06/17/2022 LETTING ITEM 212

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STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

PLANS FOR PROPOSED BRIDGE REPLACEMENT STP-BRIDGE

TR 199 (WEST PRESTON ROAD) OVER PRESTON CREEK SECTION 20-06122-00-BR STEPHENSON COUNTY PROJECT NO. FIUZ(640)

STANDARDS (IN PROPOSAL)

STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS 000001-08 001001-02 AREAS OF REINFORCEMENT BARS DECIMAL OF AN INCH AND OF A FOOT 001006 280001-07 TEMPORARY EROSION CONTROL SYSTEMS

515001-04 NAME PLATE FOR BRIDGES 701901-08 TRAFFIC CONTROL DEVICES

720006-04

720011-01 METAL POSTS FOR SIGNS, MARKERS AND DELINEATORS

SIGN PANEL ERECTION DETAILS

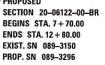
OBJECT AND TERMINAL MARKERS 725001-01 728001-01 TELESCOPING STEEL SIGN SUPPORT

APPLICATIONS OF TYPES A AND B METAL POSTS (FOR SIGNS & MARKERS) 729001-01

TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR CONSTRUCTION ON RURAL LOCAL HIGHWAYS BLR 21-9

MAILBOX TURNOUT FOR LOCAL ROADS

JOB NO. C-92-044-21

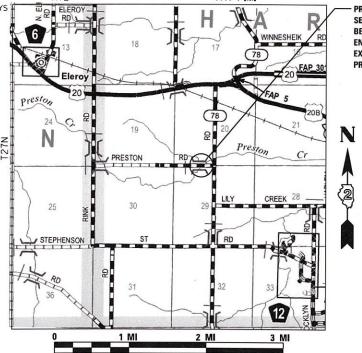


UTILITY COMPANIES:

210 W. WATER ST. PONTIAC, 61764 PHONE: (630)-576-7094

CONTACT: DESIGN STAGE LOCATE LINE

FRONTIER COMMUNICATIONS 109 E. MARKET ST. BLOOMINGTON, IL 61701 PHONE: (815) 895-1515 CONTACT: KALIN HINSHAW



LOCATION MAP

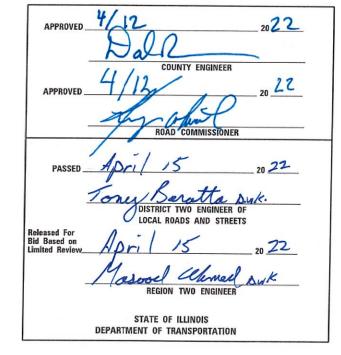
TOTAL LENGTH = 510.00 NET LENGTH = 510.00FEET (0.097 MI)

THE WORK CONSISTS OF REMOVING THE EXISTING BRIDGE AND CONSTRUCTING A NEW STRUCTURE (S.N. 089-3296) AT STA. 10+00.00 WITH A SINGLE-SPAN PRECAST PRESTRESSED CONCRETE DECK BEAM BRIDGE ON CONCRETE PILE BENT ABUTMENTS, 24'-0" CLEAR ROADWAY WIDTH AND 57'-10" BACK TO BACK OF ABUTMENTS, 0° SKEW, VARIABLE WIDTH TRANSITION APPROACHES AND OTHER COLLATERAL WORK.



LOCATION OF SECTION INDICATED THUS: -

FUNCTIONAL CLASSIFICATION = LOCAL ROAD ADT = 50DESIGN SPEED = 30 MPH **3R GUIDELINES**



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123

PROJECT MANAGER: CORY W. CHAMBERLAIN, P.E., S.E.

CONTRACT NO. 85722



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GENERAL NOTES

- 1. EXCEPT AS NOTED ON THE PLANS, PAVEMENT GRADES SHOWN ARE AT THE TOP OF PAVEMENT SURFACES.
- 2. ALL ELEVATIONS ARE BASED ON NAVD 88-DATUM.
- 3. WHERE PERMANENT SURVEY MARKERS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE DISTURBED. THE CONTRACTOR SHALL PROTECT AND PRESERVE ALL PROPERTY MARKERS AND MONUMENTS UNTIL THE OWNER AND AN AUTHORIZED SURVEYOR HAS WITNESSED OR OTHERWISE REFERENCED THE LOCATION.
- 4. ABANDONED UNDERGROUND UTILITIES THAT CONFLICT WITH CONSTRUCTION SHALL BE DISPOSED OF OUTSIDE THE LIMITS OF THE RIGHT OF WAY ACCORDING TO ARTICLE 202.03 OF THE STANDARD SPECIFICATIONS AND AS DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF CONTRACT ITEMS INVOLVED.
- 5. THE LOCATIONS OF EXISTING WATER MAINS, GAS MAINS, SEWERS, ELECTRIC POWER LINES, TELEPHONE LINES AND OTHER UTILITIES AS SHOWN ON THE PLANS ARE BASED ON THE BEST INFORMATION AVAILABLE. BUT THEY ARE NOT GUARANTEED. UNLESS ELEVATIONS ARE SHOWN. ALL UTILITY LOCATIONS SHOWN ON THE CROSS SECTIONS ARE BASED ON THE APPROXIMATE DEPTH SUPPLIED BY THE UTILITY COMPANY. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ASCERTAIN THEIR EXACT LOCATION FROM THE UTILITY COMPANIES AND BY FIELD INSPECTION.
- 6. THE FOLLOWING APPLICATION RATES HAVE BEEN USED IN CALCULATING PLAN QUANTITIES: AGGREGATE BASE/ SURFACE COURSE (& SHOULDERS): 2.05 TONS/CU YD TEMPORARY EROSION CONTROL SEEDING: 100 LB/ACRE
- 7. THE CONTRACTOR'S OPERATIONS AND TEMPORARY STORAGE ACTIVITIES SHALL BE LIMITED TO THE WORK AREA AND/OR CONSTRUCTION LIMITS. ANY ADDITIONAL STAGING AREAS ADJACENT TO THE PROJECT AREA ARE SUBJECT TO PRIOR APPROVAL BY THE ENGINEER AND MUST NOT CONFLICT WITH EXISTING SIDE ROADS, INTERSECTIONS, DRIVEWAYS, OR DRAINAGE. ALL OPERATIONS SHALL BE SUBJECT TO REGULATORY REQUIREMENTS PERMITTED FOR THIS PROJECT. NO ADDITIONAL COMPENSATION WILL BE ALLOWED TO THE CONTRACTOR FOR COMPLIANCE WITH THE ABOVE REQUIREMENTS.
- 8. DISTURBED AREAS SHALL RECEIVE PERMANENT STABILIZATION WITHIN 7 DAYS OF COMPLETION OF CONSTRUCTION ACTIVITIES. TEMPORARY STABILIZATION OF WORK AREAS IS REQUIRED FOR ALL AREAS REMAINING UNDISTURBED FOR 14 DAYS, UNLESS WORK RESUMES PRIOR TO 21 DAYS. TEMPORARY STABILIZATION MUST BE APPROVED BY THE ENGINEER.
- 9. THE CONTRACTOR SHALL USE CARE IN ALL REMOVAL ACTIVITIES NEAR ALL EXISTING ITEMS WHICH WILL REMAIN. ANY DAMAGE DONE TO EXISTING ITEMS BY THE CONTRACTOR SHALL BE REPAIRED AT THE CONTRACTORS EXPENSE.
- 10. THE CONTRACTOR SHALL RELOCATE OR REMOVE AND REPLACE ALL SIGNS THAT INTERFERE WITH THE CONSTRUCTION OPERATIONS AND TEMPORARILY RESET ALL SUCH SIGNS DURING ALL STAGES OF CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE ENGINEER WILL DETERMINE THE PERMANENT LOCATION FOR EXISTING SIGNS TO BE RE-ERECTED.
- 11. THE CONTRACTOR SHALL FOLLOW CONSTRUCTION REQUIREMENTS OF SECTION 611 WHEN EXISTING FIELD TILE IS ENCOUNTERED. ALL FIELD TILES ENCOUNTERED SHALL BE CAREFULLY PRESERVED, REPAIRED AND CONNECTED TO THE PROPOSED DRAINAGE STRUCTURES, SEWERS OR DITCHES AS DIRECTED BY THE ENGINEER. PAYMENT FOR THIS WORK SHALL BE INCLUDED IN THE VARIOUS DRAINAGE ITEMS OR AS DIRECTED BY THE ENGINEER.
- 12. EMBANKMENTS ON HILLSIDES OR SLOPES SHALL BE CONSTRUCTED ACCORDING TO ARTICLE 205.03.

GENERAL NOTES

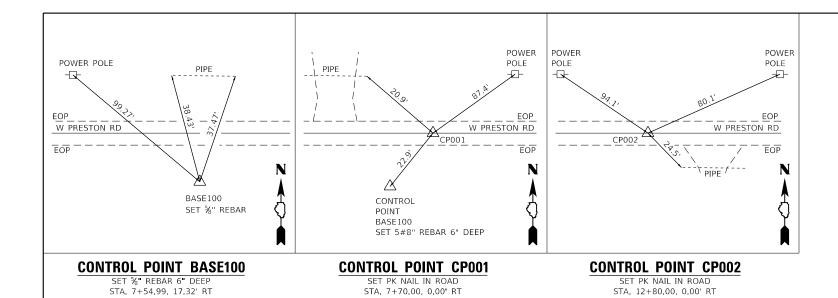
- 13. THE EXISTING A-3 ROAD SURFACE AT ALL LOCATIONS WITHIN THE PROJECT LIMITS SHALL BE DISCED, PLOWED, SCARIFIED OR OTHERWISE BROKEN UP TO A DEPTH OF NOT LESS THAN 6 INCHES PRIOR TO THE INCORPORATION OF ADDITIONAL EMBANKMENT AS DIRECTED BY THE ENGINEER. THE COST SHALL BE INCLUDED WITH EARTHWORK.
- 14. THE CONTRACTOR SHALL INSTALL ALL EROSION CONTROL MEASURES PRIOR TO STARTING ANY OTHER CONSTRUCTION WORK AT SITE.
- 15. THE CONTRACTOR SHALL VERIFY THE LENGTH OF REQUIRED PIPE CULVERTS PRIOR TO ORDERING.
- 16. THE CONTRACTOR SHALL COORDINATE THE REMOVAL OF EXISTING TRAFFIC CONTROL WITH THE COUNTY.

COMMITMENTS

1. NO COMMITMENTS AT THIS TIME.

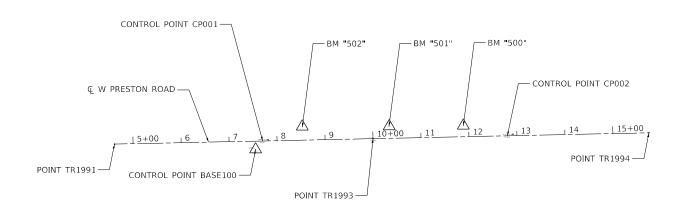
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	DRAWN -	GSJ	REVISED -	l
PLOT SCALE = 1200.0000 ' / ft.	CHECKED -	BJJ	REVISED -	l
PLOT DATE = 4/12/2022	DATE -	4/12/2022	REVISED -	

STEPHENSON COUNTY HIGHWAY DEPARTMENT



N 2,057,458.607

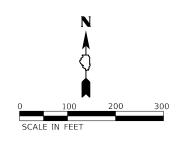
E 2,416,341.068



BM "500" STA. 11+88.82, 23.07' LT RAIL ROAD SPIKE IN SOUTH SIDE OF POWER POLE ELEV. = 792.537

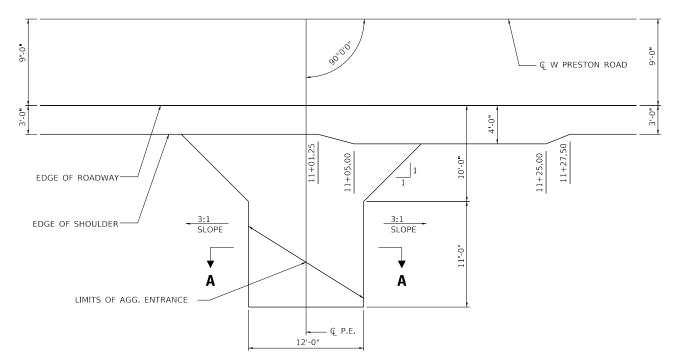
BM "501" STA. 10+33.85, 24.70' LT RAIL ROAD SPIKE IN SOUTH SIDE OF POWER POLE ELEV. = 792.446

BM "502" STA. 8+53.11. 27.07' LT RAIL ROAD SPIKE IN SOUTH SIDE OF POWER POLE ELEV. = 792.677



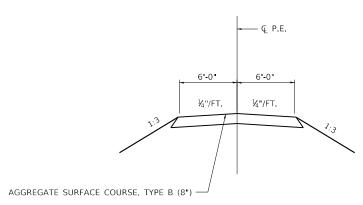
N 2,057,469.393

E 2,416,850.954



P.E. ENTRANCE

RT. STA. 11+00.00



SECTION A-A

Beginning chain TR199 description

Point TR1991 N 2,057,452.076 E 2,416,032.337 Sta

Course from TR1991 to TR1993 N 88° 47' 17.3" E Dist 538.800

10+00.00 Point TR1993 N 2,057,463.471 E 2,416,571.017 Sta

Course from TR1993 to TR1994 N 88° 47' 17.3" E Dist 575.271

N 2,057,475.638 E 2,417,146.159 Sta 15+75.27

Ending chain TR199 description



N 2,057,440.972

E 2,416,326.424

JSER NAME = gjameson	DESIGNED	-	CWC	REVISED -
	DRAWN	-	GSJ	REVISED -
PLOT SCALE = 2400.0000 / ft.	CHECKED	-	BJJ	REVISED -
PLOT DATE = 4/12/2022	DATE	-	4/12/2022	REVISED -

STEPHENSON	COUNTY	HIGHWAY	DEPARTMENT

ALIGNMENT, TIES, BENCHMARKS AND ENTRANCE DETAILS	T.R. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
PRESTON ROAD OVER PRESTON CREEK	199	20-06122-00-BR		STEPHENSON	39	3
THESTON HOAD OVER THESTON CHEEK				CONTRACT	NO.85	722
SCALE: 1" = 100' SHEET 1 OF 1 SHEETS STA. TO STA.	ILLINOIS FED. AID PROJECT					

4+61.20

		<u> </u>		
				BRIDGE
CODE	power.	.,,.,,,,,,,,	TOTAL	0010
NO.	ITEM	UNIT	QUANTITY	5.N.089-3296
20200100	EARTH EXCAVATION	CU YD	65	65
20300100	CHANNEL EXCAVATION	CU YD	235	235
20400800	FURNISHED EXCAVATION	CU YD	44	44
20000250				
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	75	75
28000305	TEMPORARY DITCH CHECKS	FOOT	55	55
28000400	PERIMETER EROSION BARRIER	FOOT	900	900
28000500	INLET AND PIPE PROTECTION	EACH	1	1
28100107	STONE RIPRAP, CLASS A4	SQ YD	299	299
28200200	FILTER FABRIC	SQ YD	299	299
35101400	AGGREGATE BASE COURSE, TYPE B	TON	432	432
40200800	AGGREGATE SURFACE COURSE, TYPE B	TON	10	10
48101200	AGGREGATE SHOULDERS, TYPE B	TON	102	102
50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1	1
50105220	PIPE CULVERT REMOVAL	FOOT	24	24

A SPECIALTY ITEMS

DESIGNED - CWC
DRAWN - G5J REVISED + REVISED engineers + planners + land surveyors PLOT SCALE = 1200:0,0000 1: 1 ft,
PLOT DATE = 4/12/2022 CHECKED - BIJ REVISEO -DATE - 4/12/2022 REVISED .

STEPHENSON COUNTY HIGHWAY DEPARTMENT

SUMMARY OF QUANTITIES PRESTON ROAD OVER PRESTON CREEK SCALE: 1" = 50' SHEET 1 OF 3 SHEETS STA. TO STA.

CONSTR. CODE

T.R., RTÉ. SECTION 199 20-06122-00-BR

CODE			TOTAL	BRIDGE 0010
NO.	ITEM	UNIT	QUANTITY	5.N.089-3296
50200100	STRUCTURE EXCAVATION	CU YD	90	90
50300225	CONCRETE STRUCTURES	CU YD	23.8	23.8
50400405	PRECAST PRESTRESSED CONCRETE DECK BEAMS (21" DEPTH)	SQ FT	1344	1344
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	3220	3220
50900205	STEEL RAILING, TYPE S1	FOOT	112	112
51200957	FURNISHING METAL SHELL PILES 12" X 0.250"	FOOT	320	320
51202305	DRIVING PILES	FOOT	320	320
51203200	TEST PILE METAL SHELLS	EACH	2	2
51500100	NAME PLATES	EACH	1	1
542D0220	PIPE CULVERTS, CLASS D, TYPE 1 15"	FOOT	28	28
59300100	CONTROLLED LOW-STRENGTH MATERIAL	CU YD	34.2	34.2
67100100	MOBILIZATION	L SUM	1	1
72501000	TERMINAL MARKER - DIRECT APPLIED	EACH	4	4
X0327902	MAILBOX REMOVE AND REPLACE	EACH	1	1

A SPECIALTY ITEMS

engineers + planners + land surveyors

STEPHENSON COUNTY HIGHWAY DEPARTMENT

SCALE: 1" = 50' SHEET 2 OF 3 SHEETS STA. TO

CONSTR. CODE

			·	
				BRIDGE
CODE			TOTAL	0010
NO.	ITEM	UNIT	QUANTITY	5,N.089-3296
X2501000	SEEDING, CLASS 2 (SPECIAL)	ACRE	0.5	0.5
			_	_
X7010216	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	L SUM	1	1
Z0013798	CONSTRUCTION LAYOUT	L SUM	1	1
		·		
			, and the second	

·····				

n firm (4001036 **whks** engineers + planners + land surveyors

A SPECIALTY ITEMS

STEPHENSON COUNTY HIGHWAY DEPARTMENT

SUMMARY OF QUANTITIES
PRESTON ROAD OVER PRESTON CREEK

SCALE: 1" = 50' SHEET 3 OF 3 SHEETS STA. TO STA.

CONSTR. CODE

	20200100	20300100	FOR INFORMATION	20400800	
			EXCAVATION TO BE		EARTHWORK
LOCATION	EARTH	CHANNEL	USED IN EMBANKMENT	EMBANKMENT	BALANCE
	EXCAVATION	EXCAVATION	(ADJUSTED FOR SHRINKAGE)	(FILL)	WASTE(+) AND
			(EXC. x 0.75)		SHORTAGE (-)
	CU. YD.	CU. YD.	CU. YD.	CU. YD.	CU. YD.
STA. 7+70.00 TO STA. 9+71.09	21		16	74	- 58
STA. 9+71.09 TO STA. 10+28.92		235	* 88		88
STA. 10+28.92 TO STA. 12+80.00	44		33	107	-74
TOTAL	65	235	137	181	- 44

EARTH EXCAVATION SHRINKAGE FACTOR = 25%

28000250 TEMPORARY EROSION CONTROL SEEDING

TR 199	(WEST PRES	TON R	OAD)			POUND
LT	7 + 70.00	TO	LT	9+71.09		18
RT	7+70.00	TO	RT	9+71.09		18
LT	10+28.92	TO	LT	12+80.00		21
RT	10+28.92	ТО	RT	12+80.00		18
					TOTAL	75

28000305 TEMPORARY DITCH CHECKS

TR 199	(WEST PRESTON ROAD)		FOOT
LT	9+25.00		11
RT	9+25.00		11
RT	10+50.00		11
LT	10+75.00		11
LT	11+75.00		11
		TOTAL	55

28000400 PERIMETER EROSION BARRIER

TR 199	(WEST PRES	TON R	OAD)			FOOT
LT	7+70.00	TO	LT	9+71.09		200
RT	7+70.00	TO	RT	9+71.09		200
LT	10+28.92	TO	LT	12+80.00		250
RT	10+28.92	TO	RT	12+80.00		250
					TOTAL	900

28000500 INLET AND PIPE PROTECTION

TR 199 (WEST PRESTON ROAD)		EACH
20.5' RT 11+12.00		1
	TOTAL	1

35101400 AGGREGATE BASE COURSE, TYPE B

TR 199	(WEST PRES	TON R	OAD)			TON
CL	7 + 70.00	TO	CL	8+20.00		50
CL	8+20.00	TO	CL	9+71.09		143
CL	10+28.92	TO	CL	12+30.00		190
CL	12+30.00	TO	CL	12+80.00		49
					TOTAL	432

40200800 AGGREGATE SURFACE COURSE, TYPE B

TR 199	(WEST PRESTON ROAD)		TON
RT	11+00.00		10
		TOTAL	10

48101200 AGGREGATE SHOULDERS, TYPE B

TR 199	(WEST PRES	TON R	OAD)			TON
LT	7 + 70.00	TO	LT	8+20.00		5
LT	8+20.00	TO	LT	9+71.09		18
LT	10+28.92	TO	LT	12+30.00		23
LT	12+30.00	TO	LT	12+80.00		5
RT	7+70.00	TO	RT	8+20.00		5
RT	8+20.00	TO	RT	9+71.09		18
RT	10+28.92	TO	RT	12+30.00		23
RT	12+30.00	ТО	RT	12+80.00		5
					TOTAL	102

50105220 PIPE CULVERT REMOVAL

TR 199 (WEST PRESTON ROAD)			FOOT
RT	11+00.00		24
		TOTAL	24

542D0220 PIPE CULVERTS, CLASS D, TYPE 1 15"

TR 199 (WEST PRESTON ROAD)			
RT 11+00.00		28	
	TOTAL	28	

67100100 MOBILIZATION

TR 199 (WEST PRESTON ROAD)	TR 199 (WEST PRESTON ROAD)			
	TOTAL	1		

X0327902 MAILBOX REMOVE AND REPLACE

TR 199	(WEST PRESTON ROAD)		EACH
RT	11+21.00		1
		TOTAL	1

X2501000 SEEDING, CLASS 2 (SPECIAL)

TR 199	(WEST PRES	TON R	OAD)			ACRE
LT	7+70.00	TO	LT	9+71.09		0.06
RT	7+70.00	TO	RT	9+71.09		0.06
LT	10+28.92	TO	LT	12+80.00		0.07
RT	10+28.92	ТО	RT	12+80.00		0.06
					TOTAL	0.50

X7010216 TRAFFIC CONTROL AND PROTECTION, (SPECIAL)

TR 199 (WEST PRESTON ROAD)		L SUM
	TOTAL	1

Z0013798 CONSTRUCTION LAYOUT

TR 199 (WEST PRESTON ROAD)		L SUM
	TOTAL	1

sign firm .184001036 **WhKS** engineers + planners + land surveyors

JSER NAME = gjameson	DESIGNED	-	CWC	REVISED -
	DRAWN	-	GSJ	REVISED -
PLOT SCALE = 1200:0.0000 ':" / ft.	CHECKED	-	BJJ	REVISED -
PLOT DATE = 4/12/2022	DATE	-	4/12/2022	REVISED -

STEPHENSON	COUNTY	HIGHWAY	DEPARTMENT

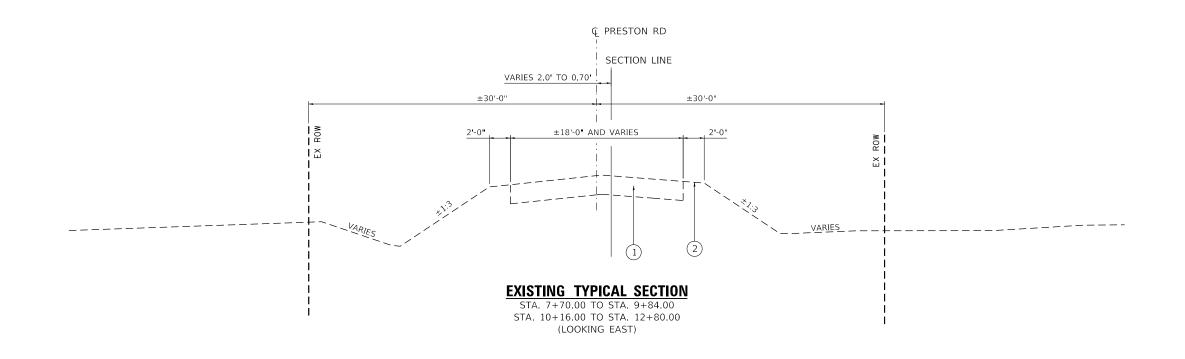
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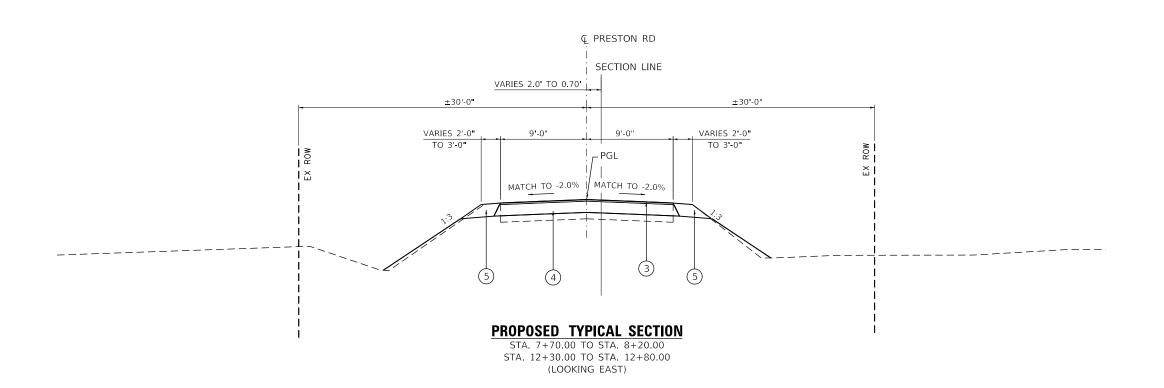
SCHEDULE OF QUANTITIES	T.R. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
PRESTON ROAD OVER PRESTON CREEK	199	20-06122-00-BR	STEPHENSON	39	7
THESTON HOAD OVER THESTON CHEEK			CONTRACT	NO.85	722
SHEET 1 OF 1 SHEETS STA TO STA		TILLINOIS FED A	D PROJECT		

^{*} ASSUMES 50% OF THE CHANNEL EXCAVATION IS SUITABLE FOR THE EMBANKMENT



- 1) EX OIL AND CHIP
- 2 EARTH SHOULDER
- 3 FUTURE OIL AND CHIP (BY OTHERS)
- (4) AGGREGATE BASE COURSE, TYPE B (8")
- (5) AGGREGATE SHOULDERS, TYPE B (8")
- (6) EMBANKMENT





SCALE: 1" = 5'

design firm no. 184001036 Whks engineers + planners + land surveyors

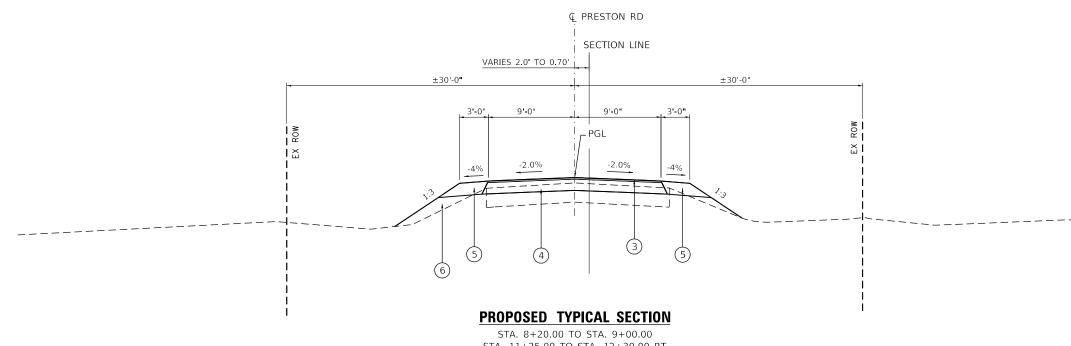
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	DRAWN	-	GSJ	REVISED -
PLOT SCALE = 120.0000 / ft.	CHECKED	-	BJJ	REVISED -
PLOT DATE = 4/12/2022	DATE	-	4/12/2022	REVISED -

STEPHENSON	COUNTY	HIGHWAY	DEPARTMENT

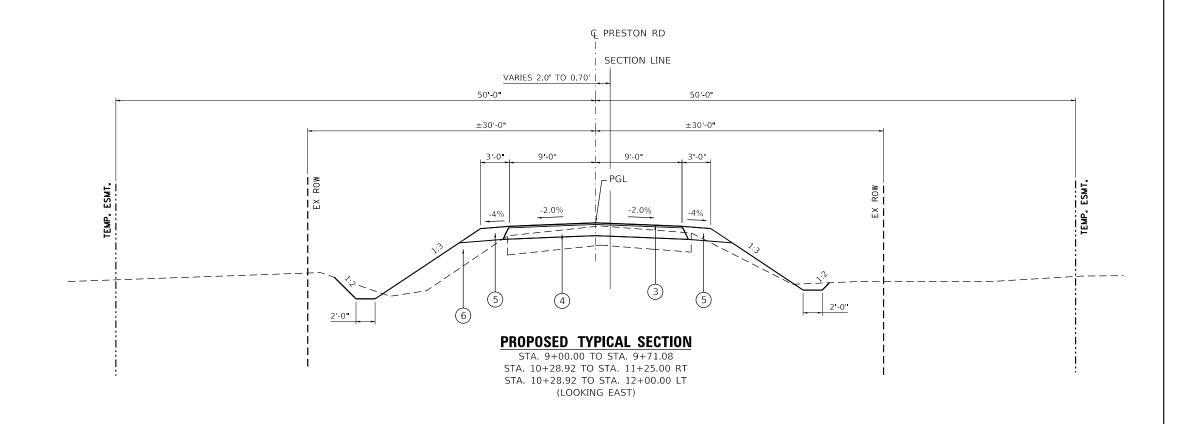
					T.R. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
PRESTON ROAD OVER PRESTON CREEK				199	20-06122-00-BR	STEPHENSON	39	8		
	THESTON	IIUAD	OVEN	IILSTON	CILLIN			CONTRACT	NO.85	722
	SHEET 1	OF 2	SHEETS	STA.	TO STA.		ILLINOIS FEE	. AID PROJECT		
_	311221 1	0. 2	SILELIS	3171.	10 3176		ILLINOIS FEE	7. AID PROJECT		_



- 1 EX OIL AND CHIP
- 2 EARTH SHOULDER
- 3 FUTURE OIL AND CHIP (BY OTHERS)
- 4) AGGREGATE BASE COURSE, TYPE B (8")
- (5) AGGREGATE SHOULDERS, TYPE B (8")
- 6 EMBANKMENT



STA. 8+20.00 TO STA. 9+00.00 STA. 11+25.00 TO STA. 12+30.00 RT STA. 12+00.00 TO STA. 12+30.00 LT (LOOKING EAST)



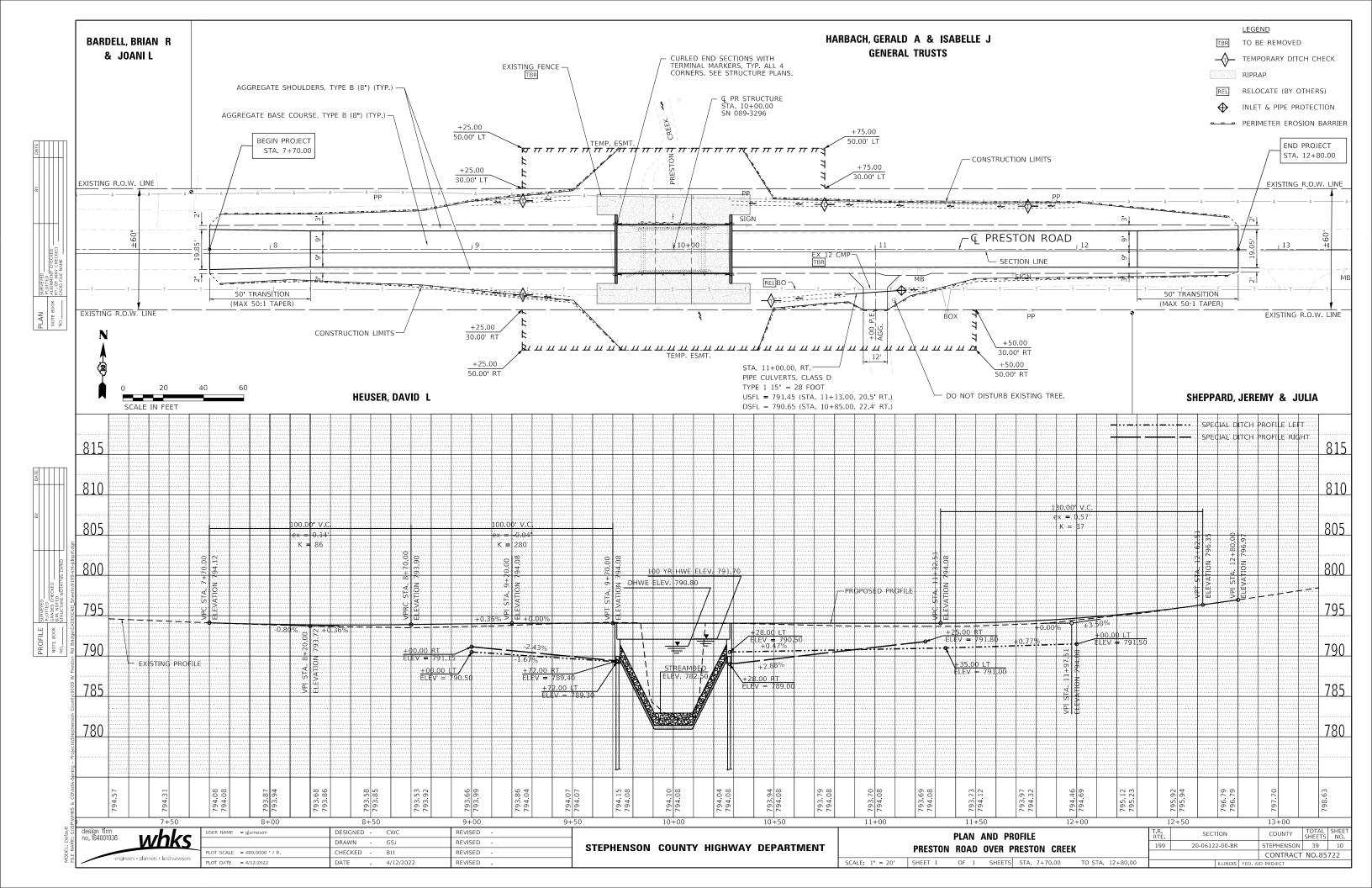
5	
j	design firm no. 184001036
į	no. 184001036
2	VULING
1	engineers + planners + land surveyors
1	

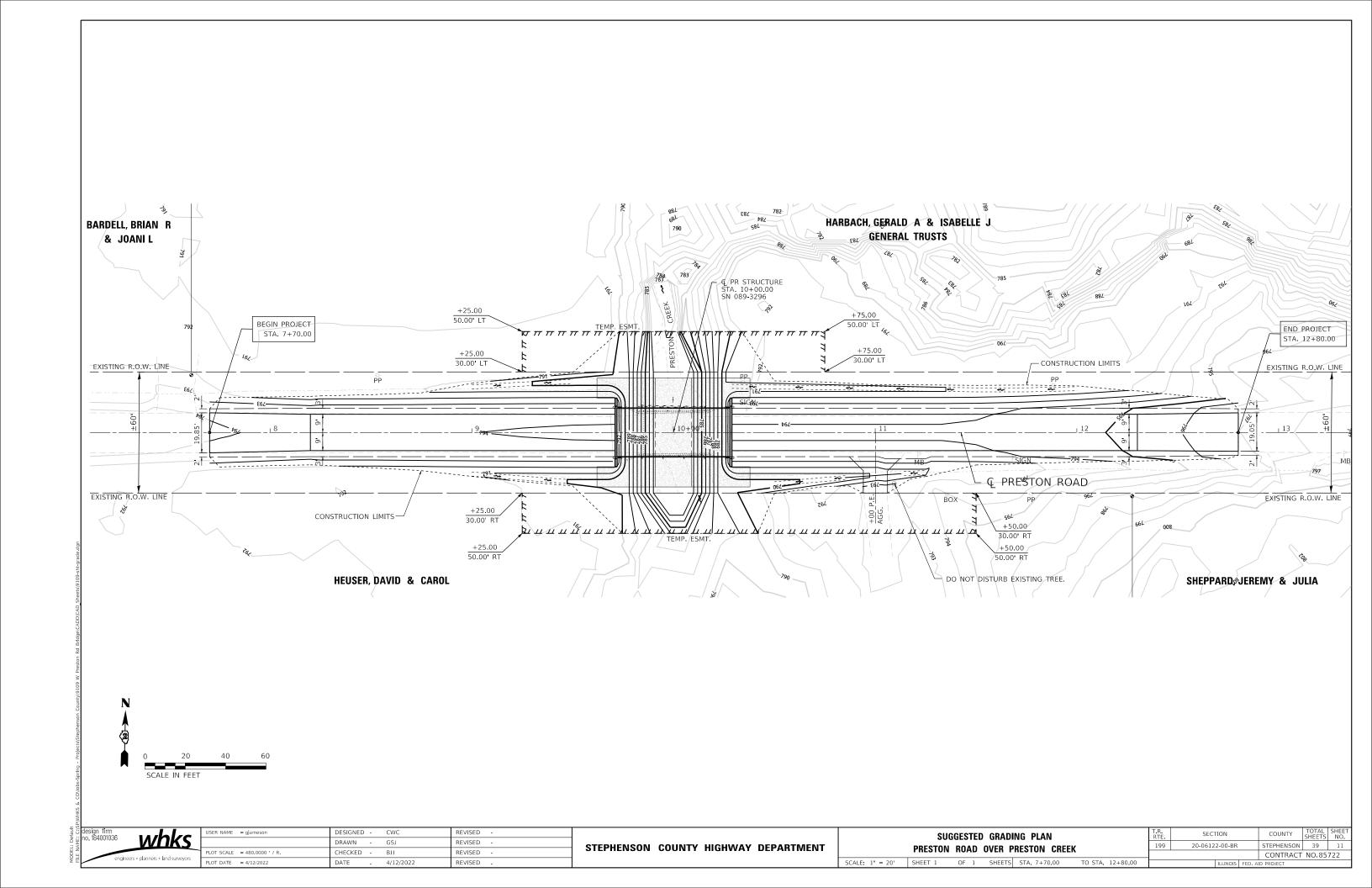
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	DRAWN -	GSJ	REVISED -
PLOT SCALE = 120.0000 / ft.	CHECKED -	BJJ	REVISED -
PLOT DATE = 4/12/2022	DATE -	4/12/2022	REVISED -

STEPHENSON	COUNTY	HIGHWAY	DEPARTMENT		
				SCALE:	1" = 5'

	TYPIC	AL SECT	IONS		
PRESTON	ROAD	OVER P	PRESTON	CREEK	
SHEET 2	OF 2	SHEETS	STA.		TO ST

T.R. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEE NO.
199	20-06122	2-00-BR		STEPHENSON	39	9
·				CONTRACT	NO.85	722
		ILLINOIS	FED. A	ID PROJECT		





Benchmark: BM "501", Sta. 10+33.85, 24.70' Lt. Railroad spike in south side of power pole., Elev. = 792.446 INDEX OF SHEETS Existing Structure: SN 089-3150 is a single span PPC deck beam bridge supported by closed concrete vertical abutments. The bridge is General Plan and Elevation 28'-0" back-to-back of abutments and the out-to-out bridge width is 22'-0". Structure is to be removed and replaced. General Data Road is to be closed during construction. 21" x 48" PPC Deck Beam Details Salvage: None Superstructure Details 56'-0" end to end beams Steel Railing, Type S1 West Abutment PPC Deck Beams Steel Railing, Type S-1 Curled End Section with Terminal See sheet 6 of 11. (21" depth) East Abutment Markers, Direct Applied, typ. Metal Shell Pile Details 10.-11. Boring Logs Elev. 789.00 Elev. 789.00 2'-0" DESIGN SPECIFICATIONS Backfill with CLSM full 2020 AASHTO LRFD Bridge Design Specifications typ. width of abutment cap Streambed Elev. 782.5 9th Edition (typ. each abutment) LOADING HL-93 Metal shell piles -Metal shell piles Allow 50#/sq. ft. for future wearing surface. ELEVATION 1. See Roadway Plans for Utilities, Grading Plan and DESIGN STRESSES $B \blacktriangleleft$ Quantities not shown on sheet 2 of 11. FIELD UNITS 2. See sheet 2 of 11 for Section A-A, Section B-B, $f'c = 3,500 \ psi$ Waterway Information Table and Design Scour Table. fy = 60,000 psi (Reinforcement) ±17'-10" Channel fy = 50,000 psi (ASTM A 252 Gr. 3, Metal Shell Piles) $B \blacktriangleleft 1$ PRECAST UNITS 10'-0" 6,000 psi Existing sheet piling to be removed. Cost included with Removal of Existing 5,000 psi typ. Structures. See Special Provisions $f'pu = 270,000 psi (\frac{1}{2}" low lax strands)$ $f'pbt = 201,960 psi (\frac{1}{2}" low lax strands)$ SEISMIC DATA Seismic Performance Zone (SPZ) = 1Design Spectral Acceleration at 1.0 sec. $(S_{D1}) = 0.053g$ Design Spectral Acceleration at 0.2 sec. (Sps) = 0.087g Soil Site Class = C CORY W. CHAMBERLAIN 081-005882 Bk. W. Abut Bk. E. Abut. G Structure Sta. 9+71.08 Sta. 10+28.92 Sta. 10+00.00 Elev. 794.08 Elev. 794.08 © TR 199 and ^{B-2}� Profile Grade PRESTON CREEK BUILT 20 BY STEPHENSON COUNTY SEC. 20-06122-00-BR TR 199 STA 10+00 STR. NO. 089-3296 LOADING HL93 NAME PLATE Name R See Std. 515001 location 57'-10" bk. to bk. abutments (Expires: 11/30/2022 4-7-22 Range 7E, 4th P.M. Project GENERAL PLAN AND ELEVATION TR 199 (WEST PRESTON RD) OVER PRESTON CREEK - Goodf SECTION 20-06122-00-BR PLANSTEPHENSON COUNTY LEGEND STATION 10+00.00 Soil Boring Location LOCATION SKETCH SN 089-3296 REVISED SECTION whks **GENERAL PLAN AND ELEVATION** STATE OF ILLINOIS CHECKED SBC REVISED 20-06122-00-BR 39 STRUCTURE NO. 089-3296 DRAWN REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 85722 SHEET 1 OF 11 SHEETS LOT DATE = 4:09:13 PM JLM/SBC REVISED CHECKED

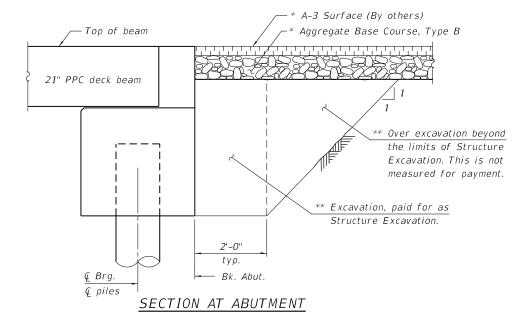
GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.
- 3. Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure.

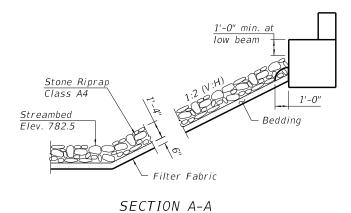
Sta. 9+50.00 Elev. 794.08	0.00%	Sta. 11+25.00 Elev. 794.08
9	PROFILE GRADE	=

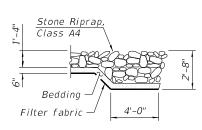
TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Channel Excavation	Cu. Yd.		235	235
Stone Riprap, Class A4	Sq. Yd.		299	299
Filter Fabric	Sq. Yd.		299	299
Removal of Existing Structures	Each	1		1
Structure Excavation	Cu. Yd.		90	90
Concrete Structures	Cu. Yd.		23.8	23.8
Precast Prestressed Concrete Deck Beams (21" Depth)	Sq. Ft.	1,344		1,344
Reinforcement Bars, Epoxy Coated	Pound		3,220	3,220
Steel Railing, Type S1	Foot	112		112
Furnishing Metal Shell Piles 12" x 0.25"	Foot		320	320
Driving Piles	Foot		320	320
Test Pile Metal Shells	Each		2	2
Name Plates	Each	1		1
Controlled Low-Strength Material	Cu. Yd.		34.2	34.2
Terminal Marker - Direct Applied	Each	4		4



^{*} See roadway plans.





SECTION B-B

WATERWAY INFORMATION TABLE

					Existing	Low Grade	Elevation:	793.06 ft. @ :	Sta. 8+50
Drainage Area	= 3.04 sq. mi				Proposed	Low Grade	Elevation:	793.62 ft. @	Sta. 8+75
Flood Event	Frequency	Discharge	Waterway O	Waterway Opening (sq ft)		Неа	nd (ft)	Headwate	r Elev. (ft)
T TOOU E VEHL	Year	cfs	Existing	Proposed	H.W.E.	Existing	Proposed	Existing	Proposed
	10	915	218	283	790.6	0.0	0.0	790.6	790.5
Design	15	1085	224	290	790.8	0.1	0.0	790.9	790.8
	20	1190	230	297	791.0	0.1	0.0	791.1	791.0
	30	1345	236	307	791.2	0.2	0.0	791.4	791.2
	50	1510	242	317	791.4	0.2	0.0	791.6	791.4
Base	100	1780	251	333	791.7	0.3	0.0	792.0	791.7
Check	200	2065	260	348	792.0	0.4	0.0	792.4	792.0
Max. Calc.	500	2470	27.5	353	792.5	0.6	0.1	793.1	792.6

10-Year Velocity through Existing: 3.7 ft/s 10-Year Velocity through Proposed: 3.2 ft/s

DESIGN SCOUR ELEVATION TABLE

Event / Limit State	Design Scou	Item 113	
	S. Abut.	N. Abut.	
Q100	_	_	
Q200	_	_	0
Design	789.00	789.00	0
Check	789.00	789.00	

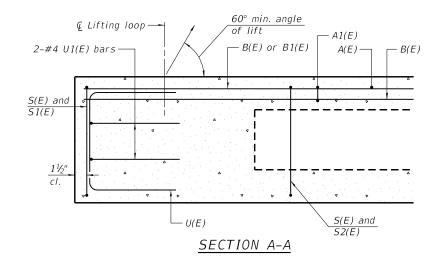


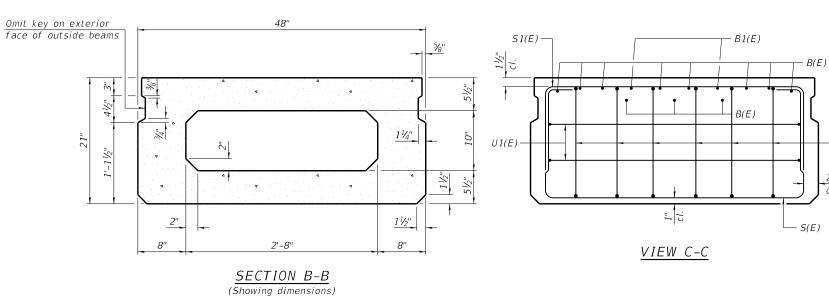
USER NAME = jmontrey	DESIGNED - JLM	REVISED -
	CHECKED - SBC	REVISED -
PLOT DATE = 4/11/2022	DRAWN - DLH	REVISED -
PLOT DATE = 4:10:54 PM	CHECKED - JLM/SBC	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL DATA	T.R. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 089-3296		20-06122-00-BR		Stephenson	39	13
				CONTRACT	NO. 857	722
SHEET 2 OF 11 SHEETS		ILLINOIS	EED ΔΙ	D PROJECT		

Backfill with Controlled Low-Strength Material (CLSM) (Mix 2) behind abutments. Backfill behind wingwalls in accordance with Article 502.10 of the Standard Specifications.





- B1(E)

S(E)

- B(E) 📆 📆

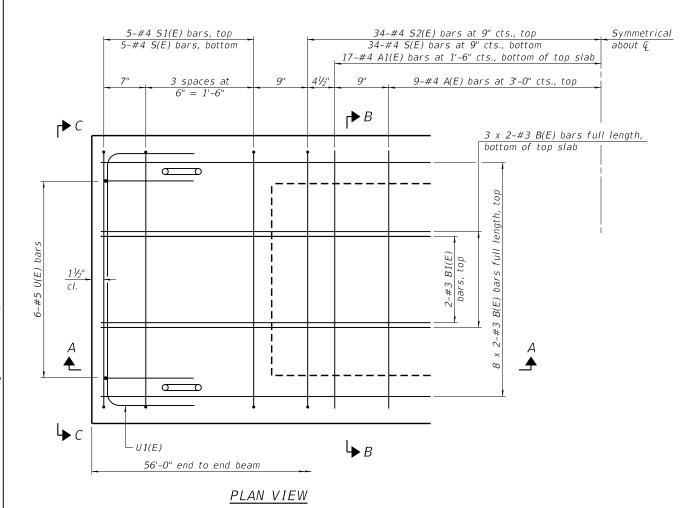
2"

— 2 strands

— 2 strands — 2 strands

— 12 strands

- 10 strands



Spacing of S(E) and S2(E) bars may be adjusted

up to 4" in the immediate area of the transverse tie diaphragms to miss the block outs for the

SECTION B-B

(Showing reinforcement and permissible strand locations)

0 0 0 0 0 0 0 0 0

0 0 0 0 0 0 0 0 0

9 spa. at 2" cts.

 \vdash A1(E) or S2(E)

A(E)-

Place the number of strands specified in each row symmetrically about the centerline of beam in the permissible strand locations shown.

 $\frac{MINIMUM BAR LAP}{\#3 bar = 1'-6''}$

BAR LIST ONE BEAM ONLY (For information only)

- U(E)

21/2"

Bar	No.	Size	Length	Shape
A(E)	18	#4	3'-7"	
A1(E)	34	#4	3'-10"	~
B(E)	22	#3	28'-8"	
B1(E)	4	#3	10'-0"	
S(E)	78	#4	7'-5"	
S1(E)	10	#4	5'-11"	
S2(E)	68	#4	6'-2"	~
U(E)	12	#5	4'-0"	
U1(E)	4	#4	6'-0"	

Notes:

- See sheet 4 of 11 for additional details and Bill of Material.
 - 2. Bars indicated thus 8 x 2-#4 etc. indicates 8 lines of bars with 2 lengths per line.

PD-2148-0

1-1-2020

transverse ties.

esign firm
o, 184001036

whise
engineers + planners + land surveyors

USER NAME	=	jmontrey	DESIGNED	-	JLIVI	KEVISED	-	
			CHECKED	-	SBC	REVISED	-	
PLOT DATE	=	4/11/2022	DRAWN	-	DLH	REVISED	-	
PLOT DATE	=	4:10:55 PM	CHECKED	-	JLM/SBC	REVISED	-	

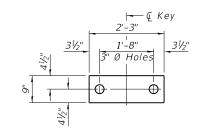
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

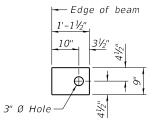
:	21"	X	48"	PP	C	DEC	K	BEAM	
	ST	RU	CTU	RE	N	0.0	89-	-3296	
	-	ucc	T 3		0.5	1.1	CHI	- CTC	

 R. E.
 SECTION
 COUNTY
 TOTAL SHEETS NO.
 SHEET NO.

 9
 20-06122-00-BR
 Stephenson
 39
 14

 CONTRACT NO. 85722



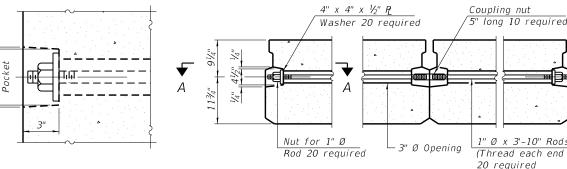


FABRIC BEARING PAD FABRIC BEARING PAD (Interior) (Exterior)

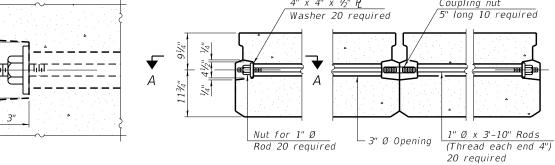
FIXED

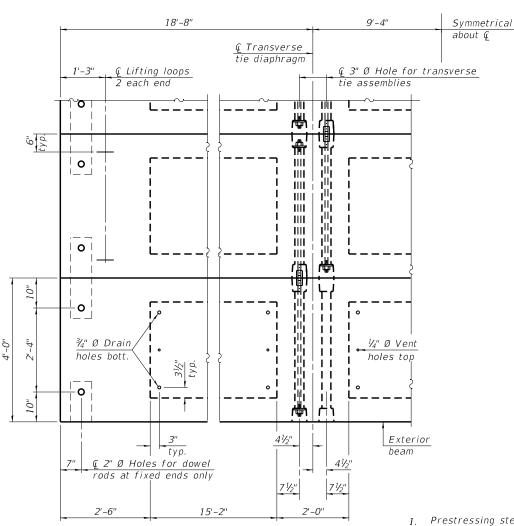
Notes:

- 1. All bearing pads shall be 1" thick.
- 2. Omit holes when using expansion bearings.
- 3. Expansion bearing pads shall be bonded to the substructure.



TYPICAL TRANSVERSE TIE ASSEMBLY





PLAN VIEW

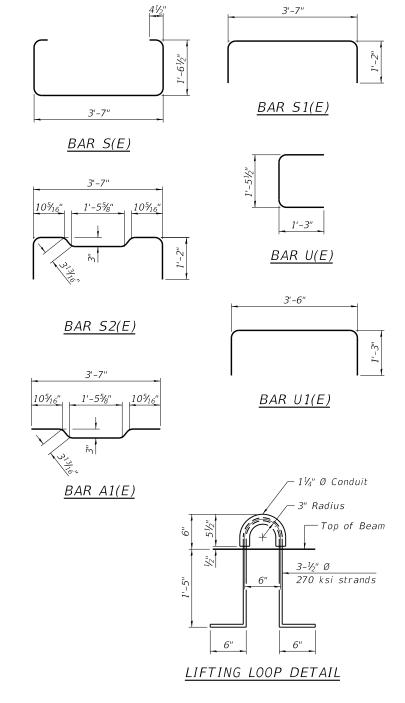
Connect beams in pairs with the transverse tie configuration shown.

NOTES

- 1. Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be V_2 " and the nominal cross-sectional area shall be 0.153 sq. in.
- 2. The 1" \emptyset rods in the transverse tie assembly shall be tightened to a snug fit and the threads set. Pockets on exterior faces of bridge shall be filled with grout after transverse tie assembly
- 3. Two $\frac{1}{16}$ fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location.
- 4. A minimum $2\frac{1}{2}$ " Ø lifting pin shall be used to engage the lifting loops during handling.
- Corrosion Inhibitor, per Article 1020.05(b)(10) and 1021.07 of the Standard Specifications, shall be used in the concrete for precast prestressed concrete deck beams.
- 6. Compressive strength of prestressed concrete, f'c, shall be 6000 psi.

SECTION A-A

7. Compressive strength of prestressed concrete at release, f'ci, shall be 5000 psi.



BILL OF MATERIAL

Precast Prestressed Sq. Ft. 1,344 Conc. Deck Bms. (21" depth)

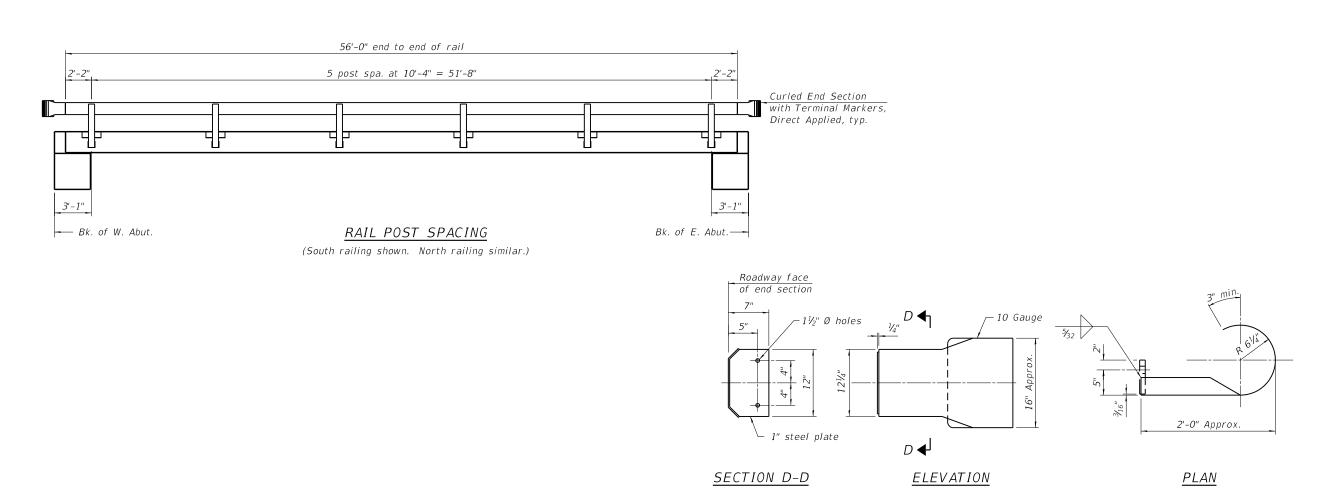
PDD-2148-0 1-1-2020

DESIGNED -REVISED CHECKED SBC REVISED LOT DATE = 4:10:55 PM CHECKED JLM/SBC REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 21" x 48" PPC DECK BEAM DETAILS STRUCTURE NO. 089-3296 SHEET 4 OF 11 SHEETS

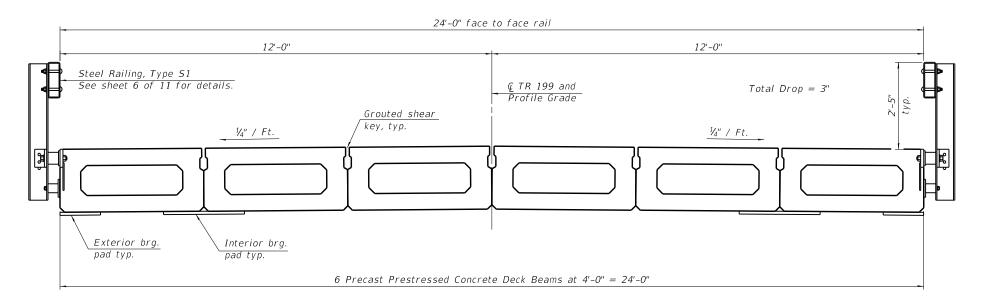
RTE. SECTION COUNTY SHEETS NO 199 20-06122-00-BR Stephenson 39 1 CONTRACT NO. 85722		ILLINOIS EED A	ID BBO IECT		
RTE. SHEETS NO			CONTRACT	NO. 85	722
RTE. SECTION COUNTY SHEETS NO	199	20-06122-00-BR	Stephenson	39	15
T.R. COUNTY TOTAL SHE		SECTION	COUNTY		SHEE NO.

whks



DETAILS OF CURLED END SECTION

(Cost included with Steel Railing, Type S1)



CROSS SECTION

(Looking East)

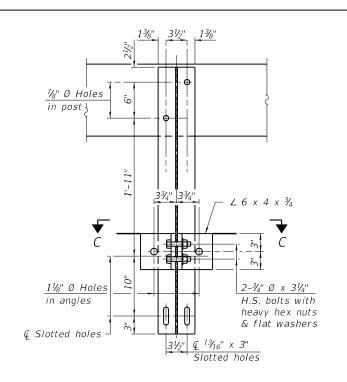
design firm	
no. 184001036	whks
	engineers + planners + land surveyors

USER NAME = jmontrey	DESIGNED	-	JLM	REVISED	-
	CHECKED	-	SBC	REVISED	=
PLOT DATE = 4/11/2022	DRAWN	-	DLH	REVISED	=
PLOT DATE = 4:10:55 PM	CHECKED	-	JLM/SBC	REVISED	=

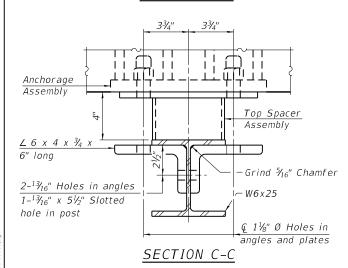
STATE C	F ILLINOIS
DEPARTMENT OF	TRANSPORTATION

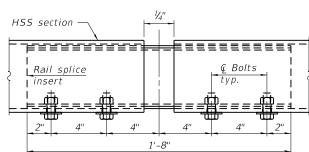
SUPERSTRUCTURE DETAILS STRUCTURE NO. 089-3296		
3111001011L 140. 003-3230		
SHEET 5 OF 11 SHEETS		

		ILLINOIS	FED. AID PROJECT			
	CONTRACT NO. 8572					722
9	20-0612	2-00-BR		Stephenson	39	16
E.	SECT	TON		COUNTY	SHEETS	SHEET NO.



SECTION B-B

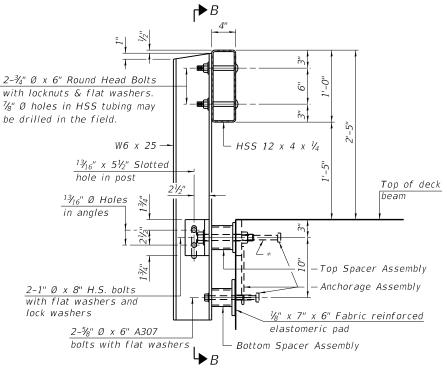




RAIL SPLICE ELEVATION

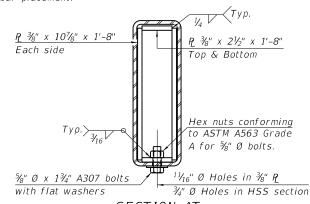
RAILING CRITERIA

NCHRP	350 Test Level	2
Railing	Weight (plf)	50

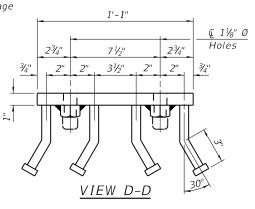


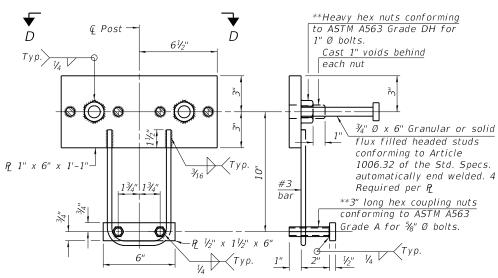
SECTION AT RAILING POST

* The outermost longitudinal reinforcement bar shall be placed directly above the studs of the rail post anchorage assembly. The anchorage studs may be bent down ½" to accommodate the top reinforcement bar placement.



SECTION AT RAIL SPLICE





ANCHORAGE ASSEMBLY

** Threaded areas shall be plugged or blocked off during casting of concrete.

No

11/8" Ø Holes for 1" Ø x 4" Round head bolts. Provide 2 flat washers & locknuts

¾" Ø Drain Hole

END OF RAIL DETAILS

for guard rail connection shown on

sheet 5 of 11.

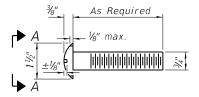
A sufficient number of shims of various thicknesses, sized to fit behind the top spacer assembly, $5" \times 11\frac{1}{2}"$, and bottom spacer assembly, $6" \times 7"$, shall be provided to adjust posts for proper alignment. If the summation of shims is greater than $\frac{1}{4}"$ (top) or $\frac{1}{2}"$ (bottom), longer bolts are required. Cost included with Steel Railing, Type S-1.

All steel rail elements including shims shall be galvanized according to Article 509.05 of the Standard Specifications.

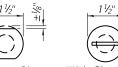
All HSS tubing serving as railing shall be CVN tested according to Article 1006.34(b) of the Standard Specifications.

Rail splice inserts may be built out of $2-\frac{3}{6}$ " bent plates in lieu of the 4 plate rail splice inserts shown, provided the outside dimensions are matched.

All round head bolts shall be ASTM A307 with locknuts according to ASTM A563 grade $\it A$.



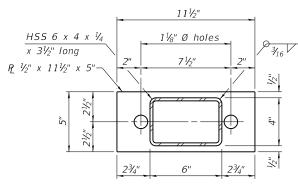
ROUND HEAD BOLT DETAIL



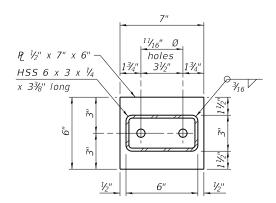
Without Slot or Recess

With Slot (shown) or Approved Recess

VIEW A-A



TOP SPACER ASSEMBLY



BOTTOM SPACER ASSEMBLY

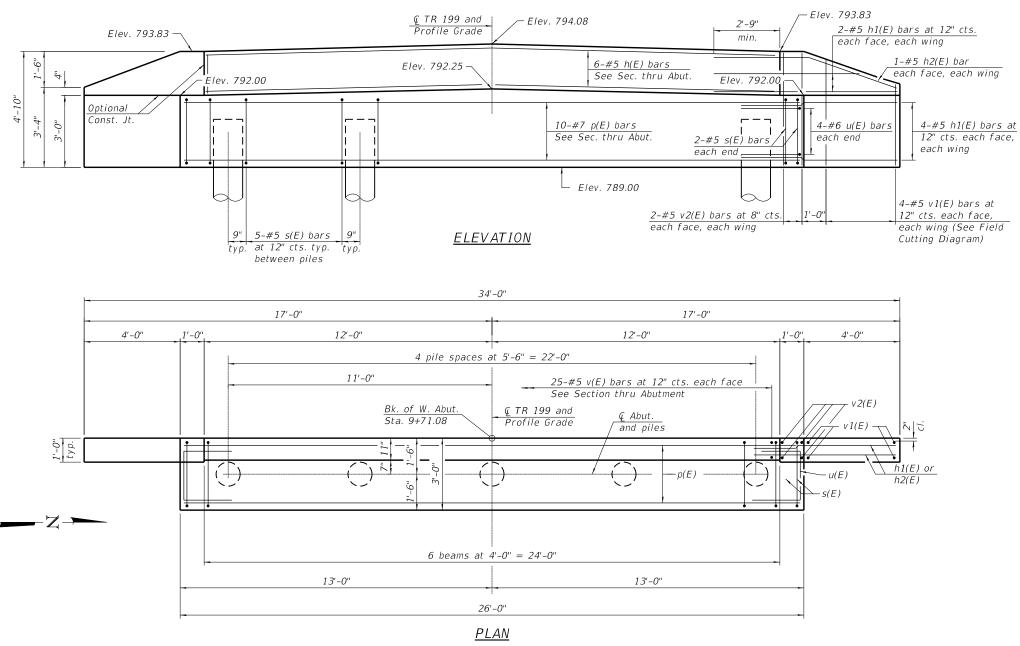
BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type S-1	Foot	112



USER NAME =	jmontrey	DESIGNED -	JLM	REVISED -
		CHECKED -	SBC	REVISED -
PLOT DATE =	4/11/2022	DRAWN -	DLH	REVISED -
PLOT DATE =	4:10:55 PM	CHECKED -	JLM/SBC	REVISED -

TE. SECTION COORT SHEETS NO. 99 20-06122-00-BR Stephenson 39 17 CONTRACT NO. 85722		III INOIE	D BBO JECT			
TE. SHEETS NO.			CONTRACT	NO. 85	722	
TE. SECTION COUNTY SHEETS NO.	19	20-06122-00-BR		Stephenson	39	17
		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	

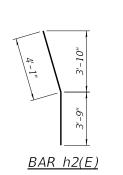


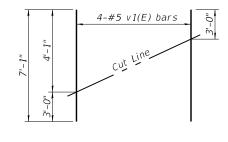
PILE DATA

Type: Metal Shell 12" x 0.25" Nominal Required Bearing: 278 kips Factored Resistance Available: 153 kips Estimated Length: 40 ft. No. Production Piles: 4 No. Test Piles: 1

Notes:

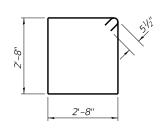
- 1. The hatched area shall be poured after the beams are in place. The backwall and wingwalls shall be cast against the beams.
- 2. Dowel holes in beams shall be filled with non-shrink grout to the top of the beam and allowed to cure for a minimum of 24 hours prior to grouting the keyways. Cost included with Precast Prestressed Concrete Deck Beams (21" Depth).
- 3. If pile interferes with dowel embedment, reduce dowel embedment at location of interference to 1'-0".
- 4. For details of piles, see sheet 9 of 11.





FIELD CUTTING DIAGRAM

Order v1(E) bars full length. Cut as shown and use remainder of bars in opposite wing.



4 | typ.

1'-6"

Bar

h(E)

h1(E)

h2(E)

3'-0"

24

v(E)

Const. jt.

h(E)-

s(E)

1" \times 2½" PJF (Full width)

1" Ø x 2'-6" Dowel rods

in cap (2 each beam end)

1" Fabric Brg. Pad

2" cl. à

- p(E)

€ abut. and piles

1'-6"

SECTION THRU ABUTMENT

See sheet 4 of 11 for Fabric Bearing Pad details.

BILL OF MATERIAL

#5

#5

No. | Size | Length | Shape

#5 7'-10"

23'-8"

7'-7"

Foot

Foot

typ.

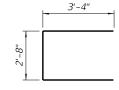
– 2" Chamfer

in 1½" Ø holes drilled

Top of beam-

Non-shrink

arout



160

160

BAR s(E)

199

BAR u(E)

oniose whks
engineers + planners + land surveyors

USER NAME = jmontrey	DESIGNED -	JLM	REVISED -
	CHECKED -	SBC	REVISED -
PLOT DATE = 4/11/2022	DRAWN -	DLH	REVISED -
DIOT DATE - 4:10:56 PM	CHECKED	II M/CDC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

				IENT 89–3296	
SHEET	7	OF	11	SHEETS	

SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
20-06122-00-BR	Stephenson	39	18
	CONTRACT	NO. 85	722
ILLINOIS FED. AL	D PROJECT		

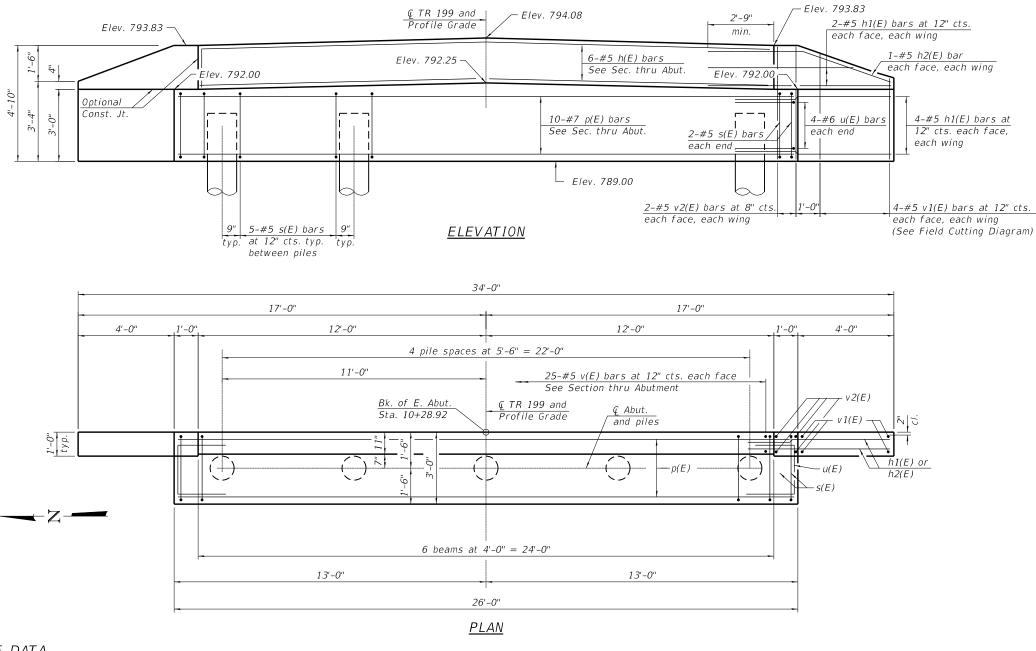
p(E) 25'-8" s(E) 24 #5 | 11'-7" 9'-4" u(E)#6 50 4'-1" v(F)#5 #5 7'-1" v1(E) 8 #5 4'-6" v2(E) Structure Excavation Concrete Structures Cu Yd 11.9 Reinforcement Bars, 1,610 Pound Epoxy Coated

Test Pile Metal Shells Each

Furnishing Metal Pile

Shells 12" x 0.25"

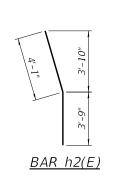
Driving Piles

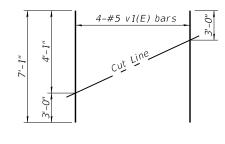


PILE DATA

Type: Metal Shell 12" x 0.25" Nominal Required Bearing: 277 kips Factored Resistance Available: 152 kips Estimated Length: 40 ft. No. Production Piles: 4 No. Test Piles: 1

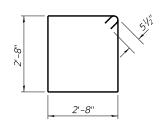
- 1. The hatched area shall be poured after the beams are in place. The backwall and wingwalls shall be cast against the beams.
- 2. Dowel holes in beams shall be filled with non-shrink grout to the top of the beam and allowed to cure for a minimum of 24 hours prior to grouting the keyways. Cost included with Precast Prestressed Concrete Deck Beams (21" Depth).
- 3. If pile interferes with dowel embedment, reduce dowel embedment at location of interference to 1'-0".
- 4. For details of piles, see sheet 9 of 11.





FIELD CUTTING DIAGRAM

and use remainder of bars in opposite wing.



Bk. Abut. — 11" 7" — & Brg.

4 | typ.

1'-6"

Bar

h(E)

h1(E)

h2(E)

p(E)

s(E)

u(E)

v1(E)

v2(E)

3'-0"

24

24

50

8

Structure Excavation

Concrete Structures

Reinforcement Bars,

Furnishing Metal Pile

Test Pile Metal Shells | Each

Shells 12" x 0.25"

Epoxy Coated

Driving Piles

v(E)

Const. jt.

h(E)-

s(E)

1" \times 2½" PJF (Full width)

1" Ø x 2'-6" Dowel rods

in cap (2 each beam end)

1" Fabric Brg. Pad

2" cl. à

- p(E)

G abut, and piles

1'-6"

SECTION THRU ABUTMENT

See sheet 4 of 11 for Fabric Bearing Pad details.

BILL OF MATERIAL

#5

#5

#6

#5

#5 #5 4'-6"

No. | Size | Length | Shape

#5 7'-10"

#5 | 11'-7"

23'-8"

7'-7"

25'-8"

9'-4"

4'-1" 7'-1"

Cu Yd

Pound

Foot

Foot

11.9

1,610

160

160

typ.

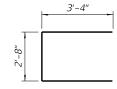
– 2" Chamfer

in 1½" Ø holes drilled

Top of beam-

Non-shrink

/ arout



BAR s(E)

 $BAR\ u(E)$

whks

DESIGNED -REVISED CHECKED SBC REVISED DRAWN REVISED LOT DATE = 4:10:56 PM CHECKED -JLM/SBC REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

EAST ABUTMENT STRUCTURE NO. 089-3296 SHEET 8 OF 11 SHEETS

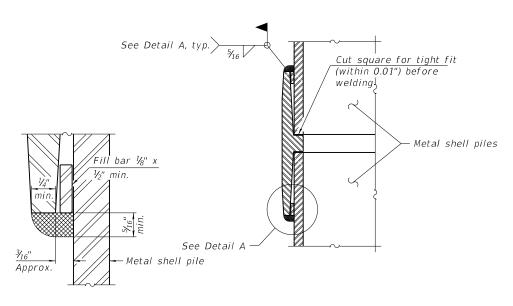
			CONTRACT	NO. 85	722	
199	20-06122-00-BR		Stephenson	39	19	
T.R. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.	

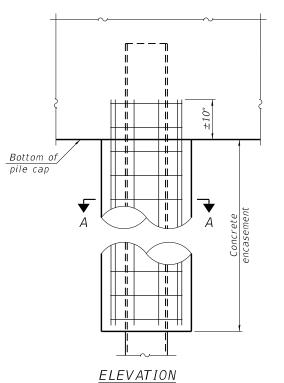
Order v1(E) bars full length. Cut as shown

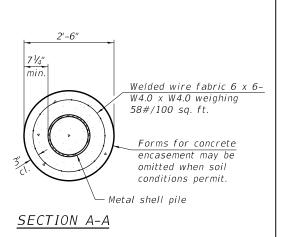


METAL SHELL PILE TABLE

Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd.³/ft.)
PP12	0.250"	31.37	0.0267
PP14	0.250"	36.71	0.0368
PP14	0.312"	45.61	0.0361
PP16	0.312"	52.32	0.0478
PP16	0.375"	62.64	0.0470





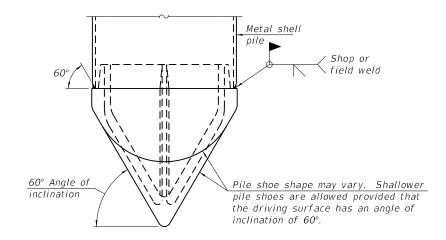


DETAIL A

Metal shell pile ¾" End plate Shop or field weld

 $s = t - \frac{1}{16}$ "

END PLATE ATTACHMENT



PILE SHOE ATTACHMENT

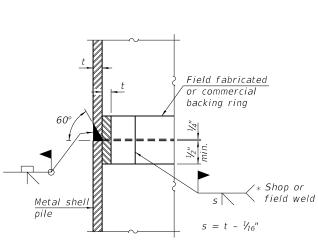
(When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A 148 Grade 80-50 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld).

WELDED COMMERCIAL SPLICE

Notes:

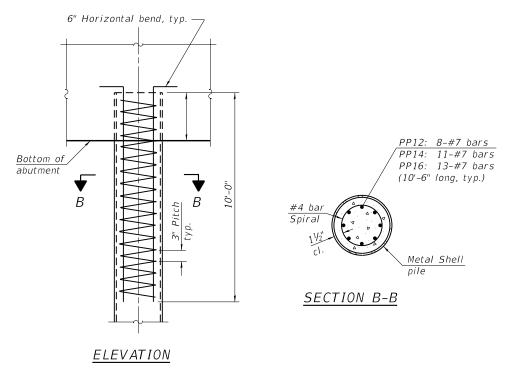
The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them. Pile segments shall be driven to solid contact with splicer before welding.

INDIVIDUAL PILE CONCRETE ENCASEMENT (When specified)



COMPLETE PENETRATION WELD SPLICE

st Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.



REINFORCEMENT AT ABUTMENTS (Omit when concrete encasement is specified)

The metal shell piles shall be according to Article 1006.05 of the Standard Specifications.



USER NAME = jmontrey	DESIGNED - JLM	REVISED -
	CHECKED - SBC	REVISED -
PLOT DATE = 4/11/2022	DRAWN - DLH	REVISED -
PLOT DATE = 4:10:56 PM	CHECKED - JLM/SBC	REVISED -

METAL SHELL PILE DETAILS		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.		
STRUCTURE NO. 089-3296	199	20-06122-00-BR			Stephenson	39	20	
31NUCTURE NO. 003-3230					CONTRACT	NO. 85	722	
SHEET 9 OF 11 SHEETS	ILLINOIS FED. AID PROJECT							

F-MS

intertek_ Professional Service Industries, Inc. 480 North Street, Springfield, Illinois 62704 Telephone: (217) 544-6663 **LOG OF BORING B-1** Fax: (217) 544-6148 Sheet 2 of 2 WATER LEVELS Drilling Method: Hollow Stem Auger PSI Job No.: 0026419-1 Sampling Method:Split Spoon Structure Replacement over Preston Creek Project: ☑ While Drilling 15.5 fee Location: Existing Structure No. 089-3150 Hammer Type: Automatic Hammer ▼ Upon Completion 14.5 feet Boring Location: See Attached Boring Location Plan Preston Road ▼ Delay Cave at 15.0 feet Stephenson County, Illinois STANDARD PENETRATION Offset: 4-ft LT TEST DATA Graphic Log Sample Type Sample No. (feet) Moisture MATERIAL DESCRIPTION Additional Remarks STRENGTH tsf ▲ Qu ₩ Qp LEAN CLAY (CL), FIRM TO VERY STIFF, GRAY, TRACE TO LITTLE SAND, TRACE # Q, = 3.0 tsf N=14 N=13 SANDY SILT (ML), MEDIUM DENSE, N=13 55-ML LEAN CLAY (CL), VERY STIFF, GRAY, TRACE SAND, moist N=12 50-CL 13-14-16 - TRACE DECOMPOSED VEGETATION N=30 45-BORING TERMINATED AT -55 FEET Completion Depth: Sample Types: Latitude: 42.3146 Longitude: -89.7229 Date Boring Started: 12/14/20 Auger Cutting Shelby Tube Drill Rig: Diedrich D-120
Remarks: 15.5-ft east of eastern bridge abutment Date Boring Completed: 12/14/20 Split-Spoon Hand Auger Logged By: M. Natali Rock Core Texas Cone

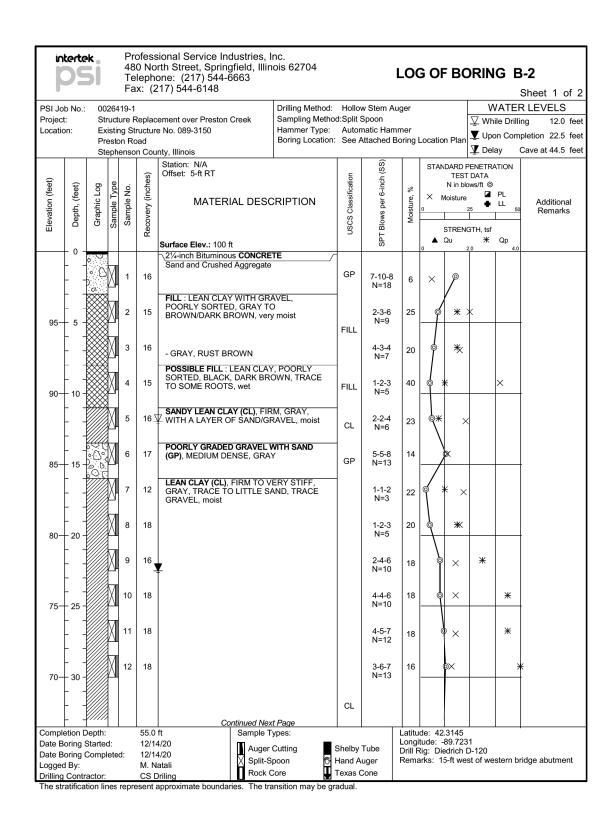
whks

DESIGNED REVISED CHECKED SBC REVISED REVISED LOT DATE = 4:10:57 PM CHECKED JLM/SBC REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

BORING LOGS STRUCTURE NO. 089-3296 SHEET 10 OF 11 SHEETS

SECTION COUNTY 20-06122-00-BR 39 21 CONTRACT NO. 85722 ILLINOIS FED. AID PROJECT



intertek_ Professional Service Industries, Inc. 480 North Street, Springfield, Illinois 62704 Telephone: (217) 544-6663 LOG OF BORING B-2 Fax: (217) 544-6148 Sheet 2 of 2 WATER LEVELS Drilling Method: Hollow Stem Auger PSI Job No.: 0026419-1 Sampling Method:Split Spoon Structure Replacement over Preston Creek Project: ☑ While Drilling 12.0 fee Location: Existing Structure No. 089-3150 Hammer Type: Automatic Hammer ▼ Upon Completion 22.5 feet Boring Location: See Attached Boring Location Plan Preston Road ▼ Delay Cave at 44.5 feet Stephenson County, Illinois STANDARD PENETRATION Offset: 5-ft RT TEST DATA Graphic Log Sample Type Sample No. (feet) Moisture MATERIAL DESCRIPTION Additional Remarks STRENGTH tsf ▲ Qu ₩ Qp LEAN CLAY (CL), FIRM TO VERY STIFF, GRAY, TRACE TO LITTLE SAND, TRACE 3-5-7 N=12 N=11 N=11 55-45 LEAN CLAY (CL), VERY STIFF, GRAY, TRACE SAND, TRACE DECOMPOSED VEGETATION, moist 5-7-9 50-CL 10-12-14 26 N=26 45-BORING TERMINATED AT -55 FEET Latitude: 42.3145 Longitude: -89.7231 Completion Depth: Sample Types: Date Boring Started: 12/14/20 Auger Cutting Shelby Tube Drill Rig: Diedrich D-120
Remarks: 15-ft west of western bridge abutment Date Boring Completed: 12/14/20 Split-Spoon Hand Auger Logged By: M. Natali Rock Core Texas Cone

The stratification lines represent approximate boundaries. The transition may be grad

nr tirm s4001036 Whks engineers + planners + land surveyors

 SER NAME
 = jmontrey
 DESIGNED
 - JLM
 REVISED

 CHECKED
 - SBC
 REVISED

 LOT DATE
 = 4/11/2022
 DRAWN
 - DLH
 REVISED

 LOT DATE
 = 4:11:01 PM
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 - JLM/SBC
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOGS
STRUCTURE NO. 089–3296

SHEET 11 OF 11 SHEETS

 T.R. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL NO.
 SHEE SHEETS
 SHEE NO.

 199
 20-06122-00-BR
 Stephenson
 39
 22

 CONTRACT NO. 85722

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