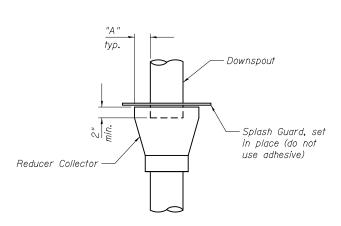
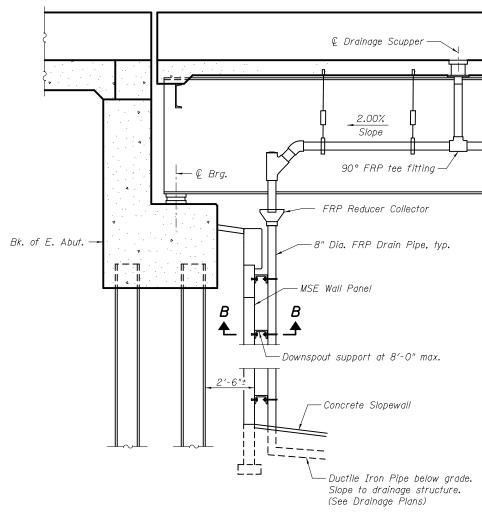


PARTIAL ELEVATION AT EAST ABUTMENT (Looking East)

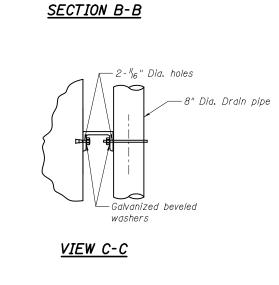


FRP REDUCER COLLECTOR DETAIL

Dim. "A" = 6" min. in longitudinal direction of deck







-Pipe Hanger at

10'-0" max., typ.

2.00%

Slope

ТΤ

45° FRP Elbow

Galvanized C8

8" Dia. FRP Drain pipe

⁵8" Dia. Stainless steel u-bolt ASTM A 276, type 304, condition A, cold finished

with 2 washers and nuts

FRP Cleanout Plug

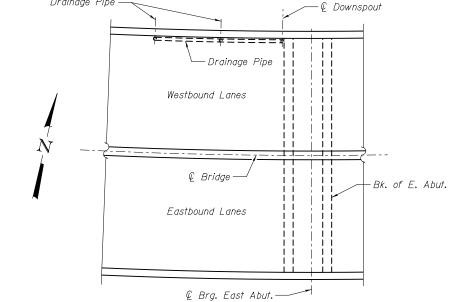
-45° FRP Y-Branch

-2- ⁵8" Dia, Galvanized expansion bolts

Drainage pipe and fittings shall be Fiberglass Reinforced Pipe (FRP), except as noted.

For location of drainage scupper stations, see table on sheet S1.

For drainage scupper details, see sheet S27. For pipe hanger supports, see sheet S57.



Drainage Pipe

DRAINAGE PLAN NEAR EAST ABUTMENT

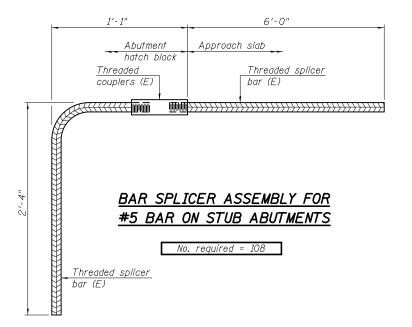
Note: Surface drainage flows west to toe of slopewall.



USER NAME =	DESIGNED - PRC	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** CLOSED DRAINAGE SYSTEM — 6 S.N. 082—0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3, VARIOUS RAILROADS, MISSOURI AVE. AND I-55SB/64WB SHEET NO. S61 OF S77 SHEETS

F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
799	1BR-1-1		ST. CLAIR	315	205
			CONTRACT	NO. 7	6G39
	ILLINOIS	FED. A	D PROJECT		



NUIE

Splicer bars shall be deformed $\overline{\text{with threaded ends}}$ and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements

for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

MODJESKI and MASTERS Experience great bridges.	

USER NAME =	DESIGNED	-	MJP	REVISED
	CHECKED	-	LNB	REVISED
PLOT SCALE =	DRAWN	-	PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED	-	MJP	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,
VARIOUS RAILROADS, MISSOURI AVE. AND I-55SB/64WB

SHEET NO. 562 OF 577 SHEETS

F.A.P. RTE.	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
799	1BR-1-1	П	ST. CLAIR	315	206	
			П	CONTRACT	NO. 7	6G39
	ILLINOIS	FED.	ΑI	D PROJECT		



Page $\underline{1}$ of $\underline{4}$

Date 6/9-6/15/2016

SECTION	1BR-1-1		_ ı	_OCAT	ION _		Saint Louis, Illinois, SEC.	, TWP. , RNG). ,				
COUNTY	Saint Clair D	RILLING	ME	THOD			de , Longitude ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
	082-6003(E), 082-0374(P)		D E P	B L O	U C S	M 0	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L O	UCS	N C
Station Offset			H	w s	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	395.5 N/A	_ ft. <u>▼</u> _ ft	H	W S	Qu	S
	ace Elev. 417.0	ft	(ft)	(/6")	(tsf)	(%)	After 16 Hrs.	393.0	_ ft <u></u>	(ft)	(/6")	(tsf)	(%
	own and brown, silty ers and brick, A-4		_	-			SAND: Brown, A-3 (d	continued)		_			
<i>3.</i>	·		_	2 4 6	2.8 P	18				<u>_</u> _	4 9 11	N/C	
									394.0				
		440.0	_	10	3.3	17	SILTY SAND: Brown Begin Mud Rotary at 2	,		_	4		
	d Rock and Brick,	413.0		18	3.3 P	17	Bogin Mud Notary at 2	_0 1001.		<u></u>	4 5	N/C	
A-1	own and brown, silty	_412.3	-5	14	>4.5	13				-25	5		
	ders and brick, A-4		_		LP_		CLAY LOAM: Gray, A	<u></u>	391.5				
~~~~~		410.8		2			CLAT LOAW. Glay, F	1-0			5		
CLAY: Brown stains, A-7	and gray, trace iron			1	0.3	33					5		
			_	2	B/20						6		
							SILTY LOAM: Gray, A	A-4	389.0				
			_				,,,,,			_	3		
				ST	1.1	36					2	N/C	
			-10		S/2.8					-30	5		
			_				-			_			
				WOH	1.2								
				2	S/15	32	SAND: Brown, A-3		<u>385.0</u>				
			_				JOAND. BIOWII, A-0			_			
				1	1.6						7	N/C	
			-15	2	S/15	34				-35	8 10	1470	
		401.5		i i							-10		
LOAM: Brown	n, A-4												
		400.0	_	1 2	1.0 P	30			200.0	_			
SAND: Brown	n, A-3	400.0		5	N/C		SANDY LOAM: Brow	n, A-4	380.0				
		399.0											
LOAM: Brown	n, A-4				0.5								
SAND: Brown	 n. A-3	398.0		1 5	0.5 B/20	31					8 7	N/C	
D. O.	.,		-20		N/C	31				-40	7		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{4}$ 

Date 6/9-6/15/2016

	SCI Engineering, Inc.										Dute	0,0 0, .	0,20.0	
ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	L0	OGGE	D BY	BDG	(SCI)	
SECTION _	1BR-1-	1	ι	OCAT	ION .		aint Louis, Illinois, SEC. , TWP. , RNG. , de , Longitude							
COUNTY	Saint Clair	DRILLING	S MET	THOD			ME 550 w/HSA	HAMMER 1	TYPE		Auto	matic		
	082-6003(l 082-0374(	P)	D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.		_ ft _ ft	D E P	B L O	UCS	M O I	
Station	BB-301 61+50		H	W S	Qu	S T	Groundwater Elev.: First Encounter	395.5	ft▼	H	W S	Qu	S T	
	80.0 ft LT ace Elev. 41		( ft)	(/6")	(tsf)	(%)	Upon Completion After 16 Hrs.	N/A 393.0	_ft _ft <u>⊽</u>	( ft)	(/6")	(tsf)	(%)	
SANDY LOAI (continued)	M: Brown, A-4			5			SAND: Brown, A-3 (c	continued)			10			
With clay nod	ules and gravel	370.0	-45 —	5 10	N/C		Becomes gray, fine to sand	medium		-65	13 10	N/C		
STOP: 6/9/20 START: 6/10	016 at 3:25pm //2016 at 7:00am		-50	7 8 10	N/C		Trace organics, with g	gravel			8 15 9	N/C		
			_				SAND: Gray, A-3 Rough Drilling ~ 72.0'		<u>345.5</u>					
Becomes bro	wnish-gray		-55	9 17 21	N/C		SANDY CLAY: Gray, SAND: Gray, A-3	A-6	342.8 342.5		7 10 11	N/C		
			-60	17 19 24	N/C		With medium to large	gravel		-80	10 11 11	N/C		



USER NAME =	DESIGNED - MJP	REVISED
	CHECKED - LNB	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED

SOIL BORING - 1	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,	799	1BR-1-1	ST. CLAIR	315	207
VARIOUS RAILROADS, MISSOURI AVE. AND I-55SB/64WB			CONTRACT	NO. 7	6G39
SHEET NO. S64 OF S77 SHEETS		TILINOIS FED AT	D PROJECT		



Page  $\underline{3}$  of  $\underline{4}$ 

Date 6/9-6/15/2016

	intorotato o i		J J				172, Item 22 - FAP 799	,		JGGE	אם ט:	טטם	(30
SECTION	1BR-1-1		_ L	OCAT	ION _	East S	aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG	• ,				
COUNTY	Saint Clair [	RILLING	MET	THOD			ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
	082-6003(E), 082-0374(P)		D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.		_ ft _ ft	D E P	B L O	U C S	N C
Station	BB-301 61+50 80.0 ft LT		T H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	395.5 N/A	_ ft. <u>▼</u> ft	H	W S	Qu	S
Ground Surfa	ce Elev. 417.0	ft	( ft)	(/6")	(tsf)	(%)	After 16 Hrs.		ft⊻	( ft)	(/6")	(tsf)	(%
SAND: Gray, A	A-3 (continued)						SAND w/ GRAVEL: (fractured rock, A-1	Gray, with			27		
			-85	5 6 7	N/C					-105	36 29 32	N/C	
SAND w/ GRA	VEL: Gray, A-1	330.0											
			-90	50/6" 23 13	N/C		GRAVEL w/ SAND: ( weathered sandstone		308.0	-110	50/5"		
		225.0	_										
SAND: Gray, A STOP: 6/10/20	016 at 2:15pm	325.0	_										
STOP: 6/10/20		325.0	-95	22 31 36	N/C					-115	41 50/6" 50/6"	N/C	
STOP: 6/10/20	016 at 2:15pm	325.U	-95	31	N/C					-115	50/6"	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **ROCK CORE LOG**

Page  $\underline{4}$  of  $\underline{4}$ 

Date 6/9-6/15/2016

SECTION		SCI Engineering, Inc.							_		<i>,,</i> 10,2010
Correct   Corr	ROUTE	Interstate 64	DESCRIPTION P	TB 172, Item 22	2 - FAP 799 (MLI	C Drive	e)	_ LO	GGED	BY BD	G (SCI)
Saint Clair   CORING METHOD   Saint Clair   CORING BARREL TYPE & SIZE   NX Wireline   D   C   Q   I   R   C   Q   I   R   C   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   R   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I   Q   I	SECTION	1BR-1-1				/P. , R	NG.,				
STRUCT. No.	COUNTY	Saint Clair CC		addade , Longit	.uuc				ь	CORE	
Sorting No.   BB-301   Top of Rock Elev.   296.50   ft   Top of Rock Elev.   296.14   Top of Rock El	STRUCT. NO. Station			_				С О		1	R E
Offset         80.0 mt L         (ft)			Top of Rock Elev.	296.50	ft	P T	R	E R			G T
LIMESTONE: Light gray, moderately hard, very finely crystalline, thin to medium bedded, race chert nodules trace styplites with chert nodules fi.8" closed vertical fracture and chert nodules fi.8" closed vertical fracture and chert nodules fi.8" closed vertical fracture and chert nodules frace chert nodul			 				/#\		(0/)	(min/ft)	
race chert nodules (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,25) (1,2				. Alain ta maadii		(11)					(151)
No chert nodules 125 634.3 2.5" closed vertical fracture 2 100 95 2.6 3.634.3 3.69 ccomes medium to thickly bedded 1129.7 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3 3 100 88 3.3	trace chert nod With chert nod 5.8" closed ver 2.75" closed v 3.25" closed v Becomes argil	dules, trace styolites dules rtical fracture ertical fracture, no che ertical fracture laceous		e, min to medium	n beadea,		1	100	94	3.07	
2 100 95 2.6  Becomes medium to thickly bedded Frace chert nodules  1129.7  3 100 88 3.3  Becomes medium bedded						-125					6343
Becomes medium to thickly bedded Frace chert nodules  1129.7  3 100 88 3.3  Becomes medium bedded  1214.6  Becomes thinly bedded  1096.3  Boring terminated at 136.9 ft.	2.5" closed ver	rtical fracture				-125	2	100	05	2.6	034.3
Frace chert nodules							2	100	90	2.0	
Becomes medium bedded 1129.7  3 100 88 3.3 1214.6  Becomes thinly bedded 1215  1096.3  Boring terminated at 136.9 ft.  Boring grouted to 136.86 ft.	Becomes med	lium to thickly bedded									
Becomes medium bedded 1214.6  Becomes thinly bedded 1214.6  Becomes thinly bedded 1096.3	Trace chert no	odules									
Becomes medium bedded  1214.6  Becomes thinly bedded  1096.3  1096.3  Boring terminated at 136.9 ft.  Boring grouted to 136.86 ft.						-130					1129.7
30 a speciment of the state of							3	100	88	3.3	
280.14 1096.3 280.14 280.14 280.14 280.14 280.14 280.19 30ring terminated at 136.9 ft.	Becomes med	lium bedded				_					1214.6
280.14 1096.3 280.14 280.14 280.14 280.14 280.19 Soring terminated at 136.9 ft.											
30 aring terminated at 136.9 ft. 30 aring grouted to 136.86 ft.	Becomes think	y bedded									
Boring terminated at 136.9 ft.  Boring grouted to 136.86 ft.						-135					
Boring terminated at 136.9 ft.											1096.3
Soring grouted to 136.86 ft.	Poring tormin	atod at 126 0. #			280.14	<u>.                                    </u>					
	ŭ					_					
<u>-140</u> -140	Boring grouted	d to 136.86 ft.									
<u>-140</u>											
						-140					

Color pictures of the cores	Yes		
Cores will be stored for exami	nation until		
The "Strength" column represe			
	·	,	BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - MJP	REVISED	_
	CHECKED - LNB	REVISED	
PLOT SCALE =	DRAWN - PRC	REVISED	
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED	



Page  $\underline{1}$  of  $\underline{5}$ 

Date 6/9-6/16/2016

	SCI Engineering, I	nc.											0,000,1	
ROUTE	Interstate	64	DES	SCRI	PTION		PTB ²	172, Item 22 - FAP 799	(MLK Drive)	L	OGGE	ED BY	TC	(SCI)
SECTION _	1BF	R-1-1		_ L	OCAT	ION _		saint Louis, Illinois, SEC. Ide , Longitude	, TWP. , RNG	• ,				
COUNTY	Saint Clair	DRIL	.LING	MET	HOD			ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
STRUCT. NO. Station	082-60 082-03	374(P)	_	D E	B L	U	М	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E	B L	U	М
			_	P T	o W	S	I S	-	IV/A	- "	P T	o W	s	I S
BORING NO. Station	64+	01	_	H	S	Qu	T	Groundwater Elev.: First Encounter	394.9	ft▼	H	s	Qu	T
	34.01 ace Elev.		ft	( ft)	(/6")	(tsf)	(%)	Upon Completion After N/A Hrs.	N/A N/A	_ ft _ ft	( ft)	(/6")	(tsf)	(%
Offset 10.0ft E				_				Concrete Rubble			_			
sampling perfo Drilled with HS switched to m	SA down to 5. ud rotary and	Oft, then		_				FILL (driller noted)		395.0	<u>*</u>			
advancing cas	sing.			_										
				_										
				-5							-25			
				_										
				_										
				_							_			
				_										
Began losing I	large quantitie	es of		-10							-30			
mud into the h	nole.													
				_										
				_							_			
				_				SAND: Gray, A-3		382.5		7		
				_				GAND. Glay, A-3				15	N/C	
				-15				STOP: 6/10/2016			-35	15		
								310F. 0/10/2010			_			
				_							_			
				_								10		
				_				Trace silt			_	10 8	N/C	
				-20				Trace Sill			-40	12		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{5}$ 

Date 6/9-6/16/2016

COUNTY TOTAL SHEET NO.

ST. CLAIR 315 209

CONTRACT NO. 76G39

	SCI Engineering, Inc.										Date	0,0 0,	0,20.0
ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	172, Item 22 - FAP 799	(MLK Drive)	L0	OGGI	D BY	TC	(SCI)
SECTION _	1BR-1-	1	L	OCAT	ION .		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG.	,				
COUNTY	Saint Clair	DRILLING	3 MET	THOD			ME 550 w/HSA	HAMMER T	YPE		Auto	matic	
STRUCT. NO. Station			D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft ft	D E P	B L O	U C S	M O I
Station	BB-302B 64+01		H	W S	Qu	S T	Groundwater Elev.: First Encounter	394.9	ft <u>▼</u>	H	W S	Qu	S T
	34.0 ft LT		( ft)	(/6")	(tsf)	(%)	Upon Completion	N/A	ft	/ ft\	(/6")	(tsf)	(%)
	ace Elev. 41	ο.υ π	(14)	(,,,	(131)	( /0)	After N/A Hrs.	N/A	ft	( 11)	(,,,	(131)	(70)
SAND: Gray,	A-3 (continued)			9			SAND: Gray, A-3 (co.	ntinuea)			10		
			-45 ————————————————————————————————————	10	N/C		Becomes gray			-65 —	17 26	N/C	
CLAY: Gray, SILTY SAND:		<u>367.5</u>		4 7 7	0.5 B/20 N/C					-70	14 16 20	N/C	
SAND: Gray,	Ā-3	363.5	-55	9 11 13	N/C		WOOD and ORGANI SAND: Gray, trace fir		342.5 342.0		6 12 17	N/C	
Becomes bro	wn, trace fine grav	el	-60	5	N/C				336.3	-80	13 8	1.0	



USER NAME =	DESIGNED - MJP	REVISED
	CHECKED - LNB	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED

SOIL BORING - 3	F.A.P. RTE.	SECTION
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,	799	1BR-1-1
VARIOUS RAILROADS, MISSOURI AVE. AND 1-55SB/64WB		
SHEET NO. S65 OF S77 SHEETS		TI I TNOTS EET



Page  $\underline{3}$  of  $\underline{5}$ 

Date 6/9-6/16/2016

ROUIE	interstate 04	DESCI	KIP I ION		FID	172, Item 22 - FAP 799 (	(IVILIX DIIVE)		JGGE	אפעב	_10	(SCI
SECTION _	1BR-1-1		LOCAT	TION _		aint Louis, Illinois, SEC.	, TWP. , RNG	٠,				
COUNTY	Saint Clair DRI	LLING MI	ETHOD			ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
STRUCT. NO. Station	082-6003(E), 082-0374(P)	_ D	L	U	M	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L	U	M 0
BORING NO. Station Offset	BB-302B 64+01 34.0 ft LT	_   T	w	S Qu	S T	Groundwater Elev.: First Encounter Upon Completion	394.9 N/A	_ ft. <u>▼</u> ft	T H	o W s	S Qu	S T
	ace Elev. 416.0	ft (f	t) (/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%
CLAY: Gray,		336.0		P		SAND: Gray, few fine	gravels, A-3					
SAND: Gray,	trace fine gravel, A-3					(continued)		242.5				
		_	7 12	N/C		GRAVEL: Gray, A-1		312.5 311.5		42 50/6"	N/C	
		-8 - 	5 14			SAND: Gray, trace cla	ay, A-3	,	-105	36		
		_							_			
		_	8	NIG						19	NIO	
No gravel		9	10 0 10	N/C					-110	24 30	N/C	
SAND W/ GRA	AVEL: Gray, A-1	324.5				GRAVEL W/ COBBLE	: Gray, A-1	305.0	_			
		_										
SAND: Gray,	few fine gravels, A-3	322.5	14 17	N/C		SAND: Gray, trace silt	t, A-3	302.5		50/5"		
		<u>-9</u>	5 14						-115			
		-						,				
		_										
		-	15	N/C						21		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **SOIL BORING LOG**

Page  $\underline{4}$  of  $\underline{5}$ 

Date 6/9-6/16/2016

	SCI Engineering, Inc.									Date o	/ <u>9-6/16/201</u> 6
ROUTE	Interstate 64	_ DE	SCRI	PTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	LOG	GED BY	TC (SCI)
SECTION _	1BR-1-1		ι	OCAT	ION _		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG	i.,		
COUNTY	Saint Clair DF	RILLING	MET	THOD			ME 550 w/HSA	HAMMER	TYPE	Autor	natic
STRUCT. NO.			D E	B L	U	M	Surface Water Elev.	N/A	ft		
		_	P	0	s	1	Stream Bed Elev.	N/A	_ ft		
BORING NO. Station	BB-302B 64+01	_	H	W S	Qu	S T	Groundwater Elev.: First Encounter	394.9	ft▼		
Offset	34.0 ft LT						Upon Completion	N/A	_ ft		
Ground Surf	ace Elev. 416.0	ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft		
SAND: Gray, (continued)	trace silt, A-3		_								
Limestone cut from 119.3' to	ttings in mud wash 122.8'		_								
		293.2	_								
Borehole cont coring.	tinued with rock		_								
			_								
			-125								
			_								
			_								
				1							
			-130								
			_								
				-							
			_	-							
				-							
			_								
			_								
			-135								
			_								
			_	-							
			-140								
			0			1	II				



USER NAME =	DESIGNED - MJP	REVISED
	CHECKED - LNB	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED



#### **ROCK CORE LOG**

Page  $\underline{5}$  of  $\underline{5}$ 

Date 6/9-6/16/2016

ROUTE	Interstate 64	DESCRIPTION	PTB 172, Item 22	- FAP 799 (MLK	Drive)	L	OGGED	BY TO	C (SCI)
SECTION _	1BR-1-1	LOCATION	East Saint Louis, II		P. , RNO	<b>∋.</b> ,			
COUNTY	Saint Clair COF	RING METHOD	Latitude , Longitu	ide		R	_	CORE	S
STRUCT. NO Station	082-6003(E), 082-0374(P)	CORING BARRE	EL TYPE & SIZE	NX Wireline		C O V	R Q	T I M	T R E N
BORING NO. Station Offset		Top of Rock E Begin Core Ele	lev. 296.70	_ ft _ ft		R E E R Y		E	G T H
	face Elev. 416.0	ft			( ft) (	#) (%	(%)	(min/ft)	(tsf)
weathered Becomes me	:: Gray, moderately hard, edium bedded	,	, ,	ly 293.20		1 95	66	5.5	766.2
Becomes thir	nly bedded, trace chert n	odules, trace styolites			-125				
	ssively bedded					2 100	93	4.4	1128.7
With chert no									
Trace chert n	odules								
Becomes thic 4.5" open ver					-130				
No chert nod	ules					3 99	78	3.8	897.9
5" closed ver	tical fracture								908.5
Trace chert n	odules				-135				
Becomes thir	n to medium bedded, trad	ce styolites				4 100	76	4.5	
No chert nod	ules								853.7
Boring termin	nated at 122.8 ft.			278.00			+		333.7
Boring groute									
_ = 59 5.040					-140				
					-				

Color pictures of the cores Yes

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{1}$  of  $\underline{5}$ 

Date 6/2-6/8/2016

ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	72, Item 22 - FAP 799 (	(MLK Drive)	L0	OGGE	D BY	TC (	(SCI)
SECTION	1BR-1-1		L	OCAT	ION .		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG	<b>i.</b> ,				
COUNTY	Saint Clair DF	RILLING	MET	HOD			ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
STRUCT. NO. Station	082-6003(E), 082-0374(P)		D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L O	U C S	M O I
Station Offset	BB-303 65+93 20.0 ft RT		T H	W S	Qu (tof)	S T	Groundwater Elev.: First Encounter Upon Completion	386.5 N/A	_ ft. <u>▼</u> _ ft	T H	W S	Qu	S T
	own and black, clay,	ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	_ ft	( ft)	(/6")	(tsf)	(%)
	own and black, clay, and crushed rock,		_	3 8 11	N/C	15	SANDY LOAM: Brown		395.5		4 8 8	N/C	
			_	8 7	N/C	11	SAND: Brown, A-3		3 <u>9</u> 3 <u>.0</u> 3 <u>9</u> 1.5	_	4 6	N/C	
			-5	3			SILT: Brown, A-6 SAND: Brown, A-3		391.3	-25	10	NIO	
FILL: Gray, sill	ty clay, A-6	409.5 408.5		6 3 2	0.3 S/15	30					3 4 9	N/C 0.5	_28_
			-10	2 3 2	N/C	17	SILTY CLAY LOAM: ( SAND: Orangish-brov	•	387.5 386.5		1 1 9	0.3 S/15 N/C	34
FILL: Brownis	h-gray, clay, A-7	405.0		1 1	1.1	20	Begin Mud Rotary at 3						
				3	S/20	32				_			
		400.5	-15	3 6 6	0.8 P	27	Becomes brown, trace	e silt		-35	5 4 4	N/C	
SANDYLOAM	l: Brown, A-4			3 4 6	N/C								
LOAM: Brown		398.0											
	,		-20	1 3 5	N/C	18	Becomes gray with bro	own		-40	3 8 13	N/C	



USER NAME =	DESIGNED - MJP	REVISED	_
	CHECKED - LNB	REVISED	
PLOT SCALE =	DRAWN - PRC	REVISED	
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED	

30L DONING - 3	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,	799	1BR-1-1	ST. CLAIR	315	211
VARIOUS RAILROADS, MISSOURI AVE. AND 1-55SB/64WB			CONTRACT	NO. 7	'6G39
SHEET NO.S67 OF S77 SHEETS		ILLINOIS FED. A	D PROJECT		



Page  $\underline{2}$  of  $\underline{5}$ 

Date 6/2-6/8/2016

							172, Item 22 - FAP 799 (						راتات
SECTION	1BR	R-1-1	L	OCAT	ION _		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG	• ,				
COUNTY	Saint Clair	DRILL	ING MET	HOD			ME 550 w/HSA	_ HAMMER	TYPE		Auto	matic	
STRUCT. NO. Station		03(E), 74(P)	D E P	B L	U	М	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L	UC	М О
SORING NO. Station Offset	BB-3 65+9 20.0 ft	93	T H	O W S	S Qu	S T	Groundwater Elev.: First Encounter Upon Completion	386.5 N/A	_ ft. <u>▼</u> ft	T H	o W s	S Qu	I S T
Ground Surfa	ice Elev.		ft (ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	- It ft	( ft)	(/6")	(tsf)	(%
SAND: Orang (continued)							SAND: Orangish-brov (continued)	wn, A-3	_				
race fine grav	vel		-45	4 12 17	N/C					-65	9 17 18	N/C	
			-50	14 18 19	N/C					-70	8 11 15	N/C	
			-55	21 19 17	N/C					-75	19 20 18	N/C	
				7							25		
			-60	12 16	N/C					-80	25 22	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **SOIL BORING LOG**

Page  $\underline{3}$  of  $\underline{5}$ 

Date 6/2-6/8/2016

	SCI Engineering, inc												
ROUTE	Interstate 64	<u> </u>	DESCR	IPTION		PTB 1	72, Item 22 - FAP 799 (	(MLK Drive)	L0	OGGI	ED BY	TC	(SCI)
SECTION	1BR-	1-1		LOCAT	ION .		aint Louis, Illinois, <b>SEC.</b> de , <b>Longitude</b>	, TWP. , RNG.	,				
COUNTY	Saint Clair	DRILLII	NG ME	THOD			ME 550 w/HSA	HAMMER T	YPE		Auto	matic	
	082-600 082-037	4(P)	D E P	B L O	U C S	M 0 1	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft	D E P	B L O	U C S	M O I
Station Offset	20.0 ft I	RT	T H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	386.5 N/A	ft. <u>▼</u>	T H	W S	Qu	S T
	ace Elev.	416.0	ft (ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%)
STOP: 6/2/20 SAND: Orang (continued) START: 6/3/2	jish-brown, A-3		-85	20 33 36	N/C		SAND: Gray, A-3 (con	ntinued)	307.7	-105	9 17 30	N/C	
SAND w/ LIMI Gray, A-1 SAND: Gray, SAND: Gray,		32i BLE:326 J	8.8	38 38 20	N/C		COBBLE w/ GRAVEL gray, A-1 STOP: 6/3/2016 START: 6/6/2016 Borehole continued with coring.		305.2	-110	50/5" 50/5" 50/3"		
With limestone	e fragments ~9:	3.5'	-95	21 19 20	N/C					-115			
Rough Drilling	from 96.0' to 9	8.0'		1	N/C								
			-100	3 12	14/0					-120			



USER NAME =	DESIGNED - MJP	REVISED
	CHECKED - LNB	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED

SUL BUNING - 0	F.A.P. RTE.	SECTION
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3, VARIOUS RAILROADS, MISSOURI AVE. AND I-55SB/64WB	799	1BR-1-1
SHEET NO. S68 OF S77 SHEETS		ILLINO:



#### **ROCK CORE LOG**

Page  $\underline{4}$  of  $\underline{5}$ 

Date 6/2-6/8/2016

ROUTE	Interstate 64	DESCRIPTION	PTB 172, Item 22	: - FAP 799 (MLI	K Driv	e)	_ LO	GGED	BY TO	C (SCI)
SECTION _	1BR-1-1	LOCATION	East Saint Louis, I		/P. , R	NG.				
COUNTY	Saint Clair COF	RING METHOD	Latitude , Longiti	uue			R E	R	CORE	S T
STRUCT. NO Station	082-6003(E), . 082-0374(P)	CORING BARRE	_	NX Wireline _ in	D	C O	С О V	Q	T I M	R E N
BORING NO. Station Offset	65+93 20.0 ft RT	Top of Rock E Begin Core Ek		ft ft	P T H	RE	E R Y	. D	E	G T H
	face Elev. 416.0	_ ft			( ft)	(#)	(%)	(%)	(min/ft)	(tsf)
	E from 110.8'-115.1' - R		,	300.9		1	14	0	4.42	
L	E from 115.1'-116.1' - R	- 	gments and fractured	gravel 299.9		2	25	0	3	
COBBLE w/	GRAVEL: Brown and gi	ray, A-1			_	SS-31				
				296.5		SS-32	!			
	: Dark and light gray, ver derately weathered	ry hard, aphanitic to c	oarsely crystalline, hi		-120	3	69	33	7.7	
Becomes ligh	t gray, moderately hard,	finely crystalline, think	ly bedded, slightly we	eathered	_					
5.5" open ver	tical fracture									
Becomes bar Becomes me					_		400	400		1243.8
Becomes che					_	4	100	100	4	1053.5
					-125	5	100	72	5	263.5
0.6" open ver	tical fracture				_					
0.9" open ver	tical fracture				_					
13.6 open ve	rtical fracture, becomes	thinly bedded			_					
Becomes thic	ckly bedded			285.2	-130 5 —	6	95	88	4.9	

Color pictures of the cores Yes

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



# **ROCK CORE LOG**

Page  $\underline{5}$  of  $\underline{5}$ 

Date 6/2-6/8/2016

SECTION1BR-1-1LOCATION _East Saint Louis, Illinois, SEC. , TWP. , RNG. , Latitude , Longitude		SCI Engineering, Inc.								ale 0/2-	0/0/2010
COUNTY   Saint Clair   CORING METHOD   STRUCT, NO.   082-00374(P)   CORING BARREL TYPE & SIZE   NX Wireline   D   C   O   Q   1   E   C   C   T   R   R   S   T   R   R   S   T   R   R   S   T   R   R   S   T   R   R   T   R   T   R   T   R   T   T	ROUTE	Interstate 64	DESCRIPTION	PTB 172, Item	22 - FAP 799 (MI	K Drive	e)	_ LO	GGED	BY TO	C (SCI)
COUNTY   Saint Clair   CORING METHOD   CORING METHOD   STRUCT. NO.   082-003/4(P)   CORING BARREL TYPE & SIZE   NX Wireline   D   C   C   Q   I   R   R   T   R   R   T   R   R   T   R   R	SECTION _	1BR-1-1	LOCATION			WP. , R	NG.,				
STRUCT. NO. 082-0374(P) CORING BARREL TYPE & SIZE NX Wireline Station 65+93 (Core Diameter 2 in Top of Rock Elev. 305.20 ft H N Y Y H N R Y Y H CORING Station 65+93 (Tip) (Ft) (Ft) (Ft) (Ft) (Ft) (Ft) (Ft) (Ft	COUNTY	Saint Clair Co	ORING METHOD	Latitude , Long	jitude					CORE	
STRUCT. NO.		082-6003(E),							R	т	
BB-303   BB-303   Bof Rock Elev.   305.20   ft   Bgln Core Elev.   305.20   ft   Bgln Core Elev.   305.20   ft   H   H   E   F   Y   C   Wood (min/fl)   (tsf)   (ts		082-0374(P)						0		- 1	E
Station	BORING NO	BB-303							D	E	
Offset 20.0 ft.RT Ground Surface Elev. 416.0 ft (ft) (#) (%) (%) (%) (%) (min/ft) (tsf)  LILIMESTONE: Light gray, moderately hard, finely crystalline, thickly bedded, slightly weathered, cherty (continued)  121.6  121.6  121.6  135  145  145  145					ft		E				
LIMESTONE: Light gray, moderately hard, finely crystalline, thickly bedded, slightly weathered, cherty (continued)  121.6  135  519.5  Boring terminated at 136.5 ft.  140  145  146  146  146						н		Y			н
soring terminated at 136.5 ft.  Boring grouted to 136.5 ft.  1145			ft			( ft)	(#)	(%)	(%)	(min/ft)	(tsf)
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.			ly hard, finely crystalline,	thickly bedded, sl	ightly						121.6
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.											
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.											
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.											
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.						-135					519.5
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.  -140 -145 -145											310.0
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.											
Boring terminated at 136.5 ft.  Boring grouted to 136.5 ft.					279.	50					
	Boring termina	ated at 136.5 ft.									
	Doring are: +-	d to 126 5 #									
-145	Boring groute	ed to 136.5 it.									
-145											
-145											
-145											
-145						-140					
						_					
						_					
						_					
						_					
						-145					
						-175					
						_					
						_					
						_					
						_					
						_					
<u>-150</u>						_					
<u>-150</u>											
<u>-150</u>											
						-150					
						_					

Color pictures of the cores	Yes	_
Cores will be stored for exam	ination until	
The "Strength" column repres	ents the uniaxial compressive	e strength of the core sample (ASTM D-29

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED -	MJP	REVISED
	CHECKED -	LNB	REVISED
PLOT SCALE =	DRAWN -	PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED -	MJP	REVISED



Page  $\underline{1}$  of  $\underline{4}$ 

Date 6/6-6/8/2016

ROUTE	Interstate 64	_ DE					172, Item 22 - FAP 799 (	,					JOC
SECTION	1BR-1-1		_ [	OCAT	ION _	East S	aint Louis, Illinois, SEC.	, TWP. , RNG. ,					
COUNTY	Saint Clair DR	ILLING	ME	THOD			ME 550 w/HSA	HAMMER TYP	E_		Auto	matic	
STRUCT. NO. Station	082-6003(E), 082-0374(P)		D E	B L	U	М	Surface Water Elev. Stream Bed Elev.	N/A ff	t	E	B L	U	M 0
BORING NO. Station Offset	BB-304 68+79 35.0 ft LT	_	P T H	W S	S Qu	S T	Groundwater Elev.: First Encounter Upon Completion	389.0 ff	t <u>▼</u>	Т	o W s	S Qu	I S T
	ace Elev. 416.0	ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A ff		ft) (	(6")	(tsf)	(%
	and black, sandy clay,		_				SAND: Brown, A-3 (c	ontinued)					
with cinders, A	I <del>-</del> 0	444.5		3					_	+	4		
FILL: Brown,	sand, A-3	414.5	_	4	N/C	16			_		6 8	N/C	
		<u>413.0</u>							_				
	vith red and gray, k and cinders, A-2		_							+	3		
			-5						_	_	7 7	N/C	
			_							+			
FILL: Red, bri	 ck, A-1	409.5 409.0		4						, -	4 5	N/C	
FILL: Brown,			_	4					<u>.y</u>	1	5		
CLAY: Dark g	ray, trace iron stains,	408.0							-				
A-7	,		_	WOH 2	1.2	38	SILTY SAND: Gray, A	<u>3</u>	87 <u>.3</u> _	Ŧ	4		
			-10	1	B/20		SAND: Gray, A-3	3	86.0	-30	7		
			_				SAND. Glay, A-3			-			
				2	1.5				_				
				3 4	S/15	36			-				
			_						_				
			_							+	5		
			-15	ST	0.9 S/5.2	33				-35	6 8	N/C	
			_							$\perp$			
				1	4.5		-		-				
				3	1.5 B/20	33			_	-			
OAND 5		398.0					-		_				
SAND: Brown	ı, A-3		_	4						+	6		
Begin Mud Ro	tary at 19.0 feet.			5	N/C		With medium sand		_	_	7	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{4}$ 

Date 6/6-6/8/2016

	SCI Engineering, Inc.										Date	6/6-6/	8/2016
ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	L0	OGGI	D BY	BDG	(SCI)
SECTION	1BR-1-1		_ ι	OCAT	ION .		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG.	,				
COUNTY	Saint Clair DRIL	LING	MET	THOD			ME 550 w/HSA	_ HAMMER T	YPE		Auto	matic	
STRUCT. NO. Station	082-6003(E), 082-0374(P)	_	D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft ft	D E P	B L O	U C S	M O I
BORING NO. Station	BB-304 68+79	_	T H	W S	Qu	s T	Groundwater Elev.: First Encounter	389.0	ft▼	T H	W S	Qu	s T
Offset	35.0 ft LT	_					Upon Completion	N/A	ft				
Ground Surfa	ace Elev. 416.0	ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%)
SAND: Gray,	A-3 (continued)		-45	11 12 14	N/C		SAND: Gray, A-3 (con	ntinued)		-65	12 13 16	N/C	
				7 12 13	N/C		SAND: Gray, coarse, A-1	with gravel,	3 <u>49.0</u>		16 15 15	N/C	
			-55	9 13 14	N/C					-75	26 35 31	N/C	
							SAND: Gray, A-3		339.0				
			-60	9 13 14	N/C					-80	26 37 37	N/C	



	USER NAME =	DESIGNED - MJP	REVISED	Γ
		CHECKED - LNB	REVISED	1
;	PLOT SCALE =	DRAWN - PRC	REVISED	1
	PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED	1

SUL DUNING - 8	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,	799	1BR-1-1	ST. CLAIR	315	214
VARIOUS RAILROADS, MISSOURI AVE. AND 1-55SB/64WB			CONTRACT	NO. 7	76G39
SHEET NO.S70 OF S77 SHEETS		ILLINOIS FED. AI	D PROJECT		



Page  $\underline{3}$  of  $\underline{4}$ 

Date 6/6-6/8/2016

ROUTE	Interstate 64		DESCR	IPTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	L0	OGGE	DBY	BDG	(SC
SECTION	1BR-	1-1		LOCAT	ION _		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG	i <b>.</b> ,				
COUNTY	Saint Clair	DRILLI	NG ME	THOD			ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
STRUCT. NO. Station	082-6003 082-037		D E	B L	U	M O	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E	B L	U	M 0
Station	BB-30-	)	P T H	W S	S Qu	S T	Groundwater Elev.: First Encounter	389.0	_ft.▼	P T H	O W S	S Qu	S T
Offset Ground Surfa	35.0 ft L		ft (ft)	(/6")	(tsf)	(%)	Upon Completion After N/A Hrs.	N/A N/A	_ ft _ ft	( ft)	(/6")	(tsf)	(%)
	A-3 (continued)					, ,	SAND: Gray, medium with gravel, A-1 (contin	to coarse,	_ ''	_	23 26	N/C	
Rough drilling	from 81.0'-82.0	,	_				Intermittent hard drilling	•					
			_										
			_	10	N/C		COBBLE W/ GRAVEL	.: Gray and	312.0	_			
			-85	14			brown, A-1	,		-105	25 33	N/C	
Rough drilling	from 85.5'-88.5		_								50/5"		
SAND: Gray, with gravel, A-	medium to coal	<u> 329</u> rse,	9 <u>.0</u>										
Advance with	casing at 88.5'		_	_						_			
			-90	17 24 28	N/C		Rock fragments in sar	nple		-110	18 37 50/5"	N/C	
				20			STOP: 6/7/2016 at 3: START: 6/8/2016 at 7				30/3		
			_							_			
			-95	14 21 27	N/C					-115	50/5"		
			_							_			
			_							_			
			-100	22			Borehole continued w	:4h 1.	296.4	120	50/0"		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **ROCK CORE LOG**

Page  $\underline{4}$  of  $\underline{4}$ 

Date 6/6-6/8/2016

	SCI Engineering, Inc.												
ROUTE	Interstate 64	DE	SCRIPTION	P	TB 172, Item	122 - FAP 79	99 (MLK I	Drive	e)	_ LO	GGED	BY BD	G (SCI)
SECTION	1BR-1-	1	LOCATIO			is, Illinois, <b>SE</b>	C., TWP	. , RI	NG.,				
COUNTY	Saint Clair	CORING	METHOD	La	atitude, Lor	ngitude				R	R	CORE	S T
STRUCT. NO. Station	082-6003(I 082-0374(		CORING BARF		<b>/PE &amp; SIZE</b>	NX Wire	eline	D E	C	0 V	Q	T I M	R E N
BORING NO. Station Offset	BB-304 68+79 35.0 ft LT		Top of Rock Begin Core E	Elev.	296.38 296.38			P T H	R E	R Y		E	G T H
Ground Surfa		6.0 <b>ft</b>						( ft)	(#)	(%)	(%)	(min/ft)	(tsf)
	Gray, soft, very	fine grained	d, medium bedo	ded, sli	ightly weathe	ered,	296.38	-120	1	100	80	1.1	
	giliaceous , becomes parted to medium bedde		t				-						6.2
Becomes band	led to thinly bedd	ded					-						
Becomes thick	lv bedded						292.65						
LIMESTONE: I	Light gray, mode	rately hard,	very finely crys	stalline,	, medium be	dded, slightly		-					
weathered							-	-125					647.0
2.6" clay seam Becomes mass	sively bedded, fin	iely crystallir	ne, with chert				=	-123	2	100	87	2.55	
							-						
2.1" open fract	ure, becomes thi	ickly bedde	d				=		3	100	86	3.2	396.9
2.5" clay seam,	, becomes bande	ed to thinly l	bedded										330.3
Becomes thinly 0.5" clay seam,	bedded , becomes thickly	y bedded					2	-130					
							-						
Becomes mass	sively bedded						-						1236.4
Becomes argilla	aceous						-						
Becomes medi	ium bedded						281.38						303.6
Boring terminal								-135					303.6
Boring grouted	to 134.62 ft.						-						
							_						
								4					
							-						
							-	_					
								_					

Color pictures of the cores	Yes		
Cores will be stored for exami	nation until		
The "Strength" column represe	ents the uniaxial compressive	strength of the core sample (ASTM D-2938)	
	•	,	BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - MJP	REVISED
	CHECKED - LNB	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED

SUL BUNING - 9	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,	799	1BR-1-1	ST. CLAIR	315	215
VARIOUS RAILROADS, MISSOURI AVE. AND 1–55SB/64WB			CONTRACT	NO. 7	6G39
SHEET NO. S71 OF S77 SHEETS		ILLINOIS FED. AI	D PROJECT		



Page  $\underline{1}$  of  $\underline{4}$ 

Date5/<u>25-5/31/201</u>6

							172, Item 22 - FAP 799						
SECTION	1BR-1-1		_ ι	OCAT	ION _		saint Louis, Illinois, SEC. Ide , Longitude	, TWP. , RNG.	,				
COUNTY	Saint Clair I	DRILLING	ME	THOD			ME 550 w/HSA	_ HAMMER 1	YPE		Auto	matic	
	082-6003(E), 082-0374(P)		D E	B L	U	М	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft ft	D E	B L	U	M
Station	BB-305 71+43		P T H	o W s	S Qu	S T	Groundwater Elev.: First Encounter	377.5	ft▼	P T H	o W S	S Qu	S T
Offset	55.0 ft LT ce Elev. 415.0	ff	(ft)	(/6")	(tsf)	(%)	Upon Completion After N/A Hrs.	N/A N/A	ft	( ft)	(/6")	(tsf)	(%
CRUSHED RO	OCK: Gray, 3-inch	<u> </u>	1 - 7	V - 7	()	(,	SANDY LOAM: Brow			,	,	()	
minus	ay, with cinders and	414.0					(continued)						
brick, limestone		ı	_	4 9 7	>4.5 P						2 4 5	N/C	
Limestone grav	vel in shoe, no othe	r	-5	2 3 3						-25	1 4 5	N/C	
			_	4							2		
			_	1 1 1	0.5 P						3 5 6	N/C	
CLAY: Gray, A		407.0											
CLAT. Glay, F	X-1		_	1	1.5				385.9	-	5	N/C	
			-10	1 2	P 0.9		CLAY: Brown, trace f	ine sand, A-6	_303.9	-30	5 9	0.3 P /	
			_	1	\S/20				383.5			\/	
Becomes brow	nish-gray			2 2	1.6 S/20		SANDY LOAM: Brow	/n, A-4					
			_	1 1	0.8					_	6	N/C	
SAND: Brown	 , A-3	399.5	-15	2	S/20					-35	11		
				3 5 7	N/C		SAND: Brown, A-3		<u>378.0</u>	_			
		396.5	_							_			
CLAY: Brown,	A-6 : Brown, A-4	396.0		2 2	0.2 S/20						3 5	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{4}$ 

Date5/25-5/31/2016

ROUTE	Interstate 64	DES	SCRI	PTION		PTB ²	172, Item 22 - FAP 799	(MLK Drive)	L0	OGGE	D BY	TC	(SCI)
SECTION	1BR-1-1		_ L	OCAT	ION		Saint Louis, Illinois, SEC.	, TWP. , RNG.	,				
COUNTY	Saint Clair	DRILLING	MET	THOD			de , Longitude ME 550 w/HSA	HAMMER T	YPE		Auto	matic	
STRUCT. NO. Station			D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft	D E P	B L O	U C S	M O I
Station			H (f)	W S	Qu (4-5)	S T	Groundwater Elev.: First Encounter Upon Completion	377.5 N/A	ft. <u>▼</u>	H H	W S	Qu (4-5)	S T
	ace Elev. 415.	0 ft	(π)	(/6")	(tsf)	(%)	After N/A Hrs. SAND: Gray, A-3 (col	N/A	ft	( ft)	(/6")	(tsf)	(%)
	tary at 40.0 feet.						SAND. Glay, A-5 (Wi	minueu)					
			-45	12 12 12	N/C					-65	14 12 10	N/C	
			_										
SANDYLOAN	1: Gray, A-4	368.0	_				GRAVEL: Gray, A-1		_348.0				
				10			SAND: Gray, A-3		346.5		11		
			-50	18 18	N/C		OAND. Glay, A-0		345.0	-70	16 17	N/C	
							GRAVEL: Gray, A-1						
SAND: Gray,	Ā-3	363.0											
			-55	9 14 16	N/C		SAND: Gray, A-3		<u>341.5</u>	-75	14 16 23	N/C	
			_	9			SANDY LOAM: Gray	- <u>-</u>	336.5	. —	11		
			-60	8	N/C		SAND: Gray, A-3		335.9 335.3		15 17	N/C	



USER NAME =	DESIGNED -	-	MJP	REVISED	Г
	CHECKED -	-	LNB	REVISED	İ
PLOT SCALE =	DRAWN -	-	PRC	REVISED	İ
PLOT DATE = 3/12/2018	CHECKED -	-	MJP	REVISED	



Page  $\underline{3}$  of  $\underline{4}$ 

Date5/<u>25-5/31/201</u>6

	SCI Engineering, Inc.										Date	)( <u>25-5)</u>	31/2
ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	172, Item 22 - FAP 799	(MLK Drive)	Lo	OGGE	ED BY	TC	(SCI
SECTION	1BR-1-1		L	OCAT	ION _		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG	i. ,				
COUNTY	Saint Clair	DRILLING	MET	THOD			ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
STRUCT. NO. Station	082-6003(E 082-0374(P		D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L O	U C S	M O I
BORING NO. Station Offset	BB-305 71+43 55.0 ft LT		H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	377.5 N/A	_ ft <u>▼</u> ft	H	W S	Qu	S T
Ground Surfa		.0 <b>ft</b>	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%
Rough drilling f	y, A-1 <i>(continued)</i> rom 80.0'-82.0'	331.5					GRAVEL: Gray, A-1		<u>311.5</u>				
SAND: Gray, A	<b>1</b> -3		-85	13 9 12	N/C		SAND: Brownish-gra gravel, A-3 Rough drilling from 10			-105	14 27 36	N/C	
				20			Weathered limestone	and granito		_	48 50/4",		
Coarse sand ar	nd rock fragments	:	-90	20 22 21	N/C		fragments in shoe; set 109.3' STOP: 5/26/2016 START: 5/27/2016 COBBLE w/ GRAVEL brown, A-1	t casing at	305.0	-110	50/5"		
Hard drilling fro	m 92.0'-93.0'						No sample recovery  Borehole continued w	ith rock	302.7		50/3"	\ /	
			-95	14 15 14	N/C					-115			
			_										
		315.0	100	8 18 23	N/C					-120			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **ROCK CORE LOG**

Page  $\underline{4}$  of  $\underline{4}$ 

Date5/25-5/31/2016

_	SCI Engineering, inc.										
ROUTE	Interstate 64	DESCRIPTION	PTB 172, Item 2	2 - FAP 79	99 (MLK	Drive	e)	_ LO	GGED	BY TO	C (SCI)
SECTION _	1BR-1-1		East Saint Louis, Latitude , Longit		C. , TWF	P. , R	NG.,				
COUNTY	Saint Clair CO		Ludiude , Longi	iuu <del>c</del>				R	_	CORE	S
STRUCT. NO.	082-6003(E), 082-0374(P)	CORING BARREL  Core Diameter	2	in	eline	D E	c o	E C O V	R Q	T I M	T R E N
BORING NO.	BB-305	Top of Rock Elev		ft ft		P T	R E	E R	D .	E	G T
Station	71+43 55.0 ft LT	Begin Core Elev.		_ "		Н		Υ	-		Н
	face Elev. 415.0	ft				( ft)	(#)	(%)	(%)	(min/ft)	(tsf)
GRAVEL: Lim	nestone and granite co	bbles and boulders			302.70		1	12	0		
						-115					
SANDSTONE	E: Gray, moderately ha	rd, very fine grained, band	ded, slightly weath	 nered,	298.50	-	2	69	56	5	
calcareous Becomes med		, , , , , , , , , , , , , , , , , , ,									
						_					94.1
Becomes thic	kly bedded					-120	3	95	82	2.8	
		thin to medium bedded				_					85.7
2.2" open vert	tical fracture					_					
Becomes ban	nded to thinly bedded, t	race styolites				-125	4	100	74	2.4	
					287.15	_					157.0
LIMESTONE: weathered	: Light gray, moderatel	/ hard, very finely crystallii	ne, medium bedd	ed, slightly			5	86	54	4	230.1
1.75" open ve	ertical fracture				284.50	-130					
	ated at 130.5 ft.										
Boring groute	ed to 130.5 ft.					_					

Color pictures of the cores	Yes	
Cores will be stored for exami	nation until	
The "Strength" column represe	ents the uniaxial compressive	strength of the core sample (ASTM D-293)

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - MJP	REVISED
	CHECKED - LNB	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED



Page  $\underline{1}$  of  $\underline{5}$ 

Date 5/26-6/2-2016

ROUTE	interstate 64	DE	SCRI	PTION		PIB 1	172, Item 22 - FAP 799 (	(IVILK Drive)	_ LC	JGGE	D BY	RDG	(SC
SECTION	1BR-1-1		_ L	OCAT	ION _		aint Louis, Illinois, SEC. de , Longitude	, TWP. , RNG.					
COUNTY	Saint Clair D	RILLING	MET	THOD			ME 550 w/HSA	HAMMER T	/PE		Auto	matic	
STRUCT. NO. Station	082-6003(E), 082-0374(P)		D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft ft	D E P	B L O	U C S	M O I
Station			H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	N/A N/A	ft ft	Н	W S	Qu	S
	ace Elev. 427.0		( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%
3" TOPSOIL FILL: Brown a	and gray, clay, trace	426.8	_				FILL: Black, cinders, A (continued)	4-4		_			
fine sand and (	cinders, A-6		_	2 4 4	>4.5 P	24	Rough drilling at 20.5'  CLAY: Gray, with iron	stains and	405.0		2 2 2	N/C 1.5	30 32
FILL: Brown,	sandy clay, A-6	<u>424.0</u>		2			nodules, A-7				1	_P_/	
FILL: Brown, s FILL: Brown, o trace sand, A-6	clay, with cinders,	422.5 422.0		3 3	0.5 P	38	Trace iron stains	- <del>-</del>	401.5	-25	2 2	0.8 P	28
		420.5	_	2	NIO		SILTY LOAM: Gray, A	A-4		_			
FILL: Brown,	sand, A-3			5 5	N/C						ST	0.5 S/10.6	33
			_	4 5	N/C					_	2 5	0.9 S/15	28
			<u>-10</u>	6						-30	7	N/C	_29
Trace clay lum	ps			3 4 6	N/C		CLAY: Gray, A-7		395.0				
			_				,			_			
			-15	4 6 8	N/C					-35	WOH 1 2	0.9 B/20	49
			_	7						_			
			_	9 12	N/C		SAND: Brown, A-3		390.0				
			_		NVC					_			
FILL: Black, c	inders. A-4	<u>407.5</u>	-20	6 7 4	N/C	36				-40	6 8 10	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{5}$ 

**Date** 5/26-6/2-2016

ROUTE	Interstate		DES	SCRII	PTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	LOGG	ED BY	BDG	(SCI)
SECTION	11	BR-1-1		_ L	OCAT	ION _	East S	aint Louis, Illinois, SEC.	, TWP. , RNG. ,				
COUNTY	Saint Clai	r DRILL	LING	MET	HOD			de,Longitude ME 550 w/HSA	HAMMER TYP	<b>=</b>	Auto	matic	
STRUCT. NO. Station	082-6 082-6	6003(E), 0374(P)	-	D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A ft		B L O	U C S	M O I
	74 31.0	I+00 D ft LT	-	T H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	N/A ft		W S	Qu (4-5)	S T
Ground Surfa			ft	(ft)	(/6")	(tsf)	(%)	After N/A Hrs. SAND: Brown, A-3 (a	N/A ft	( ft)	(/6")	(tsf)	(%)
Begin Mud Ro									ŕ				
				-45	10 12 15	N/C				-65	10 19 21	N/C	
								SAND: Brown and gr	ay, A-1	9.5 8.8			
				-50	5 7 12	N/C		-		-70	6 11 12	N/C	
Becomes brow	vnish-gray			-55	10 11 12	N/C		Trace gravel		-75	8 8 7	N/C	
				-60	4 6 10	N/C				-80	12 10 7	N/C	



USER NAME =	DESIGNED -	MJP	REVISED	
	CHECKED -	LNB	REVISED	
PLOT SCALE =	DRAWN -	PRC	REVISED	
PLOT DATE = 3/12/2018	CHECKED -	MJP	REVISED	

JUL DUNING - 12	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,	799	1BR-1-1	ST. CLAIR	315	218
VARIOUS RAILROADS, MISSOURI AVE. AND 1-55SB/64WB			CONTRACT	NO. 7	6G39
SHEET NO. S74 OF S77 SHEETS		ILLINOIS FED. A	D PROJECT		



Page  $\underline{3}$  of  $\underline{5}$ 

**Date** 5/26-6/2-2016

ROUTE	Interstate 64	DES	CRIPTIC	)N	PTB	172, Item 22 - FAP 799 (	MLK Drive)	LOGGE	D BY	BDG	(SC
SECTION	1BR-1-1		LOCA	ATION		Saint Louis, Illinois, SEC.	, TWP. , RNG. ,				
COUNTY	Saint Clair 📗	RILLING N	NETHO			ME 550 w/HSA	HAMMER TY	PE	Auto	matic	
STRUCT. NO. Station	082-6003(E), 082-0374(P)		D B E L P O	C	M O I	Surface Water Elev. Stream Bed Elev.		ft D	B L O	U C S	M 0 1
BORING NO. Station Offset	BB-306 74+00 31.0 ft LT		T W H S		S T	Groundwater Elev.: First Encounter		ft H	W S	Qu	S
	ce Elev. 427.0	ft (	ft) (/6'	') (tsf)	(%)	Upon Completion After N/A Hrs.		ft (ft)	(/6")	(tsf)	(%
SAND: Gray, A		-	_			SAND: Gray, A-1 (cor Advance with casing a	ntinued)				
		-	10 14 -85 16	N/C				-105	29 38 50/4"	N/C	
Rough drilling f	rom 87.0'-88.5'	_						_			
		-	15 14 -90 12	N/C		With gravel		-110	23 44 47	N/C	
		-									
SAND: Gray, A	Ā-1	332 <u>.5</u>	12 50/5	NI/C		No gravel		-115	27 33 37	N/C	
		-									
		_						309.0			
		_	16	11/0		WEATHERED LIMES CLAY LOAM: Brown with chert gravel, A-1			50/5"		
			100 16	-	1	II		-120			1

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **SOIL BORING LOG**

Page  $\underline{4}$  of  $\underline{5}$ 

Date 5/26-6/2-2016

ROUTE		SCI Engineering, Inc.							Date 5	<u>6/26-6/2-201</u> 6
COUNTY   Saint Clair   DRILLING METHOD   CME 550 w/HSA   HAMMER TYPE   Automatic	ROUTE	Interstate 64	DES	CRIPT	TION	PTB 1	172, Item 22 - FAP 799	(MLK Drive)	LOGGED BY	BDG (SCI)
COUNTY   Saint Clair   DRILLING METHOD   CME 550 w/HSA   HAMMER TYPE   Automatic	SECTION _	1BR-1-1	I-1	_ LO	CATION			, TWP. , RNG. ,		
STRUCT. NO.   082-0374(P)   Station   D   B   L   C   O   P   O   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   S   I   T   W   S   T   W   S   T   W   S   T   W   S   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T   W   T	COUNTY _	Saint Clair DR	DRILLING	METH	OD			_ HAMMER TYP	E Auto	matic
BORING NO.   BB-306   T		. 082-0374(P)	I(P)	E	L C	0				
Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior   Superior	Station	74+00		T   1	w	s	First Encounter			
WEATHERED LIMESTONE w/ CLAY LOAM: Brown and gray, with chert gravel, A-1 (continued)				( ft) (/	/6") (tsf	) (%)				
-135 	WEATHERE CLAY LOAM: with chert gra	D LIMESTONE w/ : Brown and gray, rvel, A-1 (continued)	w/ y, ed)	-125 50	30 41 0/5"		After N/A Hrs.	N/Aff		



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		CHECKED - LNB	REVISED	1
;	PLOT SCALE =	DRAWN - PRC	REVISED	1
	PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED	1



#### **ROCK CORE LOG**

Page  $\underline{5}$  of  $\underline{5}$ 

**Date** 5/26-6/2-2016

ROUTE	Interstate 64	DESCRIPTION	PTB 172, Item 22	- FAP 799 (MLF	C Drive	)	LO	GGED	BY BD	G (SCI)
SECTION	1BR-1-1	LOCATION	East Saint Louis, III		P., RI	NG.				
COUNTY	Saint Clair co	RING METHOD					R E	R	CORE	S T
STRUCT. NO. Station	082-6003(E), 082-0374(P)	CORING BARRE	_	NX Wireline in	D	C	С О V	Q	T I M	R E N
BORING NO.		Top of Rock E		_ ft ft	P	R E	E R	D	E	G T
Station	74+00 31.0 ft LT	Begin Core Ele	ev	_ 11	H	-	Ŷ			Ĥ
Ground Surfa		— ft			( ft)	(#)	(%)	(%)	(min/ft)	(tsf)
LIMESTONE:	Light gray, hard, very f th interbedded clay sea		medium bedded, mo	oderately 300.28		1	100	73	2.22	820.6
					-130					
Becomes frac				296.05		2	100	89	1.3	
SANDSTONE	: Gray, hard, very fine	grained, thickly bedded	d, slightly weathered,		-					
calcareous										
										228.6
					-135					
					$\Box$					225.4
LIMESTONE:	Light gray, moderately	hard very finely crysta	alline thickly bedded		!- <del>-</del>					223.4
weathered		,,,,,	,,,	9						
Pagamas han	ded, with 0.3" clay sea	m			-140					211.5
L				286.55		3	100	67	1	
1.3" interbedd	: Gray, hard, very fine ed limestone	grained, banded to thir	niy beaded, siigntiy w	eatnered			100	0.		
	m, becomes medium b	pedded								
Boring termina	ated at 142.3 ft.			284.68						
Boring grouted					-					
Boiling grouter	u to 142.02 It.									
					-145					
					-145					
					_					

Color pictures of the cores Yes

Cores will be stored for examination until

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{1}$  of  $\underline{1}$ 

**Date** ___6/1/16

	SCI Engineering, Inc.												
ROUTE	Interstate 64	_ DE	SCRI	PTION		PTB ²	172, Item 22 - FAP 799	(MLK Drive)	L0	OGGE	D BY	TC	(SCI)
SECTION _	1BR-1-1		ι	OCAT	ION .		39833282, -90.1609112 de , Longitude	2849, <b>SEC.</b> 13	3, <b>TWP.</b> 2	2N, <b>F</b>	NG. 1	OW,	
COUNTY	Saint Clair DF	RILLING	MET	THOD			ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
STRUCT. NO.	SN 082-W315 73+59 to 75+18.5	0	D E P	B L O	UCS	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L O	UCS	M O I
BORING NO. Station Offset	RW-302 72+84 67.0 ft LT	_	T H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	382.0 N/A	_ ft <u>▼</u> ft	H	W S	Qu	S T
	face Elev. 400.0	— ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	- it	( ft)	(/6")	(tsf)	(%)
1.2 ft. ASPHA	ALTIC CONCRETE		_				SAND: Brown, A-3 (d	continued)	379.5	-			
		398.9		2			SANDY LOAM: Gray	, A-5			1		
A-6	and gray, sandy clay,		_	8 10	0.9 S/15	14				_	5 9	N/C	
Ell I : Brown	sand and crushed	397.0											
rock, A-1	, sand and crushed		_	3			SAND: Gray trace bro	own, A-3	376.5	. —	1		
			-5	3	N/C	3			275.0	25	4	N/C	
		394.0	_	J			SANDY LOAM: Gray Began mud rotary at 2		375.0	-20	U		
SANDYLOAI	M: Brown, A-6	334.0	_	3 6 7	N/C	12				_	8 13 15	N/C	
			_						372.0	_	15		
		391.5					SAND: Gray trace bro	own, A-3					
SAND: Brow	n, A-3		-10	2 4 6	N/C	10				-30	7 9 11	N/C	
			-10							-30	- ' '		
			_	3 4	N/C	7							
			_	6									
			-15	3 8 12	N/C	3				-35	13 15 14		
			-13	12							1-7		
				5 9 10	N/C	13							
			<u>_</u>										
0			_	2 7	N/C						15 16	N/C	
Some silt			-20	10					360.0	-40	12		

Boring terminated at 40.0 ft. Boring grouted to 40.0 ft.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)

S.N.



USER NAME =	DESIGNED - MJP	REVISED
	CHECKED - LNB	REVISED
PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED

SUL BUNING - 14	F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
.082-0374 MLK BRIDGE APPROACH OVER RELOCATED IL RTE 3,	799	1BR-1-1		ST. CLAIR	315	220
VARIOUS RAILROADS, MISSOURI AVE. AND I-55SB/64WB				CONTRACT	NO.	76G39
SHEET NO.S76 OF S77 SHEETS		ILLINOI	FED. A	ID PROJECT		



Page  $\underline{1}$  of  $\underline{2}$ 

Date 6/3-6/6/2016

	SCI Engineering, Inc.											2.0 01	
ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	172, Item 22 - FAP 799	(MLK Drive)	L0	OGGE	ED BY	BDG	(SCI
SECTION	1BR-1-1		_ ι	_OCAT	ION .		36208892, -90.1607292 Ide , <b>Longitude</b>	2089, <b>SEC.</b> 13	, TWP. :	2N, <b>F</b>	NG. 1	0W,	
COUNTY	Saint Clair	DRILLING	MET	THOD			ME 550 w/HSA	HAMMER	TYPE		Auto	omatic	
STRUCT. NO. Station	SN 082-W31 73+59 to 75+18		D E P	B L O	U C S	М О І	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L O	U C S	M O I
BORING NO. Station Offset	73+50		H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	384.5 N/A	_ ft. <u>▼</u> _ ft	H	W S	Qu	S T
Ground Surfa		.0 <b>ft</b>	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%)
RIP-RAP with	SILTY CLAY	427.0	_				Clogging in split spoo concrete fragments		407.5				
FILL: Brown, o	clay loam, A-6		_	3 5 6	N/C	17	FILL: Brown, sand, A (continued) FILL: Gray, clay, A-7		j J		5 3 3	1.0 S/15	34
			_				SILTY SAND: Gray, i	trace iron	405.0				
FILL: Brown, s FILL: Brown, s FILL: Brown, c	sand, A-3	424.3 424.0 423.3	_	1 3 4	0.7 S/15	19	stains, A-4		402.5	-25	2 1 2	N/C	33
FILL: Brown, s	sandy loam, A-4	422.0	_	2 2	N/C	20	CLAY: Gray, A-7			_	ST	0.5	
		420.0	_	2		20	Unconfined Compres Test Performed	sion Strength	400.5		01	0.5 S/2	33
FILL: Brown, s	and, A-3		_	ST	2.3	22	SANDY LOAM: Brown SAND: Brown, A-3	n, A-4	J	_	9 10	N/C	
FILL: Brown, o	clay, A-6	418.3 417.3			Р		Stop: 6/3/2016 at 3:0 Start: 6/6/2016 at 7:4			-30	8		
FILL: Brown, s brick, A-6	silty clay loam, with			4 5 5	0.6 S/10	21				_			
				3	4.0						7	NIG	
FILL: Brown, s	sand, A-3	413.3	-15	4 16	1.9 S/15	19				-35	11 12	N/C	
With gravel and	d crushed rock			7 8 7	N/C								
With concrete	fragments		-20	3 2 3	N/C	37				-40	8 12 17	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **SOIL BORING LOG**

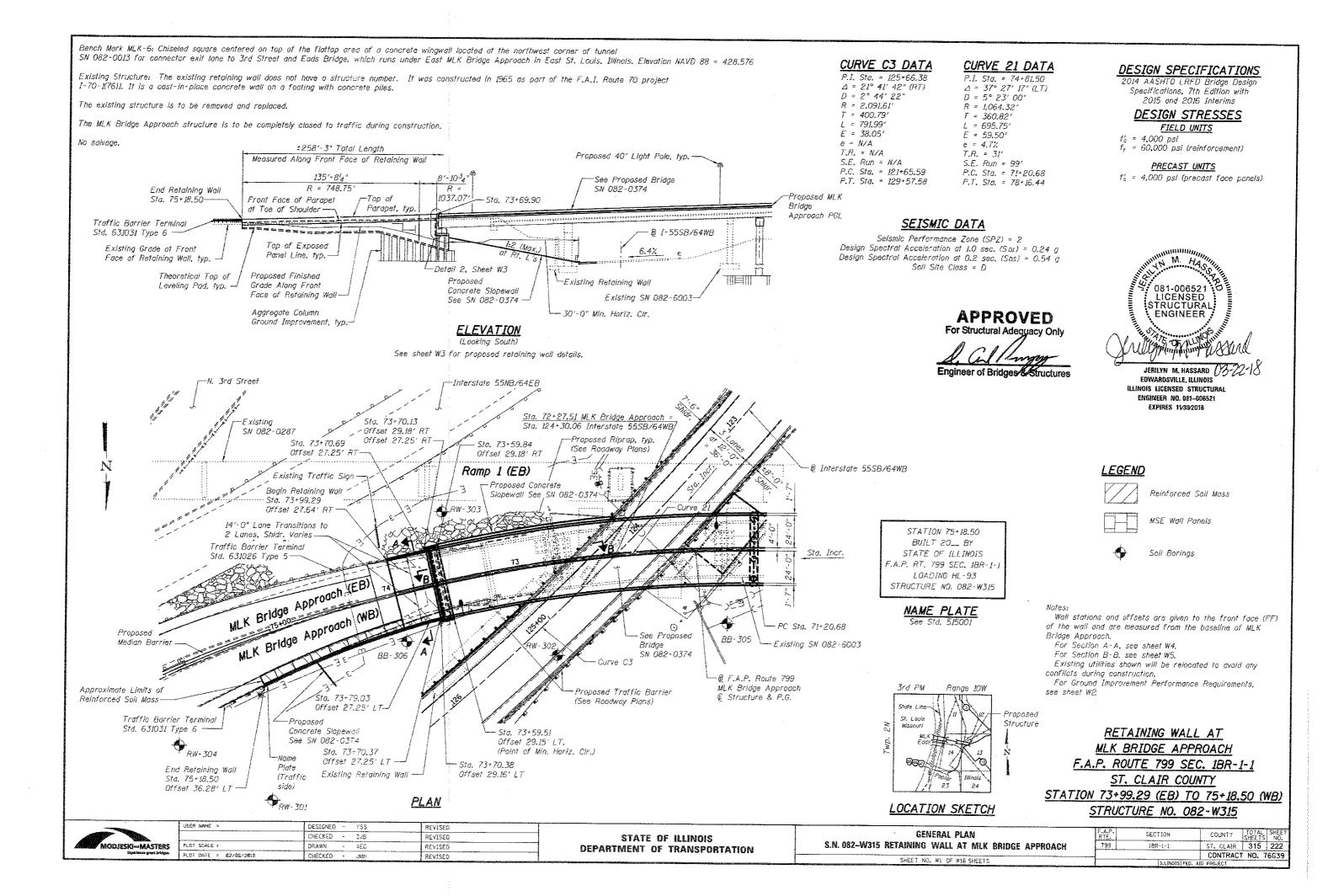
Page  $\underline{2}$  of  $\underline{2}$ 

Date 6/3-6/6/2016

	SCI Engineering, Inc.									Date 6/3-6/6/2
ROUTE	Interstate 64	DES	SCRI	PTION		PTB ²	172, Item 22 - FAP 799	(MLK Drive)	LOG	GED BY BDG (S
SECTION	1BR-1-1		_ L	OCAT	ION _		36208892, -90.1607292 ide , Longitude	2089, <b>SEC.</b> 13	3, <b>TWP.</b> 2N	I, <b>RNG.</b> 10W,
COUNTY	Saint Clair D	RILLING	MET	THOD			ME 550 w/HSA	_ HAMMER	TYPE	Automatic
STRUCT. NO.	SN 082-W315		D	В	U	М	Surface Water Elev.	N/A	ft	
Station	73+59 to 75+18.5	50	E P	L O	C S	0	Stream Bed Elev.	N/A	_ ft	
	RW-303		T	W	_	S	Groundwater Elev.:			
Station			Н	S	Qu	Т	First Encounter	384.5		
Offset	59.0 ft RT		/ ft\	(/6")	(tsf)	(%)	Upon Completion	N/A	_ ft	
Ground Surfa		ft	(11)	(,,,	(131)	(70)	After N/A Hrs.	N/A	_ ft	
SAND: BLOWN	n, A-3 (continued)	-								
			<u>_</u>	4	N/C					
			-45	6 7	14/0					
			_							
		-	_							
			_							
			_							
				4	N/C					
			_	6	14/0					
Roring termine	ited at 50.0 ft.	378.0	-50	7			-			
Boiling termina	ited at 50.0 It.		_							
		-								
Boring grouted	d to 50 ft.		_	-						
		-								
				-						
			-55							
			_							
			_							
			_							
			_							
			-60							



USER NAME =	DESIGNED - MJP	REVISED
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PLOT SCALE =	DRAWN - PRC	REVISED
PLOT DATE = 3/12/2018	CHECKED - MJP	REVISED



#### INDEX OF SHEETS

- W1 General Plan
- W2 General Notes
- W3 Unfolded Wall Elevation
- W4 MSE Details 1 W5 MSE Details - 2
- WS MSL Delails Z
- W6 Parapet and Anchorage SlabW7 Miscellaneous Details
- W8 Retaining Wall Parapet Slipforming Option
- W9 Soil Boring 1
- W10 Soil Boring 2
- W11 Soil Boring 3
- W12 Soil Boring 4
- W13 Soil Boring 5
- W14 Soil Boring 6
- W15 Soil Boring 7
- W16 Soil Boring 8

#### GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Wall stations and offsets are given to the front face (FF) of the wall and are measured from baseline of the MLK Bridge Approach structure and centerline of F.A.P. Route 799, except as noted. The FF of the wall is to be considered edge of panel.
- 3. See Special Provision for Aggregate Column Ground Improvement for design and construction requirements.
- 4. For existing soils laboratory data, see the Geotechnical Investigation Laboratory Data Special Provision.
- 5. The piles, with pile sleeves, for SN 082-0374 are located within the reinforced soil mass and will be driven prior to the placement of the reinforced soil mass. See SN 082-0374 plans for additional pile requirements.
- 6. Wall system supplier shall coordinate the proposed wall configuration with the Aggregate Column Ground Improvement subcontractor.
- 7. Wall construction shall not begin until after ground improvement has been completed in the area of the new wall.
- 8. The MSE wall supplier's internal stability design shall account for the seismic loading. For Seismic Data, see sheet WI.

#### MSE WALL SETTLEMENT

- 1. The Top of Exposed Panel Elevations shown on the plans are final elevations after any settlement
- 2. The MSE wall supplier is alerted to the fact that outside the ground improvement limits, one (1) inch of settlement is anticipated. The wall system supplier shall take appropriate measures to accommodate this settlement in the wall design.
- 3. The MSE wall supplier is alerted to the fact that within the ground improvement limits, the wall settlement will be determined by the ground improvement design. The wall system supplier shall coordinate with the Aggregate Column Ground Improvement subcontractor to accommodate this settlement in the wall design.

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	1,639
Concrete Superstructure	Cu. Yd.	61.3
Protective Coat	Sq. Yd.	138
Reinforcement Bars, Epoxy Coated	Pound	9,450
Name Plates	Each	1
Mechanically Stabilized Earth Retaining Wall	Sq. Ft.	2,313
Aggregate Column Ground Improvement	L. Sum	1

#### GROUND IMPROVEMENT PERFORMANCE REQUIREMENTS

- 1. Minimum factor of safety for global slope stability shall be 1.5 for both the permanent and temporary condition.
- 2. Allowable bearing pressure (with F.S.) shall be equal to or greater than the equivalent uniform service bearing pressure of 4300 psf.

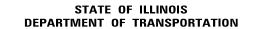
Minimum factor of safety against equivalent uniform service bearing pressure shall be 2.0 if a load test is performed.

Minimum factor of safety against equivalent uniform service bearing pressure shall be 2.5 if a load test is not performed.

- 3. Minimum coefficient of friction at the base of the wall shall be 0.624 for sliding.
- 4. Total settlement measured at the theoretical top of leveling pad shall not exceed 1.0 inch.
- 5. Total settlement measured on the pavement shall not exceed 1.0 inch.
- 6. Differential settlement measured along the theoretical top of leveling pad shall not exceed 1/100.
- 7. The assumed structure life for settlement computations shall be 75 years.
- 8. Contractor's verification program shall include monitoring points or other instrumentation to demonstrate compliance with the stated performance requirements.
- 9. The Shop Drawings and construction procedures submittal shall indicate the sequence of construction within the limits of Aggregate Column Ground Improvement. The aggregate column installation shall be coordinated with utility removal, structure removals, proposed utility installation, and bridge pile driving.
- 10. Aggregate columns shall be installed before the bridge piles are driven; however, the piles shall not be driven through the aggregate of an installed column. The aggregate column layout shall provide clearance for the bridge piles.
- 11. Primary consolidation of the soil within the depth of the ACGI to be at least 90 percent complete when the bridge piles are to be driven. Any required waiting periods shall be coordinated with the bridge construction schedule.

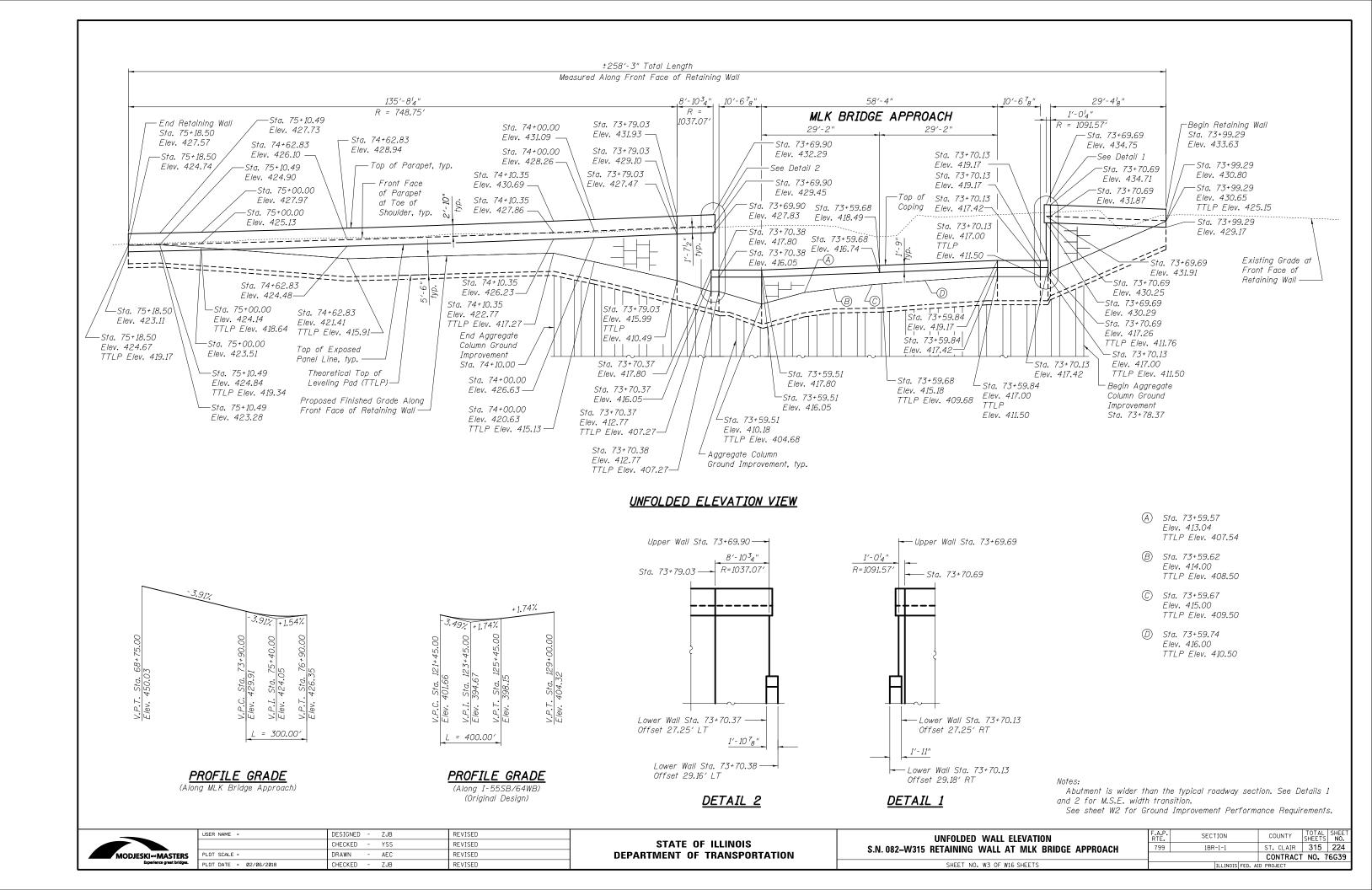


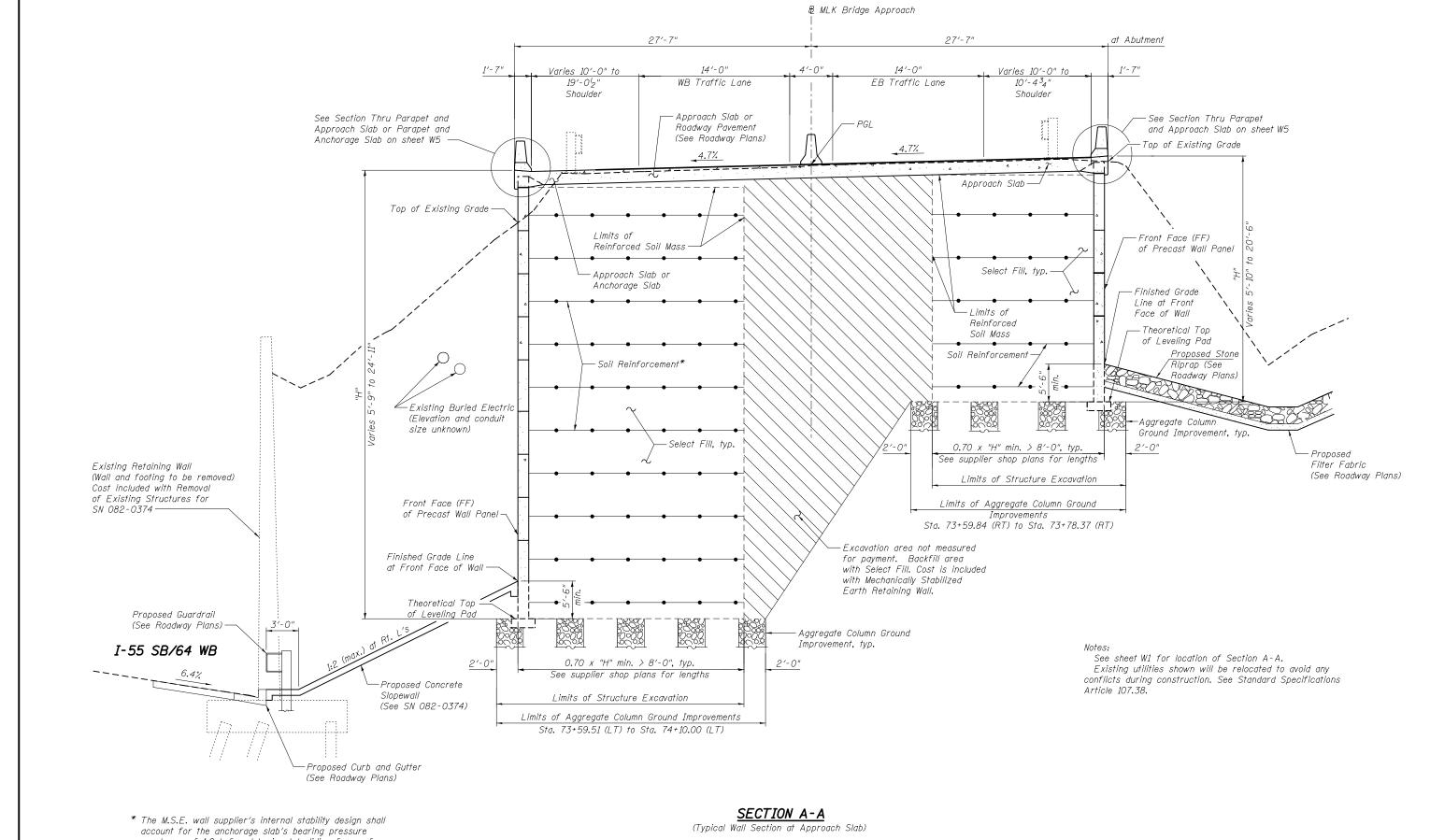
USER NAME =	DESIGNED	-	YSS	REVISED
	CHECKED	-	ZJB	REVISED
PLOT SCALE =	DRAWN	-	AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED	_	.IMH	REVISED



S.N. 082–W315	GENERAL NOTES RETAINING WALL AT MLK BRIDGE APPROACH	
	SHEET NO, W2 OF W16 SHEETS	

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
799	1BR-1-1	ST. CLAIR	315	223
		CONTRACT	NO. 7	6G39
	ILLINOIS FED. AI	ID PROJECT		



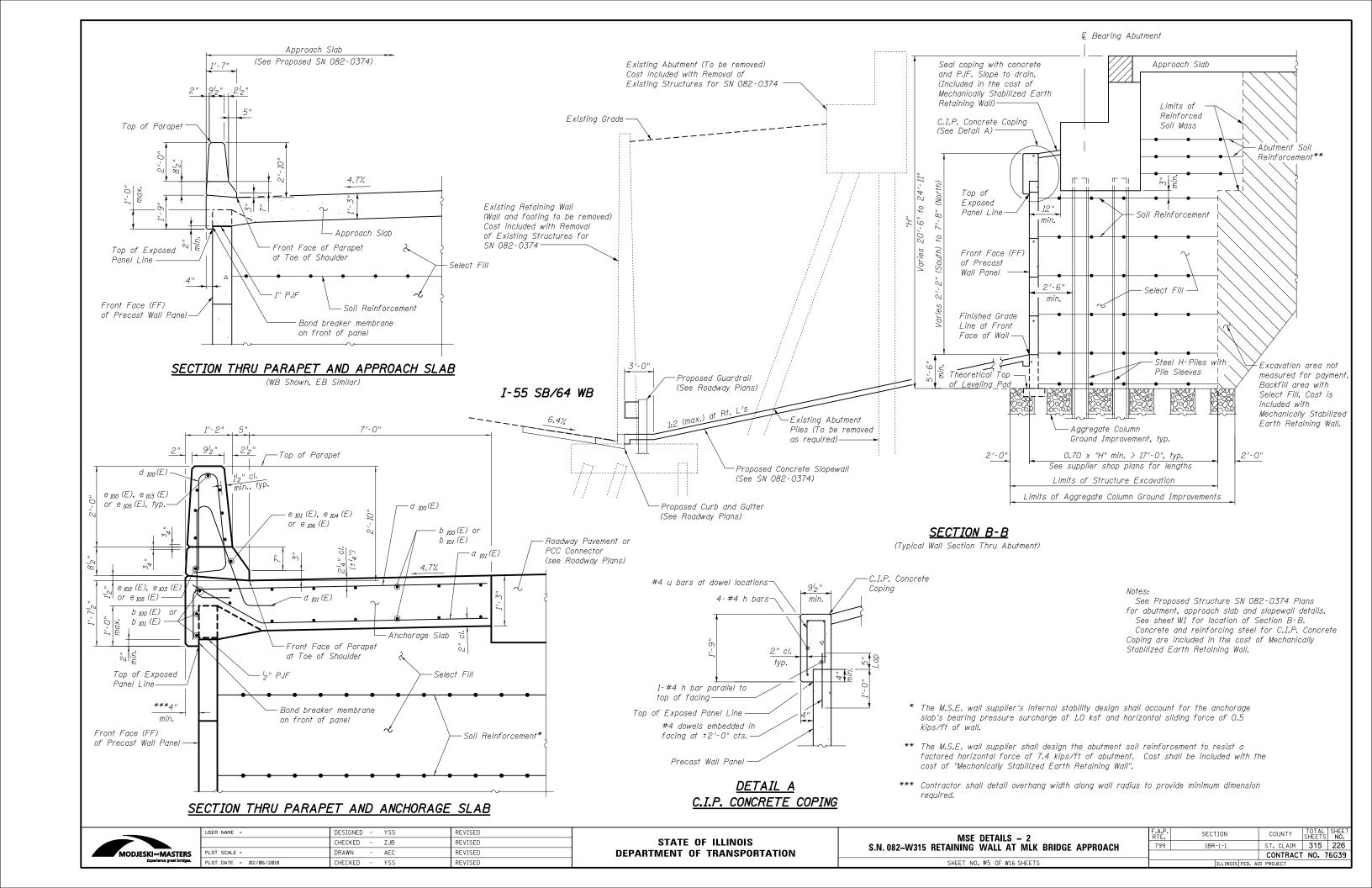


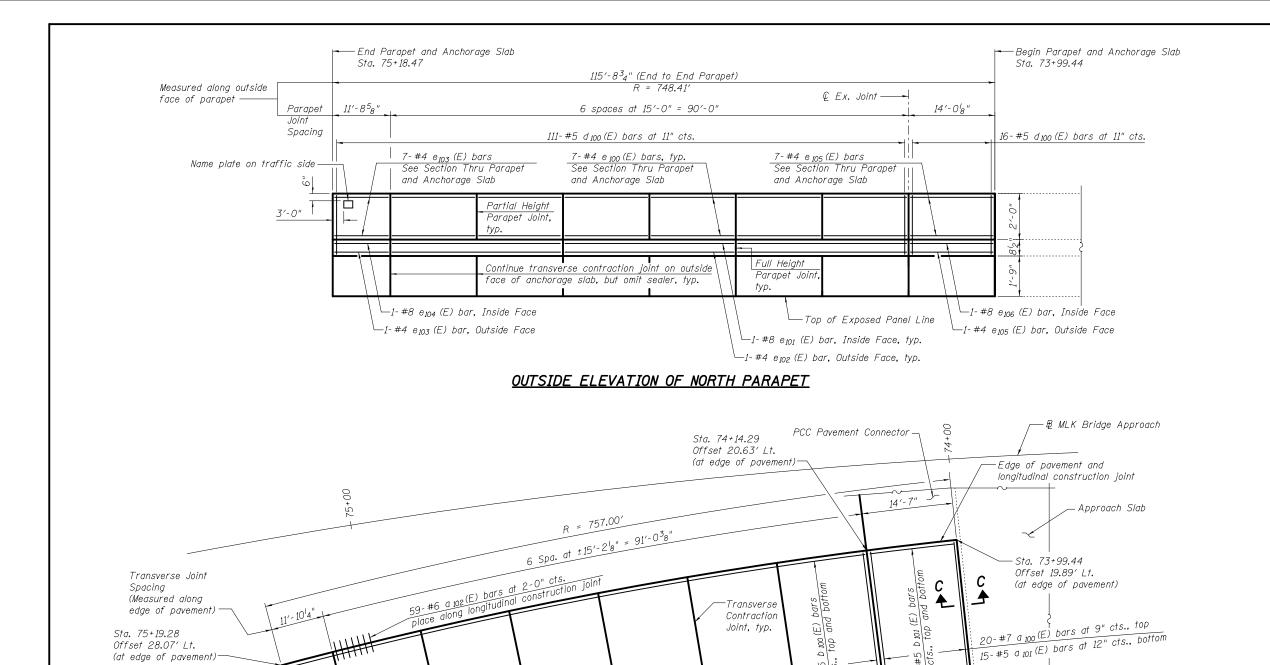
surcharge of 1.0 ksf and horizontal sliding force of 0.5 kips/ft. of wall.

MODJESKI....MASTERS

COUNTY TOTAL SHEET NO.

ST. CLAIR 315 225 USER NAME = DESIGNED - YSS REVISED SECTION MSE DETAILS - 1 STATE OF ILLINOIS CHECKED - ZJB REVISED S.N. 082-W315 RETAINING WALL AT MLK BRIDGE APPROACH 799 1BR-1-1 REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 76G39 LOT DATE = Ø2/Ø6/2018 CHECKED - YSS REVISED SHEET NO. W4 OF W16 SHEETS





137-#7 a 100(E) bars at 9" cts., top 103-#5 d 101 (E) bars at 12" cts., bottom Offset 26.90' Lt. (at toe of shoulder) - Sta. 73+99.44 Offset 28.48' Lt. <del>*3-#5</del> 16-#5 d 101 (E) bars at 11" cts. b 101 (E) bars — Front face of wall panel 14'-0¹8" 111-#5 d 101 (E) bars at 11" cts. — © Ex. Joint 6 Spa. at 15'-0" = 90'-0 3x5-#5 b 100 (E) bars * Cut bars in field to fit. PLAN - NORTH PARAPET AND ANCHORAGE SLAB

Sta. 73+99.44

For Section Thru Parapet and Anchorage Slab, see sheet W5.

For Section C-C and Bill of Material, see sheet W7.

Bars indicated thus 8x5-#5 etc. indicates 8 lines of bars with 5 lengths per line.

Joints in the adjacent pavement shall be aligned with the anchorage slab

Stations and offsets on this sheet are given to the outside face of the parapet and are measured from the baseline of MLK Bridge Approach, except as noted.



(at edge of pavement)-

Sta. 75+18.62

Offset 35.04' Lt.

(at toe of shoulder)-

Transverse Joint Spacing

(Measured along outside face of parapet)

Sta. 75+18.47

Offset 36.62' Lt. -

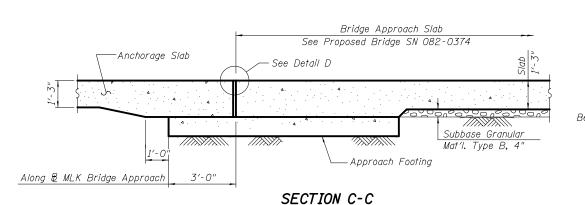
USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED - YSS	REVISED

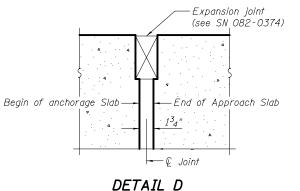
11'-858"

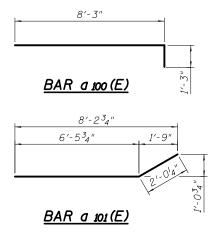
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

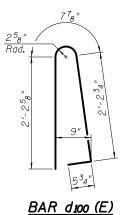
P S.N. 082–W315		D ANCHORAG Wall at Mli		APPROACH
	SHEET NO	WC OF WIC CHEET	c	

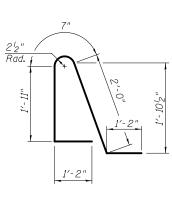
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
799	1BR-1-1	ST. CLAIR	315	227
		CONTRACT	NO. 7	6G39
	ILLINOIS FED. A	ID PROJECT		



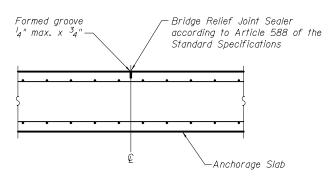




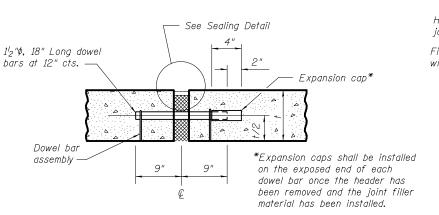




BAR dioi (E)

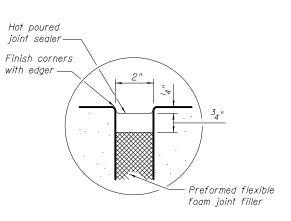


TRANSVERSE CONTRACTION JOINT



#### ANCHORAGE SLAB EXPANSION JOINT

Expansion joint and dowel bars included in the cost of Concrete Superstructure.



SEALING DETAIL

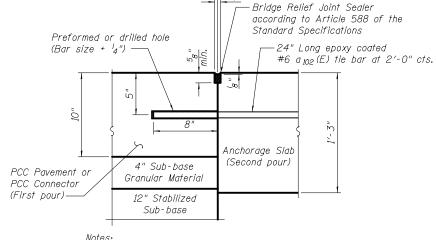
# BILL OF MATERIAL**

_				_
Bar	No.	Size	Length	Shape
a ₁₀₀ (E)	157	#7	9'-6"	$\overline{}$
a ₁₀₁ (E)	118	#5	8′-6"	
a ₁₀₂ (E)	59	#6	2'-0"	
b100 (E)	95	#5	23′-6"	
b ₁₀₁ (E)	19	#5	14'-4"	
				_
d100 (E)		#5	5′-7"	)
d101 (E)	127	#5	6'-10"	[_
e100 (E)	42	#4	14'-9"	
e101 (E)		#8	29′-9"	
e ₁₀₂ (E)	3	#4	29′-9"	
e103 (E)	8	#4	11'-5"	
e ₁₀₄ (E)	1	#8	11'-5"	
e ₁₀₅ (E)		#4	13′-9"	
e106 (E)	1	#8	13′-9"	
	cement	Bars,	Pound	9,450
Ероху			, 53/10	0,.00
Concre			Cu. Yd.	61.3
Supers	tructure			01.0

^{**}For anchorage slab and north parapet thereon

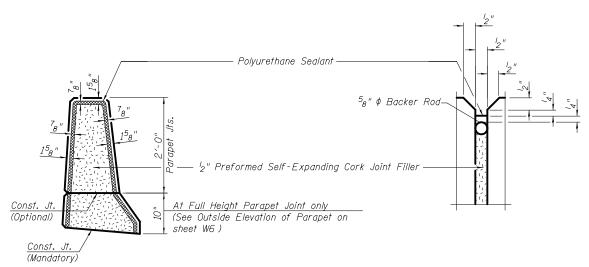
#### MIN. BAR LAP

#5 bars - 3'-6"



The Contractor may substitute at his option, formed in place tie bars provided the bar length is increased to 30" and the tie bar is centered across the joint. Preformed or drilled hole shall be in the first pour.

LONGITUDINAL CONSTRUCTION JOINT GROUTED-IN-PLACE TIE BAR



PARAPET JOINT DETAILS

Notes:

For location of Section C-C, see sheet W6.

The Polyurethane Sealant shall be non-staining gray one component non-sag elastomeric gun grade meeting the requirements of ASTM C-920, Type S, Grade NS, Class 25. Use T with a ⁵8" backer rod. Cost included with Concrete Superstructure,

The ½" Preformed Self-Expanding Cork Joint Filler shall be according to Article 1051.07 of the Standard Specifications. Cost included with Concrete Superstructure,

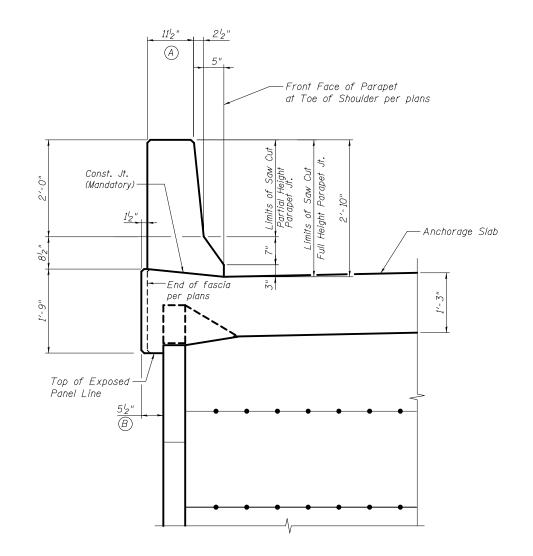


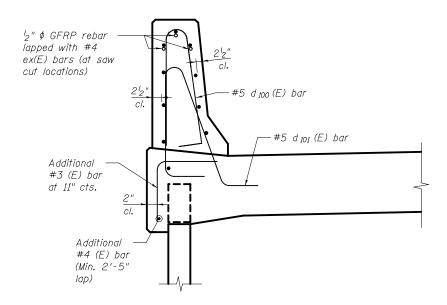
	USER NAME =	DESIGNED	-	YSS	REVISED
		CHECKED	-	ZJB	REVISED
;	PLOT SCALE =	DRAWN	-	AEC	REVISED
•	PLOT DATE = 02/06/2018	CHECKED	-	YSS	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

MISCELLANEOUS DETAILS	F.A.P. RTE.	SECTION
S.N. 082-W315 RETAINING WALL AT MLK BRIDGE APPROACH	799	1BR-1-1
CUEET NO WZ OF WAS SUFETS		

F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
799	1BR-1-1		ST. CLAIR	315	228
			CONTRACT	NO. 7	6G39
	ILLINOIS	FED. AI	D PROJECT		

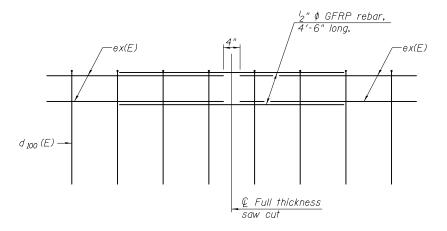




#### **SECTION**

(Showing reinforcement clearances for slip forming and additional reinforcement bars)

#### SECTION THRU PARAPET AND ANCHORAGE SLAB



#### GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)



USER NAME =	DESIGNED -	YSS	REVISED	Г
	CHECKED -	ZJB	REVISED	ı
PLOT SCALE =	DRAWN -	AEC	REVISED	ı
PLOT DATE = 02/06/2018	CHECKED -	YSS	REVISED	ı

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

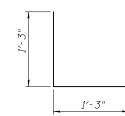
RETAINING WALL PARAPET SLIPFORMING OPTION S.N. 082-W315 RETAINING WALL AT MLK BRIDGE APPROACH
SHEET NO. W8 OF W16 SHEETS

		CONTRACT	NO. 7	6G39
799	1BR-1-1	ST. CLAIR	315	229
F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.

<u>GENERAL NOTES</u>

All dimensions shall remain the same as shown on MSE wall details, except dimensions A and B which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A and B = 0.0165 cu. yds./ft.

Full thickness saw cut at all joint locations in lieu of cork joint filler.



#3 (E) BAR



Page  $\underline{1}$  of  $\underline{1}$ 

Date 5/25/16

	SCI Engineering, Inc.										Dute		.07.10
ROUTE	Interstate 64	DE:	SCRI	PTION		PTB ²	172, Item 22 - FAP 799	(MLK Drive)	L	OGGI	ED BY	BDG	(SC
SECTION	1BR-1	-1	_ ı	OCAT	ION .		90913789, -90.1600632 de , Longitude	2111, <b>SEC.</b> 1	3, <b>TWP.</b>	2N, <b>F</b>	NG. 1	0W,	
COUNTY	Saint Clair	DRILLING	ME	THOD			ME 550 w/HSA	_ HAMMER	TYPE		Auto	omatic	
STRUCT. NO.	SN 082-W 73+59 to 75-		D E	B L	U	M	Surface Water Elev. Stream Bed Elev.		_ ft ft	D E	B L	C	M
BORING NO.	RW-30 ²	1	P T H	O W S	S Qu	S	Groundwater Elev.:			P T H	O W S	S Qu	S T
Offset	75+38 107.0 ft L	_T					First Encounter Upon Completion	N/A N/A	ft ft				
Ground Surfa			( ft)	(/6")	(tsf)	(%)	After N/A Hrs.		ft	(ft)	(/6")	(tsf)	(%
	and gray, clay, tra		. –	-			SAND: Brown, A-3 (d	continuea)		_	10		
ron stains, A-6		doc		1							10	N/C	
			_	3	1.2	37				_	10		
				3	B/20	01							
		401.0	_				1						
	Gray and brown										11	NUC	
race sand, A-	5			2	0.2						14	N/C	
			_	2	0.2 B/20	40				_	16		
			-5	2	5/20					-25			
	Gray and brown	398.5	-	-						_	7		
race iron stain		1,		WOH							7	N/C	
	,		_	1	0.9	34	Becomes gray			_	9		
				2	B/20	34	Boodines gray						
			_							_			
		395.5									6		
SAND: Brown	n, A-3						Trace organics				8	N/C	
				ST							9		
			-10							-30			
			_							_			
≾egan mud ro	tary at 11.0 ft	392.3	_	2	N/C	.				_			
	Gray and brown			4	14/0	33							
A-6		i	_	3						_			
SAND: Brown	1, A-3		_	2	0.5	45					4		
Trace silt			_	3	P	45	B			_	4	N/C	
Becomes brov	vn			5	N/C	30	Becomes brown				4		
			-15							-35			
				5	NIC								
				5	N/C								
				5			Low mud return, woo	d debris					
			_				observed			_			
				_							F		
			_	3	N/C					_	5 5	N/C	
				6					204.5		6		
			-20	0			Poring terminated at	30.5 ft	364.5	-40	0		
			-20				Boring terminated at	งซ.จ II.		-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)

Boring grouted to 39.5 ft.



# **SOIL BORING LOG**

Page  $\underline{1}$  of  $\underline{1}$ 

**Date** 6/1/16

	SCI Engineering, Inc.										Date	0/	1/10
ROUTE	Interstate 64	_ DE	SCRI	PTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	L0	OGGI	D BY	TC	(SCI)
SECTION _	1BR-1-1		_ ι	OCAT	ION _		39833282, -90.1609112 de , Longitude	2849, <b>SEC.</b> 13	, <b>TWP.</b> :	2N, <b>F</b>	NG. 1	0W,	
COUNTY _	Saint Clair DR	RILLING	MET	THOD			ME 550 w/HSA	_ HAMMER	TYPE		Auto	matic	
STRUCT. NO.	73+59 to 75+18.50	0	D E P	B L O	UCS	M 0 1	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L O	U C S	M O I
	RW-302		T	w	<b></b>	S	Groundwater Elev.:		_	T	w	<b>.</b>	S
	72+84		Н	S	Qu	Т	First Encounter	382.0	_ ft. <u>▼</u>	Н	S	Qu	Т
Offset	67.0 ft LT		/ £4\	(/6")	(tsf)	(0/)	Upon Completion	N/A	_ ft	/ <del>f</del> t\	//e!!\	(tof)	(0/)
	face Elev. 400.0	ft	( ft)	(/6 )	(tSI)	(%)	After N/A Hrs.	N/A	_ft	( ft)	(/6")	(tsf)	(%)
1.2 ft. ASPHA	ALTIC CONCRETE	200.0	_				SAND: Brown, A-3 (c SANDY LOAM: Gray		379.5				
FILL: Brown	and gray, sandy clay,	398.9	-	2							1		
A-6	and gray, sandy say,		_	8 10	0.9 S/15	14				_	5 9	N/C	
		397.0											
	sand and crushed								376.5				
rock, A-1				3			SAND: Gray trace bro	own, A-3			1		
				3	N/C	3					4	N/C	
			-5	3		-			375.0	-25	8		
		0040	_				SANDY LOAM: Gray Began mud rotary at 2			_			
SANDYLOAI	M: Brown, A-6	394.0		3			,				8		
0,410120,4	VII. BIOWII, 71 O		_	6	N/C	12				_	13	N/C	
				7		12					15		
			_	-					.=	_	13		
				-			SAND: Gray trace bro	OWD A 3	372.0				
SAND: Brow	n A 3	<u>391.5</u>		2			SAND. Glay liace bit	JWII, A-3		_	7		
SAND. BIOW	II, A-3			4	N/C						7	N/C	
			_	4	""	10				_	9	1,,0	
			-10	6						-30	11		
			_							_			
			_	3	N/C					_			
				4	1.00	7							
			_	6						_			
				-									
			_	-						_	40		
				3	N/C						13		
				8	5	3				_	15		
			-15	12		-				-35	14		
			_							_			
				_		-							
				5	N/C								
				9	""	13							
			_	10						_			
			▼										
			_							_			
				2	N/C						15	N/C	
Some silt			_	7	14/0					_	16	14/0	
			-20	10					360.0	-40	12		

Boring terminated at 40.0 ft. Boring grouted to 40.0 ft.



USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED - JMH	REVISED

SOIL BORING - 1	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-W315 RETAINING WALL AT MLK BRIDGE APPROACH	799	1BR-1-1	ST. CLAIR	315	230
			CONTRACT	NO. 7	6G39
SHEET NO. W9 OF W16 SHEETS		ILLINOIS FED. A	D PROJECT		



Page  $\underline{1}$  of  $\underline{2}$ 

Date 6/3-6/6/2016

	SCI Engineering, Inc.											2.0 01	
ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	172, Item 22 - FAP 799	(MLK Drive)	L0	OGGE	ED BY	BDG	(SCI
SECTION	1BR-1-1		_ ι	_OCAT	ION .		36208892, -90.1607292 Ide , <b>Longitude</b>	2089, <b>SEC.</b> 13	, TWP. :	2N, <b>F</b>	NG. 1	0W,	
COUNTY	Saint Clair	DRILLING	MET	THOD			ME 550 w/HSA	HAMMER	TYPE		Auto	omatic	
STRUCT. NO. Station	SN 082-W31 73+59 to 75+18		D E P	B L O	U C S	М О І	Surface Water Elev. Stream Bed Elev.	N/A N/A	_ ft _ ft	D E P	B L O	U C S	M O I
BORING NO. Station Offset	73+50		H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	384.5 N/A	_ ft. <u>▼</u> _ ft	H	W S	Qu	S T
Ground Surfa		.0 <b>ft</b>	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%)
RIP-RAP with	SILTY CLAY	427.0	_				Clogging in split spoo concrete fragments		407.5				
FILL: Brown, o	clay loam, A-6		_	3 5 6	N/C	17	FILL: Brown, sand, A (continued) FILL: Gray, clay, A-7		j J		5 3 3	1.0 S/15	34
			_				SILTY SAND: Gray, i	trace iron	405.0				
FILL: Brown, s FILL: Brown, s FILL: Brown, c	sand, A-3	424.3 424.0 423.3	_	1 3 4	0.7 S/15	19	stains, A-4		402.5	-25	2 1 2	N/C	33
FILL: Brown, s	sandy loam, A-4	422.0	_	2 2	N/C	20	CLAY: Gray, A-7			_	ST	0.5	
		420.0	_	2		20	Unconfined Compres Test Performed	sion Strength	400.5		01	0.5 S/2	33
FILL: Brown, s	and, A-3		_	ST	2.3	22	SANDY LOAM: Brown SAND: Brown, A-3	n, A-4	J	_	9 10	N/C	
FILL: Brown, o	clay, A-6	418.3 417.3			Р		Stop: 6/3/2016 at 3:0 Start: 6/6/2016 at 7:4			-30	8		
FILL: Brown, s brick, A-6	silty clay loam, with			4 5 5	0.6 S/10	21				_			
				3	4.0						7	NIG	
FILL: Brown, s	sand, A-3	413.3	-15	4 16	1.9 S/15	19				-35	11 12	N/C	
With gravel and	d crushed rock			7 8 7	N/C								
With concrete	fragments		-20	3 2 3	N/C	37				-40	8 12 17	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



#### **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{2}$ 

Date 6/3-6/6/2016

	SCI Engineering, Inc.									Date 6/3-6/6/2
ROUTE	Interstate 64	DES	CRII	PTION		PTB ′	172, Item 22 - FAP 799	(MLK Drive)	LOG	GED BY BDG (S
SECTION	1BR-1-1		_ L	OCAT	ION _		36208892, -90.1607292 de , Longitude	2089, <b>SEC.</b> 13	3, <b>TWP.</b> 2N	, <b>RNG.</b> 10W,
COUNTY	Saint Clair DI	RILLING	MET	HOD			ME 550 w/HSA	_ HAMMER	TYPE	Automatic
STRUCT. NO.	SN 082-W315 73+59 to 75+18.5		D E	B L	U	М	Surface Water Elev.	N/A	_ ft	
Station	73+39 to 73+16.3	_	Р	0	s	1	Stream Bed Elev.	N/A	_ ft	
	RW-303		T H	W S	Qu	S	Groundwater Elev.:		- <del>-</del>	
Station Offset	73+50 59.0 ft RT		_	3	Qu	'	First Encounter Upon Completion	384.5 N/A	_ ft. <u>▼</u> ft	
Ground Surfa		— _{ft}	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	– IL ft	
	n, A-3 (continued)	``	. ,				74101 11171 11101	10/1	_ ''	
0,	., , , , , , (00), (1), (1)		$\exists$							
		_								
		_								
			-							
		-	<b>,</b>							
		=	<u>-</u>	4						
		_		6	N/C					
			-45	7						
			-							
		-								
		-								
		-								
			-	4						
		_		6	N/C					
		378.0	-50	7						
Boring termina	ited at 50.0 ft.									
		-								
Boring grouted	d to 50 ft.		$\dashv$							
		-								
		_								
		_	_							
			-55							
		-	-55							
		_								
		_								
			_							
		-	-							
			$\dashv$							
		_								
			-60							



USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED - JMH	REVISED



Page  $\underline{1}$  of  $\underline{2}$ 

Date 5/25/16

ROUTE	Interstate 64	_ DES	CRI	HTION		HIR,	172, Item 22 - FAP 799 (MLK Drive)	L	UGGI	D BY	RDG	(SC
SECTION _	1BR-1-1		_ L	OCAT	ION _		39209382, -90.1598692515, <b>SEC.</b> de , <b>Longitude</b>	13, <b>TWP.</b>	2N, <b>F</b>	NG. 1	0W,	
COUNTY	Saint Clair DR	ILLING	MET	THOD			ME 550 w/HSA HAMME	RTYPE		Auto	omatic	
STRUCT. NO. Station		)	D E P	B L O	U C S	M O I	Surface Water Elev. N/A Stream Bed Elev. N/A		D E P	B L O	U C S	N C
BORING NO. Station Offset	75+98	_	T H	w s	Qu	S T	Groundwater Elev.: First Encounter N/A Upon Completion N/A		T H	W S	Qu	S
	ace Elev. 422.0	ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs. N/A		( ft)	(/6")	(tsf)	(%
FILL: Brown, sand, A-6	silty clay, trace fine		_				SILTY CLAY: Brown, A-6 (continued)	401.5				
			_	3 5 7	1.6 S/15	17	SILTY LOAM: Brown, A-5 SILTY SAND: Brown, A-5	400.5	_	1 2	1.9 B/20	30
Ell I · Brown	silty loam, A-4	419.0	_		0,10		CLAY: Gray, trace iron nodules,	399.0		3	0.5 P	
·	own, silty clay, trace	416.5	-5	3 4 5	0.6 S/15	25	A-6  Unconfined Compression Strengt Test Performed	h	-25	ST	0.7 S/6	33
	ck fragments, A-6		_	3 3 5	1.6 S/15	22	With iron stains			1 2 3	0.5 P	3
FILL: Gray, si	Ity loam, A-6	414.0	_				SAND: Brown, A-3	394.0	_	3		
Unconfined Co Test Performe	ompression Strength		-10	ST	1.4 S/4	20			-30	5 7	N/C	
	and red, gravel and ck fragments, A-2	411.0	_	4 9 50/3"	N/C	11	END OF DAY: 5/25 START OF DAY: 5/26 (Delayed due to lightning 2.25 hrs)					
FILL: Brown,	sand, A-3	409.5	_	, 30/3					_	4		
FILL: Black a	nd dark brown,	407.8	_	1 9	N/C	11	Becomes gray		_	4 5	N/C	
crushed aspha	altic, A-3		-15	7		28			-35			
Unconfined C Test Performe		405.5	_	ST	1.0 S/1.8	34						
SILTY CLAY:	Brown, A-6			2 3	1.5	25				9 10 12	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **SOIL BORING LOG**

	Division of Highways SCI Engineering, Inc.									Date	5/25/16
ROUTE	Interstate 64	DES	SCRI	PTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	LO	GGED BY	BDG (SCI)
SECTION _	1BR-1-1		_ ι	OCAT	ION .		39209382, -90.1598692	2515, <b>SEC.</b> 13,	, <b>TWP.</b> 21	N, <b>RNG.</b> 10	W,
COUNTY _	Saint Clair DI	RILLING	MET	THOD			de , Longitude ME 550 w/HSA	HAMMER 1	ГҮРЕ	Auto	matic
STRUCT. NO. Station BORING NO. Station	73+59 to 75+18.5	50	D E P T H	B L O W S	U C S	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter	N/A N/A	_ ft _ ft		
Offset	39.0 ft LT			//OW	4-6	(0/)	Upon Completion	N/A	ft		
	n, A-3 (continued)	ft	(π)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft		
		372.5	-45	5 8 12 10 14 17	N/C						
Boring groute	ated at 49.5 ft.		-50								



USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED - JMH	REVISED

SOIL BORING - 3	F.A.P. RTE.	SE
S.N. 082-W315 RETAINING WALL AT MLK BRIDGE APPROACH	799	16
SHEET NO. W11 OF W16 SHEETS		



Page  $\underline{1}$  of  $\underline{4}$ 

Date5/25-5/31/2016

SCI Engineering, Inc.												
Interstate 64	DE	SCRI	PTION		PTB ²	172, Item 22 - FAP 799	(MLK Drive)	L0	OGGE	D BY	TC	(SCI
1BR-1-1		_ ı	OCAT	ION .			3778, <b>SEC.</b> 13	TWP.	2N, <b>F</b>	NG. 1	0W,	
Saint Clair [	DRILLING	ME	THOD			, ,	HAMMER	TYPE		Auto	omatic	
082-6003(E), 082-0374(P) 73+59 to 75+18.		D E	B L	U C	М			ft ft	D E	B L	U	M O I
BB-305 71+43		T H	w s	Qu	S T	Groundwater Elev.: First Encounter		_	T H	W S	Qu	s T
	) ff	( ft)	(/6")	(tsf)	(%)				( ft)	(/6")	(tsf)	(%
MESTONE: Gray,		_	,	(,	(,	SANDY LOAM: Brow			_	<b>V</b> - 7	<b>(</b> ,	
lay, with cinders and e gravel, A-6		_	4	>4.5		. (************************************			_	2	N/C	
		_	7	Р						5		
			2							1		
vel in shoe, no		-5	3						-25	4 5	N/C	
			1	0.5						3 5	N/C	
	4 <u>07.0</u>	_	1	Р					_	6		
<b>\-7</b>		_	_	4.5					_	_	NVO	
								385.9			N/C	
		-10	1			CLAY: Brown, trace f	ine sand, A-6		-30		0.3	
				4							_P_/	
nish-gray		_	1 2 2	1.6 S/20		SANDY LOAM: Brow	/n, A-4	383.5	-			
		_							_			
			1	ΩR						6	N/C	
		-15	1 2	S/20					-35	9 11	IV/C	
, A-3	399.5								_			
			3 5 7	N/C		SAND: Brown, A-3		<u>378.0</u>	<u> </u>			
									_			
			2						_	3		
I: Brown, A-4		-20	2 6	0.2 S/20		Becomes gray Began mud rotary at 4	10.0 ft		-40	5 13	N/C	
	Interstate 64  1BR-1-1  Saint Clair  082-6003(E), 082-0374(P) 73+59 to 75+18  BB-305 71+43 55.0 ft LT  Ince Elev. 415.0  MESTONE: Gray, ay, with cinders and a gravel, A-6  wel in shoe, no  A-7  A-7  A-7  A-6	Interstate 64 DE  1BR-1-1  Saint Clair DRILLING  082-6003(E),  082-0374(P)  73+59 to 75+18.50  BB-305  71+43  55.0 ft LT  Ince Elev. 415.0 ft  MESTONE: Gray,  414.0 ay, with cinders and a gravel, A-6  wel in shoe, no  407.0  A-7  A-7  399.5  A-6  396.5  A-6  396.0	Interstate 64   DESCRI   1BR-1-1   L     Saint Clair   DRILLING MET   082-6003(E), 082-0374(P)   73+59 to 75+18.50   E   P   T   H   S5.0 ft LT   ft   (ft)   MESTONE: Gray, ay, with cinders and e gravel, A-6   well in shoe, no	Interstate 64   DESCRIPTION	Interstate 64   DESCRIPTION	Interstate 64   DESCRIPTION   PTB	Interstate 64   DESCRIPTION	Interstate 64   DESCRIPTION	Interstate 64   DESCRIPTION	Therstate 64   DESCRIPTION   PTB 172, Item 22 - FAP 799 (MLK Drive)   LOGGE	Titlerstate 64   DESCRIPTION   PTB 172, Item 22 - FAP 799 (MLK Drive)   LOGGED BY	Titlerstate 64   DESCRIPTION   PTB 172, Item 22 - FAP 799 (MLK Drive)   LOGGED BY TO

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{4}$ 

Date5/25-5/31/2016

SCI Engineering, Inc.											
ROUTE Interstate 64	DESCR	RIPTION		PTB	172, Item 22 - FAP 799	(MLK Drive)	_ L0	OGGE	D BY	TC	(SCI)
SECTION 1BR-1	-1	LOCAT	ION _		90280807, -90.1613723 ide , Longitude	778, <b>SEC.</b> 13,	TWP.	2N, R	NG. 1	0W,	
COUNTY Saint Clair	DRILLING ME	THOD			ME 550 w/HSA	_ HAMMER T	YPE		Auto	matic	
STRUCT. NO.       082-6003         Station       082-0374         73+59 to 75	(P) <b>D</b>	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft ft	D E P	B L O	U C S	M 0 1
BORING NO.         BB-305           Station         71+43           Offset         55.0 ft L	5 T	W S	Qu	S	Groundwater Elev.: First Encounter Upon Completion	377.5 N/A	ft. <u>▼</u> ft	T H	W S	Qu	S
Ground Surface Elev. 4		) (/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%)
SAND: Brown, A-3 (continued					SAND: Gray, A-3 (con	ntinued)					
	  -4!	12 12 12 5 12	N/C					-65	14 12 10	N/C	
SANDY LOAM: Gray, A-4	368.0				GRAVEL: Gray, A-1		348.0 346.5				
	-50	10 18 18	N/C		SAND: Gray, A-3		345.0		11 16 17	N/C	
					GRAVEL: Gray, A-1			_			
SAND: Gray, A-3							341.5				
	-55	9 14 5 16	N/C		SAND: Gray, A-3			-75	14 16 23	N/C	
	 - -										
	-60	9 8 13	N/C		SANDY LOAM: Gray, SAND: Gray, A-3	, A-4	336.5 335.9 335.3		11 15 17	N/C	



USER NAME =	DESIGNED -	YSS	REVISED
	CHECKED -	ZJB	REVISED
PLOT SCALE =	DRAWN -	AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED -	JMH	REVISED

SOIL BORING - 4	F.A.P. RTE.	SECTION
S.N. 082-W315 RETAINING WALL AT MLK BRIDGE APPROACH	799	1BR-1-:



Page  $\underline{3}$  of  $\underline{4}$ 

Date5/25-5/31/2016

ROUTE	Interstate 64	_ DESC	KIPTIO	N	PIB	172, Item 22 - FAP 799	(IVILK Drive)	L	JGG	-D BY	10	(SCI)
SECTION	1BR-1-1		LOCA	TION		90280807, -90.1613723	3778, <b>SEC.</b> 13	, TWP.	2N, <b>F</b>	NG. 1	0W,	
COUNTY	Saint Clair DF	RILLING M	ETHO			ide , Longitude ME 550 w/HSA	HAMMER	TYPE		Auto	matic	
	082-6003(E),	Γ,	, _B	U	M	II			_	В	U	N/A
STRUCT. NO. Station	082-0374(P) 73+59 to 75+18.5		D B	C	M	Surface Water Elev. Stream Bed Elev.	N/A N/A	– ft ft	D	L	C	M
Station	70100 to 70110.0		0	s	Ī	Stream bed Elev.	IN/A	_ "	P	ō	s	Ī
BORING NO.			ΓW		S	Groundwater Elev.:			Т	W		S
Station		'	⊣ s	Qu	Т	First Encounter	377.5	ft⊻	Н	S	Qu	Т
Offset	55.0 ft LT	,	ft) (/6"	) (tsf)	(%)	Upon Completion	N/A	_ ft	( ft)	(/6")	(tsf)	(%
	ce Elev. 415.0	ft (	11) (/6	) (ເວເ)	( /0)	After N/A Hrs.	N/A	_ ft	(11)	(10)	(LSI)	( /0,
	y, A-1 (continued)		_			GRAVEL: Gray, A-1			_			
Rough drilling i	from 80.0 ft82.0 ft.	_										
			_						_			
		_										
									_			
		331.5						311.5				
SAND: Gray, A	A-3		13	NIO		SAND: Brownish-gra	y, trace			14	NIO	
			_ 9	N/C		gravel, A-3			_	27	N/C	
		-	85 12			Dough drilling from 10	NE O # 407 0		-105	36		
			_			Rough drilling from 10 ft.	J5.U IL-1U7.6		_			
		_										
			_						_			
		_	_									
										40		
										48 50/4"		
Coarse sand a	nd rock fragments	_	20	NIO		Weathered limestone fragments in shoe; se				30/4		
	g		22			109.3 ft.	t casing at					
		_	90 21			STOP: 5/26/2016		305.0	-110	50/5"		
			_			START: 5/27/2016 COBBLE w/ GRAVEI	Cray and	j	_	30/3	\ <u> </u>	
		_	_			brown, A-1	Gray and					
						No recovery						
Rough drilling f	from 92.0 ft93.0 ft.	_				Borehole continued w	ith rook	302.7	-	50/3"	/	
		_				coring.	AUT TOCK					
			<del></del>			-			_			
		_	14 15	NI/O								
			95 14						-115			
		_	33 14			-			-113			
									_			
		_										
			_									
		_										
			8		+	-			_			
		_	ا 18	N/C								
		315.0 -1	_						-120			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **ROCK CORE LOG**

Page  $\underline{4}$  of  $\underline{4}$ 

Date5/25-5/31/2016

	SCI Engineering, Inc.								ale 3/23-	3/3 1/2
ROUTE	Interstate 64	DESCRIPTION	PTB 172, Item	22 - FAP 799 (	MLK Drive	e)	_ LO	GGED	BY TO	C (SC
SECTION	1BR-1-1	LOCATION	38.6290280807 Latitude , Long	7, -90.1613723	778, <b>SEC</b> .	13, 1	<b>TWP.</b> 2	N, <b>RN</b>	<b>G.</b> 10W,	
COUNTY	Saint Clair CO	ORING METHOD	Latitude , Long	gitude			R	_	CORE	S
STRUCT. NO. Station	082-6003(E), 082-0374(P) 73+59 to 75+18.50	CORING BARRE  Core Diameter		NX Wireline	D E	C	E C O V	R Q	T I M	T R E N
BORING NO. Station Offset	BB-305 71+43 55.0 ft LT	Top of Rock El Begin Core Ele	<b>ev.</b> 298.5	ft	P T H	R E	E R Y		E	G T H
Ground Surfa		ft			( ft)	(#)	(%)	(%)	(min/ft)	(ts
GRAVEL: Lim	estone and granite co	bbles and boulders		30	2.70	1	12	0		
					-115					
					_	2	69	56	5	
				20	98.50		09	36	5	
	: Gray, moderately ha	rd, very fine grained, ba	nded, slightly wea							
calcareous Becomes med	lium bedded				_					
					_					94.
Becomes thick	dy bedded				-120	3	95	82	2.8	
					_					
2.0"1	:! <b>6-</b>	Heim to an adition to add a								85.
3.2" open verti 2.2" open verti		thin to medium bedded	1							
z.z open veru	icai iracture				_					
Becomes band	ded to thinly bedded, t	trace styolites			-125	4	100	74	2.4	
					_					
										157
					_					
					7.45					
LIMESTONE:	Light gray, moderatel	y hard, very finely crysta	Illine, medium bed		37.15	5	86	54	4	
weathered	0 0 7/	, , , , ,	,	, 0 ,	_			• •		
										230
					-130					
						1	1	1	1	
				28	34.50_					
1.75" open ver Boring termina	rtical fracture ated at 130.5 ft.			28	34.50					

Color pictures of the cores	Yes	_
Cores will be stored for exami	nation until	
The "Strength" column repres	ente the uniquial compressive	e strength of the core sample (ASTM D-293)

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED - JMH	REVISED



Page  $\underline{1}$  of  $\underline{5}$ 

**Date** 5/26-6/2-2016

ROUTEInte	erstate 64	DESC	KIPTIÖ	·	PIR 1	172, Item 22 - FAP 799 (I	wick Drive)	LC	JGGE	ם BY	RDG	(SC
SECTION	1BR-1-1		LOCA	TION		38584526, -90.16053543 Ide , Longitude	324, <b>SEC.</b> 13,	TWP. 2	2N, <b>F</b>	NG. 1	DW,	
COUNTY Sain	t Clair DRIL	LING M	ETHOD			ME 550 w/HSA	HAMMER T	YPE		Auto	matic	
STRUCT. NO.	082-6003(E), 082-0374(P) +59 to 75+18.50	_	: L	U C S	М	Surface Water Elev. Stream Bed Elev.	N/A N/A	ft ft	D E P	B L O	U C S	М О
BORING NO. Station Offset	74+00 31.0 ft LT	-   1 - -	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	N/A N/A	ft ft	Н	W S	Qu	I S T
Ground Surface Ele	<u> </u>	_ ft (1	ft) (/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	( ft)	(/6")	(tsf)	(%
3" TOPSOIL FILL: Brown and gra	ay, clay, trace	426.8				FILL: Black, cinders, A (continued)			_			
sand and cinders, A-	6	_	2 4 4	>4.5 P	24	Rough drilling at 20.5 fl		405.0	_	2 2 2	N/C 1.5	30
		424.0				nodules, A-7					_P_/	02
FILL: Brown, sandy  FILL: Brown, sand,  FILL: Brown, clay, w	Α-3	422.5 422.0	2 3 -5 3	0.5 P	38	Trace iron stains			-25	1 2 2	0.8 P	28
trace sand, A-6	un cinders,					SILTY LOAM: Gray, A	<u>-</u> 4	401.5				
FILL: Brown, sand,		420.5	2 5 5	N/C						ST	0.5 S/10.6	33
		_	4 5	N/C						2	0.9 S/15	28
			10 6						-30	7	N/C	_29
Trace clay lumps		_	3 4 6	N/C		CLAY: Gray, A-7		395.0				
									_			
		_	4 6 15 8	N/C					-35	WOH 1 2	0.9 B/20	49
		_										
		_	7 9 12	N/C		SAND: Brown, A-3		3 <u>90.0</u>				
									_			
FILL: Black, cinders		407.5	6 7 20 4	N/C N/C	36				-40	6 8 10	N/C	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{2}$  of  $\underline{5}$ 

**Date** 5/26-6/2-2016

	SCI Engineering, Inc.											2 2010	
ROUTE	Interstate 64	DE	SCRI	PTION		PTB 1	72, Item 22 - FAP 799	(MLK Drive)	LOGG	LOGGED BY BDG (SCI)			
SECTION	1BR-1-1				8.6288584526, -90.1605354324, <b>SEC.</b> 13, <b>TW</b> Latitude , Longitude			<b>2.</b> 2N, <b>RNG.</b> 10W,					
COUNTY	Saint Clair DRIL				CME 550 w/HSA HAMMER TYP			Automatic					
STRUCT. NO. Station	082-6003(E), 082-0374(P) 73+59 to 75+18.50	-	D E P	B L O	U C S	M O I	Surface Water Elev. Stream Bed Elev.	N/A ft n/A	D E P	B L O	U C S	M O I	
BORING NO. Station Offset	BB-306 74+00 31.0 ft LT	-	H	W S	Qu	S T	Groundwater Elev.: First Encounter Upon Completion	N/A ft N/A ft	T H	w s	Qu	S T	
Ground Surfa	ace Elev. 427.0	ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A ft	( ft)	(/6")	(tsf)	(%)	
SAND: Brown Began mud ro	i, A-3 (continued) tary at 40.0 ft.						SAND: Brown, A-3 (c	continued)					
			-45	10 12 15	N/C				-65	10 19 21	N/C		
				E			SAND: Brown and gr	ay, A-1	9.5	6			
			-50	5 7 12	N/C				-70	6 11 12	N/C		
				10	N/C		Trace gravel		_	8 8	N/C		
Becomes brow	vnish-gray		-55	12					-75	-			
			-60	4 6 10	N/C				-80	12 10 7	N/C		



USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED - JMH	REVISED

SOIL BORING - 6	F.A.P. RTE.	SECTION
S.N. 082-W315 RETAINING WALL AT MLK BRIDGE APPROACH	799	1BR-1-
SHEET NO. W14 OF W16 SHEETS		ILL



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**Date** 5/26-6/2-2016

ROUTE				PTION		PIB 1	172, Item 22 - FAP 799 (	MLK Drive) I	.oggi	ED BY	BDG	(SC
SECTION	1BR-1-1		_ ı	OCAT	ION _	38.628	38584526, -90.1605354; de , Longitude	324, <b>SEC.</b> 13, <b>TWP</b>	3, <b>TWP.</b> 2N, <b>RNG.</b> 10			
COUNTY	Saint Clair DF	RILLING	ME	THOD			ME 550 w/HSA	HAMMER TYPE		Auto	matic	
CTDUCT NO	082-6003(E),		D	В	U	м	C	N1/A #4	D	В	U	М
STRUCT. NO. Station	082-0374(P) 73+59 to 75+18.50	<u> </u>	E	L	c	0	Surface Water Elev. Stream Bed Elev.	N/A ft	E	L	C	Ö
Station	70100 10 701 10.01		P	ō	s	ī	Stream bed Elev.	N/A IL	P	ō	s	ī
BORING NO.	BB-306		Т	W		S	Groundwater Elev.:		Т	W		s
Station			Н	S	Qu	Т	First Encounter	N/A ft	Н	S	Qu	T
Offset	31.0 ft LT						Upon Completion	N/A ft				
Ground Surface	ce Elev. 427.0	ft	( ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A ft	( ft)	(/6")	(tsf)	(%
SAND: Gray, A	A-3 (continued)			-			SAND: Gray, A-1 (cor Boring advance with ca 100.0 ft.	ntinued) asing at				
			_	10 14	N/C				_	29 38	N/C	
			-85	16					-105	50/4"		
			_						_			
Rough drilling f	rom 87.0 ft88.5 ft.		_									
				-								
			_	15					_	23		
				14	N/C					44	N/C	
			-90	-			With gravel		-110			
			_						_			
				12	N/C					27	N/C	
CAND: Croy A		332.5	-	50/5"			No gravel			33	14/0	
SAND: Gray, A	<b>N-</b> I		-95						-115	37		
			_						_			
			_	-					_			
				1								
			-	-				309.	n —			
				1			WEATHERED LIMES					
			_	16			CLAY LOAM: Brown a		_	50/5"		
				22	N/C		with chert gravel, A-1				<u> </u>	
			-100	16					-120	1		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



# **SOIL BORING LOG**

Page  $\underline{4}$  of  $\underline{5}$ 

	Division of Highway SCI Engineering, Inc	/s C.								<b>Date</b> 5/26-6/
OUTE	Interstate 6	4	DESCR	IPTION	ı	PTB ²	172, Item 22 - FAP 799	(MLK Drive)	LOG	GED BY BDG
ECTION	1BR	-1-1		LOCAT	TION .	38.628 <b>Latitu</b>	38584526, -90.1605354 ide , <b>Longitude</b>	1324, <b>SEC.</b> 1	3, <b>TWP.</b> 2N	I, <b>RNG.</b> 10W,
OUNTY	Saint Clair	DRILL	ING ME	THOD			ME 550 w/HSA	_ HAMMER	TYPE _	Automatic
STRUCT. NO.	082-600 082-037 73+59 to 7	74(P)	D E	B L	U	M	Surface Water Elev.	N/A	ft	
Station	73+39 10 7	3+16.50	P	0	s	1	Stream Bed Elev.	N/A	ft	
SORING NO. Station			T H	W	Qu	S	Groundwater Elev.: First Encounter	N/A	ft	
Offset			''			-	Upon Completion	N/A	= It	
Ground Surfa		427.0	ft (ft)	(/6")	(tsf)	(%)	After N/A Hrs.	N/A	ft	
WEATHERED	D LIMESTONE Brown and gr vel, A-1 (contin	ay,	_							
			_	-						
				1						
				30	N/C					
			125	41 50/5"	14/0					
			-123	30/3			-			
Darabala aant	inued with rock	,	_	50/4"			-			
coring.	inuea with rock			50/1"						
-										
			_	-						
				1						
			-130							
			_	-						
				-						
			_							
				-						
			_	-						
				1						
			-135	_						
			_	-						
				-						
				-						
			-140		1		II			



USER NAME =	DESIGNED - YSS	REVISED
	CHECKED - ZJB	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 02/06/2018	CHECKED - JMH	REVISED

S.N. 082–W315		BORIN WALL		-	BRIDGE	APPROACH	
	SULLET NO	WILOE	W1C :	CUEETS			



#### **ROCK CORE LOG**

Page  $\underline{5}$  of  $\underline{5}$ 

Date 5/26-6/2-2016

•										
ROUTE	Interstate 64	DESCRIPTION	PTB 172, Item 2	2 - FAP 799	(MLK Drive	e)	_ LO	GGED	BY BD	G (SCI)
SECTION	1BR-1-1	LOCATION	38.6288584526, <b>Latitude</b> , <b>Longi</b>		324, <b>SEC.</b>	13, <b>T</b>	<b>WP.</b> 2	N, <b>RN</b>	<b>G.</b> 10W,	
COUNTY	Saint Clair COR	ING METHOD	Latitude , Longi	lude			R		CORE	s
	082-6003(E).						E	R		Т
STRUCT. NO.	082-003(E), 082-0374(P)	CORING BARRE	L TYPE & SIZE	NX Wirelin	ie 📮	_	С	:	T	R
Station	73+59 to 75+18.50	- - <b>-</b>	-		DE	C	O V	Q	I M	E N
DODING NO	DD 206	Core Diameter Top of Rock El	<u>2</u> 300.3	in ft	P	R	Ē	D	E	G
BORING NO Station	BB-306 74+00	Begin Core Ele		ft	T	E	R			Т
Offset	31.0 ft LT	_			H		Υ			Н
Ground Surface	<b>e Elev.</b> 427.0	ft			( ft)	(#)	(%)	(%)	(min/ft)	(tsf)
WEATHERED (continued)	LIMESTONE w/ CLAY	LOAM: Brown and g	gray, with chert gra	vel, A-1 3	00.28	1	100	73	2.22	
	ight gray, hard, very fir	nely crystalline, thin to	medium bedded. r	noderately	_					820.6
weathered, with	interbedded clay sean		,	,						020.0
Becomes thinly	bedded									
					-130					
Becomes fractu	red				-130					
Becomes thinly				_2	96.05	2	100	89	1.3	
	Gray, hard, very fine gi	rained, thickly bedded	, slightly weathered	d,						
calcareous										
					_					
										228.6
										220.0
					-135					
					_					
					89.50					225.4
LIMESTONE: L weathered	ight gray, moderately h	nard, very finely crystal	lline, thickly bedded	d, slightly						
wednered					_					
Recomes hande	ed, with 0.3" clay seam	1			-140					211.5
L	. <u> </u>				86.55	3	100	67	1	
1.3" interbedded	Gray, hard, very fine gi	rained, banded to thin	iy bedded, siightiy	weathered		-				
	, becomes medium be	hdded			_					
,				2	84.68					
Boring terminate	ea at 142.3 ft.									
Boring grouted	to 142.3 ft.				_					
					-145					

Color pictures of the cores Yes

Cores will be stored for examination until

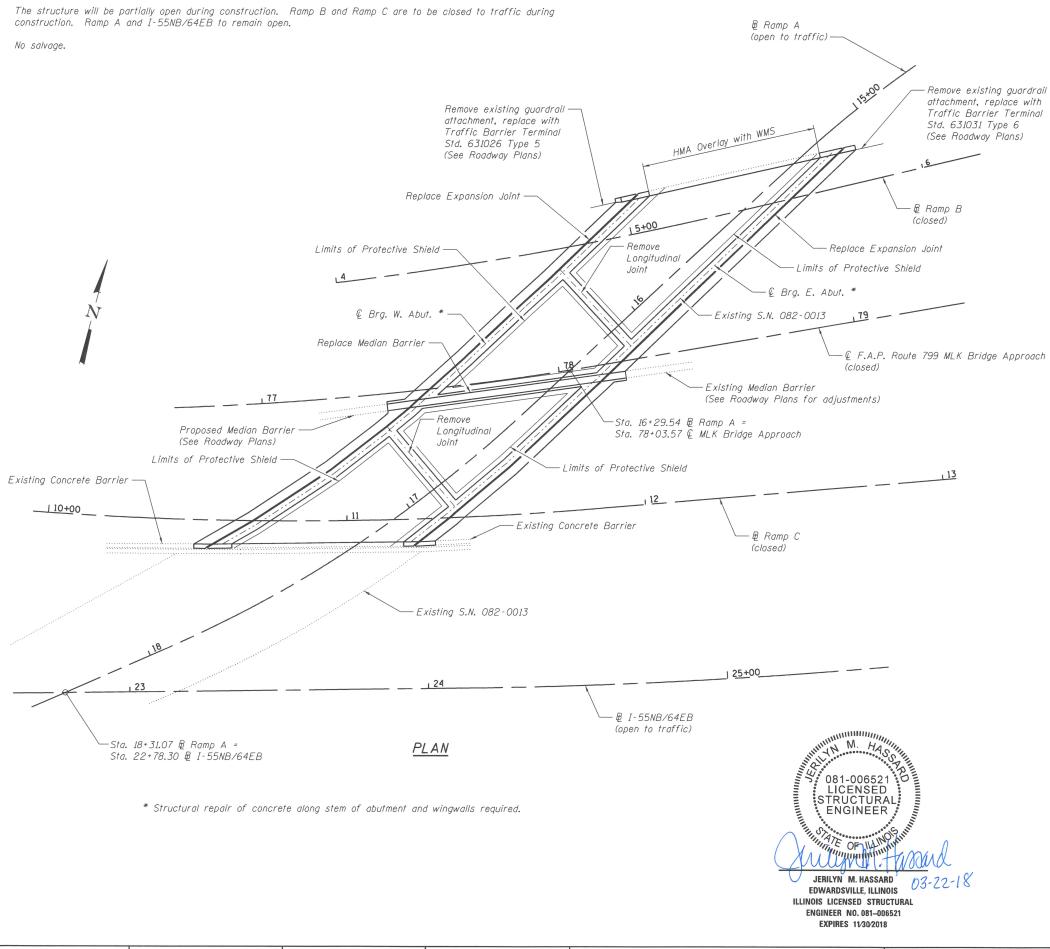
The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938)

BBS, form 138 (Rev. 8-99)



USER NAME =	DESIGNED - YSS	REVISED	
	CHECKED - ZJB	REVISED	
PLOT SCALE =	DRAWN - AEC	REVISED	
PLOT DATE = 02/06/2018	CHECKED - JMH	REVISED	

SOIL BORING - 8	F.A.P. RTE.	SEC
S.N. 082–W315 RETAINING WALL AT MLK BRIDGE APPROACH	799	1B
CUEFT NO WAS OF WAS CUEFTS	_	



#### GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Existing reinforcement shall be cleaned and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than  $50^{\circ}F$ .

Surface preparation at the construction joints shall be performed using high-pressurized water spray, using equipment capable of producing a minimum water pressure of 5000 psi.

The Contractor shall install a protective shield to protect motorists and pedestrians on Ramp A from construction debris. Installation and removal of the protective shield shall occur during one of the interstate weekend closures.

Existing Structure Plans are available for review in the District office. Contact Herve Gelin at (618) 346-3179.

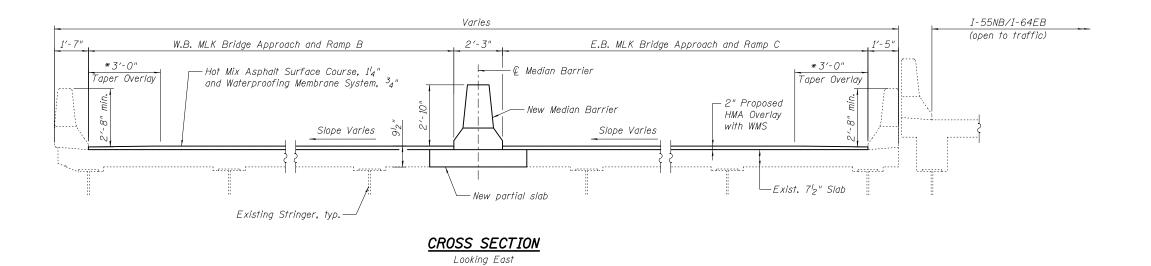
The deck surface shall have its final finish tined according to Article 420.09(e)(1) of the Standard Specifications. Cost included with Concrete Superstructure.

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Polymerized Bituminous Materials (Tack Coat)	Pound	146	-	146
Polymerized Hot-Mix Asphalt Surface Course,	Ton	46	-	46
Mix "E", N90				
Concrete Removal	Cu. Yd.	111.4	-	111.4
Protective Shield	Sq. Yd.	379	-	379
Concrete Superstructure	Cu. Yd.	126.0	-	126.0
Protective Coat	Sq. Yd.	300	-	300
Reinforcement Bars, Epoxy Coated	Pound	16,240	-	16,240
Preformed Joint Strip Seal	Foot	378.7	-	378.7
Waterproofing Membrane System	Sq. Yd.	645	-	645
Epoxy Crack Injection	Foot	-	200	200
Structural Repair of Concrete	Sq. Ft.	-	950	950
(Depth Equal to or Less than 5 Inches)				
Structural Repair of Concrete	Sq. Ft.	-	70	70
(Depth Greater than 5 Inches)				
Longitudinal Joint Sealant	Foot	330	-	330

MODJESKIMASTERS
Experience great bridges.

USER NAME =	DESIGNED - CDB	REVISED	
	CHECKED - RLM	REVISED	
PLOT SCALE =	DRAWN - AEC	REVISED	
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED	



* Taper HMA Overlay to  $\mathcal{I}_2^{l}$ " if required to maintain minimum barrier height.

#### BILL OF MATERIAL

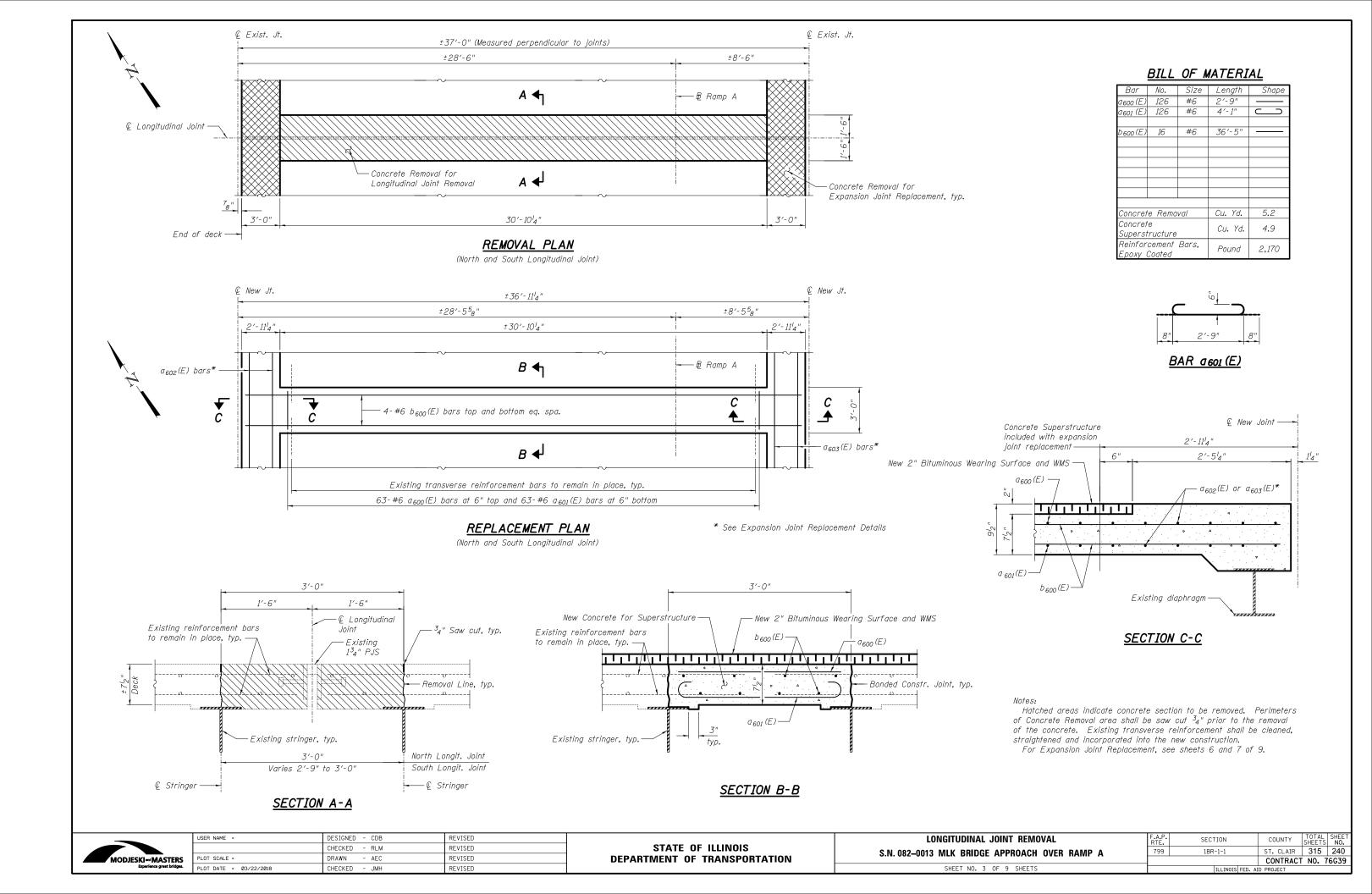
DILL OF MITTERIAL	=	
ITEM	UNIT	TOTAL
Polymerized Bituminous Materials (Tack Coat)	Pound	146
Polymerized Hot-Mix Asphalt Surface Course,	Ton	46
Mix "E", N90		
Waterproofing Membrane System	Sq. Yd.	645
Longitudinal Joint Sealant	Foot	330

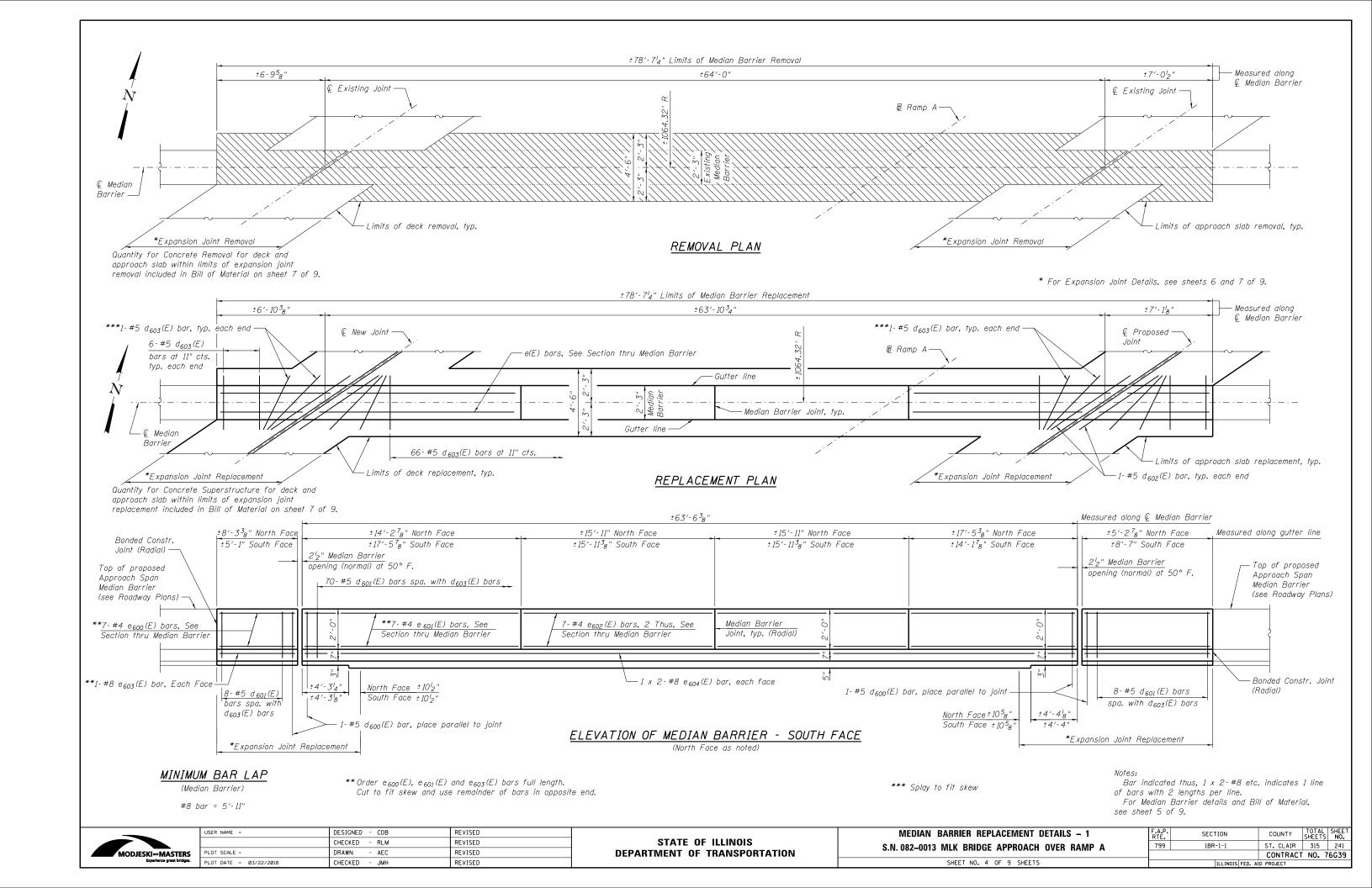
#### Notes:

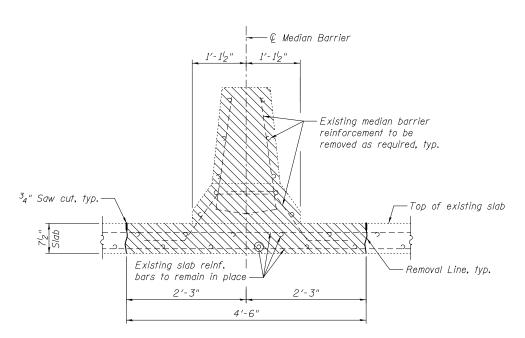
For median barrier replacement details, see sheet 4 and 5 of 9. Longitudinal Joint Sealant and Tack Coat shall be placed on top of the  $^{\prime}_2$ " thick Asphalt Sand Seal Protection Layer of the WMS.

MODJESKI and MASTERS Experience great bridges.	

	USER NAME =	DESIGNED - CDB	REVISED		HMA OVERLAY	F.A.P.	SECTION	COUNTY SHEET
		CHECKED - RLM	REVISED	STATE OF ILLINOIS	S.N. 082-0013 MLK BRIDGE APPROACH OVER RAMP A	799	1BR-1-1	ST. CLAIR 315 239
ERS	PLOT SCALE =	DRAWN - AEC	REVISED	DEPARTMENT OF TRANSPORTATION	3.IN. U02-UUTS IVILK DRIDGE AFFRUAGR OVER RAIVIF A			CONTRACT NO. 76G39
ridges.	PLOT DATE = Ø3/22/2018	CHECKED - JMH	REVISED		SHEET NO. 2 OF 9 SHEETS		ILL INOIS FED.	AID PROJECT



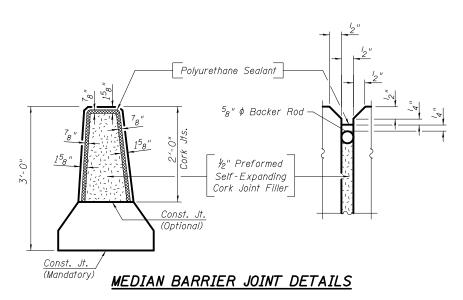




#### SECTION THRU MEDIAN BARRIER SHOWING REMOVAL

Note:

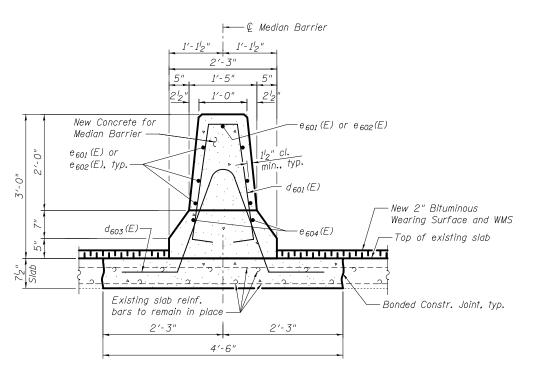
Hatched areas indicate concrete section to be removed. Perimeters of Concrete Removal area shall be saw cut 3₄" prior to the removal of the concrete. Existing slab reinforcement shall be cleaned, straightened and incorporated into the new construction.



The  $^{l}_{8}$ " Aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.

The Polyurethane Sealant shall be according to Article 1050.04 of the Std. Spec. and color shall be gray. Cost included with Concrete Superstructure.

The '2" Preformed Self-Expanding Cork Joint Filler shall be according to Article 1051.07 of the Std. Spec. Cost included with Concrete Superstructure.



#### SECTION THRU MEDIAN BARRIER SHOWING REPLACEMENT AT WEARING SURFACE

├── @ Median Barrier

 $e_{600}(E)$  or  $e_{601}(E)$ 

d₆₀₀(E) or d₆₀₁(E)

e₆₀₃(E) or e₆₀₄(E)

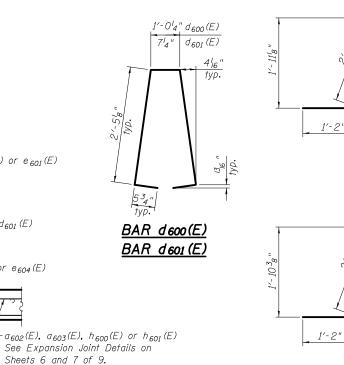
S

1'-5" 1'-0"

SECTION THRU MEDIAN BARRIER

SHOWING REPLACEMENT

AT EXPANSION JOINT



#### BAR <u>d 603(E)</u>

E) E) / <u> 6"</u> //p.	4'4" 10" Rad.  1'-2"  1'-2"	1'-11'8"
13,6 ", typ.	BAR d 602(E)  2'2" Rad. 6"	
	1,-103,"	1'-1038"

MEDIAN BARRIER

BILL OF MATERIAL

#5

#4

#4

#8

d 600 (E)

d 602(E)

<u>d 603(E)</u>

e 600 (E) e 601 (E)

e 602 (E)

e 603 (E)

e 604 (E)

Concrete Removal

Protective Coat Reinforcement Bars,

Epoxy Coated

Concrete Superstructure

d₆₀₁ (E) 86

4

Size | Lenath

#5 6'-5"

#5 7'-4"

#4 | 13'-2"

#8 13'-2"

34'-7"

Cu. Yd.

Cu. Yd.

Sq. Yd.

Pound

19.8

61

2,050

Note:

Median barrier concrete shall be paid for as Concrete Superstructure.

PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
	CHECKED - RLM	REVISED
USER NAME =	DESIGNED - CDB	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

Existing slab reinf.

bars to remain in place

New Concrete for

Median Barrier

e₆₀₀(E) or

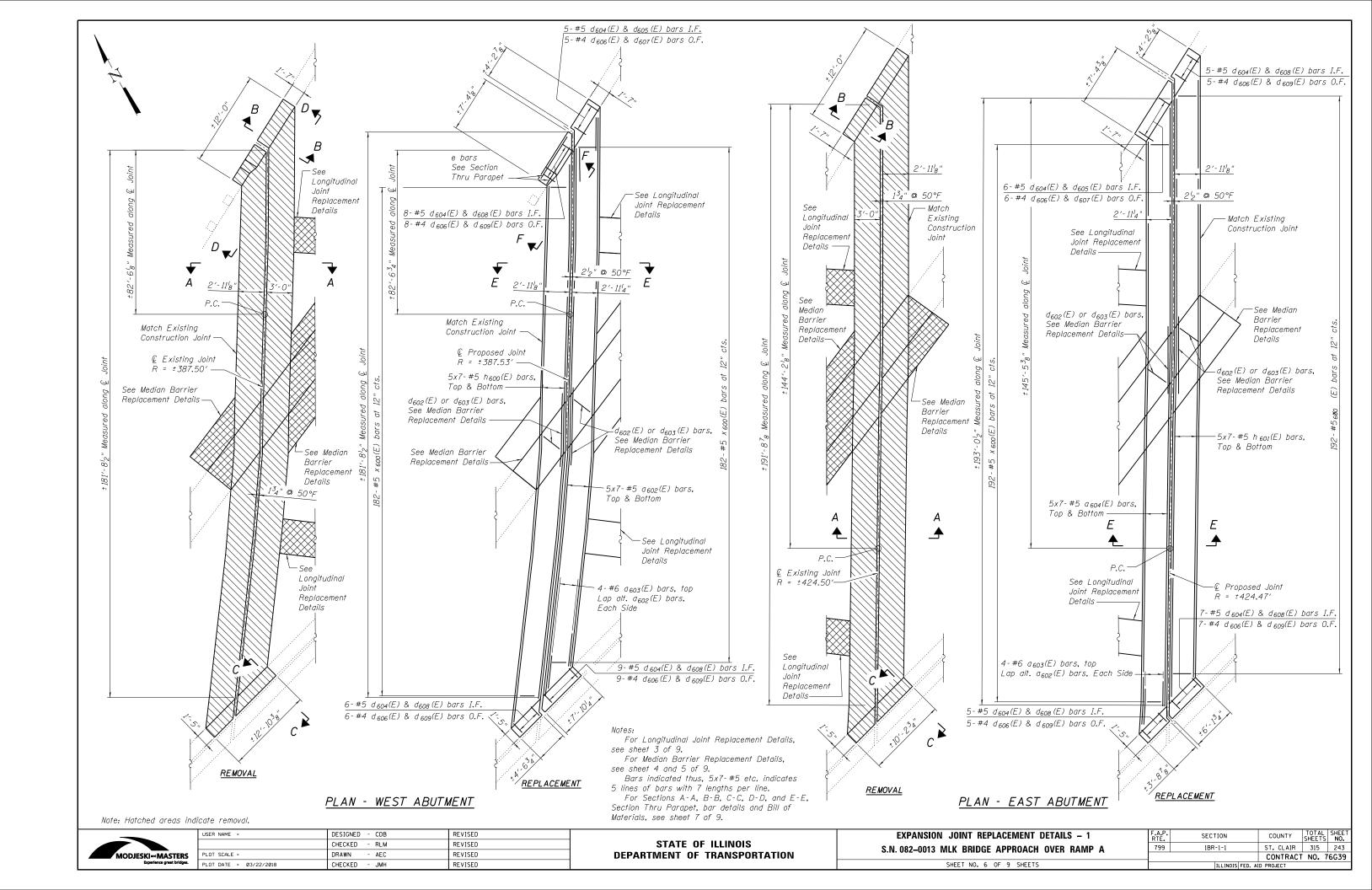
e₆₀₁ (E), typ.

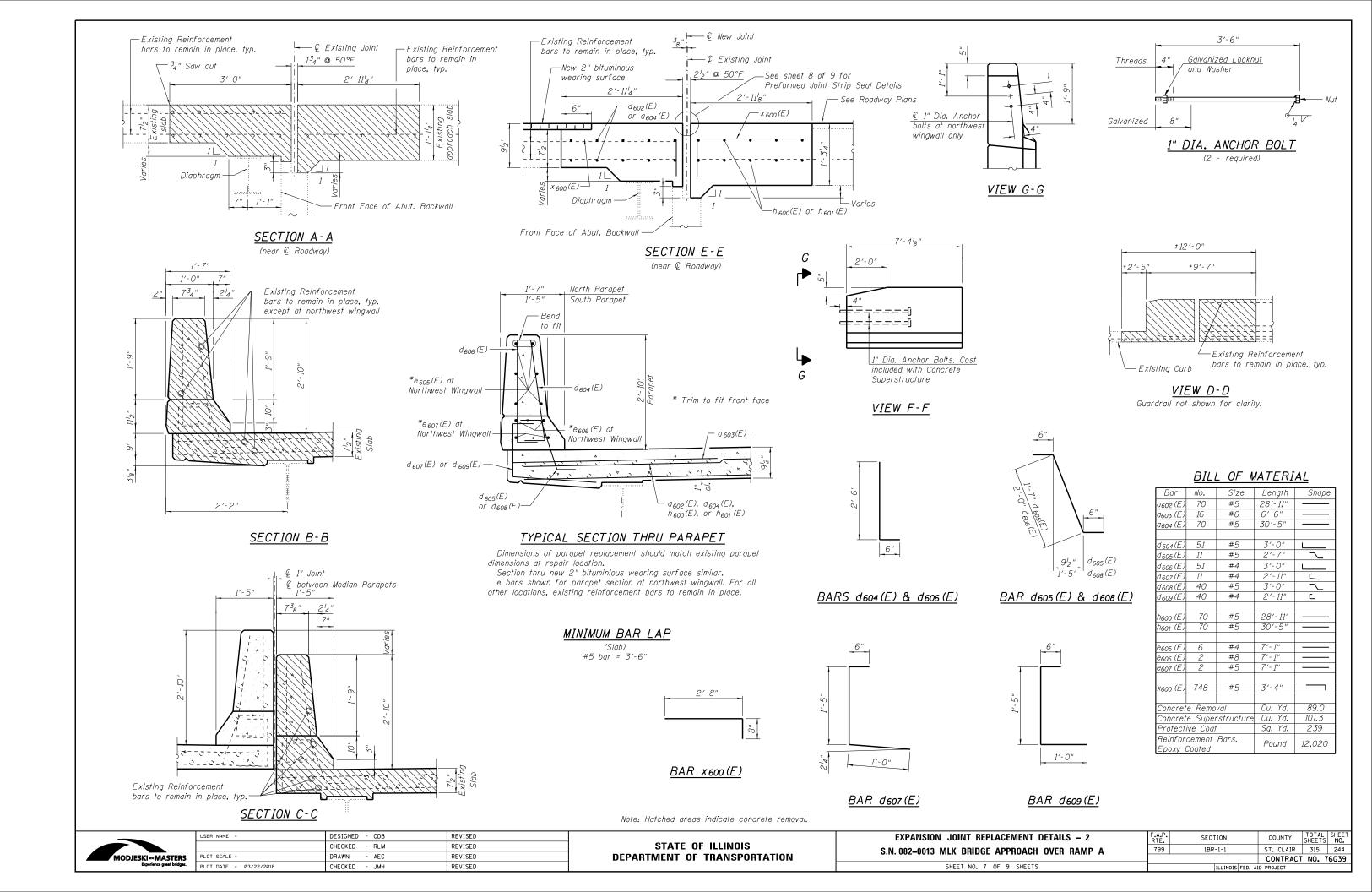
d₆₀₂(E) or d₆₀₃(E)-

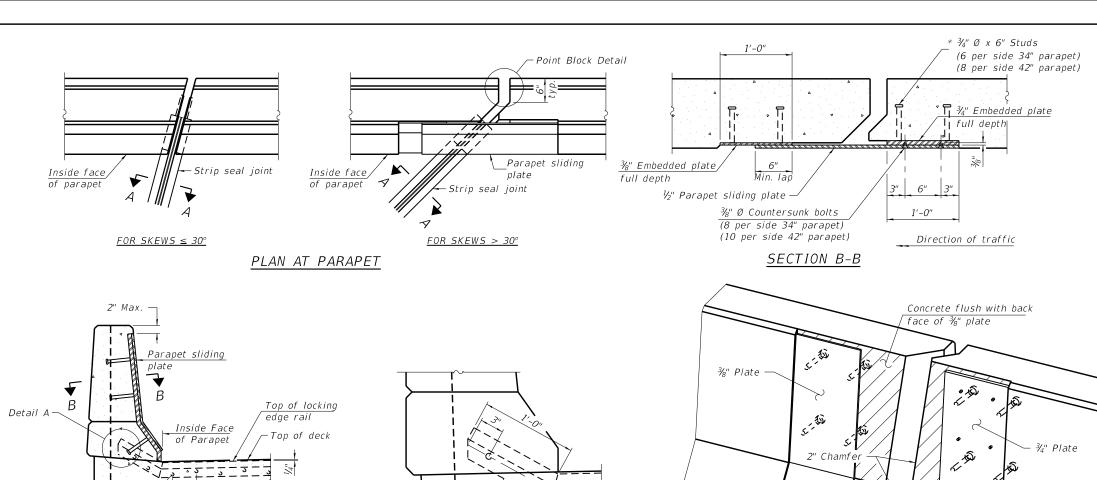
MEDIA	N BARRII	ER	REP	LACEME	NT - 2	2	
S.N. 082–0013	MLK BRID	GE	API	PROACH	OVER	RAMP	Α
	SHEET NO.	5	OF S	SHEETS			

	TI I INOT	S EED A	D PROJECT	NO. I	6633
			CONTRACT	NO 7	CCZC
799	1BR-1-1		ST. CLAIR	315	242
F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.

1'-2"



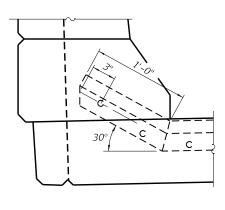




#### ELEVATION AT PARAPET

5/8" Ø x 6" Studs

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



DETAIL A

# D. 20 Concrete flush with back face of ¾" plate

TRIMETRIC VIEW (Showing embedded plates only)

15/8"

at 50° F

—Strip seal

### Locking edge rail-15/8" at 50° F Top of concrete -Strip seal at 50° F

SHOWING ROLLED RAIL JOINT

DESIGNED - CDB

CHECKED - RLM

CHECKED - JMH

- AEC

DRAWN

#### SECTION A-A

 $\frac{3}{6}$ "  $\phi$  threaded rods in  $\frac{1}{16}$ "  $\phi$  holes at  $\pm 4'-0$ " cts.

for holding the proper joint opening based on

the temperature during the deck pour. Place to

miss studs. All rods shall be burned, or sawed

off flush with the plates after concrete is set.

*  $\frac{1}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs)

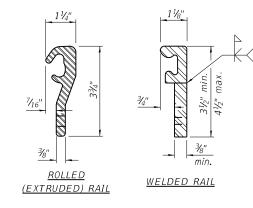
* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

REVISED

REVISED

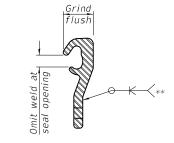
REVISED

REVISED



#### LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip

are not permitted. The gland shall be sized for a maximum

The locking edge rails depicted are configured for typical

applications and are conceptual only. The actual configuration

of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails,

however, will not be allowed. Locking edge rails may exceed the

4½" maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. The Maximum space between locking edge rail segments

shall be  $\frac{3}{6}$ " and sealed with a suitable sealant; however, any

rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge

The manufacturer's recommended installation methods

Cost of parapet sliding plates, embedded plates, and

anchorage studs included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based

on the locking edge rail chosen by the Contractor. Deck and

parapet lengths shown elsewhere in the plans are dimensioned

on the rolled locking edge rail. If the Contractor elects to use

to the concrete opening, not the joint opening, and are based

a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the

length of the bridge approach slab.

according to the manufacturer's recommendation.

seal shall match the configuration of the locking edge

rated movement of 4 inches.

shall be followed.

rail splice detail.

rails. Open or "webbed" strip seal gland configurations

#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

#### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	378.7

STATE OF ILLINOIS	
PARTMENT OF TRANSPORTATION	

PR	EFORI	MED	J0	INT	S	TRIP S	EAL			
S.N. 082-0013	MLK	BRID	GE	ΑF	PI	ROACH	OVER	RAMP	Α	
	SHEE	T NO.	8	OF	9	SHEETS				

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
799	1BR-1-1	ST. CLAIR	315	245
		CONTRACT	NO. 7	6G39
	TILLINOIS FED. A	D PROJECT		

EJ-SS

8-11-17

PLOT DATE = 03/22/2018

USER NAME =

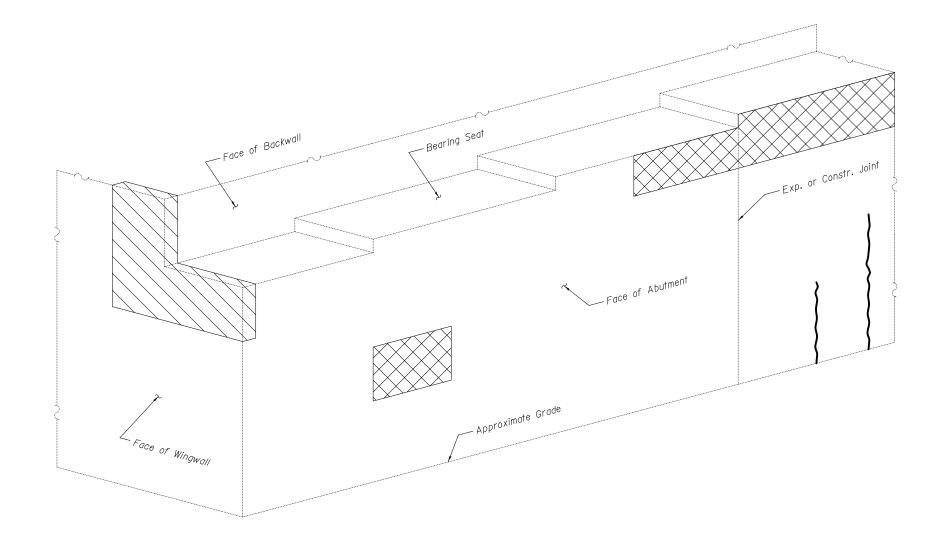
MODJESKI MASTERS

DEI

SHOWING WELDED RAIL JOINT

Locking edge rail-

Top of concrete



#### PARTIAL ELEVATION OF WINGWALL AND ABUTMENT

(General View)

#### <u>LEGEND</u>



Epoxy Crack Injection



Structural Repair of Concrete < 5 inches



Structural Repair of Concrete > 5 inches

#### Note:

Repairs shown in the Partial Elevation of Wingwall and Abutment provide a general representation of the repairs that will be required to the exposed surfaces of the East and West Abutments and wingwalls. The repairs shown do not represent actual repair locations and extents of repair. Actual locations to be repaired and the corresponding repair limits will be determined by the Engineer at the time of construction. Quantities provided in the Bill of Material are estimated.

#### BILL OF MATERIAL

ITEM	UNIT	TOTAL
Epoxy Crack Injection	Foot	200
Structural Repair of Concrete (Depth Equal to or less than 5 inches)	Sq. Ft.	950
Structural Repair of Concrete (Depth Greater than 5 inches)	Sq. Ft.	70



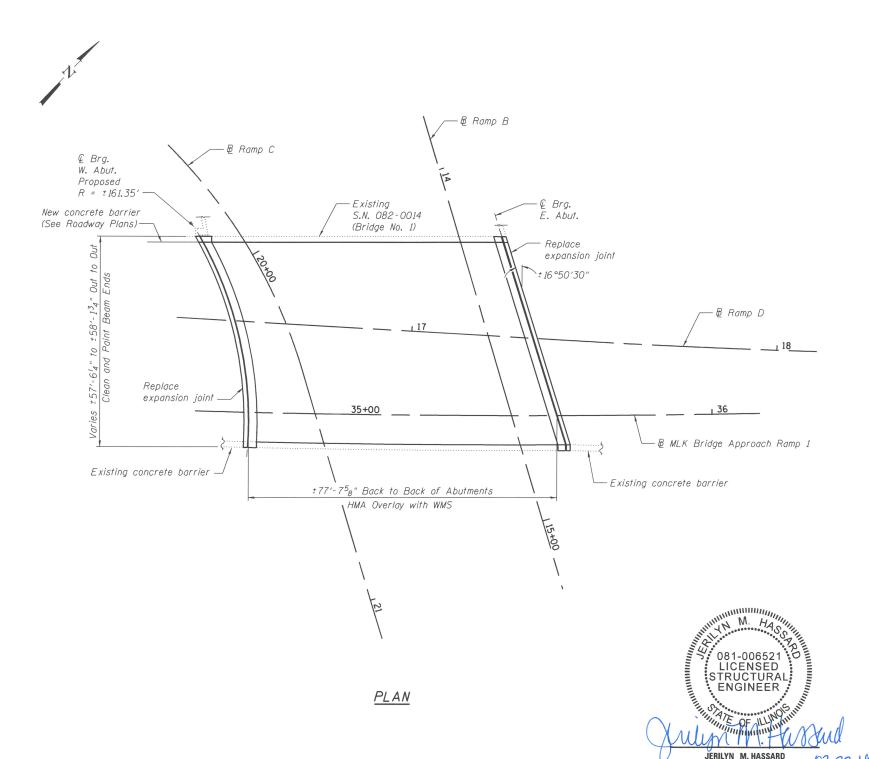
USER NAME =	DESIGNED - CDB	REVISED
	CHECKED - RLM	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WINGWALL AND ABUTMENT REPAIRS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-0013 MLK BRIDGE APPROACH OVER RAMP A	799	1BR-1-1	ST. CLAIR	315	246
			CONTRACT	NO. 7	6G39
SHEET NO. 9 OF 9 SHEETS		TILITNOIS EED AT	D PROJECT		

Ramp D is to remain open to traffic during construction. Ramp 1, Ramp B, and Ramp C are to be closed to traffic during construction. Stage construction will be utilized to perform the repairs on the structure.

No salvage



#### GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Existing reinforcement shall be cleaned and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than  $50\,^\circ\text{F}$ .

Surface preparation at the construction joints shall be performed using high-pressurized water spray, using equipment capable of producing a minimum water pressure of 5000 psi.

Cleaning and painting of the existing structural steel shall be as specified in the special provision for Cleaning and Painting Existing Steel Structures. Areas to be cleaned and painted shall consist of all beam ends, end diaphragms and steel components of the steel bearings at the abutments. Beam end painting shall extend 5 feet from centerline bearing of beams longitudinally. This surface preparations shall be accomplished according to the requirements of Near-White Metal Blast Cleaning SSPC-SP10. The paint system shall be applied as specified for System I OZ/E/U. The color of the final finish coat shall be Grey, Munsell No. 5B 7/1.

Complete cost of painting the locations described on this sheet shall be considered included with the pay item Cleaning and Painting Steel Bridge No. 1.

Care shall be taken not to damage rubber bearing or joint components during the blasting and cleaning operations. Any damage to these components shall be repaired at the Contractor's expense.

Existing Structure Plans are available for review in the District office. Contact Herve Gelin at (618) 346-3179.

The deck surface shall have its final finish tined according to Article 420.09(e)(1) of the Standard Specifications. Cost included with Concrete Superstructure.

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Polymerized Bituminous Materials (Tack Coat)	Pound	103	-	103
Polymerized Hot-Mix Asphalt Surface Course,	Ton	32	-	32
Mix "E", N90				
Concrete Removal	Cu. Yd.	16.4	-	16.4
Concrete Superstructure	Cu. Yd.	18.6	-	18.6
Protective Coat	Sq. Yd.	72	-	72
Reinforcement Bars, Epoxy Coated	Pound	2,680	-	2,680
Bar Splicers	Each	18	-	18
Preformed Joint Strip Seal	Foot	125.0	-	125.0
Waterproofing Membrane System	Sq. Yd.	459	-	459
Cleaning and Painting Steel Bridge No. 1	L. Sum	1	-	1
Longitudinal Joint Sealant	Foot	298	-	298
Containment and Disposal of Non-Lead Paint Cleaning Residues No. 1	L. Sum	1	-	1

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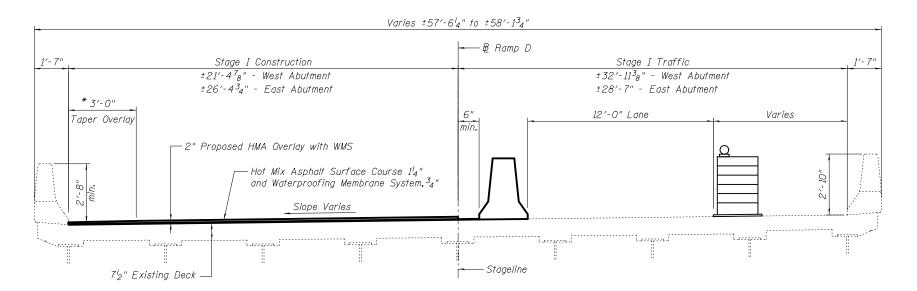
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	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - AEC	REVISED	
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

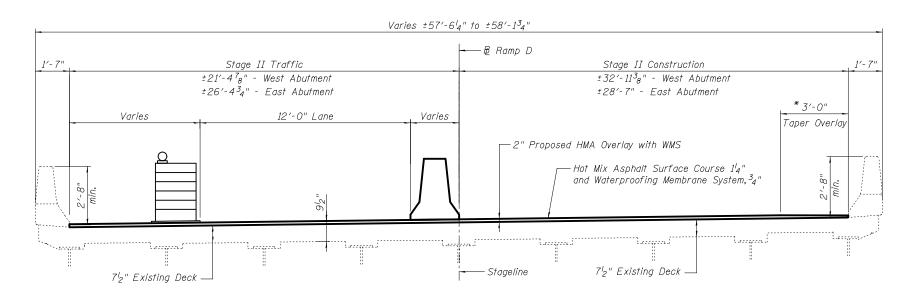
EDWARDSVILLE, ILLINOIS
ILLINOIS LICENSED STRUCTURAL
ENGINEER NO. 081–006521
EXPIRES 11/30/2018

GENERAL PLAN, GENERAL NOTES, AND TOTAL BILL OF MATERIAL
S.N. 082-0014 MLK BRIDGE APPROACH AND RAMP D
OVER RAMP B AND RAMP C
SHEET NO 1 OF 7 SHEETS

	F.A.P. RTE.	SECTION				CC	UNTY	TOTAL SHEETS	SHEE.	
	799	99 1BR-1-1					CLAIR	315	247	
4						CO	NTRACT	NO. 7	6G39	
			ILLINOIS	FED.	AID	PRO.	JECT			

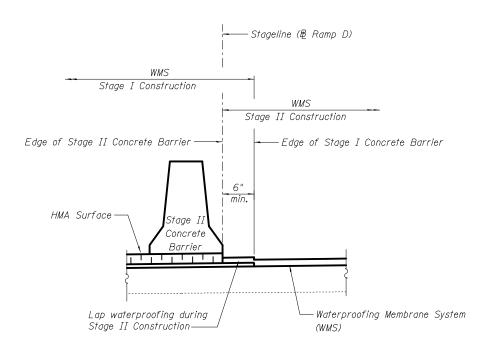


### STAGE I TYPICAL SECTION Looking Upstation



#### STAGE II TYPICAL SECTION

Looking Upstation



#### WATERPROOFING TREATMENT AT STAGE CONSTRUCTION LINE

#### BILL OF MATERIAL

<u> </u>	=	
ITEM	UNIT	TOTAL
Polymerized Bituminous Materials (Tack Coat)	Pound	103
Polymerized Hot-Mix Asphalt Surface Course, Mix "E", N90	Ton	32
Waterproofing Membrane System	Sq. Yd.	459
Longitudinal Joint Sealant	Foot	298

Note:

Longitudinal Joint Sealant and Tack Coat shall be placed on top of the  $\frac{1}{2}$ " thick Asphalt Sand Seal Protection Layer of the WMS.



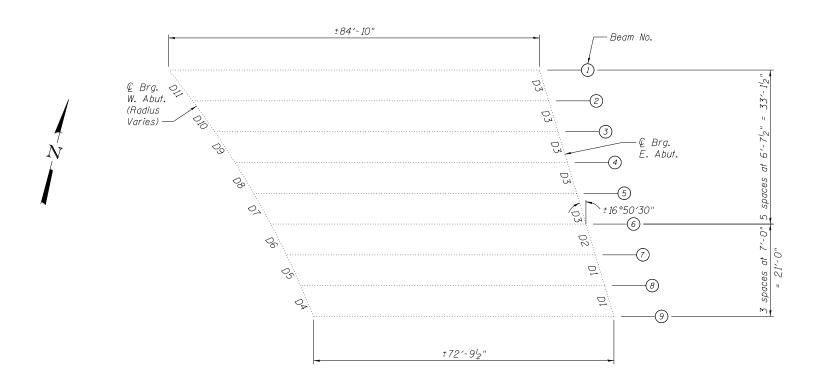
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	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

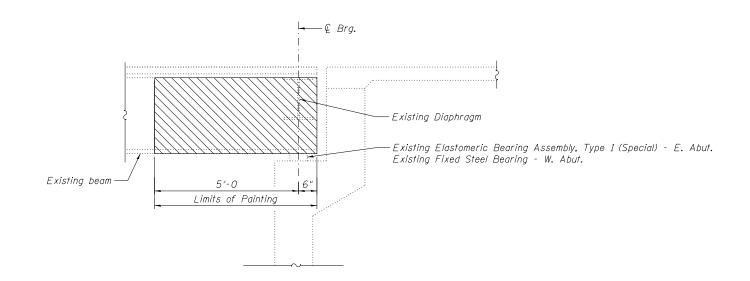
S.N. 082–0014 N OV	/ILK B		ĞĔ	Al	PPI		 RAMP	D	
	SHEET	NO.	2	0F	7	SHEETS			

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
799	1BR-1-1	ST. CLAIR	315	248
		CONTRACT	NO. 7	6G39
	TH INOIS EED AT	D PROJECT		

^{*} Taper HMA Overlay to  $1^l_2$ " if required to maintain minimum barrier height.



#### FRAMING PLAN



#### ELEVATION AT ABUTMENT

East Abutment shown, West Abutment Similar.

#### <u>DIAPHRAGM</u> SCHEDULE

#### <u>BEAM</u> SCHEDULE

DIAPHRAGM	MATERIAL
D1 thru D2	W12X40
D3	12WF27
D4 thru D6	W12X40
D7 thru D11	12WF27

BEAM	MATERIAL
1	36WF230
2	36WF194
3	36WF194
4	36WF194
5	36WF194
6	36WF194
7	W36X194
8	W36X194
9	W36X194

#### BILL OF MATERIAL

ITEM	UNIT	TOTAL
Cleaning and Painting Steel Bridge No. 1	L. Sum	1
Containment and Disposal of Non-Lead Paint Cleaning Residues No. 1	L. Sum	1



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	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - AEC	REVISED	
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED	

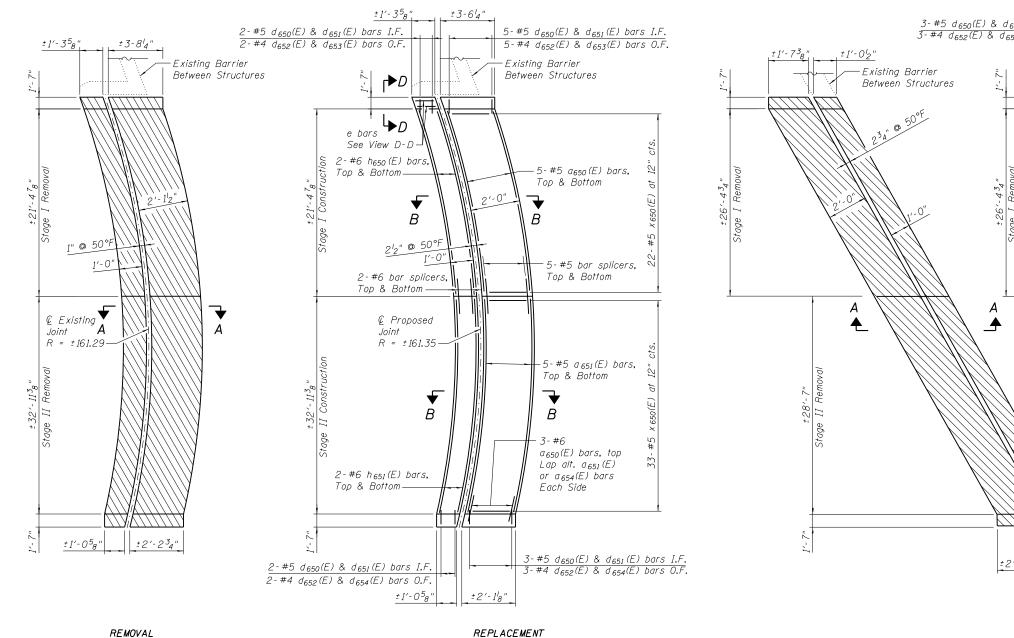
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PAINTING DETAILS
S.N. 082-0014 MLK BRIDGE APPROACH AND RAMP D
OVER RAMP B AND RAMP C

SHEET NO. 3 OF 7 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
799	1BR-1-1	ST. CLAIR	315	249
		CONTRACT	NO. 7	6G39
	ILLINOIS FED. A	ID PROJECT		





3-#5 d₆₅₀(E) & d₆₅₁(E) bars I.F. 3-#4 d₆₅₂(E) & d₆₅₃(E) bars O.F. 2-#5 d₆₅₀(E) & d₆₅₁(E) bars I.F. 2-#4 d₆₅₂(E) & d₆₅₃(E) bars O.F. Existing Barrier Between Structures e bars See View C-C 2-#6 h₆₅₂(E) bars, Top & Bottom 5-#5 a₆₅₂(E) bars, Top & Bottom 2-#6 bar splicers, 5-#5 bar splicers, Top & Bottom Top & Bottom 2-#6 h₆₅₃(E) bars, Top & Bottom 5-#5 a₆₅₃(E) bars, Top & Bottom -3-#6 a₆₅₄(E) bars, top Lap alt.  $a_{653}(E)$  or  $a_{651}(E)$  bars Each Side 4-#5 d₆₅₀(E) & d₆₅₁(E) bars I.F. 4-#4 d₆₅₂(E) & d₆₅₅(E) bars O.F. 2-#5 d₆₅₀(E) & d₆₅₁(E) bars I.F. 2-#4 d₆₅₂(E) & d₆₅₅(E) bars O.F. ±63₄"

REMOVAL

PLAN - WEST ABUTMENT

#### PLAN - EAST ABUTMENT

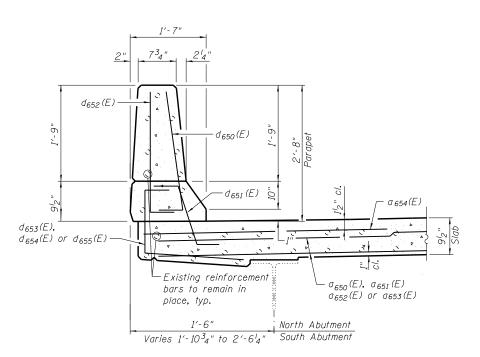
Note:

For Section A-A, B-B, View C-C, D-D, bar details and the Bill of Material, see sheet 5 of 7.

REPLACEMENT

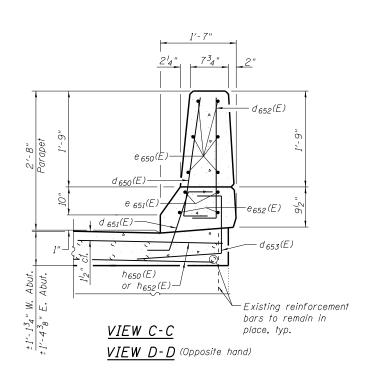
Note: Hatched areas indicate removal.

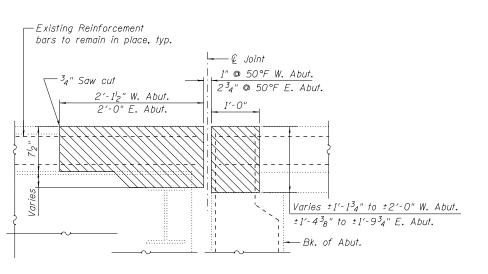
	USER NAME =	DESIGNED - CDB	REVISED		EXPANSION JOINT REPLACEMENT DETAILS – 1	F.A.P.	SECTION	COUNTY	TOTAL	SHEET NO.
		CHECKED - JMH	REVISED	STATE OF ILLINOIS	S.N. 082-0014 MLK BRIDGE APPROACH AND RAMP D	799	1BR-1-1	ST. CLAIR	315	250
MODJESKI ™ MASTERS	PLOT SCALE =	DRAWN - AEC	REVISED	DEPARTMENT OF TRANSPORTATION	OVER RAMP B AND RAMP C			CONTRAC	T NO.	76G39
Experience great bridges.	PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED		SHEET NO. 4 OF 7 SHEETS		ILLINOIS FED. A	D PROJECT		



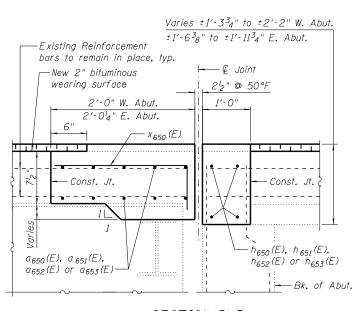
#### TYPICAL SECTION THRU PARAPET

Dimensions of parapet replacement should match existing parapet dimensions at repair location.

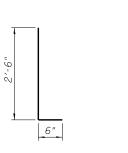


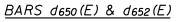


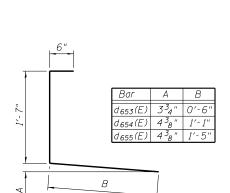
 $\frac{SECTION A - A}{(near \ \ \ \ Roadway)}$ 



SECTION B-B (near © Roadway)







BAR d653(E), d654(E) or d655(E)

#### BILL OF MATERIAL

Bar	No.	Size	Length	Shape
650 (E)	10	#5	25'-3"	
651 (E)	10	#5	38′-9"	
652 (E)	10	#5	28'-11"	
653 (E)	10	#5	31'-3"	
654 (E)	12	#5	6'-6"	
650(E)	23	#5	3'-0"	L
651 (E)	23	#5	2'-7" 3'-0" 2'-7"	
652(E)	23	#4	3'-0"	L
653(E)	12	#4	2'-7"	Γ Γ Γ
654 (E)	5	#4	3'-2"	J
655 (E)	6	#4	3′-6"	J
650 (E)	12	#4	0'-9"	
651 (E)	4	#8	0'-9"	
652 (E)	4	#5	0'-9"	
650 (E)	4	#6	25′-3"	
651 (E)	4	#6	38′-9"	
652 (E)	4	#6	28′-11"	
653 (E)	4	#6	31'-3"	
650 (E)	111	#5	2'-4"	
	e Remo		Cu. Yd.	16.4
		rstructure	Cu. Yd.	18.6
	ive Coat		Sq. Yd.	72
	cement	Bars,	Pound	2,680
роху (	Coated		, 00,10	2,000

Note: Hatched areas indicate removal.



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is.	PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED

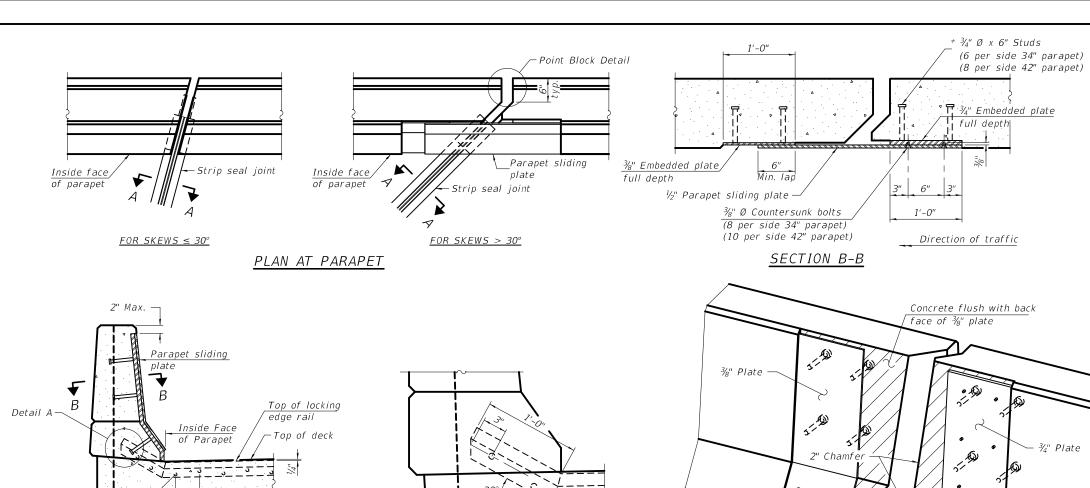
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR x 650 (E)

EXPANSION JOINT REPLACEMENT DETAILS – 2 S.N. 082-0014 MLK BRIDGE APPROACH AND RAMP D OVER RAMP B AND RAMP C						
OVER RAIMP B AND RAIMP C						

BAR d 651 (E)

A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.		
799	1BR-1-1	ST. CLAIR	315	251		
		CONTRACT	NO. 7	6G39		
	ILLINOIS FED. AID PROJECT					



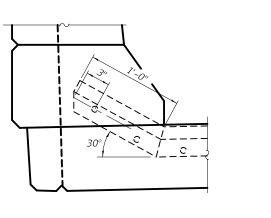
#### ELEVATION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

%" Ø x 6" Studs

EJ-SS

MODJESKI and MASTERS



DETAIL A

# 3€.7Ø) Concrete flush with back face of ¾" plate

TRIMETRIC VIEW (Showing embedded plates only)

### Locking edge railat 50° F Top of concrete Strip seal at 50° F

8-11-17

SHOWING ROLLED RAIL JOINT

#### Locking edge railat 50° F Top of concrete * $\frac{5}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs) $\frac{3}{6}$ " $\phi$ threaded rods in $\frac{7}{16}$ " $\phi$ holes at $\pm 4$ '-0" cts.

for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

#### SHOWING WELDED RAIL JOINT

## 7/16" ROLLED WELDED RAIL (EXTRUDED) RAIL

#### LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.

The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip

The locking edge rails depicted are configured for typical

applications and are conceptual only. The actual configuration

of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails,

however, will not be allowed. Locking edge rails may exceed the

 $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. The Maximum space between locking edge rail segments

shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any

rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge

The manufacturer's recommended installation methods

Cost of parapet sliding plates, embedded plates, and

a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the

length of the bridge approach slab.

anchorage studs included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based

on the locking edge rail chosen by the Contractor. Deck and

parapet lengths shown elsewhere in the plans are dimensioned

to the concrete opening, not the joint opening, and are based

on the rolled locking edge rail. If the Contractor elects to use

according to the manufacturer's recommendation.

seal shall match the configuration of the locking edge

rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum

rated movement of 4 inches.

shall be followed.

rail splice detail.

#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

#### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	125.0

#### SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

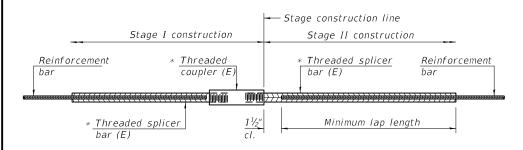
USER NAME = DESIGNED - CDB REVISED CHECKED - JMH REVISED PLOT SCALE = - AEC REVISED CHECKED - JMH LOT DATE = Ø3/22/2018 REVISED

STATE OF ILLINOIS

PREFORMED JOINT STRIP SEAL S.N. 082-0014 MLK BRIDGE APPROACH AND RAMP D OVER RAMP B AND RAMP C SHEET NO. 6 OF 7 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
799	799 1BR-1-1	ST. CLAIR	315	252
		CONTRACT	NO. 7	6G39
	TILINOIS FED. AT	ID PROJECT		

**DEPARTMENT OF TRANSPORTATION** 

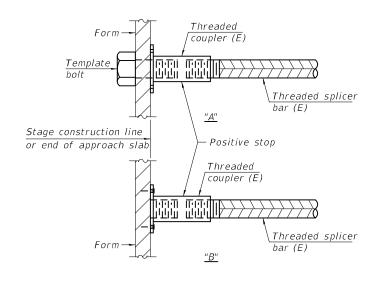


#### STANDARD BAR SPLICER ASSEMBLY

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

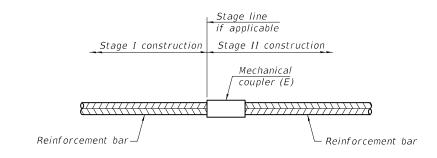
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

	0	No	Mimimo
Location	Bar	No. assemblies	Minimum
Location	size	required	lap length
W. Abut. Backwall	#6	4	4'-10"
Slab at W. Abut.	#5	10	3'-6"
Slab at E. Abut.	#5	10	3'-6"
E. Abut. Backwall	#6	4	4'-10"



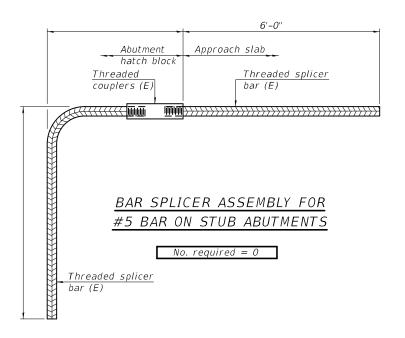
#### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
(E): Indicates epoxy coating.



#### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



#### NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

2-17-2017

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PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS S.N. 082-0014 MLK BRIDGE APPROACH AND RAMP DOVER RAMP B AND RAMP C

SHEET NO. 7 OF 7 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
799	1BR-1-1	ST. CLAIR	315	253
		CONTRACT	NO. 7	6G39
	TILLINOIS FED. A	ID PROJECT		

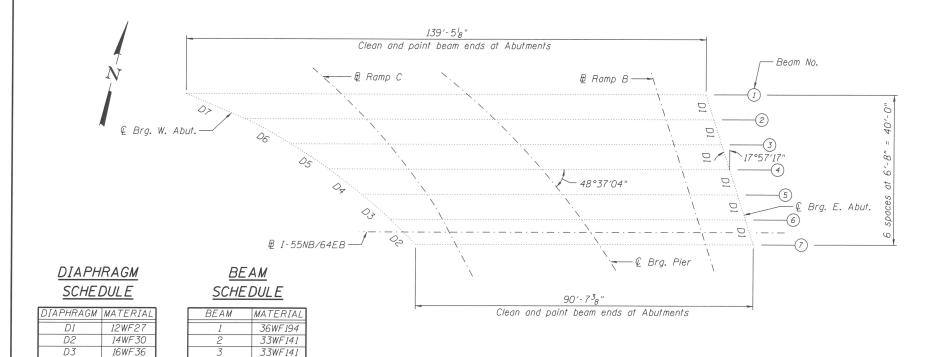
The structure is to remain open to traffic during construction. Ramp B and C are to be closed to traffic during construction.

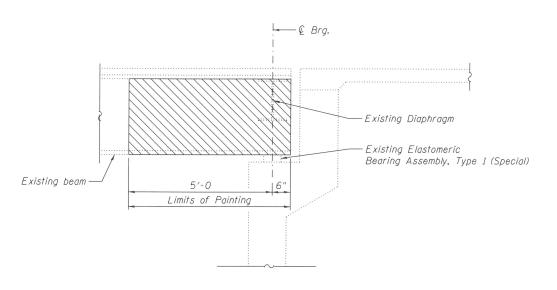
33WF141

33WF130

33WF130

36WF150





FRAMING PLAN

(S.N. 082-0023; Bridge No. 2)

#### ELEVATION AT ABUTMENT East Abutment shown, West Abutment Similar.

JERILYN M. HASSARD
EDWARDSVILLE, ILLINOIS
ILLINOIS LICENSED STRUCTURAL
ENGINEER NO. 081–006521
EXPIRES 11/30/2018

081-006521 LICENSED STRUCTURAL ENGINEER

#### <u>GENERAL NOTES</u>

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Cleaning and painting of the existing structural steel shall be as specified in the special provision for Cleaning and Painting Existing Steel Structures. Areas to be cleaned and painted shall consist of all beam ends, end diaphragms and steel components of the steel bearings at the abutments. Beam end painting shall extend 5 feet from centerline bearing longitudinally. This surface preparation shall be accomplished according to the requirements of Near-White Metal Blast Cleaning SSPC-SPIO. The paint system shall be applied as specified for System I OZ/E/U. The color of the final finish coat shall be Grey, Munsell No. 5B 7/1.

Care shall be taken not to damage rubber bearing or joint components during the blasting and cleaning operations. Any damage to these components shall be repaired at the Contractor's expense.

Complete cost of painting the locations described on this sheet shall be considered included with the pay item Cleaning and Painting Steel Bridge No. 2. Existing Structure Plans are available for review in the District office. Contact Herve Gelin at (618) 346-3179.

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL		
Cleaning and Painting Steel Bridge No. 2	L. Sum	1	-	1		
Containment and Disposal of Non-Lead	L. Sum	1	-	1		
Paint Cleaning Residues No. 2						

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	L
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D4

D6

16WF36

16WF36

16WF36

16WF40

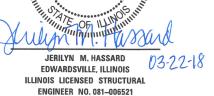
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	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - AEC	REVISED	
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

<b>GENERAL P</b>	LAN, GENERAL	NOTES. AND	PAINTING DETAILS	
		0023 I-55NB/6		
	3.IV. UOZ-(	1023 I-33146/0	4CD	
	OVER RAMI	P B AND RAI	MD C	
	OVER HAM	D AND NA	INIT	
	SHEET NO	1 OF 1 SHEE	TS	Ī

A.P. TE.	SEC1	ION		CO	UNTY	TOTAL SHEETS	SHEE NO.
99	1BR	-1-1		ST.	CLAIR	315	254
				CO	NTRACT	NO. 7	6G39
		ILLINOIS	FED. Al	D PRO	JECT		

The structure and Ramp B are to be completely closed to traffic during construction. No salvage. -Remove existing guardrail attachment, replace with Microsilica Concrete Overlay Traffic Barrier Terminal Existing S.N. 082-6006 Std. 631031 Type 6 (Bridge No. 3) (See Roadway Plans) Existing concrete barrier Brg. E. Abut.  $R = \pm 229' - 2"$ Replace expansion joint € Brg. W. Abut. R = ±189'-4" --Sta. 85+97.96 MLK Bridge Approach Out to Out nt Beam En Sta. 12+89.37 \$ Ramp B Match existing construction joint location, typ. -| <u>85+00</u> € F.A.P. Route 799 MLK Bridge Approach Existing Median Parapet Replace expansion joint Existing concrete barrier Existing concrete barrier ±60'-2'2" Back to Back Abutments PLAN 081-006521 LICENSED STRUCTURAL ENGINEER



EXPIRES 11/30/2018

GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Existing reinforcement shall be cleaned and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.

Surface preparation at the construction joints shall be performed using high-pressurized water spray, using equipment capable of producing a minimum water pressure of 5000 psi.

Cleaning and painting of the existing structural steel shall be as specified in the special provision for Cleaning and Painting Existing Steel Structures. Areas to be cleaned and painted shall consist of all beam ends, end diaphragms and steel components of the steel bearings at the abutments. Beam end painting shall extend 5 feet from centerline bearing of beams longitudinally. This surface preparation shall be accomplished according to the requirements of Near-White Metal Blast Cleaning SSPC-SPIO. The paint system shall be applied as specified for System I 02/E/U. The color of the final finish coat shall be Grey, Munsell No. 5B 7/1.

Complete cost of painting the locations described on this sheet shall be considered included with the pay item Cleaning and Painting Steel Bridge No. 3.

Care shall be taken not to damage rubber bearing or joint components during the blasting and cleaning operations. Any damage to these components shall be repaired at the Contractor's expense.

Bridge deck scarification and microsilica concrete overlay shall be as specified in the special provision for Bridge Deck Microsilica Overlay.

Protective coat shall only be applied to the deck and parapets (top and inside faces) adjacent to the proposed expansion joints and not the microsilica overlay.

Existing Structure Plans are available for review in the District office. Contact Herve Gelin at (618) 346-3179.

Synthetic fibers shall be added to the bridge deck microsilica concrete overlay. See Special Provisions.

#### TOTAL BILL OF MATERIAL

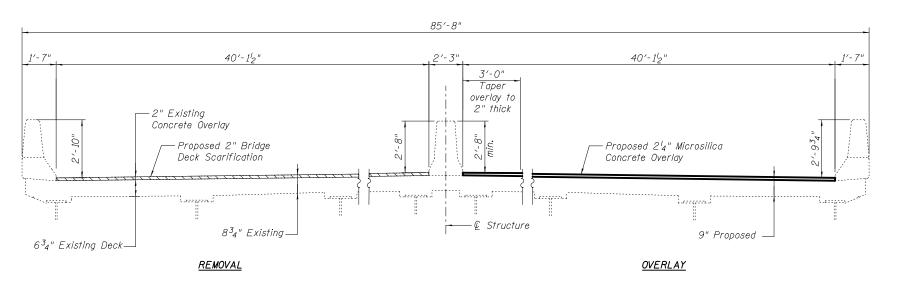
ITEM	UNIT	SUPER	SUB	TOTAL
Concrete Removal	Cu. Yd.	47.6	-	47.6
Concrete Superstructure	Cu. Yd.	48.9	-	48.9
Bridge Deck Grooving	Sq. Yd.	572	-	572
Protective Coat	Sq. Yd.	117	-	117
Reinforcement Bars, Epoxy Coated	Pound	6,170	-	6,170
Preformed Joint Strip Seal	Foot	251.0	-	251.0
Cleaning and Painting Steel Bridge No. 3	L. Sum	1	-	1
Bridge Deck Scarification 2"	Sq. Yd.	540.5	-	540.5
Bridge Deck Microsilica Concrete Overlay 21/4"	Sq. Yd.	540.5	-	540.5
Containment and Disposal of Non-Lead Paint Cleaning Residues No. 3	L. Sum	1	-	1

MODJESKI and MASTERS
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USER NAME =	DESIGNED - CDB	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL PLAN, GENERAL NOTES, AND TOTAL BILL OF MATERIAL	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.N. 082-6006 MLK BRIDGE APPROACH OVER RAMP B	799	1BR-1-1	ST. CLAIR	315	255
			CONTRACT	NO.	76G39
SUFET NO 1 OF 6 SUFETS					



#### CROSS SECTION

#### BILL OF MATERIAL

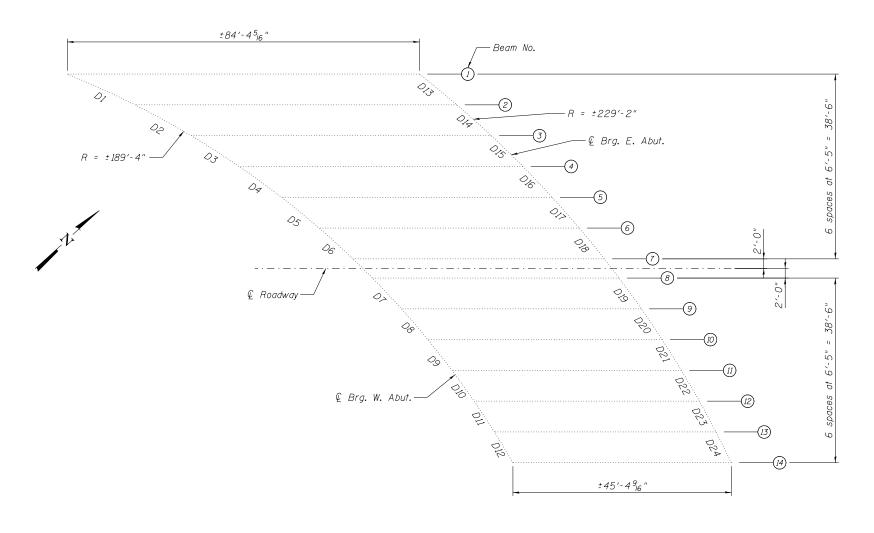
ITEM	UNIT	TOTAL
Bridge Deck Grooving	Sq. Yd.	572
Bridge Deck Scarification 2"	Sq. Yd.	540.5
Bridge Deck Microsilica Concrete Overlay 2 ¹ 4"	Sq. Yd.	540.5



USER NAME =	DESIGNED - CDB	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

MIC	ROSILICA C	ONCRETE OVI	RLAY	
S.N. 082–6006	MLK BRIDG	E APPROACH	OVER RAMP	В
	CUEET 110 0			



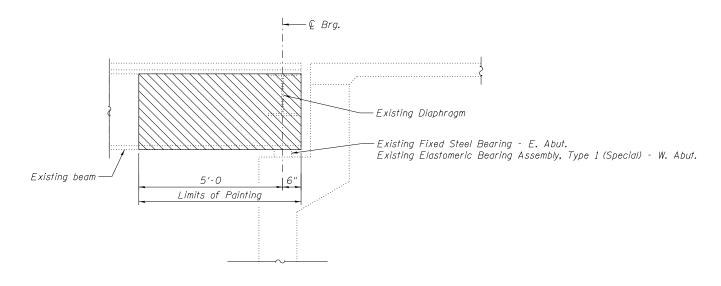
#### <u>DIAPHRAGM</u> <u>SCHEDULE</u>

# DIAPHRAGM MATERIAL DI thru D7 DI3 thru D15 D8 thru D12 D16 thru D24 D16 thru D24

#### <u>BEAM</u> SCHEDULE

BEAM	MATERIAL
1	36WF194
2	36WF194
3	36WF150
4	36WF150
5	36WF150
6	36WF150
7	36WF150
8	36WF150
9	36WF150
10	36WF150
11	36WF150
12	36WF150
13	36WF150
14	36WF150

#### FRAMING PLAN



#### ELEVATION AT ABUTMENT

East Abutment shown, West Abutment Similar.

#### BILL OF MATERIAL

ITEM	UNIT	TOTAL
Cleaning and Painting Steel Bridge No. 3	L. Sum	1
Containment and Disposal of Non-Lead	L. Sum	1
Paint Cleaning Residues No. 3		_

MODJESKI and MASTERS Experience great bridges.
------------------------------------------------

USER NAME =	DESIGNED - CDB	REVISED	
	CHECKED - JMH	REVISED	
PLOT SCALE =	DRAWN - AEC	REVISED	
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

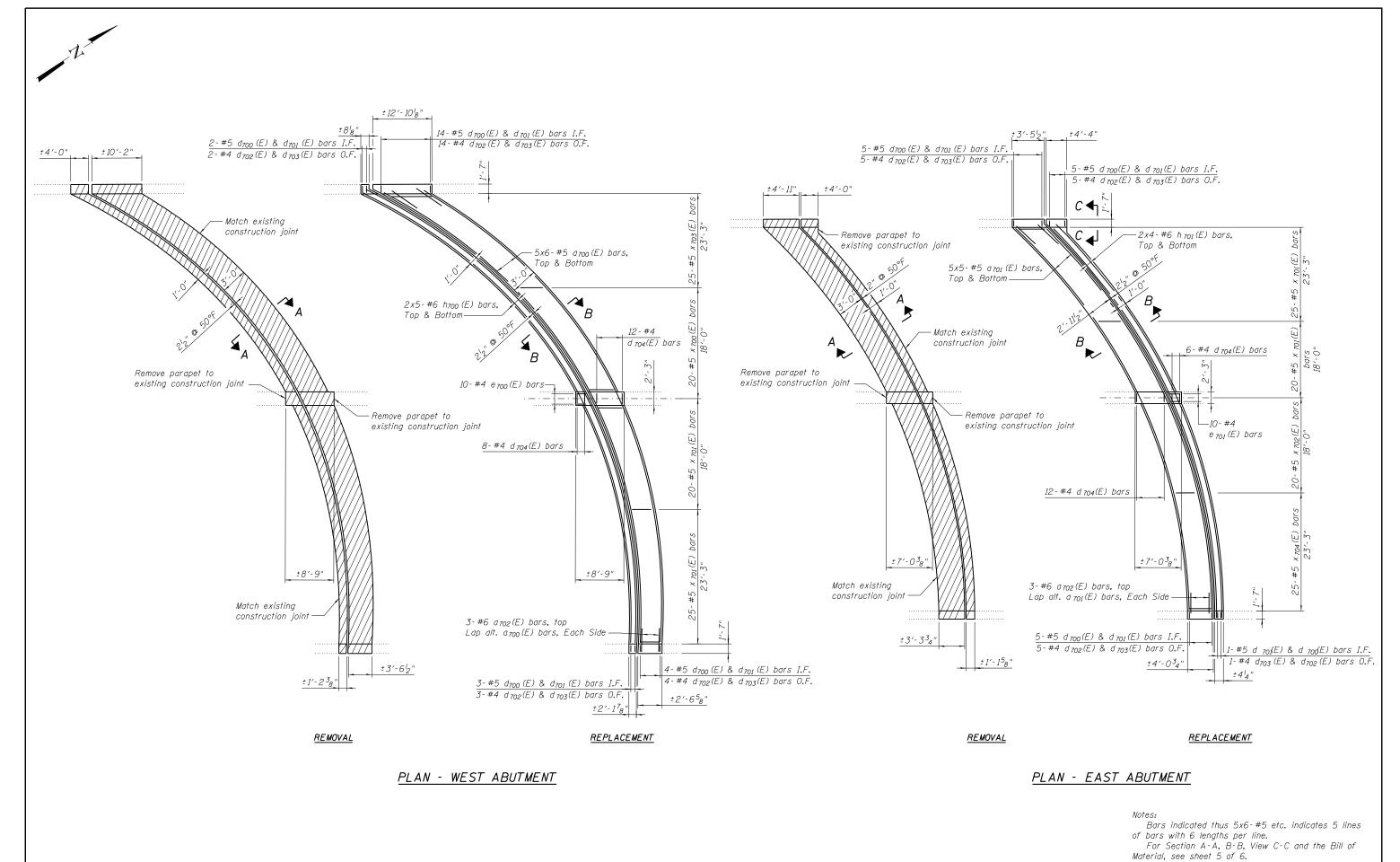
	PAINTIN	G DETAILS			
S.N. 082-6006	MLK BRIDG	E APPROACH	OVER	RAMP	В
	SHEET NO. 3	OF 6 SHEETS			

F.A.P. RTE. SECTION COUNTY TOTAL SHEET NO.

799 IBR-1-1 ST. CLAIR 315 257

CONTRACT NO. 76G39

| ILLINOIS FED. AID PROJECT



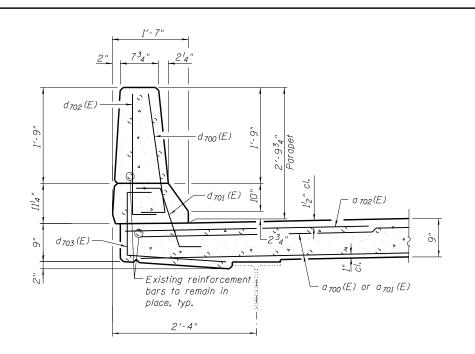
MODJESKI and MASTERS
Experience great bridges.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

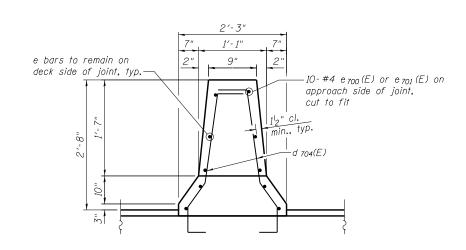
EXPANSION JOINT REPLACEMENT DETAILS - 1

S.N. 082-6006 MLK BRIDGE APPROACH OVER RAMP B

SHEET NO. 4 OF 6 SHEETS

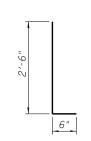


#### TYPICAL PARAPET SECTION

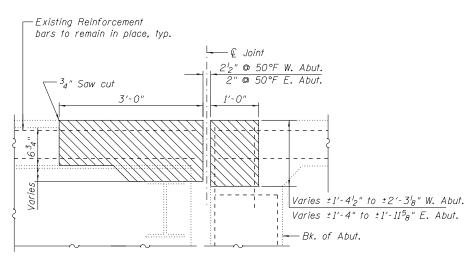


#### SECTION THRU MEDIAN BARRIER

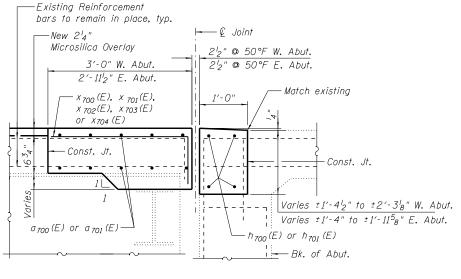
Note: Section and reinforcement replaced in kind.



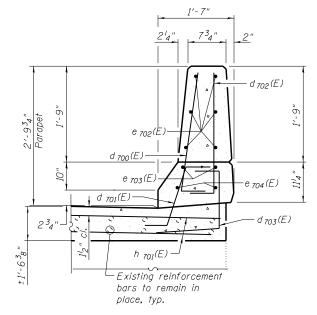
BARS d700 (E) & d702 (E)



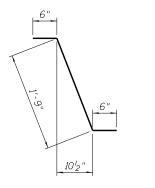
SECTION A-A (near € Roadway)



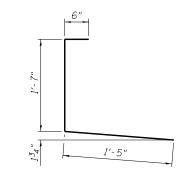
SECTION B-B (near € Roadway)



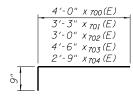
VIEW C-C



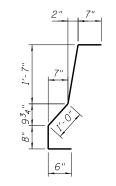
BAR d 701 (E)



BAR d ros(E)



BARS x 700 (E), x 701 (E) & x 702 (E)



BAR d 704(E)

#### BILL OF MATERIAL

Bar	No.	Size	Length	Shape
1700 (E)	60	#5	26′-4"	
1701 (E)	50	#5	25′-1"	
1702 (E)	12	#6	6′-6"	
1700 (E)	39	#5	3'-0"	
1 ₇₀₁ (E)	39	#5	3'-0" 2'-9" 3'-0"	
1702 (E)	39	#4	3'-0"	
1703 (E)		#4	3′-6"	٦
1704 (E)	38	#4	4'-4"	ĺ
9700 (E)	10	#4	4'-5"	
e701 (E)	10	#4	4'-0"	
9702 (E)	6	#4	3'-7"	
9703 (E)	2	#8	3'-7"	
9704 (E)	2	#5	3′-7"	
700 (E)	20	#6	31′-8"	
701 (E)		#6	31′-8"	
x700 (E)	20	#5	4'-9"	
x701 (E)	90	#5	4'-0"	
x702 (E)	20	#5	3′-9"	
х703 (E)	25	#5	5′-3"	
x704 (E)	25	#5	3′-6"	
Concret	e Remo	val	Cu. Yd.	47.6
Concrete Superstructure			Cu. Yd.	48.9
Profecti	ive Coat		Sq. Yd.	117
Reinfor	cement	Bars,	Pound	6 170
Ероху (	Coated		Pound	6,170

#### MINIMUM BAR LAP

COUNTY TOTAL SHEET NO.

ST. CLAIR 315 259

CONTRACT NO. 76G39

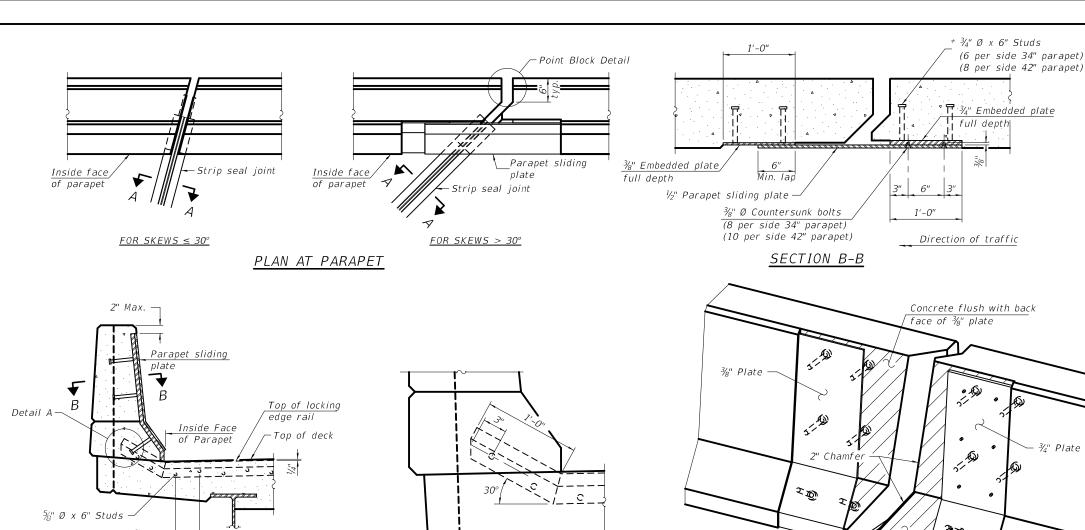
#5 bar = 3'-6" #6 bar = 4'-5"

SECTION 1BR-1-1

Note:	Hatched	areas	indicate	removal.

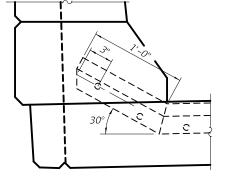
	USER NAME =	DESIGNED - CDB	REVISED		EXPANSION JOINT REPLACEMENT DETAILS – 2	F.A.P. RTE.
		CHECKED - JMH	REVISED	STATE OF ILLINOIS	S.N. 082-6006 MLK BRIDGE APPROACH OVER RAMP B	799
MODJESKI ••• MASTERS	PLOT SCALE =	DRAWN - AEC	REVISED	DEPARTMENT OF TRANSPORTATION	3.N. 002-0000 WILK BRIDGE AFFROAGII OVER RAINF B	
Experience great bridges.	PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED		SHEET NO. 5 OF 6 SHEETS	





#### ELEVATION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



DETAIL A

# (Ot. ez. Concrete flush with back face of ¾" plate

TRIMETRIC VIEW (Showing embedded plates only)

at 50° F

∕—Strip seal

### Locking edge railat 50° F Top of concrete -Strip seal at 50° F

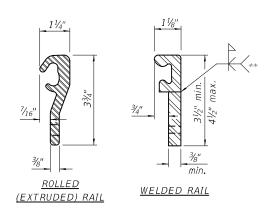
SHOWING ROLLED RAIL JOINT

#### $\frac{3}{6}$ " $\phi$ threaded rods in $\frac{7}{16}$ " $\phi$ holes at $\pm 4$ '-0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

*  $\frac{5}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs)

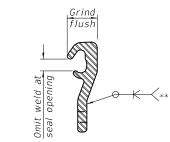
#### SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



#### LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip

The locking edge rails depicted are configured for typical

applications and are conceptual only. The actual configuration

of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails,

however, will not be allowed. Locking edge rails may exceed the

 $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. The Maximum space between locking edge rail segments

shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any

rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge

The manufacturer's recommended installation methods

Cost of parapet sliding plates, embedded plates, and

a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the

length of the bridge approach slab.

anchorage studs included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based

on the locking edge rail chosen by the Contractor. Deck and

parapet lengths shown elsewhere in the plans are dimensioned

to the concrete opening, not the joint opening, and are based

on the rolled locking edge rail. If the Contractor elects to use

according to the manufacturer's recommendation.

seal shall match the configuration of the locking edge

rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum

rated movement of 4 inches.

shall be followed.

rail splice detail.

#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

#### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	251.0

#### EJ-SS

8-11-17

MODJESKI and MASTERS Experience great bridges.	USER NAME =
MODJESKI and MASTERS	PLOT SCALE =
Experience great bridges.	PLOT DATE = Ø3/22/

USER NAME =	DESIGNED - CDB	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = Ø3/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

SHOWING WELDED RAIL JOINT

Locking edge rail-

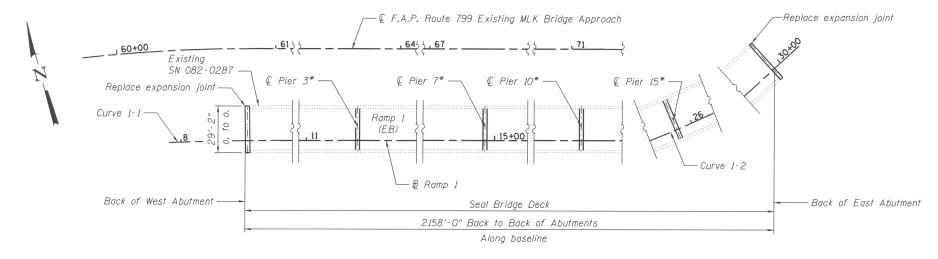
Top of concrete

PREFORMED JOINT STRIP SEAL S.N. 082-6006 MLK BRIDGE APPROACH OVER RAMP B SHEET NO. 6 OF 6 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
799	1BR-1-1	ST. CLAIR	315	260
		CONTRACT	NO. 7	6G39
	TILINOIS EED A	ID PROJECT		

The structure is to be completely closed to traffic during construction.

No salvage.



PLAN

*Finger joint trough cleaning locations

#### CURVE 1-1 DATA CURVE 1-2 DATA

P.I. Sta. = 6+77.67 P.I. Sta. = 25+77.53 △ = 13° 00′ 25″ (RT) △ = 42° 28′ 36" (LT) D = 3° 30′ 00" D = 8° 00′ 00" R = 1637.02'R = 716.20'T = 186.61' T = 278.35' L = 371.62' L = 530.96' E = 10.60' E = 52.19' P.C. Sta. = 4+91.06 P.C. Sta. = 22+99.18 P.T. Sta. = 8+62.68 P.T. Sta. = 28+30.14



ENGINEER NO. 081-006521 EXPIRES 11/30/2018

MODJESKI--MASTERS

USER NAME =	DESIGNED - CDB	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

**GENERAL PLAN** S.N. 082-0287 MLK BRIDGE RAMP 1 OVER RELOCATED IL RTE 3, VARIOUS RAILROADS, MISSOURI AVE. AND I-55SB/64WB SHEET NO. 1 OF 3 SHEETS

SECTION COUNTY TOTAL SHEE NO. 1BR-1-1 ST. CLAIR 315 261 CONTRACT NO. 76G39

#### GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Existing reinforcement shall be cleaned and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.

Surface preparation at the construction joints shall be performed using high-pressurized water spray, using equipment capable of producing a minimum water pressure of 5000 psi.

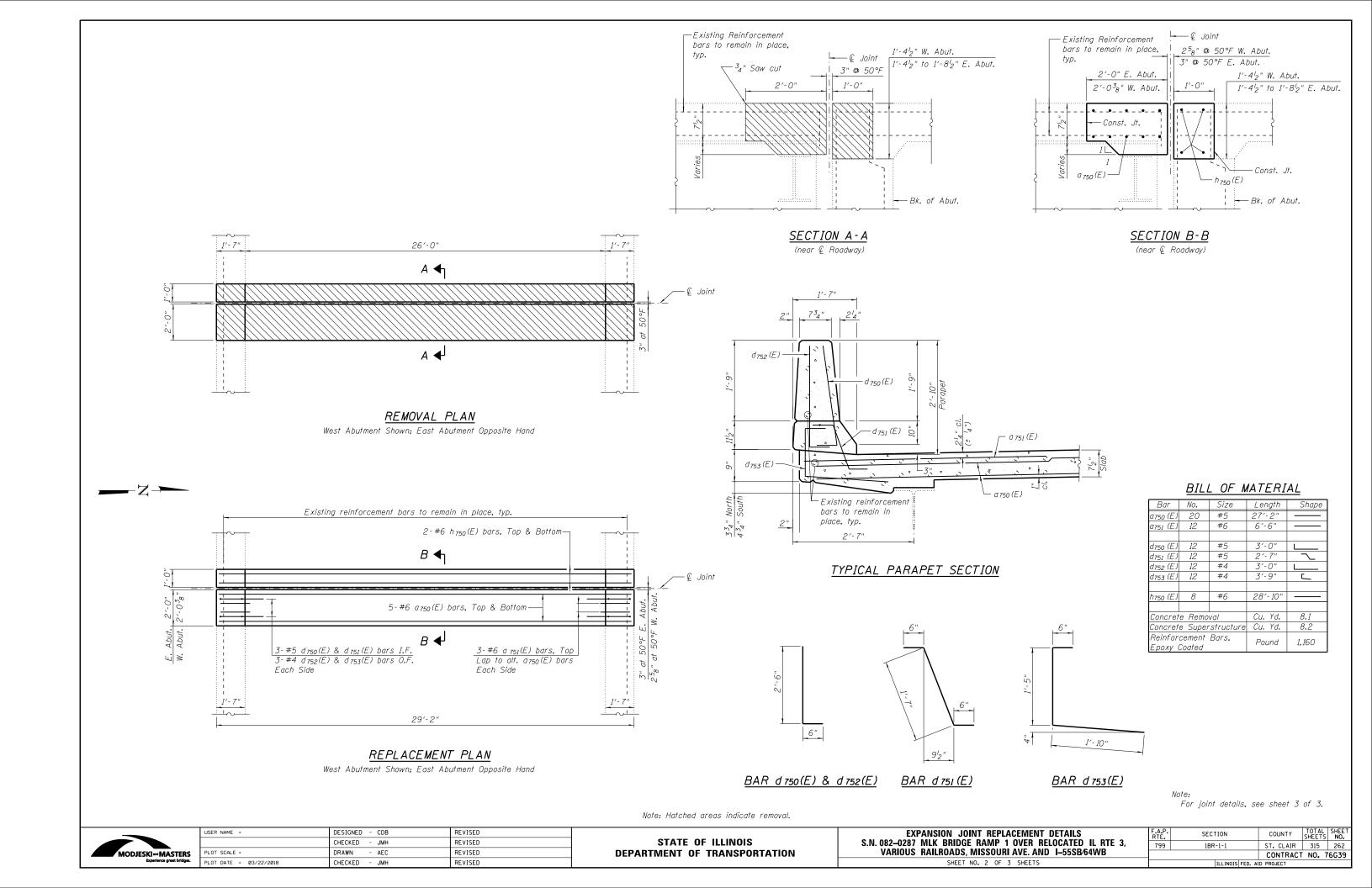
Bridge deck concrete sealer shall be applied on top and inside of all concrete barriers and top of slab for entire bridge length as specified in the special provision for Bridge Deck Concrete Sealer.

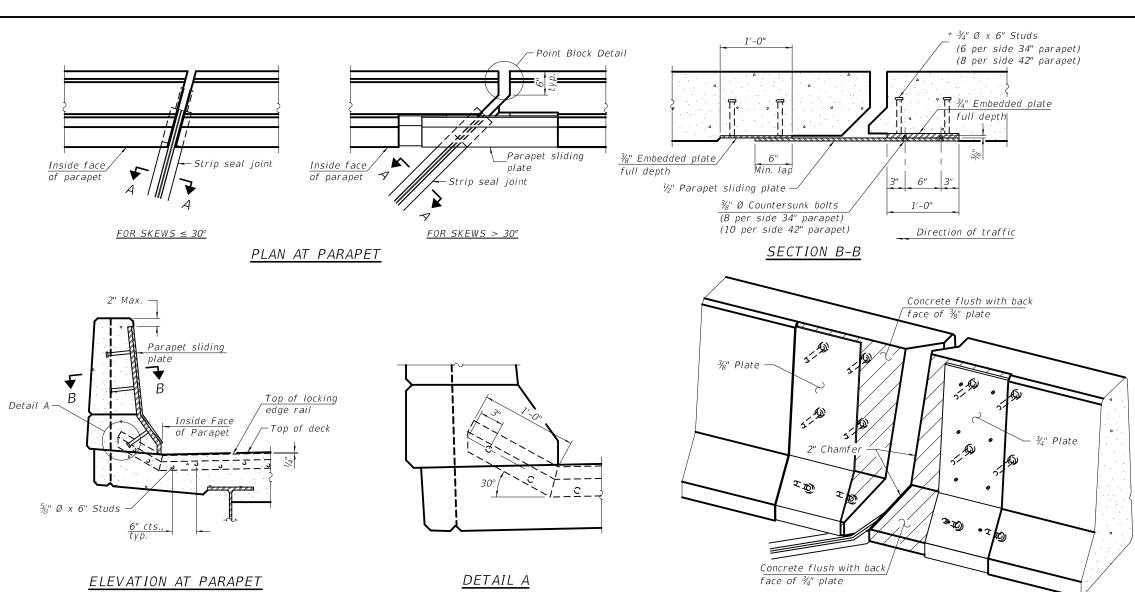
Existing Structure Plans are available for review in the District office. Contact Herve Gelin at (618) 346-3179.

The deck surface shall have its final finish tined according to Article 420.09(e)(1) of the Standard Specifications. Cost included with Concrete Superstructure.

#### TOTAL BILL OF MATERIAL

UNIT	SUPER	SUB	TOTAL
Cu. Yd.	8.1	-	8.1
Cu. Yd.	8.2	-	8.2
Pound	1,160	-	1,160
Foot	56.0	-	56.0
Each	4	-	4
Sq. Ft.	71,781	-	71,781
	Cu. Yd. Cu. Yd. Pound Foot Each	Cu. Yd. 8.1 Cu. Yd. 8.2 Pound 1,160 Foot 56.0 Each 4	Cu. Yd. 8.1 - Cu. Yd. 8.2 - Pound 1,160 - Foot 56.0 - Each 4 -





The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the  $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted.

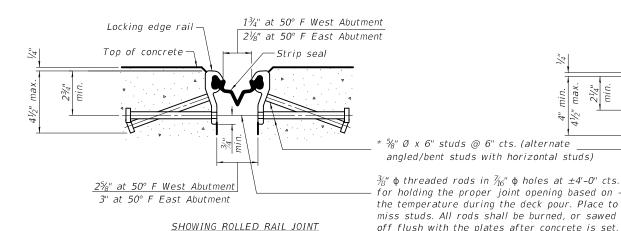
The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

TRIMETRIC VIEW (Showing embedded plates only)

Locking edge rail-

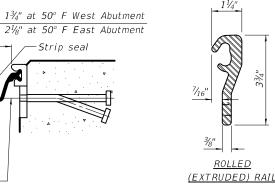
Top of concrete



8-11-17

#### off flush with the plates after concrete is set. SECTION A-A

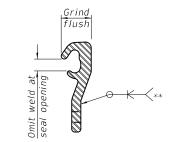
* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



# WELDED RAIL

#### LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

#### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	56.0

#### EJ-SS



USER NAME =	DESIGNED - CDB	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = 03/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

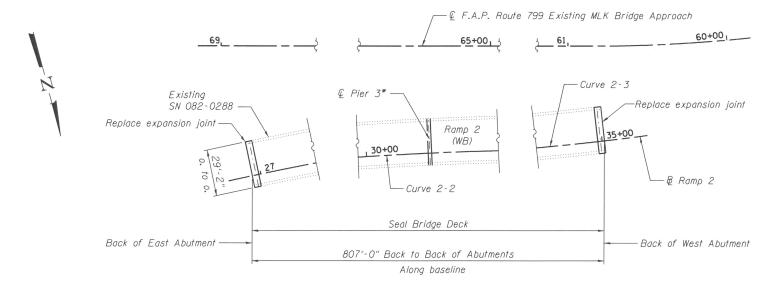
SHOWING WELDED RAIL JOINT

PREFORMED JOINT STRIP SEAL
S.N. 082-0287 MLK BRIDGE RAMP 1 OVER RELOCATED IL RTE 3, VARIOUS RAILROADS, MISSOURI AVE. AND 1-55SB/64WB SHEET NO. 3 OF 3 SHEETS

	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	799	1BR-1-1	ST. CLAIR	315	263
_	CONTRACT NO. 76G3S				6G39
ı		TH INOIS EED AT	ID PROJECT		

The structure is to be completely closed to traffic during construction.

No salvage



REVISED

REVISED

REVISED

REVISED

*Finger joint trough cleaning location

<u>PLAN</u>

#### CURVE 2-2 DATA CURVE 2-3 DATA

P.I. Sta. = 25+85.24 P.I. Sta. = 35+45.70 △ = 31° 05′ 32" (RT) △ = 16° 49′ 30" (LT) D = 4° 00′ 00" D = 2° 46′ 00" R = 2070.93' R = 1432.39'T = 576.12'T = 211.84' L = 1123.82 L = 420.63'E = 78.64' E = 15.58'P.C. Sta. = 33+33.86 P.C. Sta. = 20+09.12 P.T. Sta. = 31+32.94 P.T. Sta. = 37+54.49

JERILYN M. HASSARD
EDWARDSVILLE, ILLINOIS
ILLINOIS LICENSED STRUCTURAL
ENGINEER NO. 081–006521
ENGINEER NO. 081–006521
ENGINEER NO. 081–006521
ENGINEER NO. 081–006521
ENGINEER NO. 081–006521

#### 

#### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

## S.N. 082-0288 MLK BRIDGE RAMP 2 OVER RELOCATED IL RTE 3, VARIOUS RAILROADS, AND MISSOURI AVE. SHEET NO. 1 OF 3 SHEETS

# F.A.P. SECTION COUNTY TOTAL SHEETS NO. 799 1BR-1-1 ST. CLAIR 315 264 CONTRACT NO. 76G39 | ILLINDIS | FED. AID | PROJECT

#### GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Existing reinforcement shall be cleaned and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than  $50\,^\circ\text{F}$ .

Surface preparation at the construction joints shall be performed using high-pressurized water spray, using equipment capable of producing a minimum water pressure of 5000 psi.

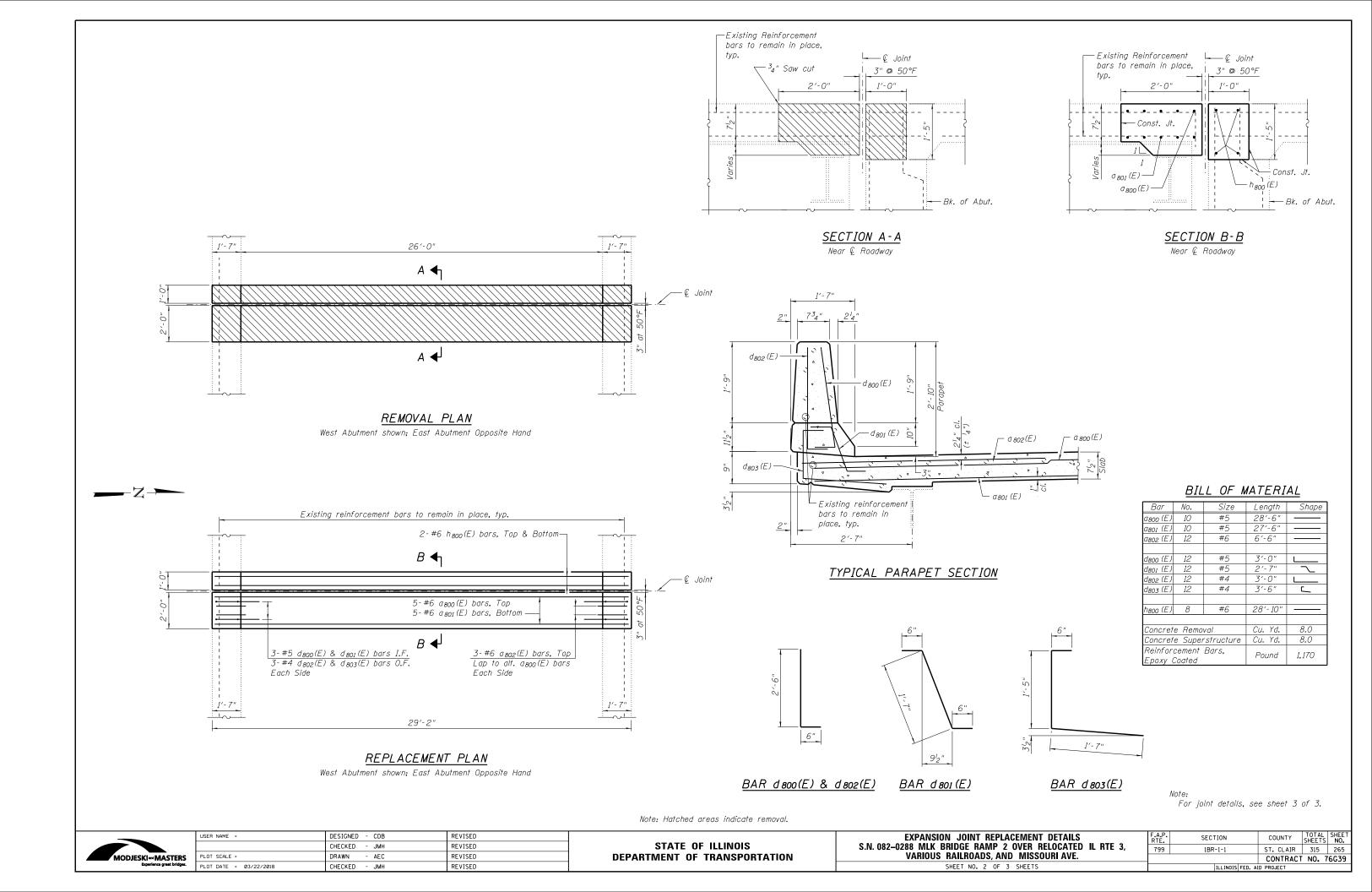
Bridge deck concrete sealer shall be applied on top and inside of all concrete barriers and top of slab for entire bridge length as specified in the special provision for Bridge Deck Concrete Sealer.

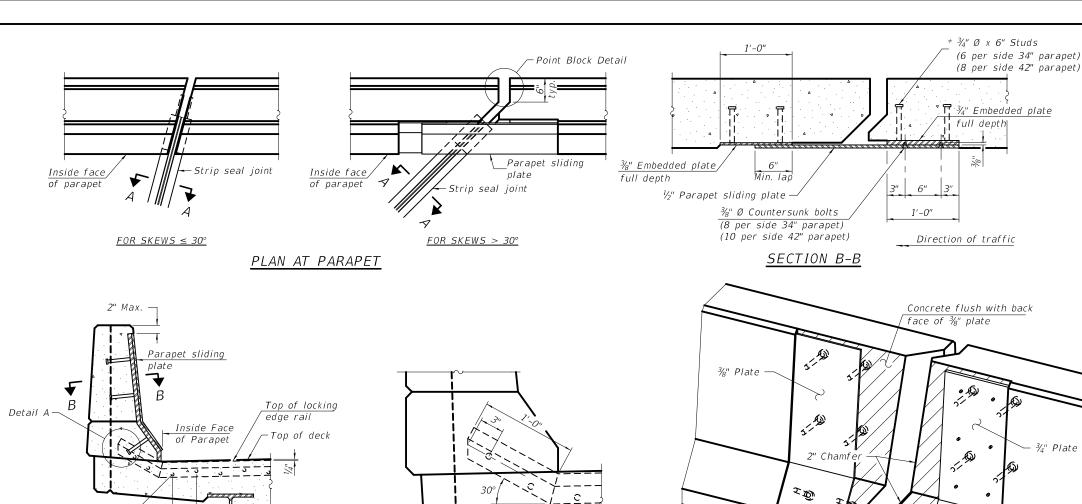
Existing Structure Plans are available for review in the District office. Contact Herve Gelin at (618) 346-3179.

The deck surface shall have its final finish tined according to Article 420.09(e)(I) of the Standard Specifications. Cost included with Concrete Superstructure.

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Concrete Removal	Cu. Yd.	8.0	-	8.0
Concrete Superstructure	Cu. Yd.	8.0	-	8.0
Reinforcement Bars, Epoxy Coated	Pound	1,170	-	1,170
Preformed Joint Strip Seal	Foot	56.0	-	56.0
Clean Trough	Each	1	-	1
Bridge Deck Concrete Sealer	Sq. Ft.	26,843	-	26,843





#### ELEVATION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

%" Ø x 6" Studs

# 3€.7Ø) Concrete flush with back face of ¾" plate

#### TRIMETRIC VIEW (Showing embedded plates only)

at 50° F

∕—Strip seal

Locking edge rail-Locking edge railat 50° F Top of concrete -Strip seal Top of concrete *  $\frac{5}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs)  $\frac{3}{6}$ "  $\phi$  threaded rods in  $\frac{7}{16}$ "  $\phi$  holes at  $\pm 4$ '-0" cts.

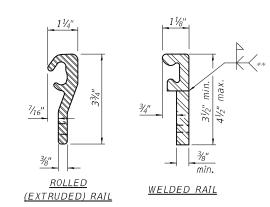
SHOWING ROLLED RAIL JOINT

#### for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

DETAIL A

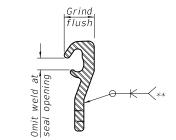
#### SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



#### LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip

The locking edge rails depicted are configured for typical

applications and are conceptual only. The actual configuration

of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails,

however, will not be allowed. Locking edge rails may exceed the

 $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. The Maximum space between locking edge rail segments

shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any

rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge

The manufacturer's recommended installation methods

Cost of parapet sliding plates, embedded plates, and

anchorage studs included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based

on the locking edge rail chosen by the Contractor. Deck and

parapet lengths shown elsewhere in the plans are dimensioned

to the concrete opening, not the joint opening, and are based

a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the

length of the bridge approach slab.

on the rolled locking edge rail. If the Contractor elects to use

according to the manufacturer's recommendation.

seal shall match the configuration of the locking edge

rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum

rated movement of 4 inches.

shall be followed.

rail splice detail.

#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

#### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	56.0

EJ-SS

MODJESKI MASTERS

8-11-17

at 50° F

USER NAME =	DESIGNED - CDB	REVISED
	CHECKED - JMH	REVISED
PLOT SCALE =	DRAWN - AEC	REVISED
PLOT DATE = Ø3/22/2018	CHECKED - JMH	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

SHOWING WELDED RAIL JOINT

PREFORMED JOINT STRIP SEAL S.N. 082-0288 MLK BRIDGE RAMP 2 OVER RELOCATED IL RTE 3, VARIOUS RAILROADS, AND MISSOURI AVE. SHEET NO. 3 OF 3 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
799	1BR-1-1	ST. CLAIR	315	266
		CONTRACT	NO. 7	6G39
	THE TNOTE EED AT	ID PROJECT		

