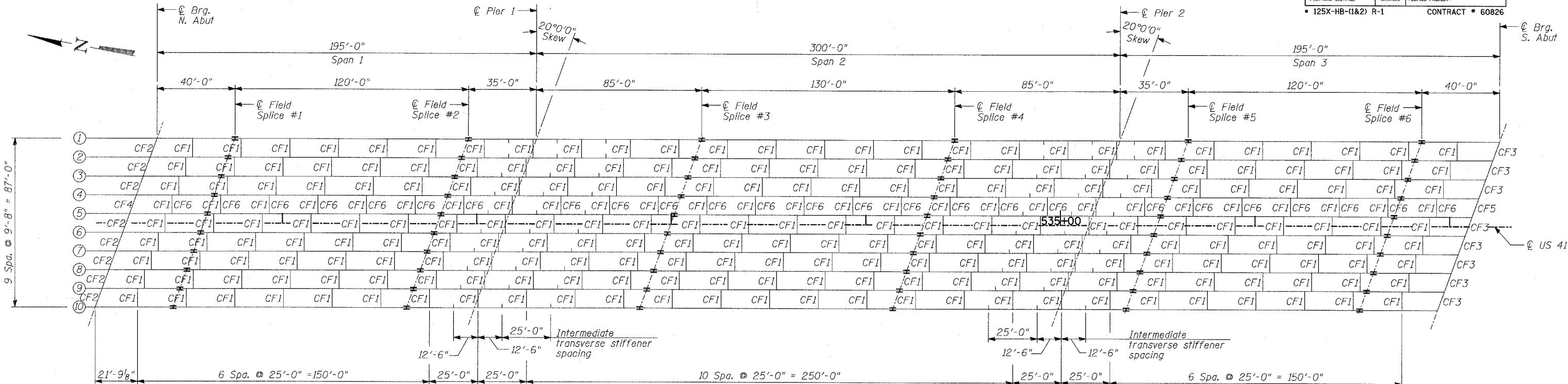


STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	*	LAKE	469	198

SHEET NO. - S-29
S-66 SHEETS



FRAMING PLAN

INTERIOR GIRDER MOMENT TABLE		
0.4 Sp. 1 & 0.6 Sp. 3	Pier	0.5 Sp. 2
<i>I_s</i> (in ⁴)	127,652	382,990
<i>I_{c(n)}</i> (in ⁴)	264,482	---
<i>I_{c(3n)}</i> (in ⁴)	192,285	293,119
<i>S_s</i> (in ³)	2,965	7,979
<i>S_{c(n)}</i> (in ³)	3,962	---
<i>S_{c(3n)}</i> (in ³)	3,544	6,324
<i>Q</i> (k/ft.)	1.43	2.47
<i>M_Q</i> ('K)	2,185	15,606
<i>s_Q</i> (k/ft.)	0.65	---
<i>M_{sQ}</i> ('K)	1,202	2,892
<i>M_L</i> ('K)	2,813	4,572
<i>M_(Imp)</i> ('K)	440	613
<i>S₃[M₄+M_(Imp)]</i> ('K)	5,422	8,642
<i>M_a</i> ('K)	11,452	31,522
<i>M_u</i> ('K)	17,609	---
<i>f_s non-comp</i> (k.s.i.)	8.84	23.47
<i>f_s comp</i> (k.s.i.)	4.08	---
<i>f_{s3(L+Imp)}</i> (k.s.i.)	16.39	13.00
<i>f_s (Overload)</i> (k.s.i.)	29.31	36.47
<i>f_s (Total)</i> (k.s.i.)	---	47.41
<i>VR</i> ('K)	87.5	---
		101.0

TYLIN INTERNATIONAL

DESIGNED	- SP
CHECKED	- AD
DRAWN	- MAF
CHECKED	- AD

INTERIOR GIRDER REACTION TABLE		
Abut.	Pier	
<i>R_Q</i> ('K)	124.9	631.6
<i>R_L</i> ('K)	72.8	178.9
<i>Imp.</i> ('K)	11.4	24.0
<i>R</i> (Total) ('K)	209.1	834.5

I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing *f_s* (Total and Overload) due to non-composite dead loads (*I_{n4}* and *I_{n3}*).
I_{c(n)}, S_{c(n)}: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "*n*", used for computing *f_s* (Total and Overload) due to short-term composite live loads (*I_{n4}* and *I_{n3}*).
I_{c(3n)}, S_{c(3n)}: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3*n*", used for computing *f_s* (Total and Overload) due to long-term composite (superimposed) dead loads (*I_{n4}* and *I_{n3}*).
Q: Un-factored non-composite dead load (kips/ft.).
M_Q: Un-factored moment due to non-composite dead load (kip-ft.).
s_Q: Un-factored long-term composite (superimposed) dead load (kips/ft.).
M_{sQ}: Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
M_L: Un-factored live load moment (kip-ft.).
M_{Imp}: Un-factored moment due to impact (kip-ft.).
M_a: Factored design moment (kip-ft.).

$$1.3 [M_Q + M_{sQ} + \frac{5}{3} (M_L + M_{Imp})]$$

M_u: Compact composite moment capacity according to AASHTO LFD 10.50.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).
f_s (Overload): Sum of stresses as computed from the moments below (ksi).

$$M_Q + M_{sQ} + \frac{5}{3} (M_L + M_{Imp})$$

f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).

$$1.3 [M_Q + M_{sQ} + \frac{5}{3} (M_L + M_{Imp})]$$

VR: Maximum *L* + impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).

NOTES:

All structural steel for girders and splices shall conform to the requirements of AASHTO M270, Grade 50. All other structural steel shall conform to the requirements of AASHTO M270, Grade 36.

FRAMING PLAN

FAP 346 (U.S. ROUTE 41 - SKOKIE HIGHWAY) OVER ILLINOIS ROUTE 132 SECTION 125X-HB-(1&2)R-1 LAKE COUNTY S.N. 049-0209