Extend spiral 2" into crash wall or pier cap. Provide 4-#4 spacers or equivalent.
2. All edges have a 3/4" chamfer, unless noted otherwise.

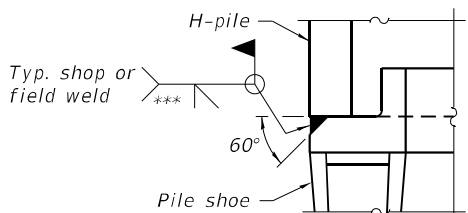


STEEL PILE TABLE

Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1 1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1 1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



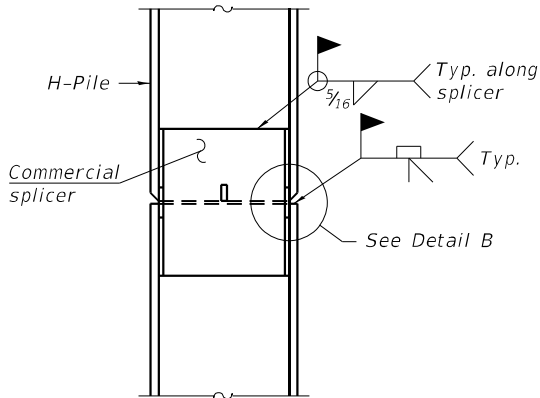
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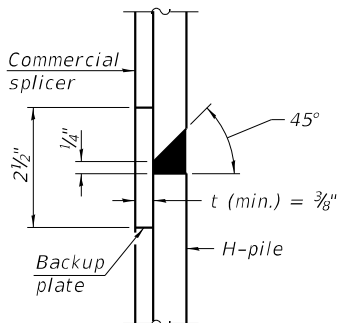
DETAIL A

SHOE ATTACHMENT

Note:
The steel H-piles shall be according to
AASHTO M270 Grade 50.

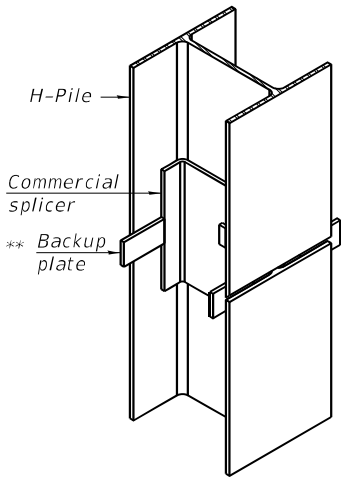


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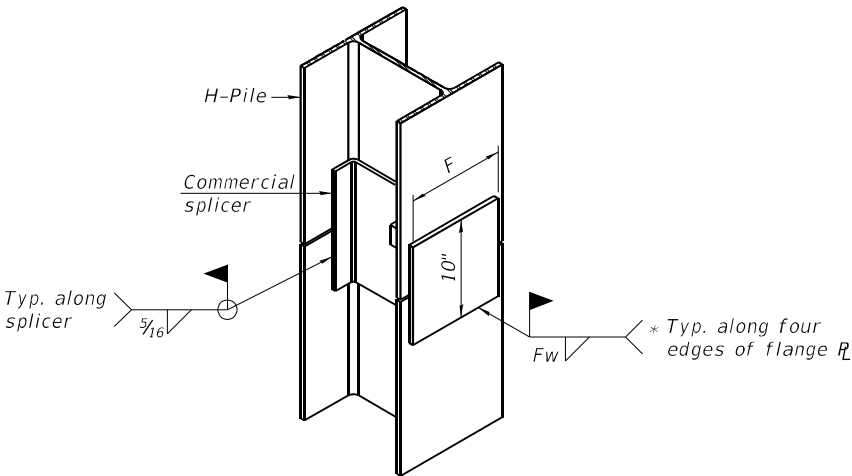


DETAIL "B"

WELDED COMMERCIAL SPLICE



ISOMETRIC VIEW



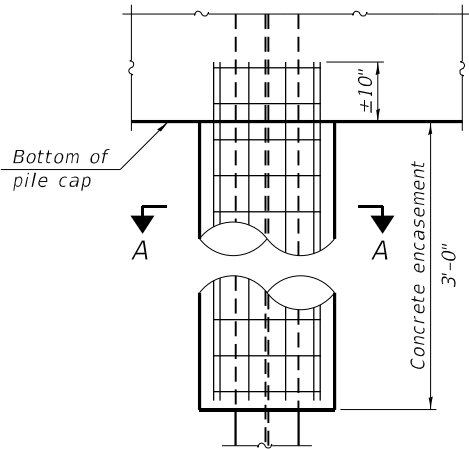
ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

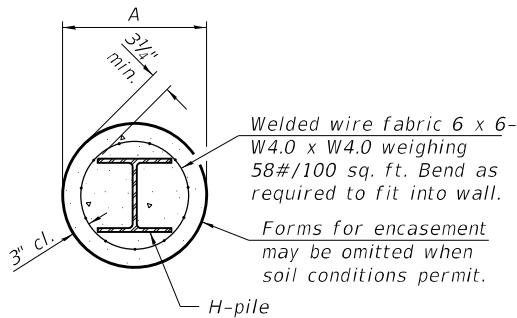
* Interrupt welds 1/4" from end of web and/or each flange.

** Remove portions of backup plates that extend outside the flanges.

*** Weld size per pile shoe manufacturer (5/16" min.).

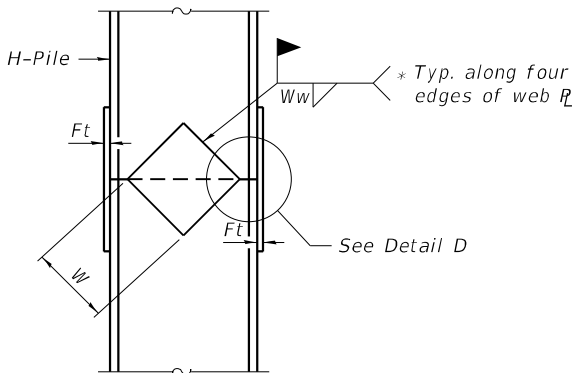


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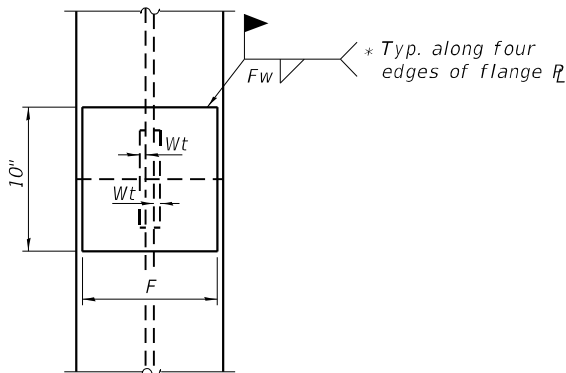


SECTION A-A

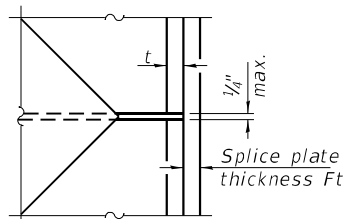
INDIVIDUAL PILE
CONCRETE ENCASEMENT
(when specified)



ELEVATION



END VIEW



DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	1 1/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

F-HP

1-1-2020

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USER NAME	= DIamb
DESIGNED	-
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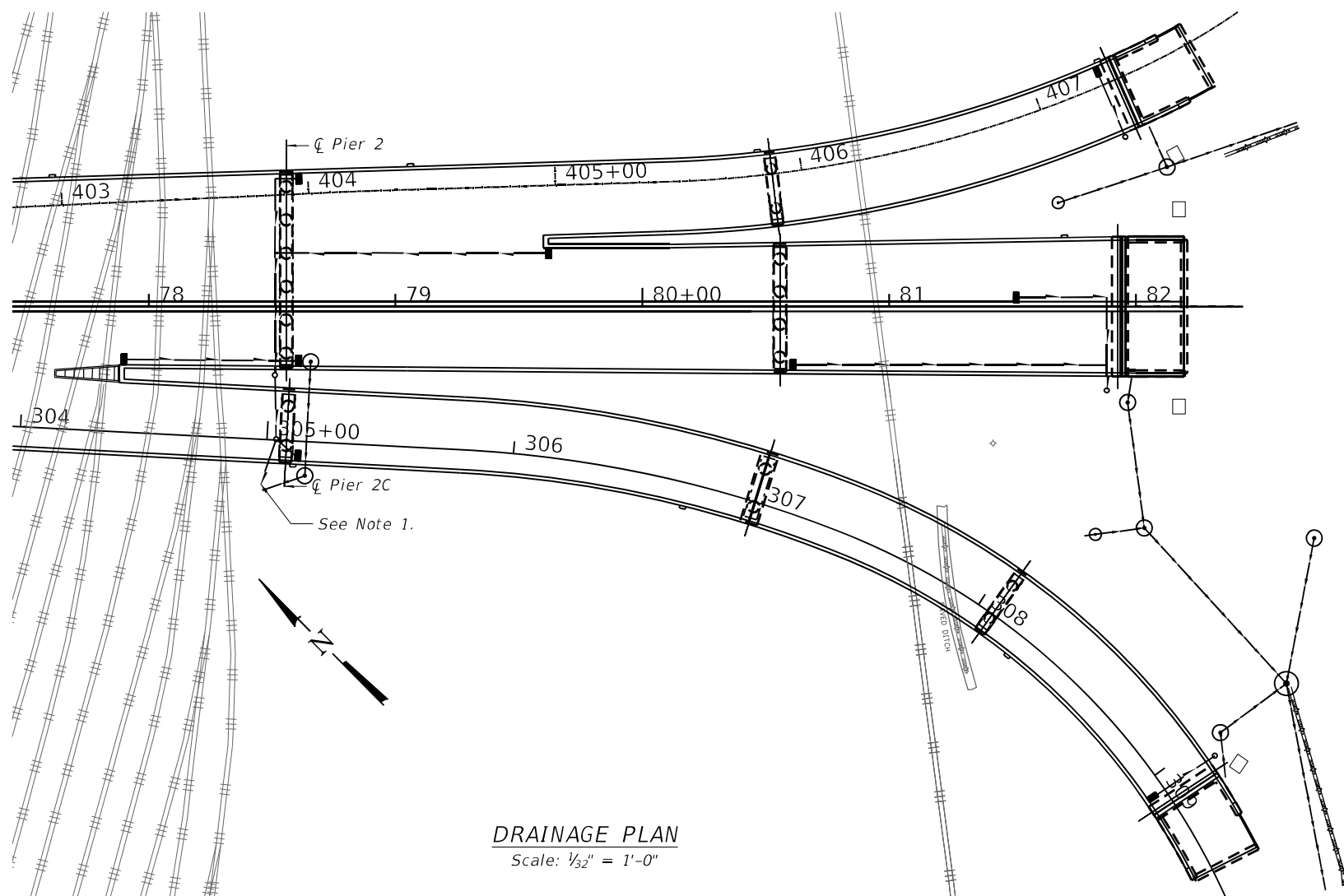
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HP PILE DETAILS
STRUCTURE NO. 090-0182

SHEET 5-178 OF 5-194 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	704
				CONTRACT NO. 68C55
				ILLINOIS FED. AID PROJECT



DRAINAGE PLAN
Scale: 1/32" = 1'-0"

LEGEND

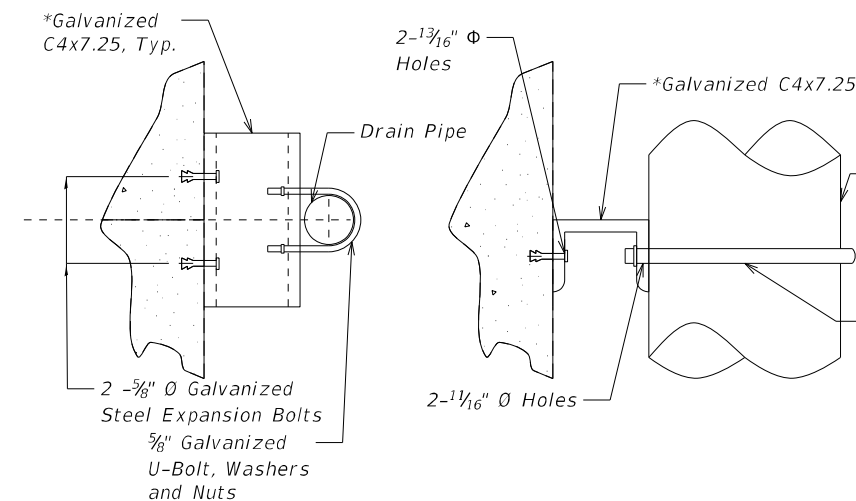
- DS-11 Scupper
- Pipe for Drainage System on bridge, arrow indicates direction of flow
- Manhole
- Pipe Drains, see Drainage Plans
- Paved Ditch, see Drainage Plans

NOTES:

- Drainage System shall connect to existing culvert or drainage structure. See drainage schedule for stationing and offset of drainage structure.
- S.S. denotes stainless steel.
- Provide structural support from proposed deck slab for drain pipe per Manufacturer's recommendations, not to exceed 5' cts. Cost included with Drainage System.
- All pipes, pipe fittings, collars, inserts, brackets, and other miscellaneous hardware shall be included with the cost of Drainage System.

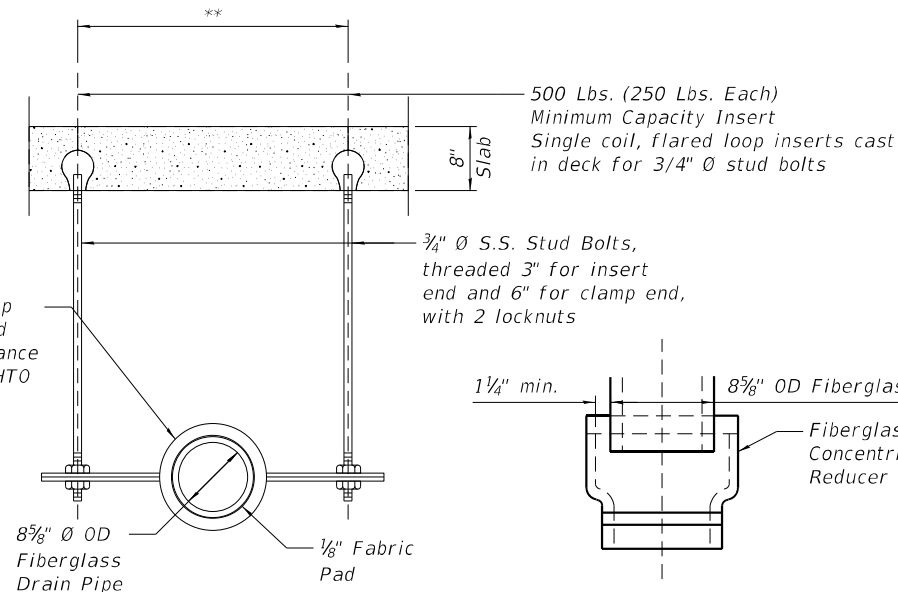
BILL OF MATERIALS

Item	Unit	Total
Drainage System	L. Sum	1



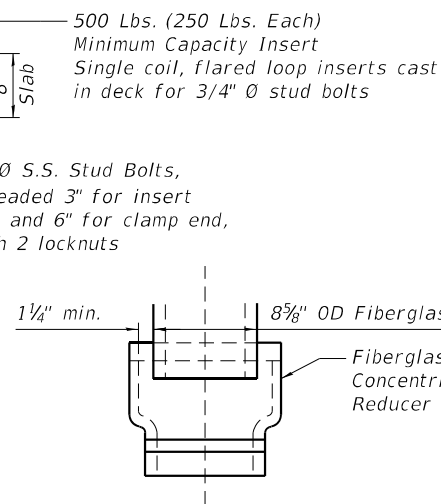
PIPE SUPPORT DETAIL

* Provide curved galvanized C6x8.2 to fit Round Pier Columns where needed

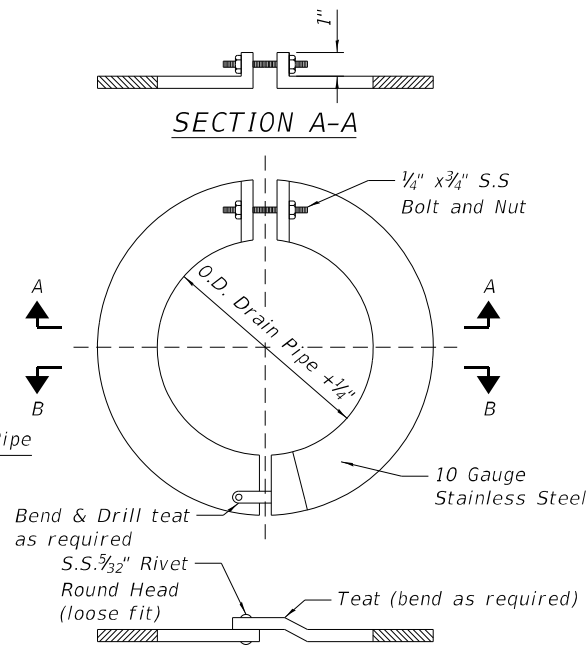


PIPE SUPPORT DETAIL

** Dimension as required by Pipe Clamp



REDUCER DETAIL



SECTION B-B

DETAIL OF EXPANSION COLLAR

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CHECKED - JMK
PLOT SCALE = 0.1667' / in.
PLOT DATE = 6/6/2023

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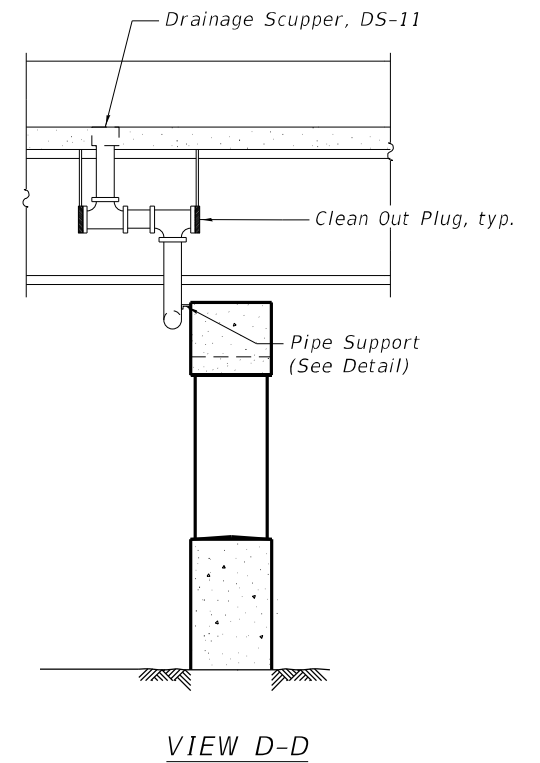
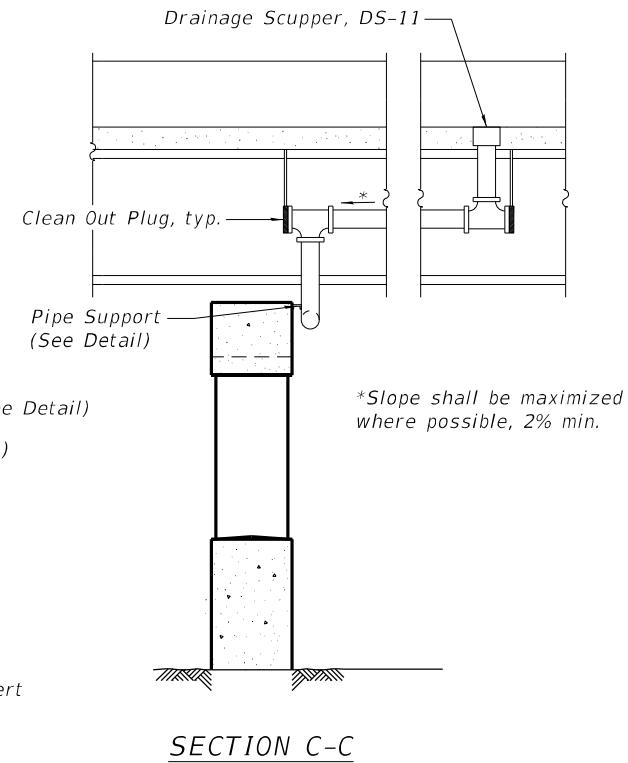
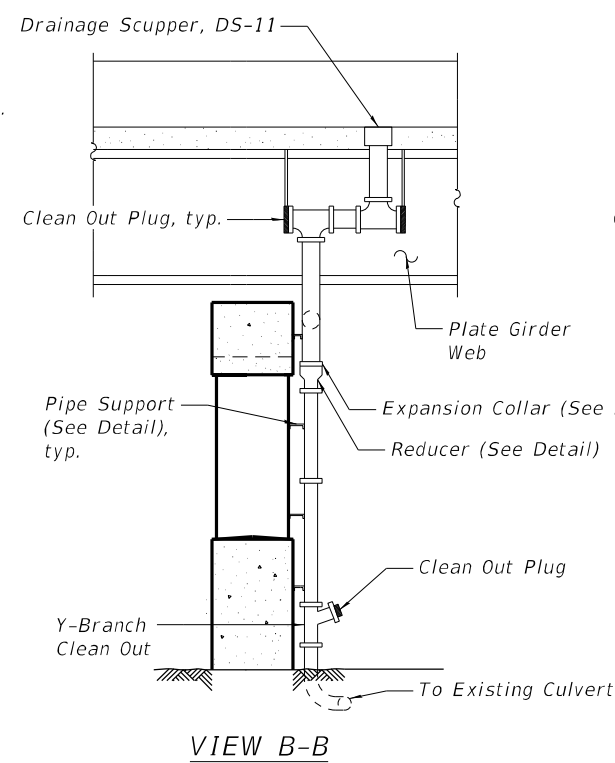
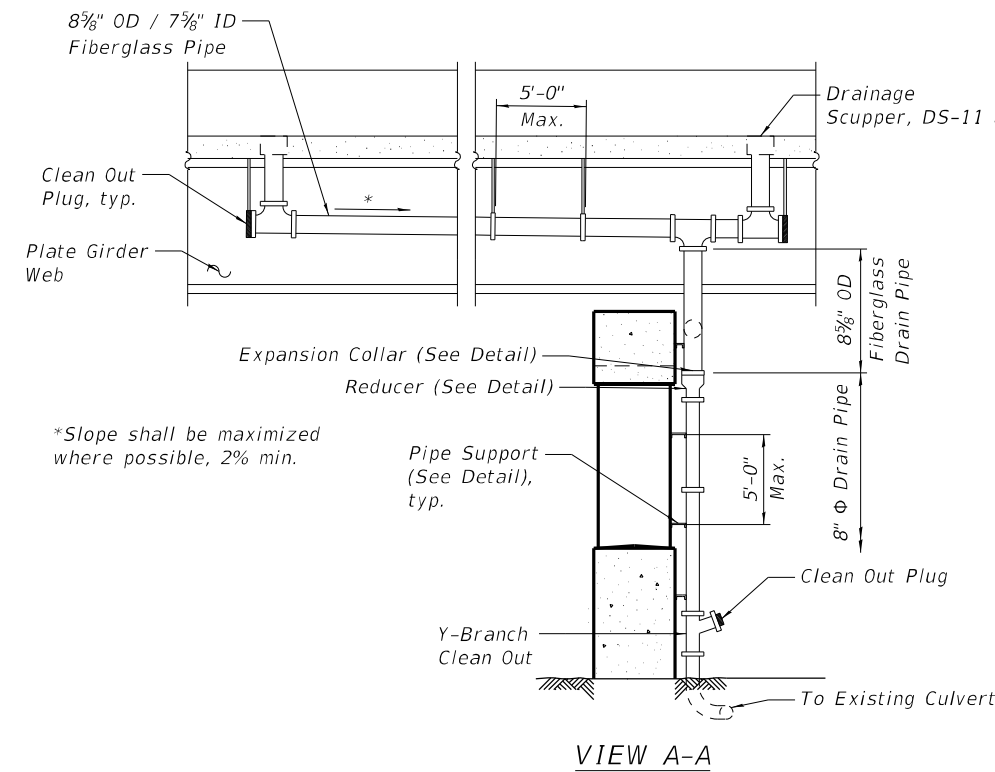
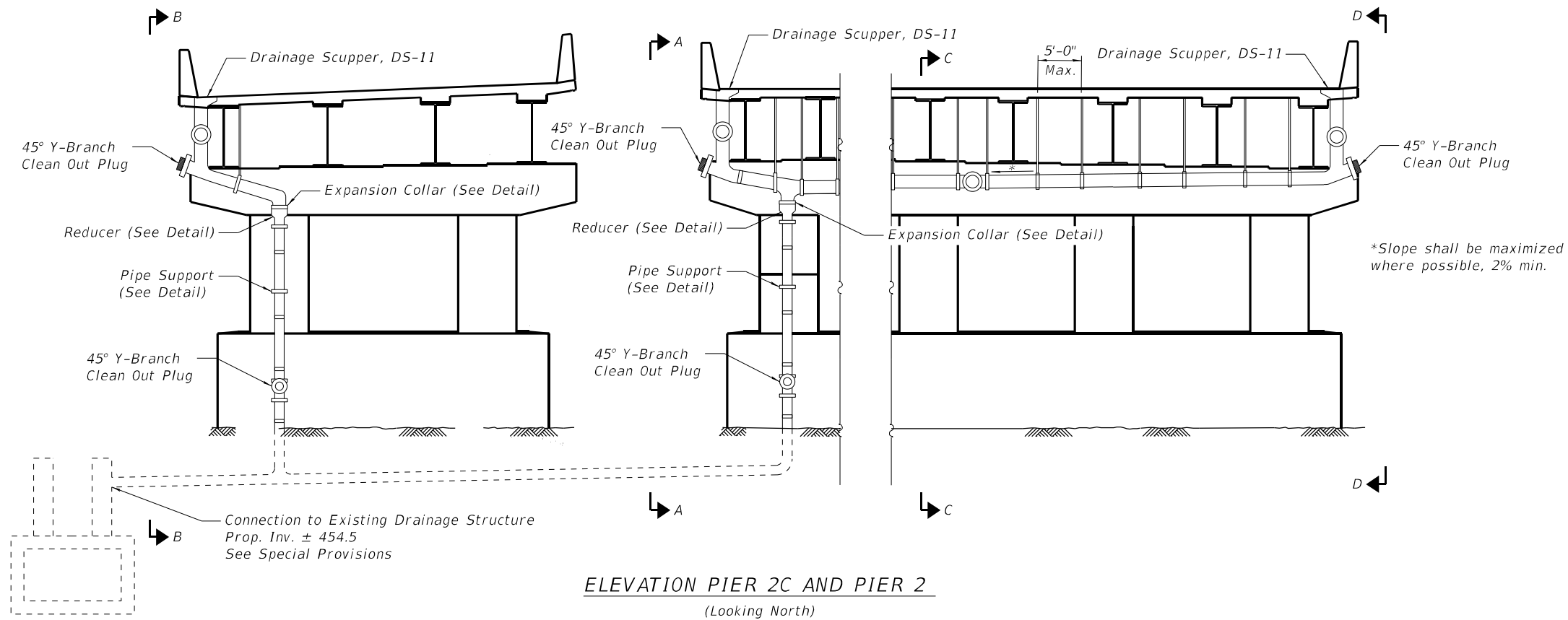
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DRAINAGE SYSTEM DETAILS 1
STRUCTURE NO. 090-0182

SHEET 5-179 OF 5-194 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	705
				CONTRACT NO. 68C55
				ILLINOIS FED. AID PROJECT

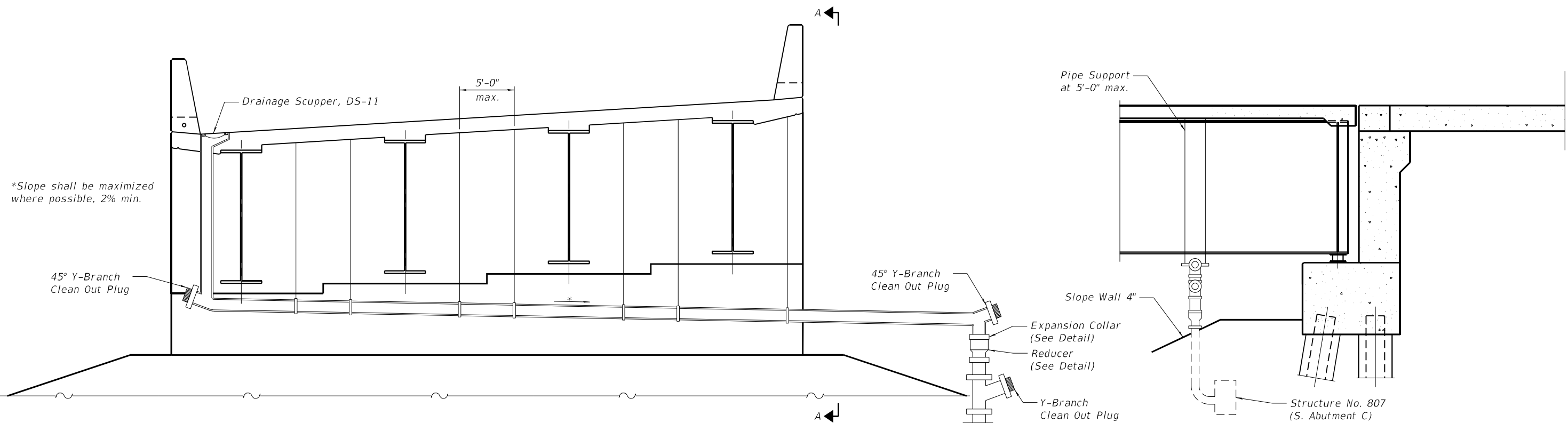


NOTES:
1. For drainage system notes, see Sheet S-179.

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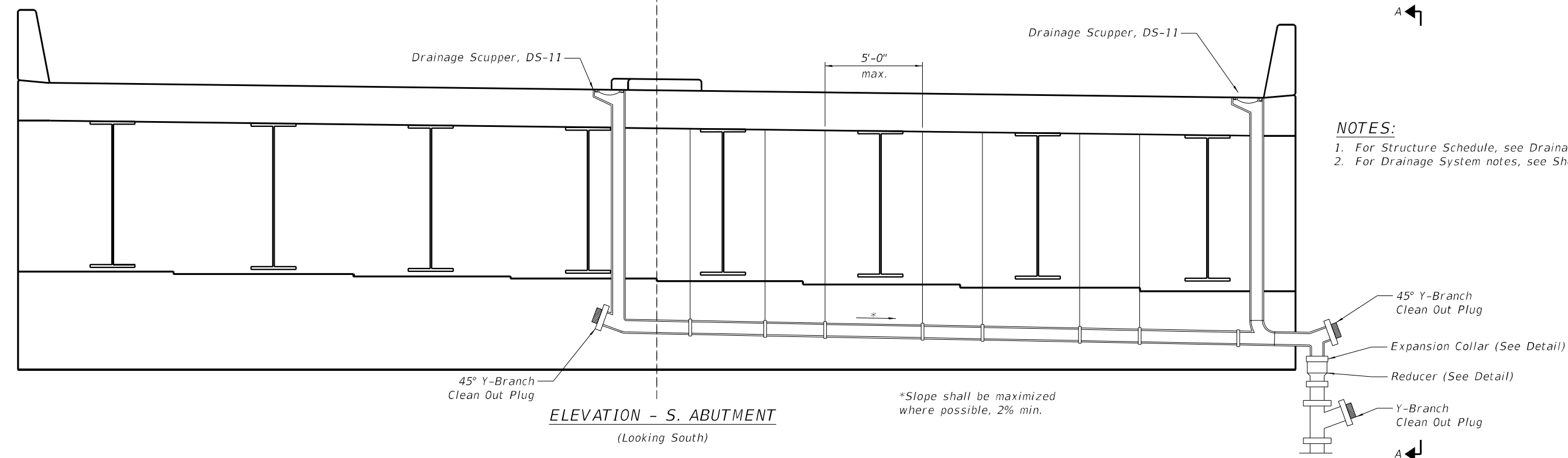
Michael Baker International 200 W Adams Street Suite 1500 Chicago, IL 60606 PH: 312.707.8910 FAX: 312.707.8854	USER NAME = DLamb		DESIGNED - MLK	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	DRAINAGE SYSTEM DETAILS 2		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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	PLOT DATE = 6/6/2023		CHECKED - JMK	REVISED -				ILLINOIS FED. AID PROJECT				

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ELEVATION - S. ABUTMENT D
(Looking South, S. Abutment C mirrored)

VIEW A-A
(S. Abutment D Shown and S. Abutment C similar)



ELEVATION - S. ABUTMENT
(Looking South)

- NOTES:**
1. For Structure Schedule, see Drainage Plans.
 2. For Drainage System notes, see Sheet S-179.

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USER NAME = DLamb	DESIGNED - MLK	REVISED -
CHECKED - JMK	REVISED -	
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DRAINAGE SYSTEM DETAILS 3
STRUCTURE NO. 090-0182

SHEET 5-181 OF S-194 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	707
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

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Lombard/IL/60148
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Fax: 6309539938

BORING LOG BSB-03

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 456.38 ft
North: 1456821.38 ft
East: 2457808.73 ft
Station: 401+41.23
Offset: 6.70' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
452.4	12-inch thick, dark brown SILTY LOAM, trace gravel --FILL-- --roadway--	1	13	6	5	NP	20	435.9	--RDR 2-- --L ₁ (%)=42, P ₁ (%)=28-- --%Gravel=0.2-- --%Sand=7.8-- --%Silt=71.9-- --%Clay=20.2-- --A-7-6 (15)-- Soft to medium stiff, gray CLAY, trace organic matter	9	3	1	2	0.82	31
452.4	Medium stiff to stiff, brown and gray SILTY CLAY to SILTY CLAY LOAM, trace organic matter, damp to moist --FILL-- --RDR 2--	2	3	1	1	0.66	26	430.9	--RDR 2-- Soft, gray GRAVELLY CLAY LOAM to LOAM, trace gravel	10	1	2	2	0.40	28
448.4	Medium stiff to stiff, brown and gray CLAY to SILTY CLAY, trace organic matter, damp to moist --RDR 2--	3	1	1	2	1.07	21	428.4	Very dense, gray and brown SANDY GRAVEL; moist to saturated --RDR 2 to 3-- --hard drilling to 33.5 feet-- --possible cobbles--	11	6	9	9	0.25	19
448.4	Medium stiff to stiff, brown and gray CLAY to SILTY CLAY, trace organic matter, damp to moist --RDR 2--	4	2	2	3	1.39	28	422.9	Very dense, gray weathered SHALE --RDR 2 to 3-- --MSPT 1--	12	13	34	43	NP	10
438.4	Soft, gray CLAY, little to some organic matter; moist --interbedded 0.5-inch thick peat lenses--	8	2	1	2	0.49	55			13	18	NP	NP	14	
										14	100/8	NP	NP	14	
										15	100/6	NP	NP	14	

GENERAL NOTES

Begin Drilling **06-04-2019** Complete Drilling **06-04-2019**
Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
Driller **K&N&K** Logger **E. Yim** Checked by **C. Marin**
Drilling Method **3.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **25.50 ft**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual



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BORING LOG BSB-03

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 456.38 ft
North: 1456821.38 ft
East: 2457808.73 ft
Station: 401+41.23
Offset: 6.70' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
407.4	Boring terminated at 49.00 ft	16	100/6.5	NP	14					17	100/5	NP	14		
		17	100/5	NP	14					18	100/6	NP	11		
		18	100/6	NP	11					19	100/5.5	NP	13		

GENERAL NOTES

Begin Drilling **06-04-2019** Complete Drilling **06-04-2019**
Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
Driller **K&N&K** Logger **E. Yim** Checked by **C. Marin**
Drilling Method **3.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **25.50 ft**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual



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BORING LOG BSB-04

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 456.20 ft
North: 1456807.18 ft
East: 2457815.26 ft
Station: 75+97.69
Offset: 0.13' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
452.1	Loose, brown, damp SILTY LOAM to SILTY CLAY, trace to little gravel --FILL-- --RDR 2--	1	4	2	2	1.00	18								
452.1	Medium stiff to stiff, brown, damp to moist SILTY CLAY to SILTY CLAY LOAM, trace gravel --FILL--	2	2	2	4	0.90	18								
450.7	Loose, brown, damp GRAVEL --FILL--	2	2	2	4	0.90	18								
449.4	Very soft, black, brown and gray SILTY CLAY; wet --FILL--	3	13	7	8	0.16	19								
449.4	Very loose to medium dense, black and gray, saturated GRAVELLY SAND --FILL-- --RDR 3 to 5-- --possible cobbles--	4	2	1	1	NP	16								
443.2	--rig chatter-- --possible cobbles--	5	6	4	2	NP	20								
443.2	Very dense, saturated SANDY LOAM, trace to little gravel --RDR 4 to 5-- --FILL-- --rig chatter, possible cobbles or concrete-- --AUGER REFUSAL, likely reinforced concrete-- Boring terminated at 14.60 ft	6	1	31	50	NP	27								

GENERAL NOTES

Begin Drilling **06-04-2019** Complete Drilling **06-04-2019**
Drilling Contractor **Wang Testing Services** Drill Rig **D50 ATV [88%]**
Driller **K&N&K** Logger **E. Yim** Checked by **C. Marin**
Drilling Method **3.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **5.70 ft**
At Completion of Drilling **DRY**
Time After Drilling **NA**
Depth to Water **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual

Michael Baker International

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USER NAME = D1amb	DESIGNED -	REVISED -
	CHECKED -	REVISED -
PLOT SCALE = 0:2.0000 " = 1" / in.	DRAWN -	REVISED -
PLOT DATE = 6/6/2023	CHECKED -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOGS 3
STRUCTURE NO. 090-0182

SHEET 5-184 OF 5-194 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	710
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

MODEL: Default
FILE NAME: p:\mms\bus-us-bentley.com\mmsbus-us-pw\03\Documents\Chicago_IL\DOT_CedarStExt_Phil15.0_CADD\CAD_Sheets\0900187-68C55-185-Bor4.dgn



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BORING LOG BSB-05

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 455.99 ft
North: 1456763.75 ft
East: 2457753.61 ft
Station: 302+61.75
Offset: 2.34' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
447.3	Soft, brown, black and gray SILTY CLAY to SILTY CLAY LOAM, trace to little gravel; damp to wet --FILL-- --RDR 2--	1	4	1	NA	17		435.5	--RDR 2-- Soft to medium stiff, gray CLAY to SILTY CLAY, trace organic matter; wet --RDR 1 to 2--	9	2	3	0.74	25	
		2	1	1	0.40	18			--wet silt lenses--	10	2	2	0.57	27	
		3	1	1	0.25	22		429.5	Medium dense, gray SANDY GRAVEL, trace silty clay lenses; moist --RDR 2--	11	6	10	0.33	28	
	Medium stiff, black SILTY CLAY, trace gravel; moist to wet --BURIED TOPSOIL-- --RDR 2--	4	2	1	0.50	30				12	4	8	NP	12	
444.7	Soft to very stiff, gray and brown SILTY CLAY to SILTY CLAY LOAM; moist to wet --RDR 2--	5	2	2	1.89	24		424.2	Very dense, bluish gray, weathered SHALE --RDR 2--	14	21	30	NP	15	
	--L _t (%)=45, P _t (%)=22-- --%Gravel=0.1-- --%Sand=3.8-- --%Silt=66.8-- --%Clay=29.3-- --A-7-6 (24)--	6	2	3	2.05	25				15	15	30	NP	15	
		7	2	1	0.98	27		421.0	Extremely weak, gray, fair to good rock mass quality, SHALE; closely spaced, moderately to highly weathered, horizontal and oblique joints, with no opening, slicken rough walls. --Run 1: 35.0 to 40.0 feet-- --Recovery = 96%-- --RQD = 63%-- --Q _u at 38.0 feet=1927 psi--	15	15	30	NP	15	
437.0	Stiff, gray CLAY, little organic matter; wet	8	2	2	1.50	44				15	15	30	NP	15	
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling 06-04-2019 Complete Drilling 06-04-2019								While Drilling DRY							
Drilling Contractor Wang Testing Services Drill Rig D50 ATV [88%]								At Completion of Drilling 21.00 ft							
Driller K&N&K Logger E. Yim Checked by C. Marin								Time After Drilling NA							
Drilling Method 3.25" HSA; boring backfilled upon completion								Depth to Water NA							
								The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual							



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BORING LOG BSB-05

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 455.99 ft
North: 1456763.75 ft
East: 2457753.61 ft
Station: 302+61.75
Offset: 2.34' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
	--Run 2: 40.0 to 45.0 feet-- --Recovery = 85%-- --RQD = 82%--	17													
	--Q _u at 44.0 feet=175 psi--	45													
	--Run 3: 45.0 to 50.0 feet-- --Recovery = 100%-- --RQD = 77%--	18													
	--Q _u at 48.0 feet= 538 psi--	50													
406.0	Boring terminated at 50.00 ft	50													
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling 06-04-2019 Complete Drilling 06-04-2019								While Drilling DRY							
Drilling Contractor Wang Testing Services Drill Rig D50 ATV [88%]								At Completion of Drilling 21.00 ft							
Driller K&N&K Logger E. Yim Checked by C. Marin								Time After Drilling NA							
Drilling Method 3.25" HSA; boring backfilled upon completion								Depth to Water NA							
								The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual							



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BORING LOG BSB-06

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 461.62 ft
North: 1456648.25 ft
East: 2457979.81 ft
Station: 403+84.58
Offset: 1.08' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Q _u (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Q _u (tsf)	Moisture Content (%)
458.6	Medium dense, brown SANDY GRAVEL; moist --FILL-- --RDR 2--	9		1	9 10 11	NP	10	441.1	Soft, gray SILTY CLAY, trace to little organic matter --RDR 1 to 2--	9		9	2 1 2	0.41 B	37
456.1	Medium stiff, brown, gravelly LOAM; moist --FILL-- --RDR 2--	5		2	3 4 4	0.75 P	15	436.1	Stiff, gray CLAY to SILTY CLAY, some gravel, trace organic matter; wet --RDR 2--	10		10	2 2 2	0.49 B	46
453.6	Loose, reddish SANDY GRAVEL, trace slag; wet --FILL-- --RDR 1--	3		3	2 2 2	NP	20	433.6	Medium dense, gray SANDY LOAM; wet --RDR 2--	11		11	6 8 7	1.00 P	31
451.1	Soft, brown and black CLAY LOAM --FILL-- --RDR 1--	10		4	1 1 2	0.41 B	24	432.6	Medium dense, brown SANDY GRAVEL; saturated --RDR 2 to 4-- --frequent rig chatter-- --possible cobbles--	12		12	3 4 10	NP	25
	Medium stiff to stiff, dark gray to gray CLAY to SILTY CLAY, trace organic matter --RDR 1--	15		5	2 3 4	1.07 B	36	426.9	Very dense, greenish gray SHALE --RDR 4-- --MSPT 1--	13		13	11 8 7	NP	8
	--L _t (%)=43, P _t (%)=17-- --%Gravel=0.2-- --%Sand=4.4-- --%Silt=69.3-- --%Clay=26.1-- --A-7-6 (26)--	20		7	2 2 2	0.66 B	29		--MSPT 2--	14		14	00/15	NP	13
		20		8	2 2 3	1.48 B	29			15		15	00/8.5	NP	14
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling 06-03-2019 Complete Drilling 06-04-2019								While Drilling 5.50 ft							
Drilling Contractor Wang Testing Services Drill Rig 18D25A [85%]								At Completion of Drilling 4 ft mud							
Driller K&N&K Logger F. Bozga Checked by C. Marin								Time After Drilling 24 hours							
Drilling Method 2.25" HSA to 10 ft, mud rotary thereafter, boring backfilled upon completion								Depth to Water 5.5 ft mud ft							
								The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual							

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USER NAME = D1amb	DESIGNED -	REVISED -
	CHECKED -	REVISED -
PLOT SCALE = 0:2.0000 " = 1" / in.	DRAWN -	REVISED -
PLOT DATE = 6/6/2023	CHECKED -	REVISED -

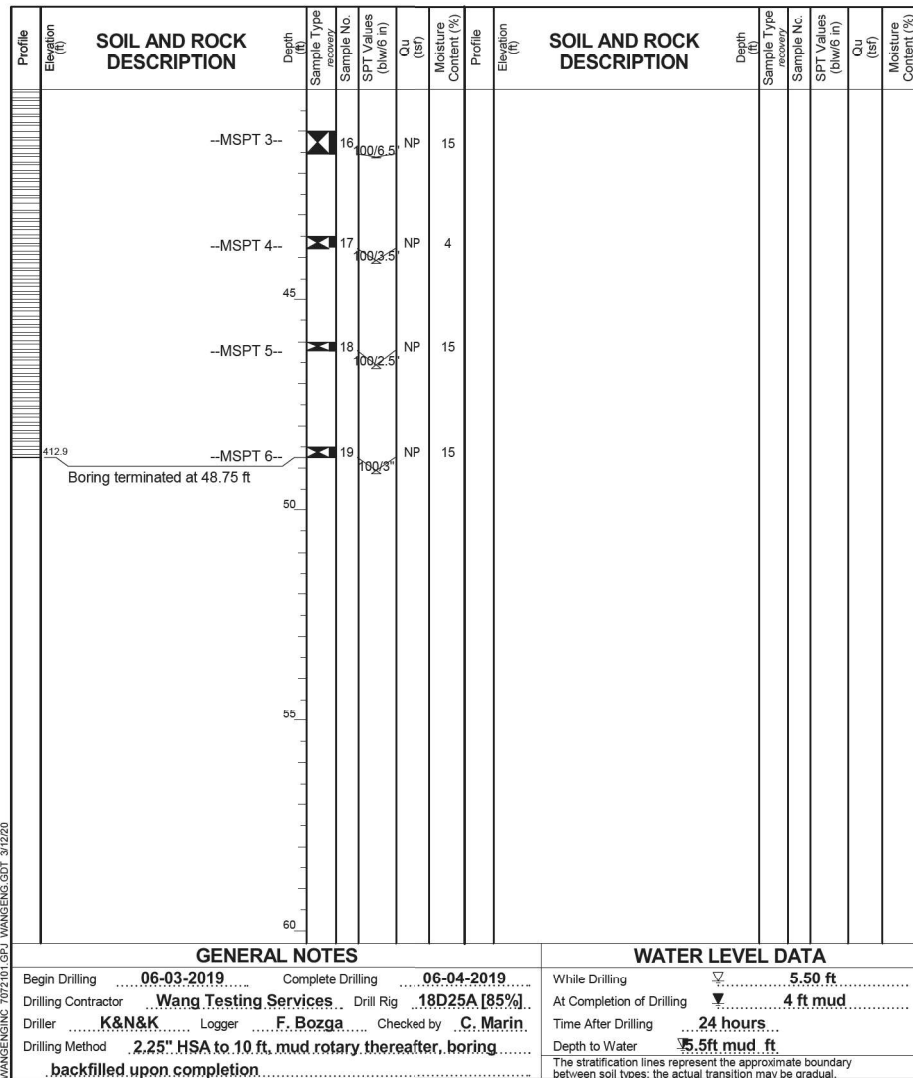
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOGS 4
STRUCTURE NO. 090-0182

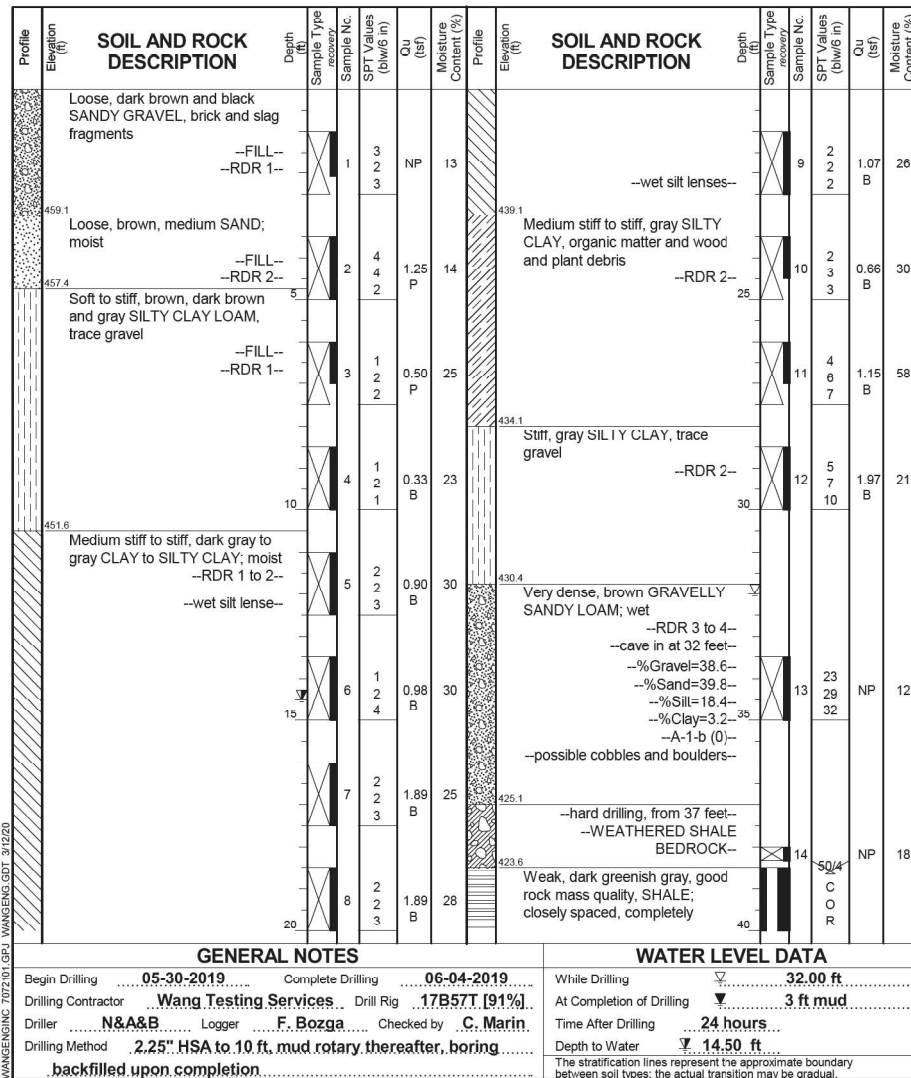
SHEET 5-185 OF 5-194 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	711
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

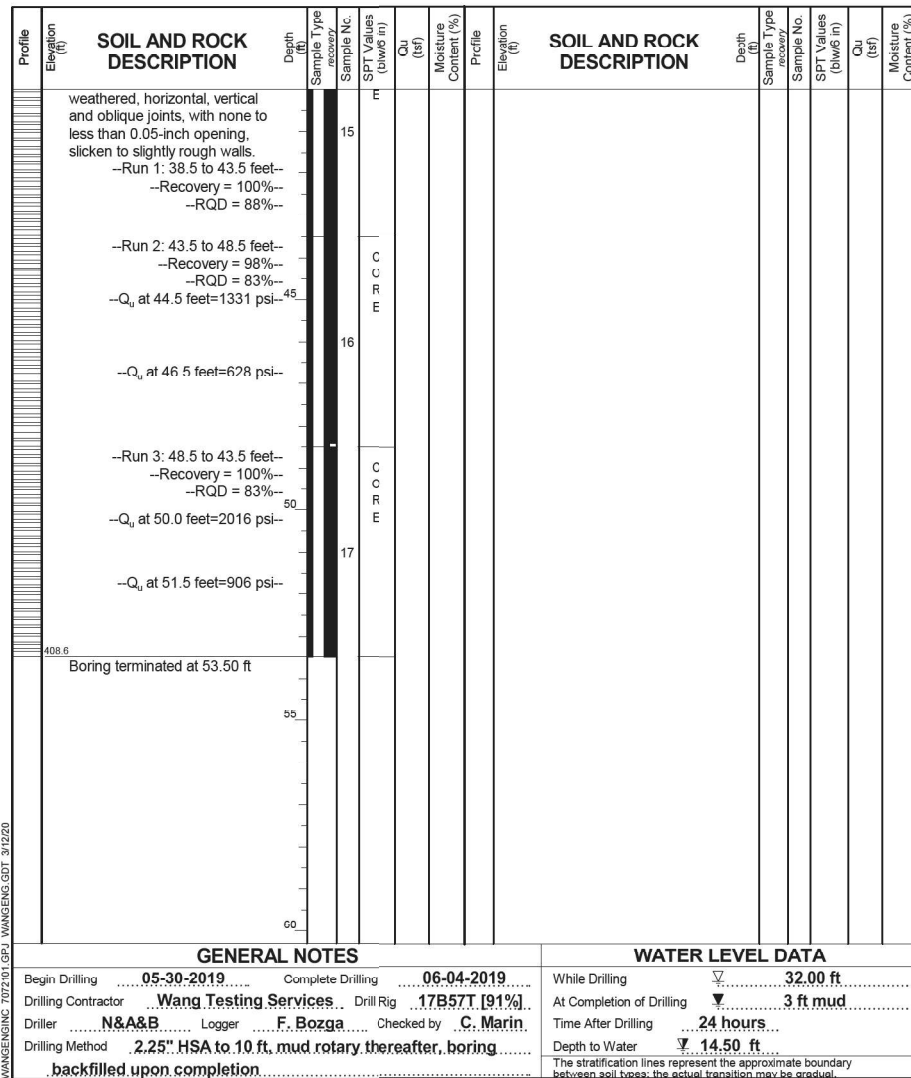
		BORING LOG BSB-06		Page 2 of 2	
wangeng@wangeng.com 1145 N. Main Street Lombard, IL 61048 Telephone: 6309539928 Fax: 6309539938		WEI Job No.: 707-21-01 Michael Baker International Cedar Street Extension at IL Route 29 Tazewell County, IL		Datum: NAVD 88 Elevation: 461.62 ft North: 1456648.25 ft East: 2457979.81 ft Station: 403+84.58 Offset: 1.08' Lt	



 wangeng@wangeng.com 1145 N. Main Street Lombard/IL/60148 Telephone: 6309539928 Fax: 6309539938	BORING LOG BSB-07 WEI Job No.: 707-21-01		Datum: NAVD 88 Elevation: 462.10 ft North: 1456601.35 ft East: 2457953.20 ft Station: 78+65.24 Offset: 5.40' Rt
	Client	Michael Baker International	
	Project	Cedar Street Extension at IL Route 29	
	Location	Tazewell County, IL	



 wangeng@wangeng.com 1145 N. Main Street Lombard/IL 60148 Telephone: 6309539928 Fax: 6309539938	BORING LOG BSB-07 WEI Job No.: 707-21-01		Datum: NAVD 88 Elevation: 462.10 ft North: 1456601.35 ft East: 2457953.20 ft Station: 78+65.24 Offset: 5.40' Rt
	Client Michael Baker International Project Cedar Street Extension at IL Route 29 Location Tazewell County, IL		



MODEL: Default
FILE NAME: p:\mbs-us-h-bentley.com\mb-us-pw\03\Documents\Chicago_IL\DOT_CedarStExt_P1115.0_CADD\CAD_Sheets\0900187-68C55-187-68C55.dgn
WANGENG INC 7072101.GPJ WANGENG.GDT 3/12/20



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BORING LOG BSB-08

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 461.91 ft
North: 1456586.70 ft
East: 2457922.92 ft
Station: 305+06.31
Offset: 16.79' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
	Medium dense, dark brown SANDY GRAVEL; moist --FILL-- --RDR 2--	1		7	6 5	NP	17	441.4	Medium stiff, gray SILTY CLAY LOAM, some organic and plant material --RDR 1--	9		1	0.96	35	
458.9	Brown, coarse SAND; moist --FILL-- --RDR 1--	2		2	2 2 2	0.74	22		--L ₁ (%)=38, P ₁ (%)=21-- --%Gravel=0.3-- --%Sand=5.9-- --%Silt=73.8-- --%Clay=20.1-- --A-6 (17)--	10		1	0.90	42	
457.9	Soft to medium stiff, dark brown to brown CLAY LOAM --FILL-- --RDR 1--	3		1	1 1	0.33	24	436.4	Stiff, dark brown SILTY CLAY LOAM, trace organic matter and peat lenses --RDR 1--	11		4	1.00	96	
		4		1	2 2	0.57	26	433.9	Medium dense, brown, coarse GRAVELLY SAND --RDR 2--	12		4	7	NP	17
451.4	Medium stiff to stiff, dark gray to gray CLAY to SILTY CLAY; moist --RDR 2--	5		1	2 2 2	0.82	34			13		6	14	NP	15
		6		2	4 5	1.07	28	428.4	Very dense, greenish gray SHALE --RDR 4 to 5--	14		00/19	NP	15	
		7		2	4 4	1.80	24		--MSPT 1--	15		00/18	NP	15	
	--trace plant material--	8		2	3 3	1.56	26		--MSPT 2--	16					
		20								40					
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling 05-30-2019 Complete Drilling 05-31-2019								While Drilling 28.00 ft							
Drilling Contractor Wang Testing Services Drill Rig 17D50T [81%]								At Completion of Drilling 3 ft mud							
Driller N&A&B Logger F. Bozga Checked by C. Marin								Time After Drilling NA							
Drilling Method 2.25" HSA to 10 ft, mud rotary thereafter, boring								Depth to Water NA							
backfilled upon completion								The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual							



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BORING LOG BSB-08

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 461.91 ft
North: 1456586.70 ft
East: 2457922.92 ft
Station: 305+06.31
Offset: 16.79' Lt

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)
		--MSPT 3--	16	100/7	NP	14											
		--MSPT 4--	17	190/4	NP	18											
		--MSPT 5--	18	190/3	NP	19											
	413.2	--MSPT 6--	19	190/3	NP	19											
		Boring terminated at 48.75 ft															
GENERAL NOTES										WATER LEVEL DATA							
Begin Drilling 05-30-2019 Complete Drilling 05-31-2019										While Drilling 28.00 ft							
Drilling Contractor Wang Testing Services Drill Rig 17D50T [81%]										At Completion of Drilling 3 ft mud							
Driller N&A&B Logger F. Bozga Checked by C. Marin										Time After Drilling NA							
Drilling Method 2.25" HSA to 10 ft, mud rotary thereafter, boring										Depth to Water NA							
backfilled upon completion										The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual							



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BORING LOG BSB-09

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 462.82 ft
North: 1456499.08 ft
East: 2458129.87 ft
Station: 405+96.07
Offset: 1.78' Rt

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
		Loose, black, gray and reddish SANDY GRAVEL; moist to wet --FILL-- --RDR 2-- --RAILROAD BALLAST--			1	6 3 3	NP	12		442.3	Medium dense to dense, brown SANDY GRAVEL, trace cobbles and boulders; saturated --RDR 3 to 4--			9	5 12 12	NP	7
					2	2 2 2	NP	24						10	5 20 21	NP	9
					3	3 3 2	NP	14		437.3	Dense, brown, medium SANDY LOAM, little gravel; saturated --RDR 2--			11	10 16 17	NP	22
	454.8	Very soft, dark gray SILTY CLAY LOAM, trace organic matter; wet --RDR 1--			4	2 1 1	0.16 B	31		434.8	Medium dense, brown GRAVELLY SAND; saturated --RDR 2-- --%Gravel=29.1-- --%Sand=57.1--30 --%Silt=12.2-- --%Clay=1.6-- --A-1-b (0)--			12	7 9 10	NP	15
	452.3	Soft, black PEAT; wet --RDR 1--			5	1 1 2	0.49 B	165			--hard drilling, 32.5 feet--						
	449.8	Medium stiff, gray SILTY CLAY LOAM, some organic matter and plant debris; wet --RDR 1--			6	1 1 1	0.74 B	42		430.3	Very dense, greenish gray SHALE --RDR 4--			13	20 40 45	NP	17
	447.3	Soft, gray SILTY CLAY LOAM, trace organic matter --RDR 1--			7	1 1 1	0.33 B	28			--MSPT 1--			14	00/11	NP	16
	444.8	Medium dense, brown SANDY LOAM, some gravel; wet --RDR 2--			8	7 5 6	NP	8			--MSPT 2--			15	100/19.5	NP	16
GENERAL NOTES									WATER LEVEL DATA								
Begin Drilling 05-28-2019 Complete Drilling 05-28-2019									While Drilling 7.00 ft								
Drilling Contractor Wang Testing Services Drill Rig 17B57T [91%]									At Completion of Drilling 3 ft mud								
Driller N&A&N Logger F. Bozga Checked by C. Marin									Time After Drilling NA								
Drilling Method 2.25" HSA to 10 ft, mud rotary thereafter, boring									Depth to Water NA								
backfilled upon completion									The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.								

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USER NAME = D1amb
DESIGNED -
CHECKED -
DRAWN -
PLOT SCALE = 0:2.0000 " = 1' / in.
PLOT DATE = 6/6/2023

DESIGNED -
CHECKED -
DRAWN -
PLOT DATE = 6/6/2023

REVISED -
REVISED -
REVISED -
REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOGS 6
STRUCTURE NO. 090-0182

SHEET 5-187 OF 5-194 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	713
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

[illegible]

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blows/6 in)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blows/6 in)	Qu (tsf)	Moisture Content (%)
	Very loose to loose, black SANDY GRAVEL, trace slag and brick fragments; moist to wet --FILL-- --RDR 1--			1	2 2 2	NP	10		--RDR 3 to 4--			9	12 14 14	NP	8
				2	2 1 1	NP	26			25		10	13 12 6	NP	11
				3	2 1 1	NP	34					11	11 13 13	NP	14
455.1	Very soft, gray CLAY, trace to little organic matter --RDR 1--			4	0 0 0	0.16 B	34			30		12	10 0 8	NP	16
452.6	Medium stiff, dark gray SILTY CLAY LOAM, trace organic matter --RDR 1-- --L ₁ (%)=39, P ₁ (%)=18-- --%Gravel=0.1-- --%Sand=7.7-- --%Silt=69.2-- --%Clay=23.0-- --A-6 (20)--			5	1 2 1	0.90 B	27								
				6	1 2 2	0.82 B	27	430.6 429.6	--slow drilling at 32.5 feet-- --WEATHERED SHALE BEDROCK--						
447.6	Medium dense, brown, GRAVELLY LOAM; wet --RDR 3 to 4-- --possible cobbles--			7	3 8 10	NP	12		Very weak, dark greenish gray, poor rock mass quality, SHALE; closely spaced, highly weathered, horizontal joints, with none to < 0.05-inch opening, slightly rough walls. --Run 1: 33.5 to 38.5 feet-- --Recovery = 66%-- --RQD = 43%-- --Q _a at 36.5 feet=139 psi--			13			
444.6	Medium dense, brown GRAVELLY SAND, trace cobbles; wet to saturated			8	8 12 10	NP	9		--Run 2: 38.5 to 43.5 feet-- --Recovery = 100%-- --RQD = 58%--						

GENERAL NOTES

Begin Drilling

05-29-2019

Complete Drilling

05-30-2019

Drilling Contractor

Wang Testing Services

Drill Rig

17B57T [91%]

Driller

N&A&B

Logger

F. Bozga

Checked by

C. Marin

Drilling Method

2.25" HSA to 10 ft, mud rotary thereafter, boring

backfilled upon completion

WATER LEVEL DATA

While Drilling

At Completion of Drilling

Time After Drilling

24 hours

Depth to Water

5.00 ft

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)
414.6	--Run 3: 43.5 to 48.5 feet-- --Recovery = 100%-- --RQD = 75%-- --Q _u at 46.0 feet=1003 psi--	14													
	Boring terminated at 48.50 ft	50													
		60													

GENERAL NOTES					WATER LEVEL DATA	
Begin Drilling	05-29-2019	Complete Drilling	05-30-2019	While Drilling	▽	5.50 ft
Drilling Contractor	Wang Testing Services	Drill Rig	17B57T [91%]	At Completion of Drilling	▼	3 ft mud
Driller	N&A&B	Logger	F. Bozga	Time After Drilling	24 hours	
Drilling Method	2.25" HSA to 10 ft, mud rotary thereafter, boring backfilled upon completion			Depth to Water	▽	5.00 ft
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.						

[illegible]

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (ksf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (ksf)	Moisture Content (%)
452.64	452.64-inch thick, black SILTY CLAY LOAM --TOPSOIL-- Medium dense to dense, brown, medium SAND to SANDY LOAM, trace gravel; moist --FILL-- --RDR 2--	1		1	5 8 12	NP	6	467.4	Very dense, black GRAVELLY SAND; damp to moist --FILL-- --RDR 2-- --possible cobbles--	9		9	7 6 11	2.05 B	16
464.9	Medium dense, brown GRAVELLY SAND; moist --FILL-- --RDR 2--	2		2	6 13 18	NP	8	464.9	Medium dense, brown, medium SAND; wet --FILL-- --RDR 2--	10		10	6 7 16	3.61 B	12
481.4	Stiff to hard, gray SILTY CLAY LOAM to CLAY LOAM, trace to little gravel --FILL-- --RDR 2--	3		3	5 8 11	NP	13	481.1	Stiff, dark gray SILTY CLAY LOAM, trace organic matter --RDR 2--	11		11	11 35 46	NP	22
		4		4	10 14 13	NP	9			12		12 12 8	NP	20	
		5		5	10 8 10	4.50 P	11			13		3 4 5	1.56 B	23	
		6		6	5 9 10	3.61 B	12			14		3 3 5	0.33 B	16	
		7		7	6 8 12	3.20 B	15								
		8		8	7 8 8	1.89 B	15								

GENERAL NOTES

Begin Drilling

05-21-2019

Complete Drilling

05-22-2019

Drilling Contractor

Wang Testing Services

Drill Rig

18D25A [85%]

Driller

N&K&N

Logger

F. Bozga

Checked by

C. Marin

Drilling Method

2.25" HSA to 15 ft, mud rotary thereafter, boring

backfilled upon completion

WATER LEVEL DATA

While Drilling

At Completion of Drilling

Time After Drilling

24 hours

Depth to Water

28.00 ft

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blows in)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blows in)	Qu (tsf)	Moisture Content (%)
451.1	Medium dense to very dense, brown GRAVELLY SANDY LOAM; wet to saturated --RDR 2 to 4--	45	X	15	7 8 11	NP	13	431.4	--hard drilling, 61.5 feet-- Very dense, greenish gray SHALE	65	X	19	60/5	NP	11
	--possible cobbles--	45	X	15	7 8 11	NP	13		--RDR 4--	65	X	19	60/5	NP	11
	--%Gravel=39.0-- --%Sand=46.9-- --%Silt=13.4-- --%Clay=0.7-- --A-1-b (0)--	50	X	16	9 14 15	NP	9		--MSPT 1--	65	X	20	00/10		16
	--possible cobbles--	50	X	16	9 14 15	NP	9		--MSPT 2--	65	X	21	00/8.5		15
		50	X	16	9 14 15	NP	9		--MSPT 3--	70	X	22	00/6.5		19
		50	X	16	9 14 15	NP	9		--MSPT 4--	70	X	23	100/7		16
		50	X	16	9 14 15	NP	9		--MSPT 5--	75	X	24	09/3.5		14
		50	X	16	9 14 15	NP	9		--MSPT 6--	75	X	25	08/2.5		20
436.1	Soft, gray SILTY CLAY --highly weathered shale-- --RDR 1--	60	X	18	5 3 5	0.33 B	29	416.7	Boring terminated at 76.25 ft	80					
		60	X	18	5 3 5	0.33 B	29			80					

GENERAL NOTES

05-21-2019

05-22-2019

Begin Drilling

Complete Drilling

Drilling Contractor

Drill Rig

Wang Testing Services

18D25A [85%]

Driller

Logger

Checked by

N&K&N

F. Bozga

C. Marin

Drilling Method

2.25" HSA to 15 ft, mud rotary thereafter, boring

backfilled upon completion

WATER LEVEL DATA

While Drilling

At Completion of Drilling

Time After Drilling

Depth to Water

28.00 ft

5 ft mud

24 hours

28.00 ft

The stratification lines represent the approximate boundary between soil types; the actual penetration may be greater.

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WANGENG INC 7072101.GPJ WANGENG.GDT 3/12/20



wangeng@wangeng.com
1145 N. Main Street
Lombard/IL/60148
Telephone: 6309539928
Fax: 6309539938

BORING LOG BSB-14

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 492.29 ft
North: 1456331.51 ft
East: 2458133.21 ft
Station: 81+85.09
Offset: 59.51' Rt

Page 1 of 2

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	
	491.5	5-inch thick, brown TOPSOIL																
	491.3	--TOPSOIL--																
	491.3	Gray SILTY CLAY LOAM to SILTY CLAY, trace gravel																
		--FILL--																
		Medium dense to dense, brown SAND, trace to little gravel; damp to wet																
		--FILL--																
		--RDR 2--																
			1		11	11	NP	7						9	4	8	4.84	11
			2		5	13	NP	7						10	7	8	2.38	12
			5		16	16						25			11			
			3		7	14	NP	10										
					15	15								11	12	8	1.56	16
															7			
	483.5	Stiff to hard, brown and gray SILTY CLAY LOAM to CLAY LOAM trace to little gravel; damp to moist	10		4	5	1.31	11										
		--FILL--																
		--RDR 2--																
			5		3	5	3.20	13										
					7	7												
			6		5	5	2.21	13										
					6	6												
			15															
		--2-inch thick silt lens--																
			7		5	7	4.10	15										
					8	8												
		--possible cobbles--																
			8		4	6	3.69	12										
					8	8												
			20															

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WANGENG INC 7072101.GPJ WANGENG.GDT 3/12/20



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BORING LOG BSB-15

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 494.68 ft
North: 1456172.28 ft
East: 2458084.27 ft
Station: 309+40.61
Offset: 43.32' Lt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
452.9	Very stiff, brown SILTY CLAY, trace gravel; damp --RDR 2--								--MSPT 3--			20	100/6	NP	13
	Dense, brown SAND, trace gravel; saturated --RDR 2--								--MSPT 4--			21	100/6	NP	13
45				15	11 18 25	NP	17		--MSPT 5--			22	100/2	NP	10
447.9	Stiff, reddish brown SILTY CLAY to SILTY CLAY LOAM, some gravel; damp to moist --RDR 2-- --1-inch gravel lens; wet to saturated--								--MSPT 6--			23	100/3	NP	12
				16	11 10 18	1.80 S	10								
442.9	Very dense, bluish gray LOAM with weathered shale; damp to moist --WEATHERED BEDROCK-- --RDR 2 to 3--														
				17	35 39 33	NP	10								
439.2	Very dense, light bluish gray, weathered SHALE --RDR 2-- --MSPT 1--														
				18	100/8.5	NP	13								
				19	100/9"	NP	14								
60															
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling				Complete Drilling				While Drilling				At Completion of Drilling			
06-13-2019				06-13-2019				31.75 ft				mud in borhole			
Drilling Contractor				Wang Testing Services				NA				NA			
Driller				K&K&G				NA				NA			
Logger				E. Yim				NA				NA			
Checked by				C. Marin				NA				NA			
Drilling Method				3.25" HSA to 25 ft, mud rotary thereafter, boring				NA				NA			
				backfilled upon completion											



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BORING LOG RWB-11

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 466.56 ft
North: 1456439.18 ft
East: 2458132.27 ft
Station: 57+52.31
Offset: 41.76' Rt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
456.1	CONCRETE DEBRIS														
445.2	Medium dense, black and brown SILTY LOAM, some gravel; damp --FILL-- --RDR 2--											9	3 10 24	NP	16
	Loose, gray, black and red SANDY GRAVEL to GRAVELLY SAND; damp --FILL-- --RDR 2--														
				2	5 9 4	NP	18		443.6	Brown SAND, trace gravel; saturated					
				3	5 5 4	NP	15		442.0	Medium dense, brown SANDY GRAVEL; saturated					
				4	1 4 2	NP	25		441	Stiff, brown SILTY CLAY LOAM, little gravel; moist					
				5	1 1 2	0.66 B	45			--RDR 2--					
				6	1 1 2	0.66 B	25			--RDR 2--					
				7	5 2 5	NP	13			--RDR 2--					
				8	7 5 8	NP	10			--RDR 2--					
456.1	Medium stiff, black SILTY CLAY, trace gravel and organics; wet --BURIED TOPSOIL-- --RDR 2--														
451.1	Loose, brown LOAM, little gravel; moist --RDR 2--								436.1	Stiff (1.64B), brown SILTY CLAY, trace gravel; moist					
									435.1	--silt laminations-- --RDR 2--					
										Very soft to soft, gray CLAY, trace gravel; wet					
										--3-inch thick, silty loam; saturated-- --RDR 2--					
										--silt laminations-- --RDR 2--					
										--1-inch sand lenses; saturated--					
									431.1	Very dense, bluish gray WEATHERED SHALE					
										--RDR 2--					
448.6	Medium dense to dense, brown SANDY GRAVEL; saturated --RDR 2--														
428.6	Extremely weak, light greenish gray, excellent rock mass quality, SHALE; very widely spaced, completely weathered, horizontal														
40															
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling				Complete Drilling				While Drilling				At Completion of Drilling			
05-30-2019				05-30-2019				18.00 ft				11.50 ft			
Drilling Contractor				Wang Testing Services				NA				NA			
Driller				K&N&K				NA				NA			
Logger				E. Yim				NA				NA			
Checked by				RKC				NA				NA			
Drilling Method				3.25" HSA; boring backfilled upon completion				NA				NA			



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BORING LOG RWB-11

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88
Elevation: 466.56 ft
North: 1456439.18 ft
East: 2458132.27 ft
Station: 57+52.31
Offset: 41.76' Rt

Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/ft)	Qu (tsf)	Moisture Content (%)
	and oblique joints, with no opening, slicken walls, and no infill. --Run 1: 38.0 to 43.0 feet-- --RECOVERY = 100%-- --RQD = 90%--														
423.6	Extremely weak, gray, excellent rock mass quality, SHALE; very widely spaced, completely weathered, horizontal joints, with no opening, slicken walls, and no infill. --Run 2: 43.0 to 48.0 feet-- --RECOVERY = 100%-- --RQD = 82%--														
418.6	Boring terminated at 48.00 ft														
50															
60															
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling				Complete Drilling				While Drilling				At Completion of Drilling			
05-30-2019				05-30-2019				18.00 ft				11.50 ft			
Drilling Contractor				Wang Testing Services				NA				NA			
Driller				K&N&K				NA				NA			
Logger				E. Yim				NA				NA			
Checked by				RKC				NA				NA			
Drilling Method				3.25" HSA; boring backfilled upon completion				NA				NA			

Michael Baker International
200 W Adams Street
Suite 1500
Chicago, IL 60606
PH: 312.707.8970
FAX: 312.707.8854

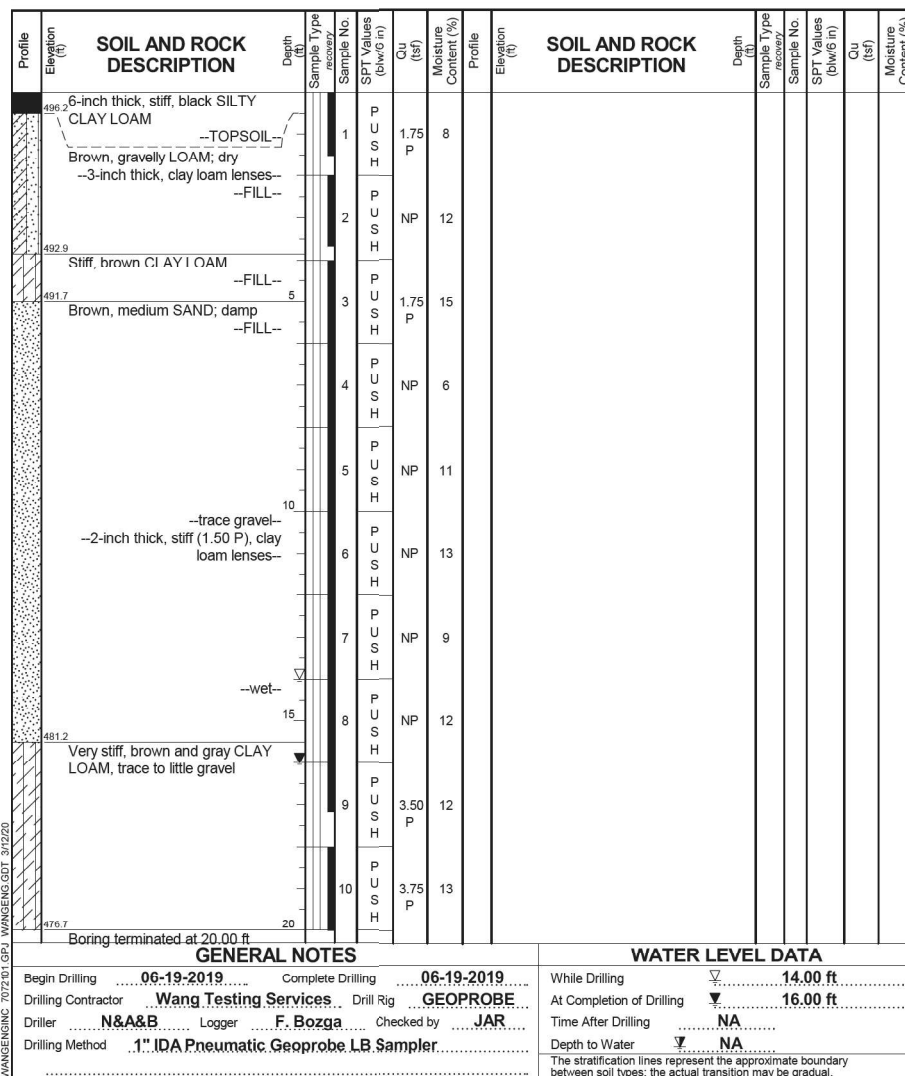
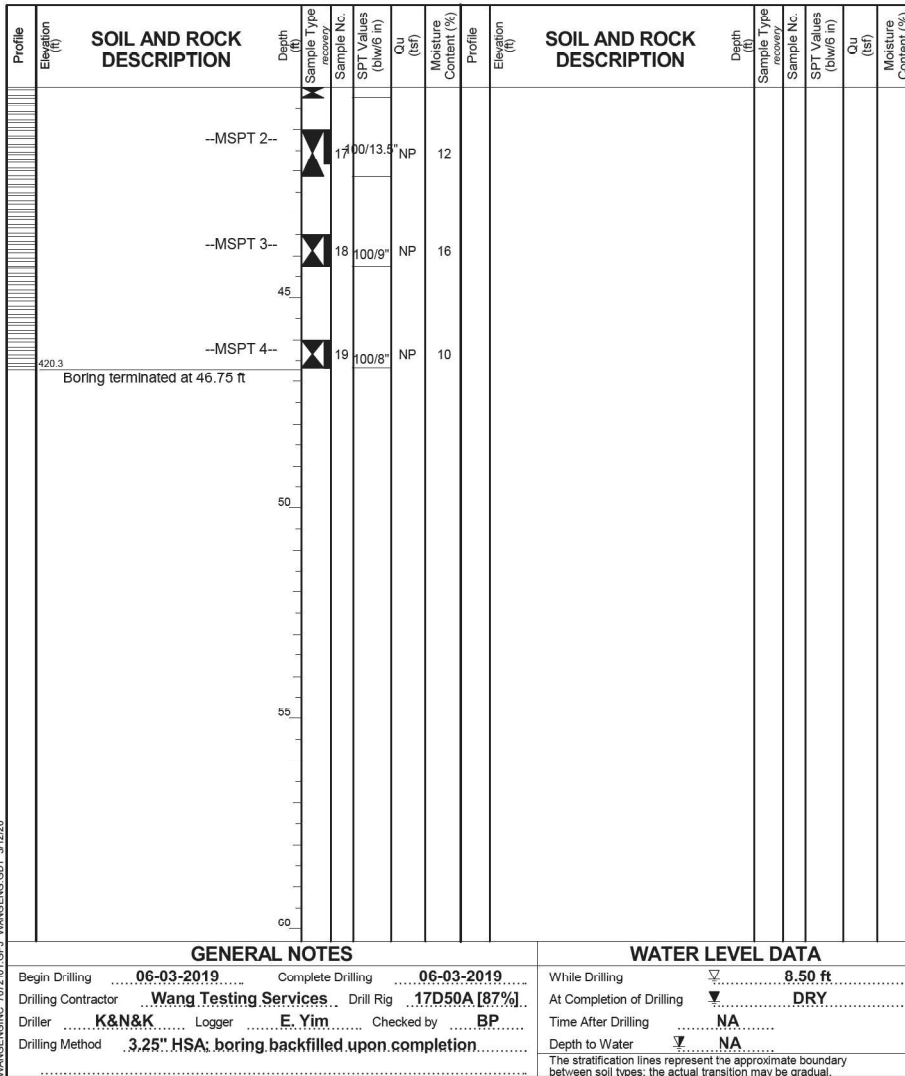
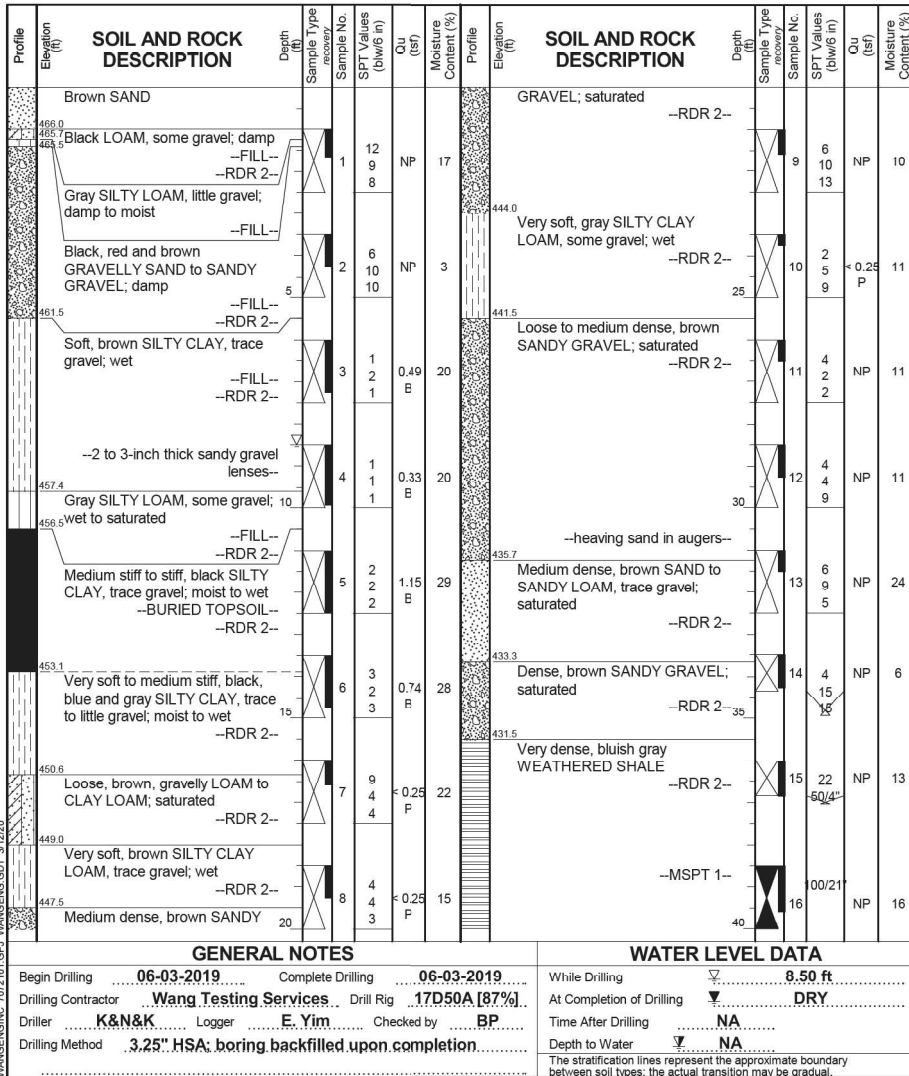
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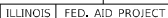
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOGS 11
STRUCTURE NO. 090-0182

SHEET 5-192 OF 5-194 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	718
CONTRACT NO. 68C55				
ILLINOIS / FED. AID PROJECT				





Existing Structure: S.N. 090-0035 was built as Route FA-10, Section 13-HB-3 at Station 83+67.13 in 1963. The existing structure consists of 3 spans. The beams are three simple spans with different skews in each span. The back to back abutment measures 157'-11" long perpendicular to the substructure and 48'-0" out to out of deck. The structure is to be removed and replaced. Traffic is to be detoured.

LEGEND

- | | | |
|---|---|------------------------|
|  | Exist. <i>Underground Cable TV Line</i> | |
|  | Exist. <i>Underground Electric Line</i> | |
|  | Exist. <i>Underground Telephone Line</i> | |
|  | Exist. <i>Underground Fiber Optic Line</i> | |
|  | Abandoned <i>Underground Gas Line</i> | |
|  | Exist. <i>Underground Water Line</i> | |
|  | Exist. <i>Underground Sanitary Sewer Line to be relocated</i> | |
|  | Exist. <i>Manhole to be relocated</i> | |
|  | Exist. <i>Light pole to be relocated</i> | |
|  | Exist. <i>Traffic sign to be relocated</i> | |
|  | Soil Boring | 23'-8 1/4" Nonstandard |
-  Embankment
 Cone

Up to 1/4" may be ground off of the bridge deck and bridge approach slabs.

SCUPPER TABLE

ID	Station	Reference Line	Offset	Discharge	Type
1	82+61.00	℄ Cedar St	1.67' (Lt.)	Freefall	DS-11
2	82+61.00	℄ Cedar St	27.00' (Rt.)	Freefall	DS-11
3	82+66.00	℄ Cedar St	1.67' (Lt.)	Freefall	DS-11
4	82+71.00	℄ Cedar St	1.67' (Lt.)	Freefall	DS-11
5	84+80.00	℄ Cedar St	1.67' (Lt.)	Freefall	DS-11
6	84+90.00	℄ Cedar St	27.00' (Rt.)	Freefall	DS-11

CURVE DATA

(Main St)
 PI Sta = 105+43.03
 $\Delta = 13^{\circ} 13' 30''$ (LT)
 $D = 1^{\circ} 57' 00''$
 $R = 2938.24'$
 $T = 340.62'$
 $L = 678.21'$
 $E = 19.68'$
 P.C Sta = 102+02.42
 P.T Sta = 108+80.63

CURVE DATA

(Cedar St)
 PI Sta = 84+91.93
 $\Delta = 66^\circ 30' 01''$ (RT)
 $D = 18^\circ 36' 09''$
 $R = 308.00'$
 $T = 201.93'$
 $L = 357.48'$
 $E = 60.30'$
 P.C Sta = 82+90.00
 P.T Sta = 86+47.48



Signed Mary Louise Kutska
II. Lic. No. 081-008008
Expires 11-30-2024

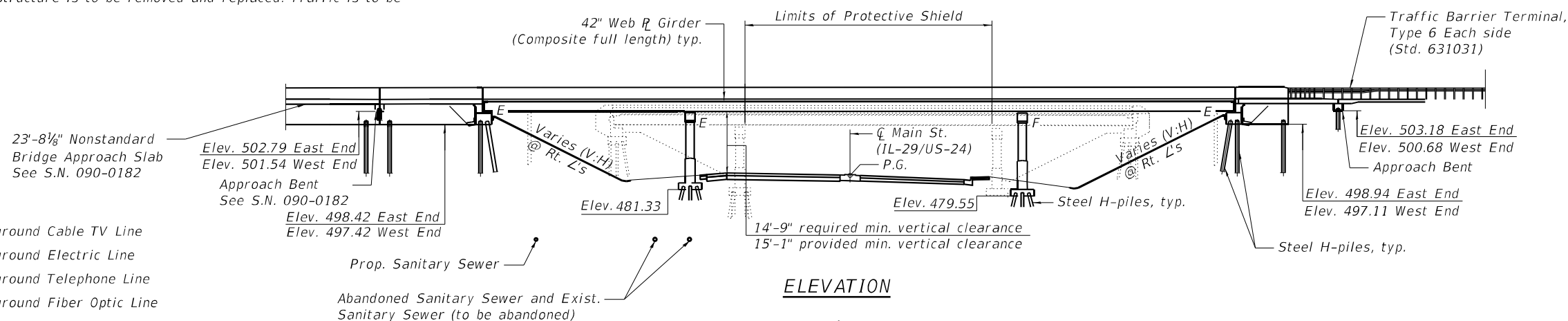
Date 6/8/2023
For Sheets S2-5 Thru S2-29
& S2-41



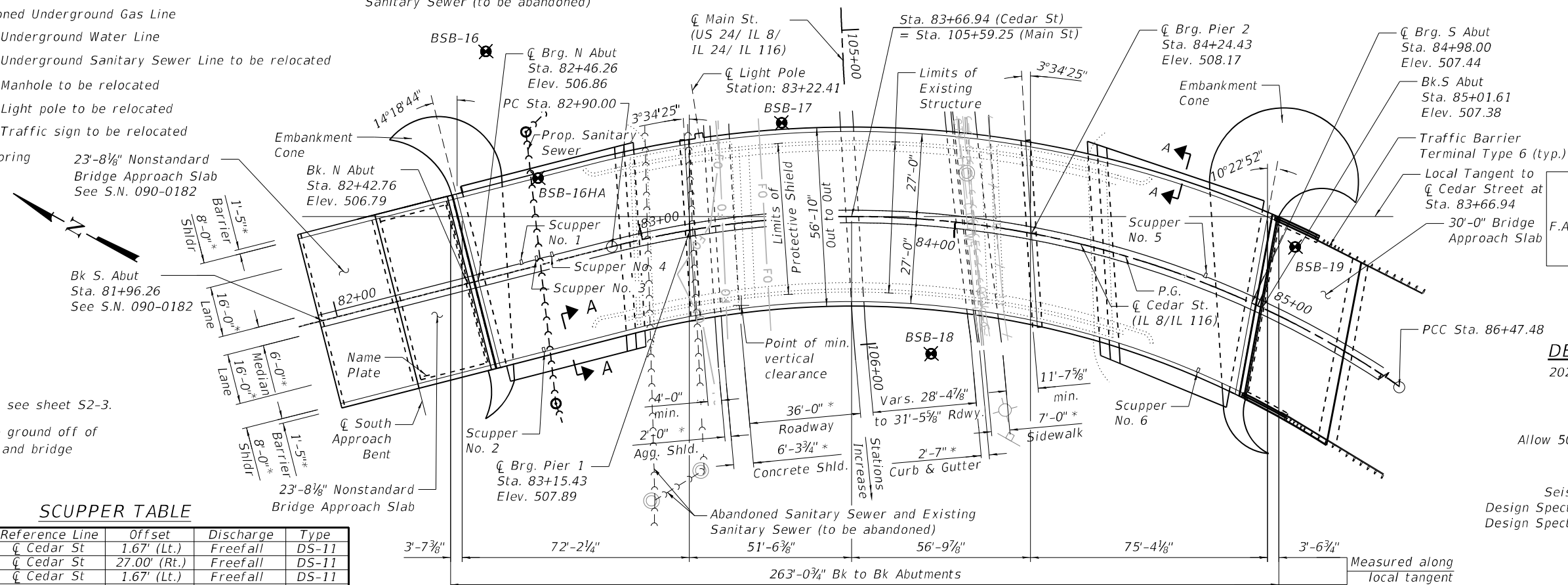
Signed Pankaj Kumar
Pankaj Kumar
II. Lic. No. 081-007577
Expires 11-30-2024

Date 6/8/2023
For Sheets S2-1 Thru S2-4, S2-30
Thru S2-40 & S2-42 Thru S2-47

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**



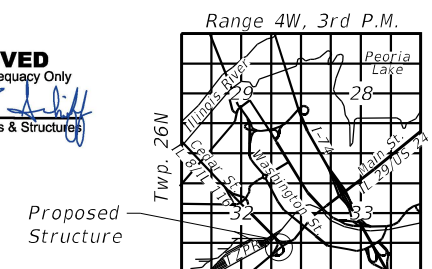
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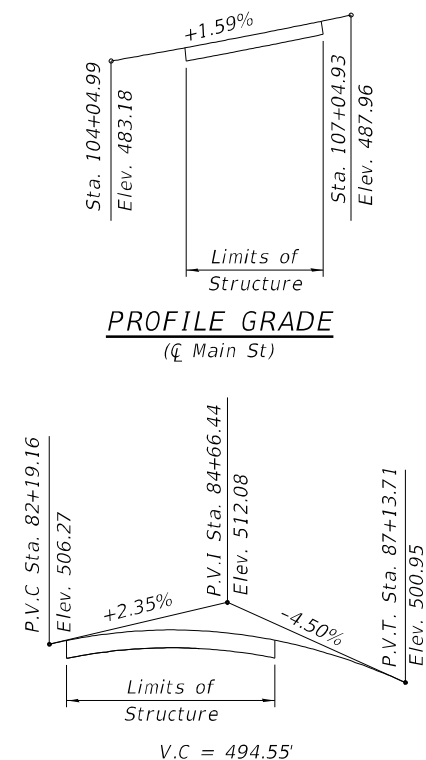
PLAN

* Measured Radially

APPROVED
For Structural Adequacy Only
Jayson F. L...
Engineer of Bridges & Structures



LOCATION SKETCH



PROFILE GRADE

(@ Main St)

PROFILE GRADE

(@ Cedar St)

The profile grade shows the final elevations after grinding

STATION 83+66.94
BUILT BY
STATE OF ILLINOIS
F.A.P. RT. 669 SEC. 13 (HVB, HB-3) BR-1;R
LOADING HL-93
STR. NO. 090-0183

NAME PLATE

See Std. 515001

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications, 9th Edition

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration at 1.0 sec. (SD1) = 0.080
Design Spectral Acceleration at 0.2 sec. (SDS) = 0.134
Soil Site Class = C

DESIGN STRESSES

$f'_c = 3,500 \text{ psi}$ (Substructure Concrete)
 $f'_c = 4,000 \text{ psi}$ (Superstructure Concrete)
 $f_y = 60,000 \text{ psi}$ (Reinforcement)
 $f_v = 50,000 \text{ psi}$ (M270 Grade 50)

GENERAL PLAN AND ELEVATION
IL 8/IL 116 (CEDAR STREET) OVER
US 24/IL 8/IL 29/IL 116 (MAIN STREET)
F.A.P RTE 669 - SECTION 13 (HVB, HB-3) BR-1;R

TAZEWELL COUNTY

STATION 83+66.94

STRUCTURE NUMBER 090-0183

 INFRASTRUCTURE ENGINEERING	USER NAME = Joseph.Kauzlarich	DESIGNED - SPK	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		CHECKED - PK	REVISED -			VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	721	
	PLOT SCALE = 40,0000' = 1 in.	DRAWN - CLL	REVISED -			CONTRACT NO. 68C55					
	PLOT DATE = 5/10/2023	CHECKED - SPK	REVISED -			SHEET S2-1 OF S2-47 SHEETS					
						ILLINOIS FED. AID PROJECT					

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GENERAL NOTES:
Fasteners shall be hot-dip galvanized ASTM F3125 Grade A325 Type 1, bolts $\frac{7}{8}$ in. $\odot$, holes $1\frac{5}{16}$ in. $\odot$, unless otherwise noted. See Special Provision Hot-Dip Galvanizing for Structural Steel.

- | | 27'-0" | 27'-0" | |
|--------------------------------------|--------|--------|---------------|
| Begin Lane 1 | -2.00% | -2.00% | |
| Shoulder Transition
Sta. 80+93.61 | | | |
| Sta. 81+51.37 | 0.00% | -2.00% | |
| Sta. 82+09.13 | 2.00% | -2.00% | |
| Sta. 82+42.76 | | | |
| All Lanes full Super | 6.00% | -6.00% | Bridge Limits |
| Sta. 83+24.66 | | | |
| Sta. 85+01.61 | | | |
| End Full Super | 6.00% | -6.00% | |
| Sta. 88+28.03 | | | |

INDEX OF SHEETS

**

ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Structures No. 2	Each	---	---	1
Protective Shield	Sq. Yd.	412	---	412
Structure Excavation	Cu. Yd.	---	601	601
Concrete Structures	Cu. Yd.	---	448.6	448.6
Concrete Superstructure	Cu. Yd.	507.1	---	507.1
Concrete Encasement	Cu. Yd.	---	16.4	16.4
Protective Coat	Sq. Yd.	2,179	---	2,179
Concrete Superstructure (Approach Slab)	Cu. Yd.	146.9	---	146.9
Furnishing And Erecting Structural Steel	L. Sum	0.11	---	0.11
Stud Shear Connectors	Each	7,248	---	7,248
Reinforcement Bars, Epoxy Coated	Pound	199,120	60,190	259,310
Slope Wall 4 Inch	Sq. Yd.	---	775	775
Furnishing Steel Piles HP12x53	Foot	---	4,757	4,757
Driving Piles	Foot	---	4,757	4,757
Test Pile Steel HP12x53	Each	---	5	5
Pile Shoes	Each	---	95	95
Name Plates	Each	1	---	1
Preformed Joint Strip Seal	Foot	114	---	114
Anchor Bolts, $\frac{3}{4}$ "	Each	64	---	64
Anchor Bolts, 1"	Each	32	---	32
Anchor Bolts, $1\frac{1}{4}$ "	Each	32	---	32
Granular Backfill For Structures	Cu. Yd.	---	243	243
Concrete Sealer	Sq. Ft.	---	3,896	3,896
Geocomposite Wall Drain	Sq. Yd.	---	143	143
Pipe Underdrains For Structures 4"	Foot	---	179	179
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	1,100	---	1,100
High Load Multi-Rotational Bearings, Pot, Fixed-400K	Each	8	---	8
High Load Multi-Rotational Bearings, Pot, Guided Expansion-100K	Each	8	---	8
High Load Multi-Rotational Bearings, Pot, Guided Expansion-200K	Each	8	---	8
High Load Multi-Rotational Bearings, Pot, Guided Expansion-300K	Each	8	---	8
Drainage Scuppers, DS-11	Each	6	---	6
Diamond Grinding (Bridge Section)	Sq. Yd.	1,444	---	1,444

* Quantity is for roadway, median, and inside face and top of parapets and curbs

Plan view of the Cedar Street Bridge showing the bridge structure, approach roads, and stationing. Key dimensions and angles are provided for the bridge structure and the approach roads.

Bridge Structure Dimensions:

- Span 1: $72'-2\frac{3}{16}"$
- Span 2: $108'-4\frac{5}{16}"$
- Span 3: $75'-4\frac{3}{16}"$
- Span 4: $51'-6\frac{7}{16}"$
- Span 5: $56'-9\frac{7}{8}"$
- Span 6: $3\frac{3}{4}"$
- Span 7: $4'-3\frac{9}{16}"$
- Span 8: $5'-2\frac{3}{8}"$
- Span 9: $5'-0\frac{3}{8}"$
- Span 10: $5'-4\frac{7}{16}"$
- Span 11: $27'-5\frac{7}{8}"$

Angles:

- Angle 1: $14^\circ 18' 44"$
- Angle 2: $3^\circ 34' 25"$
- Angle 3: $3^\circ 34' 25"$
- Angle 4: $3^\circ 34' 25"$
- Angle 5: $10^\circ 22' 52"$
- Angle 6: $2,938'-27\frac{1}{8}" R$
- Angle 7: $308'-0" R$

Stationing and Key Points:

- PC Sta. 82+90.00
- CL Cedar St.
- CL Brg. Pier 1 Sta. 83+15.43
- CL Brg. Pier 2 Sta. 84+24.43
- CL Brg. S. Abut. Sta. 84+98.00
- CL Main St.
- Tangent to CL Main St. at Sta. 105+59.25
- CL Brg. N. Abut. Sta. 82+46.26
- CL Cedar St. at Sta. 83+66.94
- CL Cedar St. at Sta. 105+59.25 (Main St.)

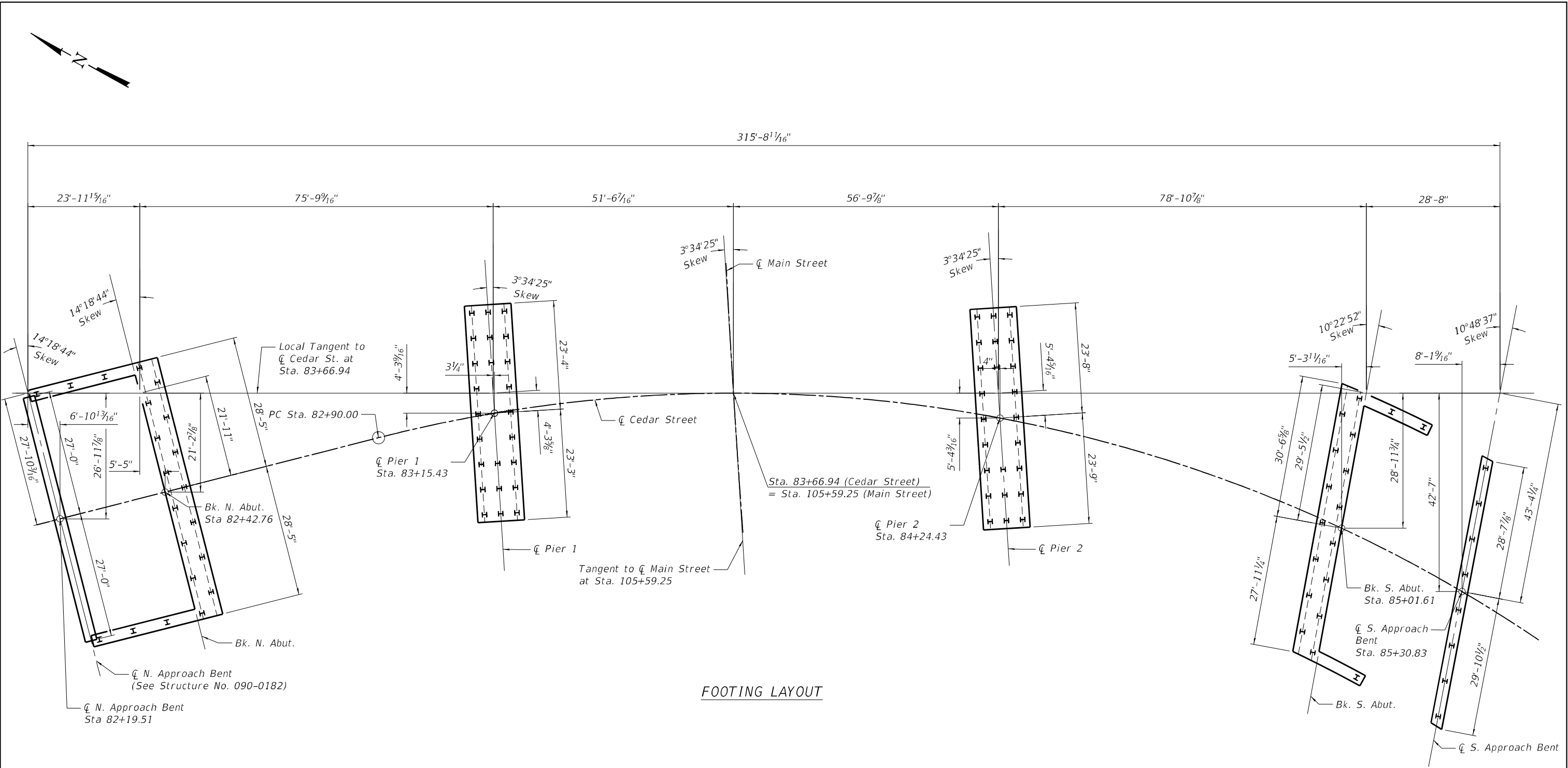
Other Labels:

- Bk. N. Abut.
- Bk. S. Abut.
- Local Tangent to CL Cedar St. at Sta. 83+66.94

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 INFRASTRUCTURE ENGINEERING INCORPORATED	USER NAME = Jaime.Rivera	DESIGNED - SPK -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL NOTES, OFFSET SKETCH AND CROSS SLOPE TRANSITIONS STRUCTURE NO. 090-0183	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
	PLOT SCALE = 0.1667 ' / in.	CHECKED - PK -	REVISED -			VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	722	
	PLOT DATE = 5/1/2025	DRAWN - CLL -	REVISED -			CONTRACT NO. 68C55					
		CHECKED - SPK -	REVISED -			SHEET S2-2 OF S2-47 SHEETS					

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FOOTING LAYOUT

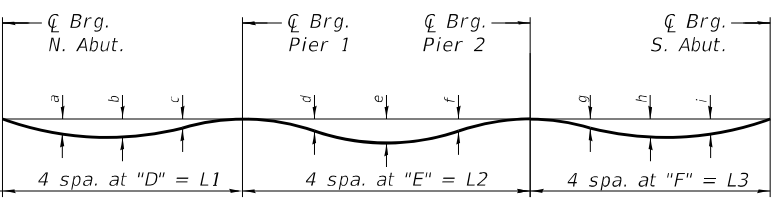
Notes:

For location of structural soil boring logs, see sheet S2-1.

For soil boring logs, see sheets S2-43, S2-44, S2-45, and S2-46.

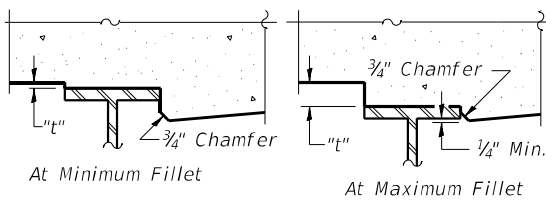
	USER NAME = DLamb		DESIGNED - SPK		REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	FOOTING LAYOUT STRUCTURE NO. 090-0183				F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE = 21.3332 ' / in.		CHECKED - PK		REVISED -			SHEET S2-4 OF S2-47 SHEETS				VAR.	13 (HVB, HB-3) BR-1;R		TAZEWELL	1173	724
	PLOT DATE = 6/6/2023		DRAWN - CLL		REVISED -							CONTRACT NO. 68C55					
			CHECKED - SPK		REVISED -							ILLINOIS FED. AID PROJECT					

VARIABLE DIMENSION TABLE																		
Location	L	L1	L2	L3	"A"	"B"	"C"	"D"	"E"	"F"	a	b	c	d	e	f	g	i
Girder 1	262'- 7½"	73'-11⅞"	108'-10⅞"	79'-9½"	13'-11⅞"	8'-10⅞"	9'-9½"	±18'-5⅞"	±27'-2⅝"	19'-11⅜"	⅛"	⅛"	0"	¾"	1¼"	¾"	⅛"	¾"
Girder 2	259'- 6⅜"	72'-7⅞"	108'-10⅞"	78'-0⅜"	12'-7⅞"	8'-10⅞"	8'-0⅜"	±18'-1⅜"	±27'-2⅜"	±19'-6⅜"	⅛"	⅛"	0"	¾"	1¼"	¾"	0"	⅛"
Girder 3	256'-4⅜"	71'-2⅜"	108'-11⅜"	76'- 2⅜"	11'-2⅜"	8'-11⅜"	6'-2⅜"	±17'-9⅜"	±27'-2⅜"	±19'-0⅜"	⅛"	⅛"	0"	¾"	1⅜"	¾"	0"	⅛"
Girder 4	253'- 3½"	69'-10¼"	108'-11⅜"	74'-5½"	9'-10¼"	8'-11⅜"	14'-5½"	17'-5⅞"	27'-2⅜"	18'-7⅜"	⅛"	⅛"	0"	¾"	1⅜"	¾"	0"	⅛"
☐ Cedar St.	251'- 8⅞"	69'-2⅜"	109'-0"	73'-6⅜"	9'-2⅜"	9'-0"	13'-6⅜"	±17'-3½"	27'-3"	±18'-4⅜"	⅛"	⅛"	0"	¾"	1⅜"	¾"	0"	⅛"
Girder 5	250'- 2¼"	68'-5⅜"	109'-0¼"	72'- 8⅜"	8'-5⅜"	9'-0¼"	12'-8⅜"	±17'-1⅞"	27'-3⅜"	±18'-2⅜"	⅛"	⅛"	0"	¾"	1⅜"	¾"	0"	⅛"
Girder 6	247'- 1"	67'-1⅜"	109'-0⅜"	70'-10⅜"	7'-1⅜"	9'-0⅜"	10'-10⅜"	±16'-9⅜"	±27'-3⅜"	±17'-8⅜"	⅛"	0"	0"	¾"	1⅜"	¾"	0"	⅛"
Girder 7	243'- 11⅜"	65'-8⅜"	109'-1⅜"	69'- 1½"	5'-8⅜"	9'-1⅜"	9'-1½"	±16'-5¼"	±27'-3⅜"	17'-3⅜"	⅛"	0"	0"	⅝"	1⅜"	⅝"	0"	⅛"
Girder 8	240'- 10⅝"	64'-4½"	109'-2"	67'-4½"	14'-4½"	9'-2"	7'-4½"	16'-1⅞"	27'-3½"	±16'-10⅜"	⅛"	0"	0"	⅝"	1"	⅝"	0"	⅛"



DEAD LOAD DEFLECTION DIAGRAM
(Includes weight of concrete only.)

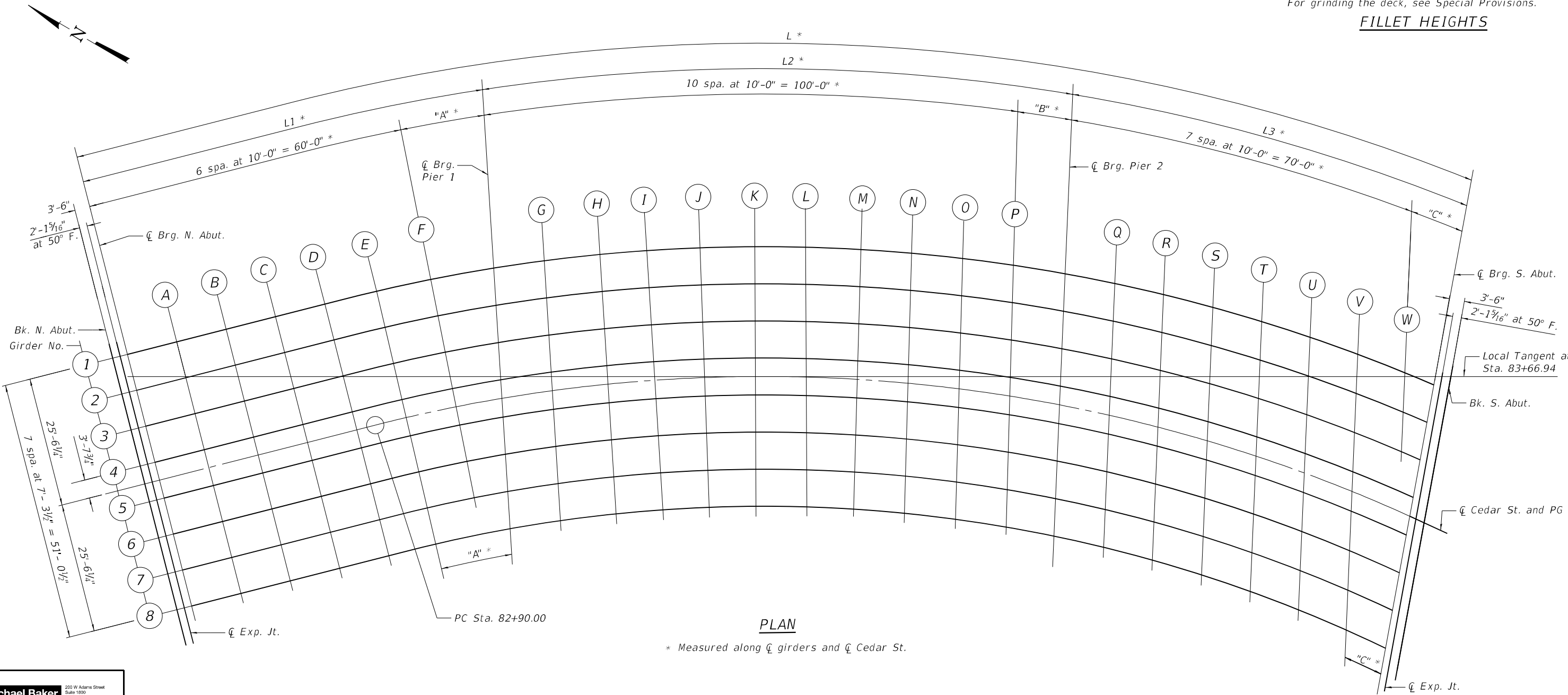
The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown on sheets S2-6, S2-7, & S2-8.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the girders shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations adjusted for Dead Load Deflection and Grinding" shown on sheets S2-6, S2-7, & S2-8 minus the initial slab thickness prior to grinding, equals the fillet heights "t" above top flange of beams.

The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown on sheets S2-6, S2-7, & S2-8. For grinding the deck, see Special Provisions.

FILLET HEIGHTS



PLAN

* Measured along ☐ girders and ☐ Cedar St.

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USER NAME = DIamb	DESIGNED - SPK	REVISED -
	CHECKED - PK	REVISED -
PLOT SCALE = 19.9999 * / in.	DRAWN - CLL	REVISED -
PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 090-0183

SHEET S2-5 OF S2-17 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	725
		CONTRACT NO. 68C55		
		ILLINOIS FED. AID PROJECT		

MODEL: Default
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GIRDER 1

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	-25.52	507.59	507.61
☒ Exp. Jt.	82+44.87	-25.52	507.65	507.67
☒ Brg. N. Abut.	82+46.26	-25.52	507.69	507.71
A	82+56.26	-25.52	507.97	508.00
B	82+66.26	-25.52	508.24	508.27
C	82+76.26	-25.52	508.49	508.52
D	82+86.26	-25.52	508.73	508.75
E	82+95.78	-25.52	508.94	508.96
F	83+05.02	-25.52	509.13	509.15
☒ Pier 1	83+17.91	-25.52	509.39	509.41
G	83+27.14	-25.52	509.53	509.57
H	83+36.38	-25.52	509.60	509.67
I	83+45.61	-25.52	509.66	509.75
J	83+54.85	-25.52	509.71	509.82
K	83+64.08	-25.52	509.75	509.87
L	83+73.32	-25.52	509.78	509.90
M	83+82.55	-25.52	509.79	509.90
N	83+91.79	-25.52	509.79	509.88
O	84+01.02	-25.52	509.78	509.84
P	84+10.26	-25.52	509.76	509.80
☒ Pier 2	84+18.45	-25.52	509.73	509.75
Q	84+27.68	-25.52	509.69	509.71
R	84+36.92	-25.52	509.63	509.66
S	84+46.15	-25.52	509.57	509.60
T	84+55.39	-25.52	509.49	509.53
U	84+64.62	25.52	509.40	509.45
V	84+73.86	-25.52	509.29	509.34
W	84+83.09	-25.52	509.18	509.21
☒ Brg. S. Abut.	84+92.14	-25.52	509.05	509.07
☒ Exp. Jt.	84+93.45	-25.52	509.03	509.05
Bk. S. Abut.	84+95.46	-25.52	509.01	509.03

GIRDER 2

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	-18.23	507.36	507.38
☒ Exp. Jt.	82+44.87	-18.23	507.42	507.44
☒ Brg. N. Abut.	82+46.26	-18.23	507.45	507.47
A	82+56.26	-18.23	507.71	507.74
B	82+66.26	-18.23	507.95	507.98
C	82+76.26	-18.23	508.17	508.21
D	82+86.26	-18.23	508.39	508.41
E	82+95.91	-18.23	508.58	508.60
F	83+05.35	-18.23	508.75	508.77
☒ Pier 1	83+17.24	-18.23	508.96	508.98
G	83+26.68	-18.23	509.09	509.13
H	83+36.12	-18.23	509.16	509.23
I	83+45.56	-18.23	509.23	509.31
J	83+55.00	-18.23	509.28	509.38
K	83+64.45	-18.23	509.32	509.43
L	83+73.89	-18.23	509.34	509.46
M	83+83.33	-18.23	509.35	509.46
N	83+92.77	-18.23	509.36	509.44
O	84+02.21	-18.23	509.34	509.40
P	84+11.65	-18.23	509.32	509.36
☒ Pier 2	84+20.06	-18.23	509.29	509.31
Q	84+29.50	-18.23	509.24	509.26
R	84+38.94	-18.23	509.18	509.21
S	84+48.38	-18.23	509.11	509.15
T	84+57.82	-18.23	509.03	509.07
U	84+67.27	-18.23	508.93	508.98
V	84+76.71	-18.23	508.82	508.86
W	84+86.15	-18.23	508.70	508.73
☒ Brg. S. Abut.	84+93.72	-18.23	508.59	508.61
☒ Exp. Jt.	84+95.06	-18.23	508.57	508.59
Bk. S. Abut.	84+97.11	-18.23	508.54	508.56

GIRDER 3

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	-10.94	507.13	507.15
☒ Exp. Jt.	82+44.87	-10.94	507.18	507.20
☒ Brg. N. Abut.	82+46.26	-10.94	507.22	507.24
A	82+56.26	-10.94	507.44	507.47
B	82+66.26	-10.94	507.66	507.69
C	82+76.26	-10.94	507.86	507.89
D	82+86.26	-10.94	508.05	508.07
E	82+96.05	-10.94	508.22	508.24
F	83+05.70	-10.94	508.37	508.39
☒ Pier 1	83+16.54	-10.94	508.53	508.55
G	83+26.20	-10.94	508.65	508.68
H	83+35.85	-10.94	508.72	508.79
I	83+45.51	-10.94	508.79	508.87
J	83+55.17	-10.94	508.84	508.95
K	83+64.83	-10.94	508.88	508.99
L	83+74.48	-10.94	508.90	509.02
M	83+84.14	-10.94	508.92	509.02
N	83+93.80	-10.94	508.92	509.00
O	84+03.45	-10.94	508.90	508.96
P	84+13.11	-10.94	508.88	508.91
☒ Pier 2	84+21.75	-10.94	508.84	508.86
Q	84+31.41	-10.94	508.79	508.81
R	84+41.06	-10.94	508.73	508.75
S	84+50.72	-10.94	508.65	508.69
T	84+60.38	-10.94	508.56	508.60
U	84+70.04	-10.94	508.46	508.50
V	84+79.69	-10.94	508.35	508.38
W	84+89.35	-10.94	508.22	508.25
☒ Brg. S. Abut.	84+95.37	-10.94	508.13	508.15
☒ Exp. Jt.	84+96.75	-10.94	508.11	508.13
Bk. S. Abut.	84+98.85	-10.94	508.08	508.10

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INFRASTRUCTURE
ENGINEERING INTERNATIONAL

USER NAME = D1amb	DESIGNED - CLL	REVISED -
	CHECKED - SPK	REVISED -
PLOT SCALE = 0.1667 ' / in.	DRAWN - CLL	REVISED -
PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 090-0183

SHEET S2-6 OF S2-17 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	726
CONTRACT NO. 68C55				
		ILLINOIS	FED. AID PROJECT	

Model: Default
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GIRDER 4

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	-3.65	506.90	506.92
℄ Exp. Jt.	82+44.87	-3.65	506.95	506.97
℄ Brg. N. Abut.	82+46.26	-3.65	506.98	507.00
A	82+56.26	-3.65	507.18	507.21
B	82+66.26	-3.65	507.37	507.40
C	82+76.26	-3.65	507.54	507.57
D	82+86.26	-3.65	507.71	507.73
E	82+96.19	-3.65	507.85	507.87
F	83+06.07	-3.65	507.98	508.00
℄ Pier 1	83+15.81	-3.65	508.10	508.12
G	83+25.69	-3.65	508.21	508.24
H	83+35.58	-3.65	508.29	508.35
I	83+45.46	-3.65	508.35	508.44
J	83+55.34	-3.65	508.40	508.51
K	83+65.23	-3.65	508.44	508.56
L	83+75.11	-3.65	508.47	508.58
M	83+84.99	-3.65	508.48	508.58
N	83+94.87	-3.65	508.48	508.56
O	84+04.76	-3.65	508.46	508.52
P	84+14.64	-3.65	508.44	508.47
℄ Pier 2	84+23.51	-3.65	508.40	508.42
Q	84+33.39	-3.65	508.34	508.36
R	84+43.28	-3.65	508.28	508.30
S	84+53.16	-3.65	508.19	508.22
T	84+63.04	-3.65	508.10	508.14
U	84+72.93	-3.65	507.99	508.03
V	84+82.81	-3.65	507.87	507.90
℄ Brg. S. Abut	84+97.10	-3.65	507.67	507.69
℄ Exp. Jt.	84+98.52	-3.65	507.65	507.67
Bk. S. Abut.	85+00.67	-3.65	507.61	507.63

℄ CEDAR ST. & PG

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	0.00	506.79	506.81
℄ Exp. Jt.	82+44.87	0.00	506.83	506.85
℄ Brg. N. Abut.	82+46.26	0.00	506.86	506.88
A	82+56.26	0.00	507.05	507.07
B	82+66.26	0.00	507.22	507.25
C	82+76.26	0.00	507.39	507.41
D	82+86.26	0.00	507.53	507.56
E	82+96.26	0.00	507.67	507.69
F	83+06.26	0.00	507.79	507.81
℄ Pier 1	83+15.43	0.00	507.89	507.91
G	83+25.43	0.00	507.98	508.02
H	83+35.43	0.00	508.07	508.13
I	83+45.43	0.00	508.13	508.22
J	83+55.43	0.00	508.19	508.29
K	83+65.43	0.00	508.22	508.34
L	83+75.43	0.00	508.25	508.36
M	83+85.43	0.00	508.26	508.36
N	83+95.43	0.00	508.26	508.34
O	84+05.43	0.00	508.24	508.30
P	84+15.43	0.00	508.21	508.25
℄ Pier 2	84+24.43	0.00	508.17	508.19
Q	84+34.43	0.00	508.12	508.13
R	84+44.43	0.00	508.05	508.07
S	84+54.43	0.00	507.96	507.99
T	84+64.43	0.00	507.87	507.90
U	84+74.43	0.00	507.75	507.79
V	84+84.43	0.00	507.63	507.66
℄ Brg. S. Abut	84+98.00	0.00	507.44	507.46
℄ Exp. Jt.	84+99.43	0.00	507.41	507.43
Bk. S. Abut.	85+01.61	0.00	507.38	507.40

GIRDER 5

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	3.65	506.67	506.69
℄ Exp. Jt.	82+44.87	3.65	506.71	506.73
℄ Brg. N. Abut.	82+46.26	3.65	506.74	506.76
A	82+56.26	3.65	506.91	506.94
B	82+66.26	3.65	507.08	507.11
C	82+76.26	3.65	507.23	507.26
D	82+86.26	3.65	507.36	507.39
E	82+96.33	3.65	507.49	507.51
F	83+06.45	3.65	507.60	507.61
℄ Pier 1	83+15.04	3.65	507.68	507.70
G	83+25.16	3.65	507.76	507.80
H	83+35.28	3.65	507.85	507.91
I	83+45.40	3.65	507.91	508.00
J	83+55.52	3.65	507.97	508.07
K	83+65.64	3.65	508.01	508.12
L	83+75.76	3.65	508.03	508.15
M	83+85.88	3.65	508.04	508.15
N	83+96.00	3.65	508.04	508.12
O	84+06.12	3.65	508.02	508.08
P	84+16.24	3.65	507.99	508.03
℄ Pier 2	84+25.37	3.65	507.95	507.97
Q	84+35.49	3.65	507.89	507.91
R	84+45.61	3.65	507.82	507.84
S	84+55.73	3.65	507.73	507.76
T	84+65.85	3.65	507.63	507.67
U	84+75.97	3.65	507.52	507.55
V	84+86.09	3.65	507.39	507.42
℄ Brg. S. Abut	84+98.92	3.65	507.20	507.22
℄ Exp. Jt.	85+00.37	3.65	507.18	507.20
Bk. S. Abut.	85+02.58	3.65	507.15	507.17

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 090-0183

SHEET S2-7 OF S2-17 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	727
CONTRACT NO. 68C55				
		ILLINOIS	FED. AID PROJECT	

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	10.94	506.44	506.46
℄ Exp. Jt.	82+44.87	10.94	506.47	506.49
℄ Brg. N. Abut.	82+46.26	10.94	506.50	506.52
A	82+56.26	10.94	506.65	506.67
B	82+66.26	10.94	506.79	506.82
C	82+76.26	10.94	506.91	506.94
D	82+86.26	10.94	507.02	507.05
E	82+96.49	10.94	507.12	507.14
F	83+06.86	10.94	507.21	507.22
℄ Pier 1	83+14.24	10.94	507.26	507.28
G	83+24.61	10.94	507.32	507.36
H	83+34.98	10.94	507.41	507.47
I	83+45.34	10.94	507.48	507.56
J	83+55.71	10.94	507.53	507.63
K	83+66.08	10.94	507.57	507.68
L	83+76.45	10.94	507.60	507.71
M	83+86.82	10.94	507.61	507.71
N	83+97.19	10.94	507.60	507.68
O	84+07.55	10.94	507.58	507.64
P	84+17.92	10.94	507.55	507.58
℄ Pier 2	84+27.32	10.94	507.50	507.52
Q	84+37.69	10.94	507.44	507.46
R	84+48.06	10.94	507.36	507.38
S	84+58.42	10.94	507.27	507.30
T	84+68.79	10.94	507.16	507.19
U	84+79.16	10.94	507.04	507.07
V	84+89.53	10.94	506.90	506.93
℄ Brg. S. Abut	85+00.83	10.94	506.74	506.76
℄ Exp. Jt.	85+02.32	10.94	506.71	506.73
Bk. S. Abut.	85+04.59	10.94	506.68	506.70

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	18.23	506.21	506.23
☒ Exp. Jt.	82+44.87	18.23	506.24	506.26
☒ Brg. N. Abut.	82+46.26	18.23	506.26	506.28
A	82+56.26	18.23	506.38	506.41
B	82+66.26	18.23	506.50	506.52
C	82+76.26	18.23	506.60	506.62
D	82+86.26	18.23	506.68	506.70
E	82+96.65	18.23	506.76	506.77
F	83+07.28	18.23	506.82	506.83
☒ Pier 1	83+13.39	18.23	506.85	506.87
G	83+24.02	18.23	506.88	506.92
H	83+34.65	18.23	506.97	507.02
I	83+45.28	18.23	507.04	507.12
J	83+55.91	18.23	507.09	507.20
K	83+66.54	18.23	507.13	507.25
L	83+77.16	18.23	507.16	507.27
M	83+87.79	18.23	507.17	507.27
N	83+98.42	18.23	507.16	507.24
O	84+09.05	18.23	507.14	507.20
P	84+19.68	18.23	507.10	507.14
☒ Pier 2	84+29.37	18.23	507.05	507.07
Q	84+40.00	18.23	506.99	507.00
R	84+50.63	18.23	506.90	506.92
S	84+61.26	18.23	506.80	506.83
T	84+71.89	18.23	506.69	506.72
U	84+82.52	18.23	506.56	506.59
V	84+93.14	18.23	506.41	506.44
☒ Brg. S. Abut	85+02.84	18.23	506.27	506.29
☒ Exp. Jt.	85+04.37	18.23	506.24	506.26
Bk. S. Abut.	85+06.70	18.23	506.21	506.23

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elevations Adjusted for Dead Load Deflection & Grinding
Bk. N. Abut.	82+42.76	25.52	505.98	506.00
℄ Exp. Jt.	82+44.87	25.52	506.00	506.02
℄ Brg. N. Abut.	82+46.26	25.52	506.02	506.04
A	82+56.26	25.52	506.12	506.14
B	82+66.26	25.52	506.21	506.23
C	82+76.26	25.52	506.28	506.31
D	82+86.26	25.52	506.34	506.36
E	82+96.83	25.52	506.39	506.41
℄ Pier 1	83+12.50	25.52	506.44	506.46
G	83+23.40	25.52	506.45	506.48
H	83+34.31	25.52	506.53	506.58
I	83+45.21	25.52	506.60	506.68
J	83+56.11	25.52	506.66	506.75
K	83+67.02	25.52	506.70	506.80
L	83+77.92	25.52	506.72	506.83
M	83+88.82	25.52	506.73	506.83
N	83+99.73	25.52	506.72	506.80
O	84+10.63	25.52	506.70	506.75
P	84+21.53	25.52	506.66	506.69
℄ Pier 2	84+31.53	25.52	506.60	506.62
Q	84+42.43	25.52	506.53	506.55
R	84+53.34	25.52	506.44	506.46
S	84+64.24	25.52	506.34	506.36
T	84+75.14	25.52	506.21	506.24
U	84+86.05	25.52	506.08	506.10
V	84+96.95	25.52	505.92	505.95
℄ Brg. S. Abut	85+04.96	25.52	505.80	505.82
℄ Exp. Jt.	85+06.53	25.52	505.77	505.79
Bk. S. Abut.	85+08.92	25.52	505.73	505.75

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EAST EDGE OF SHOULDER

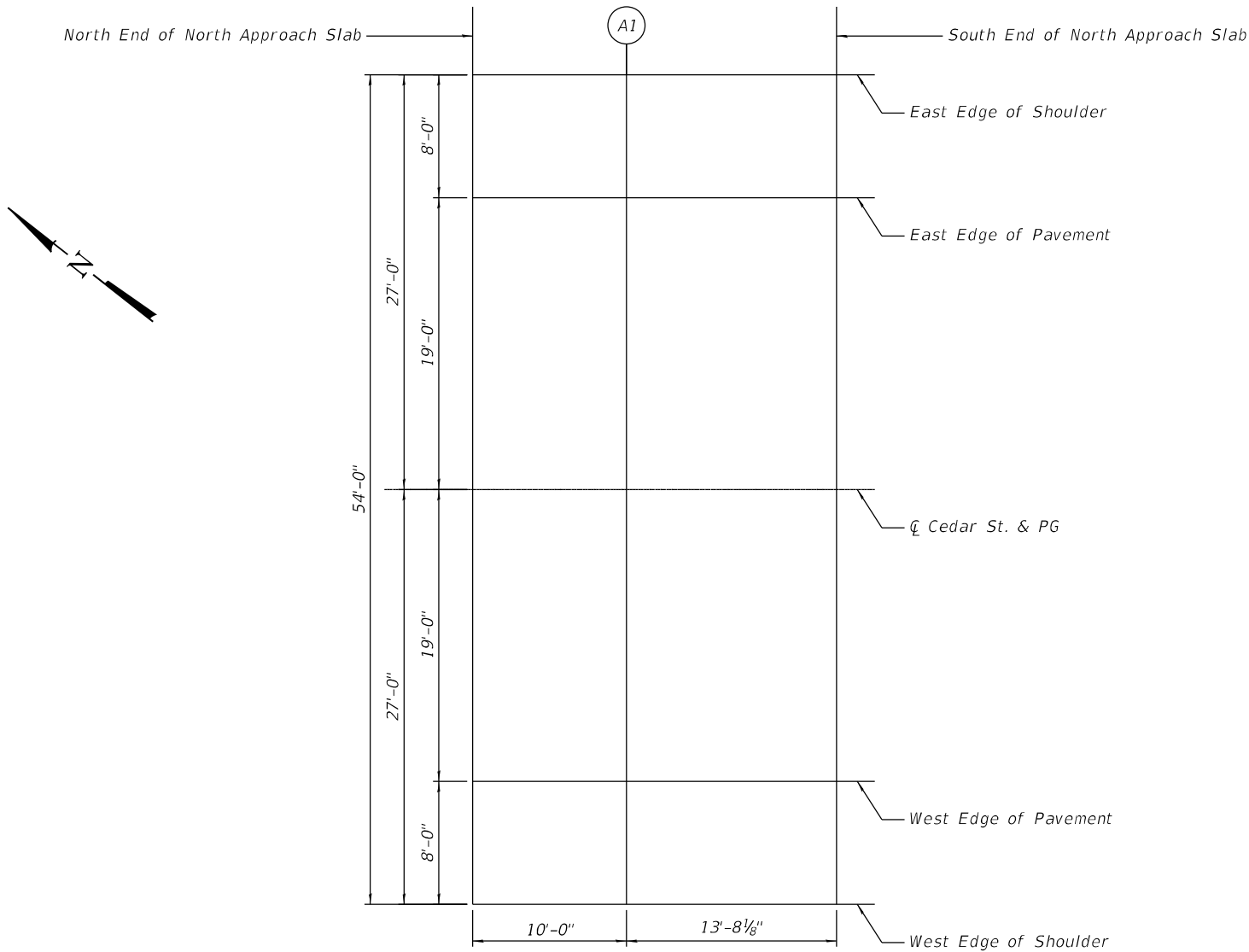
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End N. Appr. Slab	82+19.58	-27.00	506.92	506.94
A1	82+29.58	-27.00	507.24	507.26
S. End N. Appr. Slab	82+43.26	-27.00	507.66	507.68

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End N. Appr. Slab	82+19.58	-19.00	506.73	506.75
A1	82+29.58	-19.00	507.02	507.04
S. End N. Appr. Slab	82+43.26	-19.00	507.40	507.42

⊘ CEDAR ST. & PG

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End N. Appr. Slab	82+19.58	0.00	506.28	506.30
A1	82+29.58	0.00	506.51	506.53
S. End N. Appr. Slab	82+43.26	0.00	506.80	506.82



PLAN

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End N. Appr. Slab	82+19.58	19.00	505.83	505.85
A1	82+29.58	19.00	505.99	506.01
S. End N. Appr. Slab	82+43.26	19.00	506.19	506.21

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End N. Appr. Slab	82+19.58	27.00	505.64	505.66
A1	82+29.58	27.00	505.78	505.80
S. End N. Appr. Slab	82+43.26	27.00	505.94	505.96

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INFRASTRUCTURE
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	CHECKED - SPK	REVISED -
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PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF NORTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 090-0183

SHEET S2-9 OF 52-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	729
				CONTRACT NO. 68C55
		ILLINOIS	FED. AID PROJECT	

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EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End S. Appr. Slab	84+94.66	-27.00	509.11	509.13
A2	85+03.85	-27.00	508.97	508.99
A3	85+13.05	-27.00	508.81	508.83
S. End S. Appr. Slab	85+22.24	-27.00	508.65	508.67

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End S. Appr. Slab	84+96.45	-19.00	508.60	508.62
A2	85+05.87	-19.00	508.45	508.47
A3	85+15.29	-19.00	508.29	508.31
S. End S. Appr. Slab	85+24.71	-19.00	508.12	508.14

CL CEDAR ST. & PG

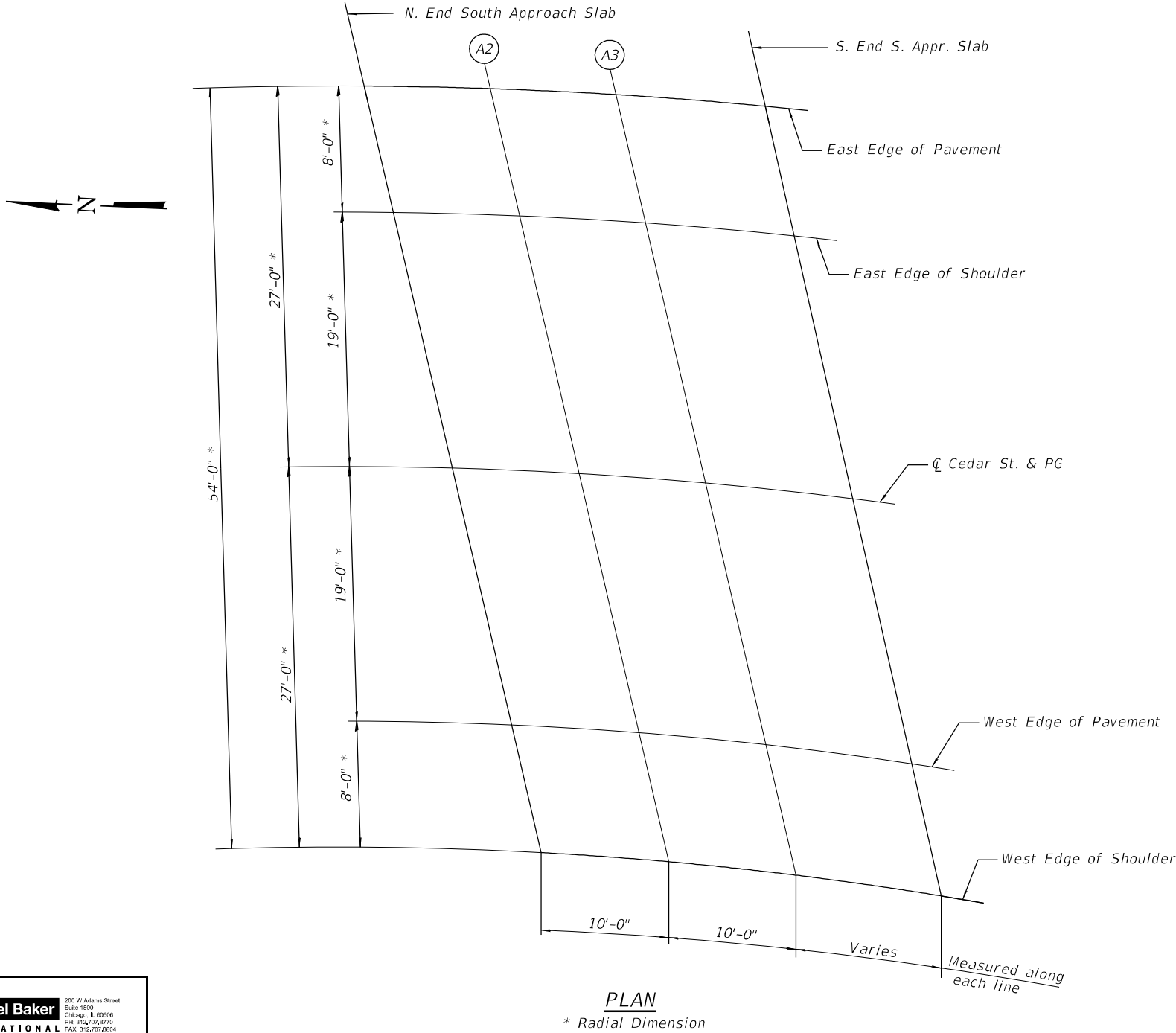
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End S. Appr. Slab	85+01.10	0.00	507.39	507.41
A2	85+11.10	0.00	507.23	507.25
A3	85+21.10	0.00	507.05	507.07
S. End S. Appr. Slab	85+31.10	0.00	506.86	506.88

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End S. Appr. Slab	85+06.37	19.00	506.16	506.18
A2	85+17.03	19.00	505.98	506.00
A3	85+27.68	19.00	505.79	505.81
S. End S. Appr. Slab	85+38.39	19.00	505.57	505.59

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
N. End S. Appr. Slab	85+08.82	27.00	505.64	505.66
A2	85+19.78	27.00	505.45	505.47
A3	85+30.74	27.00	505.25	505.27
S. End S. Appr. Slab	85+41.78	27.00	505.02	505.04



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INFRASTRUCTURE
ENGINEERING

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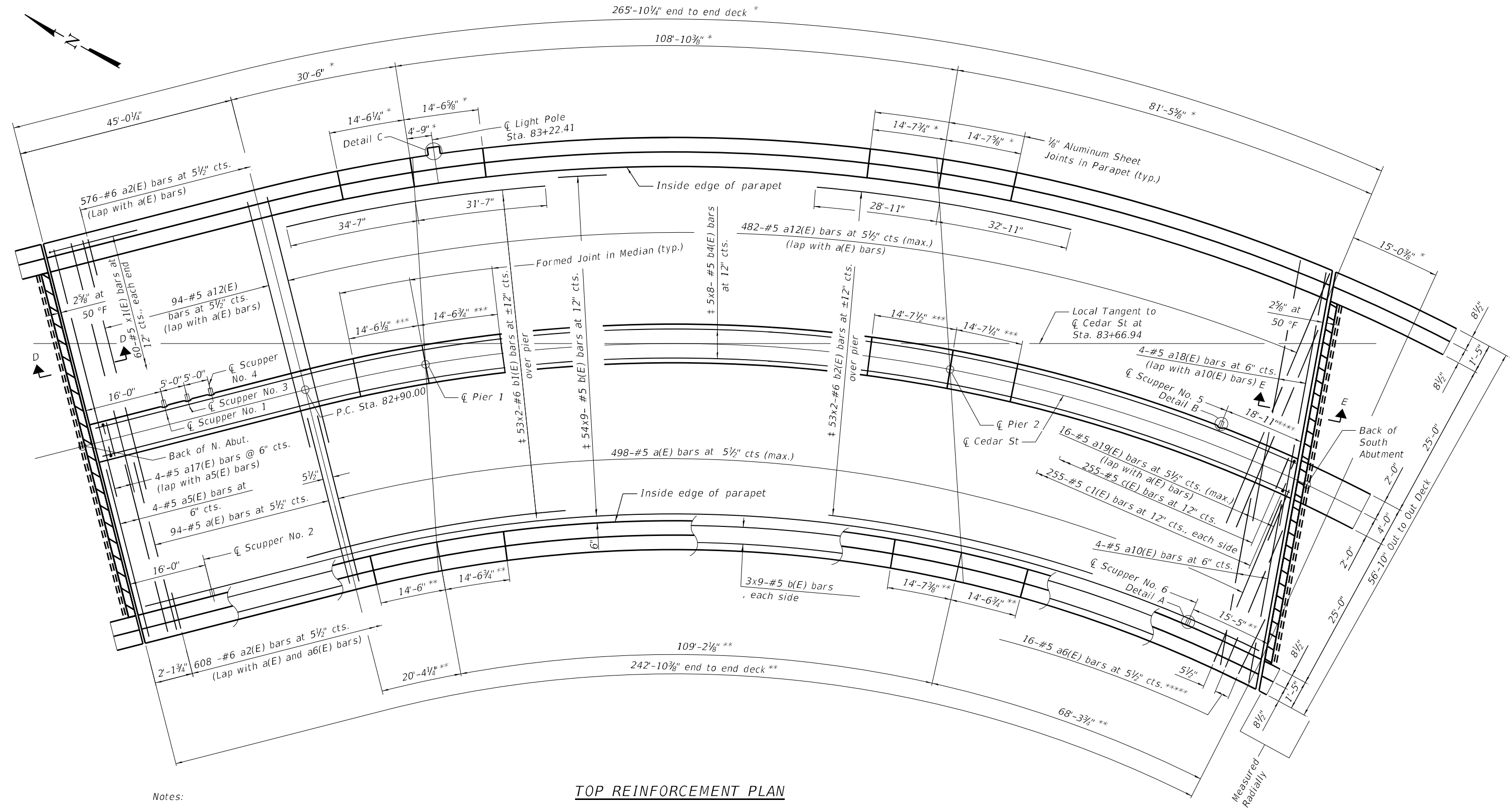
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SOUTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 090-0183

SHEET S2-10 OF S2-17 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	730
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

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Notes:

See sheet S2-12 for reinforcement bars in bottom of deck.

See sheet S2-13 for Deck Cross Section.

See sheet S2-14 for Parapets.

See sheet S2-15 for Section D-D and Section E-E.

See sheet S2-16 for Details A, B, and C, light pole details, reinforcement bar bending details, and Bill of Material.

Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.

TOP REINFORCEMENT PLAN

MINIMUM BAR LAP

#5 bar = 3'-6"
#6 bar = 3'-7"

- * Measured along inside edge of east parapet
- ** Measured along inside edge of west parapet
- *** Measured along $\varnothing$ Cedar St
- **** Measured along east edge of median
- ***** See field cutting diagram on sheet S2-16
- ‡ Bars are to be provided straight and are to be sprung and tied at required radius in field

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INFRASTRUCTURE
ENGINEERING

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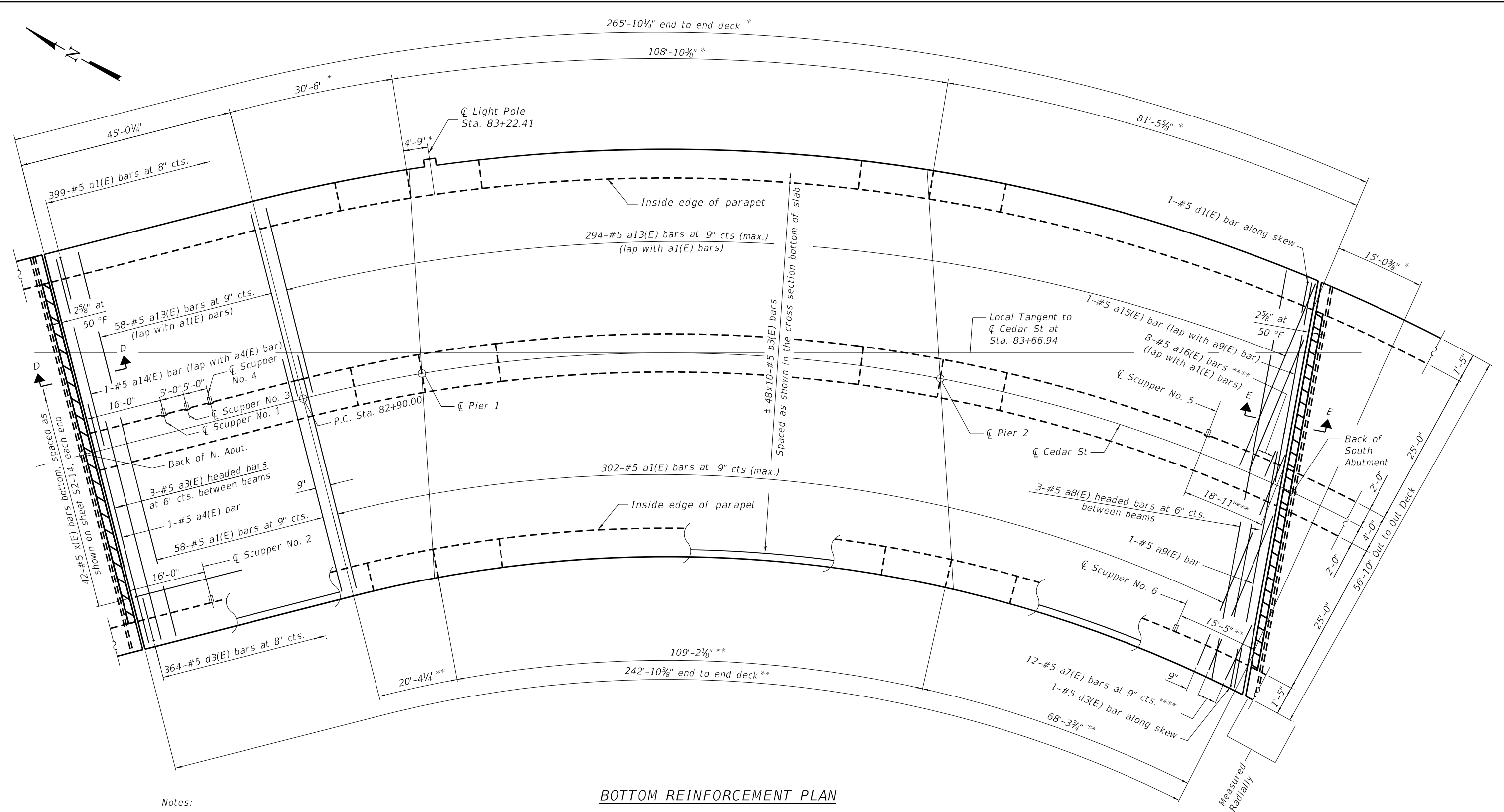
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DECK TOP REINFORCEMENT
STRUCTURE NO. 090-0183

SHEET S2-11 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	731
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

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Notes:

See sheet S2-11 for reinforcement bars in top of deck.

See sheet S2-13 for Deck Cross Section.

See sheet S2-14 for Parapets.

See sheet S2-15 for Section D-D and Section E-E.

See sheet S2-16 for reinforcement bar bending details and Bill of Material.

Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.

BOTTOM REINFORCEMENT PLAN

MINIMUM BAR LAP

#5 bar = 3'-6"
#6 bar = 3'-7"

- * Measured along inside edge of east parapet
- ** Measured along inside edge of west parapet
- *** Measured along east edge of median
- **** See field cutting diagram on sheet S2-16
- † Bars are to be provided straight and are to be sprung and tied at required radius in field

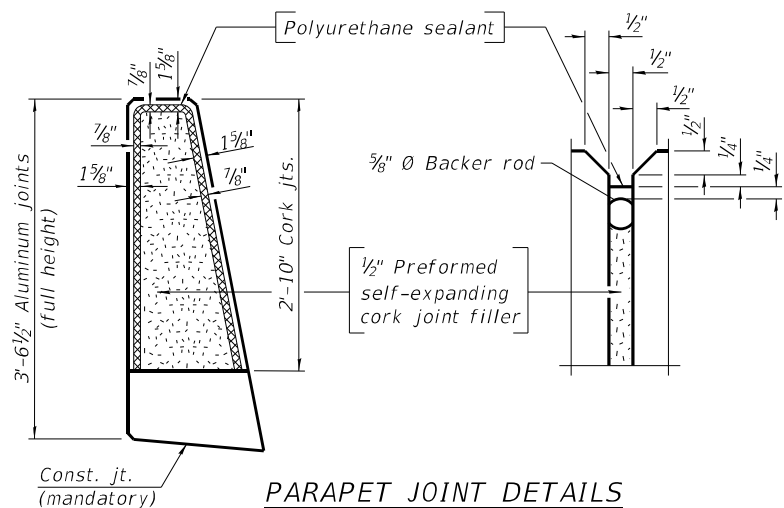
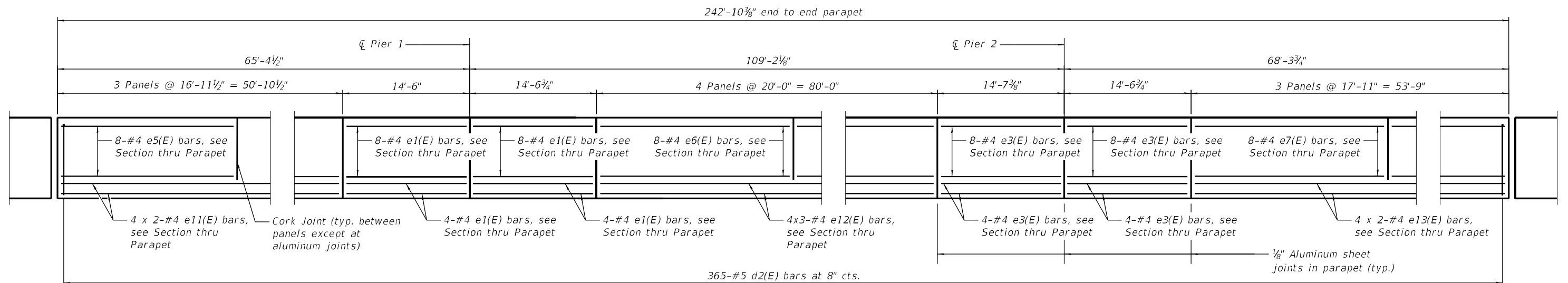
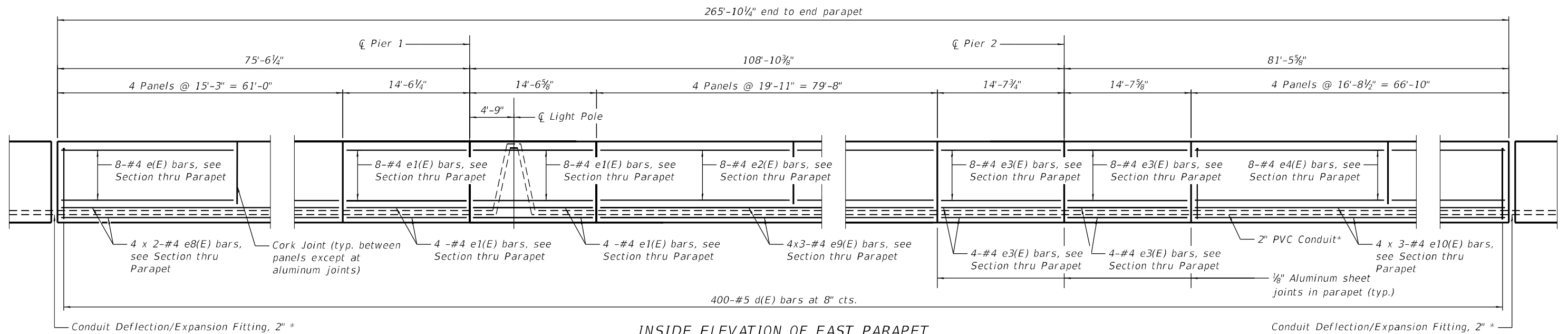


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PLOT DATE = 6/6/2023	CHECKED - PK	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

DECK BOTTOM REINFORCEMENT STRUCTURE NO. 090-0183
SHEET S2-12 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	732
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



MINIMUM BAR LAP

Notes:

All horizontal dimensions are measured along the inside edge of the parapet.

The 1/8" aluminum sheet shall be ASTM B209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.

* For Pay Item See Electrical Drawings.

USER NAME = DIamb	DESIGNED - SPK	REVISED -
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Hatched area to be poured after superstructure forms have been removed. Quantity of concrete included with Concrete Superstructure for abutment.

For details of expansion joint, See sheets S2-21 and S2-22.

a(E) or a12(E)
6- #5 x(E) bars at 12" cts. (typical between girders)

a(E), a6(E), a12(E), or a19(E)
6- #5 x(E) bars at 12" cts. (typical between girders)

For details of expansion joint, See sheets S2-21 and S2-22.

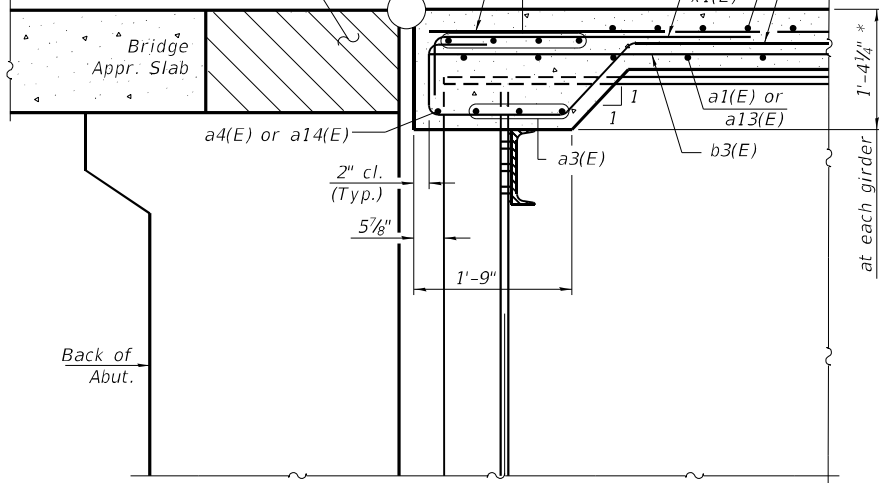
Hatched area to be poured after superstructure forms have been removed. Quantity of concrete included with Concrete Superstructure for abutment.

Notes:

See sheet S2-16 for superstructure details and Bill of Material.

The x(E) and x1(E) bars shall be placed parallel to the girders. Spacing for these bars shall be at right angles to the girders.

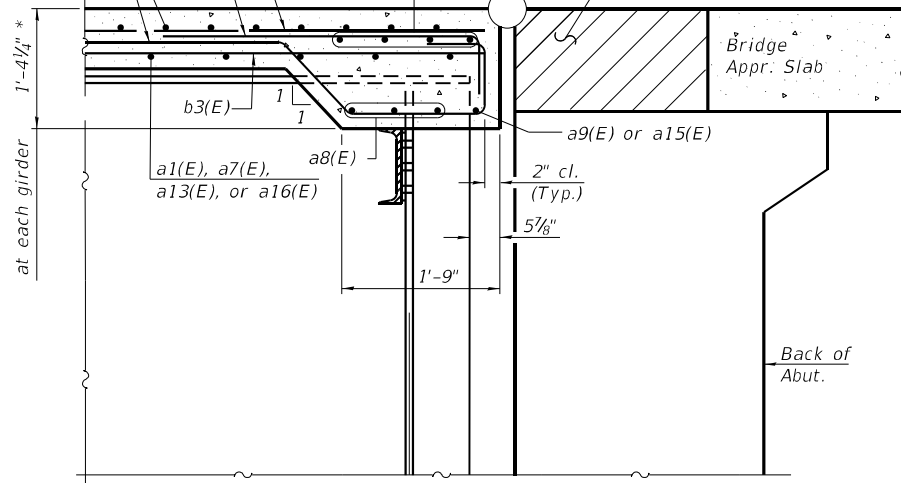
* Prior to grinding



SECTION D-D

(at Rt. L's)

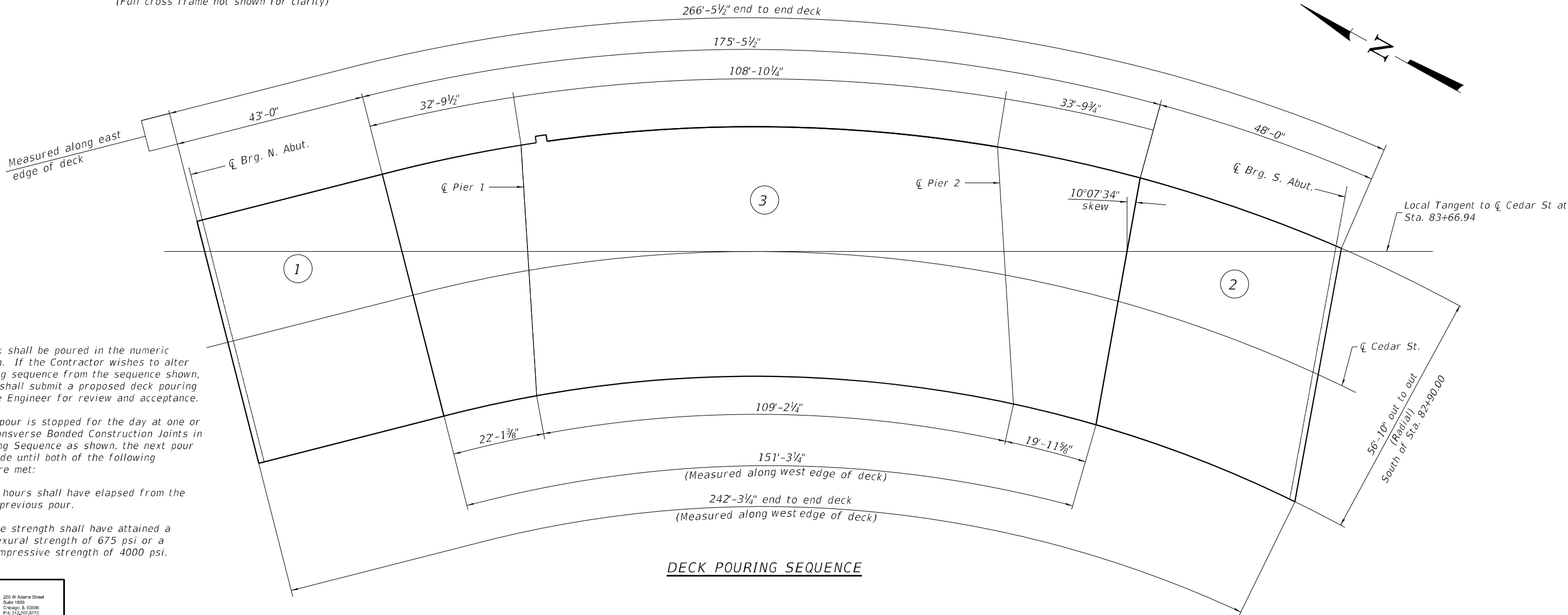
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SECTION E-E

(at Rt. L's)

(Full cross frame not shown for clarity)



Notes:

The bridge deck shall be poured in the numeric sequence shown. If the Contractor wishes to alter the deck pouring sequence from the sequence shown, the Contractor shall submit a proposed deck pouring sequence to the Engineer for review and acceptance.

When the deck pour is stopped for the day at one or more of the transverse Bonded Construction Joints in the Deck Pouring Sequence as shown, the next pour shall not be made until both of the following requirements are met:

1. At least 72 hours shall have elapsed from the end of the previous pour.
2. The concrete strength shall have attained a minimum flexural strength of 675 psi or a minimum compressive strength of 4000 psi.

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INFRASTRUCTURE
ENGINEERING

USER NAME = DLamb	DESIGNED - SPK	REVISED -
	CHECKED - PK	REVISED -
PLOT SCALE = 0.1667' / in.	DRAWN - CLL	REVISED -
PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

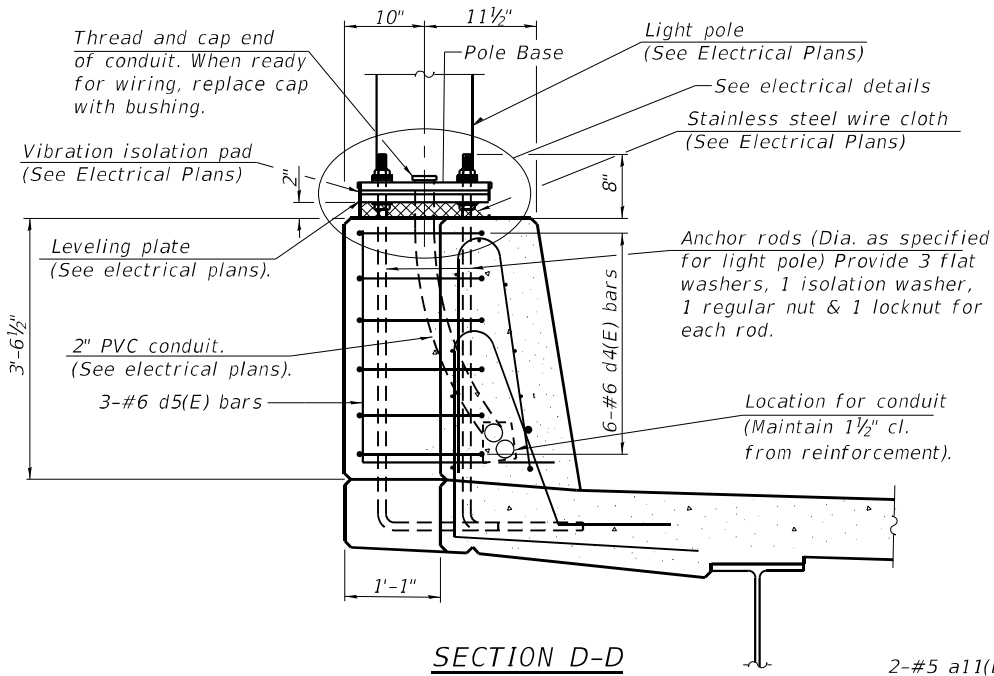
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE SECTIONS & DECK POURING SEQUENCE
STRUCTURE NO. 090-0183

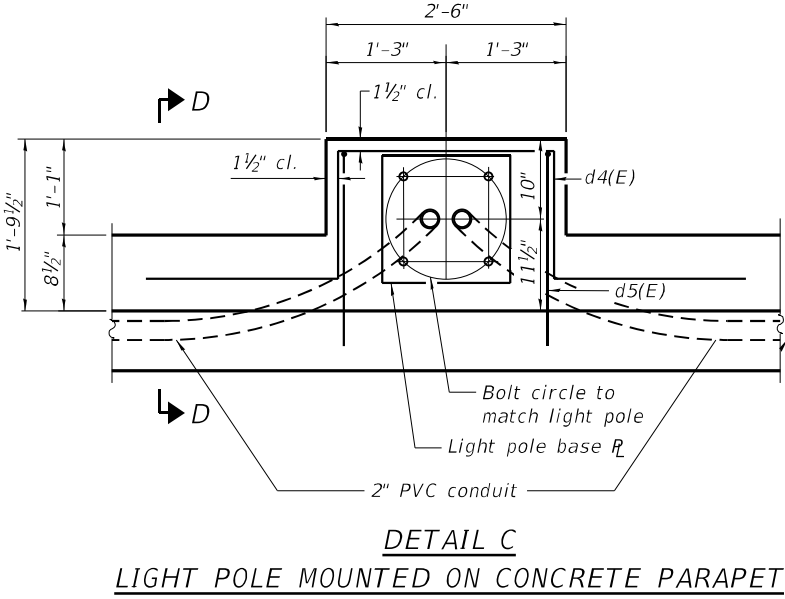
SHEET S2-15 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	735
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

MODEL: Default
FILE: 200 W Adams Street, Suite 1000, Chicago, IL 60606
P: 312.707.8770
F: 312.707.8854
PROJECT: 179740, IDOT CedarStExt, PH115.0, CAD/CAD Sheets/Main St, Bridge Sheets/09001B3-48C55-015-SuperDet2.dgn



SECTION D-D



DETAIL C
LIGHT POLE MOUNTED ON CONCRETE PARAPET

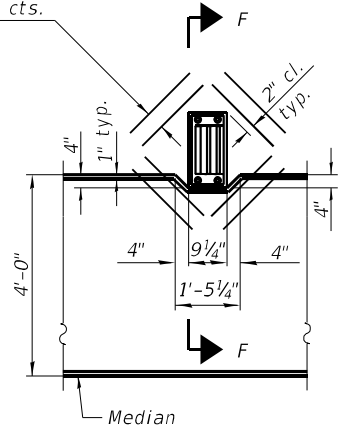
Notes:

Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated.

Cost of anchor rods is included with Concrete Superstructure.

Cut longitudinal reinforcement to clear drainage scuppers.

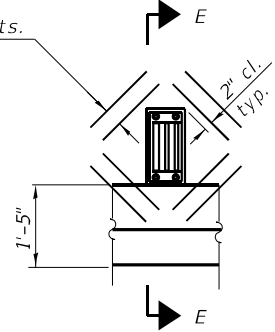
2-#5 a11(E) bars at 4" cts.
tied to bottom of top
reinforcement mat. typ.



DETAIL B

Typical for scuppers along
the median

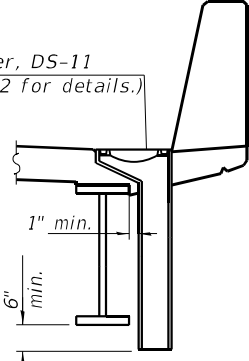
2-#5 a11(E) bars at 4" cts.
tied to bottom of top
reinforcement mat. typ.



DETAIL A

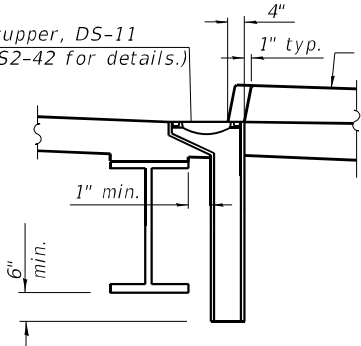
Typical for scuppers along
the parapet

Drainage Scupper, DS-11
(See sheet S2-42 for details.)

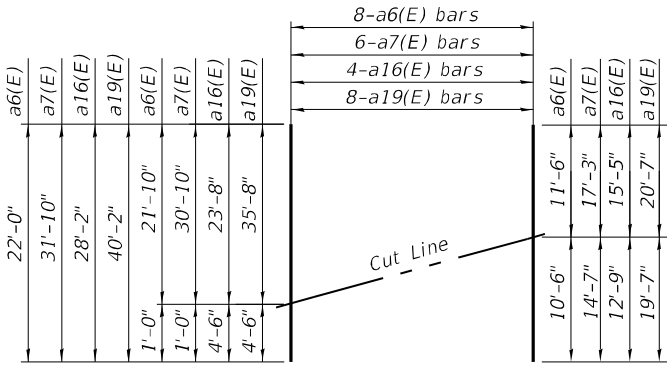


SECTION E-E

Drainage Scupper, DS-11
(See sheet S2-42 for details.)

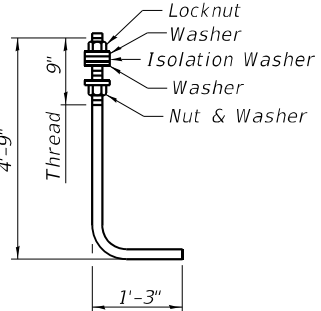


SECTION F-F



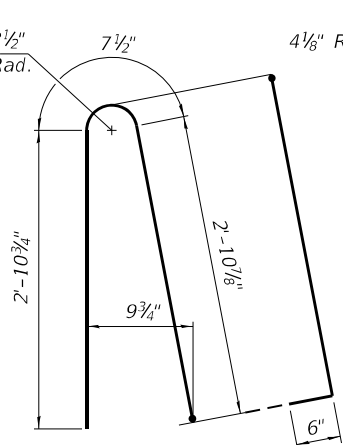
FIELD CUTTING DIAGRAM

Order a6(E), a7(E), a16(E), and a19(E) bars full length.
Cut to fit as shown and place at one end of the dimension line shown on the plan.
Place remainder of bars at the opposite end of the dimension line.

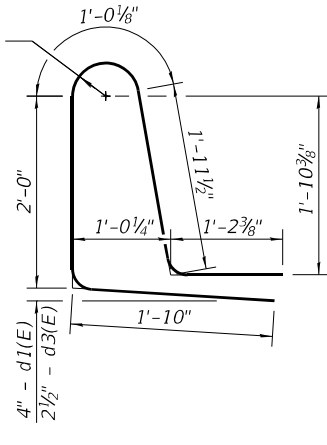


ANCHOR ROD

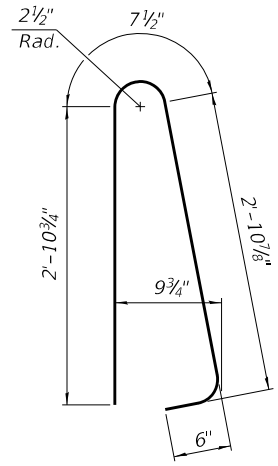
Diameter as specified for light poles.
(ASTM F 1554 Grade 105) full length
hot dipped galvanized



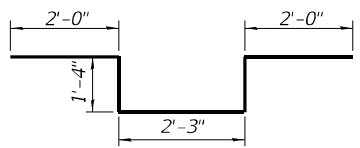
BAR d(E)



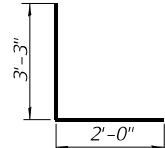
BARS d1(E) & d3(E)



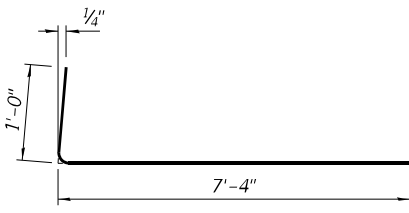
BAR d2(E)



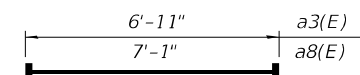
BAR d4(E)



BAR d5(E)

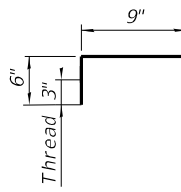


BAR a2(E)

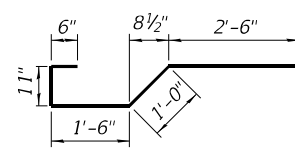


BARS a3(E) & a8(E)

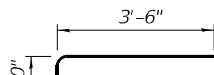
(Headed)



BAR c1(E)



BAR x(E)



BAR x1(E)

SUPERSTRUCTURE
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a1(E)	592	#5	22'-9"	
a2(E)	360	#5	33'-8"	
a3(E)	1,184	#6	8'-4"	
a4(E)	21	#5	6'-11"	
a5(E)	1	#5	33'-8"	
a6(E)	4	#5	22'-9"	
a7(E)	8	#5	22'-0"	
a8(E)	6	#5	31'-10"	
a9(E)	21	#5	7'-1"	
a10(E)	1	#5	34'-7"	
a11(E)	4	#5	23'-4"	
a12(E)	48	#5	1'-6"	
a13(E)	576	#5	37'-4"	
a14(E)	352	#5	26'-5"	
a15(E)	1	#5	26'-5"	
a16(E)	1	#5	27'-1"	
a17(E)	4	#5	28'-2"	
a18(E)	4	#5	37'-4"	
a19(E)	4	#5	38'-4"	
a19(E)	8	#5	40'-2"	
b(E)				
b1(E)	540	#5	32'-8"	
b2(E)	106	#6	34'-11"	
b3(E)	106	#6	32'-9"	
b4(E)	480	#5	29'-9"	
b4(E)	40	#5	34'-11"	
c(E)				
c1(E)	255	#5	3'-6"	
c1(E)	510	#5	1'-3"	
d(E)				
d1(E)	400	#5	7'-0"	
d2(E)	400	#5	8'-0"	
d3(E)	365	#5	7'-0"	
d4(E)	365	#5	8'-0"	
d4(E)	6	#6	8'-11"	
d5(E)	3	#6	5'-3"	
e(E)				
e1(E)	32	#4	14'-11"	
e2(E)	48	#4	14'-3"	
e3(E)	32	#4	19'-7"	
e4(E)	48	#4	14'-3"	
e5(E)	32	#4	16'-5"	
e6(E)	24	#4	16'-8"	
e7(E)	32	#4	19'-8"	
e8(E)	24	#4	17'-5"	
e9(E)	8	#4	31'-7"	
e10(E)	12	#4	28'-1"	
e11(E)	12	#4	23'-10"	
e12(E)	8	#4	26'-6"	
e13(E)	12	#4	28'-2"	
e13(E)	8	#4	27'-11"	
x(E)				
x1(E)	84	#5	6'-5"	
x1(E)	120	#5	4'-4"	
Concrete Superstructure		Cu. Yd.	491.3	
Protective Coat		Sq. Yd.	1,806	
Reinforcement Bars, Epoxy Coated		Pound	139,860	
Diamond Grinding (Bridge Section)		Sq. Yd.	1,187	
Bridge Deck Grooving (Longitudinal)		Sq. Yd.	904	

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INFRASTRUCTURE
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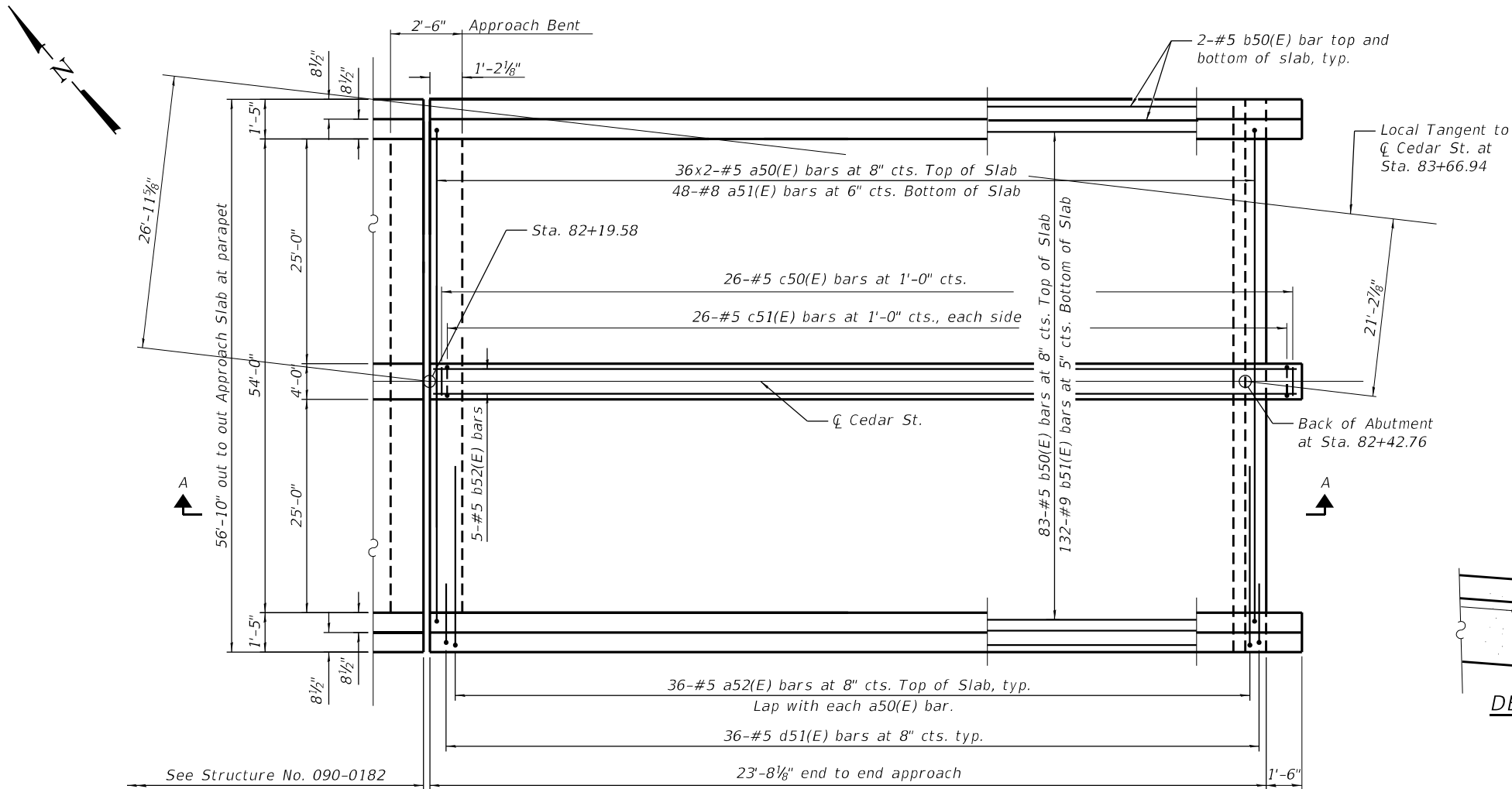
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

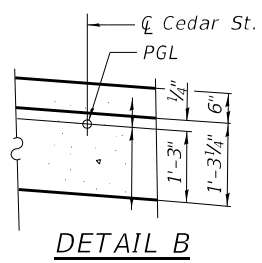
SUPERSTRUCTURE DETAILS
STRUCTURE NO. 090-0183

SHEET S2-16 OF S2-47 SHEETS

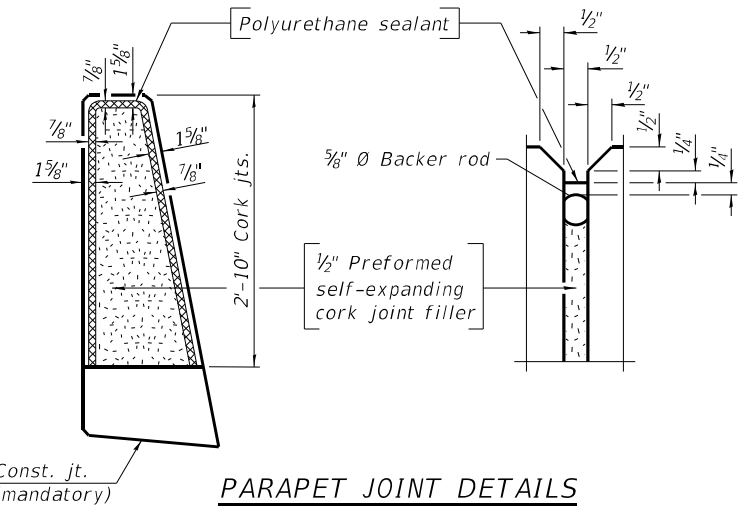
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VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	736
				CONTRACT NO. 68C55
				ILLINOIS FED. AID PROJECT



PLAN

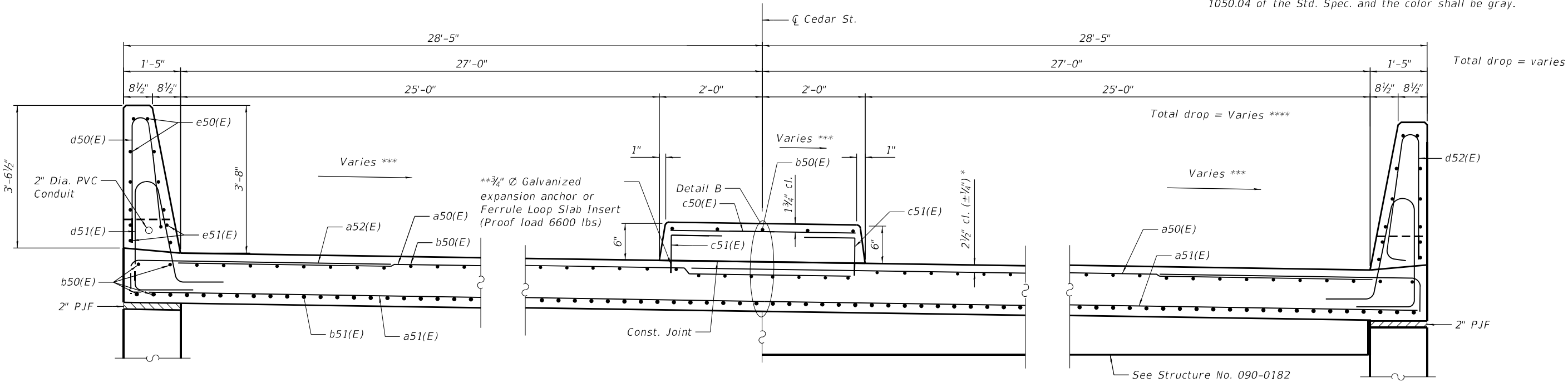


DETAIL B



PARAPET JOINT DETAILS

The Polyurethane Sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.



CROSS SECTION
(Looking South)

AT APPROACH BENT

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PLOT SCALE = 0.1667' / in.	CHECKED - PK	REVISED -
PLOT DATE = 6/6/2023	DRAWN - CLL	REVISED -
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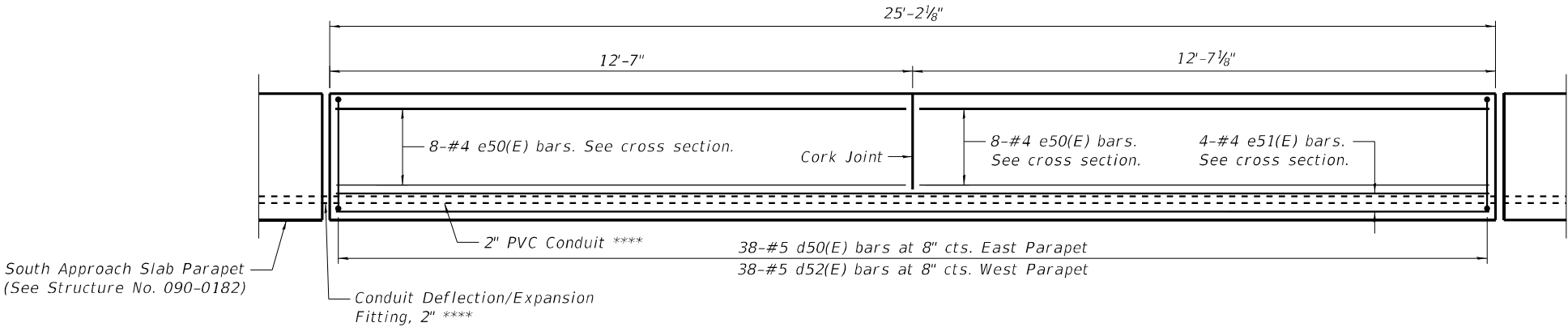
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH BRIDGE APPROACH SLAB PLAN & CROSS-SECTION
STRUCTURE NO. 090-0183

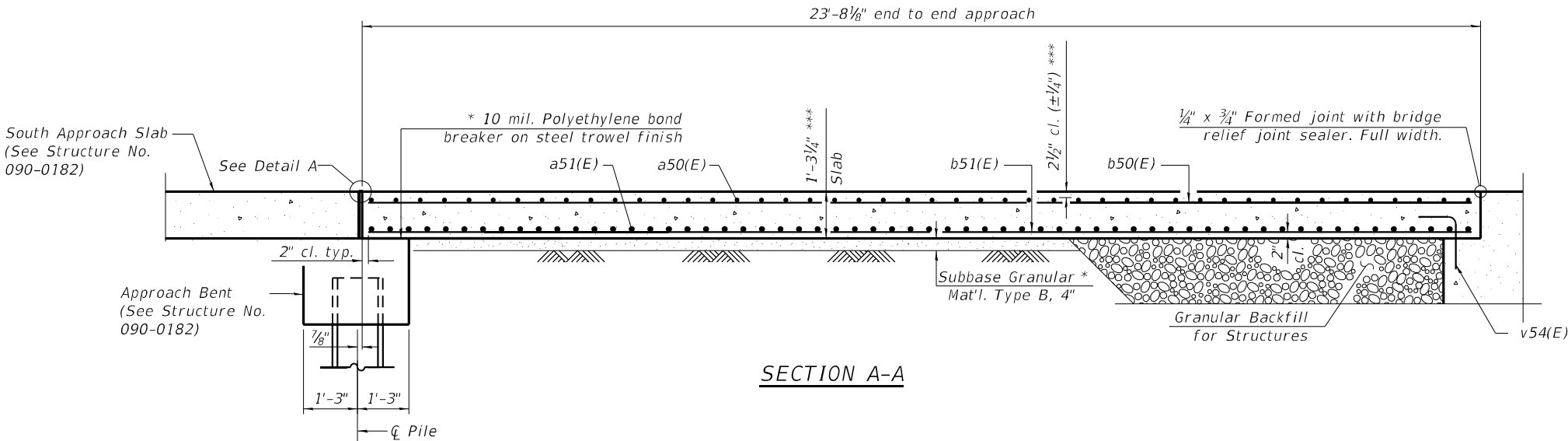
SHEET S2-17 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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				CONTRACT NO. 68C55
				ILLINOIS FED. AID PROJECT

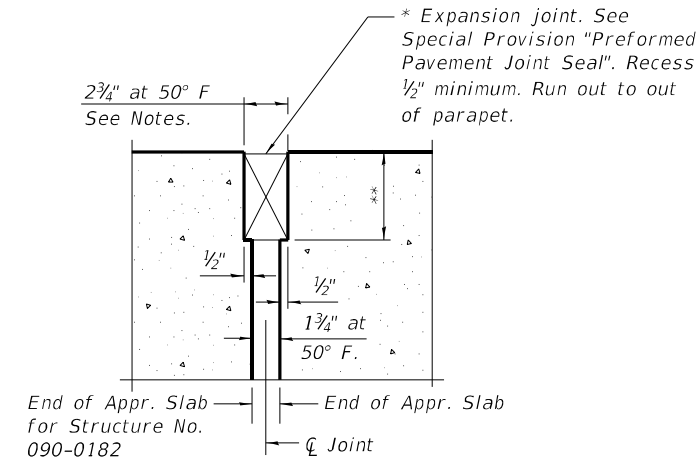
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INSIDE ELEVATION OF PARAPET



SECTION A-A



DETAIL A

* Cost included with Concrete Superstructure (Approach Slab).
** Per manufacturer recommendations
*** Prior to Grinding
**** East parapet only. For pay items see Electrical Drawings

Notes:

The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. The length used to calculate the adjustment shall be equal to the length of both bridge approach slabs on the footing.

Parapet concrete shall be paid for as Concrete Superstructure.

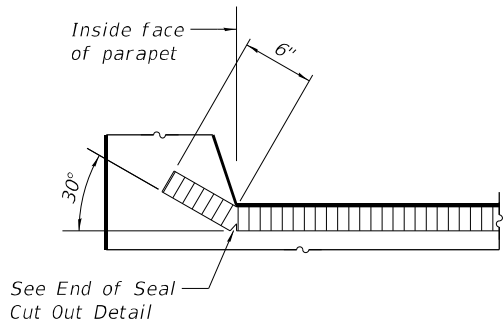
Approach slab shall be paid for as Concrete Superstructure (Approach Slab).

The cost of the North Approach Bent is included with the cost of Structure No. 090-0182.

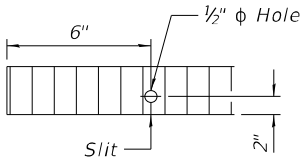
For Parapet Joint Detail see sheet S2-17.

For Granular Backfill for Structures and drainage treatment details, see sheet S2-3.

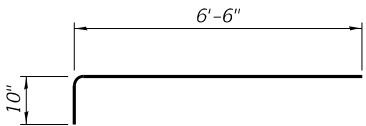
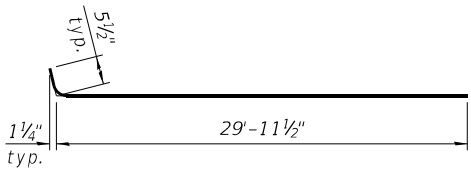
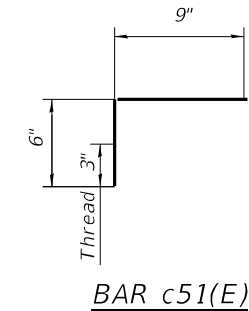
Bar V54(E) is included in the bill of material for the North Abutment. See sheet S2-33.



END OF SEAL



END OF SEAL CUT-OUT



NORTH APPROACH
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a50(E)	72	#5	30'-5"	
a51(E)	48	#8	56'-6"	
a52(E)	72	#5	7'-4"	
b50(E)	91	#5	23'-4"	
b51(E)	132	#9	23'-4"	
b52(E)	5	#5	24'-10"	
c50(E)	26	#5	3'-6"	
c51(E)	52	#5	1'-3"	
d50(E)	38	#5	6'-11"	
d51(E)	72	#5	8'-6"	
d52(E)	38	#5	6'-11"	
e50(E)	32	#4	12'-3"	
e51(E)	8	#4	24'-10"	
Concrete Superstructure			Cu. Yd.	8.9
Protective Coat			Sq. Yd.	170
Concrete Superstructure (Approach Slab)			Cu. Yd.	63.5
Reinforcement Bars, Epoxy Coated			Pound	24,640
Diamond Grinding (Bridge Section)			Sq. Yd.	110
Bridge Deck Grooving (Longitudinal)			Sq. Yd.	84

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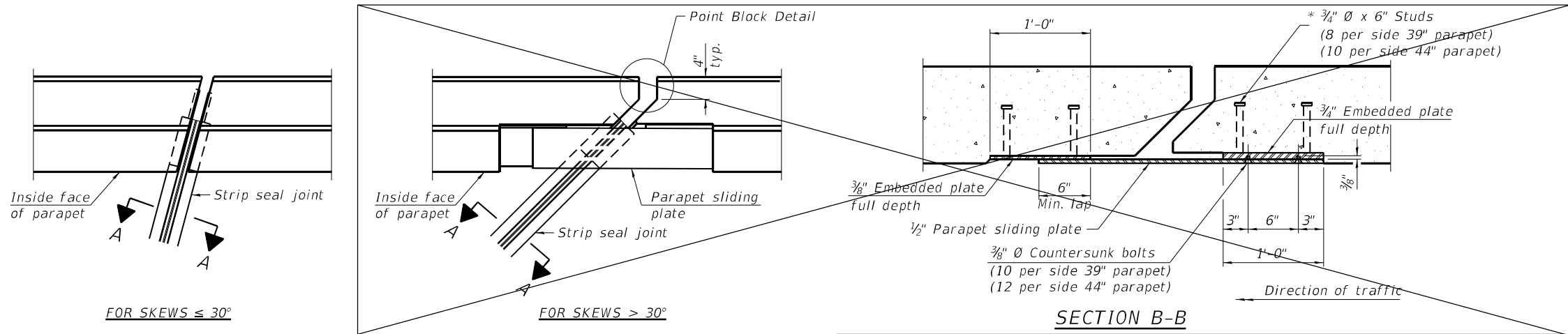
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PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 090-0183

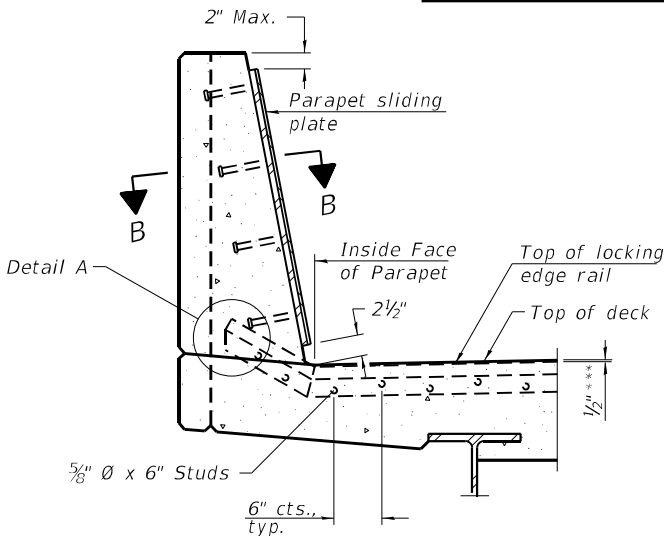
SHEET S2-18 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 68C55	
			ILLINOIS FED. AID PROJECT	



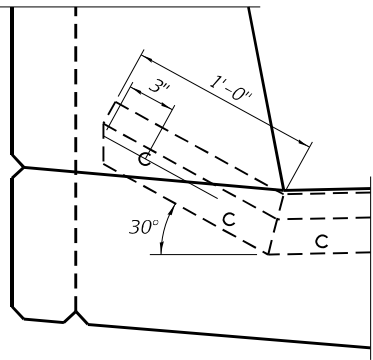
PLAN AT PARAPET

SECTION B-B

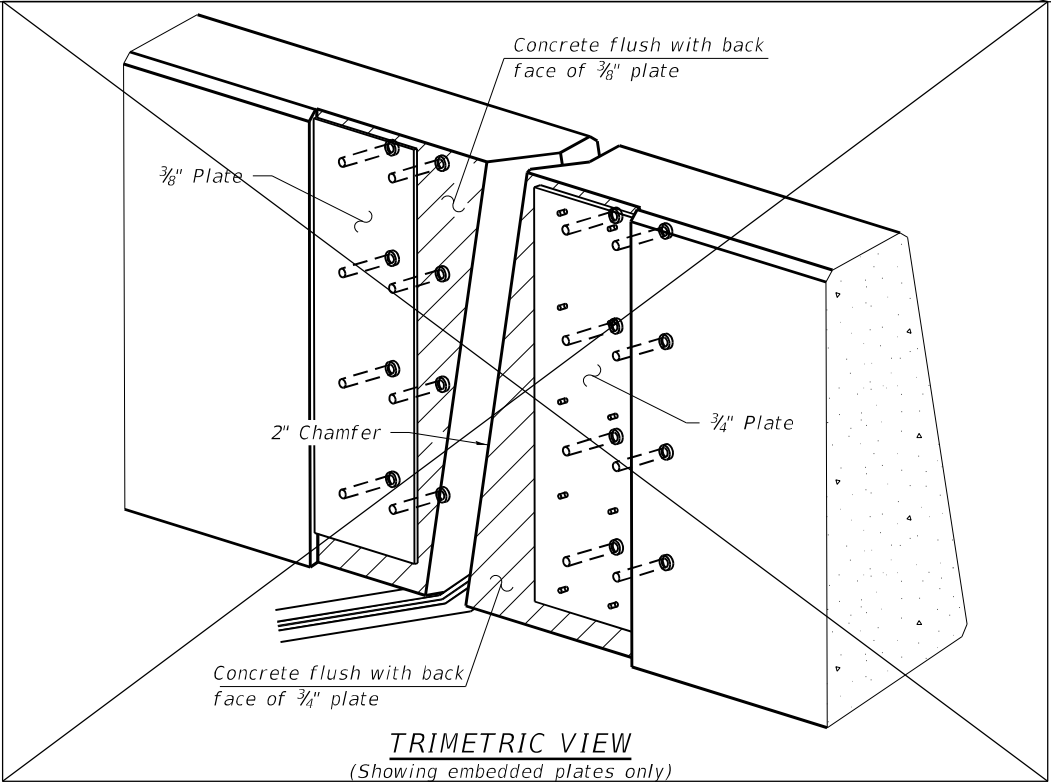


SECTION AT PARAPET

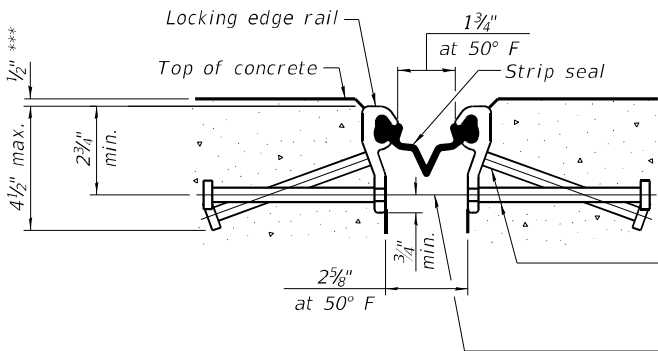
(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



DETAIL A



TRIMETRIC VIEW
(Showing embedded plates only)



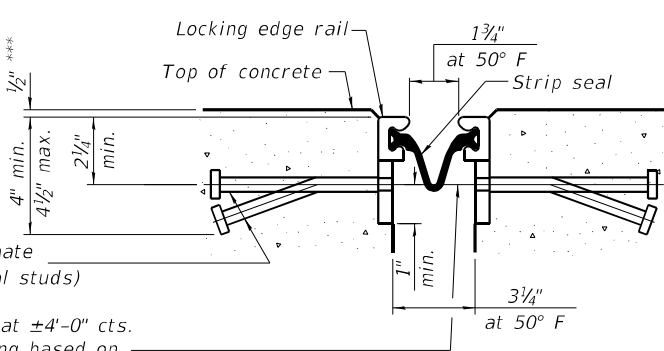
SHOWING ROLLED RAIL JOINT

* 3/8" Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs)

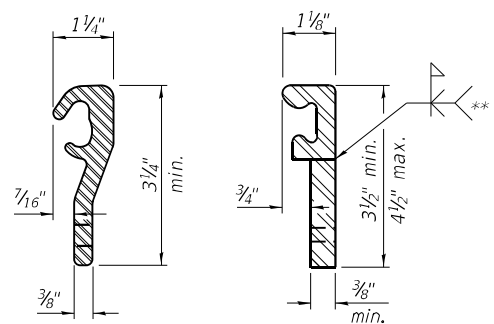
3/8" Ø threaded rods in 7/16" Ø holes at ±4'-0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

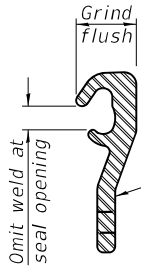


SHOWING WELDED RAIL JOINT



LOCKING EDGE RAILS

** Back gauge not required if complete joint penetration is verified by mock-up.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	114

Notes:

The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4 1/2" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be 3/16" and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier similar as noted.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

*** Prior to grinding

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INFRASTRUCTURE
ENGINEERING

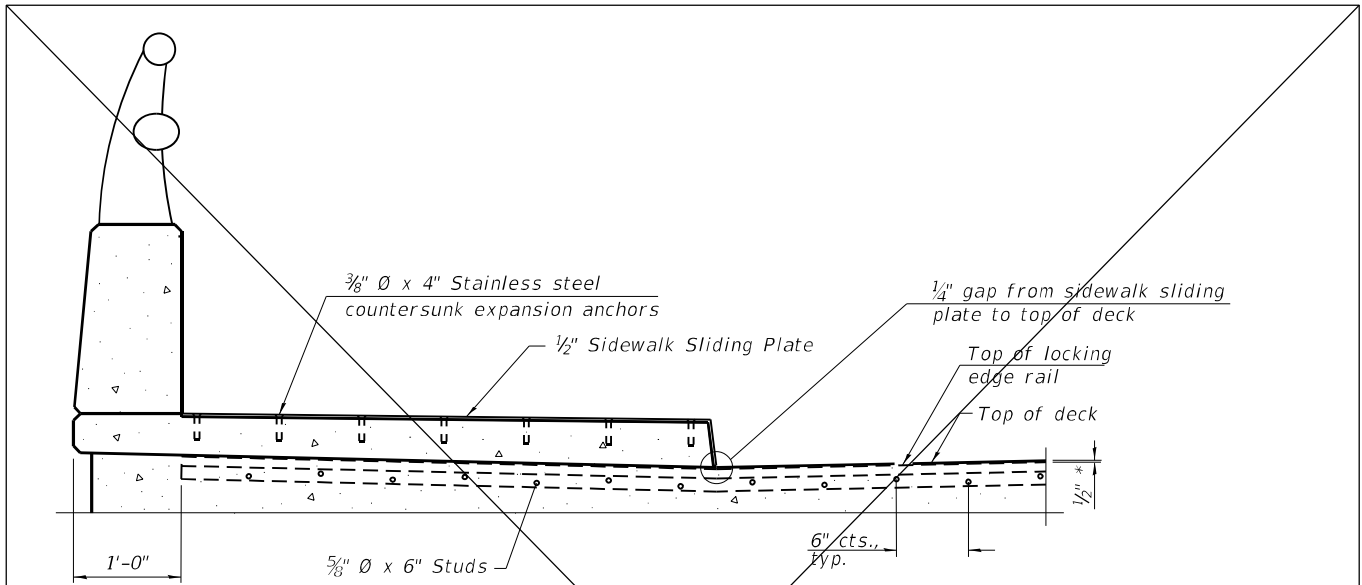
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	CHECKED - PK	REVISED -
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PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

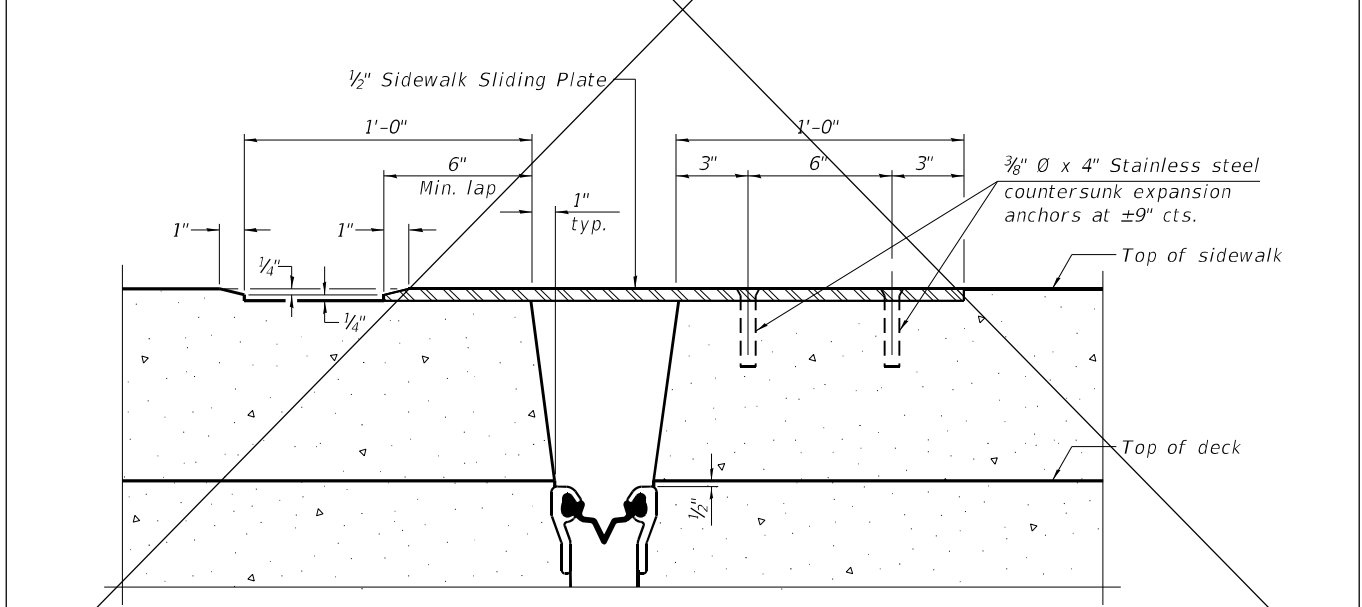
MODIFIED PREFORMED JOINT STRIP SEAL – MEDIAN (1 of 2)
STRUCTURE NO. 090-0183

SHEET S2-21 OF S2-47 SHEETS

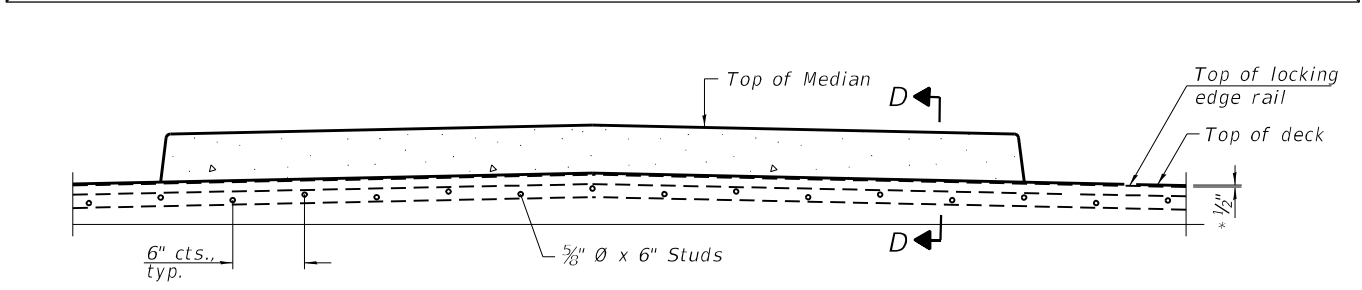
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VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	741
CONTRACT NO. 68C55				
ILLINOIS				FED. AID PROJECT



SECTION AT RAISED SIDEWALK



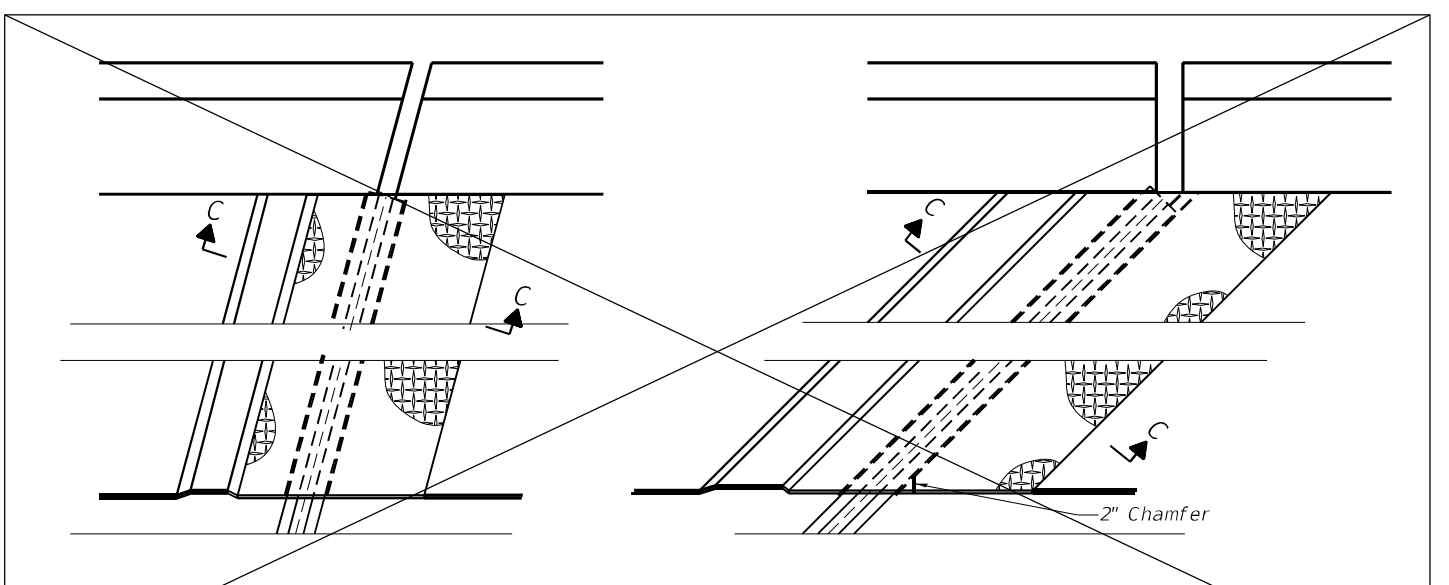
SECTION C-C



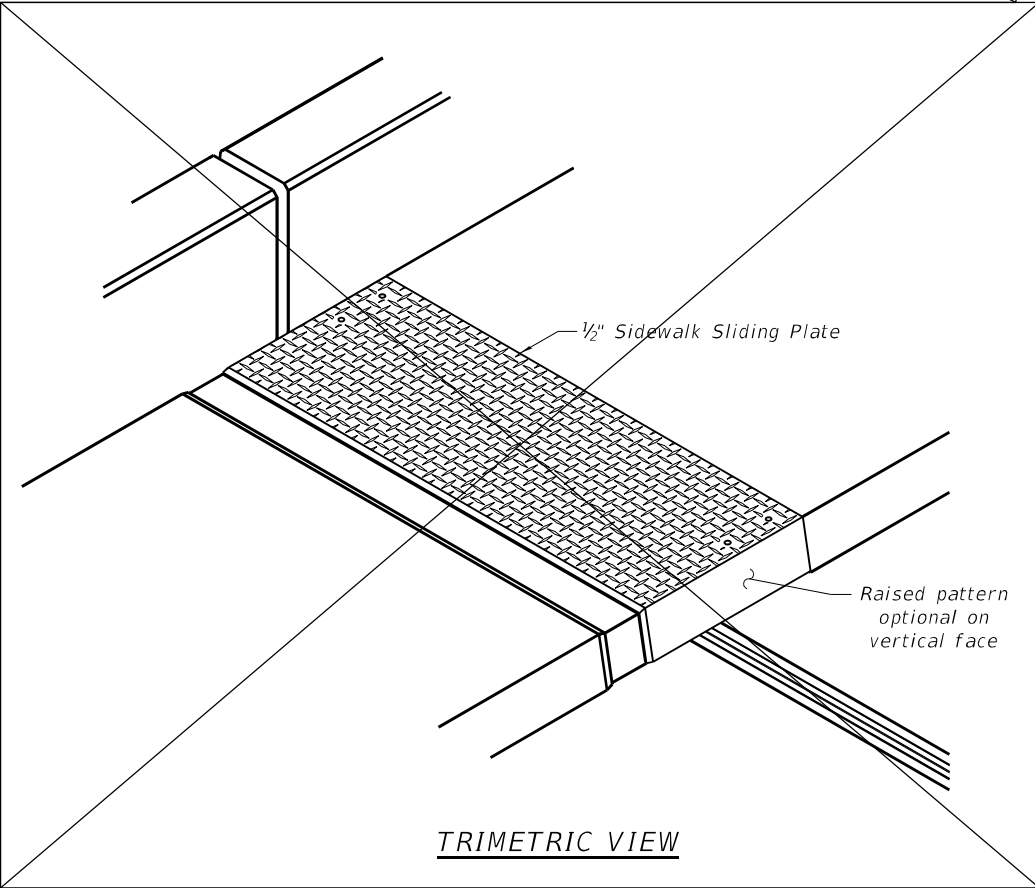
SECTION AT MEDIAN

For skews > 30°, chamfer acute corners 2" similar to sidewalk.

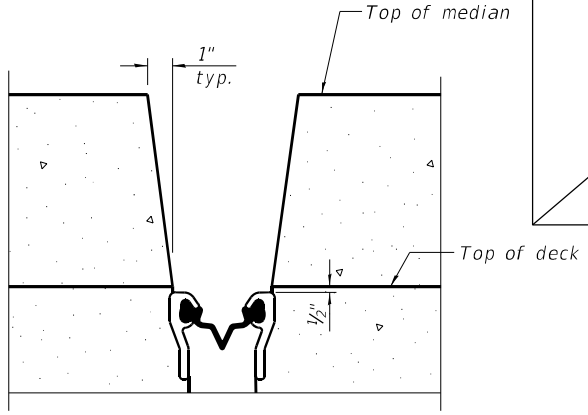
* Prior to grinding



PLAN AT RAISED SIDEWALK



TRIMETRIC VIEW



SECTION D-D
(at Rt. L's)

Michael Baker
INTERNATIONAL

USER NAME = D1amb	DESIGNED - SPK	REVISED -
PLOT SCALE = 0.1667' / in.	CHECKED - PK	REVISED -
PLOT DATE = 6/6/2023	DRAWN - CLL	REVISED -
	CHECKED - SPK	REVISED -

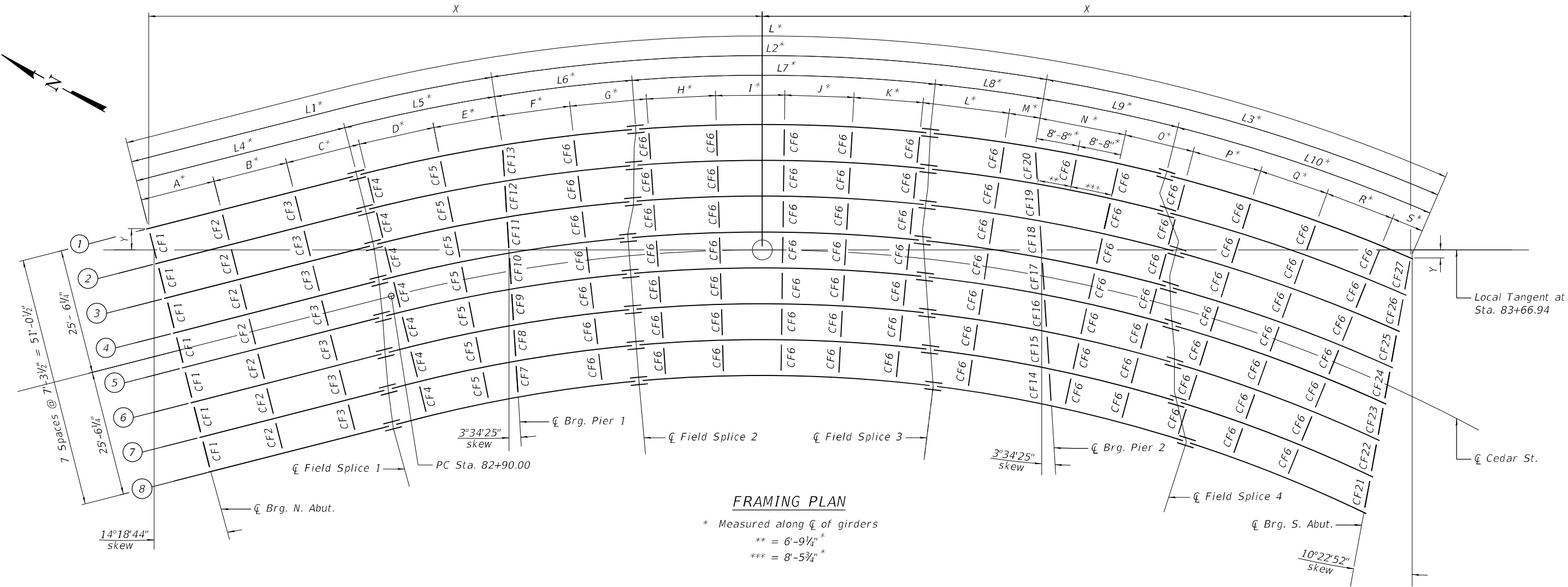
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

MODIFIED PREFORMED JOINT STRIP SEAL – MEDIAN (2 of 2)
STRUCTURE NO. 090–0183

SHEET S2-22 OF 52-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	742
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

MODEL: Default
FILE: \bentley\project\bus-pw\03\Documents\Chicago_I\U.I. Project\179740_IDOT_CedarStExt_Plan_S0_CADD\CAD_Sheet\MaIn_St_Bridge_Sheets\09001B3-6C55-023-FramingPlan.dgn



FRAMING PLAN

* Measured along ζ of girders
** = 6'-9 1/4" *
*** = 8'-5 3/4" *

GIRDER DIMENSIONS

Girder	Radius	L	L1	L2	L3	L4	L5	L6	L7	L8	L9	L10
1	333'-6 1/4"	262'-7 1/2"	73'-11 9/16"	108'-10 7/16"	79'-9 1/2"	43'-8"	30'-3 3/16"	27'-3 7/16"	60'-0"	21'-7"	27'-2"	52'-7 1/2"
2	326'-2 3/4"	259'-6 3/16"	72'-7 1/8"	108'-10 7/8"	78'-0 3/16"	43'-8"	28'-11 1/8"	26'-5 7/8"	60'-0"	22'-5"	26'-11"	51'-1 3/16"
3	318'-11 1/4"	256'-4 1 3/16"	71'-2 1 1/16"	108'-11 1/16"	76'-2 1 3/16"	43'-6"	27'-8 1 1/16"	25'-10 3/16"	60'-0"	23'-1"	28'-7"	47'-7 1 3/16"
4	311'-7 3/4"	253'-3 1/2"	69'-10 1/4"	108'-11 3/4"	74'-5 1/2"	42'-9"	27'-1 1/4"	24'-4 3/4"	60'-0"	24'-7"	26'-7"	47'-10 1/2"
5	304'-4 1/4"	250'-2 1/4"	68'-5 1 3/16"	109'-0 1/4"	72'-8 3/16"	41'-8"	26'-9 1 3/16"	24'-5 3/16"	60'-0"	24'-7 1/16"	26'-7 1 5/16"	46'-0 1/4"
6	297'-0 3/4"	247'-1"	67'-1 3/8"	109'-0 1 3/16"	70'-10 1 3/16"	40'-4"	26'-9 3/8"	24'-5 5/8"	60'-0"	24'-7 3/16"	26'-7 1 3/16"	44'-3"
7	289'-9 1/4"	243'-11 1 3/16"	65'-8 1 5/16"	109'-1 3/8"	69'-1 1 1/2"	39'-1"	26'-7 1 5/16"	24'-7 1/16"	60'-0"	24'-6 5/16"	26'-10 1 1/16"	42'-2 1 3/16"
8	282'-5 3/4"	240'-10 5/8"	64'-4 1/2"	109'-2"	67'-4 1/8"	38'-2"	26'-2 1/2"	24'-9 1/2"	60'-0"	24'-4 1/2"	28'-5 1/2"	38'-10 5/8"

GIRDER COORDINATES

Girder	ζ Brg. N. Abutment		Field Splice 1		Pier 1		Field Splice 2		Field Splice 3		Pier 2		Field Splice 4		ζ Brg. S. Abutment	
	X	Y	X	Y	X	Y	X	Y	X	Y	X	Y	X	Y	X	Y
1	-124'-10"	4'-4 1/4"	-82'-6 1/4"	15'-1 3/16"	-52'-10 3/8"	21'-3 5/8"	-25'-9 9/16"	24'-6 1/4"	34'-1 5/8"	23'-9 1/4"	55'-6 1/4"	20'-10 3/8"	82'-1 1/8"	15'-3 1/8"	131'-10 7/16"	-1'-7 7/8"
2	-123'-0 7/16"	-2'-8 1/2"	-80'-8 5/8"	8'-1"	-52'-4 1 5/16"	13'-11 7/8"	-26'-1 7/16"	17'-2 3/16"	33'-9 1/2"	16'-5 1 1/16"	55'-11 7/8"	13'-4 1 1/16"	82'-3 7/16"	7'-8 3/16"	130'-6 1/4"	-9'-0 1/4"
3	-121'-2 1 1/16"	-9'-9 3/16"	-79'-0 1 3/16"	11 3/4"	-51'-11 1/16"	6'-8 1/8"	-26'-3 9/16"	9'-10 1/4"	33'-7 3/16"	9'-1 1 3/16"	56'-5 1/2"	5'-10 1 3/16"	84'-3 1 5/16"	-4 1/16"	129'-2 1/16"	-16'-4 1 1/16"
4	-119'-5 1/16"	-16'-10 1/16"	-78'-0"	-6'-3 1/4"	-51'-5 1 5/16"	-7 1 1/16"	-27'-3 5/8"	2'-5 3/8"	32'-7 1/4"	1'-11 1/4"	56'-11 1/8"	-1'-7 7/16"	82'-9 7/8"	-7'-6 3/4"	127'-9 1 3/16"	-23'-9 1/2"
5	-117'-7 7/16"	-23'-10 7/8"	-77'-2 1 5/16"	-13'-7 1/4"	-51'-0 7/16"	-7'-11 1/2"	-26'-9 3/4"	-4'-9 1 5/16"	33'-1"	-5'-5 3/8"	57'-4 3/4"	-9'-1 1/4"	83'-3 7/8"	-15'-3 5/16"	126'-5 5/16"	-31'-2"
6	-115'-9 1 3/16"	-30'-11 3/8"	-76'-8 3/16"	21'-0"	-50'-7"	-15'-3 5/16"	-26'-3 1 5/16"	-12'-1 1/4"	33'-6 1 3/16"	-12'-10 1/16"	57'-10 3/8"	-16'-7 1/2"	83'-8 7/8"	-22'-11 3/16"	125'-1 1/16"	-38'-6 1 3/16"
7	-114'-0 3/16"	-38'-0 7/16"	-76'-1 3/4"	-28'-4 1/2"	-50'-1 1/2"	-22'-7 3/16"	-25'-9 1/16"	-19'-4 1/2"	34'-1 1/16"	-20'-2 1 5/16"	58'-4"	-24'-1 1 5/16"	84'-4 7/8"	-30'-9 7/16"	123'-9"	-45'-11 1 3/16"
8	-112'-2 9/16"	-45'-1 3/16"	-75'-2 3/4"	-35'-8"	-49'-8"	-29'-11 1/16"	-25'-1 1/4"	-26'-7 1 1/16"	34'-9 3/16"	-27'-8 7/16"	58'-9 1 1/16"	-31'-8 1/2"	86'-3 9/16"	-39'-0 9/16"	122'-4 1 1/16"	-53'-4 1 5/16"

Notes:

All cross frames between girders shall be installed with erection pins and bolts in accordance with the erection plan approved by the Engineer. Individual cross frames at supports may be temporarily disconnected to install bearing anchor rods.

For dimensions A thru S see Cross Frame Spacing Table on Sheet S2-27.

For steel self weight deflection, see sheet S2-26.

For cross frame steel angle at Pier 1, Pier 2, and South Abutment, see angle "B" on sheet S2-28.

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INFRASTRUCTURE
ENGINEERING

USER NAME = D1amb	DESIGNED - SPK	REVISED -
	CHECKED - PK	REVISED -
PLOT SCALE = 21.3332" = 1'	DRAWN - MD	REVISED -
PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN
STRUCTURE NO. 090-0183

SHEET S2-23 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	743
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

MODEL: Default
FILE: \\miller-prod\bus-sch\ben\ben\com\bus-pw\03\Documents\Chicago_I\U01_Projects\179740_IDOT_CedarStExt_Phl15.0_CADD\C40_Sheets\Main_St_Bridge_Sheets\09001013-48C55-02-1-Girder\68_1.dgn

EXTERIOR GIRDER 1 MOMENT TABLE					
	0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.6 Sp. 3
Is	(in ⁴) 20,583	28,642	20,583	28,642	20,583
Ic(n)	(in ⁴) 54,123	56,758	54,123	56,758	54,123
Ic(3n)	(in ⁴) 38,611	-	38,611	-	38,611
Ic(cr)	(in ⁴) -	32,867	-	32,867	-
Ss	(in ³) 1,167	1,273	1,167	1,273	1,167
Sc(n)	(in ³) 1,536	-	1,536	-	1,536
Sc(3n)	(in ³) 1,423	-	1,423	-	1,423
Sc(cr)	(in ³) -	1,337	-	1,337	-
Sxc	(in ³) 1,514	1,325	1,483	1,324	1,504
DC1	(k/')	0.96	1.01	0.96	1.01
MDC1	('k)	252	1,009	619	1,081
DC2	(k/')	0.20	0.20	0.20	0.20
MDC2	('k)	53	192	129	207
DW	(k/')	0.31	0.31	0.31	0.31
MDW	('k)	82	299	203	323
M _ƫ + I _M	('k)	986	1,391	1,397	1,482
f _ƫ (Strength I)	(ksi)	0	8.88	0.29	3.01
Mu + 1/3 f _ƫ Sxc	('k)	2,230	4,709	3,696	4,799
Øf Mn	('k)	-	-	-	-
f _s DC1	(ksi)	2.59	9.51	6.37	10.19
f _s DC2	(ksi)	0.45	1.72	1.09	1.86
f _s DW	(ksi)	0.69	2.67	1.71	2.90
f _s (ƫ+I _M)	(ksi)	7.70	12.48	10.91	13.30
f _ƫ (Service II)	(ksi)	0	6.58	0.21	2.22
f _s + f ¹ / ₂ (Service II)	(ksi)	13.74	33.41	23.46	33.35
0.95Rh Fyf	(ksi)	47.50	47.50	47.50	47.50
f _s + f ¹ / ₃ (Total)(Strength I)	(ksi)	18.31	42.84	31.08	43.69
Øf F _n	(ksi)	50.00	47.50	50.00	49.00
Vf	(k)	32.0	33.9	35.0	35.3

EXTERIOR GIRDER 1 REACTION TABLE				
	N. Abut.	Pier 1	Pier 2	S. Abut.
RDC1	(k) 23.9	95.3	98.0	30.1
RDC2	(k) 4.8	19.3	19.6	6.1
RDW	(k) 7.4	29.5	30.7	9.5
R _ƫ	(k) 53.4	98.8	104.1	69.2
R _{I_M}	(k) 12.6	20.7	21.6	16.8
RTotal	(k) 102.1	263.6	274.0	131.7

INTERIOR GIRDER 2 MOMENT TABLE					
	0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.6 Sp. 3
Is	(in ⁴) 20,583	28,642	20,583	28,642	20,583
Ic(n)	(in ⁴) 55,688	58,229	55,688	58,229	55,688
Ic(3n)	(in ⁴) 39,993	-	39,993	-	39,993
Ic(cr)	(in ⁴) -	33,310	-	33,310	-
Ss	(in ³) 1,167	1,273	1,167	1,273	1,167
Sc(n)	(in ³) 1,546	-	1,546	-	1,546
Sc(3n)	(in ³) 1,435	-	1,435	-	1,435
Sc(cr)	(in ³) -	1,343	-	1,343	-
Sxc	(in ³) 1,525	1,329	1,494	1,328	1,517
DC1	(k/')	1.05	1.10	1.05	1.10
MDC1	('k)	241	1,044	593	1,142
DC2	(k/')	0.20	0.20	0.20	0.20
MDC2	('k)	49	185	118	198
DW	(k/')	0.31	0.31	0.31	0.31
MDW	('k)	79	297	196	320
M _ƫ + I _M	('k)	898	1,184	1,101	1,222
f _ƫ (Strength I)	(ksi)	0	8.10	0.54	2.87
Mu + 1/3 f _ƫ Sxc	('k)	2,025	4,376	3,141	4,420
Øf Mn	('k)	-	-	-	-
f _s DC1	(ksi)	2.48	9.84	6.10	10.77
f _s DC2	(ksi)	0.41	1.65	0.99	1.77
f _s DW	(ksi)	0.66	2.65	1.64	2.86
f _s (ƫ+I _M)	(ksi)	6.97	10.58	8.55	10.92
f _ƫ (Service II)	(ksi)	0	6.01	0.39	2.13
f _s + f ¹ / ₂ (Service II)	(ksi)	12.61	30.90	20.04	30.66
0.95Rh Fyf	(ksi)	47.50	47.50	47.50	47.50
f _s + f ¹ / ₃ (Total)(Strength I)	(ksi)	16.80	39.55	26.47	40.03
Øf F _n	(ksi)	50.00	47.40	50.00	49.30
Vf	(k)	25.4	24.4	24.8	26.1

INTERIOR GIRDER 2 REACTION TABLE				
	N. Abut.	Pier 1	Pier 2	S. Abut.
RDC1	(k) 24.7	120.6	128.4	28.3
RDC2	(k) 4.6	22.6	23.4	5.1
RDW	(k) 7.3	35.4	37.5	8.2
R _ƫ	(k) 55.7	107.1	112.0	59.3
R _{I_M}	(k) 13.8	21.9	22.8	14.9
RTotal	(k) 106.1	307.6	324.1	115.8

- Is, Ss: Non-composite moment of inertia and section modulus of the steel section used for computing fs(Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).
- Ic(n), Sc(n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing fs(Total-Strength I, and Service II) in uncracked sections due to short term composite live loads (in.⁴ and in.³).
- Ic(3n), Sc(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing fs(Total-Strength I, and Service II) in uncracked sections due to long-term composite (superimposed) dead loads (in.⁴ and in.³).
- Ic(cr), Sc(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing fs(Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.⁴ and in.³).
- Sxc: Section modulus about the major axis of section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.³).
- DC1: Un-factored non-composite dead load (kips/ft.).
- MDC1: Un-factored moment due to non-composite dead load (kip-ft.).
- DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
- MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
- DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
- MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
- M_ƫ + I_M: Un-factored live load moment plus dynamic load allowance (impact)(kip-ft.).
- Mu (Strength I): Factored design moment (kip-ft.).
1.25 (MDC1+ MDC2) + 1.5 MDW + 1.75 M_ƫ + I_M
- f_ƫ: Factored calculated normal stress at edge of flange for controlling flange plate due to lateral bending, Strength I or Service II as applicable (kip-ft.).
- Øf Mn: Factored resistance available according to Article A6.1.1 (kips).
- f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
MDC1 / Ss
- f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
MDC2 / Sc(3n) or MDC2 / Sc(cr) as applicable.
- f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
MDW / Sc(3n) or MDW / Sc(cr) as applicable.
- f_s (ƫ+I_M): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live plus impact loads as calculated below (ksi).
M_ƫ + I_M / Sc(n) or M_ƫ + I_M / Sc(cr) as applicable.
- f_s + f¹/₂ (Service II): Sum of stresses as computed below (ksi).
fsDC1 + fsDC2 + fsDW + 1.3 fs(ƫ+ I_M) + f¹/₂
- 0.95RhFyf: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
- f_s + f¹/₃ (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
1.25 (fsDC1 + fsDC2) + 1.5 fsDW + 1.75 fs(ƫ + I_M) + f¹/₃
- Øf F_n: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
- Vf: Maximum factored shear range in span computed according to Article 6.10.10.

Note:
M_ƫ and R_ƫ include the effects of centrifugal force and superelevation.

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	USER NAME = DIamb	DESIGNED - SPK	REVISED -
		CHECKED - PK	REVISED -
	PLOT SCALE = 0.1667 ' / in.	DRAWN - CLL	REVISED -
	PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DESIGN DATA TABLES
STRUCTURE NO. 090-0183

SHEET S2-24 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	744
		CONTRACT NO. 68C55		
		ILLINOIS	FED. AID PROJECT	

TOP OF GIRDER WEB ELEVATION TABLE

Girder	℄ Bearing N. Abut.	℄ Field Splice 1	℄ Pier 1	℄ Field Splice 2	℄ Field Splice 3	℄ Pier 2	℄ Field Splice 4	℄ Bearing S Abut.
1	506.88	507.88	508.46	508.78	508.90	508.79	508.66	508.22
2	506.64	507.54	508.05	508.33	508.46	508.35	508.21	507.76
3	506.40	507.18	507.63	507.89	508.03	507.90	507.74	507.30
4	506.16	506.82	507.14	507.44	507.60	507.45	507.29	506.83
5	505.92	506.46	506.74	507.00	507.16	507.00	506.83	506.37
6	505.68	506.10	506.34	506.56	506.72	506.55	506.37	505.90
7	505.44	505.75	505.94	506.12	506.27	506.10	505.90	505.43
8	505.20	505.40	505.52	505.64	505.83	505.70	505.42	504.96

Top of girder web elevations are the elevations with no load applied to the girders (for fabrication use only).

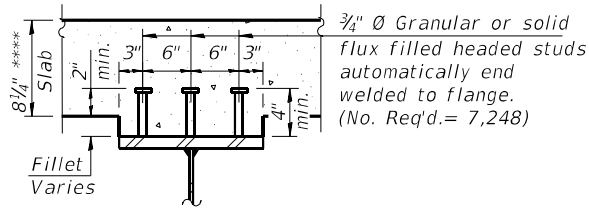
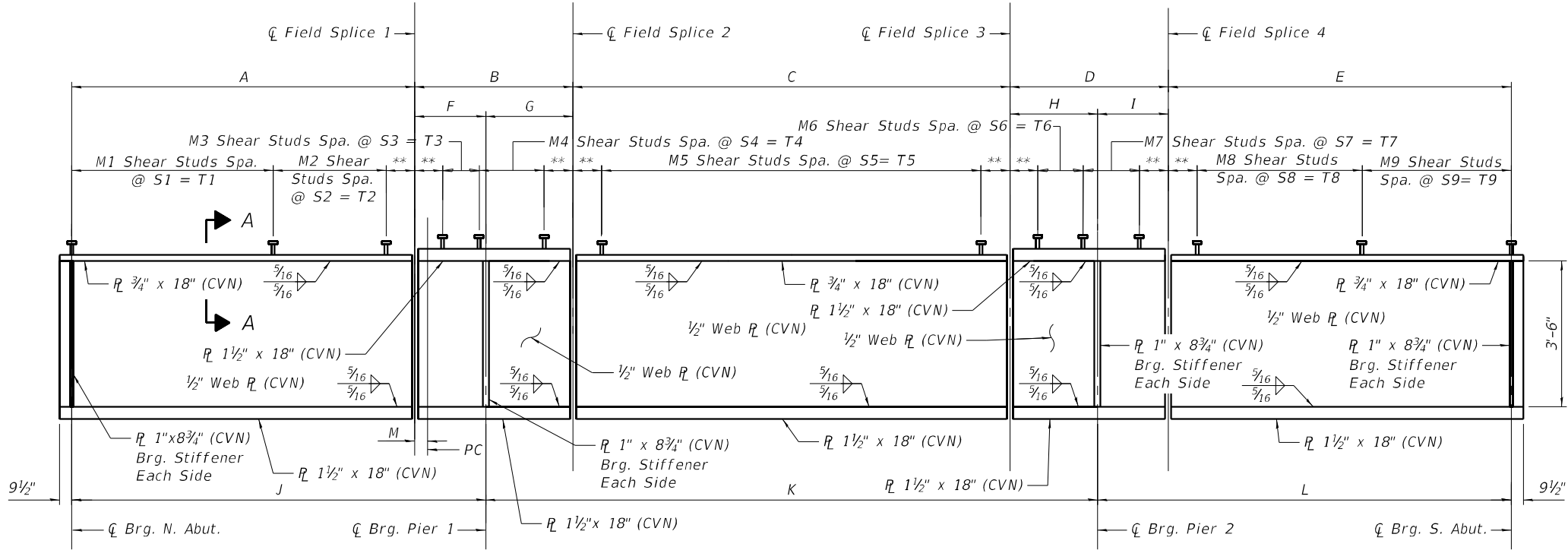
Notes:
All structural steel shall be AASHTO M270 Grade 50.

Girders have bearing stiffeners and connection plates as required by design. Additional stiffeners may be added at the Contractor's expense as necessary to prevent distortion of the girders during galvanizing. The Contractor shall coordinate with the fabricator and the galvanizer to determine if additional stiffeners are necessary, and where these should be placed. Any proposed changes shall be submitted to the Engineer for approval prior to making any changes and documented on the shop drawings.

Temporary stiffener angles shall be bolted to each side of the splice ends of each girder segment to prevent distortion during galvanizing. Temporary stiffener angles shall bolt or fit tight against top and bottom flanges and include spacer tubes to minimize damage to galvanizing during removal. Cost included with Furnishing and Erecting Structural Steel.

Curved girder segments do not meet the requirements for heat bending. Flanges shall be cut for horizontal curvature. Webs shall be cut for vertical girder camber.

For Camber and Field Splice Detail see sheet S2-26.



SECTION A-A

**** Prior to grinding

GIRDER ELEVATION

All dimensions are measured along ℄ of girder.
CVN denotes Charpy-V-Notch impact energy requirements, zone 2

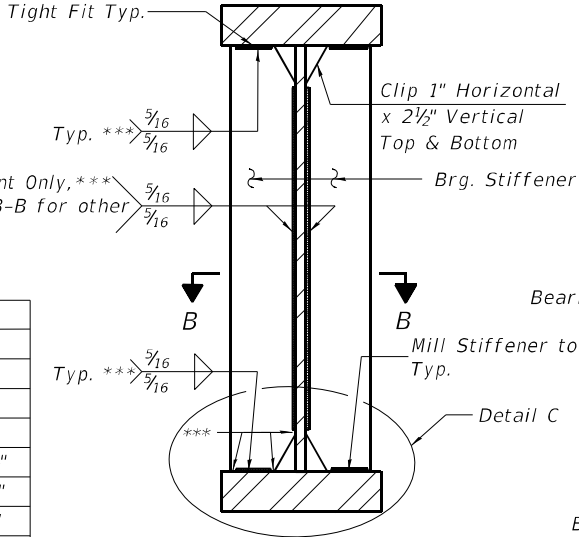
** 1'-9 1/2"

GIRDER DIMENSIONS TABLE

Girder	A	B	C	D	E	F	G	H	I	J	K	L	M
1	43'-8"	57'-7"	60'-0"	48'-9"	52'-7 1/2"	30'-3 3/16"	27'-3 7/16"	21'-7"	27'-2"	73'-11 1/16"	108'-10 1/16"	79'-9 1/2"	1/8"
2	43'-8"	55'-5"	60'-0"	49'-4"	51'-1 3/16"	28'-11 1/8"	26'-5 7/8"	22'-5"	26'-11"	72'-7 1/8"	108'-10 7/8"	78'-0 3/16"	7/8"
3	43'-6"	53'-7"	60'-0"	51'-8"	47'-7 1 1/16"	27'-8 1 1/16"	25'-10 5/16"	23'-1"	28'-7"	71'-2 1 1/16"	108'-11 5/16"	76'-2 1 3/16"	2 7/8"
4	42'-9"	51'-6"	60'-0"	51'-2"	47'-10 1/2"	27'-1 1/4"	24'-4 3/4"	24'-7"	26'-7"	69'-10 1/4"	108'-11 3/4"	74'-5 1/2"	11 7/8"
5	41'-8"	51'-3"	60'-0"	51'-3"	46'-0 1/4"	26'-9 1 3/16"	24'-5 3/16"	24'-7 1/16"	26'-7 1 5/16"	68'-5 1 3/16"	109'-0 1/4"	72'-8 3/16"	2'-0 7/8"
6	40'-4"	51'-3"	60'-0"	51'-3"	44'-3"	26'-9 3/8"	24'-5 5/8"	24'-7 3/16"	26'-7 1 3/16"	67'-1 3/8"	109'-0 1 3/16"	70'-10 1 3/16"	3'-4 7/8"
7	39'-1"	51'-3"	60'-0"	51'-5"	42'-2 1 3/16"	26'-7 1 5/16"	24'-7 1/16"	24'-6 5/16"	26'-10 1 1/16"	65'-8 1 3/16"	109'-1 3/8"	69'-1 1/2"	4'-7 7/8"
8	38'-2"	51'-0"	60'-0"	52'-10"	38'-10 3/8"	26'-2 1/2"	24'-9 1/2"	24'-4 1/2"	28'-5 1/2"	64'-4 1/2"	109'-2"	67'-4 7/8"	5'-6 7/8"

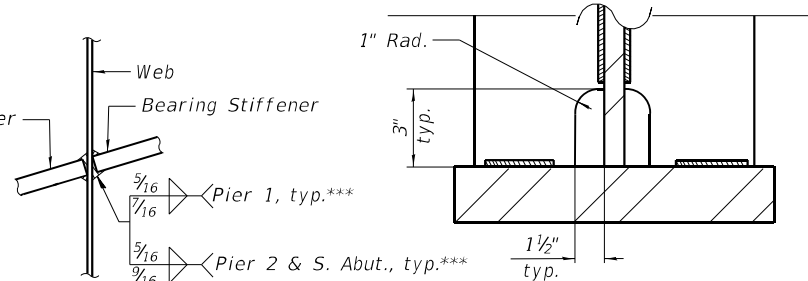
SHEAR STUD SPACING TABLE

Girder	M1	S1	T1	M2	S2	T2	M3	S3	T3	M4	S4	T4	M5	S5	T5	M6	S6	T6	M7	S7	T7	M8	S8	T8	M9	S9	T9
1	39	9	29'-3"	17	9	12'-9"	39	9	29'-3"	27	11	24'-9"	57	12	57'-0"	23	10	19'-2"	31	10	25'-10"	22	10	18'-4"	39	10	32'-6"
2	38	9	28'-6"	18	9	13'-6"	36	9	27'-0"	30	10	25'-0"	57	12	57'-0"	27	9	20'-3"	34	9	25'-6"	21	10	17'-6"	38	10	31'-8"
3	37	9	27'-9"	19	9	14'-3"	39	8	26'-0"	29	10	24'-2"	57	12	57'-0"	28	9	21'-0"	36	9	27'-0"	18	10	15'-0"	37	10	30'-10"
4	37	9	27'-9"	18	9	13'-6"	37	8	24'-8"	28	10	23'-4"	57	12	57'-0"	30	9	22'-6"	34	9	25'-6"	21	9	15'-9"	36	10	30'-0"
5	36	9	27'-0"	17	9	12'-9"	38	8	25'-4"	27	10	22'-6"	57	12	57'-0"	30	9	22'-6"	34	9	25'-6"	20	9	15'-0"	39	9	29'-3"
6	35	9	26'-3"	17	9	12'-9"	38	8	25'-4"	27	10	22'-6"	57	12	57'-0"	30	9	22'-6"	34	9	25'-6"	22	8	14'-8"	37	9	27'-9"
7	35	9	26'-3"	17	8	11'-4"	38	8	25'-4"	27	10	22'-6"	57	12	57'-0"	30	9	22'-6"	38	8	25'-4"	20	8	13'-4"	36	9	27'-0"
8	35	9	26'-3"	15	8	10'-0"	37	8	24'-8"	27	10	22'-6"	57	12	57'-0"	30	9	22'-6"	40	8	26'-8"	16	8	10'-8"	35	9	26'-3"



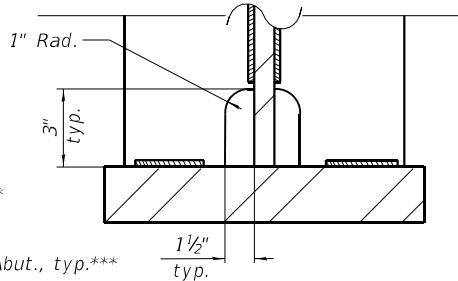
SECTION AT BEARING STIFFENERS

*** Stop welds 1/4" (± 1/8") from edges as shown. Typical.



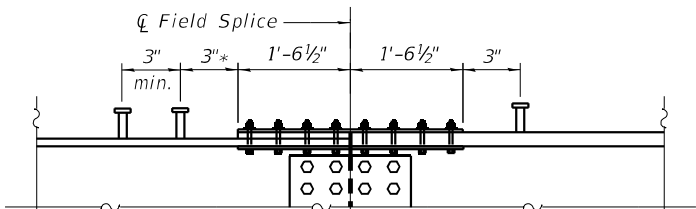
SECTION B-B

Bearing Stiffeners shall be skewed to match ℄ Bearing at Pier 1, Pier 2 and South Abutment. For skew angle see Angle B on sheet S2-28.



Detail C

Alternative Bearing Clip Detail



STUD SHEAR CONNECTOR DETAIL AT FIELD SPLICES

DO NOT place stud shear connectors on splice plates.
* If measured location places a row of studs on a splice plate or within 3" of a splice plate, move the row of studs to 3" beyond nearest edge of the splice plate but no closer than 3" to the nearest shear stud.

MODEL: Default
FILE: \\baker\pwworkspace\ben\ben\com\bus-us-pw\03\Documents\Chicago\1\01\Projectal\79740.IDOT\Cadd\Sheet\Main St_Bridge_Sheets\0900103-48C55-025-GirderElev.dgn

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INFRASTRUCTURE
ENGINEERING

USER NAME = DIamb
DESIGNED - SPK
CHECKED - PK
PLOT SCALE = 0.1667' / in.
PLOT DATE = 6/6/2023

DESIGNED - SPK
CHECKED - PK
DRAWN - MD
CHECKED - SPK

REVISED -
REVISED -
REVISED -
REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

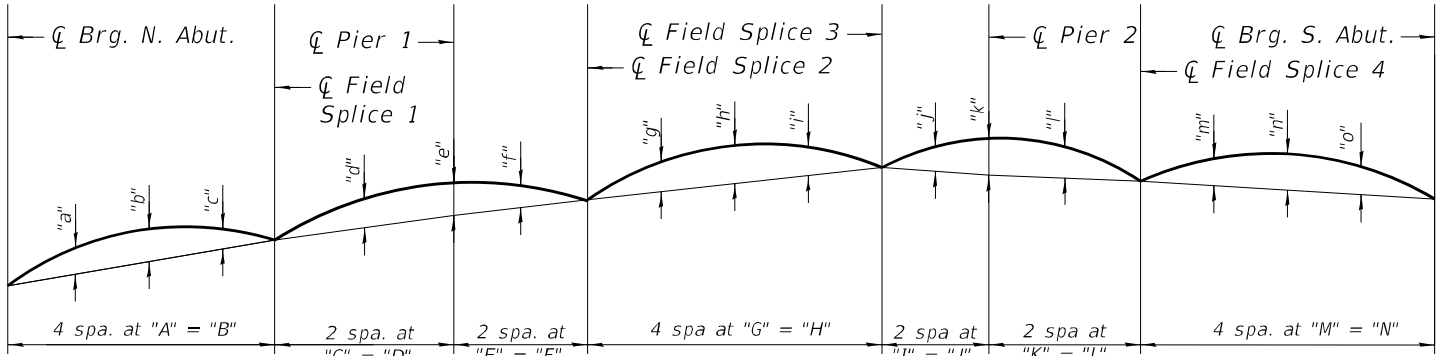
STEEL GIRDER DETAILS
STRUCTURE NO. 090-0183

SHEET S2-25 OF S2-47 SHEETS

F.A.P. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO.
VAR. 13 (HVB, HB-3) BR-1:R TAZEWEILL 1173 745
CONTRACT NO. 68C55
ILLINOIS FED. AID PROJECT

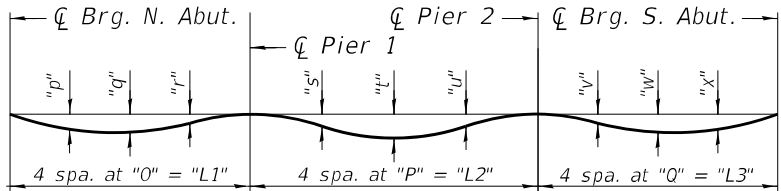
CAMBER DIMENSION TABLE

Girder	"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H"	"I"	"J"	"K"	"L"	"M"	"N"	"a"	"b"	"c"	"d"	"e"	"f"	"g"	"h"	"i"	"j"	"k"	"l"	"m"	"n"	"o"
1	10'-11"	43'-8"	± 15'-1 ³ / ₁₆ "	30'-3 ³ / ₁₆ "	± 13'-7 ¹ / ₁₆ "	27'-3 ⁷ / ₁₆ "	15'-0"	60'-0"	10'-9 ¹ / ₂ "	21'-7"	13'-7"	27'-2"	13'-1 ⁷ / ₈ "	52'-7 ¹ / ₂ "	1 ¹ / ₂ "	1"	3 ³ / ₄ "	1"	1 ¹ / ₄ "	1"	2"	2 ¹ / ₂ "	2"	0"	0"	0"	1"	1 ¹ / ₄ "	3 ³ / ₄ "
2	10'-11"	43'-8"	14'-5 ⁹ / ₁₆ "	28'-11 ¹ / ₈ "	13'-2 ⁵ / ₁₆ "	26'-5 ⁷ / ₈ "	15'-0"	60'-0"	11'-2 ¹ / ₂ "	22'-5"	13'-5 ¹ / ₂ "	26'-11"	± 12'-9 ⁵ / ₁₆ "	51'-1 ³ / ₁₆ "	3 ³ / ₄ "	1"	3 ³ / ₄ "	3 ³ / ₄ "	1 ¹ / ₄ "	1"	2"	2 ¹ / ₂ "	2"	0"	0"	0"	1"	1 ¹ / ₄ "	3 ³ / ₄ "
3	10'-10 ¹ / ₂ "	43'-6"	± 13'-10 ³ / ₁₆ "	27'-8 ¹ / ₁₆ "	± 12'-11 ³ / ₁₆ "	25'-10 ³ / ₁₆ "	15'-0"	60'-0"	± 11'-6 ⁷ / ₁₆ "	23'-0 ¹⁵ / ₁₆ "	± 14'-3 ⁹ / ₁₆ "	28'-7 ¹ / ₁₆ "	± 11'-10 ¹ / ₁₆ "	47'-7 ¹ / ₁₆ "	3 ³ / ₄ "	1"	3 ³ / ₄ "	3 ³ / ₄ "	1"	3 ³ / ₄ "	2"	2 ¹ / ₂ "	2"	0"	0"	0"	1"	1 ¹ / ₄ "	3 ³ / ₄ "
4	10'-8 ¹ / ₄ "	42'-9"	13'-6 ⁵ / ₈ "	27'-1 ¹ / ₄ "	12'-2 ³ / ₈ "	24'-4 ³ / ₄ "	15'-0"	60'-0"	12'-3 ¹ / ₂ "	24'-7"	13'-3 ¹ / ₂ "	26'-7"	11'-11 ⁵ / ₈ "	47'-10 ¹ / ₂ "	3 ³ / ₄ "	1"	3 ³ / ₄ "	0"	0"	0"	2"	2 ¹ / ₂ "	2"	0"	0"	0"	1"	1 ¹ / ₄ "	3 ³ / ₄ "
5	10'-5"	41'-8"	± 13'-4 ¹ / ₁₆ "	26'-9 ¹ / ₁₆ "	± 12'-2 ³ / ₁₆ "	24'-5 ⁷ / ₁₆ "	15'-0"	60'-0"	± 12'-3 ⁹ / ₁₆ "	24'-7 ¹ / ₁₆ "	± 13'-3 ⁵ / ₁₆ "	26'-7 ¹ / ₁₆ "	11'-6 ¹ / ₁₆ "	46'-0 ¹ / ₄ "	1 ¹ / ₂ "	1"	3 ³ / ₄ "	0"	0"	0"	2"	2 ¹ / ₂ "	2"	0"	0"	0"	1"	1 ¹ / ₄ "	3 ³ / ₄ "
6	10'-1"	40'-4"	13'-4 ¹ / ₁₆ "	26'- 9 ³ / ₈ "	12'-2 ¹ / ₁₆ "	24'-5 ³ / ₈ "	15'-0"	60'-0"	± 12'-3 ⁹ / ₁₆ "	24'-7 ¹ / ₁₆ "	± 13'-3 ¹ / ₁₆ "	26'-7 ¹ / ₁₆ "	11'-0 ³ / ₄ "	44'-3"	1 ¹ / ₂ "	1"	3 ³ / ₄ "	0"	0"	0"	2"	2 ¹ / ₂ "	2"	0"	0"	0"	1"	1 ¹ / ₄ "	3 ³ / ₄ "
7	9'-9 ¹ / ₄ "	39'-1"	± 13'-3 ¹ / ₁₆ "	26'-7 ¹ / ₁₆ "	± 12'-3 ⁹ / ₁₆ "	24'-7 ¹ / ₁₆ "	15'-0"	60'-0"	± 12'-3 ⁹ / ₁₆ "	24'-6 ⁵ / ₁₆ "	± 13'-5 ⁷ / ₁₆ "	26'-10 ¹ / ₁₆ "	± 10'-6 ¹ / ₁₆ "	42'-2 ¹ / ₁₆ "	1 ¹ / ₂ "	1"	3 ³ / ₄ "	0"	0"	0"	2"	2 ¹ / ₂ "	2"	0"	0"	0"	3 ³ / ₄ "	1"	3 ³ / ₄ "
8	9'-6 ¹ / ₂ "	38'-2"	13'-1 ¹ / ₄ "	26'-2 ¹ / ₂ "	12'-4 ³ / ₄ "	24'-9 ¹ / ₂ "	15'-0"	60'-0"	12'-2 ¹ / ₄ "	24'-4 ¹ / ₂ "	14'-2 ³ / ₄ "	28'-5 ¹ / ₂ "	± 9'-8 ¹ / ₁₆ "	38'-10 ⁵ / ₈ "	1 ¹ / ₂ "	1"	3 ³ / ₄ "	0"	0"	0"	2 ¹ / ₄ "	2 ³ / ₄ "	2 ¹ / ₄ "	1 ¹ / ₂ "	3 ³ / ₄ "	1 ¹ / ₂ "	3 ³ / ₄ "	1"	1 ¹ / ₂ "



CAMBER DIAGRAM

(Camber includes correction for vertical curve)
Upward Camber is postive

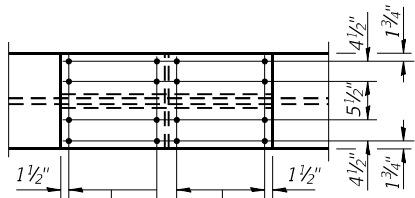


STEEL SELF-WEIGHT DEFLECTION DIAGRAM

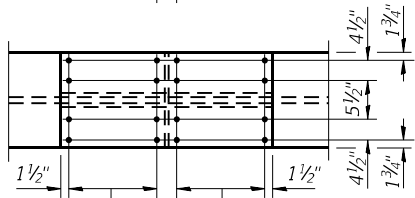
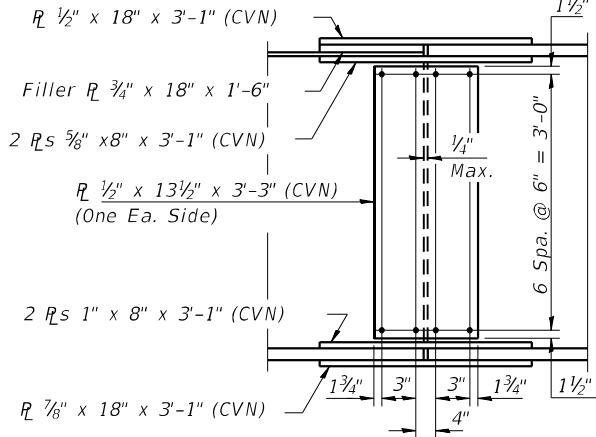
The calculated deflections of the primary girders under steel self-weight shall be used to detail the cross frame connections, and to erect the structural steel such that the girders will be plumb within a tolerance of ±1/8" per vertical foot throughout when supporting their own weight.

STEEL SELF-WEIGHT DEFLECTION TABLE

Girder	"L1"	"L2"	"L3"	"O"	"P"	"Q"	"p"	"q"	"r"	"s"	"t"	"u"	"v"	"w"	"x"
1	73'-11 ⁹ / ₁₆ "	108'-10 ⁷ / ₁₆ "	79'-9 ¹ / ₂ "	±18'-5 ⁷ / ₈ "	±27'-2 ⁵ / ₈ "	19'-11 ³ / ₈ "	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "	0"	1 ¹ / ₄ "	3 ³ / ₈ "	3 ³ / ₁₆ "	0"	1 ¹ / ₈ "	1 ¹ / ₈ "
2	72'-7 ⁷ / ₈ "	108'-10 ⁷ / ₈ "	78'-0 ³ / ₁₆ "	±18'-1 ¹ / ₁₆ "	±27'-2 ¹ / ₁₆ "	±19'-6 ¹ / ₁₆ "	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "	0"	3 ¹ / ₁₆ "	3 ³ / ₈ "	3 ³ / ₁₆ "	0"	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "
3	71'-2 ¹ / ₁₆ "	108'-11 ⁵ / ₁₆ "	76'- 2 ¹ / ₁₆ "	±17'-9 ¹ / ₁₆ "	±27'-2 ¹ / ₁₆ "	±19'-0 ¹ / ₁₆ "	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "	0"	3 ¹ / ₁₆ "	3 ³ / ₈ "	3 ³ / ₁₆ "	0"	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "
4	69'-10 ¹ / ₄ "	108'-11 ³ / ₄ "	74'-5 ¹ / ₂ "	17'-5 ⁷ / ₁₆ "	27'-2 ¹ / ₁₆ "	18'-7 ³ / ₈ "	1 ¹ / ₁₆ "	0"	0"	3 ¹ / ₁₆ "	3 ³ / ₈ "	1 ¹ / ₄ "	0"	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "
5	68'-5 ¹ / ₁₆ "	109'-0 ¹ / ₄ "	72'- 8 ³ / ₁₆ "	±17'-1 ⁷ / ₁₆ "	27'-3 ¹ / ₁₆ "	±18'-2 ¹ / ₁₆ "	0"	0"	0"	1 ¹ / ₄ "	3 ³ / ₈ "	1 ¹ / ₄ "	0"	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "
6	67'-1 ³ / ₈ "	109'-0 ¹ / ₁₆ "	70'-10 ¹ / ₁₆ "	±16'-9 ⁵ / ₁₆ "	±27'-3 ³ / ₁₆ "	±17'-8 ¹ / ₁₆ "	0"	0"	0"	3 ¹ / ₁₆ "	3 ³ / ₈ "	1 ¹ / ₄ "	0"	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "
7	65'-8 ¹ / ₁₆ "	109'-1 ³ / ₈ "	69'- 1 ¹ / ₂ "	±16'-5 ¹ / ₄ "	±27'-3 ³ / ₁₆ "	17'-3 ³ / ₈ "	0"	0"	0"	3 ¹ / ₁₆ "	3 ³ / ₈ "	3 ¹ / ₁₆ "	0"	0"	1 ¹ / ₁₆ "
8	64'-4 ¹ / ₂ "	109'-2"	67'-4 ¹ / ₈ "	16'-1 ¹ / ₈ "	27'-3 ¹ / ₂ "	±16'-10 ¹ / ₁₆ "	0"	0"	0"	3 ¹ / ₁₆ "	3 ¹ / ₁₆ "	3 ¹ / ₁₆ "	0"	0"	0"



5 Spaces @ 3" = 1'-3"



5 Spaces @ 3" = 1'-3"

FIELD SPLICE DETAIL

All structural steel shall be AASHTO M270 Grade 50.

(CVN denotes Charpy-V-Notch impact energy requirements, zone 2)

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INFRASTRUCTURE
ENGINEERING

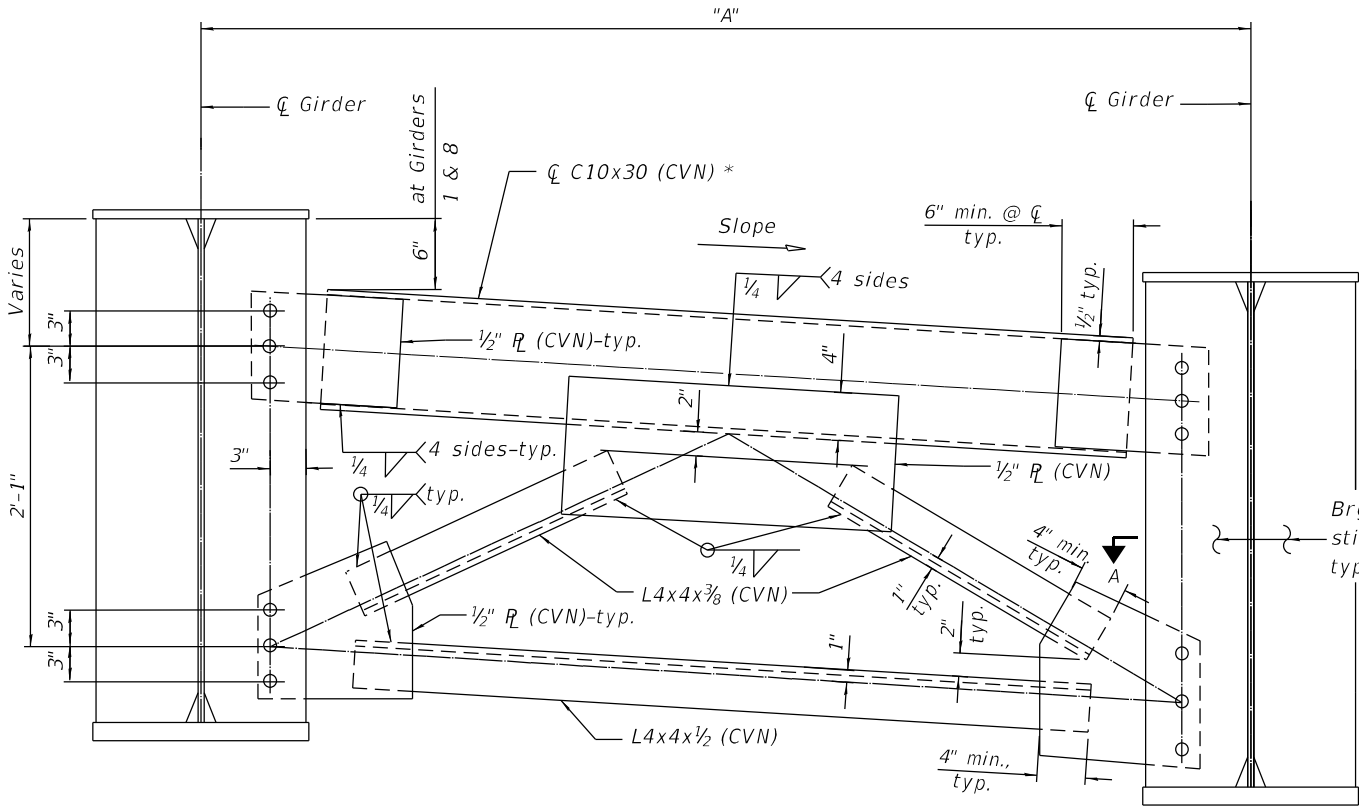
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	CHECKED - PK	REVISED -
PLOT SCALE = 0.1667 ' / in.	DRAWN - CLL	REVISED -
PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

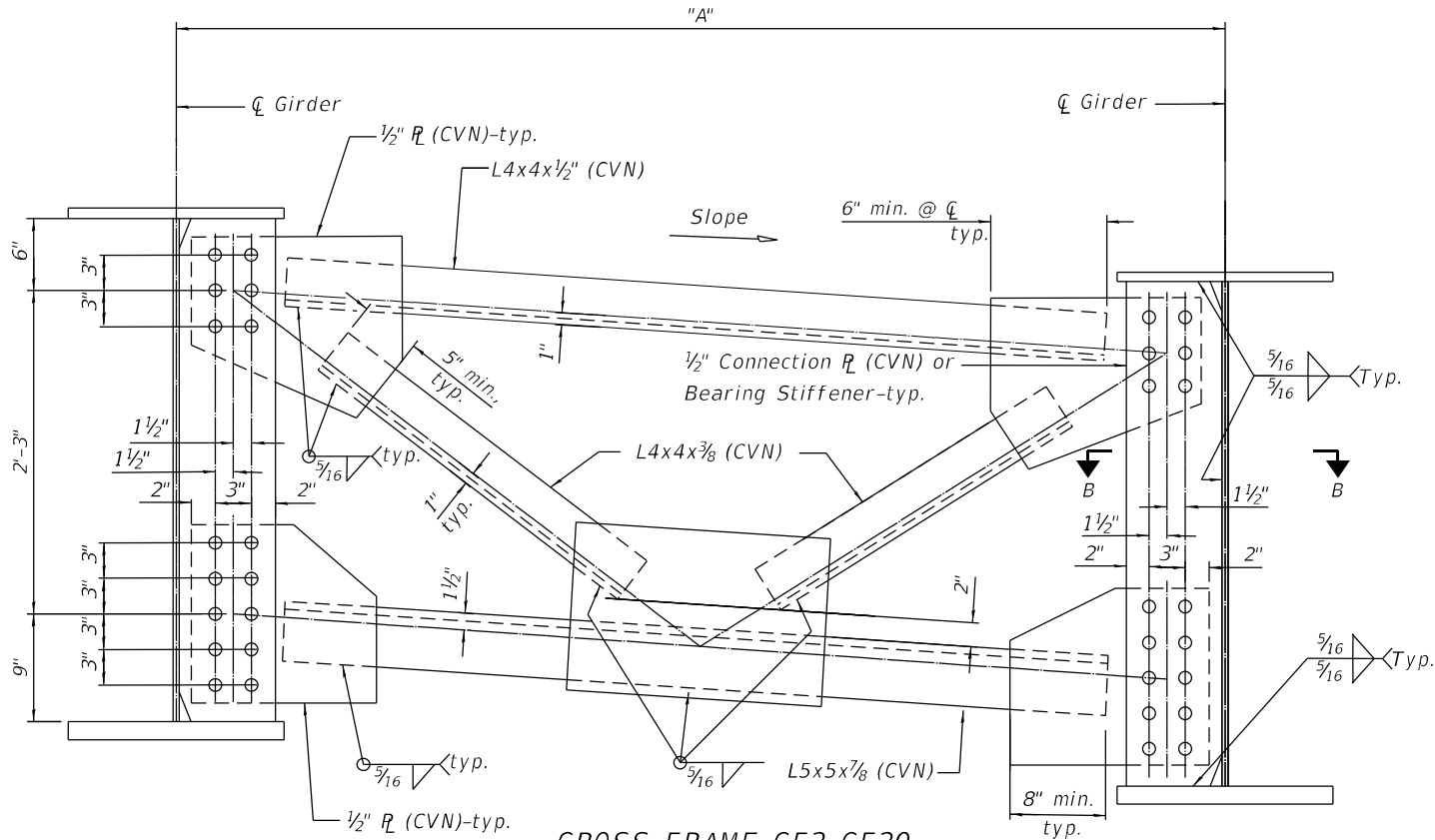
STEEL GIRDER CAMBER AND FIELD SPLICE
STRUCTURE NO. 090-0183

SHEET S2-26 OF S2-47 SHEETS

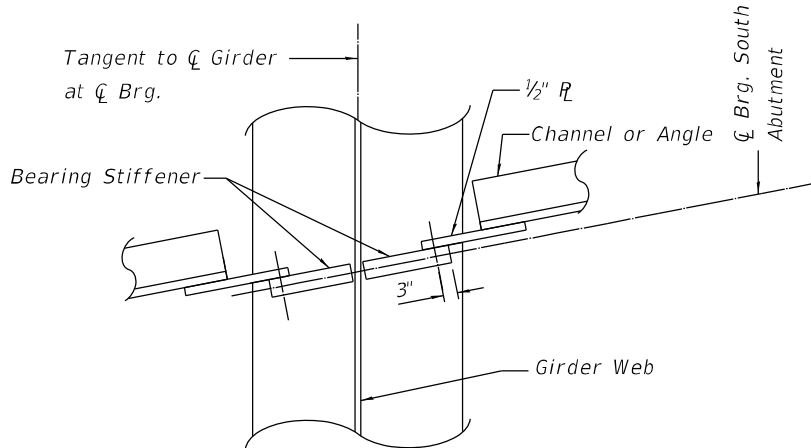
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	746
			CONTRACT NO. 68C55	
		ILLINOIS	FED. AID PROJECT	



CROSS-FRAME CF1 & CF21-CF27

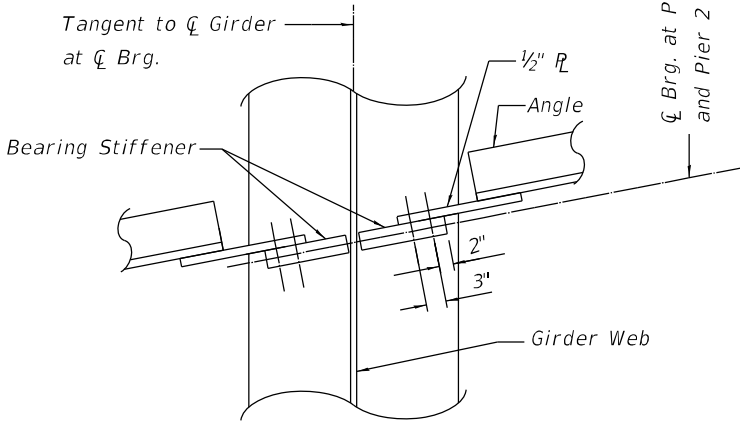


CROSS-FRAME CF2-CF20



SECTION A-A

Typical at $\varnothing$ Brg. South Abutment. For skew angle see angle "B" on sheet S2-28



SECTION B-B

Typical at $\varnothing$ Brg. Pier 1, and Pier 2. For skew angle see angle "B" on sheet S2-28

CROSS-FRAME SPACING

Girder	A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S
1	15'-3"	15'-3"	15'-3"	15'-3"	12'-11 ⁹ / ₁₆ "	14'-7"	15'-4"	13'-10"	13'-10"	13'-10"	13'-10"	17'-4"	6'-3 ⁷ / ₁₆ "	17'-4"	14'-0"	14'-0"	14'-0"	14'-0"	6'-5 ¹ / ₂ "
2	15'-3"	15'-3"	15'-2 ¹ / ₂ "	14'-11"	11'-11 ⁵ / ₈ "	14'-11 ¹ / ₁₆ "	15'-0"	13'-6 ³ / ₈ "	13'-6 ³ / ₈ "	13'-6 ³ / ₈ "	13'-6 ³ / ₈ "	16'-11 ⁷ / ₁₆ "	7'-10 ¹ / ₄ "	15'-3"	13'-8 ⁵ / ₁₆ "	13'-8 ⁵ / ₁₆ "	13'-8 ⁵ / ₁₆ "	13'-8 ⁵ / ₁₆ "	7'-11 ⁵ / ₁₆ "
3	15'-3"	15'-3"	15'-1 ⁵ / ₁₆ "	14'-7"	10'-11 ³ / ₄ "	15'-4 ³ / ₈ "	14'-7 ¹ / ₁₆ "	13'-2 ³ / ₄ "	13'-2 ³ / ₄ "	13'-2 ³ / ₄ "	13'-2 ³ / ₄ "	16'-6 ⁷ / ₈ "	9'-5 ¹ / ₈ "	13'-2"	13'-4 ⁵ / ₈ "	13'-4 ⁵ / ₈ "	13'-4 ⁵ / ₈ "	13'-4 ⁵ / ₈ "	9'-6 ⁵ / ₁₆ "
4	15'-3"	15'-3"	15'-1 ⁷ / ₁₆ "	14'-3"	9'-11 ¹ / ₁₆ "	15'-9"	14'-3 ¹ / ₁₆ "	12'-11 ¹ / ₈ "	12'-11 ¹ / ₈ "	12'-11 ¹ / ₈ "	12'-11 ¹ / ₈ "	16'-2 ³ / ₈ "	10'-11 ¹ / ₁₆ "	11'-0 ¹ / ₁₆ "	13'-1"	13'-1"	13'-1"	13'-1"	11'-0 ¹ / ₁₆ "
5	15'-3"	15'-3"	15'-0 ³ / ₈ "	13'-11"	8'-11 ¹ / ₁₆ "	16'-11 ¹ / ₁₆ "	13'-11 ¹ / ₈ "	12'-7 ¹ / ₂ "	12'-7 ¹ / ₂ "	12'-7 ¹ / ₂ "	12'-7 ¹ / ₂ "	15'-9 ¹ / ₁₆ "	12'-6 ⁷ / ₈ "	8'-11 ³ / ₄ "	12'-9 ⁵ / ₁₆ "	12'-9 ⁵ / ₁₆ "	12'-9 ⁵ / ₁₆ "	12'-9 ⁵ / ₁₆ "	12'-7 ³ / ₁₆ "
6	15'-3"	15'-3"	15'-0 ³ / ₈ "	13'-7"	8'-0"	16'-6 ³ / ₈ "	13'-7 ⁷ / ₈ "	12'-3 ⁷ / ₈ "	12'-3 ⁷ / ₈ "	12'-3 ⁷ / ₈ "	12'-3 ⁷ / ₈ "	15'-5 ¹ / ₄ "	14'-1 ¹ / ₁₆ "	6'-10 ⁵ / ₈ "	12'-5 ⁵ / ₈ "	12'-5 ⁵ / ₈ "	12'-5 ⁵ / ₈ "	12'-5 ⁵ / ₈ "	14'-1 ¹ / ₁₆ "
7	15'-3"	15'-3"	14'-11 ¹ / ₁₆ "	13'-3"	7'-0 ¹ / ₈ "	16'-11 ¹ / ₁₆ "	13'-3 ¹ / ₁₆ "	12'-0 ¹ / ₄ "	12'-0 ¹ / ₄ "	12'-0 ¹ / ₄ "	12'-0 ¹ / ₄ "	15'-0 ¹ / ₁₆ "	15'-8 ¹ / ₁₆ "	4'-9 ¹ / ₂ "	12'-1 ⁵ / ₁₆ "	12'-1 ⁵ / ₁₆ "	12'-1 ⁵ / ₁₆ "	12'-1 ⁵ / ₁₆ "	15'-8 ¹ / ₄ "
8	15'-3"	15'-3"	14'-11 ¹ / ₁₆ "	12'-11"	6'-0 ³ / ₁₆ "	17'-3 ³ / ₄ "	12'-11 ¹ / ₁₆ "	11'-8 ⁵ / ₈ "	11'-8 ⁵ / ₈ "	11'-8 ⁵ / ₈ "	11'-8 ⁵ / ₈ "	14'-8 ¹ / ₈ "	17'-3 ¹ / ₁₆ "	2'-8 ³ / ₁₆ "	11'-10 ⁵ / ₁₆ "	11'-10 ⁵ / ₁₆ "	11'-10 ⁵ / ₁₆ "	11'-10 ⁵ / ₁₆ "	17'-2 ¹ / ₁₆ "

CROSS-FRAME
DIMENSION "A" AND
SLOPE TABLE

Name	"A"	Slope	Quantity
CF1	7'-3 ¹ / ₂ "	3.29%	7
CF2	7'-3 ¹ / ₂ "	3.81%	7
CF3	7'-3 ¹ / ₂ "	4.34%	7
CF4	7'-3 ¹ / ₂ "	4.86%	7
CF5	7'-3 ¹ / ₂ "	5.35%	7
CF6	7'-3 ¹ / ₂ "	6.00%	84
CF7	7'-4 ¹ / ₁₆ "	5.74%	1
CF8	7'-4 ¹ / ₁₆ "	5.74%	1
CF9	7'-4"	5.74%	1
CF10	7'-4"	5.74%	1
CF11	7'-3 ⁵ / ₁₆ "	5.74%	1
CF12	7'-3 ⁵ / ₁₆ "	5.74%	1
CF13	7'-3 ⁵ / ₁₆ "	5.74%	1
CF14	7'-6 ³ / ₄ "	5.90%	1
CF15	7'-6 ³ / ₁₆ "	5.90%	1
CF16	7'-6 ⁷ / ₁₆ "	5.90%	1
CF17	7'-6 ⁷ / ₁₆ "	5.90%	1
CF18	7'-6 ¹ / ₈ "	5.90%	1
CF19	7'-6 ¹ / ₁₆ "	5.90%	1
CF20	7'-5 ¹ / ₁₆ "	5.90%	1
CF21	7'-6 ³ / ₈ "	6.16%	1
CF22	7'-6 ¹ / ₂ "	6.16%	1
CF23	7'-6 ³ / ₁₆ "	6.16%	1
CF24	7'-6 ⁷ / ₁₆ "	6.16%	1
CF25	7'-6 ¹ / ₁₆ "	6.16%	1
CF26	7'-5 ¹ / ₁₆ "	6.16%	1
CF27	7'-5 ¹ / ₁₆ "	6.16%	1

Notes:

All structural steel shall be AASHTO M270 Grade 50.

CVN denotes Charpy-V-Notch impact energy requirements, zone 2.

Dimension "A" measured along $\varnothing$ of Cross Frame

Bearing stiffeners and connection plates have the same weld limits and clip details. See Section at Bearing Stiffeners on sheet S2-25.

* Alternate channels of equal depth and larger weight are permitted to facilitate material acquisition. Alternate channels, if utilized, shall be provided at no additional cost to the Department.

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USER NAME = DIamb	DESIGNED - SPK	REVISED -
PLOT SCALE = 0:2.0000 " = 1" / in.	CHECKED - PK	REVISED -
PLOT DATE = 6/6/2023	DRAWN - CLL	REVISED -
	CHECKED - SPK	REVISED -

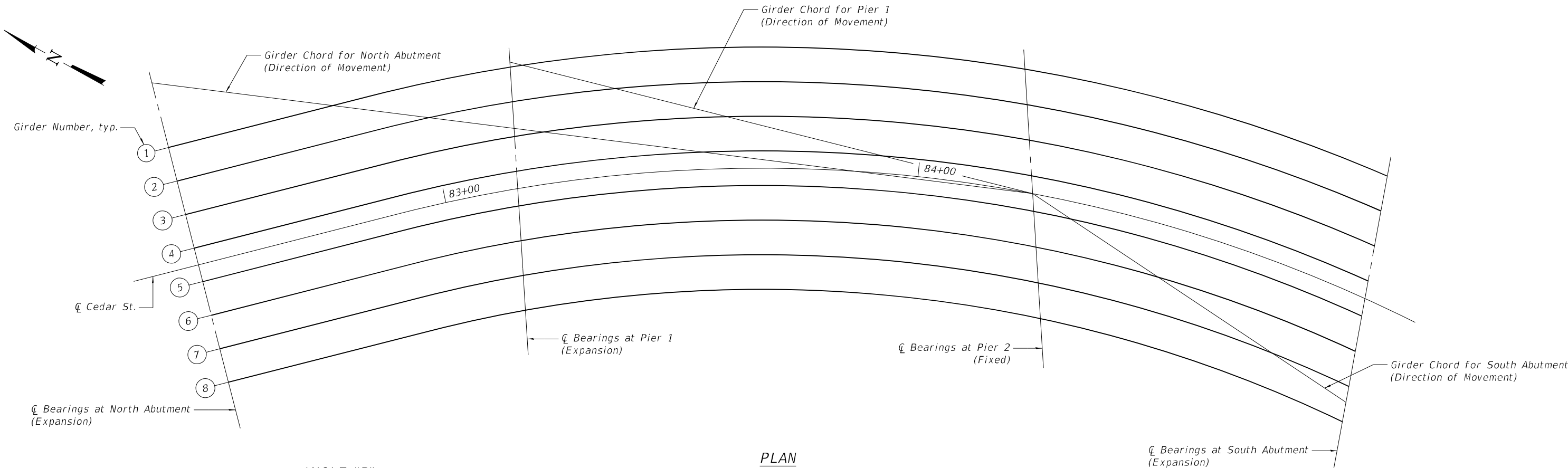
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CROSS FRAMES
STRUCTURE NO. 090-0183

SHEET S2-27 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	747
		CONTRACT NO. 68C55		
		ILLINOIS / FED. AID PROJECT		

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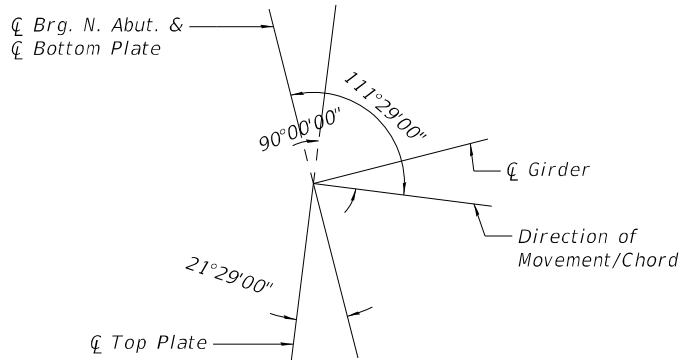


ANGLE "B"

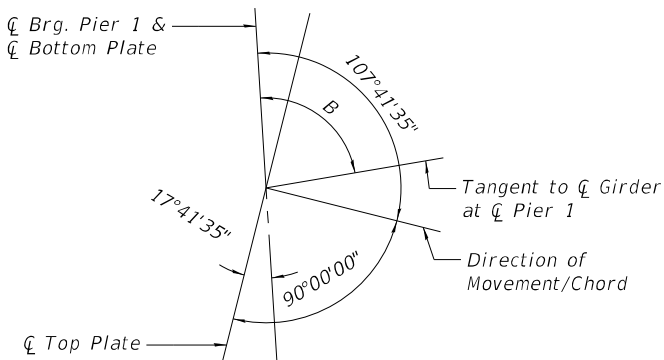
Girder	Pier 1	Pier 2	S. Abutment
1	84°27'12"	103°09'24"	102°54'34"
2	84°19'44"	103°27'22"	103°12'11"
3	84°11'56"	103°46'11"	103°30'38"
4	84°03'45"	104°05'54"	103°49'59"
5	83°55'12"	104°26'36"	104°10'16"
6	83°46'12"	104°48'22"	104°31'36"
7	83°36'45"	105°11'15"	104°54'02"
8	83°26'49"	105°35'22"	105°17'40"

NOTES:

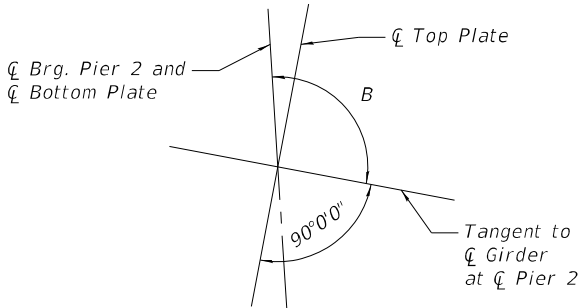
Set Top Bearing Plate at right angle to the Direction of Movement/Chord.



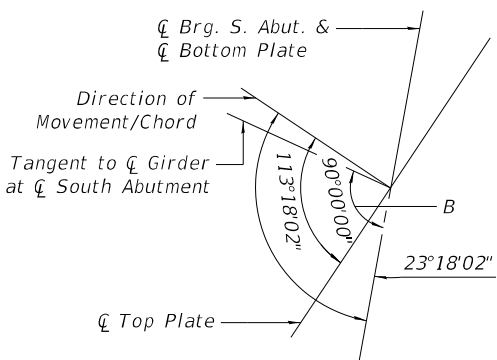
Station Increases
NORTH ABUTMENT



Station Increases
PIER 1



Station Increases
PIER 2



Station Increases
SOUTH ABUTMENT

BEARING ORIENTATION

Michael Baker

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INFRASTRUCTURE
ENGINEERING

USER NAME = DLamb

DESIGNED - SPK

REVISD -

CHECKED - PK

REVISD -

PLOT SCALE = 21.3333 ' / in.

DRAWN - MD

REVISD -

PLOT DATE = 6/6/2023

CHECKED - PK

REVISD -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEARING LAYOUT & ORIENTATION
STRUCTURE NO. 090-0183

SHEET S2-28 OF S2-47 SHEETS

F.A.P. RTE.

SECTION

COUNTY

TOTAL SHEETS

SHEET NO.

VAR.

13 (HVB, HB-3) BR-1;R

TAZEWELL

1173

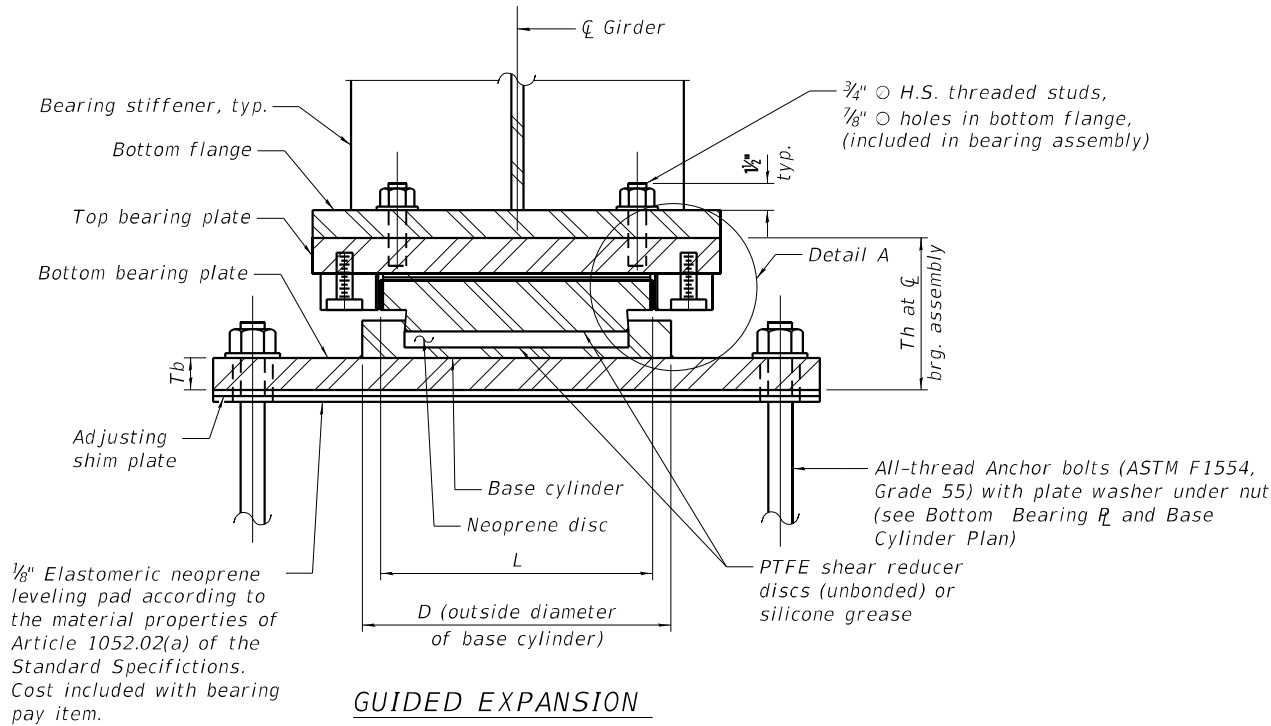
748

CONTRACT NO. 68C55

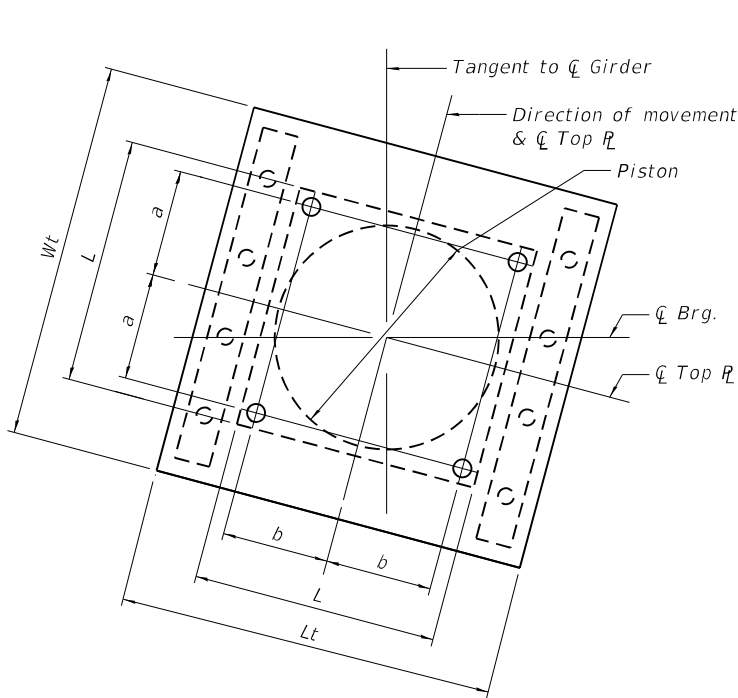
ILLINOIS

FED. AID PROJECT

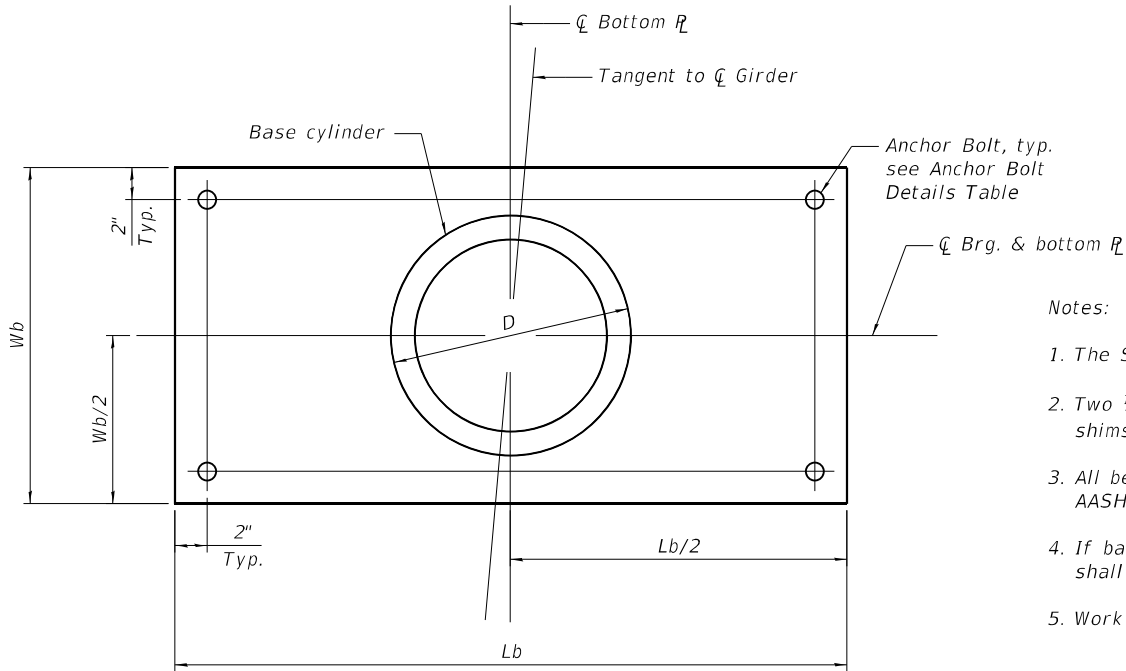
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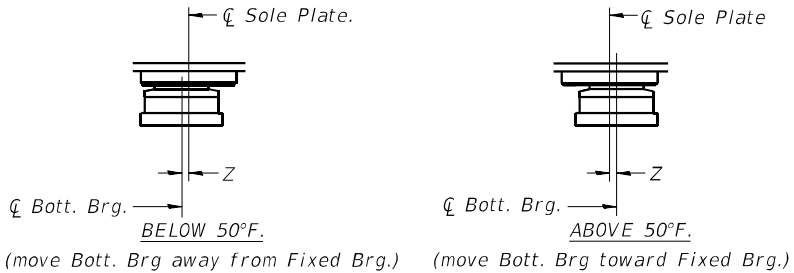
GUIDED EXPANSION
HLMR BEARING



TOP BEARING PLATE AND
PISTON PLAN

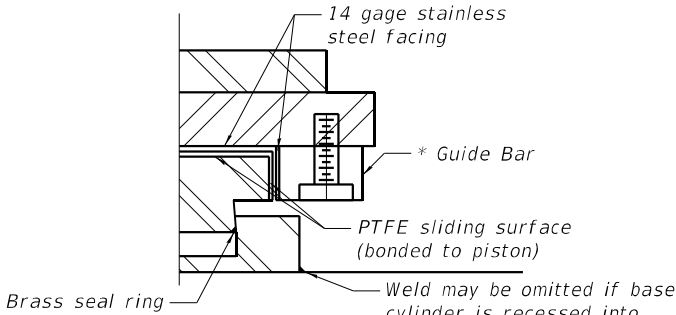


BOTTOM BEARING PLATE AND
BASE CYLINDER PLAN



SETTING ANCHOR BOLTS AT EXP. BRG.

Z = 1/8" per 100' of expansion for every 15° temp. change from the normal temp. of 50° F.



DETAIL A

* As alternates to the bolted connection shown, the guide bars may be connected to the top bearing plate by groove welds or the guide bars and top bearing plate may be fabricated as a single piece.

DIMENSIONS (IN)

Member	Dimension	N. Abut.	Pier 1	S. Abut.
Top Bearing Plate	Wt	13	14½	11½
	Lt	18	18	18
	Tt	1½	2	1½
Bearing	D	7½	12½	8
	L	8	12½	8½
Bottom Bearing Plate	Wb	12	17	12
	Lb	30	28	30
	Tb	2	2	2
Bearing Assembly	Th	6¾	9⅝	7⅝
H.S. Stud	a	4½	5¼	3¾
	b	5¾	5¼	3¼

DESIGN DATA

Data	N. Abut.	Pier 1	S. Abut.
Service Vertical Design Load (kips)	98	286	115
Pay Item Size (kips)	100	300	150
Factored Horizontal Design Load (kips)	21	67	33
Factored Design Rotation (rad)	0.023	0.033	0.026
Total Required Longitudinal Movement (in)	2.76	1.65	1.20

BILL OF MATERIAL

Item	Unit	Total
High Load Multi-Rotational Bearings, Pot, Guided Expansion-100K	Each	8
High Load Multi-Rotational Bearings, Pot, Guided Expansion-200K	Each	8
High Load Multi-Rotational Bearings, Pot, Guided Expansion-300K	Each	8
Anchor Bolts, ¾"	Each	64
Anchor Bolts, 1"	Each	32

ANCHOR BOLT DETAILS TABLE

Location	Bolt Ø x Length **	Plate Washer	Base Plate Hole Ø
N. Abut. & S. Abut.	¾" x 12"	2" x 2" x ⅝"	1¼"
Pier 1	1" x 12"	2¼" x 2¼" x ⅝"	1½"

**Length shown is minimum required embedment length

Notes:

- The Structural Steel for the top and bottom bearing plates shall be AASHTO M270 Grade 50.
- Two 1/8" adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on the bearing details. Cost included with bearing pay item.
- All bearing plates, H.S. studs, anchor bolts, nuts and washers shall be galvanized according to AASHTO M111 or M232 as applicable.
- If base cylinder is recessed into the bottom bearing plate, the thickness of the bottom plate shall be Tb plus the depth of the recess.
- Work this sheet with Sheet S2-27 for bearing orientations.
- The Contractor shall determine final dimensions of all bearing components including bottom plate, top plate, threaded studs, bolted flange connections, anchor rods, and overall height. The contractor shall be responsible for verifying bearing heights and adjusting concrete pedestals, if required.



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	CHECKED - SPK	REVISED -
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PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

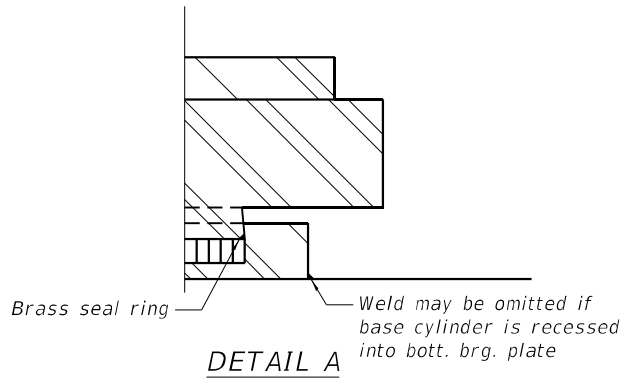
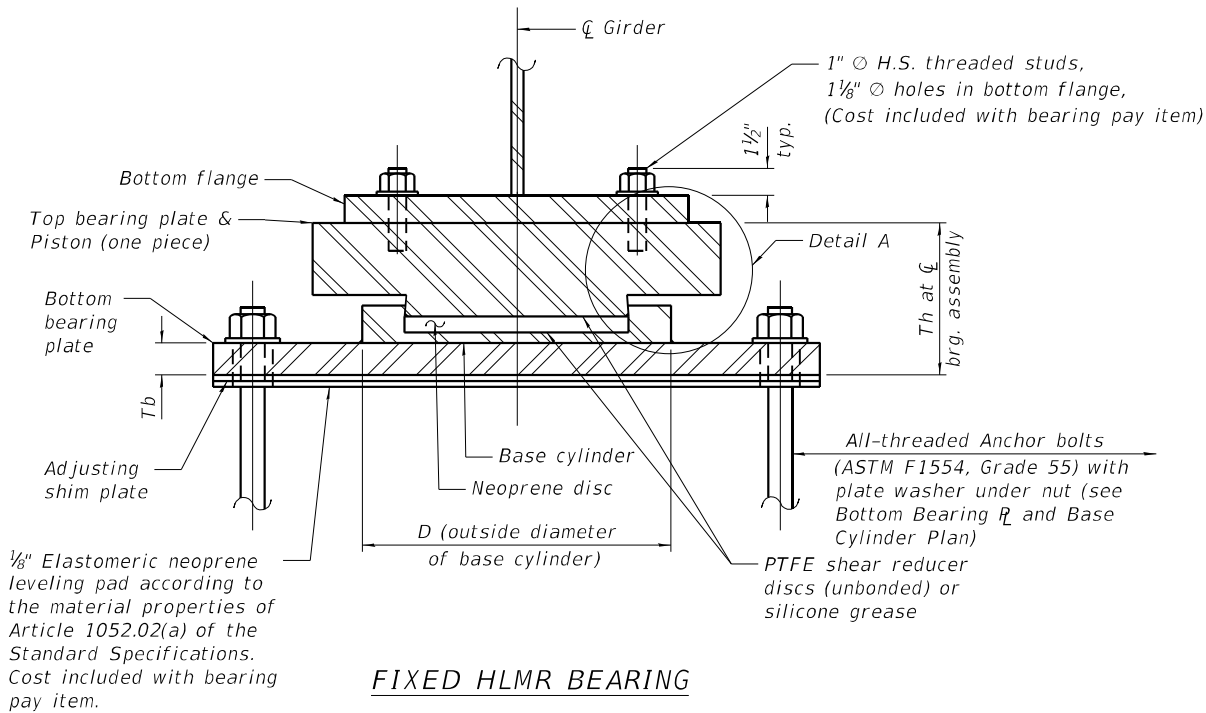
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEARINGS - ABUTMENTS AND PIER 1
STRUCTURE NO. 090-0183

SHEET S2-29 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	749
				CONTRACT NO. 68C55
				ILLINOIS FED. AID PROJECT

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DIMENSIONS (IN)

Member	Dimension	Pier 2
Top Bearing Plate	Wt	12½
	Lt	18
	Tt	2
Bearing	D	12½
Bottom Bearing Plate	Wb	14½
	Lb	31
	Tb	2¼
Bearing Assembly	Th	6⅞
H.S. Stud	a	4¼
	b	7

DESIGN DATA

Data	Pier 2
Service Vertical Design Load (kips)	302
Pay Item Size (kips)	350
Factored Horizontal Design Load (kips)	115
Factored Design Rotation (rad)	0.016

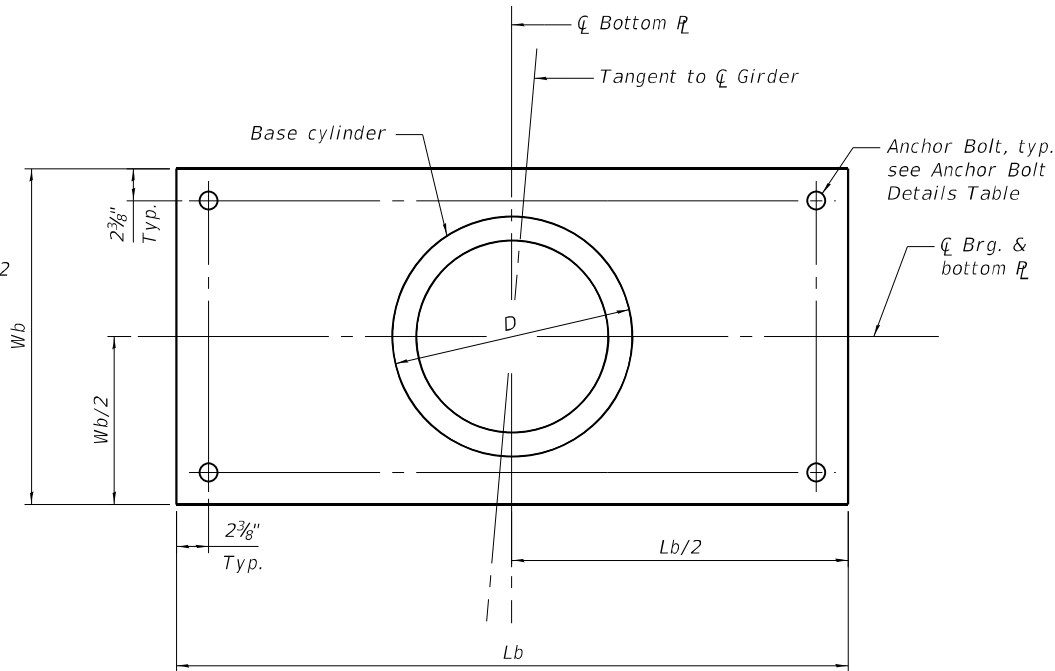
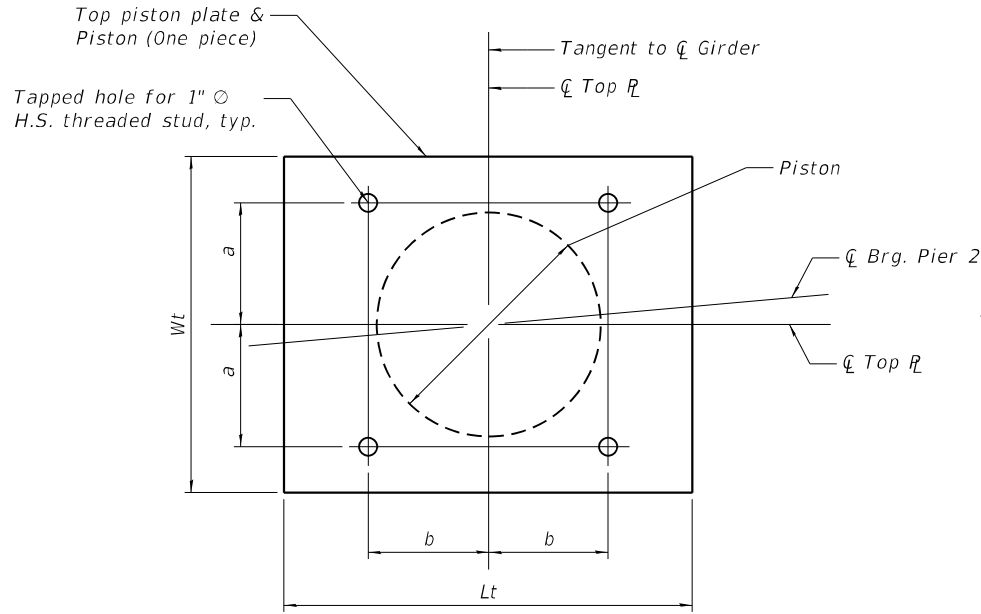
BILL OF MATERIAL

Item	Unit	Total
High Load Multi-Rotational Bearings, Pot, Fixed-400K	Each	8
Anchor Bolts, 1 1/4"	Each	32

ANCHOR BOLT DETAILS TABLE

Location	Bolt Ø x Length **	Plate Washer	Base Plate Hole Ø
Pier 2	1¼" x 15"	2¾" x 2¾" x ⅝"	1¾"

**Length shown is minimum required embedment length



NOTES:

- The Structural Steel for the top and bottom bearing plates shall be AASHTO M270 Grade 50.
- Two ⅛" adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on the bearing details. Cost included with bearing pay item.
- All bearing plates, H.S. studs, anchor bolts, nuts and washers shall be galvanized according to AASHTO M111 or M232 as applicable.
- If base cylinder is recessed into the bottom bearing plate, the thickness of the bottom plate shall be Tb plus the depth of the recess.
- Work this sheet with Sheet S2-27 for bearing orientations.
- The Contractor shall determine final dimensions of all bearing components including bottom plate, top plate, threaded studs, bolted flange connections, anchor rods, and overall height. The contractor shall be responsible for verifying bearing heights and adjusting concrete pedestals, if required.



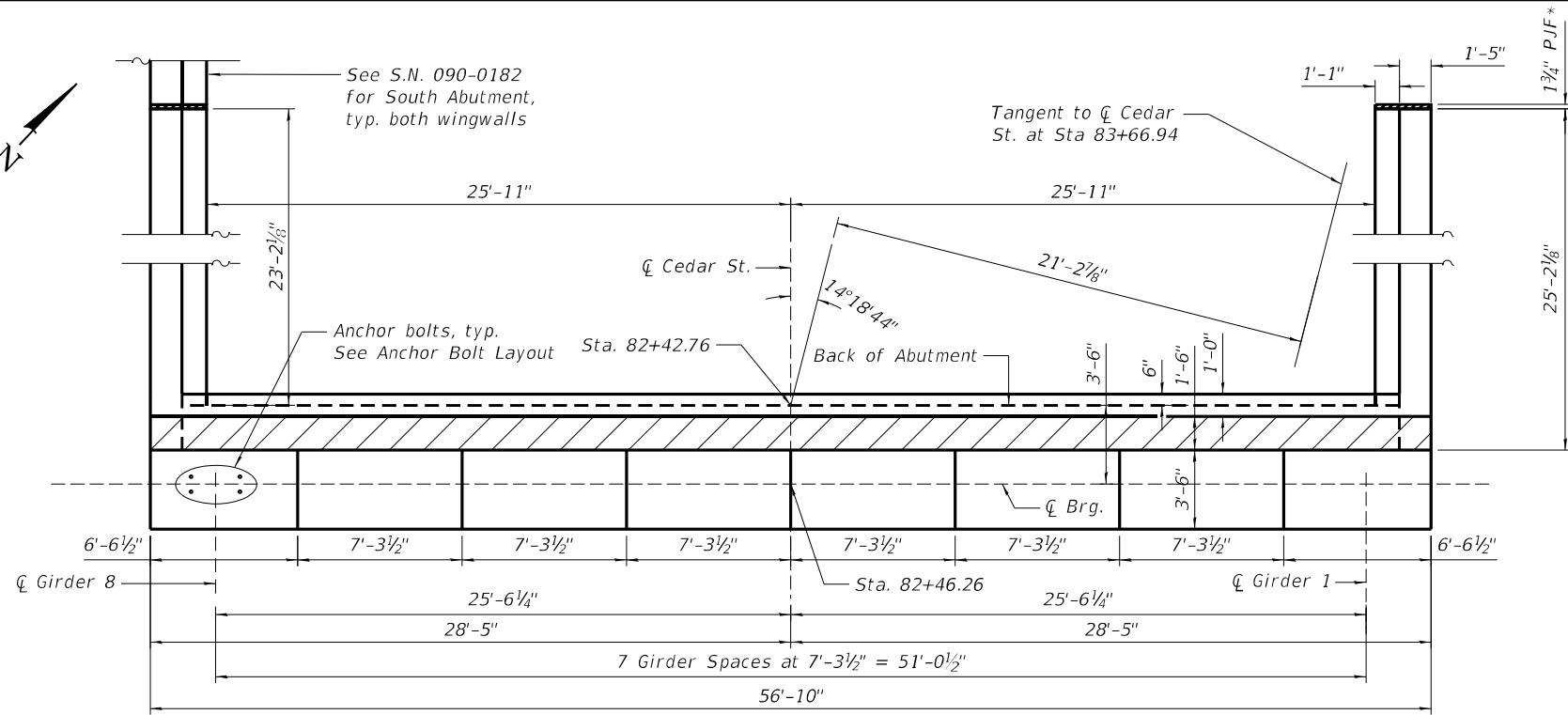
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

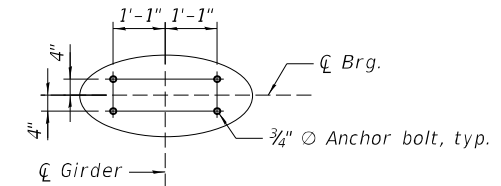
BEARINGS – PIER 2
STRUCTURE NO. 090–0183

SHEET S2-30 OF S2-47 SHEETS

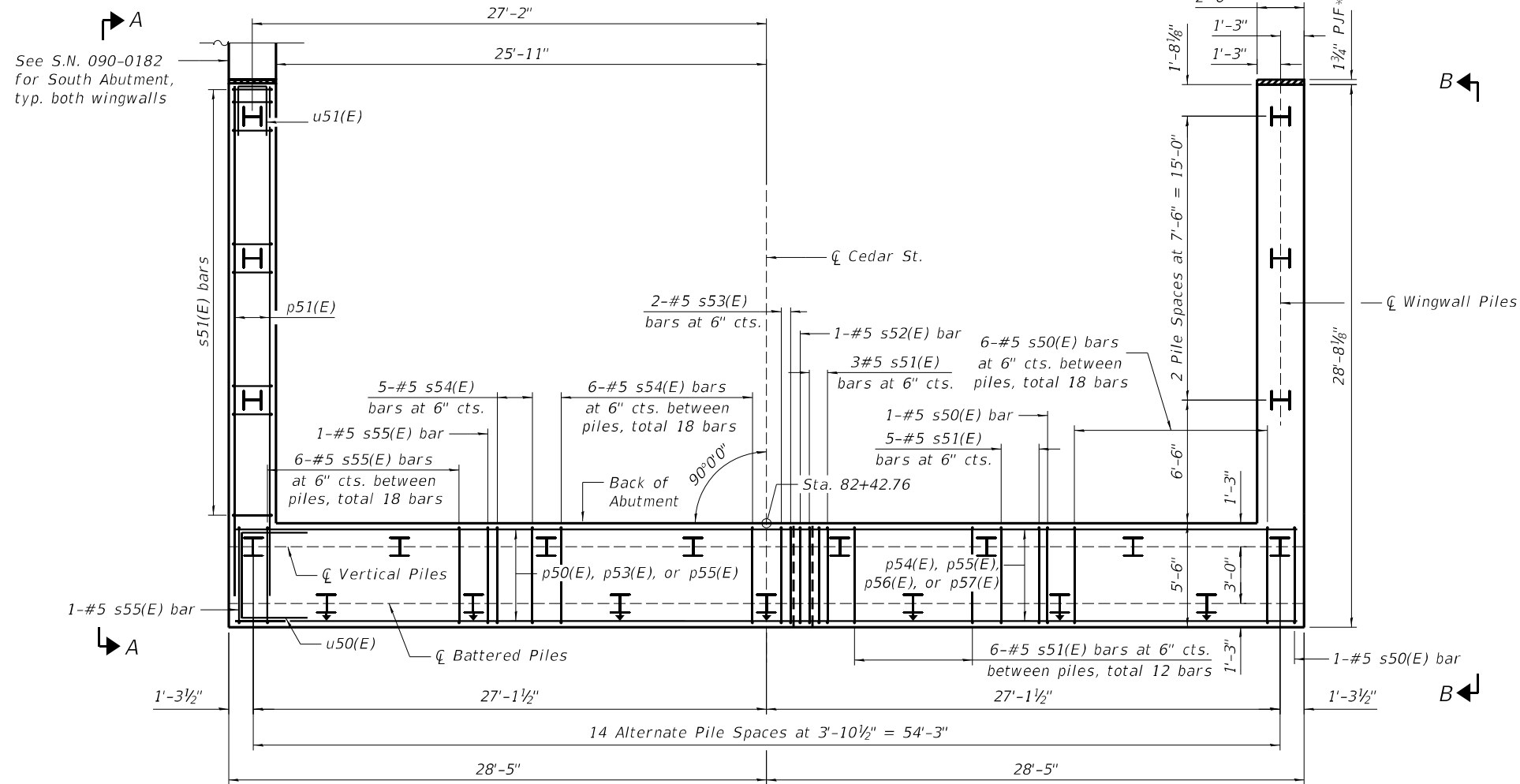
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VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	750
CONTRACT NO. 68C55				
ILLINOIS				FED. AID PROJECT



PLAN



ANCHOR BOLT LAYOUT



PILE CAP PLAN

NOTES

- For section A-A and B-B, see Sheet S2-32.
- For pile data, bill of material and bar bending details, see Sheet S2-33.

* Cost of 1 3/4" PJF included and installed with S.N. 090-0182

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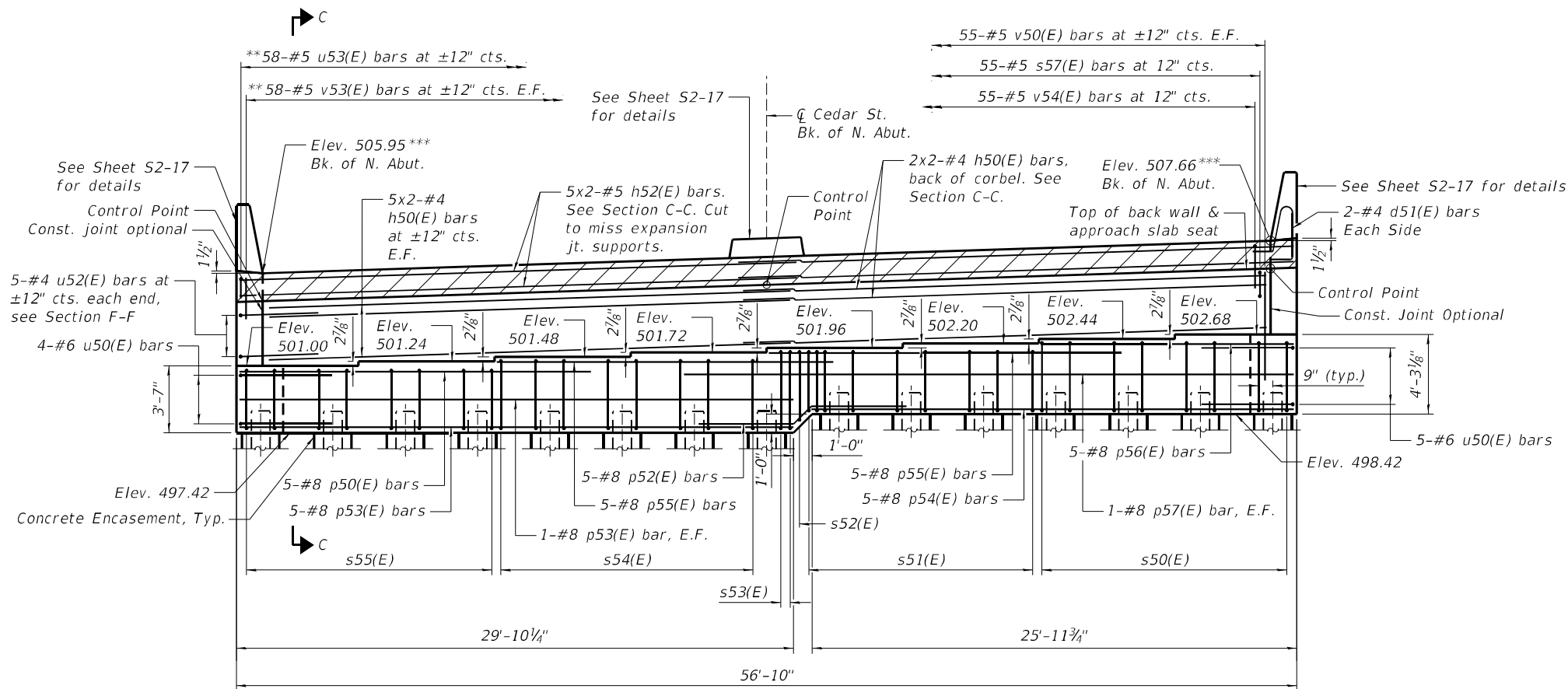
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT PLANS
STRUCTURE NO. 090-0183

SHEET S2-31 OF S2-47 SHEETS

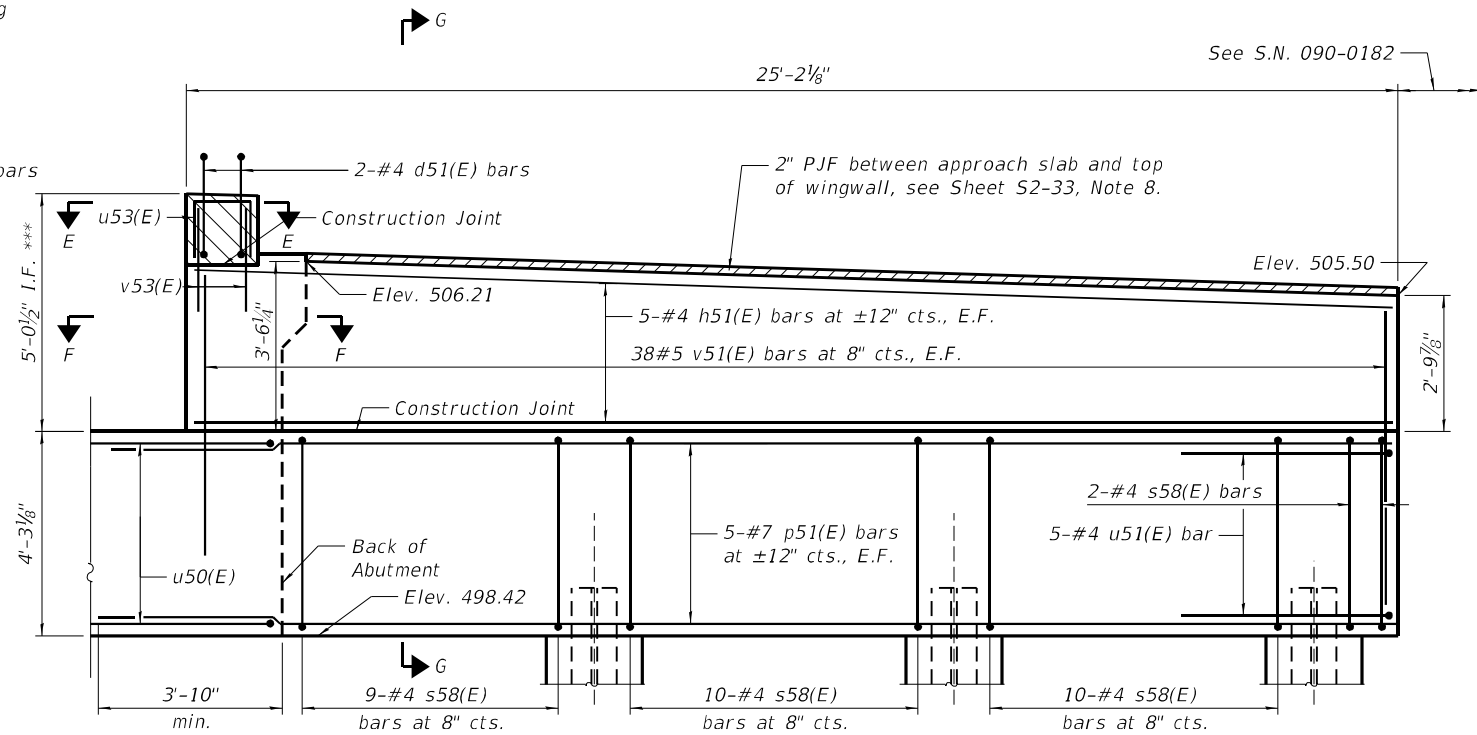
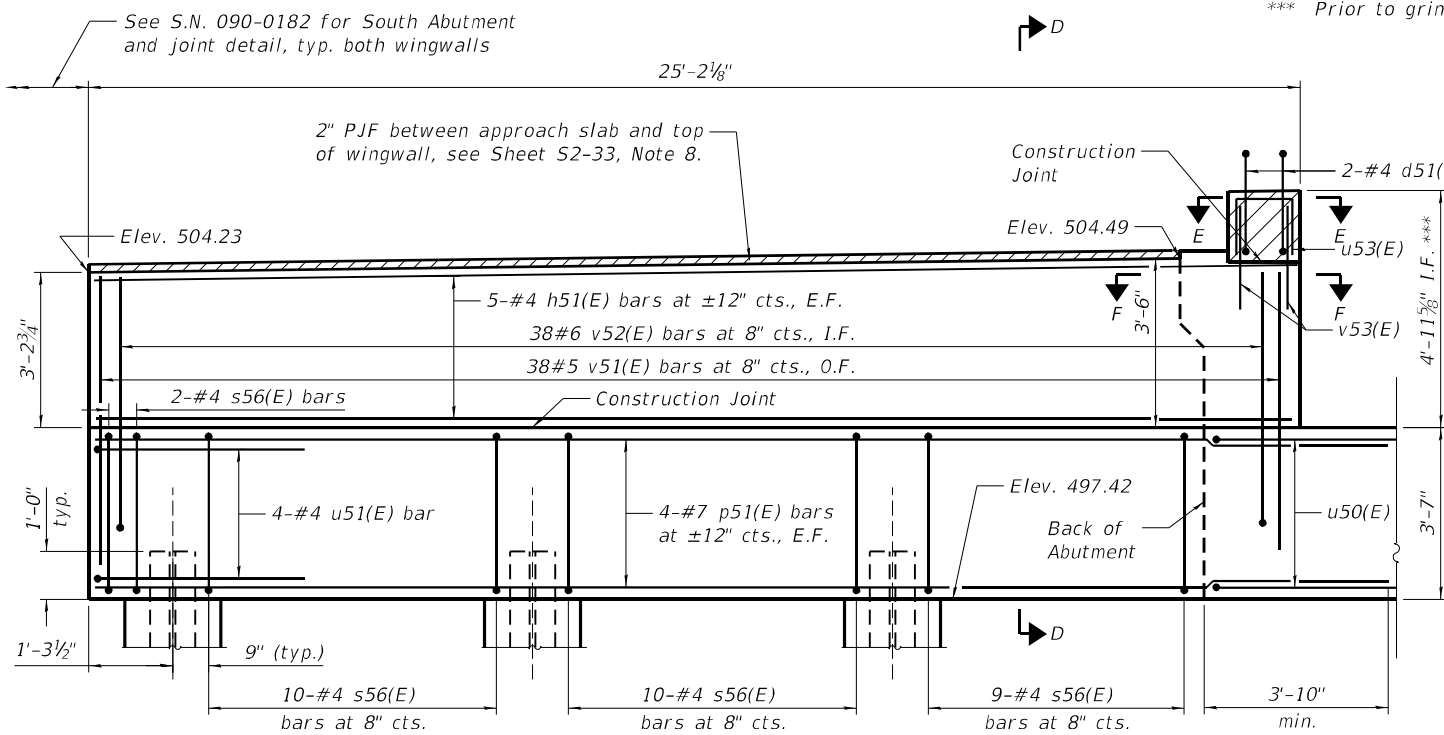
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CONTRACT NO. 68C55				
ILLINOIS		FED. AID PROJECT		

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** Spaced to miss expansion joint supports or members according to the approved shop drawings and as approved by the Engineer.

*** Prior to grinding



Note: For sections D-D, E-E, F-F, G-G, Bill of Material and Bar Bending details, see Sheet S2-33.



USER NAME = D1amb	DESIGNED - PK	REVISED -
	CHECKED - SPK	REVISED -
PLOT SCALE = 0.1667' / in.	DRAWN - MD	REVISED -
PLOT DATE = 6/6/2023	CHECKED - PK	REVISED -

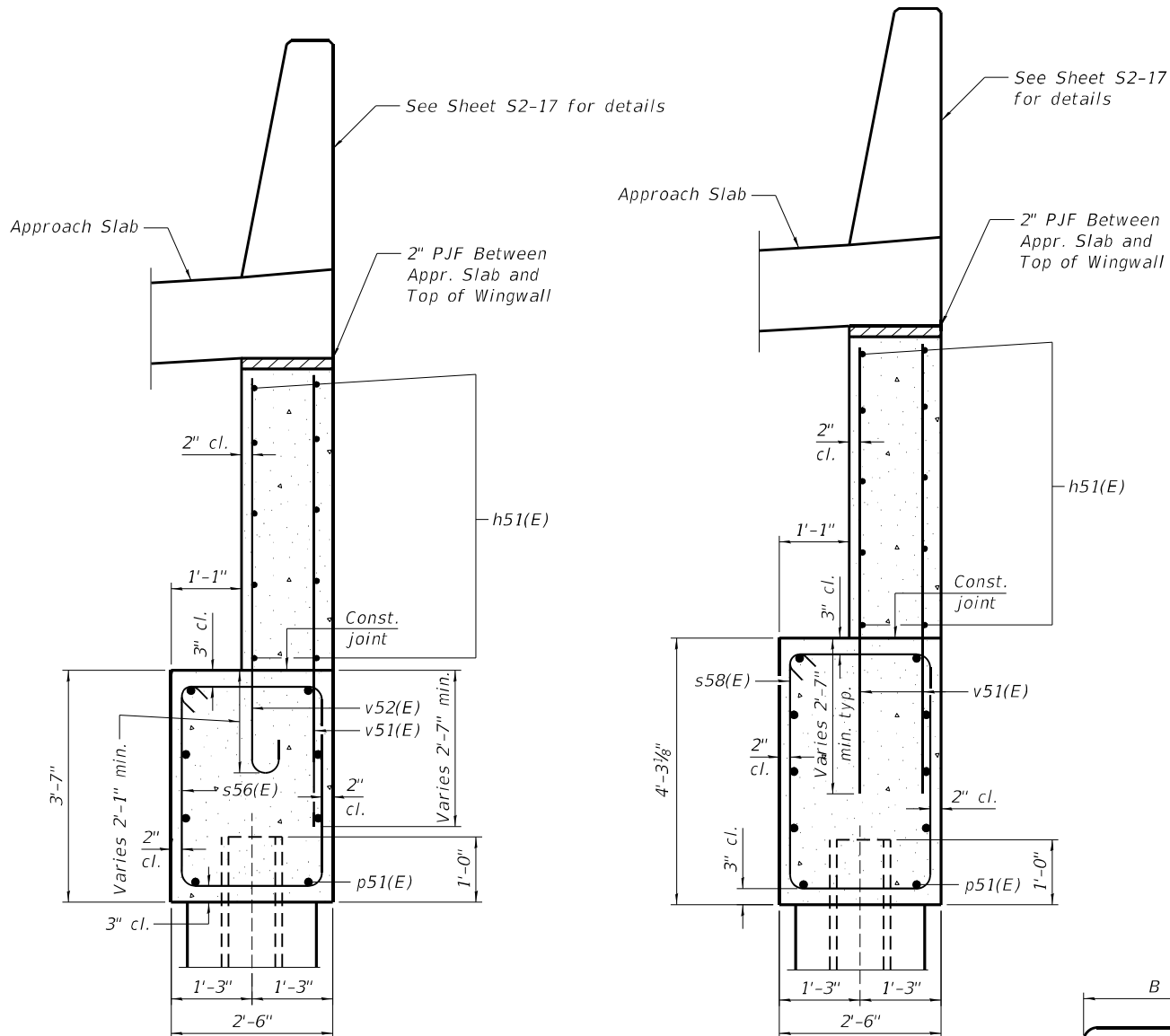
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT ELEVATION & SECTIONS
STRUCTURE NO. 090-0183

SHEET S2-32 OF S2-47 SHEETS

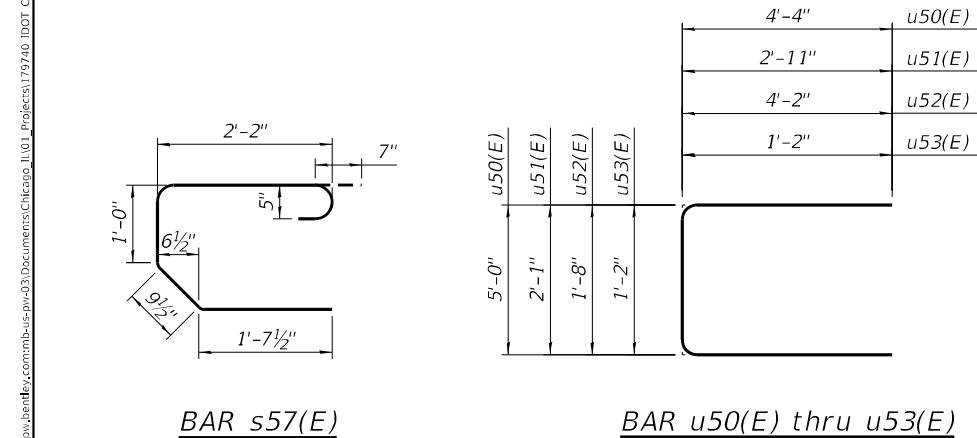
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CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

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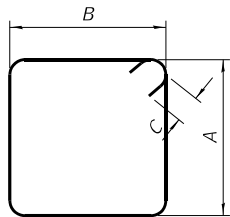
SECTION D-D

SECTION G-G



BAR s57(E)

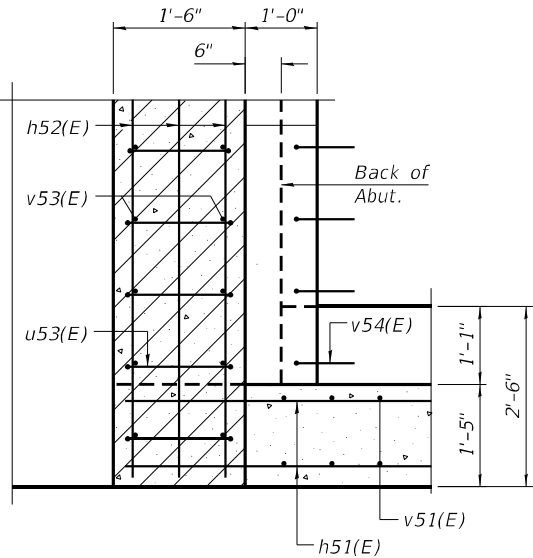
BAR u50(E) thru u53(E)



BAR s50(E) thru s56(E), & s58(E)

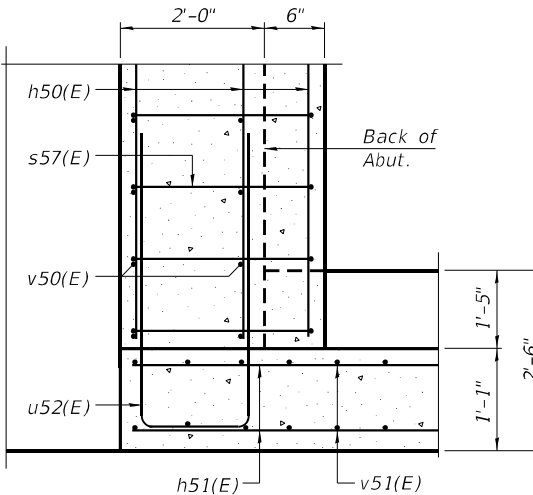
DIMENSIONS

Bar	A	B	C
s50(E)	3'-8"	5'-2"	5 1/2"
s51(E)	3'-2"	5'-2"	5 1/2"
s52(E)	3'-8 1/2"	5'-2"	5 1/2"
s53(E)	4'-3"	5'-2"	5 1/2"
s54(E)	3'-9"	5'-2"	5 1/2"
s55(E)	3'-3"	5'-2"	5 1/2"
s56(E)	3'-3"	2'-2"	4 1/2"
s58(E)	3'-11"	2'-2"	4 1/2"



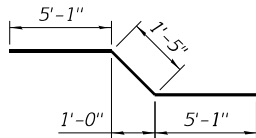
SECTION E-E

(Shown for east wingwall, mirror image for west wingwall)

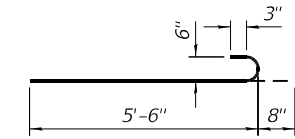


SECTION F-F

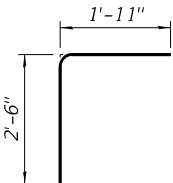
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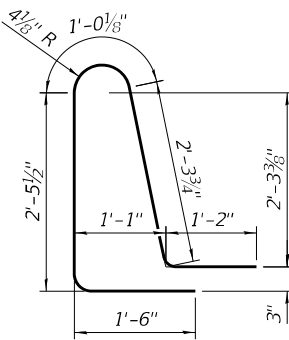
BAR p52(E)



BAR v52(E)



BAR v54(E)



BAR d51(E)

NORTH ABUTMENT
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d51(E)	4	#5	8'-6"	
h50(E)	14	#4	28'-2"	
h51(E)	20	#4	24'-10"	
h52(E)	10	#5	30'-0"	
p50(E)	5	#8	18'-1"	
p51(E)	18	#7	26'-11"	
p52(E)	5	#8	11'-7"	
p53(E)	7	#8	29'-8"	
p54(E)	5	#8	25'-10"	
p55(E)	10	#8	18'-10"	
p56(E)	5	#8	13'-6"	
p57(E)	2	#8	32'-10"	
s50(E)	20	#5	18'-7"	
s51(E)	20	#5	17'-7"	
s52(E)	1	#5	18'-8"	
s53(E)	2	#5	19'-9"	
s54(E)	23	#5	18'-9"	
s55(E)	20	#5	17'-9"	
s56(E)	31	#4	11'-7"	
s57(E)	55	#5	6'-2"	
s58(E)	31	#4	12'-11"	
u50(E)	9	#6	13'-8"	
u51(E)	9	#4	7'-11"	
u52(E)	10	#4	10'-0"	
u53(E)	58	#5	3'-6"	
v50(E)	110	#5	6'-7"	
v51(E)	114	#5	6'-0"	
v52(E)	38	#6	6'-2"	
v53(E)	116	#5	3'-1"	
v54(E)	55	#5	4'-5"	
Structure Excavation			Cu. Yd.	184
Concrete Structures			Cu. Yd.	86.9
Concrete Encasement			Cu. Yd.	7.3
Reinforcement Bars, Epoxy Coated			Pound	9,550
Furnishing Steel Piles HP12x53			Foot	1,360
Driving Piles			Foot	1,360
Test Pile Steel HP12x53			Each	1
Pile Shoes			Each	21
Granular Backfill for Structures			Cu. Yd.	130
Concrete Sealer			Sq. Ft.	612
Geocomposite Wall Drain			Sq. Yd.	91
Pipe Underdrains for Structures 4"			Foot	103

NOTES:

- Hatched area to be poured separately after superstructure falsework has been removed and after approach slab side formwork has been removed.
- Quantity of concrete in hatched area included with Concrete Superstructure on sheet S2-16.
- Space reinforcement in cap to miss anchor bolts.
- Pour steps monolithically with cap.
- For details of piles and Concrete Encasement, see sheet S2-44.
- The top of back wall and approach slab seat shall have a constant slope determined from the control points shown.
- Concrete Sealer shall be applied to the bearing seats and front faces of the hatched block, back wall, and abutment cap.
- The 2" P.J.F. shall be included with the cost of Concrete Structures. The P.J.F. should be per Article 1051.09 of the standard specifications, and bonded to wingwall with suitable adhesive as recommended by supplier.

N. ABUT. PILE DATA

Type: HP 12x53 with pile shoes
Nominal Required Bearing: 418 kips
Factored Resistance Available: 230 kips
Est. Length: 68
No. Production Piles: 20
No. Test Piles: 1



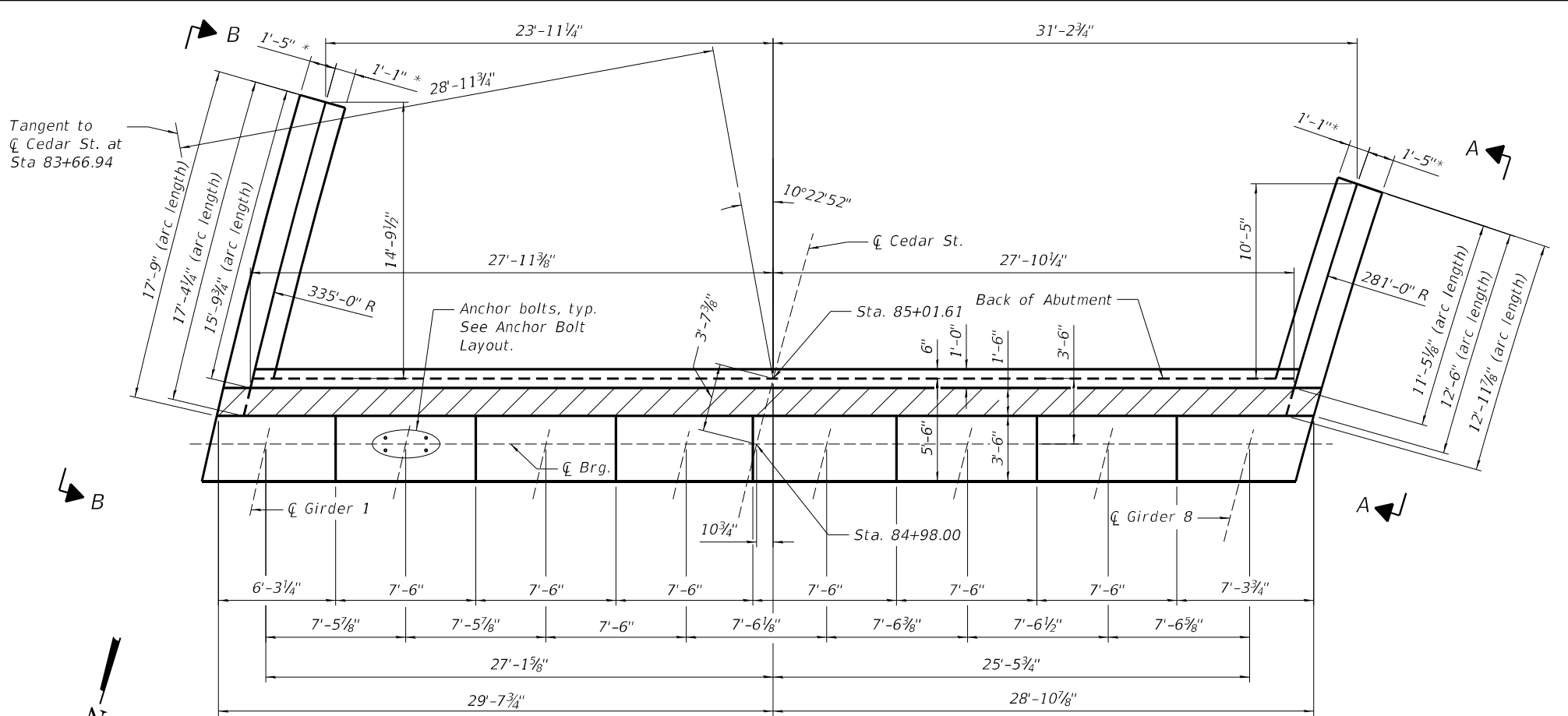
USER NAME = D1amb	DESIGNED - PK	REVISED -
	CHECKED - SPK	REVISED -
PLOT SCALE = 0.1667' / in.	DRAWN - MD	REVISED -
PLOT DATE = 6/6/2023	CHECKED - PK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT DETAILS
STRUCTURE NO. 090-0183

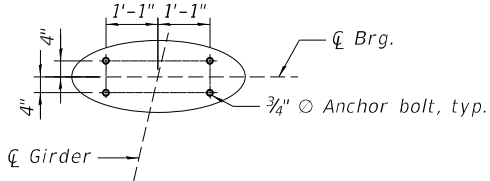
SHEET S2-33 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	753
				CONTRACT NO. 68C55
				ILLINOIS FED. AID PROJECT

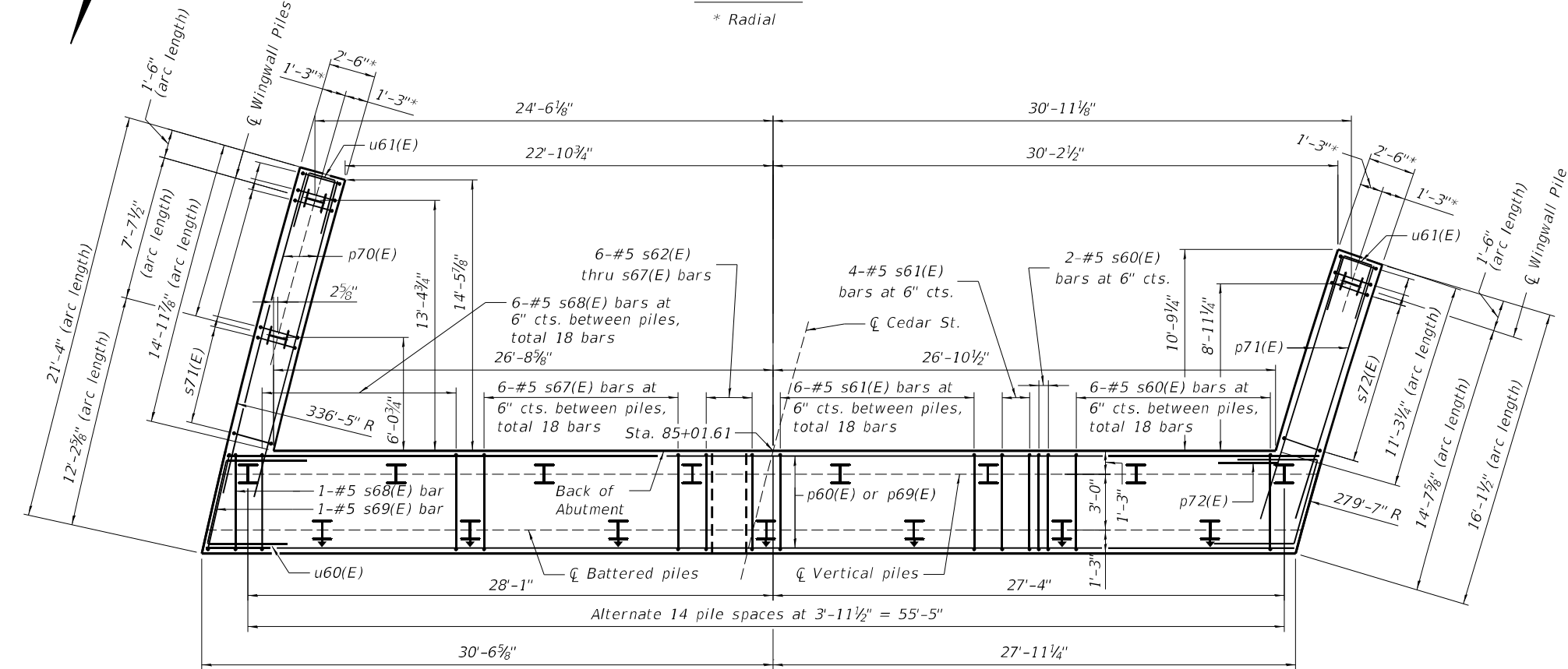


TOP VIEW

* Radial



ANCHOR BOLT LAYOUT



PILE CAP PLAN

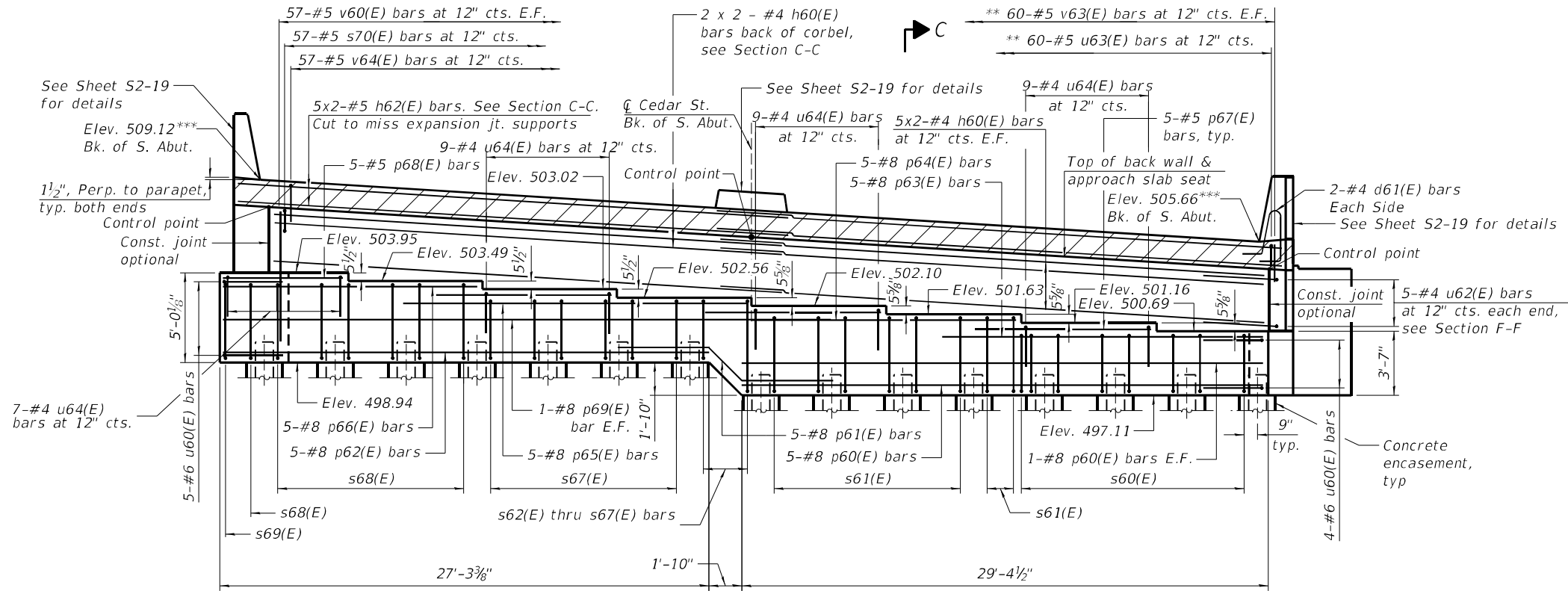
NOTES

- For View A-A and View B-B, see Sheet S2-35.
- For pile data, bill of material and bar bending details, see Sheet S2-36.

MODEL: Default
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	USER NAME = DIamb		DESIGNED - PK		REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SOUTH ABUTMENT PLANS STRUCTURE NO. 090-0183	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
			CHECKED - SPK		REVISED -				VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	754
	PLOT SCALE = 0.1667' / in.		DRAWN - TY		REVISED -				CONTRACT NO. 68C55				
	PLOT DATE = 6/6/2023		CHECKED - PK		REVISED -								
SHEET S2-34 OF S2-47 SHEETS								ILLINOIS FED. AID PROJECT					

MODEL: Default
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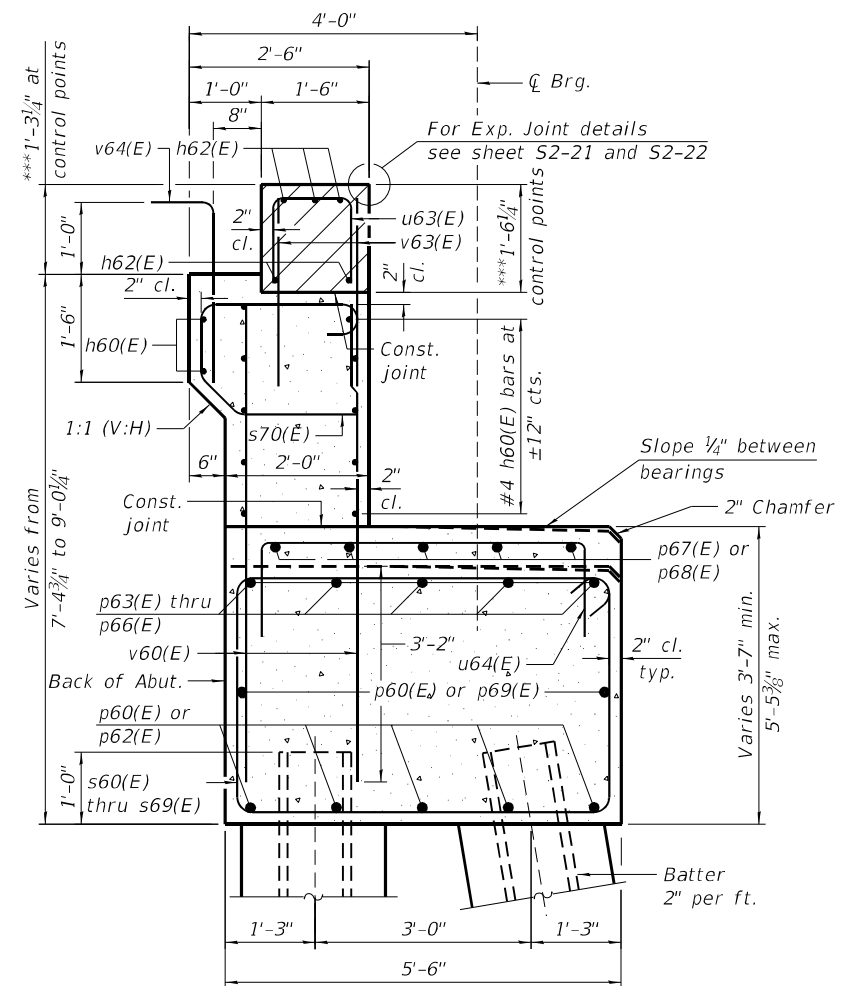


ELEVATION

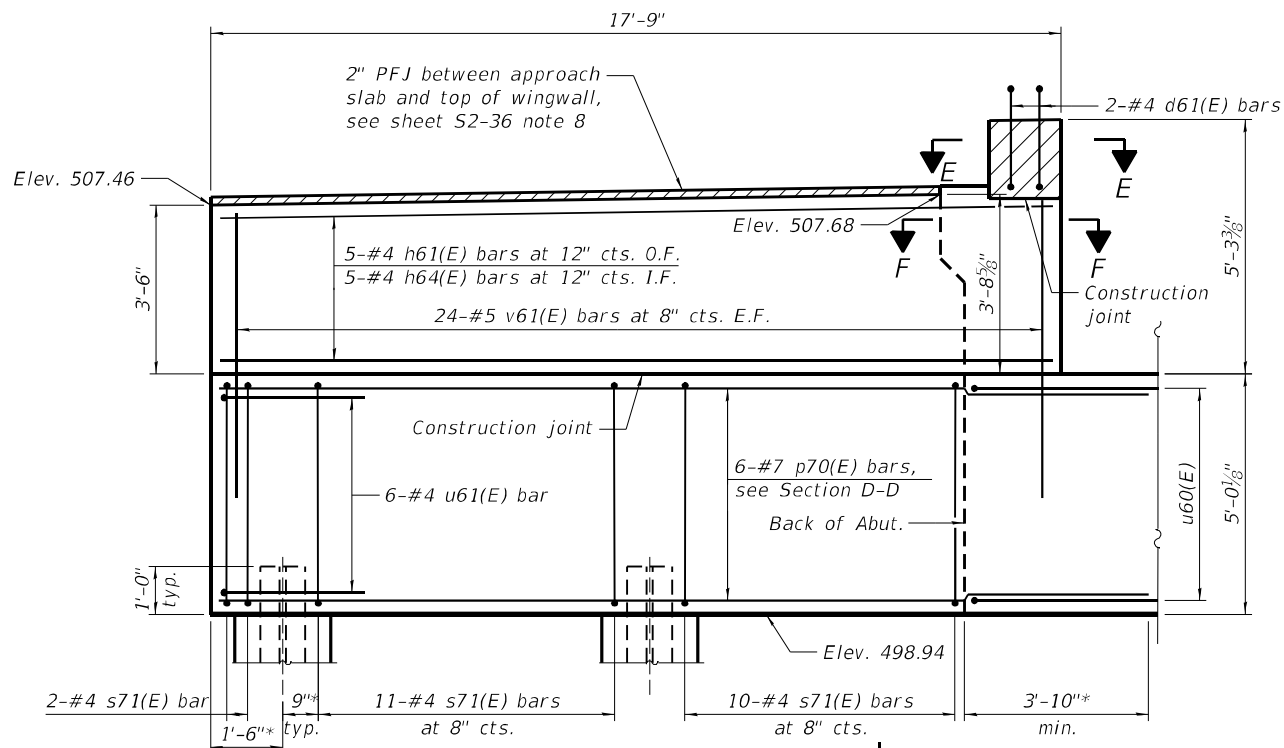
C

** Spaced to miss expansion joint supports or members according to the approved shop drawings and as approved by the Engineer.

*** Prior to grinding

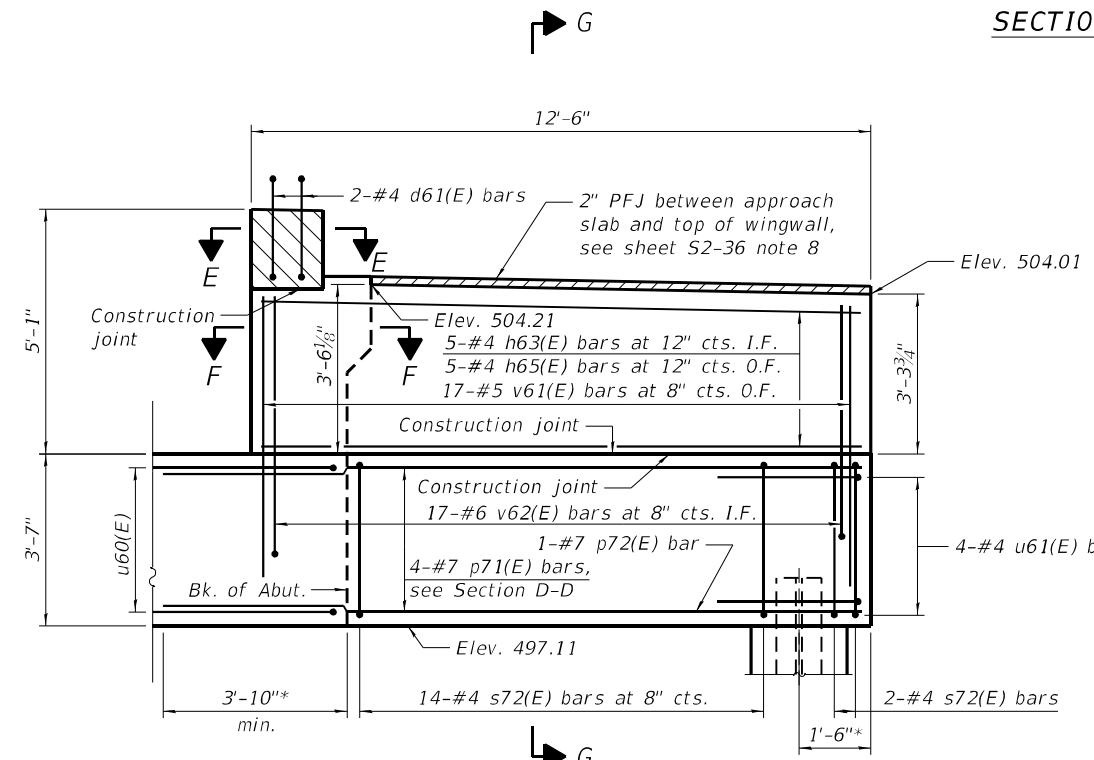


SECTION C-C



VIEW B-B

D



VIEW A-A

G

* Arc length measured along front face

NOTE:

For Sections D-D, E-E, F-F, G-G, bill of material, and bar bending details, see Sheet S2-36.



USER NAME = D1amb	DESIGNED - PK	REVISED -
	CHECKED - SPK	REVISED -
PLOT SCALE = 0.1667' / in.	DRAWN - TY	REVISED -
PLOT DATE = 6/6/2023	CHECKED - PK	REVISED -

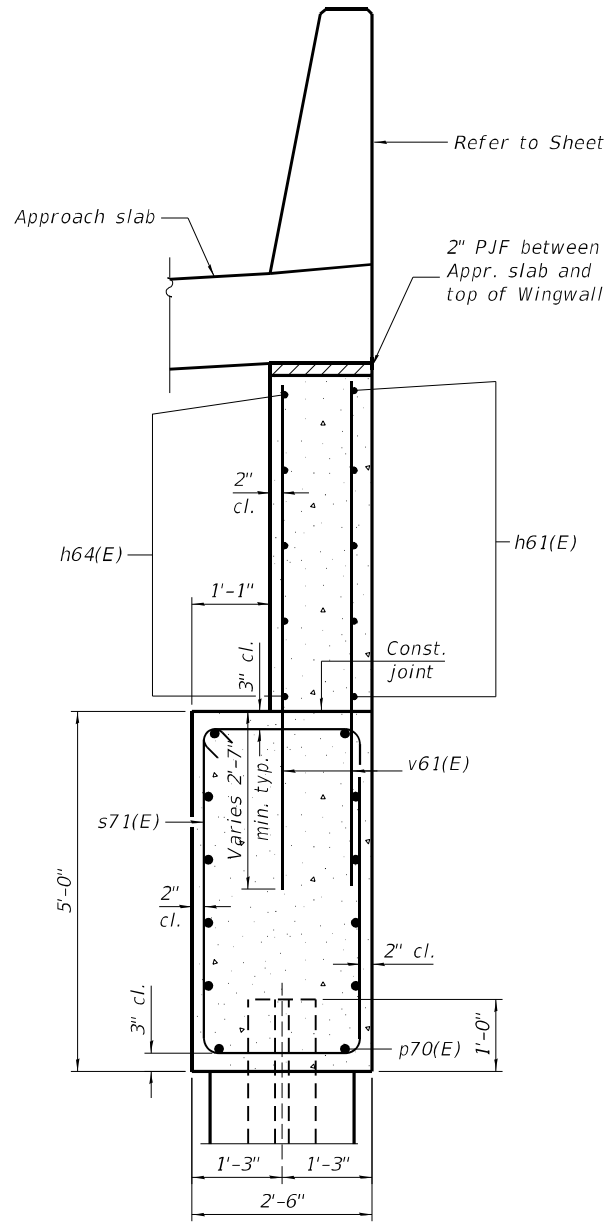
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOUTH ABUTMENT DETAILS
STRUCTURE NO. 090-0183

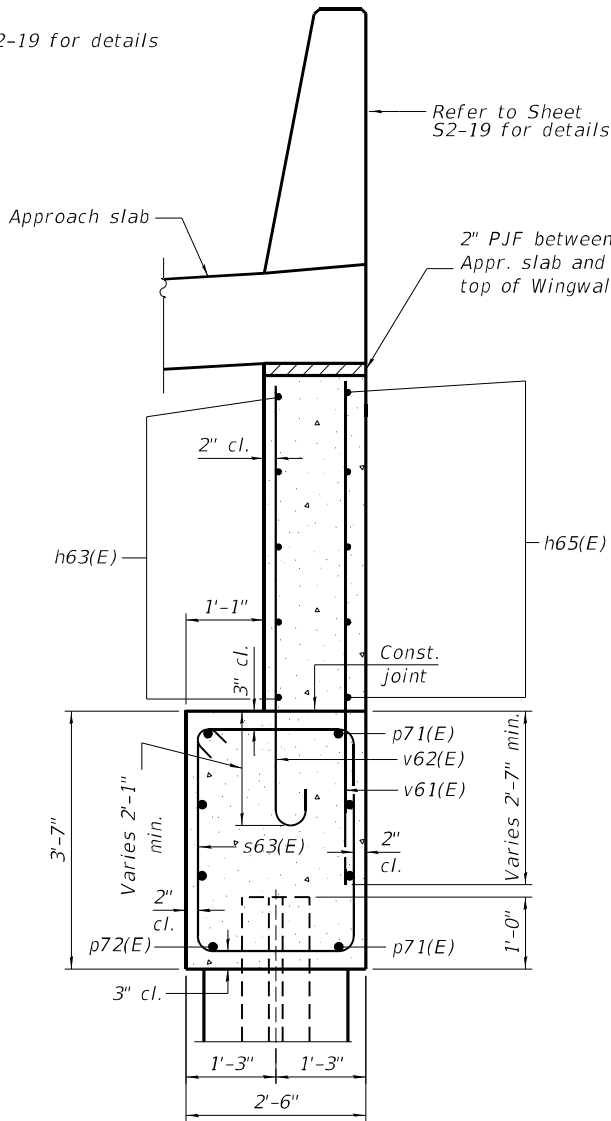
SHEET S2-35 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	755
CONTRACT NO. 68C55				
ILLINOIS		FED. AID PROJECT		

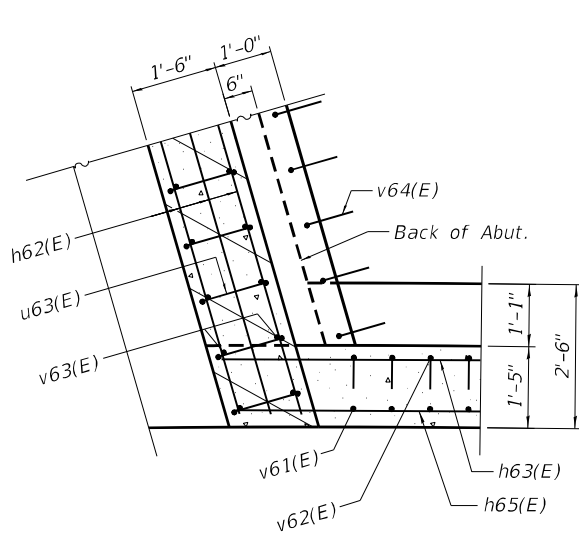
MODEL: Default
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SECTION D-D

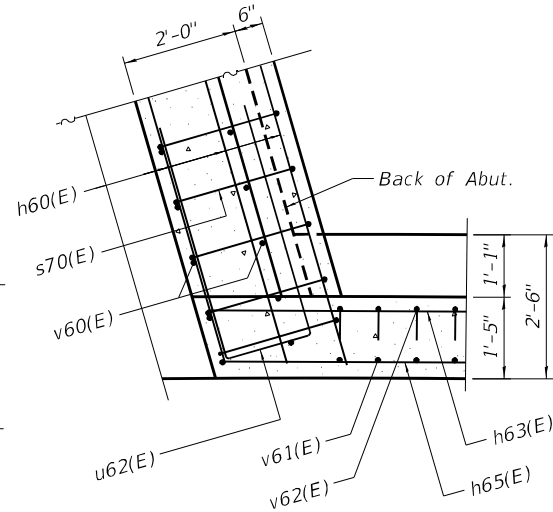


SECTION G-G



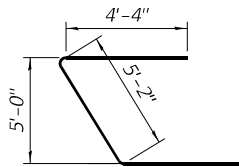
SECTION E-E

(Shown for west wingwall,
similar for east wingwall)

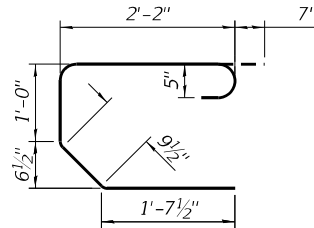


SECTION F-F

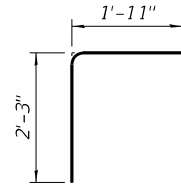
(Shown for west wingwall,
similar for east wingwall)



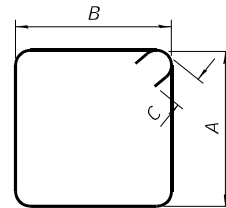
BAR u60(E)



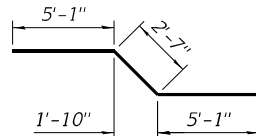
BAR s70(E)



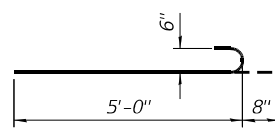
BAR v64(E)



BAR s60(E) thru s69(E), s71(E), s72(E)



BAR p61(E)



BAR v62(E)

DIMENSIONS

Bar	A	B	C
s60(E)	3'-3"	5'-2"	5 1/2"
s61(E)	4'-2"	5'-2"	5 1/2"
s62(E)	5'-1 1/2"	5'-2"	5 1/2"
s63(E)	4'-9"	5'-2"	5 1/2"
s64(E)	4'-4 1/2"	5'-2"	5 1/2"
s65(E)	4'-0"	5'-2"	5 1/2"
s66(E)	3'-7 1/2"	5'-2"	5 1/2"
s67(E)	3'-3 1/2"	5'-2"	5 1/2"
s68(E)	4'-2 1/2"	5'-2"	5 1/2"
s69(E)	4'-2 1/2"	5'-3 1/2"	5 1/2"
s71(E)	4'-6"	2'-2"	4 1/2"
s72(E)	3'-1"	2'-2"	4 1/2"

NOTES

- Hatched area to be poured separately after superstructure falsework has been removed and after approach slab side formwork has been removed.
- Quantity of concrete in hatched area included with Concrete Superstructure on sheet S2-16.
- Space reinforcement in cap to miss anchor bolts.
- Pour steps monolithically with cap.
- For details of piles and Concrete Encasement, see sheet S2-44.
- The top of back wall and approach slab seat shall have a constant slope determined from the control points shown.
- Concrete Sealer shall be applied to the bearing seats and front faces of the hatched block, back wall, and abutment cap.
- The 2" PJF shall be included with the cost of Concrete Structures. The PJF should be per Article 1051.09 of the standard specifications, and bonded to wingwall with suitable adhesive as recommended by supplier.

SOUTH ABUTMENT
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d61(E)	4	#5	8'-6"	
h60(E)	24	#4	29'-4"	
h61(E)	5	#4	17'-5"	
h62(E)	10	#5	30'-11"	
h63(E)	5	#4	12'-7"	
h64(E)	5	#4	17'-0"	
h65(E)	5	#4	12'-2"	
p60(E)	7	#8	29'-3"	
p61(E)	5	#8	12'-9"	
p62(E)	5	#8	27'-2"	
p63(E)	5	#8	18'-1"	
p64(E)	5	#8	14'-11"	
p65(E)	5	#8	19'-3"	
p66(E)	5	#8	14'-3"	
p67(E)	15	#5	9'-11"	
p68(E)	5	#5	6'-10"	
p69(E)	2	#8	33'-11"	
p70(E)	12	#7	19'-5"	
p71(E)	7	#7	14'-7"	
p72(E)	1	#7	15'-5"	
s60(E)	20	#5	17'-9"	
s61(E)	22	#5	19'-7"	
s62(E)	1	#5	21'-6"	
s63(E)	1	#5	20'-9"	
s64(E)	1	#5	20'-0"	
s65(E)	1	#5	19'-3"	
s66(E)	1	#5	18'-6"	
s67(E)	19	#5	17'-10"	
s68(E)	19	#5	19'-8"	
s69(E)	1	#5	19'-11"	
s70(E)	57	#5	6'-2"	
s71(E)	23	#4	14'-1"	
s72(E)	16	#4	11'-3"	
u60(E)	9	#6	13'-10"	
u61(E)	2	#4	7'-11"	
u62(E)	10	#4	10'-0"	
u63(E)	60	#5	3'-6"	
u64(E)	34	#4	9'-4"	
v60(E)	114	#5	6'-7"	
v61(E)	65	#5	6'-3"	
v62(E)	17	#6	5'-8"	
v63(E)	120	#5	2'-4"	
v64(E)	57	#5	4'-2"	
Structure Excavation		Cu. Yd.	158	
Concrete Structures		Cu. Yd.	84.9	
Concrete Encasement		Cu. Yd.	6.3	
Reinforcement Bars, Epoxy Coated		Pound	9,070	
Furnishing Steel Piles HP12x53		Foot	1,020	
Driving Piles		Foot	1,020	
Test Pile Steel HP12x53		Each	1	
Pile Shoes		Each	18	
Granular Backfill for Structures		Cu. Yd.	113	
Concrete Sealer		Sq. Ft.	642	
Geocomposite Wall Drain		Sq. Yd.	52	
Pipe Underdrains for Structures 4"		Foot	76	

S. ABUT. PILE DATA

Type: HP 12x53 with pile shoes
Nominal Required Bearing: 418 kips
Factored Resistance Available: 230 kips
Est. Length: 60 feet
No. Production Piles: 17
No. Test Piles: 1



USER NAME = Jaime.Rivera
PLOT SCALE = 0.1667 ' / in.
PLOT DATE = 5/1/2025

DESIGNED - PK
CHECKED - SPK
DRAWN - TY
CHECKED - PK

REVISED -
REVISED -
REVISED -
REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

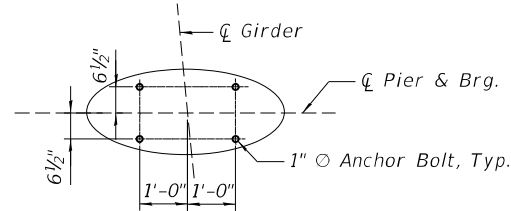
SOUTH ABUTMENT DETAILS
STRUCTURE NO. 090-0183

SHEET S2-36 OF S2-47 SHEETS

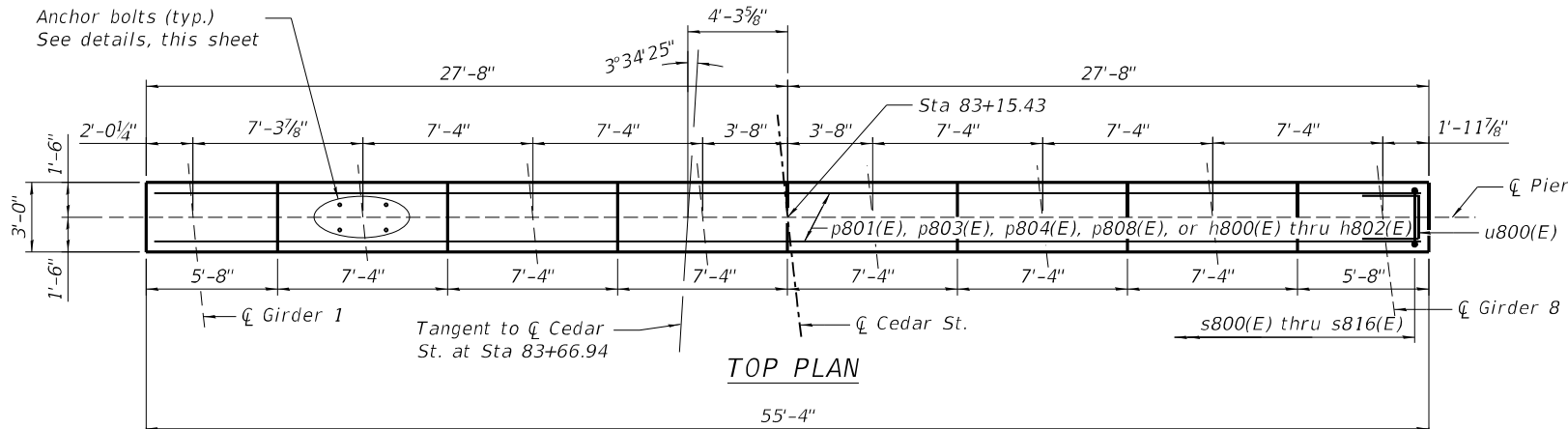
F.A.P. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO.
VAR. 13 (HVB, HB-3) BR-1:R TAZEWELL 1173 756
CONTRACT NO. 68C55
ILLINOIS FED. AID PROJECT

PILE DATA

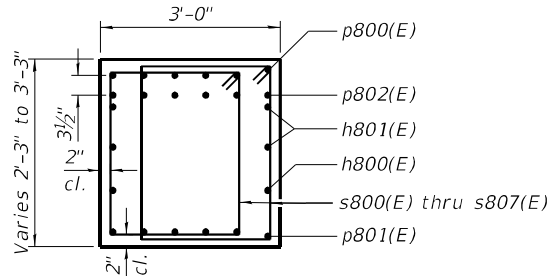
Type: Steel HP 12x53 with pile shoes
Nominal Required Bearing: 418 kips
Factored Resistance Available: 230 kips
Est. Length: 43 ft
No. Production Piles: 23
No. Test Piles: 1



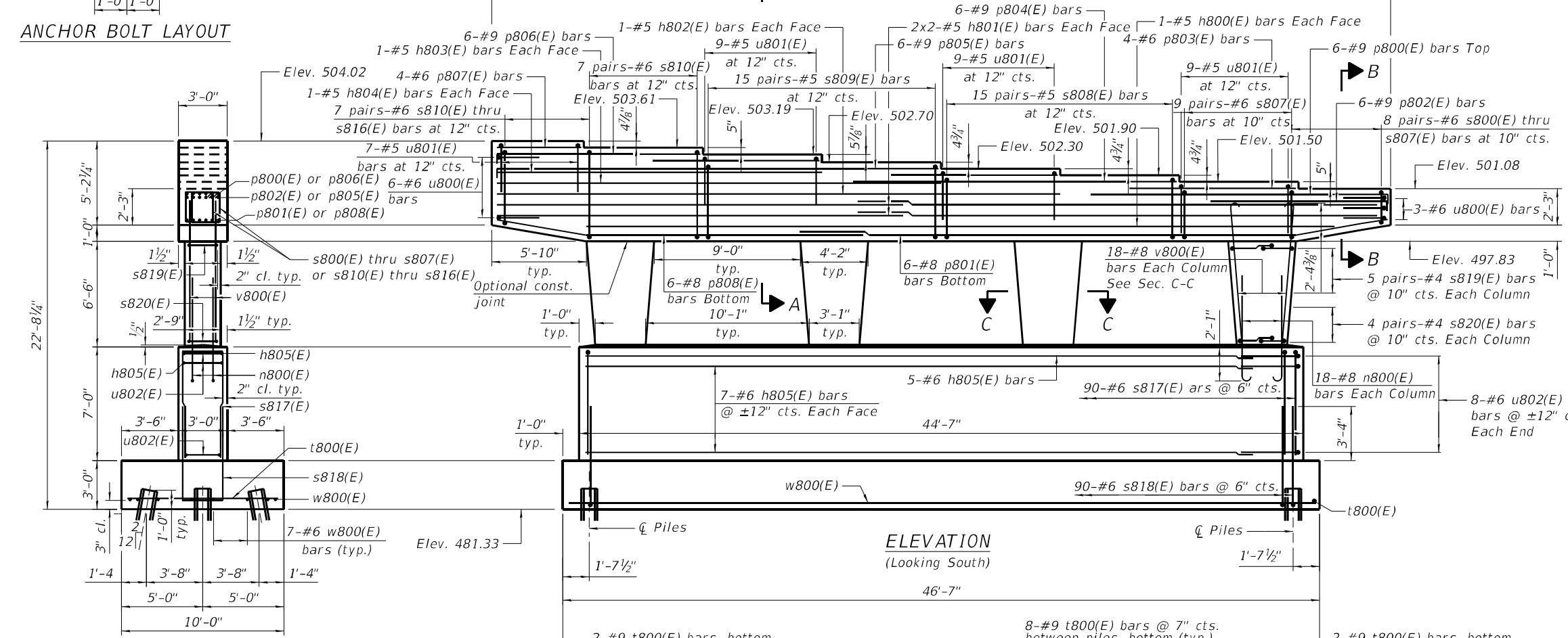
ANCHOR BOLT LAYOUT



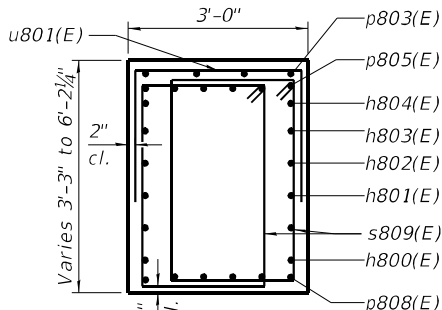
TOP PLAN



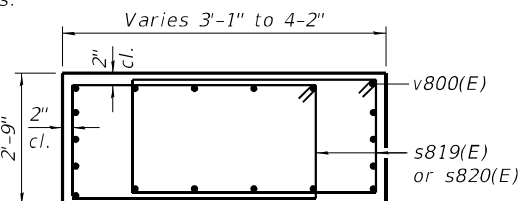
SECTION B-B



ELEVATION
(Looking South)

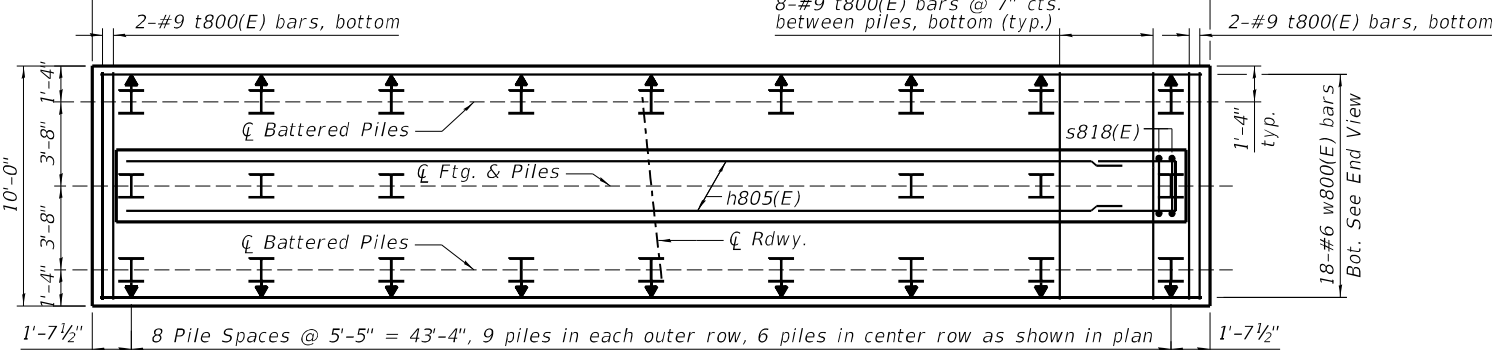


SECTION A-A



SECTION C-C

END VIEW



FOOTING PLAN

MIN. BAR LAP

- #4 = 1'-4" (Hor. U-bar)
- #5 = 3'-7"
- #8 = 7'-2"

NOTES

- Space reinforcement in cap to miss anchor bolts.
- Pour steps monolithically with cap.
- For details of piles, see sheet S2-44.
- For bar bending details and Bill of Material, see S2-39.
- Concrete Sealer shall be applied to the exposed area of the cap, columns, and crashwall.

MODEL: Default
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PLOT SCALE = 0.1667' / in.	CHECKED - SPK	REVISED -
PLOT DATE = 6/6/2023	DRAWN - TY	REVISED -
	CHECKED - PK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

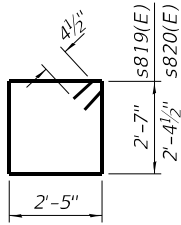
PIER 1
STRUCTURE NO. 090-0183

SHEET S2-38 OF S2-47 SHEETS

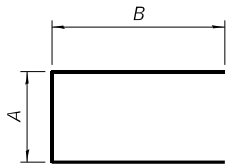
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	758
				CONTRACT NO. 68C55

ILLINOIS FED. AID PROJECT

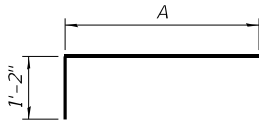
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BARS *s819(E)* & *s820(E)*



BARS *u800(E)*, *u801(E)*, & *u802(E)*



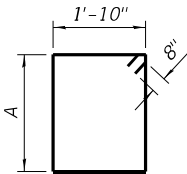
BARS *p800(E)*, *p802(E)*, *p805(E)*, & *p806(E)*

A & B DIMENSIONS

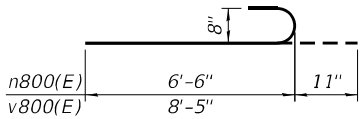
Bar	A	B
<i>u800(E)</i>	2'-6"	3'-10"
<i>u801(E)</i>	2'-8"	2'-9"
<i>u802(E)</i>	2'-6"	3'-4"

A DIMENSIONS

Bar	A
<i>p800(E)</i>	18'-3"
<i>p802(E)</i>	10'-11"
<i>p805(E)</i>	27'-4"
<i>p806(E)</i>	12'-8"



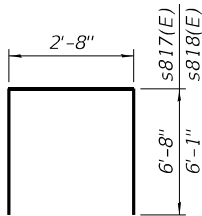
BARS *s800(E)* THRU *s816(E)*



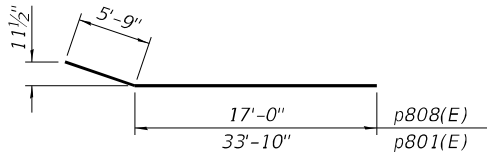
BARS *n800(E)* & *v800(E)*

A DIMENSIONS

Bar	A
<i>s800(E)</i>	1'-11"
<i>s801(E)</i>	2'-1"
<i>s802(E)</i>	2'-3"
<i>s803(E)</i>	2'-5"
<i>s804(E)</i>	2'-7"
<i>s805(E)</i>	2'-8"
<i>s806(E)</i>	2'-9"
<i>s807(E)</i>	2'-11"
<i>s808(E)</i>	3'-9"
<i>s809(E)</i>	4'-7"
<i>s810(E)</i>	5'-6"
<i>s811(E)</i>	5'-4"
<i>s812(E)</i>	5'-2"
<i>s813(E)</i>	5'-0"
<i>s814(E)</i>	4'-10"
<i>s815(E)</i>	4'-8"
<i>s816(E)</i>	4'-6"



BARS *s817(E)* & *s818(E)*



BARS *p801(E)* & *p808(E)*

PIER 1
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
<i>h800(E)</i>	2	#5	46'-1"	=====
<i>h801(E)</i>	8	#5	29'-4"	=====
<i>h802(E)</i>	2	#5	34'-10"	=====
<i>h803(E)</i>	2	#5	27'-6"	=====
<i>h804(E)</i>	2	#5	20'-2"	=====
<i>h805(E)</i>	19	#6	44'-3"	=====
<i>n800(E)</i>	72	#8	7'-5"	=====
<i>p800(E)</i>	6	#9	19'-5"	=====
<i>p801(E)</i>	6	#8	39'-7"	=====
<i>p802(E)</i>	6	#9	12'-1"	=====
<i>p803(E)</i>	12	#6	10'-3"	=====
<i>p804(E)</i>	6	#9	19'-11"	=====
<i>p805(E)</i>	6	#9	28'-6"	=====
<i>p806(E)</i>	6	#9	13'-10"	=====
<i>p807(E)</i>	4	#6	5'-4"	=====
<i>p808(E)</i>	6	#8	22'-9"	=====
<i>s800(E)</i>	2	#6	8'-10"	=====
<i>s801(E)</i>	2	#6	9'-2"	=====
<i>s802(E)</i>	2	#6	9'-6"	=====
<i>s803(E)</i>	2	#6	9'-10"	=====
<i>s804(E)</i>	2	#6	10'-2"	=====
<i>s805(E)</i>	2	#6	10'-4"	=====
<i>s806(E)</i>	2	#6	10'-6"	=====
<i>s807(E)</i>	20	#6	10'-10"	=====
<i>s808(E)</i>	30	#5	12'-6"	=====
<i>s809(E)</i>	30	#5	14'-2"	=====
<i>s810(E)</i>	16	#6	16'-0"	=====
<i>s811(E)</i>	2	#6	15'-8"	=====
<i>s812(E)</i>	2	#6	15'-4"	=====
<i>s813(E)</i>	2	#6	15'-0"	=====
<i>s814(E)</i>	2	#6	14'-8"	=====
<i>s815(E)</i>	2	#6	14'-4"	=====
<i>s816(E)</i>	2	#6	14'-0"	=====
<i>s817(E)</i>	90	#6	16'-0"	=====
<i>s818(E)</i>	90	#6	14'-10"	=====
<i>s819(E)</i>	40	#4	10'-9"	=====
<i>s820(E)</i>	32	#4	10'-4"	=====
<i>t800(E)</i>	68	#9	9'-8"	=====
<i>u800(E)</i>	9	#6	10'-2"	=====
<i>u801(E)</i>	34	#5	8'-2"	=====
<i>u802(E)</i>	16	#6	9'-2"	=====
<i>v800(E)</i>	72	#8	9'-4"	=====
<i>w800(E)</i>	18	#6	46'-3"	=====
Structure Excavation			Cu. Yd.	121
Concrete Structures			Cu. Yd.	124.3
Reinforcement Bars, Epoxy Coated			Pound	18,950
Furnishing Steel Piles HP12x53			Foot	989
Driving Piles			Foot	989
Test Pile Steel HP12x53			Each	1
Pile Shoes			Each	24
Concrete Sealer			Sq. Ft.	1,268



USER NAME	= DIamb
PLOT SCALE	= 0.1667 ' / in.
PLOT DATE	= 6/6/2023

DESIGNED	- PK
CHECKED	- SPK
DRAWN	- TY
CHECKED	- PK

REVISED	-
REVISED	-
REVISED	-
REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIER 1 DETAILS
STRUCTURE NO. 090-0183

SHEET 52-39 OF 52-47 SHEETS

F.A.P.
RTE.

SECTION

VAR.

13 (HVB, HB-3) BR-1;R

COUNTY

TAZEWELL

CONTRACT NO. 68C55

TOTAL
SHEETS

1173

SHEET
NO.

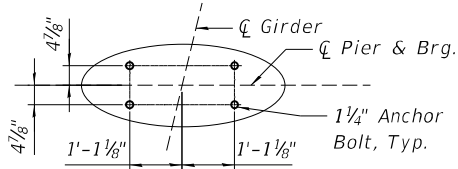
759

ILLINOIS

FED. AID PROJECT

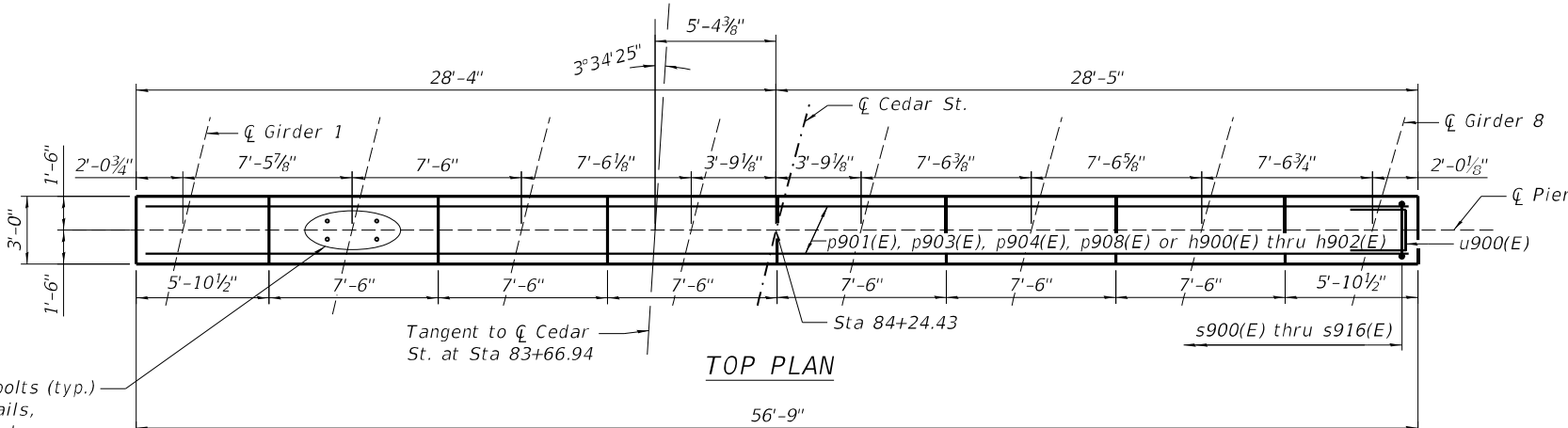
PILE DATA

Type: Steel HP 12x53 with pile shoes
Nominal Required Bearing: 418 kips
Factored Resistance Available: 230 kips
Est. Length: 43 ft
No. Production Piles: 23
No. Test Piles: 1

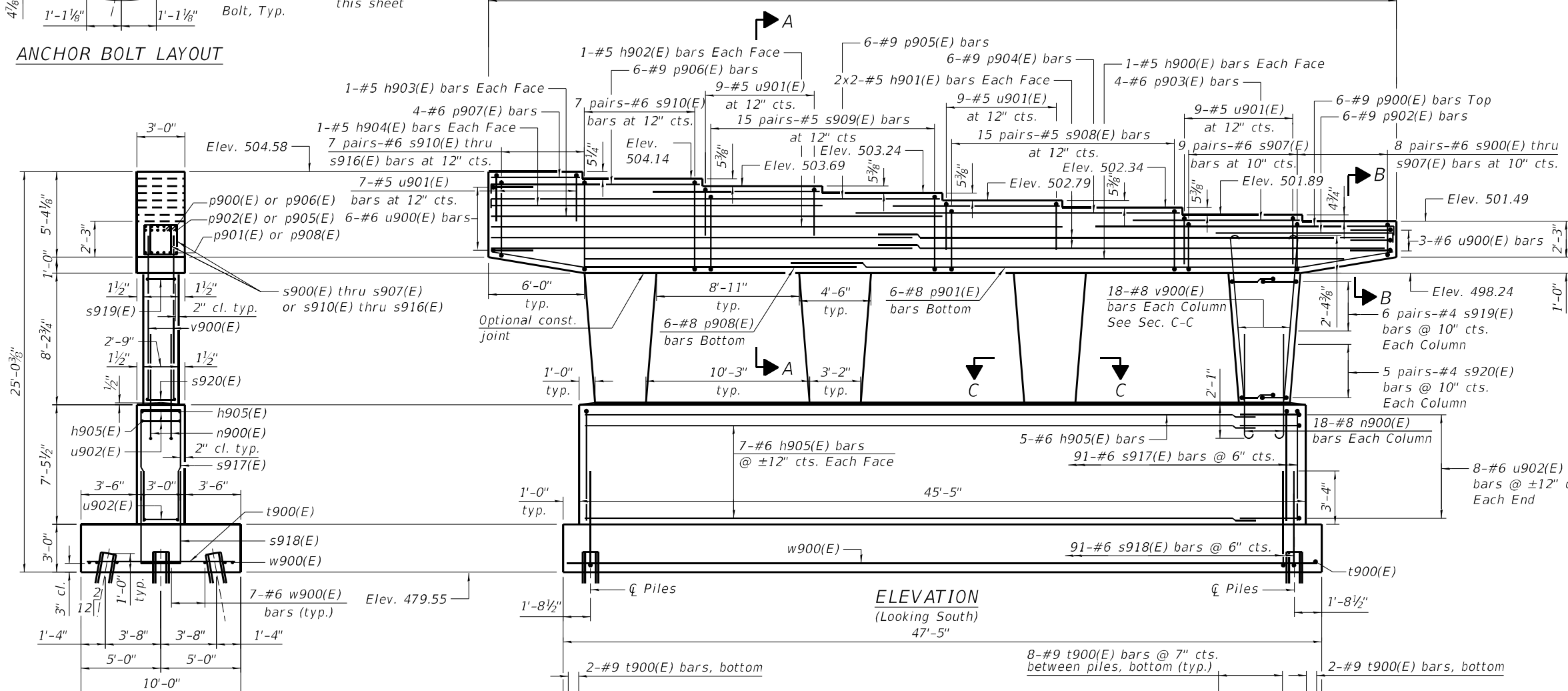


ANCHOR BOLT LAYOUT

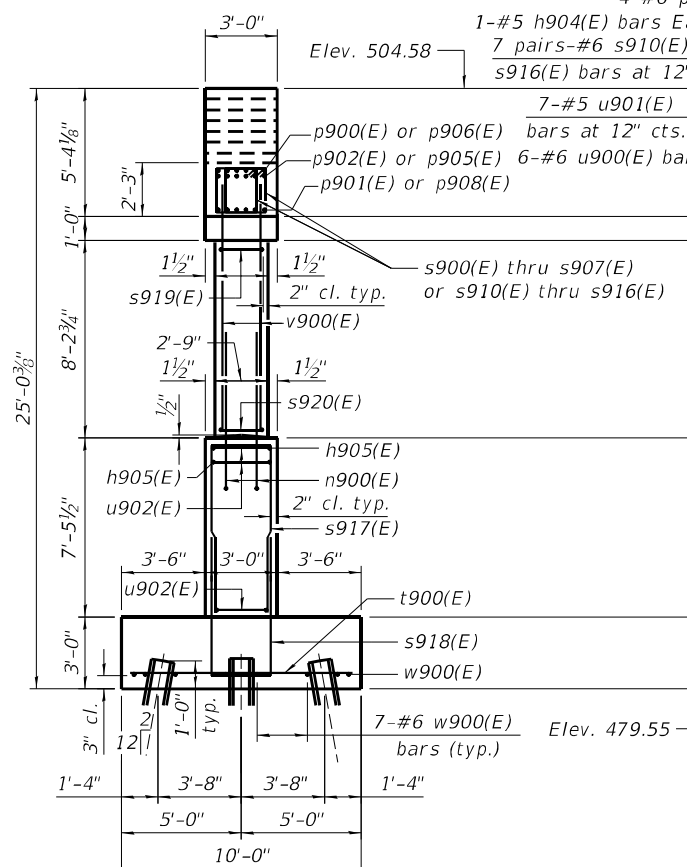
Anchor bolts (typ.)
See details,
this sheet



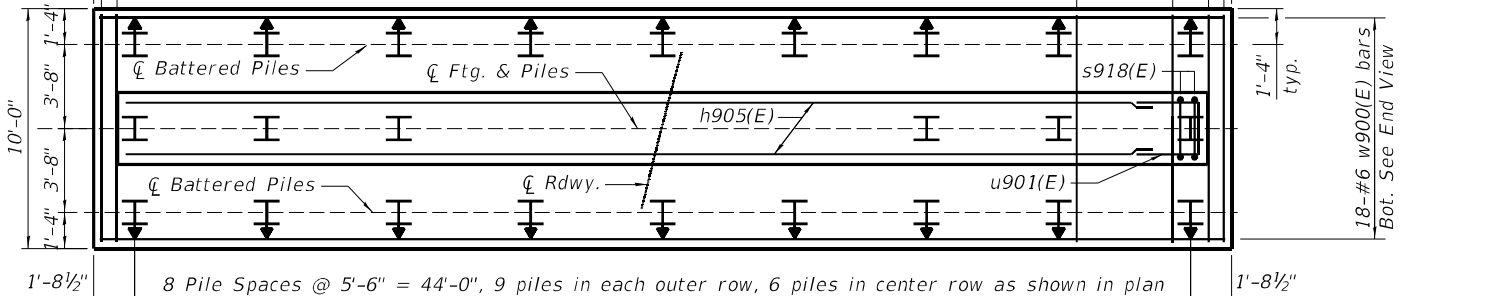
TOP PLAN



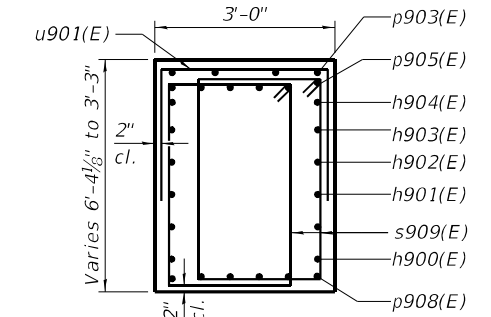
ELEVATION
(Looking South)
47'-5"



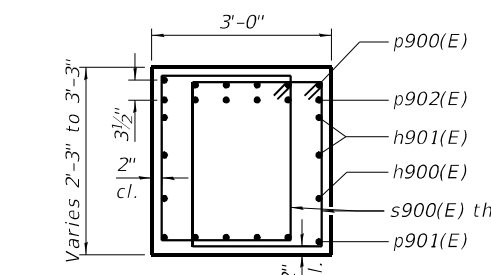
END VIEW



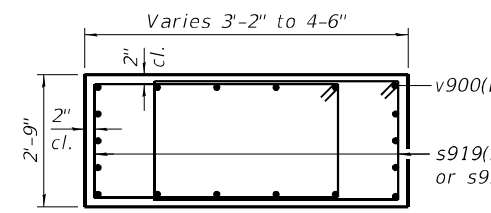
FOOTING PLAN



SECTION A-A



SECTION B-B



SECTION C-C

MIN. BAR LAP

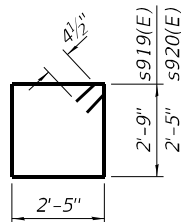
#4 = 1'-4" (Hor. U-bar)
#5 = 3'-7"
#8 = 7'-2"

NOTES

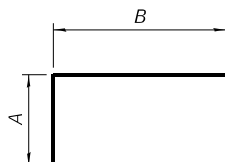
- Space reinforcement in cap to miss anchor bolts.
- Pour steps monolithically with cap.
- For details of piles, see sheet S2-44.
- For bar bending details and Bill of Material, see sheet S2-41.
- Concrete Sealer shall be applied to the exposed area of the cap, columns, and crashwall.

MODEL: Default
FILE: \naulie_g\p\mbus\sub\benley.com\mbus-pw\03\Documents\Chicago_1\011 Projectal\79740_IDOT CedarStExt_Philis.O_CADD\CAD_Sheets\Main St. Bridge Sheets\09001B3-48C55-03B.pier2.dgn

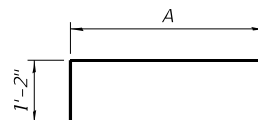
	USER NAME = DIamb			DESIGNED - PK	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PIER 2 STRUCTURE NO. 090-0183			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE = 0.1667' / in.			CHECKED - SPK	REVISED -		SHEET S2-40 OF S2-47 SHEETS			VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	760
	PLOT DATE = 6/6/2023			DRAWN - TY	REVISED -					CONTRACT NO. 68C55				
				CHECKED - PK	REVISED -					ILLINOIS / FED. AID PROJECT				



BARS s919(E) & s920(E)



BARS $u900(E)$, $u901E$, & $u902(E)$



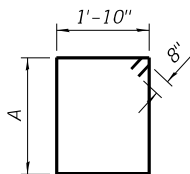
BARS p900(E), p902(E), p905(E), & p906(E)

A & B DIMENSIONS

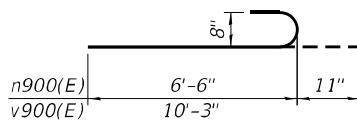
<i>Bar</i>	<i>A</i>	<i>B</i>
<i>u900(E)</i>	2'-6"	3'-10"
<i>u901(E)</i>	2'-8"	2'-9"
<i>u902(E)</i>	2'-6"	3'-4"

A DIMENSIONS

Bar	A
p900(E)	18'-10"
p902(E)	11'-2"
p905(E)	28'-1"
p906(E)	13'-1"



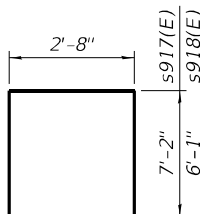
BARS s900(E) THRU s916(E)



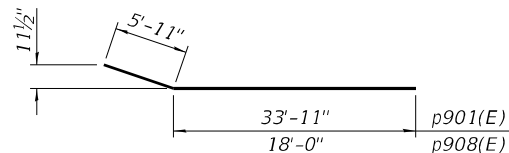
BARS n900(E) & v900(E)

A DIMENSIONS

Bar	A
s900(E)	1'-1-11"
s901(E)	2'-1-11"
s902(E)	2'-3-11"
s903(E)	2'-5-11"
s904(E)	2'-7-11"
s905(E)	2'-8-11"
s906(E)	2'-9-11"
s907(E)	2'-11-11"
s908(E)	3'-10-11"
s909(E)	4'-8-11"
s910(E)	5'-7-11"
s911(E)	5'-5-11"
s912(E)	5'-3-11"
s913(E)	5'-1-11"
s914(E)	4'-1-11"
s915(E)	4'-9-11"
s916(E)	4'-7-11"

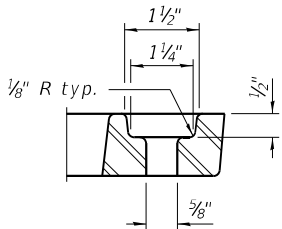
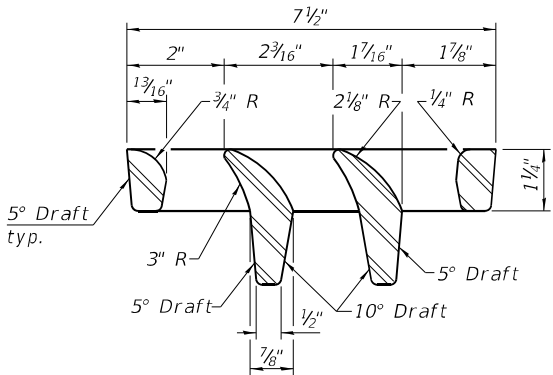
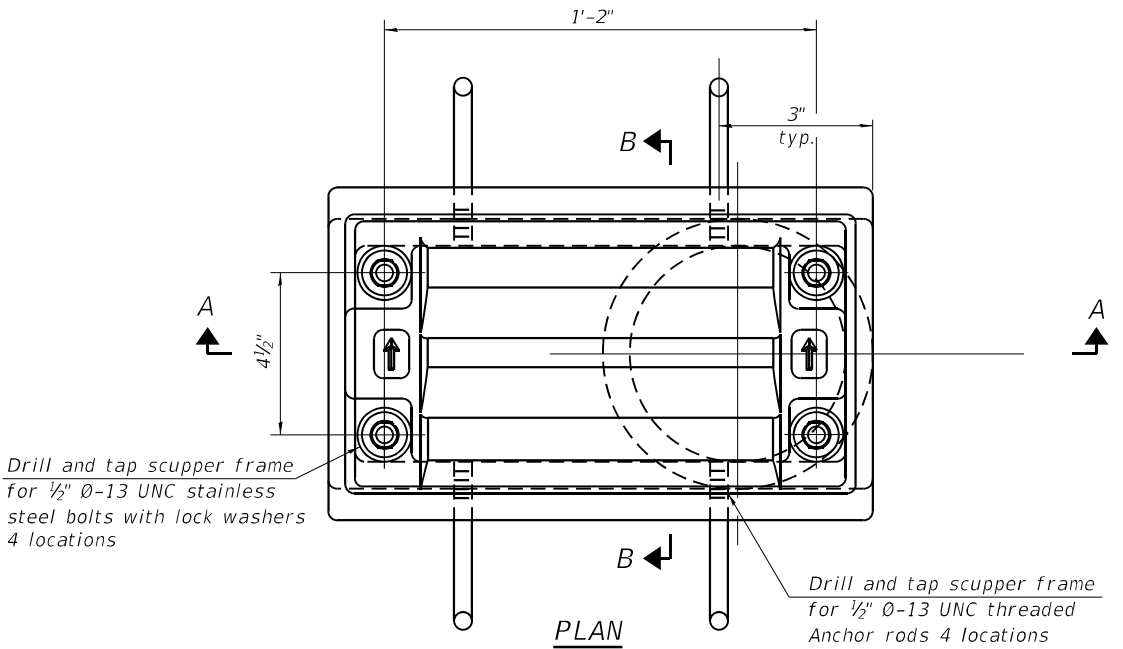


BARS s917(E) & s918(E)



BARS p901(E) & p908(E)

PIER 2				
BILL OF MATERIAL				
Bar	No.	Size	Length	Shape
h900(E)	2	#5	47'-5"	————
h901(E)	8	#5	30'-0"	————
h902(E)	2	#5	35'-9"	————
h903(E)	2	#5	28'-3"	————
h904(E)	2	#5	20'-9"	————
h905(E)	19	#6	45'-1"	————
n900(E)	72	#8	7'-5"	———┐
p900(E)	6	#9	20'-0"	———┐
p901(E)	6	#8	39'-10"	———┐
p902(E)	6	#9	12'-4"	———┐
p903(E)	12	#6	10'-5"	————
p904(E)	6	#9	20'-3"	————
p905(E)	6	#9	29'-3"	———┐
p906(E)	6	#9	14'-3"	———┐
p907(E)	4	#6	5'-7"	————
p908(E)	6	#8	23'-11"	———┐
s900(E)	2	#6	8'-10"	☐
s901(E)	2	#6	9'-2"	☐
s902(E)	2	#6	9'-6"	☐
s903(E)	2	#6	9'-10"	☐
s904(E)	2	#6	10'-2"	☐
s905(E)	2	#6	10'-4"	☐
s906(E)	2	#6	10'-6"	☐
s907(E)	20	#6	10'-10"	☐
s908(E)	30	#5	12'-9"	☐
s909(E)	30	#5	14'-5"	☐
s910(E)	16	#6	16'-2"	☐
s911(E)	2	#6	15'-10"	☐
s912(E)	2	#6	15'-6"	☐
s913(E)	2	#6	15'-2"	☐
s914(E)	2	#6	14'-10"	☐
s915(E)	2	#6	14'-6"	☐
s916(E)	2	#6	14'-2"	☐
s917(E)	91	#6	17'-0"	☐
s918(E)	91	#6	14'-10"	☐
s919(E)	48	#4	11'-10"	☐
s920(E)	40	#4	11'-2"	☐
t900(E)	68	#9	9'-8"	————
u900(E)	9	#6	10'-2"	☐
u901(E)	34	#5	8'-2"	☐
u902(E)	16	#6	9'-2"	☐
v900(E)	72	#8	11'-2"	———┐
w900(E)	18	#6	47'-1"	————
Structure Excavation			Cu. Yd.	138
Concrete Structures			Cu. Yd.	132.6
Reinforcement Bars, Epoxy Coated			Pound	19,820
Furnishing Steel Piles HP12x53			Foot	989
Driving Piles			Foot	989
Test Pile Steel HP12x53			Each	1
Pile Shoes			Each	24
Concrete Sealer			Sq. Ft.	1,374



Notes:

All cast iron parts shall be gray iron conforming to the requirements of AASHTO M105, Class 35B and AASHTO M306.

Bolts, anchor rods, nuts and washers shall be according to ASTM A307 and shall be galvanized according to AASHTO M232. As an alternate stainless steel may be used.

Stainless steel hardware shall be according to Article 1006.29(d) of the Standard Specifications.

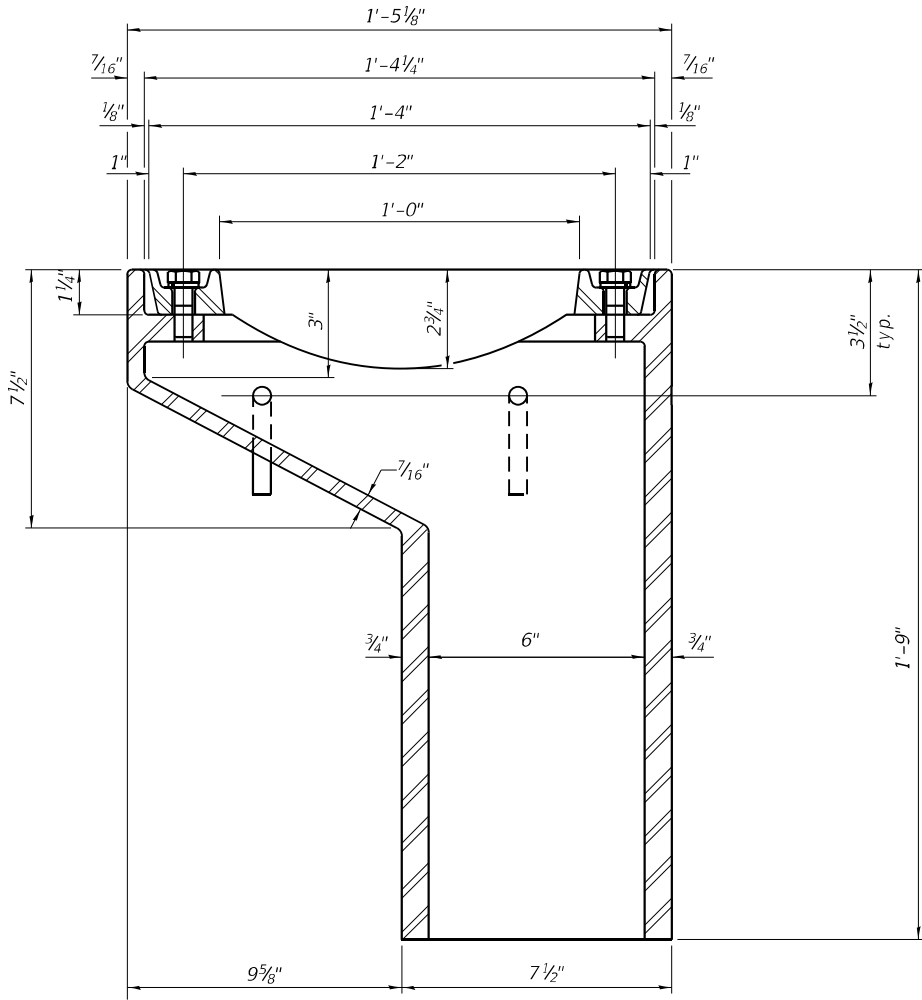
Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frames and downspouts; however, the scupper grates shall remain cast iron. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval.

Structural steel scupper frames and downspouts, when utilized, shall be galvanized according to AASHTO M111.

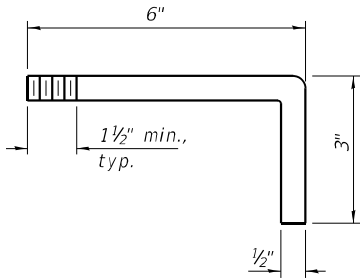
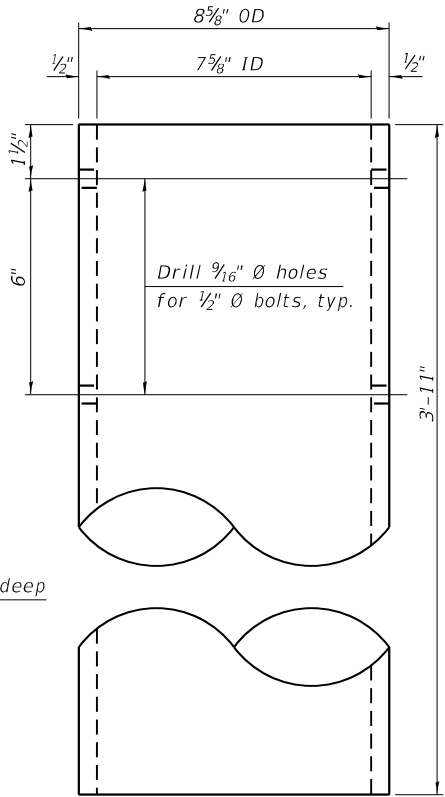
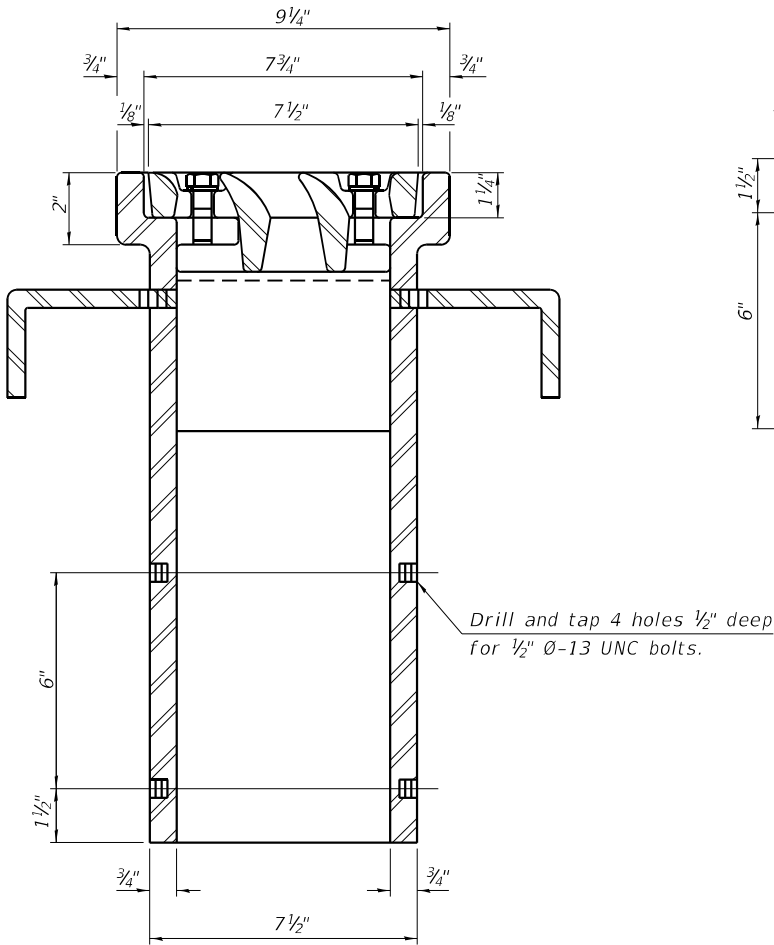
As an alternate, fiberglass may be used for downspouts according to ASTM D2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. in lieu of the cast iron or structural steel.

The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.

Cost of the grate, frame, downspout, anchor rods, nuts and washers including complete installation of the scupper shall be paid for at the contract unit price for Drainage Scupper, DS-11.



See sheet S2-15 for scupper location relative to parapet.



ANCHOR ROD DETAIL

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper, DS-11	Each	6

Michael Baker
INTERNATIONAL

DS-11

1-1-2020

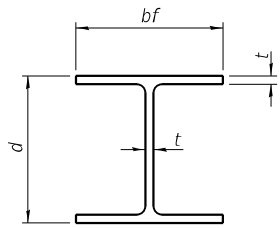
USER NAME = DIamb	DESIGNED - SPK	REVISED -
	CHECKED - PK	REVISED -
PLOT SCALE = 0.1667 ' / in.	DRAWN - CLL	REVISED -
PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DRAINAGE SCUPPER, DS-11
STRUCTURE NO. 090-0183

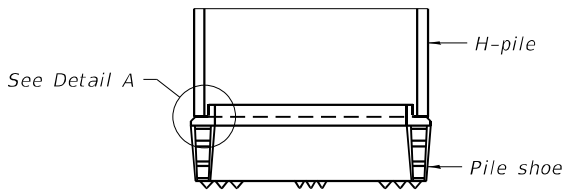
SHEET S2-42 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	762
CONTRACT NO. 68C55				
ILLINOIS		FED. AID PROJECT		

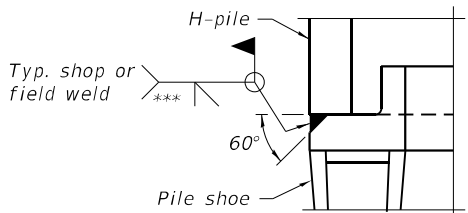


STEEL PILE TABLE

Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1 1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1 1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"

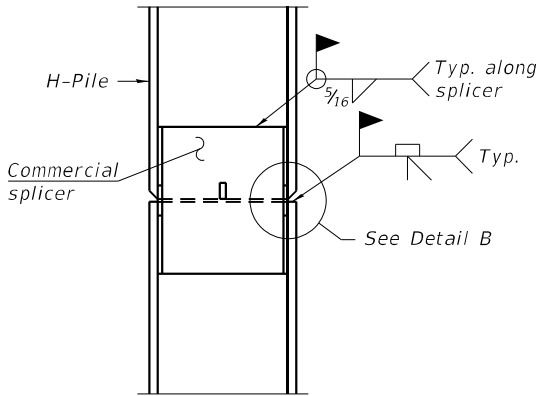


ELEVATION

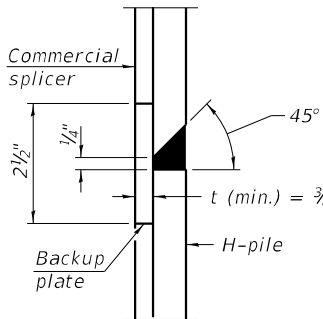


SHOE ATTACHMENT

Note:
The steel H-piles shall be according to
AASHTO M270 Grade 50.

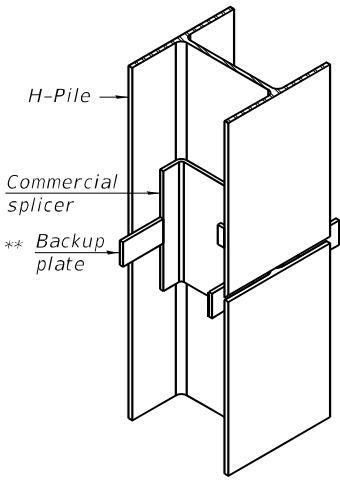


ELEVATION

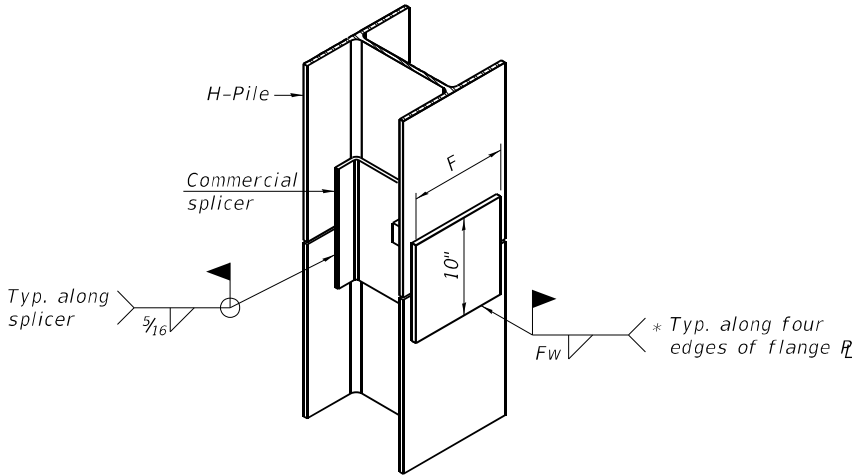


DETAIL "B"

WELDED COMMERCIAL SPLICE



ISOMETRIC VIEW



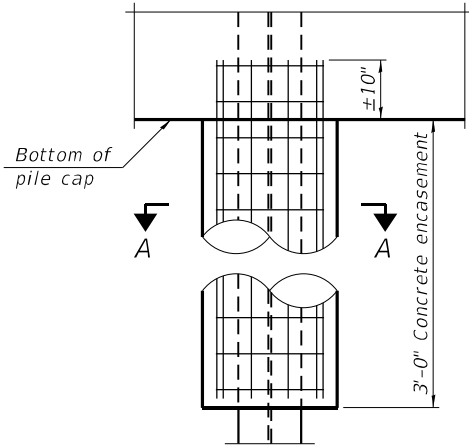
ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

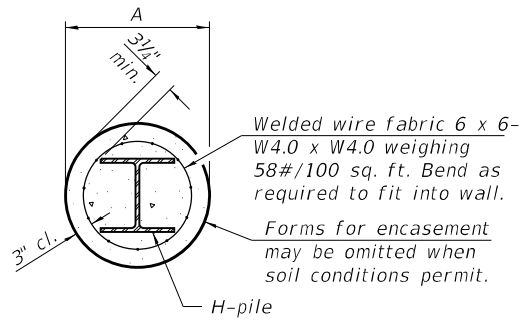
* Interrupt welds 1/4" from end of web and/or each flange.

** Remove portions of backup plates that extend outside the flanges.

*** Weld size per pile shoe manufacturer (5/16" min.).

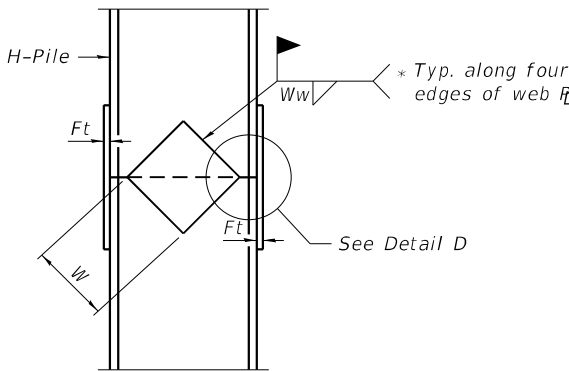


ELEVATION

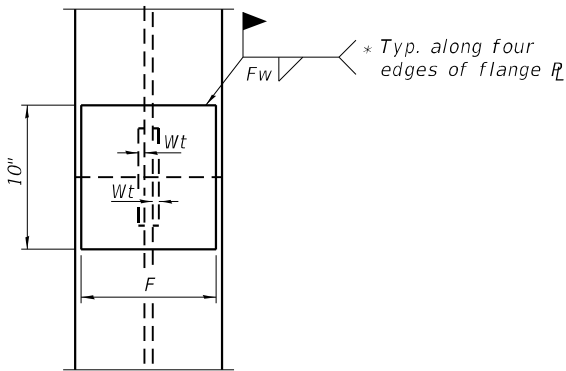


SECTION A-A

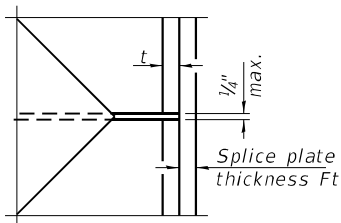
INDIVIDUAL PILE
CONCRETE ENCASEMENT
(at abutments and bents only)



ELEVATION



END VIEW



DETAIL D

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	1 1/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

WELDED PLATE FIELD SPLICE

F-HP

1-1-2020



USER NAME	= D1amb
PLOT SCALE	= 0.1667 ' / in.
PLOT DATE	= 6/6/2023

DESIGNED	- SPK
CHECKED	- PK
DRAWN	- CLL
CHECKED	- SPK

REVISED	-
REVISED	-
REVISED	-
REVISED	-

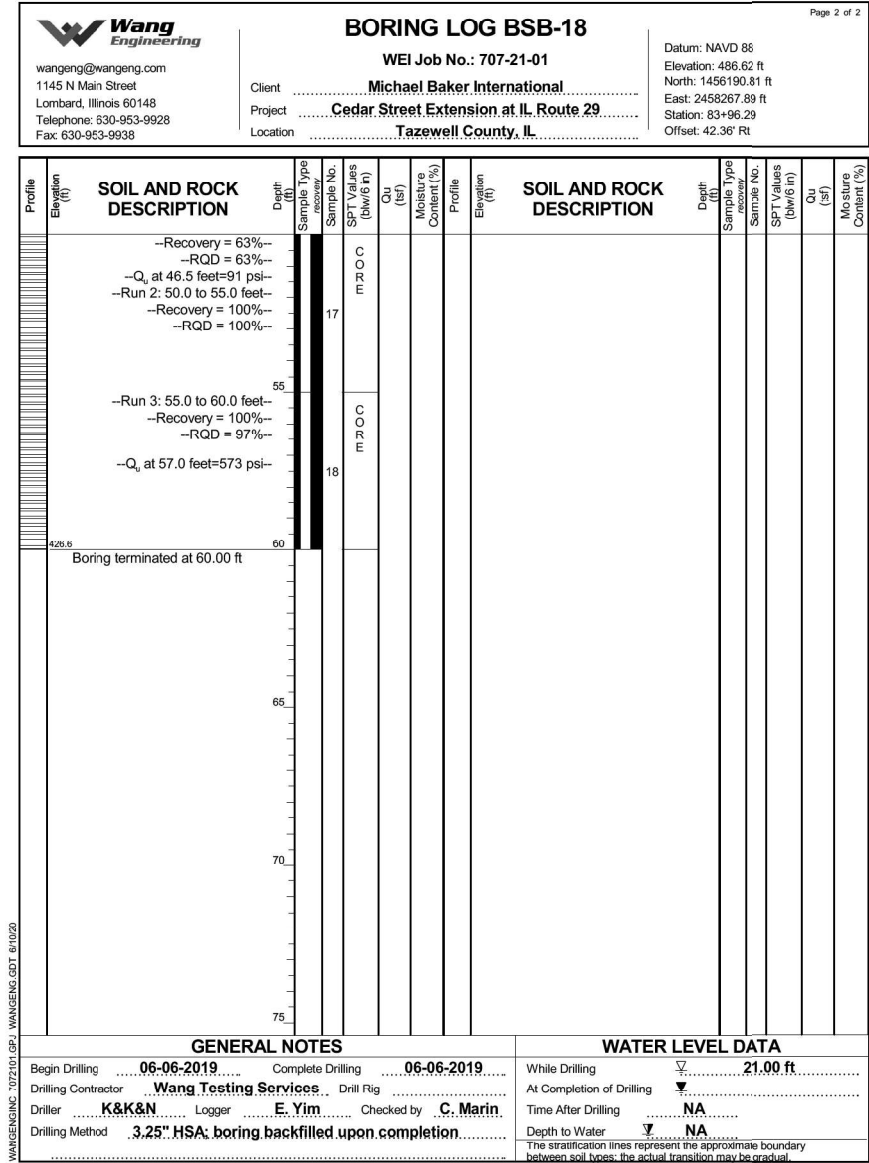
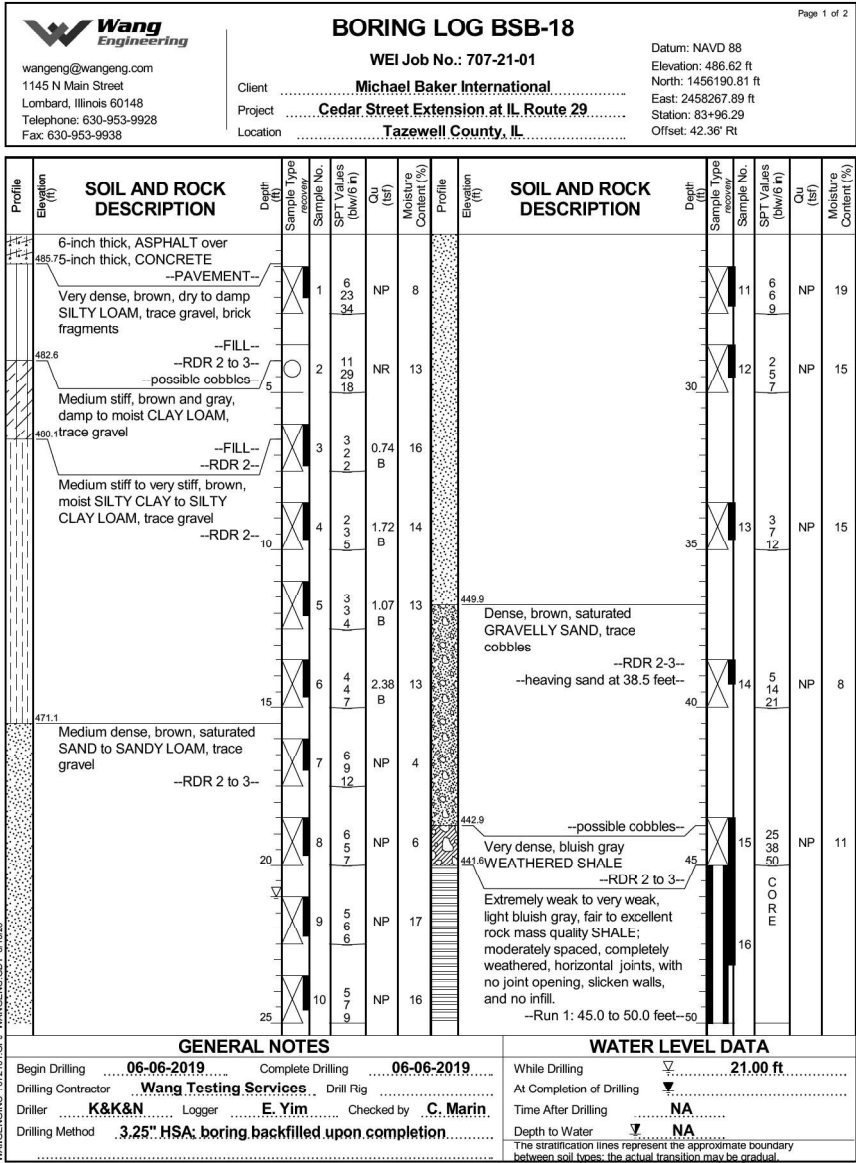
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HP PILE DETAILS
STRUCTURE NO. 090-0183

SHEET S2-43 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	763
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

MODEL: Default
FILE NAME: p:\wms\bus\sh\ben\c\comrb\bus-pw\03\Documents\Chicago IL\011 Project\179740 IDOT CedarStExt PH115.0 CADD\CAD Sheets\Main St, Bridge Sheets\09001B3-48C55-045-Bor3.dgn



USER NAME = DLamb	DESIGNED - WANG	REVISED -
	CHECKED - WANG	REVISED -
PLOT SCALE = 0.1667' / in.	DRAWN - MD	REVISED -
PLOT DATE = 6/6/2023	CHECKED - SPK	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOG BSB-18
STRUCTURE NO. 090-0183

SHEET S2-46 OF S2-47 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	766
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

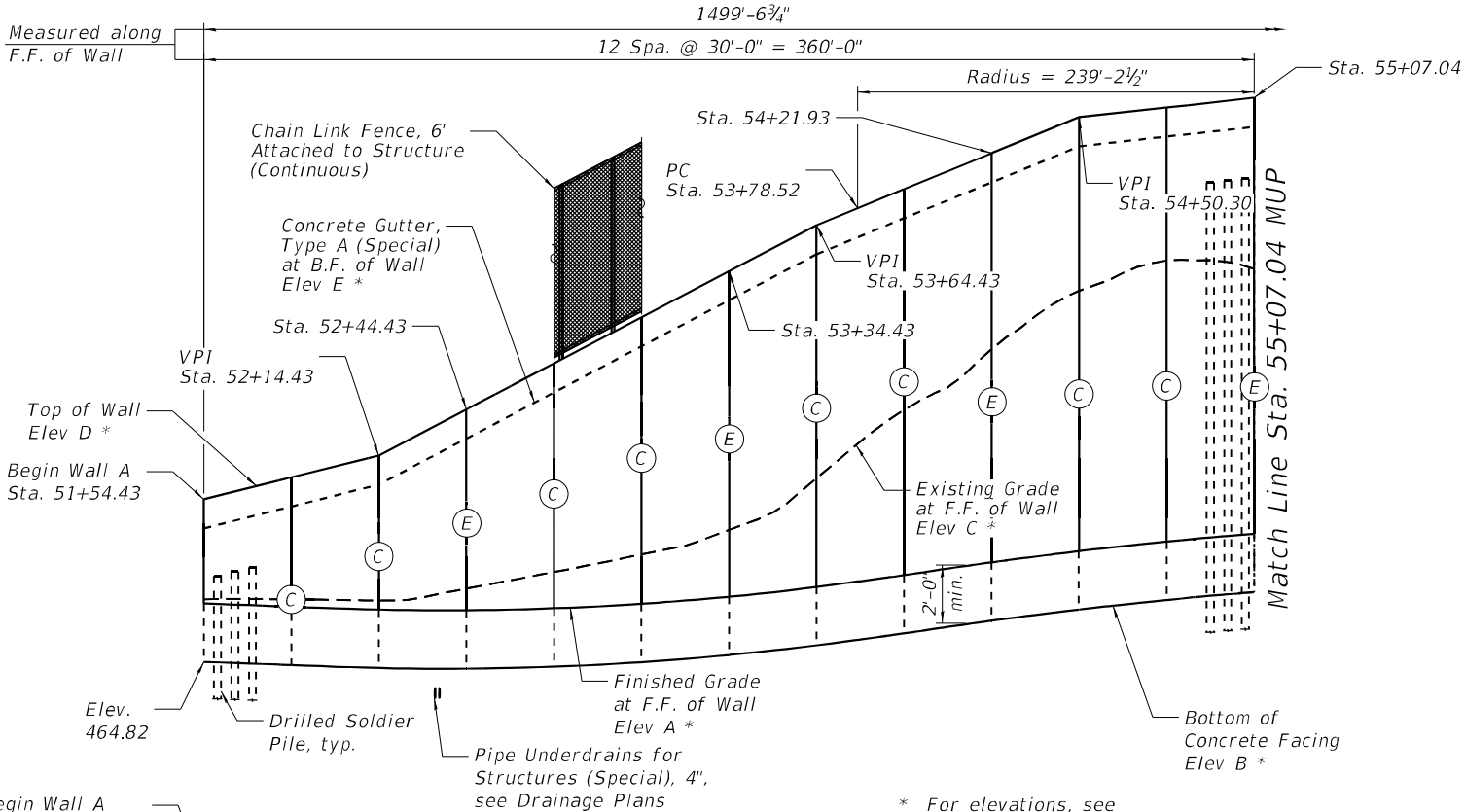
Bench Mark: Chiseled "□" on top of the SE Wingwall of the main span of the Cedar St. Bridge over the railyard. Elev. 505.64

Existing structure: None

Traffic will be closed on Cedar St. mainline, and the Cedar St. ramps during construction of the Cedar St. Bridge and Retaining Wall A.

NOTES:

- 1. Stations and offsets are measured along Prop. Multi-Use Path to the front face of wall.
- 2. F.F. denotes Front Face.
- 3. B.F. denotes Back Face.

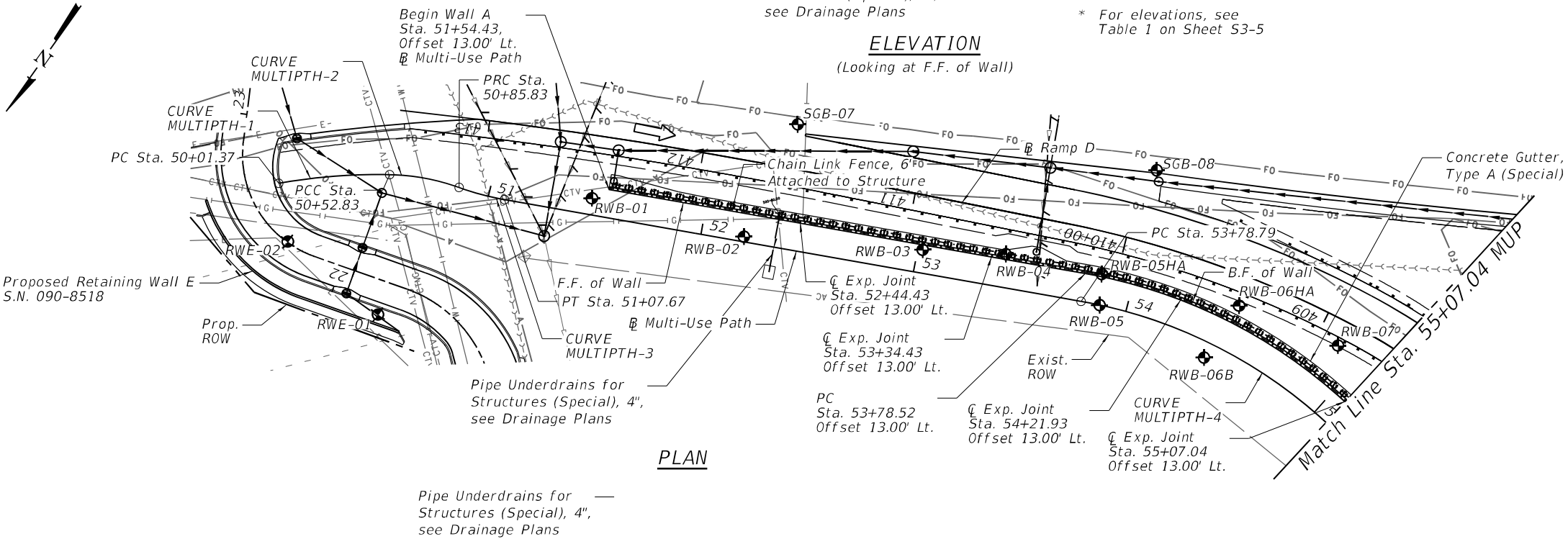
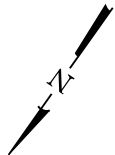


HORIZONTAL CURVE DATA

(Prop. Curve Multi-Use Path)

CURVE MULTIPTH-1	CURVE MULTIPTH-2
PI STA. = 50+27.26	PI STA. = 50+69.55
Δ = 11° 04' 11" (RT)	Δ = 18° 49' 13" (RT)
D = 21° 26' 55"	D = 57° 17' 45"
R = 267.13'	R = 100.00'
T = 25.89'	T = 16.57'
L = 51.61'	L = 32.85'
E = 1.25'	E = 1.36'
P.C. STA. = 50+01.37	P.C. STA. = 50+52.98
P.T. STA. = 50+52.98	P.T. STA. = 50+85.83

CURVE MULTIPTH-3	CURVE MULTIPTH-4
PI STA. = 50+96.78	PI STA. = 54+75.12
Δ = 9° 52' 13" (LT)	Δ = 46° 07' 51" (RT)
D = 45° 11' 27"	D = 25° 19' 43"
R = 126.79'	R = 226.21'
T = 10.95'	T = 96.33'
L = 21.84'	L = 182.13'
E = 0.47'	E = 19.65'
P.C. STA. = 50+85.83	P.C. STA. = 53+78.79
P.T. STA. = 51+07.67	P.T. STA. = 55+60.92



LEGEND

- ⊙ Construction Joint
- ⊕ Expansion Joint
- ⬢ Soil Boring Location
- T — Existing Underground Telephone
- A — Existing Aerial Electric Line
- E — Existing Underground Electric
- CTV — Existing Underground Cable TV
- ⊙ Existing Drainage Inlet
- Existing Storm Sewer
- Existing Sanitary Sewer
- Existing Water Main
- FO — Existing Underground Fiber Optic
- Guardrail
- Chain Link Fence, 6'
- ⊙ Proposed Drainage Inlet
- Proposed Storm Sewer
- ← Direction of Traffic
- Existing Sewer Removal

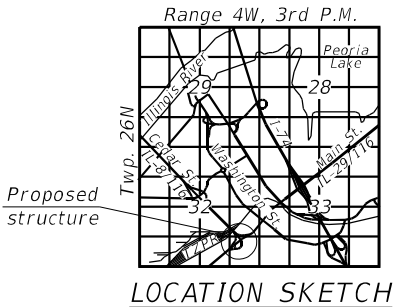
DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications, 9th Edition

DESIGN STRESSES

FIELD UNITS

f'c = 3,500 psi
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (M270 Grade 50) (Soldier Piles)



GENERAL PLAN

RETAINING WALL A ALONG MULTI-USE PATH
F.A.P. 669 - SECTION 13(HVB, HB-3) BR
TAZEWELL COUNTY
STATION 51+54.43 TO 66+64.32
STRUCTURE NO. 090-8517

APPROVED
For Structural Adequacy Only
Mary Louise Kutska
Engineer of Bridges & Structures



SIGNED: Mary Louise Kutska
Mary Louise Kutska
Il. Lic. No. 081-008008
Expires 11-30-2024
DATE: 6/8/2023

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL PLAN AND ELEVATION 1
STRUCTURE NO. 090-8517

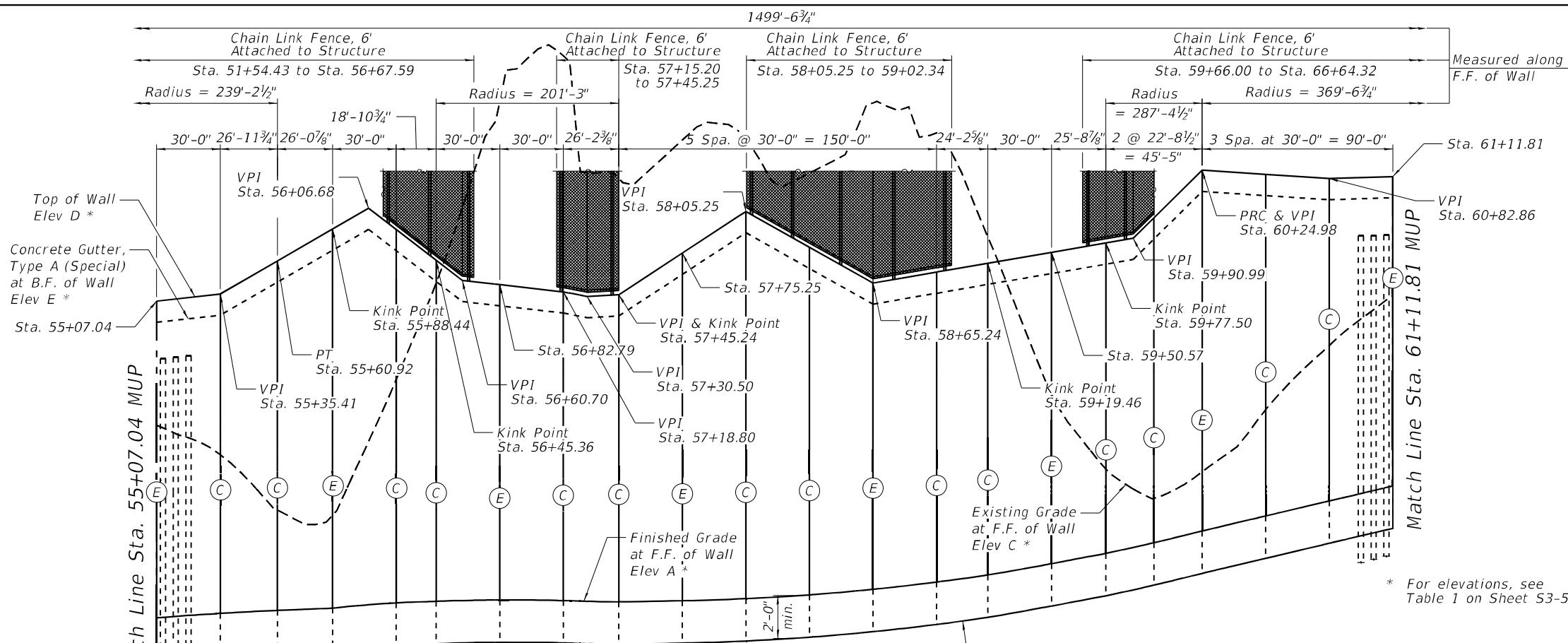
SHEET S3-1 OF 53-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
669	13 (HVB, HB-3) BR	TAZEWELL	1173	768
CONTRACT NO. 68C55				

ILLINOIS FED. AID PROJECT

Michael Baker International
200 W Adams Street
Suite 1500
Chicago, IL 60606
PH: 312.707.8000
FAX: 312.707.8804

USER NAME = DLamb	DESIGNED - YC	REVISED -
CHECKED - MLK	REVISOR -	
PLOT SCALE = 63.9997' / in.	DRAWN - EML	REVISED -
PLOT DATE = 5/8/2023	CHECKED - JMK	REVISED -



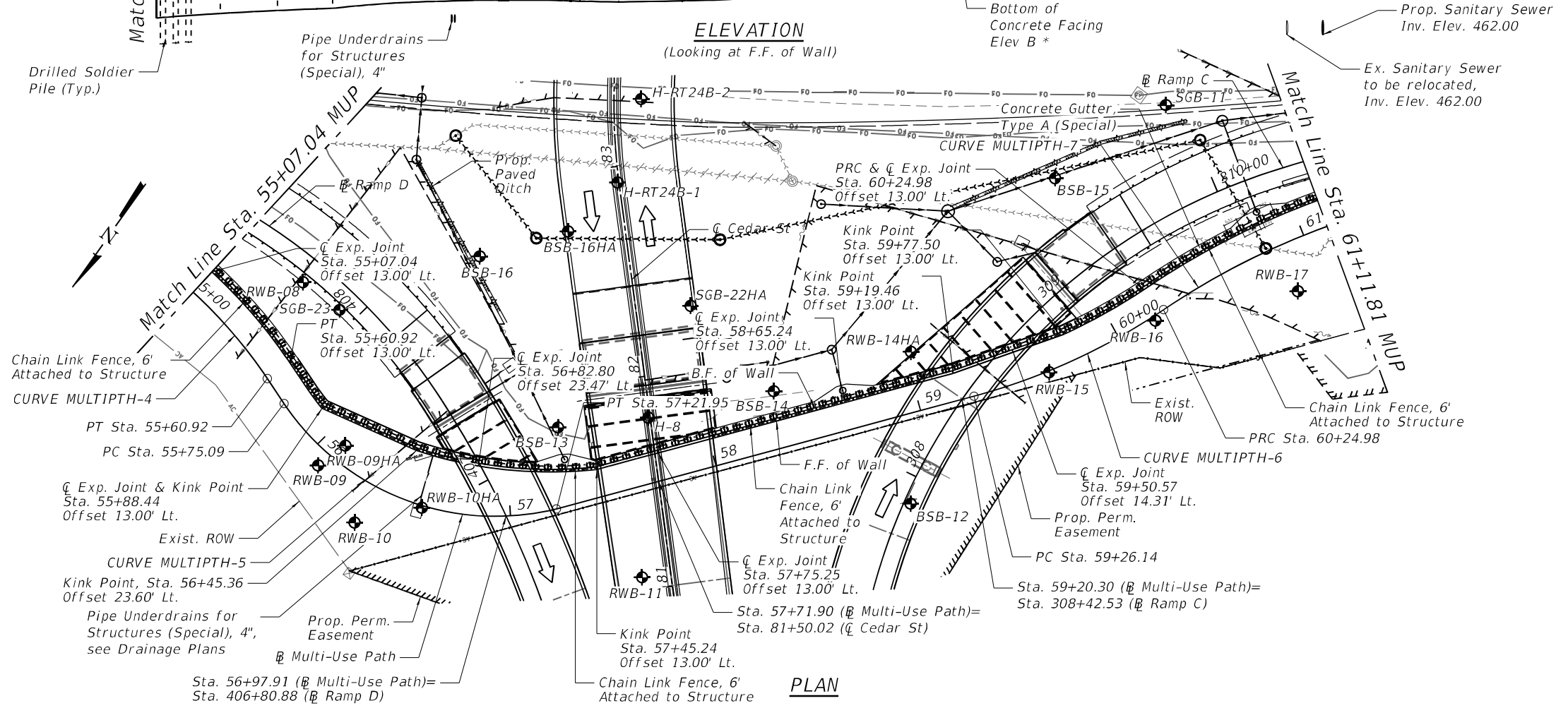
HORIZONTAL CURVE DATA
(Prop. Curve Multi-Use Path)

CURVE MULTIPTH-4 PI STA. = 54+75.12 Δ = 46° 07' 51" (RT) D = 25° 19' 43" R = 226.21' T = 96.33' L = 182.13' E = 19.65' P.C. STA. = 53+78.79 P.T. STA. = 55+60.92	CURVE MULTIPTH-5 PI STA. = 56+59.29 Δ = 70° 04' 51" (LT) D = 47° 43' 07" R = 120.07' T = 84.20' L = 146.86' E = 26.58' P.C. STA. = 55+75.09 P.T. STA. = 57+21.95
CURVE MULTIPTH-6 PI STA. = 59+76.01 Δ = 18° 52' 32" (LT) D = 19° 05' 55" R = 300.00' T = 49.87' L = 98.83' E = 4.12' P.C. STA. = 59+26.14 P.T. STA. = 60+24.98	CURVE MULTIPTH-7 PI STA. = 61+22.07 Δ = 30° 27' 55" (RT) D = 16° 04' 07" R = 356.57' T = 97.09' L = 189.59' E = 12.98' P.C. STA. = 60+24.98 P.T. STA. = 62+14.57

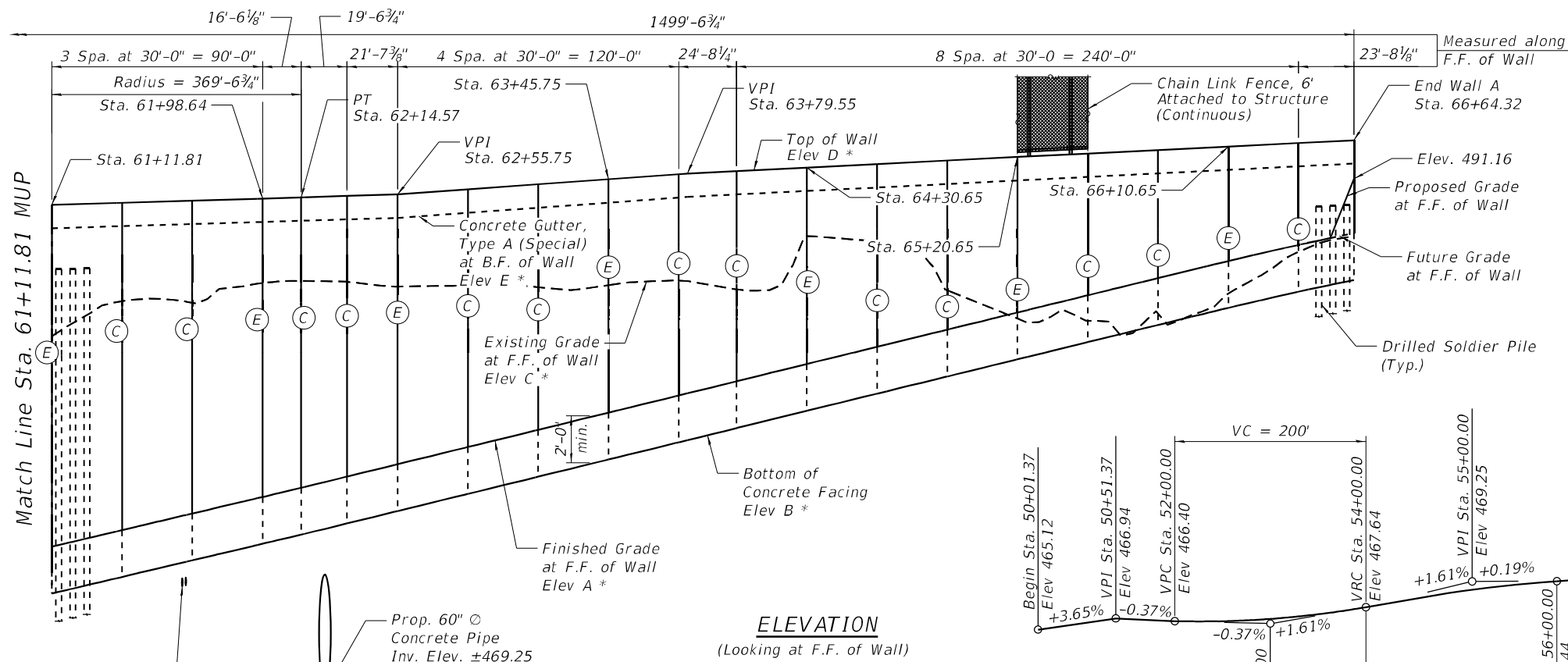
* For elevations, see Table 1 on Sheet S3-5

LEGEND

- (C) Construction Joint
- (E) Expansion Joint
- Soil Boring Location
- Existing Underground Telephone
- Existing Aerial Electric Line
- Existing Underground Electric
- Existing Underground Cable TV
- Existing Drainage Inlet
- Existing Storm Sewer
- Existing Sanitary Sewer
- Existing Water Main
- Existing Underground Fiber Optic
- Guardrail
- Chain Link Fence, 6'
- Proposed Drainage Inlet
- Proposed Storm Sewer
- Direction of Traffic
- Existing Sewer Removal



GENERAL PLAN
RETAINING WALL A ALONG MULTI-USE PATH
F.A.P. 669 - SECTION 13(HVB, HB-3) BR
TAZEWELL COUNTY
STATION 51+54.43 TO 66+64.32
STRUCTURE NO. 090-8517

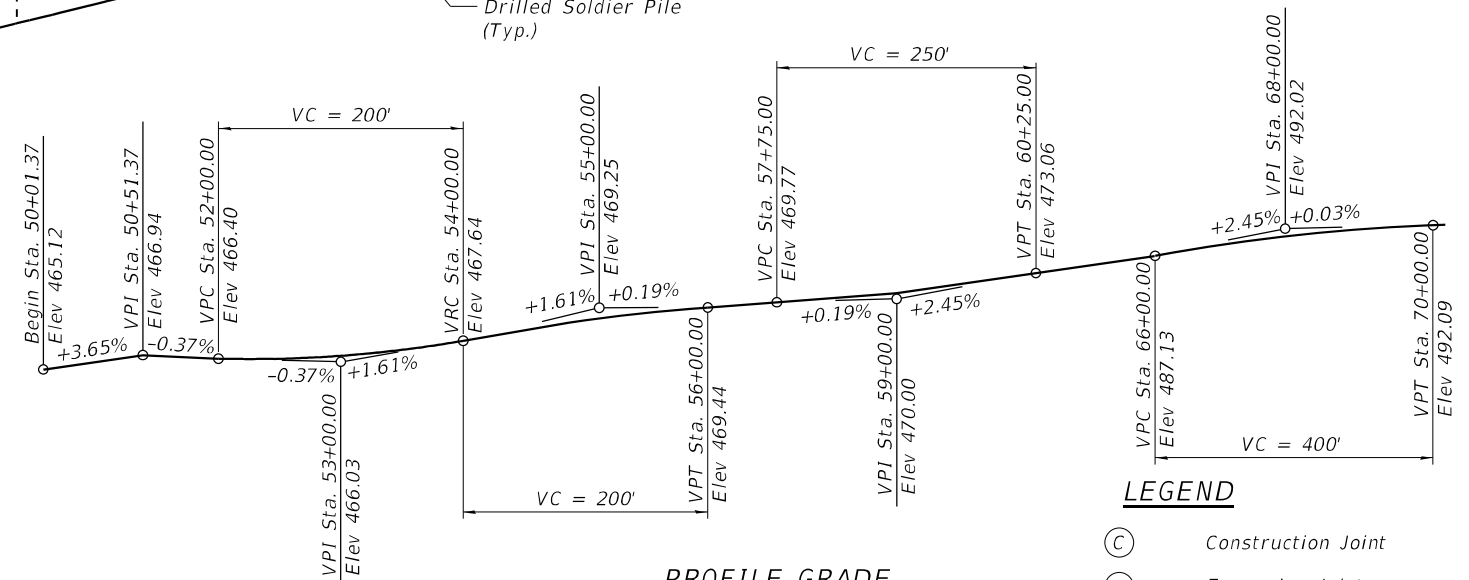


HORIZONTAL CURVE DATA

(Prop. Curve Multi-Use Path)

CURVE MULTIPTH-7
PI STA. = 61+22.07
 $\Delta = 30^\circ 27' 55''$ (RT)
D = 16° 04' 07"
R = 356.57'
T = 97.09'
L = 189.59'
E = 12.98'
P.C. STA. = 60+24.98
P.T. STA. = 62+14.57

* For elevations see
Table 1 on Sheet S3-5



PROFILE GRADE

(along Multi-Use Path)

LEGEND

- (C) Construction Joint
- (E) Expansion Joint
- Soil Boring Location
- T- Existing Underground Telephone
- A- Existing Aerial Electric Line
- E- Existing Underground Electric
- CTV- Existing Underground Cable TV
- Existing Drainage Inlet
- Existing Storm Sewer
- Existing Sanitary Sewer
- Existing Water Main
- FO- Existing Underground Fiber Optic
- Guardrail
- Chain Link Fence, 6'
- Proposed Drainage Inlet
- Proposed Storm Sewer
- Direction of Traffic
- Existing Sewer Removal

GENERAL PLAN

RETAINING WALL A ALONG MULTI-USE PATH

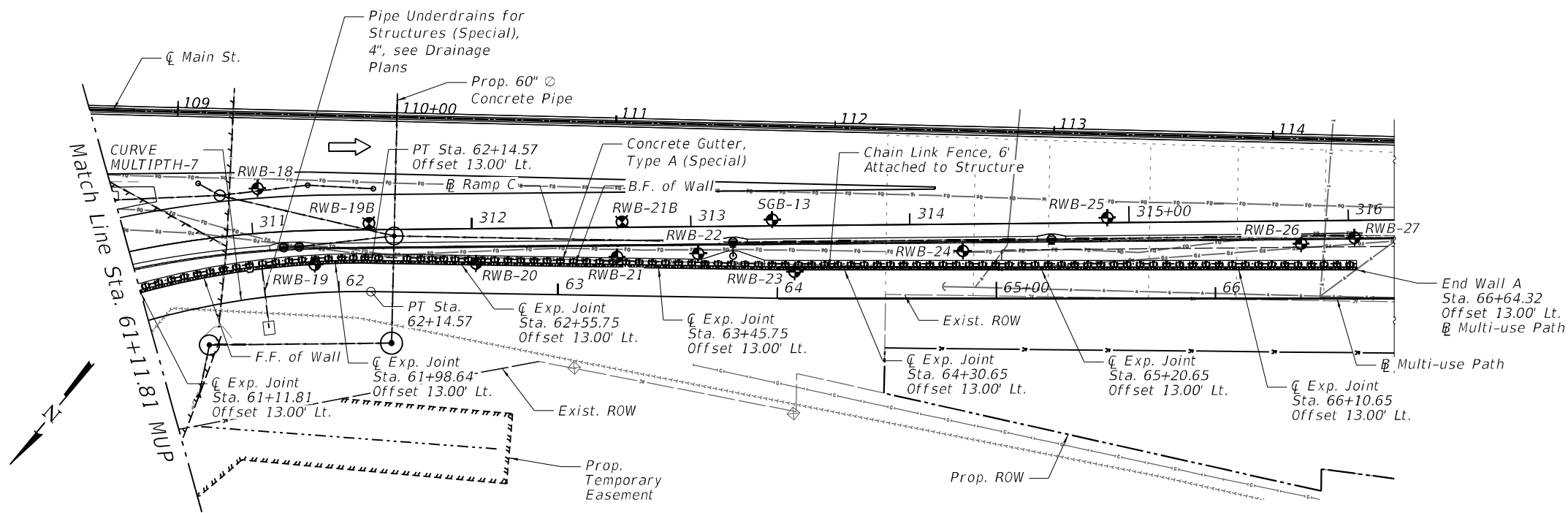
F.A.P. 669 - SECTION 13(HVB, HB-3) BR

TAZEWELL COUNTY

STATION 51+54.43 TO 66+64.32

STRUCTURE NO. 090-8517

PLAN



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL PLAN AND ELEVATION 3
STRUCTURE NO. 090-8517

SHEET S3-3 OF 53-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
669	13 (HVB, HB-3) BR	TAZEWELL	1173	770
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

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Michael Baker
INTERNATIONAL

USER NAME = D1amb	DESIGNED - YC	REVISED -
PLOT SCALE = 63.9997' / in.	CHECKED - MLK	REVISED -
PLOT DATE = 6/6/2023	DRAWN - EML	REVISED -
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GENERAL NOTES

1. Reinforcement bars designated "(E)" shall be epoxy coated.
2. Anti-Graffiti Coating shall be applied to exposed surfaces of the facing.
3. Existing utilities in conflict with retaining wall construction shall be protected, abandoned, or relocated according to Existing Drainage and Utility Plans. The Contractor shall field verify utilities to be protected to ensure they do not conflict with proposed construction and make necessary approved adjustments prior to construction or ordering of materials. Piles shall be located at a distance of at least 2 times the diameter of utility pipe away from the utility, measured from edge of pile to edge of utility pipe.
4. The Contractor shall provide a method to assure the soldier piles achieve at least the plan tip elevations. The soldier pile locations and elevations shall meet the tolerances provided in the Standard Specifications. Any additional measures required to satisfy the construction tolerances will not be paid for separately but shall be included in Drilling and Setting Soldier Piles (In Soil) or Drilling and Setting Soldier Piles (In Rock).
5. The Contractor is responsible for the design and performance of the untreated timber lagging using no less than a 3 in. nominal rough-sawn thickness and timber with a minimum allowable bending stress of 1000 psi.
6. Contractor shall coordinate the timing of the installation of soldier piles located beneath the Cedar Street over TZPR Bridge (S.N. 090-0182) with the bridge construction.
7. Wall is permitted to be built as straight chords between construction joints.
8. Surface treatment shall be continuous across construction and expansion joints. See Special Provisions.
9. The concrete facing shall not be poured before all grading work behind and above the wall is complete.
10. The coarse aggregate for Pipe Underdrains for Structures shall be CA-7 or CA-11 gradation.

INDEX OF SHEETS

Sheet No.	Sheet Title
1	General Plan and Elevation 1
2	General Plan and Elevation 2
3	General Plan and Elevation 3
4	General Notes, Index of Sheets, Total Bill of Material
5	Wall Elevations
6	Detailed Plan and Elevation 1
7	Detailed Plan and Elevation 2
8	Detailed Plan and Elevation 3
9	Detailed Plan and Elevation 4
10	Detailed Plan and Elevation 5
11	Detailed Plan and Elevation 6
12	Detailed Plan and Elevation 7
13	Detailed Plan and Elevation 8
14	Detailed Plan and Elevation 9
15	Detailed Plan and Elevation 10
16	Detailed Plan and Elevation 11
17	Wall A Bill of Material
18	Pile Schedule 1
19	Pile Schedule 2
20	Pile Schedule 3
21	Cross Sections and Details 1
22	Cross Sections and Details 2
23	Pile Details
24	Fence Details
25	Soil Borings 1
26	Soil Borings 2
27	Soil Borings 3
28	Soil Borings 4
29	Soil Borings 5
30	Soil Borings 6
31	Soil Borings 7
32	Soil Borings 8
33	Soil Borings 9
34	Soil Borings 10
35	Soil Borings 11
36	Soil Borings 12
37	Soil Borings 13
38	Soil Borings 14

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	1,788
Concrete Structures	Cu. Yd.	983.3
Stud Shear Connectors	Each	4,540
Reinforcement Bars, Epoxy Coated	Pound	111,330
Name Plates	Each	1
Furnishing Soldier Piles (W Section)	Foot	9,504
Drilling and Setting Soldier Piles (In Soil)	Cu. Ft.	41,340
Drilling and Setting Soldier Piles (In Rock)	Cu. Ft.	1,560
Untreated Timber Lagging	Sq. Ft.	15,626
Geocomposite Wall Drain	Sq. Yd.	1,095
Pipe Underdrains for Structres 4"	Foot	1,504
Anti-Graffiti Coating	Sq. Ft.	20,086
Chain Link Fence, 6' Attached to Structure	Foot	1,350

INFRASTRUCTURE
ENGINEERING

USER NAME = DLamb	DESIGNED - TY	REVISED -
	CHECKED - KS	REVISED -
PLOT SCALE = 0.1667 ' / in.	DRAWN - TY	REVISED -
PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

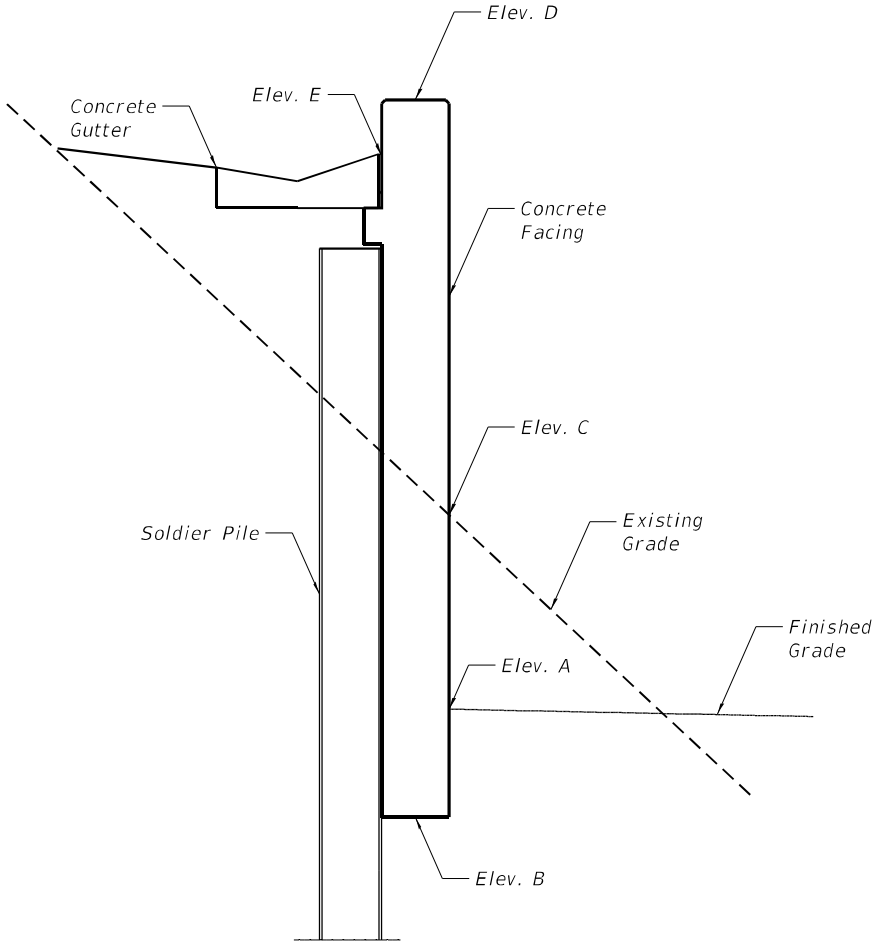
GENERAL NOTES, INDEX OF SHEETS, TOTAL BILL OF MATERIAL
STRUCTURE NO. 090-8517

SHEET S3-4 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	771
CONTRACT NO. 68C55				
		ILLINOIS	FED. AID PROJECT	

WALL ELEVATION TABLE

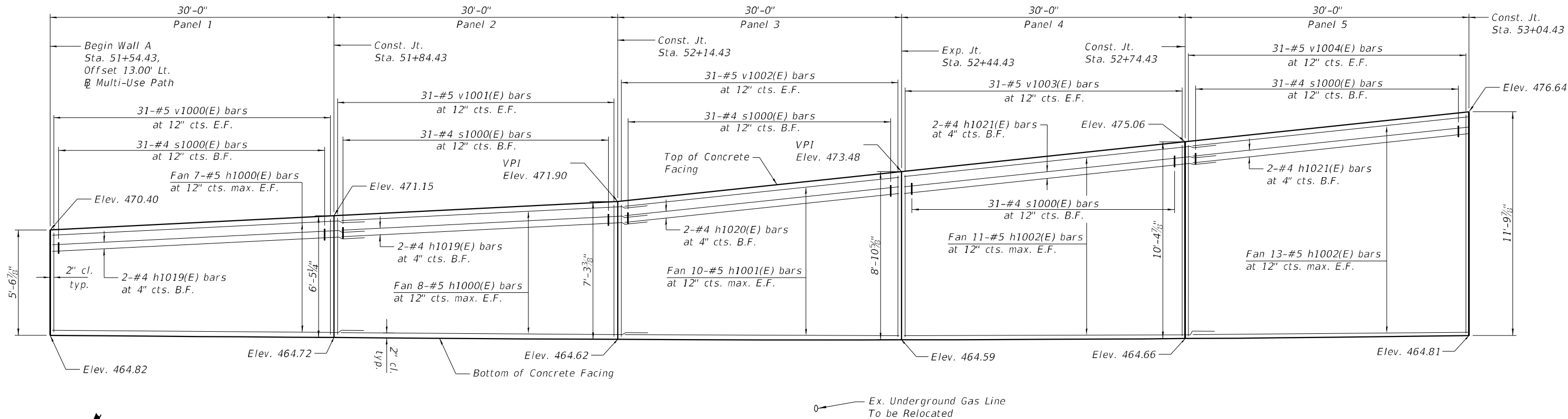
Station	Location	Offset	Elevation A	Elevation B	Elevation C	Elevation D	Elevation E
51+54.43	Beginning of Wall A	13.00'	466.82	464.82	466.97	470.40	469.40
51+84.43	Constr. Joint	13.00'	466.72	464.72	466.95	471.15	470.15
52+14.43	Constr. Joint/VPI	13.00'	466.62	464.62	466.93	471.90	470.90
52+44.43	Exp. Joint/VPI	13.00'	466.59	464.59	467.33	473.48	472.48
52+74.43	Constr. Joint	13.00'	466.66	464.66	467.91	475.06	474.06
53+04.43	Constr. Joint	13.00'	466.81	464.81	468.51	476.64	475.64
53+34.43	Exp. Joint/VPI	13.00'	467.06	465.06	469.35	478.22	477.22
53+64.43	Constr. Joint/VPI	13.00'	467.39	465.39	471.12	479.80	478.80
53+93.56	Constr. Joint/VPI	13.00'	467.80	465.80	473.46	481.05	480.05
54+21.93	Exp. Joint/VPI	13.00'	468.24	466.24	475.55	482.27	481.27
54+50.30	Constr. Joint/VPI	13.00'	468.62	466.62	477.55	483.50	482.50
54+78.67	Constr. Joint	13.00'	468.95	466.95	478.58	483.83	482.83
55+07.04	Exp. Joint/VPI	13.00'	469.22	467.22	478.30	484.17	483.17
55+35.41	Constr. Joint/VPI	13.00'	469.43	467.43	476.92	484.50	483.50
55+60.92	Constr. Joint/VPI	13.00'	469.57	467.57	474.31	486.06	485.06
55+88.45	Exp. Joint/VPI	13.00'	469.68	467.68	474.07	487.57	486.57
56+06.68	VPI	13.00'	469.83	467.83	477.54	488.54	487.54
56+22.04	Constr. Joint	22.23'	469.92	467.92	480.47	487.54	486.54
56+45.37	Constr. Joint/VPI	23.61'	470.00	468.00	485.02	486.09	485.09
56+60.70	VPI	24.14'	470.03	468.03	488.56	485.14	484.14
56+82.79	Exp. Joint/VPI	23.47'	470.06	468.06	493.60	484.94	483.94
57+18.80	Constr. Joint/VPI	18.61'	470.03	468.03	495.74	484.60	483.60
57+30.50	VPI	13.00'	470.00	468.00	490.21	484.38	483.38
57+45.25	Constr. Joint/VPI	13.00'	469.97	467.97	489.95	484.46	483.46
57+75.25	Exp. Joint/VPI	13.00'	470.03	468.03	491.76	486.43	485.43
58+05.25	Constr. Joint/VPI	13.00'	470.12	468.12	491.80	488.39	487.39
58+35.25	Constr. Joint	13.00'	470.30	468.30	490.21	486.70	485.70
58+65.24	Exp. Joint/VPI	13.00'	470.56	468.56	493.52	485.02	484.02
58+95.25	Constr. Joint	13.00'	470.91	468.91	492.10	485.53	484.53
59+19.46	Constr. Joint/VPI	13.00'	471.24	469.24	487.16	485.95	484.95
59+50.57	Exp. Joint/VPI	14.31'	471.77	469.77	479.80	486.47	485.47
59+77.50	Constr. Joint/VPI	13.00'	472.26	470.26	476.24	486.91	485.91
59+90.99	VPI	13.00'	472.54	470.54	475.30	487.13	486.13
60+01.24	Constr. Joint	13.00'	472.76	470.76	474.86	488.10	487.10
60+24.98	Exp. Joint/VPI	13.00'	473.32	471.32	476.11	490.34	489.34
60+53.92	Constr. Joint	13.00'	474.03	472.03	479.04	490.14	489.14
60+82.86	Constr. Joint/VPI	13.00'	474.73	472.73	482.07	489.95	488.95
61+11.81	Exp. Joint/VPI	13.00'	475.44	473.44	484.43	490.04	489.04
61+40.75	Constr. Joint	13.00'	476.15	474.15	485.93	490.13	489.13
61+69.70	Constr. Joint	13.00'	476.86	474.86	485.85	490.22	489.22
61+98.64	Exp. Joint/VPI	13.00'	477.57	475.57	486.70	490.31	489.31
62+14.57	Constr. Joint/VPI	13.00'	477.96	475.96	486.80	490.36	489.36
62+34.13	Constr. Joint	13.00'	478.44	476.44	486.76	490.42	489.42
62+55.75	Exp. Joint/VPI	13.00'	478.96	476.96	486.55	490.49	489.49
62+85.75	Constr. Joint	13.00'	479.70	477.70	486.60	490.70	489.70
63+15.75	Constr. Joint	13.00'	480.43	478.43	486.52	490.92	489.92
63+45.75	Exp. Joint/VPI	13.00'	481.17	479.17	486.61	491.14	490.14
63+75.75	Constr. Joint	13.00'	481.90	479.90	486.81	491.35	490.35
63+79.55	VPI	13.00'	481.99	479.99	486.79	491.38	490.38
64+00.44	Constr. Joint	13.00'	482.50	480.50	486.43	491.48	490.48
64+30.65	Exp. Joint/VPI	13.00'	483.24	481.24	488.71	491.63	490.63
64+60.65	Constr. Joint	13.00'	483.98	481.98	488.48	491.78	490.78
64+90.65	Constr. Joint	13.00'	484.71	482.71	486.39	491.93	490.93
65+20.65	Exp. Joint/VPI	13.00'	485.45	483.45	485.18	492.08	491.08
65+50.65	Constr. Joint	13.00'	486.18	484.18	485.09	492.23	491.23
65+80.65	Constr. Joint	13.00'	486.91	484.91	485.40	492.38	491.38
66+10.65	Exp. Joint/VPI	13.00'	487.64	485.64	486.26	492.53	491.53
66+40.65	Constr. Joint	13.00'	488.33	486.33	488.04	492.68	491.68
66+64.32	End of Wall A	13.00'	488.84	486.84	488.71	492.80	491.80



TYPICAL SECTION FOR ELEVATIONS

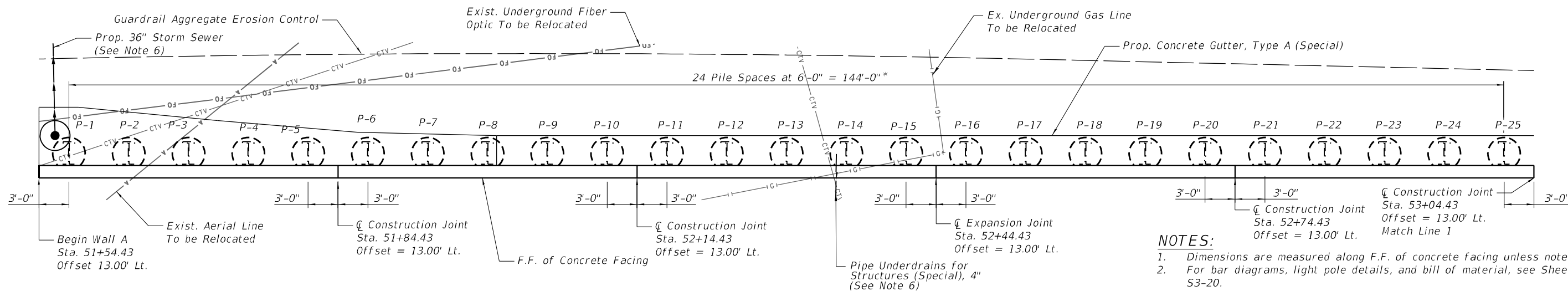
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Elevation B - Bottom of Concrete Facing
Elevation C - Existing Grade at Front Face of Wall
Elevation D - Top of Wall
Elevation E - Concrete Gutter at Back Face of Wall

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PARTIAL ELEVATION

MIN. BAR LAP
#4 Bars = 2'-11"
#5 Bars = 3'-7"



PARTIAL PLAN

- NOTES:
1. Dimensions are measured along F.F. of concrete facing unless noted otherwise.
 2. For bar diagrams, light pole details, and bill of material, see Sheet S3-17 and S3-20.
 3. Longitudinal bars are to be provided straight and are to be sprung and tied at required radius in the field.
 4. All edges shall be chamfered $\frac{3}{4}$ ".
 5. For Fence Details, see Sheet S3-24.
 6. See Proposed Drainage Plans for drainage pipe, drainage structure, and pipe underdrain connection sizes, stations, and offsets.

LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.

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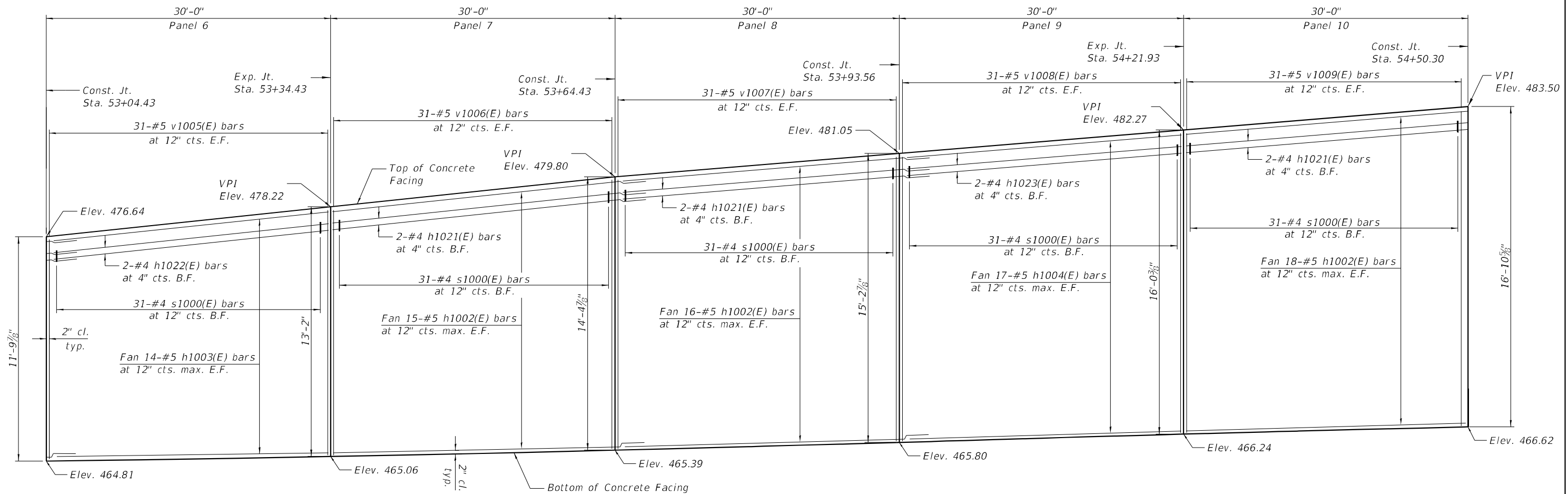
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PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 1
STRUCTURE NO. 090-8517

SHEET S3-6 OF S3-38 SHEETS

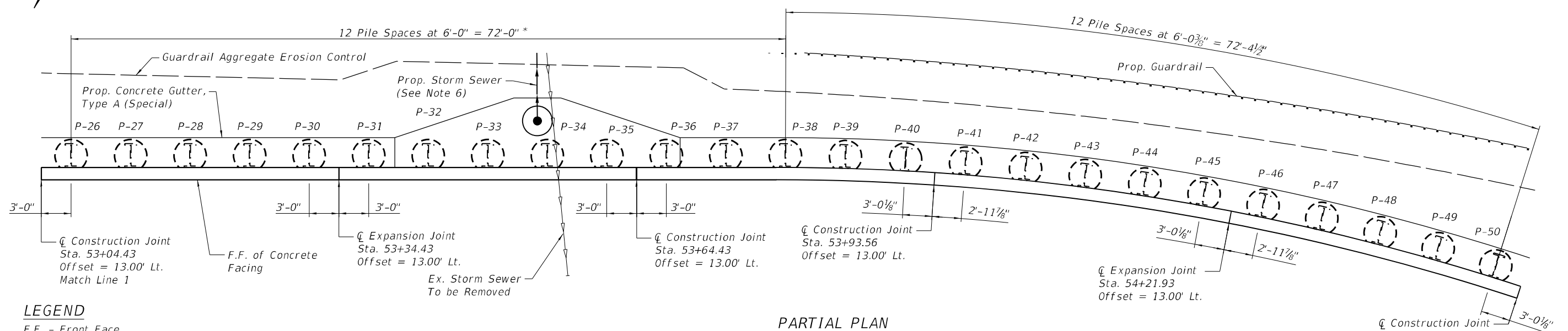
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VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	773
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



PARTIAL ELEVATION

MIN. BAR LAP

#4 Bars = 2'-11"
#5 Bars = 3'-7"



LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

PARTIAL PLAN

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.

NOTES:

1. See Sheet S3-6 for typical notes.

Construction Joint
Sta. 54+50.30
Offset = 13.00' Lt.
Match Line 2

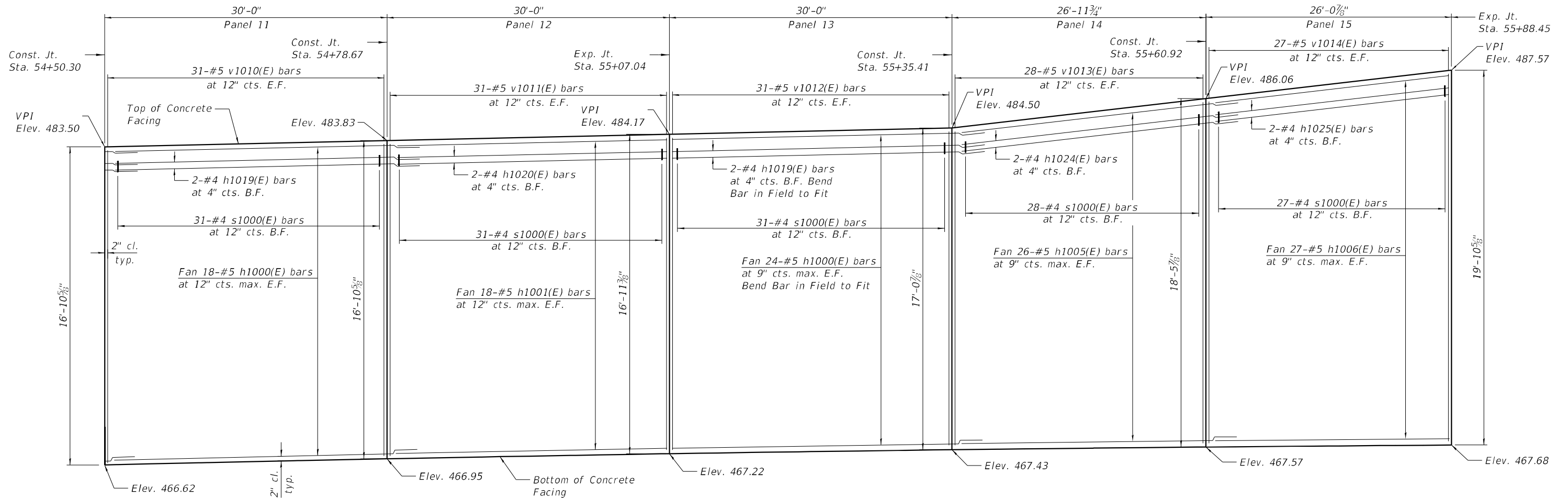
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 2
STRUCTURE NO. 090-8517

SHEET S3-7 OF S3-38 SHEETS

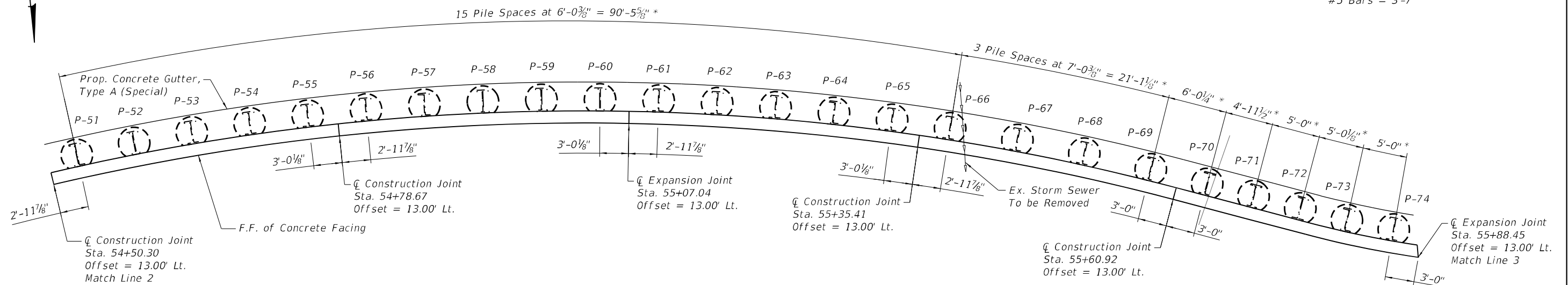
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VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	774
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

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PARTIAL ELEVATION

MIN. BAR LAP
#4 Bars = 2'-11"
#5 Bars = 3'-7"



PARTIAL PLAN

NOTES:
1. See Sheet S3-6 for typical notes.

LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.



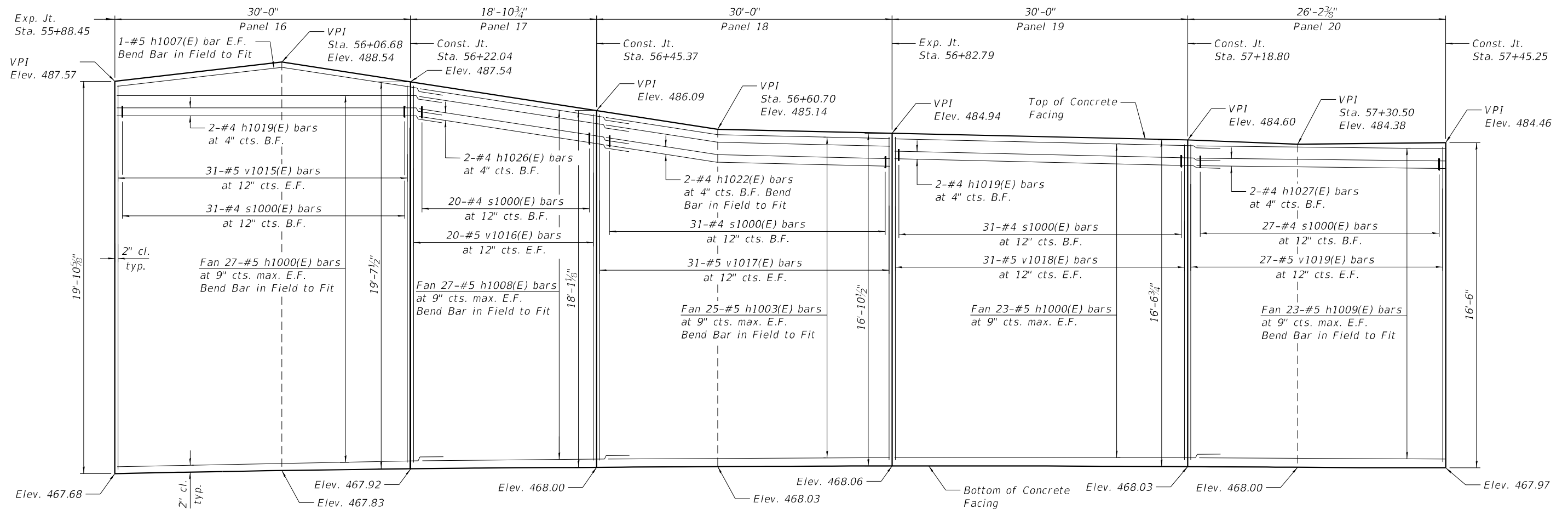
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PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 3
STRUCTURE NO. 090-8517

SHEET S3-8 OF S3-38 SHEETS

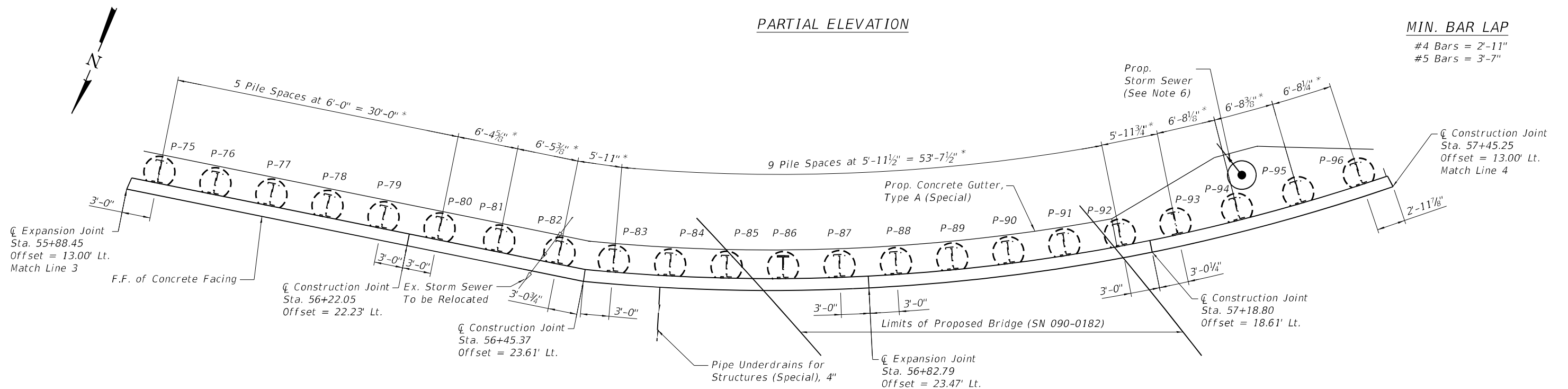
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	775
CONTRACT NO. 68C55				
ILLINOIS / FED. AID PROJECT				



PARTIAL ELEVATION

MIN. BAR LAP

#4 Bars = 2'-11"
#5 Bars = 3'-7"



LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

PARTIAL PLAN

NOTES:

1. See Sheet S3-6 for typical notes.

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 4
STRUCTURE NO. 090-8517

SHEET S3-9 OF S3-38 SHEETS

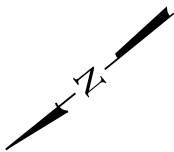
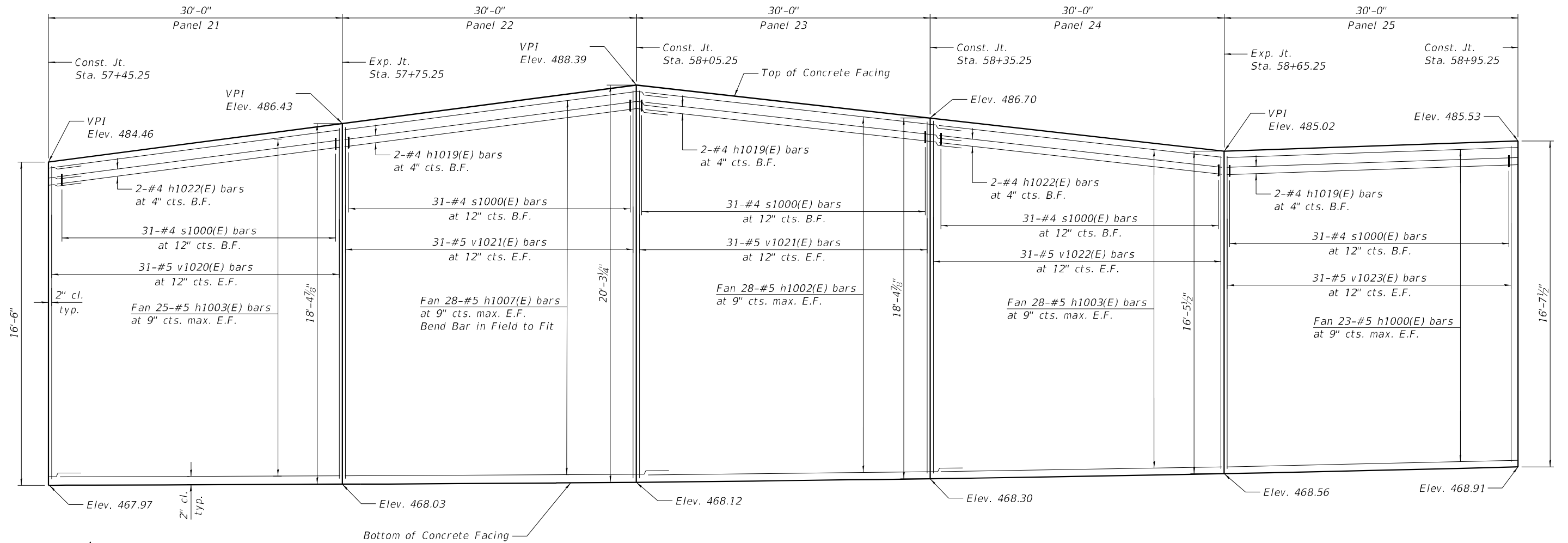
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	776
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

INFRASTRUCTURE
ENGINEERING

USER NAME = Dlamb	DESIGNED - MW	REVISED -
	CHECKED - JTB	REVISED -
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PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

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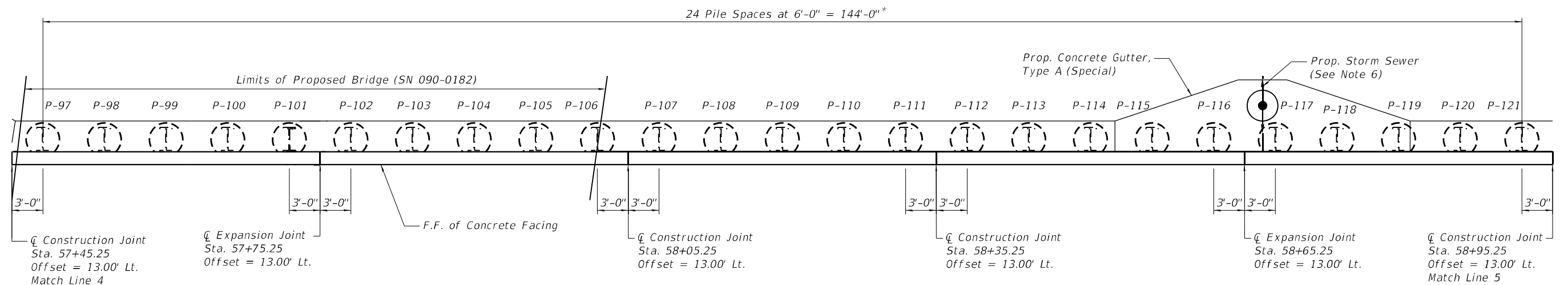
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PARTIAL ELEVATION

MIN. BAR LAP

#4 Bars = 2'-11"
#5 Bars = 3'-7"



LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

PARTIAL PLAN

NOTES:

1. See Sheet S3-6 for typical notes.

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.



USER NAME = DIamb	DESIGNED - MW	REVISED -
	CHECKED - JTB	REVISED -
PLOT SCALE = 10,6666' / in.	DRAWN - TY	REVISED -
PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

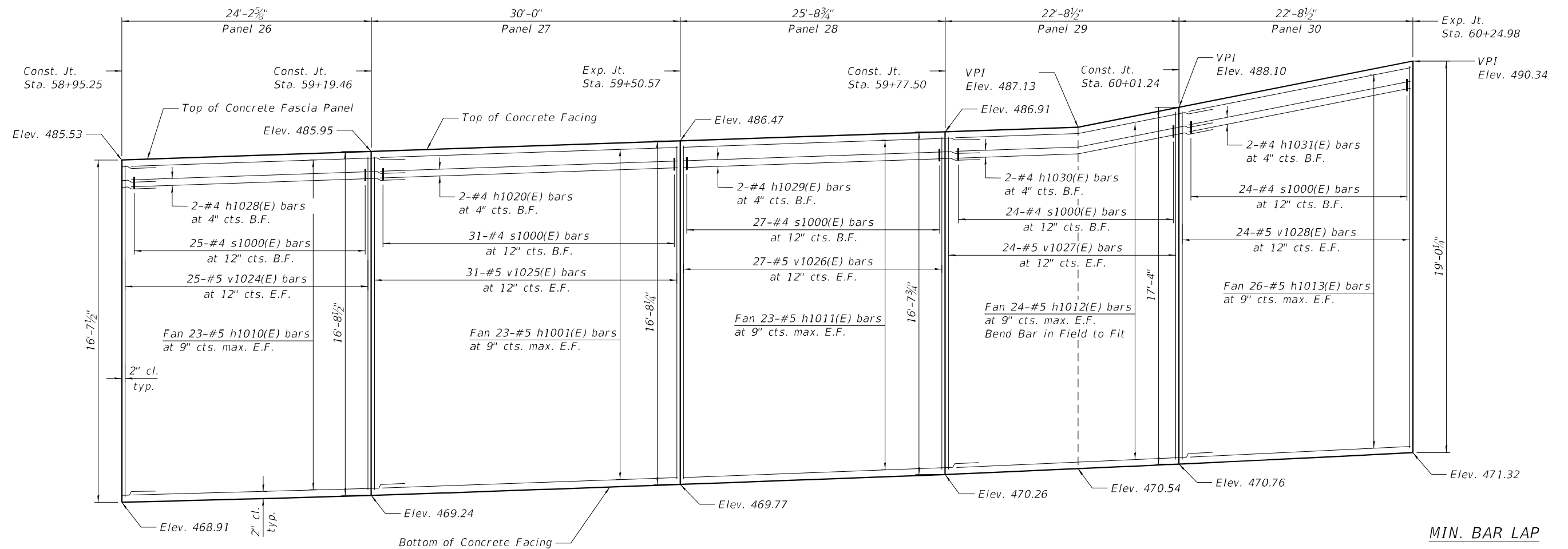
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 5
STRUCTURE NO. 090-8517

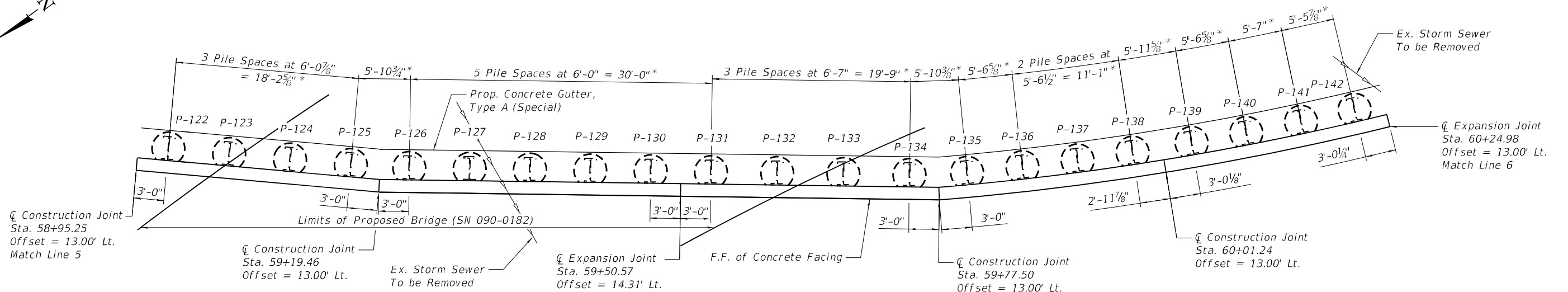
SHEET S3-10 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	777
CONTRACT NO. 68C55				
ILLINOIS		FED. AID PROJECT		

MODEL: Default
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PARTIAL ELEVATION



PARTIAL PLAN

LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.

NOTES:

1. See Sheet S3-6 for typical notes.



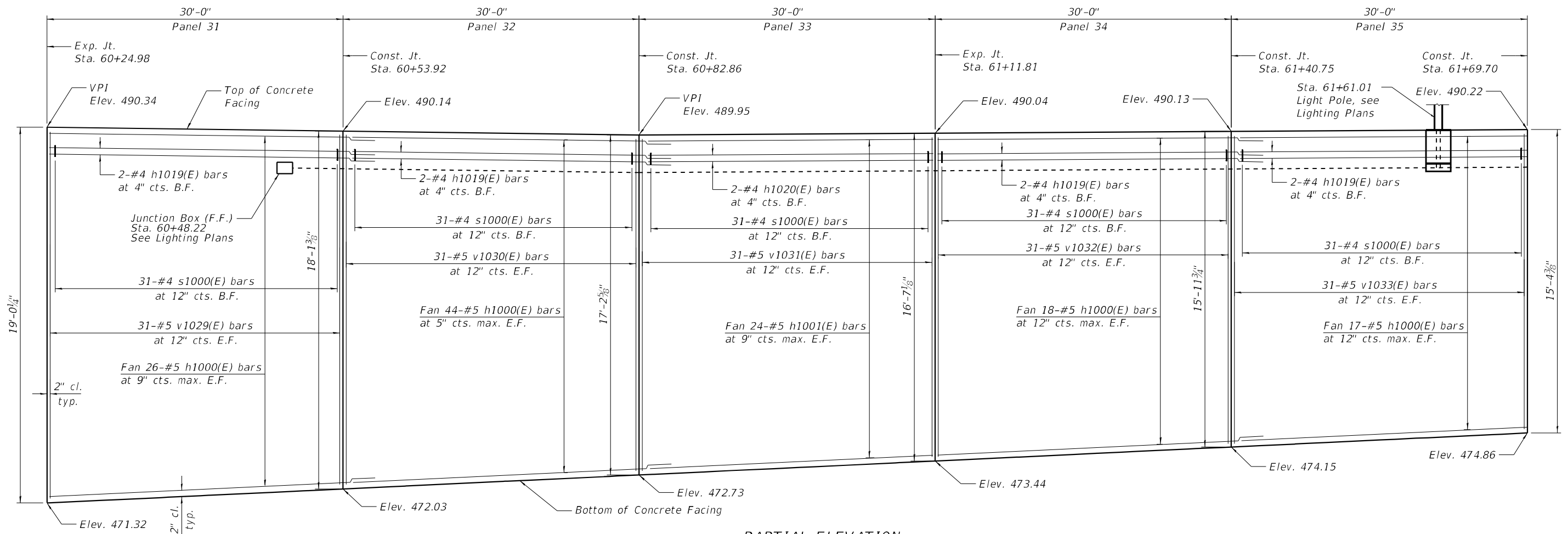
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PLOT DATE = 6/6/2023	DRAWN - TY	REVISED -
	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

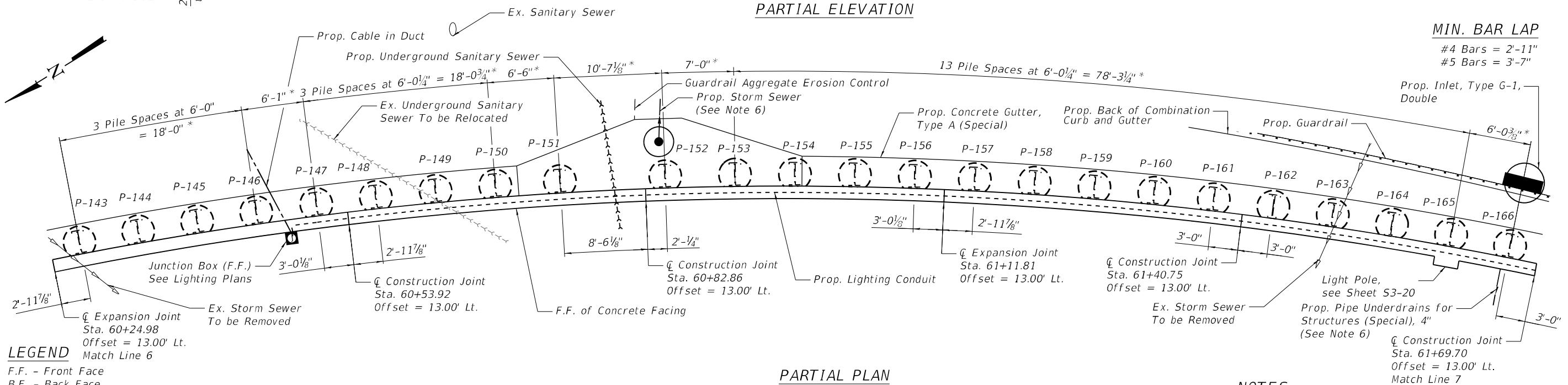
DETAILED PLAN AND ELEVATION 6
STRUCTURE NO. 090-8517

SHEET S3-11 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	778
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



PARTIAL ELEVATION



LEGEND
F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

PARTIAL PLAN

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.

NOTES:

1. See Sheet S3-6 for typical notes.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 7
STRUCTURE NO. 090-8517

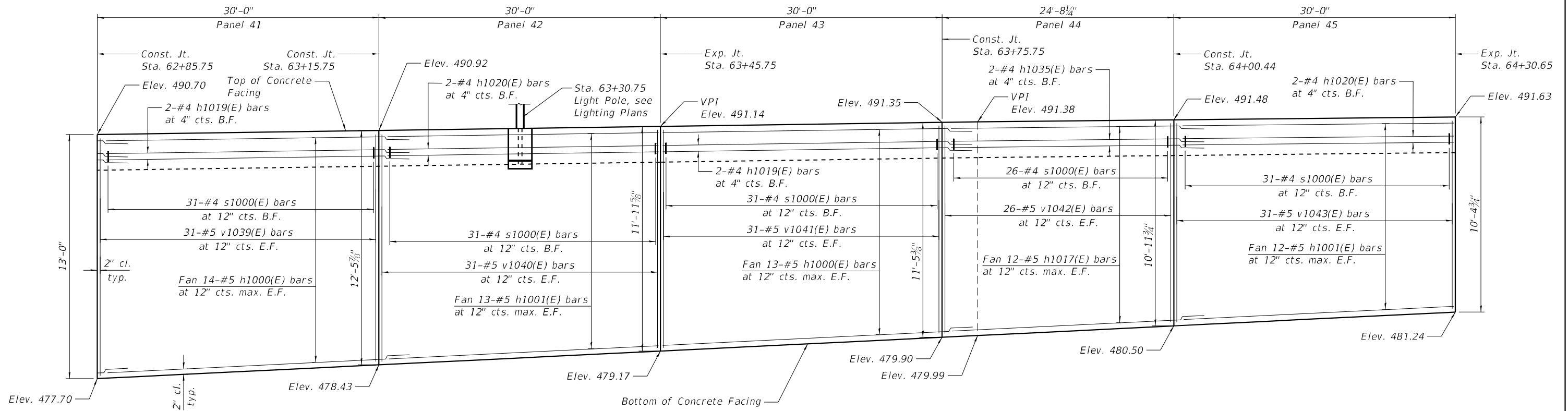
SHEET S3-12 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	779
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				

INFRASTRUCTURE
ENGINEERING

USER NAME = DLamb	DESIGNED - MW	REVISED -
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PLOT DATE = 6/6/2023	DRAWN - TY	REVISED -
	CHECKED - KS	REVISED -

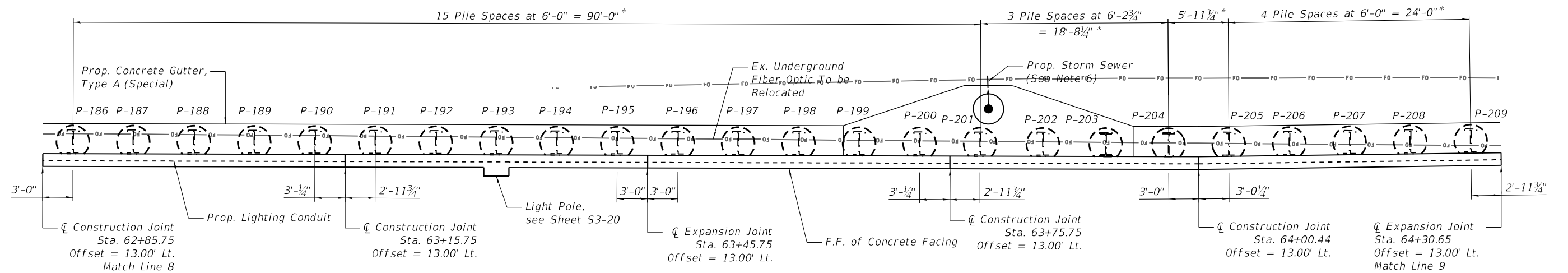
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PARTIAL ELEVATION

MIN. BAR LAP

#4 Bars = 2'-11"
#5 Bars = 3'-7"



PARTIAL PLAN

NOTES:

1. See Sheet S3-6 for typical notes.

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.



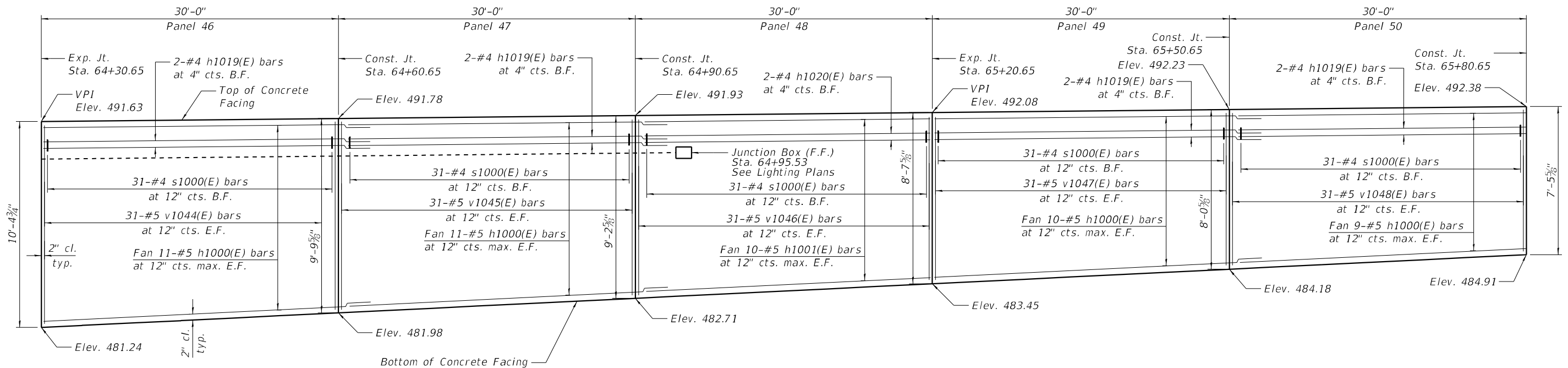
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CHECKED - JTB	REVISED -	
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PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 9
STRUCTURE NO. 090-8517

SHEET S3-14 OF S3-38 SHEETS

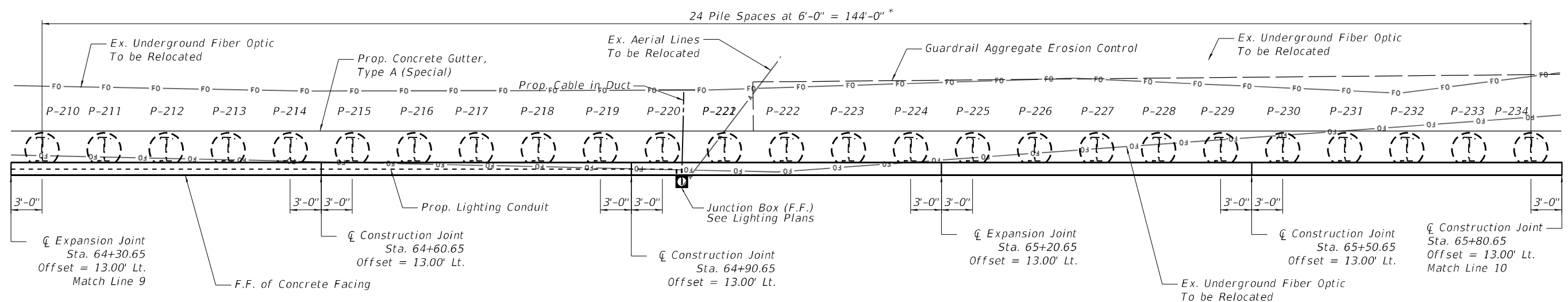
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	781
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



PARTIAL ELEVATION

MIN. BAR LAP

#4 Bars = 2'-11"
#5 Bars = 3'-7"



PARTIAL PLAN

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.

NOTES:

1. See Sheet S3-6 for typical notes.

LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face



INFRASTRUCTURE
ENGINEERING

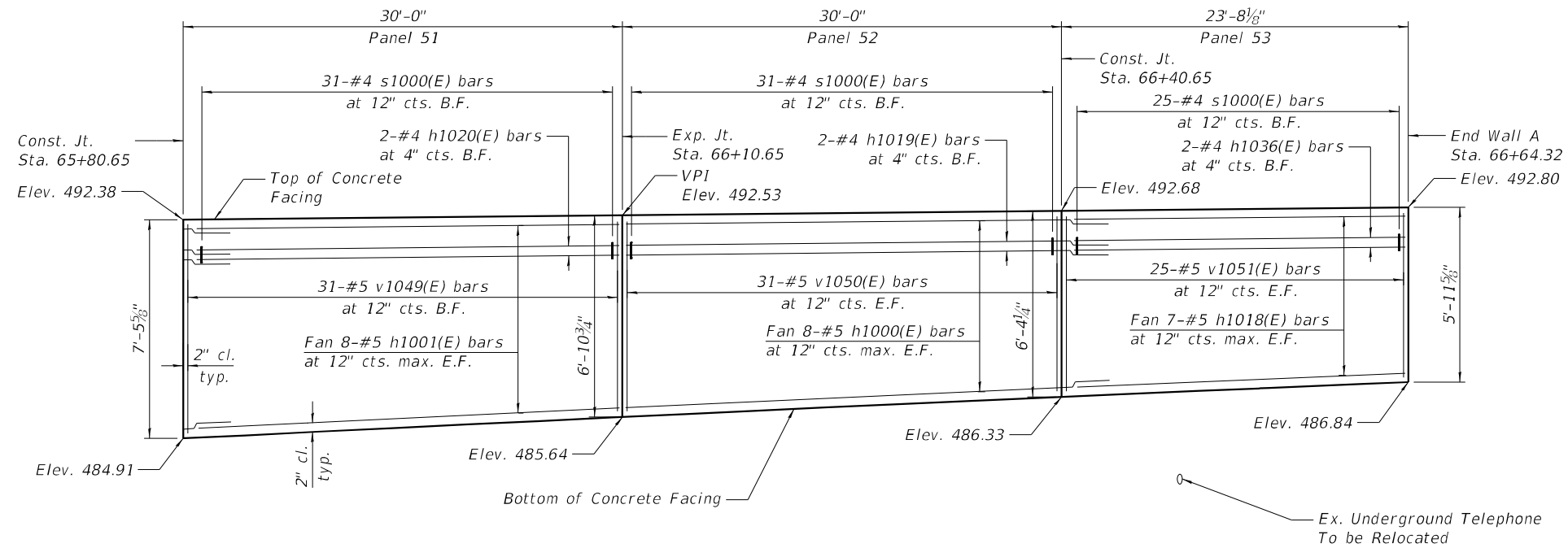
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PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

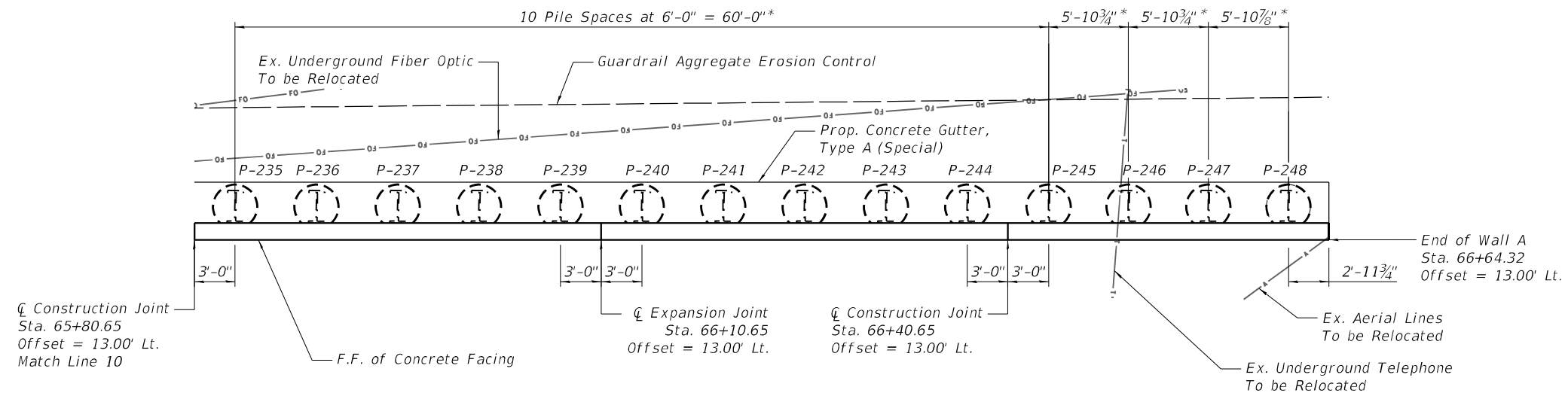
DETAILED PLAN AND ELEVATION 10
STRUCTURE NO. 090-8517

SHEET S3-15 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	782
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



PARTIAL ELEVATION



MIN. BAR LAP

#4 Bars = 2'-11"
#5 Bars = 3'-7"

LEGEND

F.F. - Front Face
B.F. - Back Face
E.F. - Each Face

PARTIAL PLAN

* Pile spacing is denoted from center-to-center of current pile to next leading pile along wall.

NOTES:

1. See Sheet S3-6 for typical notes.

MODEL: Default
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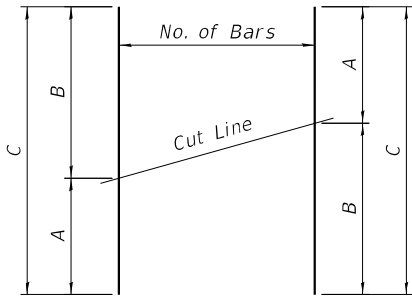
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PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DETAILED PLAN AND ELEVATION 11
STRUCTURE NO. 090-8517

SHEET S3-16 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	783
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



FIELD CUTTING DIAGRAM

See table for dimensions. Make all cuts normal to bar axis.

BAR TABLE SCHEDULE

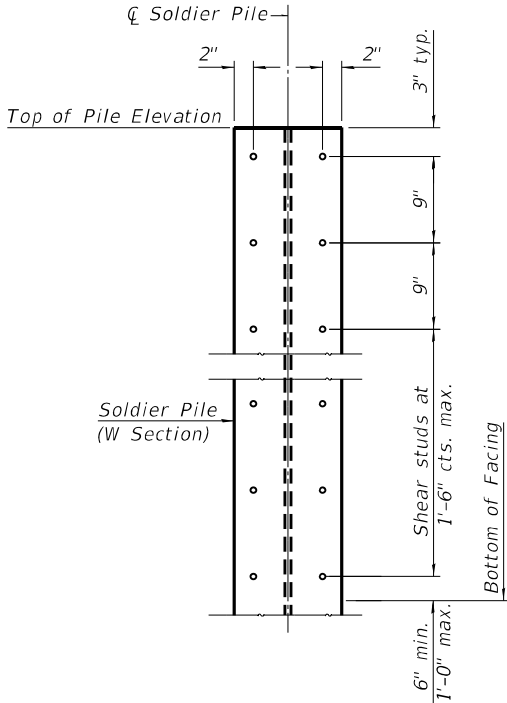
Bar	No. of Sets Req'd	Bar No.	No. of Bars Per Set	A	B	C
v1000(E)	1	#5	31	5'-2"	6'-1"	11'-3"
v1001(E)	1	#5	31	6'-1"	6'-11"	13'-0"
v1002(E)	1	#5	31	6'-11"	8'-6"	15'-5"
v1003(E)	1	#5	31	8'-6"	10'-0"	18'-6"
v1004(E)	1	#5	31	11'-5"	10'-0"	21'-5"
v1005(E)	1	#5	31	11'-5"	12'-10"	24'-3"
v1006(E)	1	#5	31	12'-10"	14'-0"	26'-10"
v1007(E)	1	#5	31	14'-0"	14'-10"	28'-10"
v1008(E)	1	#5	31	14'-10"	15'-8"	30'-6"
v1009(E)	1	#5	31	15'-8"	16'-6"	32'-2"
v1010(E)	1	#5	31	16'-6"	16'-6"	33'-0"
v1011(E)	1	#5	31	16'-6"	16'-7"	33'-1"
v1012(E)	1	#5	31	16'-7"	16'-8"	33'-3"
v1013(E)	1	#5	28	16'-8"	18'-1"	34'-9"
v1014(E)	1	#5	27	18'-1"	19'-6"	37'-7"
v1015(E)	1	#5	31	20'-4"	19'-3"	39'-7"
v1016(E)	1	#5	20	19'-3"	17'-9"	37'-0"
v1017(E)	1	#5	31	17'-9"	16'-6"	34'-3"
v1018(E)	1	#5	31	16'-6"	16'-2"	32'-8"
v1019(E)	1	#5	27	16'-2"	16'-2"	32'-4"
v1020(E)	1	#5	31	16'-2"	18'-0"	34'-2"
v1021(E)	2	#5	31	19'-11"	18'-0"	37'-11"
v1022(E)	1	#5	31	18'-0"	16'-1"	34'-1"
v1023(E)	1	#5	31	16'-1"	16'-3"	32'-4"
v1024(E)	1	#5	25	16'-3"	16'-4"	32'-7"
v1025(E)	1	#5	31	16'-4"	16'-4"	32'-8"
v1026(E)	1	#5	27	16'-4"	16'-3"	32'-7"
v1027(E)	1	#5	24	16'-3"	17'-0"	33'-3"
v1028(E)	1	#5	24	17'-0"	18'-8"	35'-8"
v1029(E)	1	#5	31	18'-8"	17'-9"	36'-5"
v1030(E)	1	#5	31	17'-9"	16'-10"	34'-7"
v1031(E)	1	#5	31	16'-10"	16'-3"	33'-1"
v1032(E)	1	#5	31	16'-3"	15'-7"	31'-10"
v1033(E)	1	#5	31	15'-7"	15'-0"	30'-7"
v1034(E)	1	#5	31	15'-0"	14'-5"	29'-5"
v1035(E)	1	#5	17	14'-5"	14'-0"	28'-5"
v1036(E)	1	#5	20	14'-0"	13'-7"	27'-7"
v1037(E)	1	#5	22	13'-7"	13'-2"	26'-9"
v1038(E)	1	#5	31	13'-2"	12'-8"	25'-10"
v1039(E)	1	#5	31	12'-8"	12'-1"	24'-9"
v1040(E)	1	#5	31	12'-1"	11'-7"	23'-8"
v1041(E)	1	#5	31	11'-7"	11'-1"	22'-8"
v1042(E)	1	#5	26	11'-1"	10'-7"	21'-8"
v1043(E)	1	#5	31	10'-7"	10'-0"	20'-7"
v1044(E)	1	#5	31	10'-0"	9'-5"	19'-5"
v1045(E)	1	#5	31	9'-5"	8'-10"	18'-3"
v1046(E)	1	#5	31	8'-10"	8'-3"	17'-1"
v1047(E)	1	#5	31	8'-3"	7'-8"	15'-11"
v1048(E)	1	#5	31	7'-8"	7'-1"	14'-9"
v1049(E)	1	#5	31	7'-1"	6'-6"	13'-7"
v1050(E)	1	#5	31	6'-6"	6'-0"	12'-6"
v1051(E)	1	#5	25	6'-0"	5'-7"	11'-7"

BILL OF MATERIAL

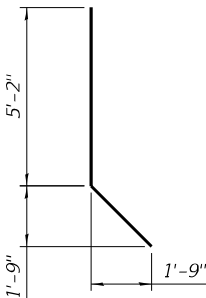
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h1000(E)	650	#5	33'-7"	=====
h1001(E)	268	#5	29'-8"	=====
h1002(E)	202	#5	33'-9"	=====
h1003(E)	184	#5	29'-10"	=====
h1004(E)	34	#5	29'-9"	=====
h1005(E)	52	#5	30'-7"	=====
h1006(E)	54	#5	25'-9"	=====
h1007(E)	58	#5	33'-11"	=====
h1008(E)	54	#5	22'-7"	=====
h1009(E)	46	#5	29'-11"	=====
h1010(E)	46	#5	27'-10"	=====
h1011(E)	46	#5	29'-4"	=====
h1012(E)	48	#5	26'-4"	=====
h1013(E)	52	#5	22'-5"	=====
h1014(E)	32	#5	20'-1"	=====
h1015(E)	70	#5	23'-2"	=====
h1016(E)	30	#5	21'-4"	=====
h1017(E)	24	#5	28'-3"	=====
h1018(E)	14	#5	23'-4"	=====
h1019(E)	42	#4	32'-11"	=====
h1020(E)	18	#4	29'-8"	=====
h1021(E)	10	#4	33'-1"	=====
h1022(E)	8	#4	29'-10"	=====
h1023(E)	2	#4	29'-9"	=====
h1024(E)	2	#4	29'-11"	=====
h1025(E)	2	#4	25'-9"	=====
h1026(E)	2	#4	21'-11"	=====
h1027(E)	2	#4	29'-3"	=====
h1028(E)	2	#4	27'-2"	=====
h1029(E)	2	#4	28'-8"	=====
h1030(E)	2	#4	25'-8"	=====
h1031(E)	2	#4	22'-5"	=====
h1032(E)	2	#4	19'-5"	=====
h1033(E)	2	#4	22'-6"	=====
h1034(E)	2	#4	21'-4"	=====
h1035(E)	2	#4	27'-7"	=====
h1036(E)	2	#4	23'-4"	=====
s1000(E)	1,552	#4	2'-5"	=====
v1000(E)	31	#5	11'-3"	=====
v1001(E)	31	#5	13'-0"	=====
v1002(E)	31	#5	15'-5"	=====
v1003(E)	31	#5	18'-6"	=====
v1004(E)	31	#5	21'-5"	=====
v1005(E)	31	#5	24'-3"	=====
v1006(E)	31	#5	26'-10"	=====
v1007(E)	31	#5	28'-10"	=====
v1008(E)	31	#5	30'-6"	=====
v1009(E)	31	#5	32'-2"	=====
v1010(E)	31	#5	33'-0"	=====
v1011(E)	31	#5	33'-1"	=====

BILL OF MATERIAL

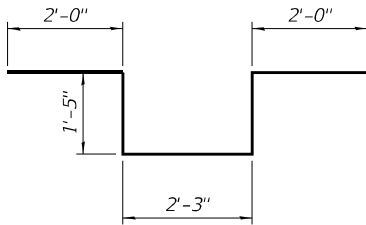
Bar	No.	Size	Length	Shape
v1012(E)	31	#5	33'-3"	=====
v1013(E)	28	#5	34'-9"	=====
v1014(E)	27	#5	37'-7"	=====
v1015(E)	31	#5	38'-9"	=====
v1016(E)	20	#5	37'-0"	=====
v1017(E)	31	#5	34'-3"	=====
v1018(E)	31	#5	32'-8"	=====
v1019(E)	27	#5	32'-4"	=====
v1020(E)	31	#5	34'-2"	=====
v1021(E)	62	#5	37'-11"	=====
v1022(E)	31	#5	34'-1"	=====
v1023(E)	31	#5	32'-4"	=====
v1024(E)	25	#5	32'-7"	=====
v1025(E)	31	#5	32'-8"	=====
v1026(E)	27	#5	32'-7"	=====
v1027(E)	24	#5	33'-3"	=====
v1028(E)	24	#5	35'-8"	=====
v1029(E)	31	#5	36'-5"	=====
v1030(E)	31	#5	34'-7"	=====
v1031(E)	31	#5	33'-1"	=====
v1032(E)	31	#5	31'-10"	=====
v1033(E)	31	#5	30'-7"	=====
v1034(E)	31	#5	29'-5"	=====
v1035(E)	17	#5	28'-5"	=====
v1036(E)	20	#5	27'-7"	=====
v1037(E)	22	#5	26'-9"	=====
v1038(E)	31	#5	25'-10"	=====
v1039(E)	31	#5	24'-9"	=====
v1040(E)	31	#5	23'-8"	=====
v1041(E)	31	#5	22'-8"	=====
v1042(E)	26	#5	21'-8"	=====
v1043(E)	31	#5	20'-7"	=====
v1044(E)	31	#5	19'-5"	=====
v1045(E)	31	#5	18'-3"	=====
v1046(E)	31	#5	17'-1"	=====
v1047(E)	31	#5	15'-11"	=====
v1048(E)	31	#5	14'-9"	=====
v1049(E)	31	#5	13'-7"	=====
v1050(E)	31	#5	12'-6"	=====
v1051(E)	25	#5	11'-7"	=====
d1000(E)	6	#6	7'-8"	=====
d1001(E)	20	#6	9'-1"	=====
Concrete Structures			Cu. Yd.	983.3
Reinforcement Bars, Epoxy Coated			Pound	111,330



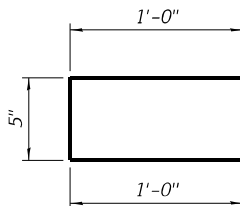
SHEAR STUD DETAIL



BAR d1000(E)



BAR d1001(E)



BAR s1000(E)

MODEL: Default
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USER NAME = DIamb
PLOT SCALE = 0.1667' / in.
PLOT DATE = 6/6/2023

DESIGNED - TY
CHECKED - KS
DRAWN - TY
CHECKED - KS

REVISED -
REVISED -
REVISED -
REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WALL A BILL OF MATERIAL
STRUCTURE NO. 090-8517

SHEET S3-17 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	784
CONTRACT NO. 68C55				
ILLINOIS / FED. AID PROJECT				

MODEL: Default
FILE: \\miller-gp\mbs\us-sh-bentley.com\mbs-us-pw\03\Documents\Chicago_IL\01_Projects\179140_IDOT_CedarStExt_Phil5.0_CADD\C40_Sheets\Rebasing_Wall_Sheets\090851-68C55-WallA-017.dgn

PILE SCHEDULE 1

Pile Number	Pile Type	Station	Offset	Top of Pile Elevation	Pile Tip Elevation	Estimated Top of Rock Elevation	Estimated Pile Length	Number of Studs per Pile	Top of Encasement Elevation	Encasement Diameter (Inches)
01	W18X76	51+57.43	15.44'	467.72	442.72	433.00	25'-0"	8	464.81	30
02	W18X76	51+63.43	15.44'	467.87	442.87	433.00	25'-0"	8	464.79	30
03	W18X76	51+69.43	15.44'	468.02	443.02	433.00	25'-0"	8	464.77	30
04	W18X76	51+75.43	15.44'	468.17	443.17	433.00	25'-0"	8	464.75	30
05	W18X76	51+81.43	15.44'	468.32	443.32	433.00	25'-0"	8	464.73	30
06	W18X76	51+87.43	15.44'	468.47	443.47	433.00	25'-0"	8	464.71	30
07	W18X76	51+93.43	15.44'	468.62	443.62	433.00	25'-0"	8	464.69	30
08	W18X76	51+99.43	15.44'	468.77	443.77	433.00	25'-0"	8	464.67	30
09	W18X76	52+05.43	15.44'	468.92	443.92	433.00	25'-0"	10	464.65	30
10	W18X76	52+11.43	15.44'	469.07	444.07	433.00	25'-0"	10	464.63	30
11	W18X76	52+17.43	15.44'	469.31	444.31	433.00	25'-0"	10	464.61	30
12	W18X76	52+23.43	15.44'	469.62	444.62	433.00	25'-0"	10	464.61	30
13	W18X76	52+29.43	15.44'	469.94	444.94	433.00	25'-0"	10	464.60	30
14	W18X76	52+35.43	15.44'	470.26	445.26	433.00	25'-0"	10	464.60	30
15	W18X76	52+41.43	15.44'	470.57	445.57	433.00	25'-0"	12	464.59	30
16	W18X119	52+47.43	15.44'	470.89	431.89	433.00	39'-0"	12	464.60	30
17	W18X119	52+53.43	15.44'	471.20	432.20	433.00	39'-0"	12	464.61	30
18	W18X119	52+59.43	15.44'	471.52	432.52	433.00	39'-0"	12	464.63	30
19	W18X119	52+65.43	15.44'	471.84	432.84	433.00	39'-0"	12	464.64	30
20	W18X119	52+71.43	15.44'	472.15	433.15	433.00	39'-0"	14	464.65	30
21	W18X119	52+77.43	15.44'	472.47	433.47	433.00	39'-0"	14	464.68	30
22	W18X119	52+83.43	15.44'	472.78	433.78	433.00	39'-0"	14	464.71	30
23	W18X119	52+89.43	15.44'	473.10	434.10	433.00	39'-0"	14	464.74	30
24	W18X119	52+95.43	15.44'	473.42	434.42	433.00	39'-0"	14	464.77	30
25	W18X119	53+01.43	15.44'	473.73	434.73	433.00	39'-0"	16	464.80	30
26	W18X119	53+07.43	15.44'	474.05	435.05	433.00	39'-0"	16	464.83	30
27	W18X119	53+13.43	15.44'	474.36	435.36	433.00	39'-0"	16	464.88	30
28	W18X119	53+19.43	15.44'	474.68	435.68	433.00	39'-0"	16	464.93	30
29	W18X119	53+25.43	15.44'	475.00	436.00	433.00	39'-0"	16	464.98	30
30	W18X119	53+31.43	15.44'	475.31	436.31	433.00	39'-0"	18	465.03	30
31	W18X119	53+37.43	15.44'	475.63	436.63	433.00	39'-0"	18	465.09	30
32	W18X119	53+43.43	15.44'	475.94	436.94	433.00	39'-0"	18	465.16	30
33	W18X119	53+49.43	15.44'	476.26	437.26	433.00	39'-0"	18	465.22	30
34	W18X119	53+55.43	15.44'	476.58	437.58	433.00	39'-0"	18	465.29	30
35	W18X119	53+61.43	15.44'	476.89	437.89	433.00	39'-0"	18	465.36	30
36	W18X119	53+67.43	15.44'	477.18	438.18	433.00	39'-0"	18	465.43	30
37	W18X119	53+73.43	15.44'	477.44	438.44	433.00	39'-0"	20	465.52	30
38	W18X119	53+79.37	15.44'	477.69	438.69	433.00	39'-0"	20	465.60	30
39	W18X119	53+85.04	15.44'	477.94	438.94	433.00	39'-0"	20	465.68	30
40	W18X119	53+90.72	15.44'	478.18	439.18	433.00	39'-0"	20	465.76	30
41	W18X119	53+96.39	15.44'	478.42	439.42	433.00	39'-0"	20	465.84	30
42	W18X119	54+02.06	15.44'	478.67	439.67	433.00	39'-0"	20	465.93	30
43	W18X119	54+07.74	15.44'	478.91	439.91	433.00	39'-0"	20	466.02	30
44	W18X119	54+13.41	15.44'	479.15	440.15	433.00	39'-0"	20	466.10	30
45	W18X119	54+19.09	15.44'	479.40	440.40	433.00	39'-0"	20	466.19	30
46	W18X119	54+24.76	15.44'	479.64	440.64	433.00	39'-0"	22	466.27	30
47	W18X119	54+30.43	15.44'	479.89	440.89	433.00	39'-0"	22	466.35	30
48	W18X119	54+36.11	15.44'	480.13	441.13	433.00	39'-0"	22	466.43	30
49	W18X119	54+41.78	15.44'	480.38	441.38	433.00	39'-0"	22	466.50	30
50	W18X119	54+47.46	15.44'	480.63	441.63	433.00	39'-0"	22	466.58	30

PILE SCHEDULE 2

Pile Number	Pile Type	Station	Offset	Top of Pile Elevation	Pile Tip Elevation	Estimated Top of Rock Elevation	Estimated Pile Length	Number of Studs per Pile	Top of Encasement Elevation	Encasement Diameter (Inches)
51	W18X119	54+53.13	15.44'	480.78	441.78	432.00	39'-0"	22	466.65	30
52	W18X119	54+58.80	15.44'	480.85	441.85	432.00	39'-0"	22	466.72	30
53	W18X119	54+64.48	15.44'	480.92	441.92	432.00	39'-0"	22	466.78	30
54	W18X119	54+70.15	15.44'	480.98	441.98	432.00	39'-0"	22	466.85	30
55	W18X119	54+75.83	15.44'	481.05	442.05	432.00	39'-0"	22	466.92	30
56	W18X119	54+81.50	15.44'	481.12	442.12	432.00	39'-0"	22	466.98	30
57	W18X119	54+87.17	15.44'	481.19	442.19	432.00	39'-0"	22	467.03	30
58	W18X119	54+92.85	15.44'	481.25	442.25	432.00	39'-0"	22	467.08	30
59	W18X119	54+98.52	15.44'	481.32	442.32	432.00	39'-0"	22	467.14	30
60	W18X119	55+04.19	15.44'	481.39	442.39	432.00	39'-0"	22	467.19	30
61	W18X211	55+09.87	15.44'	481.45	434.45	432.00	47'-0"	22	467.24	30
62	W18X211	55+15.54	15.44'	481.52	434.52	432.00	47'-0"	22	467.28	30
63	W18X211	55+21.22	15.44'	481.58	434.58	432.00	47'-0"	22	467.32	30
64	W18X211	55+26.89	15.44'	481.65	434.65	432.00	47'-0"	22	467.36	30
65	W18X211	55+32.56	15.44'	481.72	434.72	432.00	47'-0"	22	467.41	30
66	W18X211	55+38.24	15.44'	481.92	434.92	432.00	47'-0"	22	467.44	30
67	W18X211	55+44.86	15.44'	482.33	435.33	432.00	47'-0"	24	467.48	30
68	W18X211	55+51.47	15.44'	482.73	435.73	432.00	47'-0"	24	467.52	30
69	W18X211	55+58.08	15.44'	483.14	436.14	432.00	47'-0"	24	467.55	30
70	W18X211	55+63.92	15.44'	483.47	436.47	432.00	47'-0"	24	467.58	30
71	W18X211	55+68.88	15.44'	483.75	436.75	432.00	47'-0"	24	467.60	30
72	W18X211	55+73.88	15.44'	484.02	437.02	432.00	47'-0"	26	467.62	30
73	W18X211	55+79.41	15.44'	484.32	437.32	432.00	47'-0"	26	467.64	30
74	W18X211	55+85.08	15.44'	484.64	437.64	432.00	47'-0"	26	467.67	30
75	W18X211	55+90.34	16.49'	484.92	437.92	432.00	47'-0"	26	467.70	30
76	W18X211	55+96.80	18.87'	485.26	438.26	432.00	47'-0"	26	467.75	30
77	W18X211	56+03.56	20.93'	485.62	438.62	432.00	47'-0"	28	467.81	30
78	W18X211	56+10.57	22.68'	485.54	438.54	432.00	47'-0"	26	467.85	30
79	W18X211	56+17.82	24.07"	485.06	438.06	432.00	47'-0"	26	467.90	30
80	W18X211	56+25.25	25.11'	484.59	437.59	432.00	47'-0"	26	467.93	30
81	W18X211	56+33.31	25.81'	484.09	437.09	432.00	47'-0"	24	467.96	30
82	W18X211	56+41.52	26.08'	483.58	436.58	432.00	47'-0"	24	467.98	30
83	W18X211	56+48.90	26.25'	483.12	436.12	432.00	47'-0"	24	468.00	30
84	W18X211	56+56.49	26.52'	482.65	435.65	432.00	47'-0"	22	468.02	30
85	W18X211	56+64.10	26.60'	482.36	435.36	432.00	47'-0"	22	468.04	30
86	W18X211	56+71.71	26.47'	482.29	435.29	432.00	47'-0"	22	468.05	30
87	W18X211	56+79.29	26.14'	482.22	435.22	432.00	47'-0"	22	468.06	30
88	W18X211	56+86.82	25.62'	482.15	435.15	432.00	47'-0"	22	468.06	30
89	W18X211	56+94.27	24.91'	482.08	435.08	432.00	47'-0"	22	468.05	30
90	W18X211	57+01.63	24.01'	482.01	435.01	432.00	47'-0"	22	468.05	30
91	W18X211	57+08.87	22.93'	481.94	434.94	432.00	47'-0"	22	468.04	30
92	W18X211	57+15.99	21.67"	481.88	434.88	432.00	47'-0"	22	468.03	30
93	W18X211	57+23.24	20.22'	481.77	434.77	432.00	47'-0"	22	468.02	30
94	W18X211	57+29.67	18.57'	481.65	434.65	432.00	47'-0"	22	468.00	30
95	W18X211	57+36.16	17.12'	481.66	434.66	432.00	47'-0"	22	467.99	30
96	W18X211	57+42.70	15.89'	481.70	434.70	432.00	47'-0"	22	467.98	30
97	W18X211	57+48.25	15.44'	481.91	434.91	432.00	47'-0"	22	467.98	30
98	W18X211	57+54.25	15.44'	482.30	435.30	432.00	47'-0"	22	467.99	30
99	W18X211	57+60.25	15.44'	482.69	435.69	432.00	47'-0"	22	468.00	30
100	W18X211	57+66.25	15.44'	483.09	436.09	432.00	47'-0"	24	468.01	30



USER NAME = DIamb	DESIGNED - YC	REVISED -
	CHECKED - MLK	REVISED -
PLOT SCALE = 0.1667 ' / in.	DRAWN - TY	REVISED -
PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PILE SCHEDULE 1
STRUCTURE NO. 090-8517

SHEET S3-18 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	785
CONTRACT NO. 68C55				
ILLINOIS		FED. AID PROJECT		

MODEL: Default
FILE: \\miller-pc\mybus\sub\benley\com\mb-us-pw\03\Documents\Chicago_I\U1\Projectal179140_IDOT_CedarStExt_Phl15.0_CADD\C40_Sheets\Rebining_Wall_Sheets\090851-68C55-Wall4-018.dgn

PILE SCHEDULE 3

Pile Number	Pile Type	Station	Offset	Top of Pile Elevation	Pile Tip Elevation	Estimated Top of Rock Elevation	Estimated Pile Length	Number of Studs per Pile	Top of Encasement Elevation	Encasement Diameter (Inches)
101	W18X211	57+72.25	15.44'	483.48	436.48	432.00	47'-0"	24	468.02	30
102	W18X211	57+78.25	15.44'	483.88	436.88	432.00	47'-0"	24	468.04	30
103	W18X211	57+84.25	15.44'	484.27	437.27	432.00	47'-0"	24	468.06	30
104	W18X211	57+90.25	15.44'	484.66	437.66	432.00	47'-0"	26	468.07	30
105	W18X211	57+96.25	15.44'	485.05	438.05	432.00	47'-0"	26	468.09	30
106	W18X211	58+02.25	15.44'	485.44	438.44	444.00	47'-0"	26	468.11	30
107	W18X211	58+08.25	15.44'	485.47	438.47	444.00	47'-0"	26	468.14	30
108	W18X211	58+14.25	15.44'	485.13	438.13	444.00	47'-0"	26	468.18	30
109	W18X211	58+20.25	15.44'	484.80	437.80	444.00	47'-0"	26	468.21	30
110	W18X211	58+26.25	15.44'	484.46	437.46	444.00	47'-0"	24	468.25	30
111	W18X211	58+32.25	15.44'	484.12	437.12	444.00	47'-0"	24	468.28	30
112	W18X211	58+38.25	15.44'	483.79	436.79	444.00	47'-0"	24	468.33	30
113	W18X211	58+44.25	15.44'	483.45	436.45	444.00	47'-0"	24	468.38	30
114	W18X211	58+50.25	15.44'	483.11	436.11	444.00	47'-0"	22	468.43	30
115	W18X211	58+56.25	15.44'	482.78	435.78	444.00	47'-0"	22	468.48	30
116	W18X211	58+62.25	15.44'	482.44	435.44	444.00	47'-0"	22	468.54	30
117	W18X211	58+68.25	15.44'	482.32	435.32	444.00	47'-0"	22	468.60	30
118	W18X211	58+74.25	15.44'	482.42	435.42	444.00	47'-0"	22	468.67	30
119	W18X211	58+80.25	15.44'	482.53	435.53	444.00	47'-0"	22	468.74	30
120	W18X211	58+86.25	15.44'	482.63	435.63	444.00	47'-0"	22	468.81	30
121	W18X211	58+92.25	15.44'	482.73	435.73	444.00	47'-0"	22	468.88	30
122	W18X211	58+98.25	15.44'	482.84	435.84	444.00	47'-0"	22	468.95	30
123	W18X211	59+04.32	15.44'	482.94	435.94	444.00	47'-0"	22	469.03	30
124	W18X211	59+10.39	15.44'	483.04	436.04	444.00	47'-0"	22	469.12	30
125	W18X211	59+16.46	15.44'	483.15	436.15	444.00	47'-0"	22	469.20	30
126	W18X211	59+22.27	15.66'	483.25	436.25	444.00	47'-0"	22	469.29	30
127	W18X211	59+28.37	16.11'	483.35	436.35	444.00	47'-0"	22	469.39	30
128	W18X211	59+34.70	16.45'	483.45	436.45	444.00	47'-0"	22	469.50	30
129	W18X211	59+41.05	16.67'	483.56	436.56	444.00	47'-0"	22	469.61	30
130	W18X211	59+47.40	16.76'	483.67	436.67	444.00	47'-0"	22	469.72	30
131	W18X211	59+53.76	16.72'	483.77	436.77	444.00	47'-0"	22	469.83	30
132	W18X211	59+60.72	16.54'	483.89	436.89	444.00	47'-0"	22	469.96	30
133	W18X211	59+67.67	16.20'	484.00	437.00	444.00	47'-0"	22	470.08	30
134	W18X211	59+74.60	15.71'	484.11	437.11	444.00	47'-0"	22	470.21	30
135	W18X211	59+80.64	15.44'	484.21	437.21	444.00	47'-0"	22	470.32	30
136	W18X211	59+86.47	15.44'	484.31	437.31	444.00	47'-0"	22	470.45	30
137	W18X211	59+92.29	15.44'	484.50	437.50	444.00	47'-0"	22	470.57	30
138	W18X211	59+98.11	15.44'	485.05	438.05	444.00	47'-0"	22	470.69	30
139	W18X211	60+04.38	15.44'	485.64	438.64	444.00	47'-0"	24	470.83	30
140	W18X211	60+10.20	15.44'	486.19	439.19	444.00	47'-0"	24	470.97	30
141	W18X211	60+16.06	15.44'	486.75	439.75	444.00	47'-0"	24	471.11	30
142	W18X211	60+21.81	15.44'	487.29	440.29	444.00	47'-0"	24	471.24	30
143	W18X211	60+27.86	15.44'	487.57	440.57	444.00	47'-0"	24	471.39	30
144	W18X211	60+33.63	15.44'	487.53	440.53	444.00	47'-0"	24	471.53	30
145	W18X211	60+39.40	15.44'	487.49	440.49	444.00	47'-0"	24	471.67	30
146	W18X211	60+45.17	15.44'	487.45	440.45	444.00	47'-0"	24	471.81	30
147	W18X211	60+51.02	15.44'	487.41	440.41	444.00	47'-0"	24	471.96	30
148	W18X211	60+56.81	15.44'	487.38	440.38	444.00	47'-0"	24	472.10	30
149	W18X211	60+62.60	15.44'	487.34	440.34	444.00	47'-0"	24	472.24	30
150	W18X211	60+68.38	15.44'	487.30	440.30	444.00	47'-0"	24	472.38	30

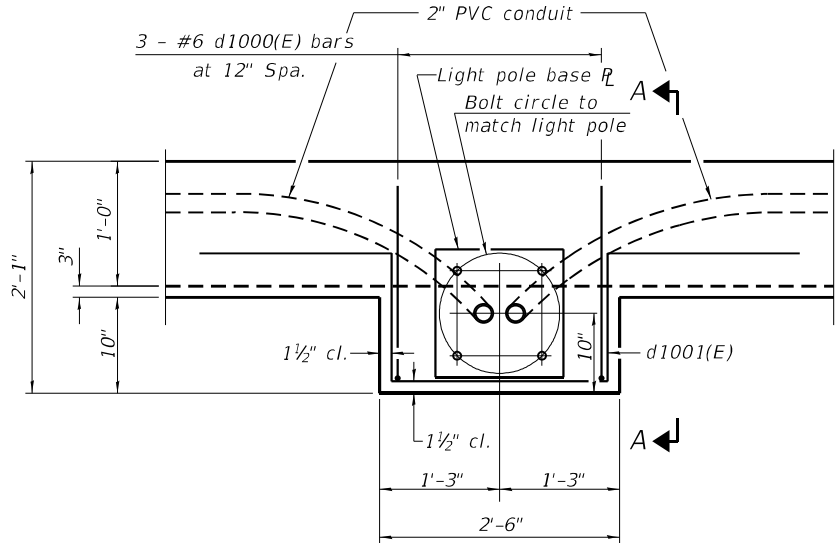
PILE SCHEDULE 4

Pile Number	Pile Type	Station	Offset	Top of Pile Elevation	Pile Tip Elevation	Estimated Top of Rock Elevation	Estimated Pile Length	Number of Studs per Pile	Top of Encasement Elevation	Encasement Diameter (Inches)
151	W18X211	60+74.66	15.44'	487.26	440.26	444.00	47'-0"	22	472.53	30
152	W18X211	60+84.80	15.44'	487.21	440.21	444.00	47'-0"	22	472.78	30
153	W18X211	60+91.55	15.44'	487.23	440.23	444.00	47'-0"	22	472.95	30
154	W18X211	60+97.33	15.44'	487.24	440.24	444.00	47'-0"	22	473.09	30
155	W18X211	61+03.12	15.44'	487.26	440.26	444.00	47'-0"	22	473.23	30
156	W18X211	61+08.91	15.44'	487.28	440.28	444.00	47'-0"	22	473.37	30
157	W18X119	61+14.70	15.44'	487.30	448.30	444.00	39'-0"	22	473.51	30
158	W18X119	61+20.48	15.44'	487.32	448.32	444.00	39'-0"	22	473.65	30
159	W18X119	61+26.27	15.44'	487.33	448.33	444.00	39'-0"	22	473.80	30
160	W18X119	61+32.06	15.44'	487.35	448.35	444.00	39'-0"	22	473.94	30
161	W18X119	61+37.85	15.44'	487.37	448.37	444.00	39'-0"	22	474.08	30
162	W18X119	61+43.64	15.44'	487.39	448.39	444.00	39'-0"	20	474.22	30
163	W18X119	61+49.43	15.44'	487.41	448.41	444.00	39'-0"	20	474.36	30
164	W18X119	61+55.22	15.44'	487.42	448.42	444.00	39'-0"	20	474.50	30
165	W18X119	61+61.01	15.44'	487.44	448.44	444.00	39'-0"	20	474.65	30
166	W18X119	61+66.81	15.44'	487.46	448.46	444.00	39'-0"	20	474.79	30
167	W18X119	61+72.59	15.44'	487.48	448.48	444.00	39'-0"	20	474.93	30
168	W18X119	61+78.37	15.44'	487.50	448.50	444.00	39'-0"	20	475.07	30
169	W18X119	61+84.16	15.44'	487.51	448.51	444.00	39'-0"	20	475.21	30
170	W18X119	61+89.95	15.44'	487.53	448.53	444.00	39'-0"	20	475.35	30
171	W18X119	61+95.74	15.44'	487.55	448.55	444.00	39'-0"	20	475.50	30
172	W18X119	62+01.53	15.44'	487.57	448.57	444.00	39'-0"	20	475.64	30
173	W18X119	62+06.60	15.44'	487.58	448.58	444.00	39'-0"	20	475.76	30
174	W18X119	62+11.67	15.44'	487.60	448.60	444.00	39'-0"	18	475.89	30
175	W18X211	62+17.90	15.44'	487.62	451.62	444.00	36'-0"	18	476.04	30
176	W18X211	62+30.80	15.44'	487.66	451.66	444.00	36'-0"	18	476.35	30
177	W18X119	62+37.13	15.44'	487.68	448.68	444.00	39'-0"	18	476.51	30
178	W18X119	62+42.34	15.44'	487.70	448.70	444.00	39'-0"	18	476.64	30
179	W18X119	62+47.54	15.44'	487.71	448.71	444.00	39'-0"	18	476.76	30
180	W18X119	62+52.75	15.44'	487.73	448.73	444.00	39'-0"	18	476.89	30
181	W18X119	62+58.75	15.44'	487.76	448.76	444.00	39'-0"	18	477.04	30
182	W18X119	62+64.75	15.44'	487.80	448.80	444.00	39'-0"	18	477.18	30
183	W18X119	62+70.75	15.44'	487.84	448.84	444.00	39'-0"	18	477.33	30
184	W18X119	62+76.75	15.44'	487.89	448.89	444.00	39'-0"	18	477.48	30
185	W18X119	62+82.75	15.44'	487.93	448.93	444.00	39'-0"	18	477.62	30
186	W18X119	62+88.75	15.44'	487.97	448.97	444.00	39'-0"	16	477.77	30
187	W18X119	62+94.75	15.44'	488.02	449.02	444.00	39'-0"	16	477.92	30
188	W18X119	63+00.75	15.44'	488.06	449.06	444.00	39'-0"	16	478.06	30
189	W18X119	63+06.75	15.44'	488.11	449.11	444.00	39'-0"	16	478.21	30
190	W18X119	63+12.75	15.44'	488.15	449.15	444.00	39'-0"	16	478.36	30
191	W18X119	63+18.75	15.44'	488.19	449.19	444.00	39'-0"	16	478.51	30
192	W18X119	63+24.75	15.44'	488.24	449.24	444.00	39'-0"	16	478.65	30
193	W18X119	63+30.75	15.44'	488.28	449.28	444.00	39'-0"	16	478.80	30
194	W18X119	63+36.75	15.44'	488.32	449.32	444.00	39'-0"	16	478.95	30
195	W18X119	63+42.75	15.44'	488.37	449.37	444.00	39'-0"	16	479.09	30
196	W18X76	63+48.75	15.44'	488.41	463.41	444.00	25'-0"	16	479.24	30
197	W18X76	63+54.75	15.44'	488.45	463.45	444.00	25'-0"	16	479.39	30
198	W18X76	63+60.75	15.44'	488.50	463.50	444.00	25'-0"	16	479.53	30
199	W18X76	63+66.75	15.44'	488.54	463.54	444.00	25'-0"	16	479.68	30
200	W18X76	63+72.75	15.44'	488.58	463.58	444.00	25'-0"	16	479.83	30

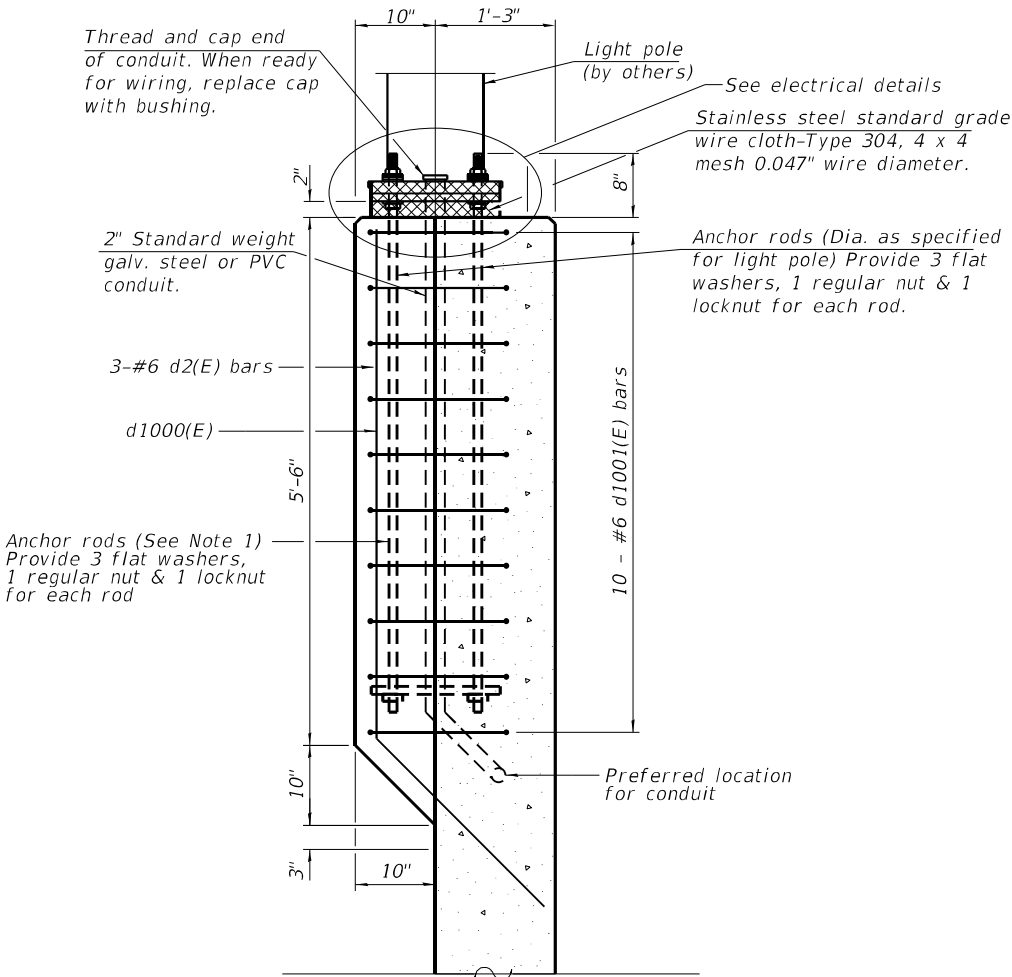
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PILE SCHEDULE 5

Pile Number	Pile Type	Station	Offset	Top of Pile Elevation	Pile Tip Elevation	Estimated Top of Rock Elevation	Estimated Pile Length	Number of Studs per Pile	Top of Encasement Elevation	Encasement Diameter (Inches)
201	W18X76	63+78.75	15.44'	488.62	463.62	444.00	25'-0"	16	479.97	30
202	W18X76	63+84.98	15.44'	488.66	463.66	444.00	25'-0"	14	480.13	30
203	W18X76	63+91.21	15.44'	488.69	463.69	444.00	25'-0"	14	480.28	30
204	W18X76	63+97.44	15.44'	488.72	463.72	444.00	25'-0"	14	480.43	30
205	W18X76	64+03.67	15.44'	488.75	463.75	444.00	25'-0"	14	480.58	30
206	W18X76	64+09.67	15.44'	488.78	463.78	444.00	25'-0"	14	480.73	30
207	W18X76	64+15.67	15.44'	488.81	463.81	444.00	25'-0"	14	480.88	30
208	W18X76	64+21.67	15.44'	488.84	463.84	444.00	25'-0"	14	481.02	30
209	W18X76	64+27.67	15.44'	488.87	463.87	444.00	25'-0"	14	481.17	30
210	W18X76	64+33.67	15.44'	488.90	463.90	444.00	25'-0"	14	481.32	30
211	W18X76	64+39.67	15.44'	488.93	463.93	444.00	25'-0"	14	481.46	30
212	W18X76	64+45.67	15.44'	488.96	463.96	444.00	25'-0"	14	481.61	30
213	W18X76	64+51.67	15.44'	488.99	463.99	444.00	25'-0"	12	481.76	30
214	W18X76	64+57.67	15.44'	489.02	464.02	444.00	25'-0"	12	481.90	30
215	W18X76	64+63.67	15.44'	489.05	464.05	444.00	25'-0"	12	482.05	30
216	W18X76	64+69.67	15.44'	489.08	464.08	444.00	25'-0"	12	482.20	30
217	W18X76	64+75.67	15.44'	489.11	464.11	444.00	25'-0"	12	482.34	30
218	W18X76	64+81.67	15.44'	489.14	464.14	444.00	25'-0"	12	482.49	30
219	W18X76	64+87.67	15.44'	489.17	464.17	444.00	25'-0"	12	482.64	30
220	W18X76	64+93.67	15.44'	489.20	464.20	444.00	25'-0"	12	482.79	30
221	W18X76	64+99.67	15.44'	489.23	464.23	444.00	25'-0"	12	482.93	30
222	W18X76	65+05.67	15.44'	489.26	464.26	444.00	25'-0"	12	483.08	30
223	W18X76	65+11.67	15.44'	489.29	464.29	444.00	25'-0"	12	483.23	30
224	W18X76	65+17.67	15.44'	489.32	464.32	444.00	25'-0"	12	483.37	30
225	W18X76	65+23.67	15.44'	489.35	464.35	444.00	25'-0"	12	483.52	30
226	W18X76	65+29.67	15.44'	489.38	464.38	444.00	25'-0"	10	483.67	30
227	W18X76	65+35.67	15.44'	489.41	464.41	444.00	25'-0"	10	483.81	30
228	W18X76	65+41.67	15.44'	489.44	464.44	444.00	25'-0"	10	483.96	30
229	W18X76	65+47.67	15.44'	489.47	464.47	444.00	25'-0"	10	484.11	30
230	W18X76	65+53.67	15.44'	489.50	464.50	444.00	25'-0"	10	484.25	30
231	W18X76	65+59.67	15.44'	489.53	464.53	444.00	25'-0"	10	484.40	30
232	W18X76	65+65.67	15.44'	489.56	464.56	444.00	25'-0"	10	484.54	30
233	W18X76	65+71.67	15.44'	489.59	464.59	444.00	25'-0"	10	484.69	30
234	W18X76	65+77.67	15.44'	489.62	464.62	444.00	25'-0"	10	484.84	30
235	W18X76	65+83.67	15.44'	489.65	464.65	444.00	25'-0"	10	484.98	30
236	W18X76	65+89.67	15.44'	489.68	464.68	444.00	25'-0"	10	485.13	30
237	W18X76	65+95.67	15.44'	489.71	464.71	444.00	25'-0"	10	485.28	30
238	W18X76	66+01.67	15.44'	489.74	464.74	444.00	25'-0"	10	485.42	30
239	W18X76	66+07.67	15.44'	489.77	464.77	444.00	25'-0"	8	485.57	30
240	W18X76	66+13.67	15.44'	489.80	464.80	444.00	25'-0"	8	485.71	30
241	W18X76	66+19.67	15.44'	489.83	464.83	444.00	25'-0"	8	485.85	30
242	W18X76	66+25.67	15.44'	489.86	464.86	444.00	25'-0"	8	485.99	30
243	W18X76	66+31.67	15.44'	489.89	464.89	444.00	25'-0"	8	486.12	30
244	W18X76	66+37.67	15.44'	489.92	464.92	444.00	25'-0"	8	486.26	30
245	W18X76	66+43.67	15.44'	489.95	464.95	444.00	25'-0"	8	486.39	30
246	W18X76	66+49.56	15.44'	489.98	464.98	444.00	25'-0"	8	486.52	30
247	W18X76	66+55.45	15.44'	490.01	465.01	444.00	25'-0"	8	486.65	30
248	W18X76	66+61.34	15.44'	490.04	465.04	444.00	25'-0"	8	486.77	30



LIGHT POLE PLAN



SECTION A-A

NOTES

- For anchor rod, grouting, and ring plate details, see Std. 836001-04.
- Cost of anchor rods and conduit is included with Concrete Structures.



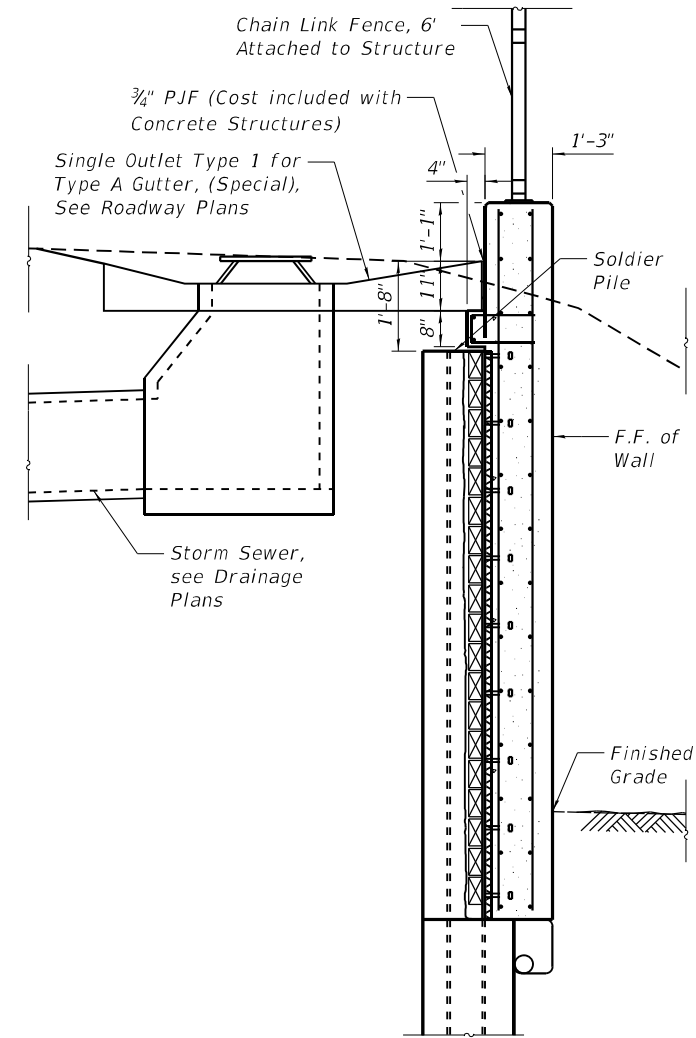
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

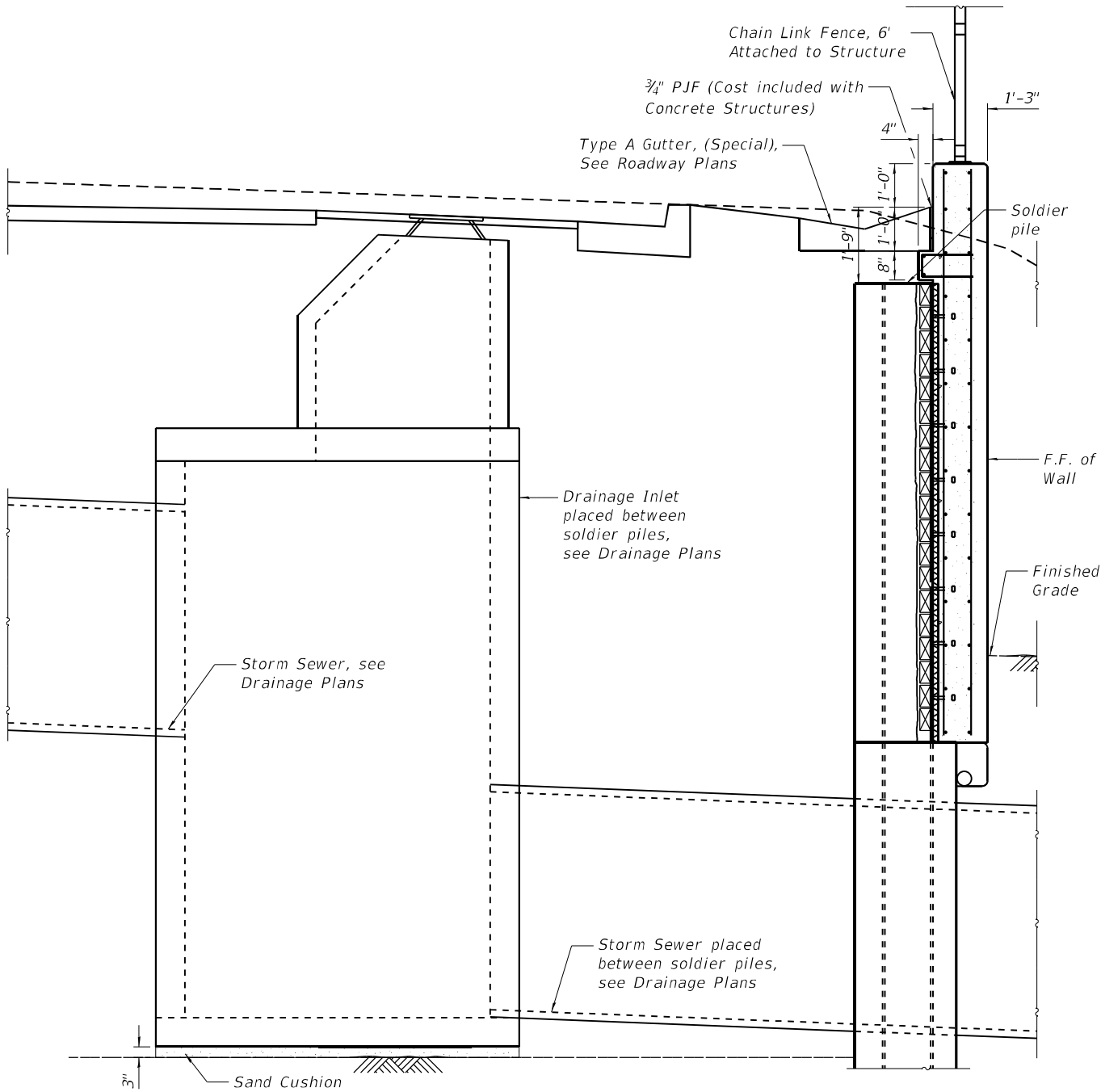
PILE SCHEDULE 3
STRUCTURE NO. 090-8517

SHEET S3-20 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1:R	TAZEWELL	1173	787
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



SECTION THRU
SOLDIER PILE WALL
at Typical Drainage Inlet



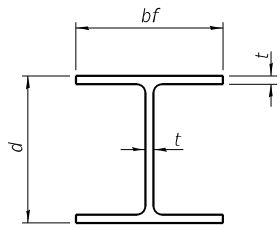
SECTION THRU
SOLDIER PILE WALL
at Drainage Inlet (Sta. 62+24.63)

SUGGESTED SEQUENCE OF CONSTRUCTION
AT DRAINAGE INLET (Sta 62+24.63)

1. Drill holes for soldier piles.
2. Set soldier pile.
3. Place soldier pile encasement concrete and controlled low-strength material (CLSM), as shown on plans.
4. Install timber lagging and excavate in front of wall.
5. Install trench protection and excavate behind wall for storm sewer installation. Cost of trench excavation and protection included with Storm Sewers, Class A, Type 2 60" per Standard Specification 550.04.
6. Install storm sewers and drainage inlet. See Drainage Plans.
7. Install geocomposite wall drain.
8. Install stud shear connectors.
9. Provide trench backfill in front of wall. Trench backfill measured with Storm Sewers, Class A, Type 2 60". See Drainage Plans.
10. Provide trench backfill behind wall. See Drainage Plans.
11. Construct concrete facing.

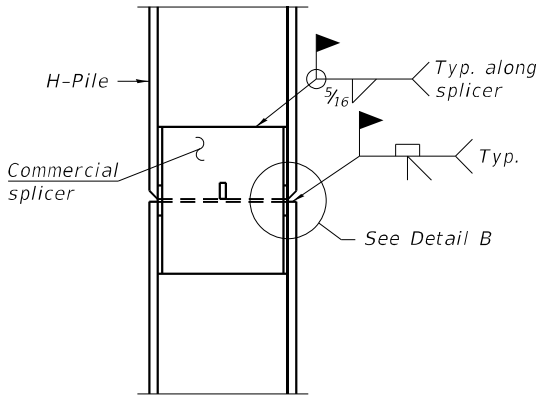
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Michael Baker INTERNATIONAL 200 W Adams Street Suite 1500 Chicago, IL 60606 P: 312.707.8910 FAX: 312.707.8804	USER NAME = DLamb	DESIGNED - YC	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CROSS SECTIONS AND DETAILS 2 STRUCTURE NO. 090-8517	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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	PLOT DATE = 6/6/2023	CHECKED - EML	REVISED -			CONTRACT NO. 68C55				
		CHECKED - JMK	REVISED -			ILLINOIS FED. AID PROJECT				
SHEET S3-22 OF S3-38 SHEETS										

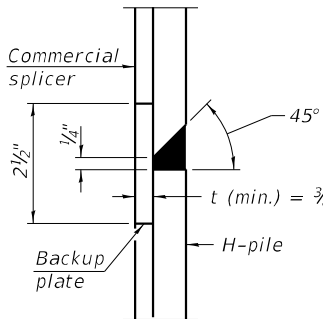


STEEL PILE TABLE

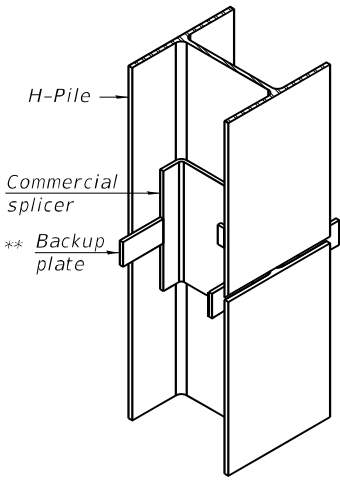
Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
W 18x211	20 ⁵ / ₈ "	11 ¹ / ₂ "	1 ¹ / ₁₆ "	30"
W 18x119	19"	11 ¹ / ₄ "	⁵ / ₈ "	30"
W 18x76	18 ¹ / ₄ "	11"	⁷ / ₁₆ "	30"



ELEVATION

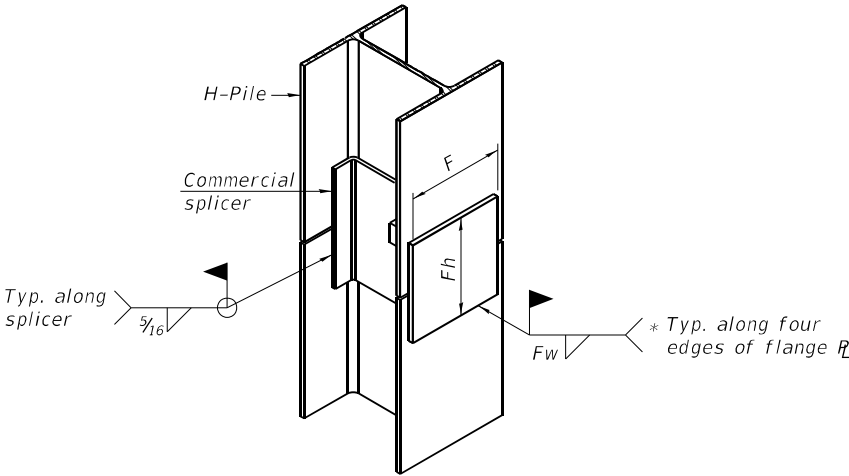


DETAIL "B"



ISOMETRIC VIEW

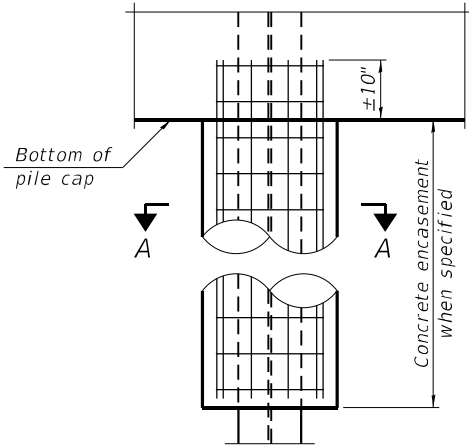
WELDED COMMERCIAL SPLICE



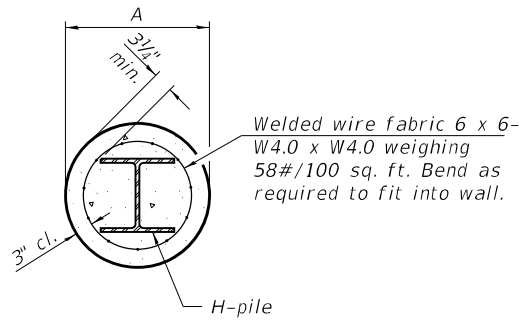
ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).

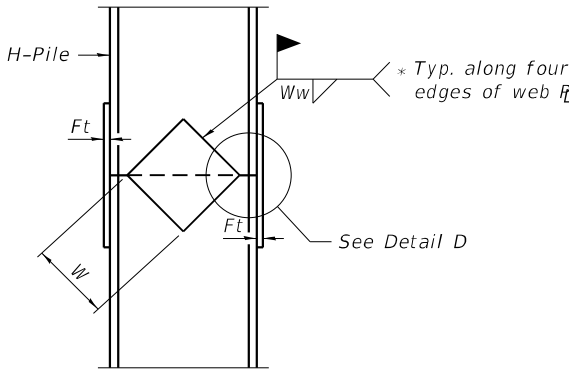


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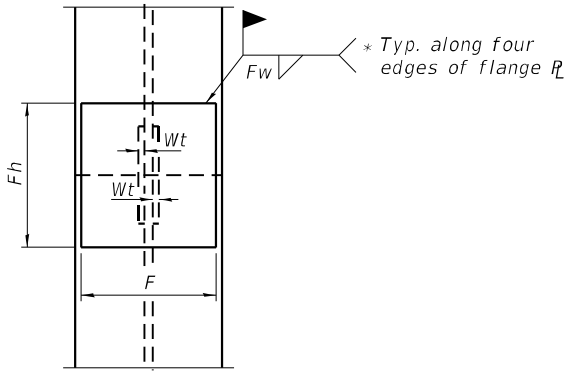


SECTION A-A

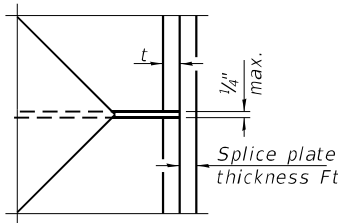
INDIVIDUAL PILE
CONCRETE ENCASEMENT
(Forms for encasement may be omitted when soil conditions permit).



ELEVATION



END VIEW



DETAIL D

Designation	F	Fh	Ft	Fw	W	Wt	Ww
W 18x211	9"	36"	2 ¹ / ₂ "	1"	10"	⁷ / ₈ "	³ / ₄ "
W 18x119	9 ¹ / ₂ "	18"	1 ¹ / ₂ "	⁷ / ₈ "	9"	⁵ / ₈ "	¹ / ₂ "
W 18x76	9 ¹ / ₂ "	12"	1"	³ / ₄ "	8"	⁵ / ₈ "	¹ / ₂ "

WELDED PLATE FIELD SPLICE

NOTES:

- The steel soldier piles shall be according to AASHTO M270 Grade 50.
- Cost of furnishing and erecting splice plates shall be included with Furnishing Soldier Piles.

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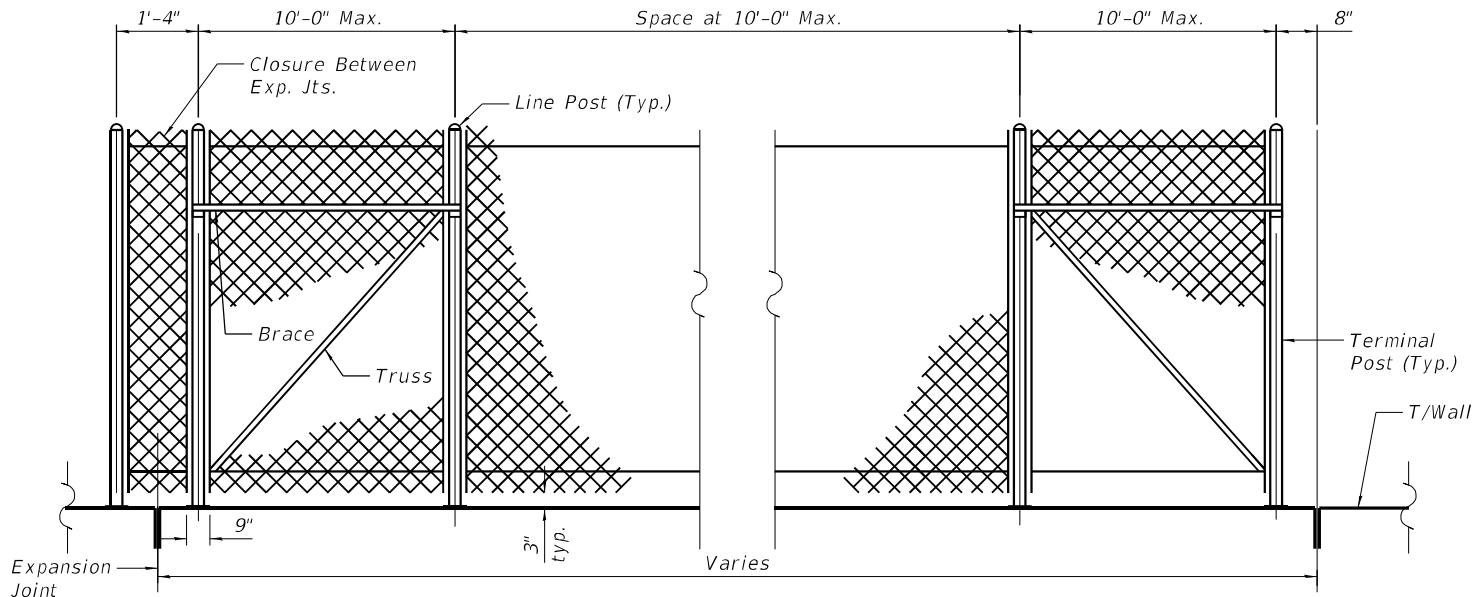
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

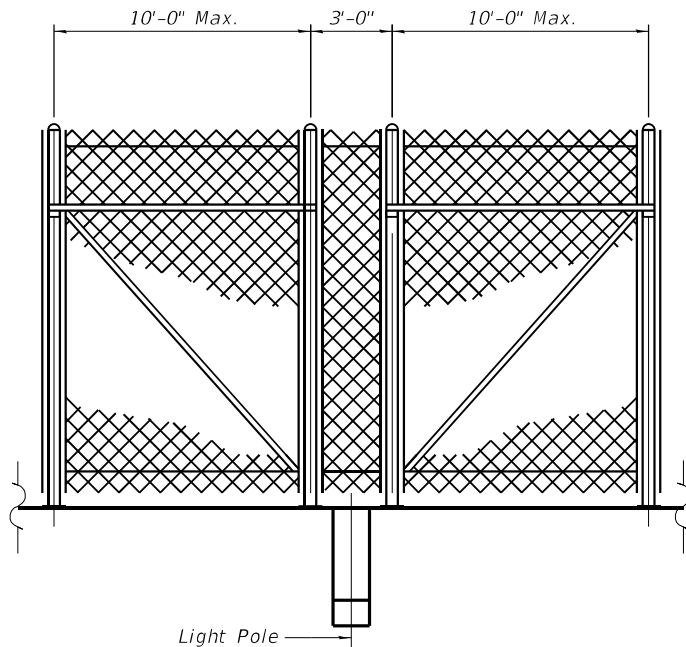
PILE DETAILS
STRUCTURE NO. 090-8517

SHEET S3-23 OF S3-38 SHEETS

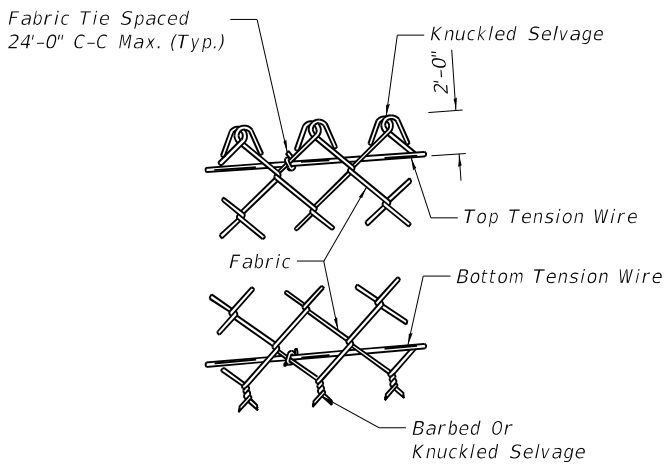
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VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	790
CONTRACT NO. 68C55				
ILLINOIS FED. AID PROJECT				



TYPICAL CHAIN LINK FENCE BETWEEN EXPANSION JOINTS



TYPICAL CHAIN LINK FENCE BETWEEN LIGHT POLES



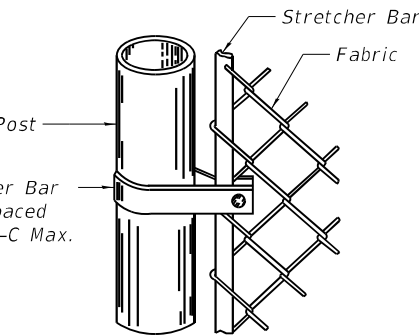
METHOD OF TYING FABRIC TO TENSION WIRES

NOTES

1. Posts, chain link fence, and attaching hardware shall be according to section 1006 of the standard specifications, except as noted, and shall be paid for at the contract unit price per meter for chain link fence (attached to structure).
2. The 9 gauge (3.759 mm) fabric ties shall be according to article 1006.27(d) of the standard specifications.
3. Installation of the chain link fabric shall be according to section 664 of the standard specifications.
4. Stretcher bars shall be used at all end posts.
5. Posts shall be according to aashto m 281, grades a or b. all other steel shapes and plates shall conform to the requirements of aashto m 270m grade 250.
6. The chain link fabric shall be placed along the side of the posts facing the railroad.
7. If the option of drilling and epoxy grouting the anchor rods is chosen, the contractor shall use the capsule or the adhesive cartridge type anchor rods that have been previously tested and given prior approval by the department. The contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures. the capsule or the adhesive cartridge shall be sealed with premeasured amounts of the adhesive chemical.
8. Space reinforcement to miss anchor rods.
9. All posts, railings, splices, anchor devices, and bent plates shall be galvanized according to article 509.05 of the standard specifications.
10. Vent holes for galvanizing shall be placed in the posts and rails at locations that will not allow the accumulation of moisture in the members.
11. The chain link fabric shall conform to the requirements of article 1006.27(a)(1)a, b or c of the standard specifications.

ANCHOR PLATE PLAN AT EXPANSION JOINT

SECTION C-C



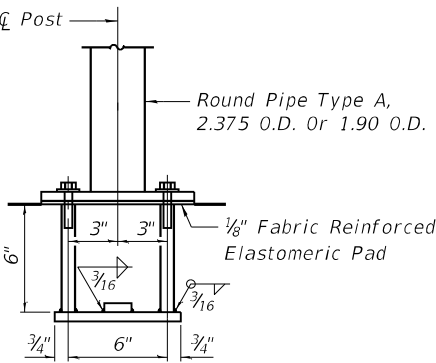
METHOD OF FASTENING STRETCHER BAR TO POST

SECTION PROPERTIES

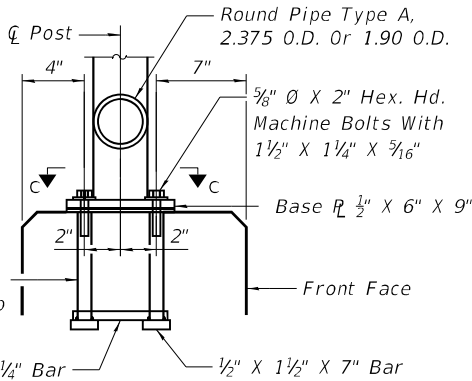
Component	Section	lbs./ft.
Line Post	Pipe Type A 1.90 O.D.	2.72
Terminal Post	Pipe Type A 2.375 O.D.	3.65
Horizontal Braces	Pipe Type A 1.66 O.D.	2.27
	Pipe Type B 1.66 O.D.	1.83

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Chain Link Fence, 6' Attached to Structure	Foot	1,350



ANCHOR BOLT DETAILS



In lieu of the cast-in-place anchor device shown, the contractor has the option of drilling and setting 5/8" ø anchor rods according to article 509.06 of the standard specifications. embedment shall be according to the manufacturer's specifications. place reinforcement bars to miss anchor rod locations.

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PLOT DATE = 6/6/2023	CHECKED - KS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FENCE DETAILS
STRUCTURE NO. 090-8517

SHEET S3-24 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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		ILLINOIS	FED. AID PROJECT	

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WANGENG 707210T.GPJ WANGENG GDT 4/26/22



wangeng@wangeng.com

Telephone:

Fax:

BORING LOG BSB-13

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88

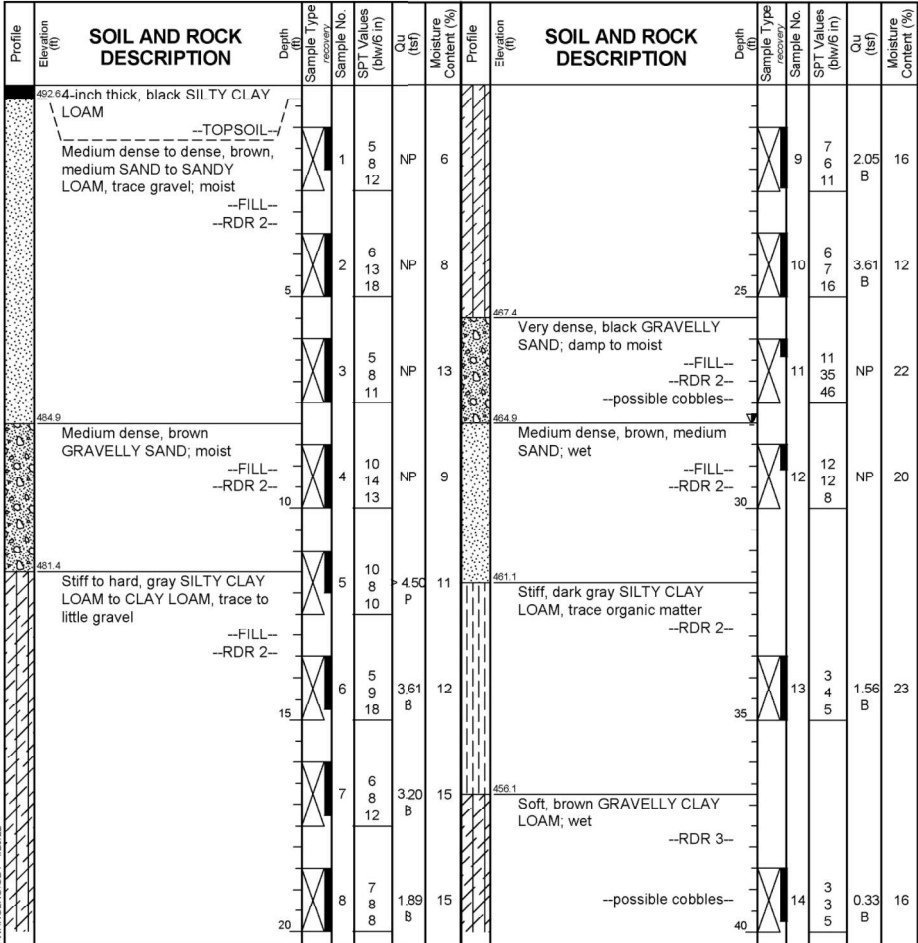
Elevation: 492.89 ft

North: 1456404.92 ft

East: 2458205.83 ft

Station: 81+81.49

Offset: 43.69 LT



GENERAL NOTES		WATER LEVEL DATA	
Begin Drilling	05-21-2019	Complete Drilling	05-22-2019
Drilling Contractor	Wang Testing Services	Drill Rig	18D25A [85%]
Driller	N&K&N	Logger	F. Bozga
Checked by	C. Marin	Time After Drilling	24 hours
Drilling Method	2.25" HSA to 15 ft, mud rotary thereafter, boring backfilled upon completion	Depth to Water	28.00 ft
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.			



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BORING LOG BSB-13

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88

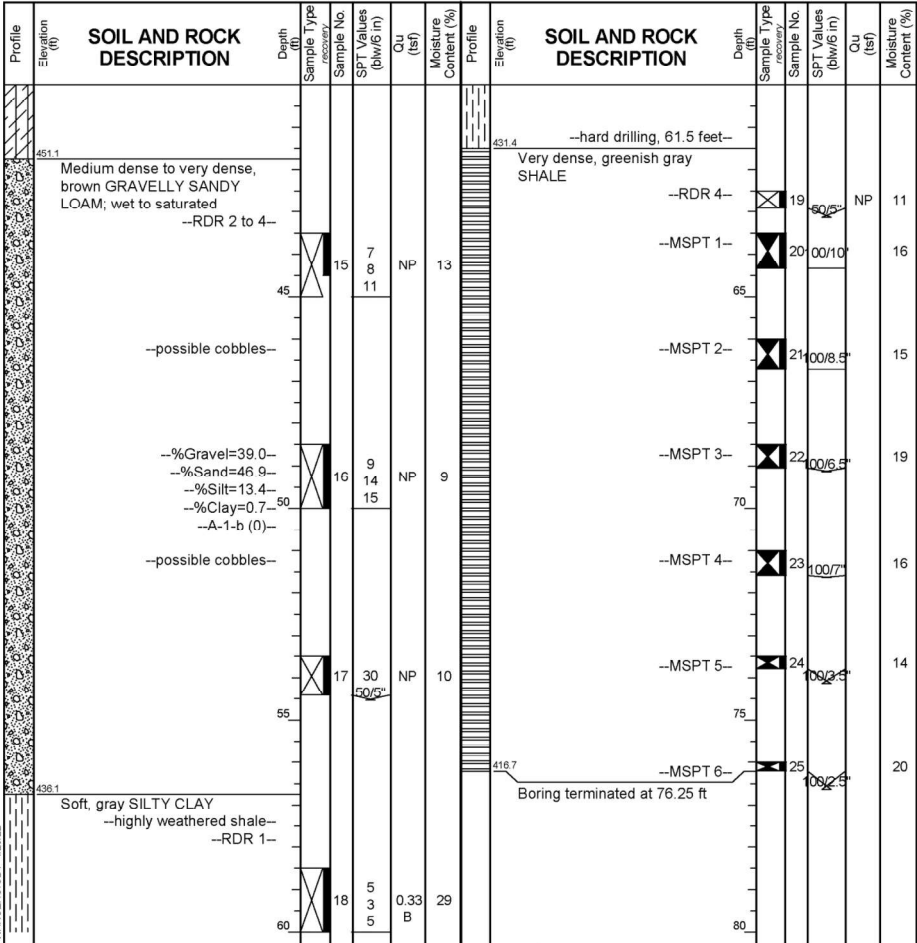
Elevation: 492.89 ft

North: 1456404.92 ft

East: 2458205.83 ft

Station: 81+81.49

Offset: 43.69 LT



GENERAL NOTES		WATER LEVEL DATA	
Begin Drilling	05-21-2019	Complete Drilling	05-22-2019
Drilling Contractor	Wang Testing Services	Drill Rig	18D25A [85%]
Driller	N&K&N	Logger	F. Bozga
Checked by	C. Marin	Time After Drilling	24 hours
Drilling Method	2.25" HSA to 15 ft, mud rotary thereafter, boring backfilled upon completion	Depth to Water	28.00 ft
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.			



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Telephone:

Fax:

BORING LOG BSB-14

WEI Job No.: 707-21-01

Client **Michael Baker International**

Project **Cedar Street Extension at IL Route 29**

Location **Tazewell County, IL**

Datum: NAVD 88

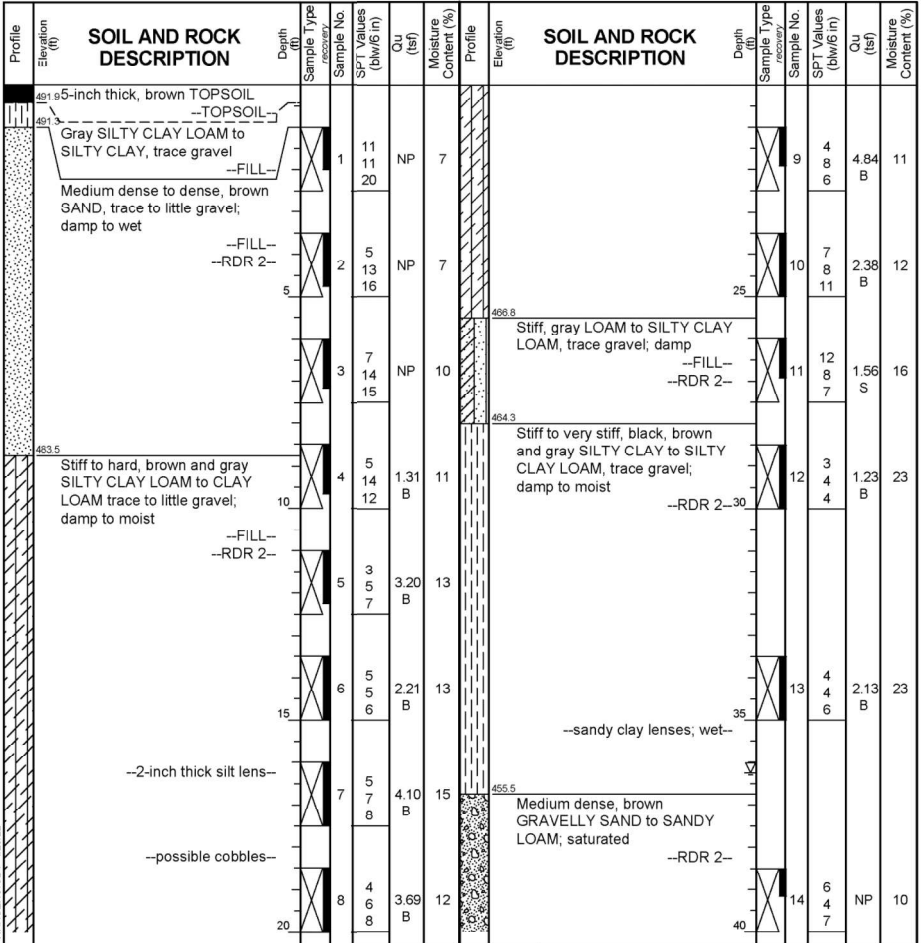
Elevation: 492.29 ft

North: 1456331.51 ft

East: 2458133.21 ft

Station: 81+85.09

Offset: 59.51 RT



GENERAL NOTES		WATER LEVEL DATA	
Begin Drilling	06-05-2019	Complete Drilling	06-05-2019
Drilling Contractor	Wang Testing Services	Drill Rig	17D50A [87%]
Driller	K&K	Logger	E. Yim
Checked by	C. Marin	Time After Drilling	NA
Drilling Method	3.25" HSA; boring backfilled upon completion	Depth to Water	NA
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.			



USER NAME	= D1amb	DESIGNED	-	REVISED	-
PLOT SCALE	= 0.1667 ' / in.	CHECKED	-	REVISED	-
PLOT DATE	= 6/6/2023	DRAWN	-	REVISED	-
		CHECKED	-	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORINGS 1
STRUCTURE NO. 090-8517

SHEET S3-25 OF S3-38 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
VAR.	13 (HVB, HB-3) BR-1;R	TAZEWELL	1173	792
CONTRACT NO. 68C55				
ILLINOIS / FED. AID PROJECT				

MODEL: Default
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WANGENG 7072101.GPJ WANGENG.GDT 4/26/22



wangeng@wangeng.com

Telephone:

Fax:

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MODEL: Default
FILE NAME: p:\wms\bus\sh\ben\w\com\bus\bus\p\w\03\Documents\Chicago\1\0\1\Project\179740_IDOT_CedarStExt_Phl15.0_CADD\CAD_Sheets\Retaining_Wall_Sheets\0908517-68C55-WallA-028.dgn
WANGENG 7072107.GPJ WANGENG GDT 4/26/22



wangeng@wangeng.com

BORING LOG RWB-06HA

WEI Job No.: 707-21-01

Datum: NAVD 88
Elevation: 481.91 ft
North: 1456433.32 ft
East: 2458447.17 ft
Station: 409+39.72
Offset: 13.9673 LT

Client: **Michael Baker International**

Project: **Cedar Street Extension at IL Route 29**

Location: **Tazewell County, IL**

Telephone:

Fax:

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)
	481.4	2-inch thick, gray SILTY CLAY, trace gravel; wet --TOPSOIL-- Soft, gray SILTY CLAY; wet --FILL-- Soft to very stiff, brown and gray CLAY LOAM, trace gravel; wet --FILL--	1	PUSH	1		0.25 P	14									
			2	PUSH	2		1.50 P	13									
			3	PUSH	3		2.00 P	13									
	474.9		4	PUSH	4		0.50 P	14									
		--sampler refusal-- Boring terminated at 7.00 ft															
			5														
			10														
			15														
			20														
GENERAL NOTES									WATER LEVEL DATA								
Begin Drilling 06-19-2019 Complete Drilling 06-19-2019									While Drilling ☒ DRY								
Drilling Contractor Wang Testing Services Drill Rig GEOPROBE									At Completion of Drilling ☒ DRY								
Driller KS&KG Logger E. Yim Checked by C. Marin									Time After Drilling NA								
Drilling Method 1" IDA Pneumatic Geoprobe LB Sampler									Depth to Water ☒ NA								
									The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.								



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BORING LOG RWB-07

WEI Job No.: 707-21-01

Datum: NAVD 88
Elevation: 485.57 ft
North: 1456421.57 ft
East: 2458399.21 ft
Station: 408+89.02
Offset: 11.6172 LT

Client: **Michael Baker International**

Project: **Cedar Street Extension at IL Route 29**

Location: **Tazewell County, IL**

Telephone:

Fax:

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)
	484.3	15-inch thick ASPHALT PAVEMENT								485.1	Medium stiff, brown CLAY LOAM						
		Stiff to hard, brown and gray CLAY LOAM to SILTY CLAY LOAM, trace to little gravel; damp	1		3	2	1.00 P	11			--RDR 1--	9		2	1	0.57 B	19
		--FILL--															
		--RDR 2--															
		--2-inch thick sand seams--	2		3	6	2.21 B	15		482.6	Stiff, brown and gray SILTY CLAY to SILTY CLAY LOAM trace gravel; damp	25		3	4	1.89 B	24
			5		6	7					--RDR 2--	10		4	6		
										458.5	Medium dense, brown GRAVELLY SAND; wet			4	5	1.72 B	20
											--RDR 2--	11		5			
										453.8	Medium dense, brown, fine SAND; saturated						
											--RDR 2--						
										448.8	Medium dense, brown, coarse SAND; saturated						
											--RDR 2--						



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BORING LOG RWB-07

WEI Job No.: 707-21-01

Datum: NAVD 88
Elevation: 485.57 ft
North: 1456421.57 ft
East: 2458399.21 ft
Station: 408+89.02
Offset: 11.6172 LT

Client: **Michael Baker International**

Project: **Cedar Street Extension at IL Route 29**

Location: **Tazewell County, IL**

Telephone:

Fax:

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blows/in)	Qu (tsf)	Moisture Content (%)
	443.8	Very stiff, gray SILTY CLAY LOAM; trace gravel								440.6	Boring terminated at 45.00 ft						
			15		5	7	2.46 B	13									
			45														
			50														
			55														
			60														
GENERAL NOTES									WATER LEVEL DATA								
Begin Drilling 06-14-2019 Complete Drilling 06-14-2019									While Drilling ☒ 27.00 ft								
Drilling Contractor Wang Testing Services Drill Rig 17B57T [91%]									At Completion of Drilling ☒ 27.00 ft								
Driller N&A&B Logger F. Bozga Checked by C. Marin									Time After Drilling NA								
Drilling Method 3.25" HSA; boringbackfilled upon completion									Depth to Water ☒ NA								
									The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.								



INFRASTRUCTURE
ENGINEERING

USER NAME = DLamb
PLOT SCALE = 0.1667' / in.
PLOT DATE = 6/6/2023

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

REVISED -
REVISED -
REVISED -
REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORINGS 5
STRUCTURE NO. 090-8517

SHEET S3-29 OF S3-38 SHEETS

F.A.P.
RTE.

SECTION

COUNTY

TOTAL
SHEETS

SHEET
NO.

VAR.

13 (HVB, HB-3) BR-1;R

TAZEWELL

1173

796

CONTRACT NO. 68C55

ILLINOIS FED. AID PROJECT

