FOR INDEX OF SHEETS, SEE SHEET NO. 2

THIS PROJECT IS LOCATED IN THE

**VILLAGE OF RIVERDALE.** 

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EXISTING ADT = 15,900 (2021)

POSTED SPEED LIMIT = 40 MPH

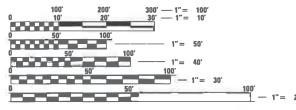
CLASSIFICATION = MINOR ARTERIAL



AMISH T. BHATT, P.E., S.E. LICENSE NO.: 081-006249 EXPIRES: NOVEMBER 30, 2024 FOR SHEETS 38-51, 53-90 ONLY



BRIAN M. CZOSNYKA, P.E. EXPIRES: NOVEMBER 30, 2025 FOR SHEETS 1-37 & 91-109 ONLY



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.

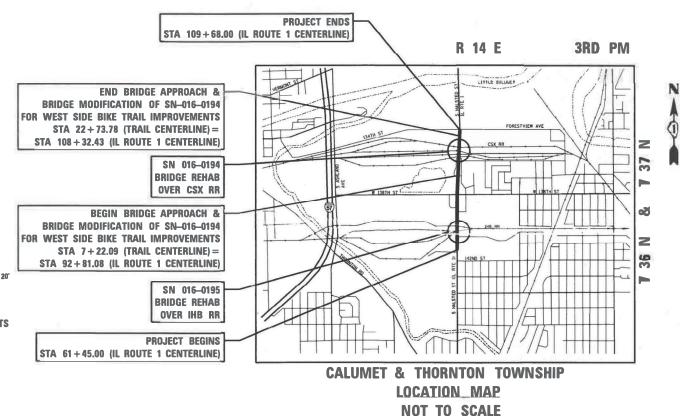
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123
OR 811

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

# PROPOSED HIGHWAY PLANS

F.A.U. ROUTE 3730: IL 1 (HALSTED STREET)
OVER IHB RR AND OVER CSX RR
BRIDGE REHABILITATION / BIKEWAY
SECTION 15-00131-01-BR
PROJECT NO. STP- HTDL (389)
COOK COUNTY

C-91-294-19



IDOT PROJECT MANAGER: VESELIN VELICHKOV CONSULTANT PROJECT MANAGER: BRIAN CZOSNYKA

GROSS LENGTH = 4,823.00 FT. = 0.91 MILE NET LENGTH = 4,823.00 FT. = 0.91 MILE F.A.U. RTE. SECTION COUNTY TOTAL SHEETS NO.

3730 15-00131-01-BR COOK 109 1

ILLINOIS CONTRACT NO. 62W48





PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS



# INDEX OF SHEETS

104-109

701901-09

704001-08

720001-01

720006-04

720011-01

725001-01

729001-01

781001-04

782006-01

SHEET NO.	DESCRIPTION
1	COVER SHEET
2-3	INDEX OF SHEETS, STATE STANDARDS, & GENERAL NOTES
4-13	SUMMARY OF QUANTITIES AND EARTHWORK SCHEDULE
14-15	TYPICAL SECTIONS
16-17	ALIGNMENT, TIES, & BENCHMARKS
18-19	ROADWAY REMOVAL PLANS
20-21	PROPOSED ROADWAY & BIKE TRAIL PLANS
22	PROPOSED BIKE TRAIL PROFILE
23-30	SUGGESTED MAINTENANCE OF TRAFFIC STAGING
31	OVERSIZED LOAD SIGNAGE PLACEMENT DIAGRAM
32-37	PROPOSED PAVEMENT MARKINGS PLANS
38-60	BRIDGE SN 016-0195 PLANS
61-90	BRIDGE SN 016-0194 PLANS
91-92	ROADWAY DETAILS
93-103	DISTRICT ONE STANDARDS

CROSS SECTIONS - BIKE TRAIL

# STATE STANDARDS

-	
STANDARD NO.	DESCRIPTION
000001-08	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
280001-07	TEMPORARY EROSION CONTROL SYSTEMS
442101-09	CLASS B PATCHES
602001-02	CATCH BASIN, TYPE A
604086-05	FRAME AND GRATE, TYPE 23
606001-08	CONCRETE CURB TYPE B AND COMBINATION CONCRETE CURB AND GUTTER
606301-04	PC CONCRETE ISLANDS AND MEDIANS
606306-04	CORRUGATED PC CONCRETE MEDIANS
630001-13	STEEL PLATE BEAM GUARDRAIL
630301-09	SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
631031-18	TRAFFIC BARRIER TERMINAL, TYPE 6
701101-05	OFF-ROAD OPERATIONS, MULTILANE, 15 FT TO 24 IN
	FROM PAVEMENT EDGE
701106-02	OFF-ROAD OPERATIONS, MULTILANE, MORE THAN 15 FT AWAY
701427-05	LANE CLOSURE MULTILANE, INTERMITTENT OR MOVING OPERATION,
	FOR SPEEDS 40 MPH OR LESS
701601-09	URBAN LANE CLOSURE, MULTILANE, 1W OR 2W WITH
	NONTRAVERSABLE MEDIAN
701606-10	URBAN SINGLE LANE CLOSURE, MULTILANE, 2W, WITH
	MOUNTABLE MEDIAN
701701-10	URBAN LANE CLOSURE, MULTILANE INTERSECTION
701801-06	SIDEWALK, CORNER OR CROSSWALK CLOSURE

METAL POSTS FOR SIGNS, MARKERS AND DELINEATORS

APPLICATIONS OF TYPES A AND B METAL POSTS (FOR SIGNS AND

TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS

GUARDRAIL AND BARRIER WALL REFLECTOR MOUNTING DETAILS

# DISTRICT 1 STANDARDS

TRAFFIC CONTROL DEVICES

MARKERS)

TEMPORARY CONCRETE BARRIER

SIGN PANEL MOUNTING DETAILS

SIGN PANEL ERECTION DETAILS

OBJECT AND TERMINAL MARKERS

STANDARD NO.	DESCRIPTION
BD-07	DETAIL OF STORM SEWER CONNECTION TO EXISTING SEWER
BD-24	CURB AND GUTTER REMOVAL AND REPLACEMENT
BD-32	BUTT JOINT AND HMA TAPER DETAILS
BD-34	DETAILS FOR DEPRESSED CURB & GUTTER AND SHOULDER
	TREATMENT AT TBT TY, 1 SPL,
BD-51	BENCHING DETAIL FOR EMBANKMENT WIDENING
TC-10	TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS,
	INTERSECTIONS AND DRIVEWAYS
TC-11	TYPICAL APPLICATIONS/RAISED REFLECTIVE PAVEMENT
	MARKERS (SNOW-PLOW RESISTANT)
TC-13	DISTRICT ONE TYPICAL PAVEMENT MARKINGS
TC-14	TRAFFIC CONTROL AND PROTECTION AT TURN BAYS
	(TO REMAIN OPEN TO TRAFFIC)
TC-16	SHORT TERM PAVEMENT MARKING LETTERS AND SYMBOLS
TC-22	ARTERIAL ROAD INFORMATION SIGN

# GENERAL NOTES - ROADWAY

- BEFORE STARTING ANY EXCAVATION, THE CONTRACTOR SHALL CALL "J.U.L.I.E." AT (800) 892-0123 FOR FIELD LOCATIONS OF BURIED ELECTRIC, TELEPHONE AND GAS UTILITIES. (48 HOUR NOTIFICATION IS REQUIRED.)
- 2. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH UTILITY COMPANIES, AND THE LOCAL MUNICIPALITY. THE LOCATIONS OF PUBLIC OR PRIVATE UTILITIES SHOWN ON THE PLANS ARE APPROXIMATE, AND THE DEPARTMENT AND DISTRICT DO NOT GUARANTEE THEIR ACCURACY. THE CONTRACTOR SHALL BE REQUIRED TO ASCERTAIN THE EXACT LOCATIONS OF SUCH UTILITIES AND EXERCISE CARE DURING CONSTRUCTION OPERATIONS SO AS NOT TO DAMAGE THEM, IN ACCORDANCE WITH THE SPECIAL PROVISIONS AND ARTICLE 107.20 OF THE "STANDARD SPECIFICATIONS". THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING THE UTILITY OWNERS SO THAT THEIR FACILITIES MAY BE ADJUSTED OR RELOCATED, IF NECESSARY, PRIOR TO THE START OF CONSTRUCTION OPERATIONS UNLESS OTHERWISE NOTED IN THE PLANS. ALL RELOCATION WORK ON EXISTING PRIVATE UTILITIES WILL BE DONE BY THE OWNER OF THAT UTILITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE OR DESTRUCTION OF PUBLIC OR PRIVATE PROPERTY, AND SHALL RESTORE SUCH PROPERTY IN ACCORDANCE WITH ARTICLE 107.20.
- THE CONTRACTOR WILL NOT BE ALLOWED TO SET UP A YARD OR FIELD OFFICE ON STATE PROPERTY WITHOUT WRITTEN PERMISSION FROM THE DEPARTMENT.
- 4. ALL DAMAGE TO EXISTING PAVEMENT MARKING OR RAISED REFLECTIVE PAVEMENT MARKERS OUTSIDE THE REMOVAL LINE SHOWN ON THE PLANS SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE.
- WHEN MILLED PAVEMENT IS OPENED TO TRAFFIC THE MAXIMUM GRADE DIFFERENTIAL BETWEEN PASSES OF THE MILLING MACHINE SHALL NOT EXCEED 1 1/2 INCHES WHERE THE SPEED LIMIT IS 45 MPH OR LESS AND 1 INCH WHERE THE SPEED LIMIT IS GREATER THAN 45 MPH. WITH WRITTEN APPROVAL FROM THE ENGINEER, A MAXIMUM GRADE DIFFERENTIAL OF 3 INCHES MAY BE ALLOWED IF THE EDGE OF THE MILLING IS SLOPED A MINIMUM 1:3 (V:H).
- 6. BUTT JOINTS WILL BE INSTALLED AT THE ENDS OF ALL RESURFACING (WHERE RESURFACING MEETS EXISTING PAVEMENT). IN ACCORDANCE WITH "BUTT JOINT AND HOT- MIX ASPHALT TAPER DETAILS" SHEET INCLUDED IN THE PLANS, UNLESS OTHERWISE SPECIFIED.
- 7. THE CONTRACTOR SHALL CONTACT KALPANA KANNAN-HOSADURGA, THE DISTRICT ONE TRAFFIC CONTROL SUPERVISOR AT KALPANA.KANNAN-HOSADURGA@ILLINOIS.GOV A MINIMUM OF 72 HOURS IN ADVANCE OF BEGINNING WORK.
- WHEN ARTIFICIAL LIGHTING IS UTILIZED IN NIGHT OPERATIONS, THE CONTRACTOR SHALL EXERCISE THE UTMOST PRECAUTIONS IN PREVENTING ADVERSE VISIBILITY TO THE MOTORING PUBLIC AND ADJOINING RESIDENTIAL AREA.
- BEFORE BEGINNING ANY WORK, THE CONTRACTOR SHALL RETAIN AND RECORD (FOR FUTURE REFERENCES), ALL EXISTING PAVEMENT MARKING LINES AND RAISED REFLECTIVE PAVEMENT MARKERS IN ORDER THAT THESE LOCATIONS CAN BE RE- ESTABLISHED FOR STRIPING. EXACT LOCATIONS OF ALL STRIPING SHALL BE AS DIRECTED BY THE ENGINEER.
- 10. THE ENGINEER SHALL CONTACT PATRICE HARRIS, AREA TRAFFIC FIELD TECHNICIAN, AT PATRICE.HARRIS@ILLINOIS.GOV A MINIMUM OF TWO WEEKS PRIOR TO PLACEMENT OF PERMANENT PAVEMENT MARKINGS.
- 11. THE "ADVANCE WARNING SIGN DETAIL FOR ARTERIAL TRAFFIC" LOCATED IN THE SPECIAL PROVISIONS FOR "TEMPORARY INFORMATION SIGNING FOR LANE CLOSURES" IS APPLICABLE ONLY TO ARTERIAL ROADS.
- 12. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS. DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS.
- 13. THESE PLANS HAVE BEEN PREPARED FROM NOTES RECEIVED FROM DISTRICT 1 BRIDGE
- 14. ALL ELEVATIONS IN THE PLANS ARE BASED UPON THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88). ALL BEARINGS AND COORDINATES REFERENCED IN THE PLAN DRAWINGS ARE BASED ON THE ILLINOIS STATE PLAN COORDINATE SYSTEM, EAST ZONE,
- 15. PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING PLANS ARE SUBJECT TO ROUTINE VARIATION. THE CONTRACTOR SHALL FIELD VERIFY EXISTING DIMENSIONS AND DETAILS AFFECTING NEW CONSTRUCTION OR ORDERING MATERIALS
- 16. THE CONTRACTOR SHALL BE REQUIRED TO DISPOSE OF ALL SIDEWALK, CURB AND GUTTER, PAVEMENT, AND ALL OTHER MATERIALS EXCAVATED OR REMOVED DUE TO CONSTRUCTION OPERATIONS.
- 17. BEFORE REMOVAL OF ANY UNSUITABLE MATERIAL, THE CONTRACTOR SHALL TREAT THE SUBGRADE AS PER ARTICLE 301.04 OF THE "STANDARD SPECIFICATIONS" TO THE SATISFACTION OF THE ENGINEER. UNSUITABLE MATERIAL SHALL NOT BE USED AS EMBANKMENT OR FILL UNDER THE PROPOSED HMA TRAIL AS SHOWN ON THE TYPICAL
- 18. RAILROAD FLAGGING MUST BE COORDINATED AND PROVIDED FOR WORK ON CSX AND IHB RIGHT-OF-WAY. FLAGGING FOR EACH BRIDGE STRUCTURE (SN 016-0194 & SN 016-0195) SHALL BE PROVIDED FOR A MINIMUM OF 25 WORKING DAYS EACH. THIS ITEM WILL BE PAID FOR ACCORDING TO ARTICLE 107.12 AND WILL BE REIMBURSED ACCORDING TO

- 19. STOCKPILES OF TOPSOIL AND OTHER MATERIALS SHALL NOT BE LOCATED WITHIN A SPECIAL MANAGEMENT AREA. APPROVAL OF THE LOCATION MUST BE OBTAINED FROM THE ENGINEER PRIOR TO PLACEMENT. IF A STOCKPILE IS TO REMAIN IN PLACE FOR MORE THAN THREE DAYS, EROSION CONTROL MEASURES SHALL BE PROVIDED. STOCKPILES TO REMAIN IN PLACE FOR 30 DAYS OR MORE SHALL RECEIVE TEMPORARY
- 20. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ABUTTING PROPERTY DURING THE CONSTRUCTION OF THIS PROJECT, EXCEPT FOR PERIODS OF SHORT DURATION AS APPROVED BY THE ENGINEER. SHOULD CLOSURE OF ACCESS BE REQUIRED, THE CONTRACTOR SHALL NOTIFY THE PROPERTY OWNER A MINIMUM OF 24 HOURS IN ADVANCE OF THE SHORT DURATION CLOSURE.
- 21. GRASS AREAS DISTURBED DURING CONSTRUCTION SHALL BE RESTORED WITH SIX INCHES OF TOPSOIL AND SEED OR SOD.
- 22. ALL STATION-OFFSET CALL OUTS AND CURVE DATA ON THE PLANS REFER TO THE PROPOSED CENTERLINE UNLESS OTHERWISE SHOWN.
- 23. ANY WORK ON HOLIDAYS AND WEEKENDS WILL NEED PRIOR APPROVAL FROM THE
- 24. DRAINAGE STRUCTURES: DURING CONSTRUCTION OPERATIONS, WHENEVER ANY LOOSE MATERIAL IS DEPOSITED IN THE FLOW LINE OF DRAINAGE STRUCTURES, SUCH THAT THE NATURAL FLOW OF WATER IS OBSTRUCTED, IT SHALL BE REMOVED BY THE CONTRACTOR AT THE CLOSE OF EACH WORKING DAY. AT THE CONCLUSION OF CONSTRUCTION OPERATIONS, ALL DRAINAGE STRUCTURES SHALL BE FREE FROM DIRT
- 25. WHEN EXISTING DRAINAGE FACILITIES ARE DISTURBED, THE CONTRACTOR SHALL PROVIDE AND MAINTAIN TEMPORARY OUTLETS AND CONNECTIONS FOR ALL FACILITIES TO TAKE IN ALL STORM WATER WHICH WILL BE RECEIVED BY THESE DRAINS AND SEWERS, AND DISCHARGE SAME. HE SHALL PROVIDE AND MAINTAIN AN EFFICIENT PUMPING PLANT, IF NECESSARY, AND A TEMPORARY OUTLET, AND BE PREPARED AT ALL TIMES TO DISPOSE OF THE WATER RECEIVED FROM THESE TEMPORARY CONNECTIONS UNTIL SUCH TIME AS THE PERMANENT CONNECTIONS WITH SEWERS ARE BUILT AND IN SERVICE. BEFORE ORDERING PIPE CULVERTS, THE CONTRACTOR SHALL CONTACT THE ENGINEER AS TO THE EXACT LENGTH AND QUANTITY REQUIRED.
- 26. THE SUBGRADE STABILITY SHALL BE VERIFIED BY PROOF ROLLING WITH A FULLY LOADED TANDEM-AXLE TRUCK.
- 27. GEOTECHNICAL FABRIC FOR GROUND STABILIZATION AND/OR AGGREGATE SUBGRADE IMPROVEMENT (CU YD) HAVE BEEN PROVIDED FOR USE AT THE LOCATIONS INDICATED FOR SOILS THAT TEND TO BE UNSTABLE AND/OR UNSUITABLE. THE ACTUAL NEED FOR REMOVAL AND REPLACEMENT WITH ABOVE ITEM WILL BE DETERMINED IN THE FIELD AT THE TIME OF CONSTRUCTION BY THE GEOTECHNICAL ENGINEER. ALL POTENTIALLY UNSTABLE SOILS SHOULD BE TESTED WITH A STATIC OR DYNAMIC CONE PENETROMETER AND TREATED IN ACCORDANCE WITH ARTICLE 301.04 OF THE SSRBC AND IDOT SUBGRADE STABILITY MANUAL. IF UNSTABLE AND/OR UNSUITABLE SOILS ARE NOT ENCOUNTERED, THEN THE QUANTITY SHALL BE DEDUCTED AND NO ADDITIONAL COMPENSATION WILL BE DUE TO THE CONTRACTOR.

# GENERAL NOTES - TRAFFIC CONTROL & PROTECTION

- THE CONTRACTOR SHALL CONTACT KALPANA KANNAN-HOSADURGA. THE DISTRICT ONE TRAFFIC CONTROL SUPERVISOR AT KALPANA, KANNAN-HOSADURGA@ILLINOIS.GOV A MINIMUM OF 72 HOURS IN ADVANCE OF BEGINNING WORK.
- TRAFFIC CONDITIONS, ACCIDENTS AND OTHER UNFORESEEN EMERGENCY CONDITIONS MAY REQUIRE THE ENGINEER TO RESTRICT, MODIFY OR REMOVE LANE CLOSURES OF CHANNELIZATION SHOWN IN THE PLANS. THE CONTRACTOR SHALL MAKE THE NECESSARY ADJUSTMENTS AS DIRECTED BY THE ENGINEER WITHOUT DELAY. THE CONTRACTOR SHALL RESPOND TO ANY REQUEST MADE BY THE ENGINEER FOR CORRECTION WITHIN TWO HOURS FROM THE TIME OF NOTIFICATION.
- 3. ALL TEMPORARY PAVEMENT MARKINGS PROPOSED WITHIN THE WORK AREA SHALL BE COMPLETED PRIOR TO THE CONSTRUCTION PHASE CHANGE.

			ANTICLE 103
USER NAME = ken.moy	DESIGNED - KLM	REVISED -	
	DRAWN - KLM	REVISED -	
PLOT SCALE = 2.00000 '/ in.	CHECKED - BMC	REVISED -	
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -	

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  INDEX

SCALE: NO

OF SHEETS, STATE STANDARDS, AND GENERAL NOTES					F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
	· · · · · · · · · · · · · · · · · · ·				3730	15-00131-01-BR	COOK	109	2	
ILLINOIS ROUTE 1 (HALSTED STREET)							CONTRACT	NO. 6	2W48	
IONE	SHEET NO. 1 OF 1	SHEETS	STA.	TO STA.	FED. RO	DAD DIST, NO. 1 THE INDIS FED. AT	D PROJECT			

- ALL TREE PROTECTION, TREE REMOVAL, PRUNING AND ROOT PRUNING SHALL BE COMPLETED BEFORE CONSTRUCTION OPERATIONS COMMENCE IN ANY AREA. AT NO TIME SHALL THE CONTRACTOR PRUNE OR REMOVE ANY TREES UNLESS SPECIFICALLY DIRECTED BY THE ENGINEER.
- 3. THE CONTRACTOR SHALL ERECT A TEMPORARY FENCE AROUND ALL TREES WITHIN THE CONSTRUCTION AREA TO ESTABLISH A "TREE PROTECTION ZONE" BEFORE ANY WORK BEGINS OR ANY MATERIAL IS DELIVERED TO THE JOBSITE. NO WORK IS TO BE PERFORMED (OTHER THAN ROOT PRUNING), MATERIALS STORED, OR VEHICLES DRIVEN OR PARKED WITHIN THE "TREE PROTECTION ZONE". REMOVE PROTECTIVE TEMPORARY FENCE ONLY AFTER ALL CONSTRUCTION WORK HAS BEEN COMPLETED.
- 4. TREE ROOT PRUNING IS TO BE USED ON EXISTING TREES/SHRUBS WHEN NECESSARY TO CUT MAJOR TREE/SHRUB ROOTS TO PREVENT THE RIPPING UP OF ROOTS WHEN TRENCHING OR EXCAVATION IS WITHIN THE ROOT ZONE OF ADJACENT TREES/SHRUBS TO REMAIN. SUPPLEMENTAL WATERING IS SPECIFIED FOR TREES AND SHRUBS THAT WILL BE DISTURBED BY CONSTRUCTION BUT WILL REMAIN. SUPPLEMENTAL WATERING OF TREES/SHRUBS SHALL BEGIN IMMEDIATELY AFTER ROOT PRUNING OF THE TREES/SHRUBS HAS OCCURRED.
- 5, THE CONTRACTOR SHALL ENSURE THAT NO CONSTRUCTION ACTIVITY, STORING, OR PARKING OF EQUIPMENT OR VEHICLES OCCURS BEYOND THE PERIMETER EROSION CONTROL BARRIER AND/ OR LIMITS OF CONSTRUCTION.
- 6. THE CONTRACTOR SHALL TAKE EXTRA CARE IN GRADING AND EXCAVATING NEAR TREES WHICH ARE NOT MARKED FOR REMOVAL SO AS NOT TO CAUSE INJURY TO THE ROOT SYSTEM OR TRUNKS. ANY DAMAGE DONE TO EXISTING ITEMS BY THE CONTRACTOR SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S OWN EXPENSE.
- 7. THE ENGINEER WILL CONTACT FABIOLA QUIROZ OF THE ROADSIDE DEVELOPMENT UNIT AT (847) 705-4596, AT LEAST 7 DAYS PRIOR TO PLANTING FOR LAYOUT OF THE SEEDING AND TREES.
- 8. THE CONTRACTOR SHALL OBSERVE AND COMPLY WITH ALL SECTIONS OF THE ILLINOIS CUSTOM SPRAY LAW, INCLUDING LICENSING. CONTRACTOR PERSONNEL APPLYING HERBICIDES SHALL HAVE A VALID PESTICIDE APPLICATOR LICENSE ISSUED BY THE ILLINOIS DEPARTMENT OF AGRICULTURE. THE LICENSED PESTICIDE APPLICATOR SHALL SUBMIT THEIR CURRENT LICENSE TO THE ENGINEER. THE LICENSED PESTICIDE APPLICATOR SHALL BE QUALIFIED AT A MINIMUM IN RIGHT-OF-WAY AND AQUATICS. THE LICENSED APPLICATOR SHALL WORK ON-SITE.

### SEDIMENTATION AND EROSION CONTROL NOTES:

- A. SOIL DISTURBANCE SHALL BE CONDUCTED IN SUCH A MANNER AS TO MINIMIZE EROSION. SOIL STABILIZATION MEASURES SHALL CONSIDER THE TIME OF YEAR, SITE CONDITIONS AND THE USE OF TEMPORARY OR PERMANENT MEASURES.
- B. SOIL EROSION AND SEDIMENT CONTROL FEATURES SHALL BE CONSTRUCTED PRIOR TO THE COMMENCEMENT OF UPLAND DISTURBANCE.
- C. TEMPORARY SOIL STABILIZATION SHALL BE APPLIED TO DISTURBED AREAS WITHIN 14 CALENDAR DAYS OF THE END OF ACTIVE HYDROLOGIC DISTURBANCE. PERMANENT STABILIZATION SHALL BE DONE WITHIN 14 DAYS AFTER COMPLETION OF FINAL GRADING.
- D. ALL STORM SEWER FACILITIES THAT ARE OR WILL BE FUNCTIONING DURING CONSTRUCTION SHALL BE PROTECTED, FILTERED, OR OTHERWISE TREATED TO REMOVE SEDIMENT.
- E. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED WITHIN 30 DAYS AFTER FINAL SITE STABILIZATION IS ACHIEVED OR AFTER THE TEMPORARY MEASURES ARE NO LONGER NEEDED. TRAPPED SEDIMENT AND OTHER DISTURBED SOIL AREAS SHALL BE PERMANENTLY STABILIZED.
- F. ALL TEMPORARY AND PERMANENT EROSION CONTROL MEASURES SHALL BE MAINTAINED AND REPAIRED AS NEEDED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSPECTION AND REPAIR DURING CONSTRUCTION.
- G. THE EROSION CONTROL MEASURES INDICATED ON THE PLANS ARE THE MINIMUM REQUIREMENTS. ADDITIONAL MEASURES MAY BE REQUIRED, AS DIRECTED BY THE ENGINEER OR GOVERNING AGENCY.
- H. THE CONTRACTOR SHALL INSTALL ALL EROSION CONTROL PRIOR TO THE START OF ANY FARTHWORK.
- IF A STOCKPILE IS TO REMAIN IN PLACE FOR MORE THAN THREE DAYS, EROSION CONTROL MEASURES SHALL BE PROVIDED BY THE CONTRACTOR.
- J. EROSION CONTROL MEASURES SHALL COMPLY WITH THE MINIMUM REQUIREMENTS OF THE COOK COUNTY STORMWATER AND FLOODPLAIN ORDINANCE SPECIFICATIONS AT ALL TIMES.

### GENERAL NOTES - RAILROAD:

- 1. REFER TO THE CSX TRANSPORTATION PUBLIC PROJECT INFORMATION MANUAL FOR ADDITIONAL REQUIREMENTS NEEDED FOR WORKING ON/ABOVE/ADJACENT TO CSXT. SPECIFIC SECTIONS THAT PERTAIN TO THIS PROJECT ARE: SPECIAL PROVISIONS FOR CONSTRUCTION NEAR CSXT PROPERTY, OVERHEAD BRIDGE CRITERIA, CONSTRUCTION SUBMISSION CRITERIA, SOIL AND WATER MANAGEMENT POLICY, AND INSURANCE REQUIREMENTS FOR PUBLIC PROJECTS.
- CONTRACTOR ACCESS WILL BE LIMITED TO THE IMMEDIATE PROJECT AREA ONLY. THE CSXT RIGHT-OF-WAY OUTSIDE THE PROJECT AREA MAY NOT BE USED FOR CONTRACTOR ACCESS TO THE PROJECT SITE AND NO TEMPORARY AT-GRADE CROSSINGS WILL BE ALLOWED.
- 3. CSXT MAY REQUIRE THE CONTRACTOR TO INSTALL FILTER FABRIC OVER THE TRACK(S)
  AND BALLAST TO PREVENT ANY CONSTRUCTION DEBRIS FROM FOULING THE BALLAST.
  THIS WILL BE DETERMINED DURING ACTUAL CONSTRUCTION ACTIVITIES BY CSXT OR ITS
  REPRESENTATIVE. FABRIC WILL REMAIN IN PLACE UNTIL ALL CONSTRUCTION
  ACTIVITIES ARE COMPLETE.
- 4. IF CONTRACTOR HAS THE POTENTIAL TO PENETRATE THE DECK DURING THE DECK REHABILITATION WORK, THEN CONTRACTOR WILL BE REQUIRED TO INSTALL FALSEWORK/DEMO SHIELD PROTECTION DIRECTLY OVER THE CSXT RAILROAD SPAN. THE FALSEWORK/DEMO SHIELD PROTECTION WILL BE INSTALLED PRIOR TO THE DECK BEING PENETRATED AND WILL STAY IN PLACE FOR THE DURATION OF THE CONSTRUCTION ACTIVITIES. THE FALSEWORK/DEMO SHIELD SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE TO CSXT'S CONSTRUCTION SUBMISSION CRITERIA.
- 5. THE CONTRACTOR MAY NOT USE CSXT RIGHT-OF-WAY FOR STORAGE OF MATERIALS OR EQUIPMENT DURING CONSTRUCTION WITHOUT PRIOR CSXT APPROVAL. THE CSXT RIGHT-OF-WAY MUST REMAIN CLEAR FOR RAILROAD USE AT ALL TIMES. EQUIPMENT MAY NOT BE POSITIONED TO BLOCK THE RAILROAD ACCESS ROAD, TRACK AREA OR ANY PART OF THE CSXT RIGHT-OF-WAY WITHOUT PRIOR CSXT APPROVAL.
- 6. TEMPORARY CONSTRUCTION CLEARANCE ENSURE ALL FALSEWORK, BRACING OR FORMS HAVE A MINIMUM HORIZONTAL CLEARANCE OF 9 FEET OR EXISTING, WHICHEVER IS LESS, MEASURED PERPENDICULAR TO THE CENTERLINE OF THE NEAREST TRACK, AND A MINIMUM VERTICAL CLEARANCE OF 21.5 FEET OR EXISTING, WHICHEVER IS LESS, AS MEASURED FROM THE TOP OF RAIL PROFILE.
- 7. THE CONTRACTOR WILL BE REQUIRED TO ABIDE BY THE PROVISIONS OF THE AGENCY/CSXT CONSTRUCTION AGREEMENT. PERIODICALLY, THROUGHOUT THE PROJECT DURATION, THE CONTRACTOR WILL BE REQUIRED TO MEET, DISCUSS AND, IF NECESSARY, TAKE IMMEDIATE ACTION AT THE DISCRETION OF CSXT PERSONNEL AND/OR THEIR AUTHORIZED REPRESENTATIVE, TO COMPLY WITH PROVISIONS OF THAT AGREEMENT AND THESE SPECIFICATIONS
- 8. UPON COMPLETION OF THE WORK ON CSXT PROPERTY, THE CONTRACTOR SHALL REQUEST THE OWNER TO ARRANGE A FINAL INSPECTION OF THE PROJECT WITH THE RAILROAD'S PROJECT ENGINEER OR HIS AUTHORIZED REPRESENTATIVE.
- CSXT SHALL BE NOTIFIED AT LEAST 5 DAYS IN ADVANCE OF THE PRE-CONSTRUCTION MEETING.
- 10.CSXT HAS SOLE AUTHORITY TO DETERMINE THE NEED FOR TRACK PROTECTION REQUIRED TO PROTECT ITS OPERATIONS AND PROPERTY. IN GENERAL, TRACK PROTECTION WILL BE REQUIRED WHENEVER CONTRACTOR OR EQUIPMENT ARE, OR ARE LIKELY TO BE, WORKING WITHIN FIFTY FEET OF TRACK OR OTHER TRACK CLEARANCES AS SPECIFIED BY CSX.

TO STA.

USER NAME = ken.moy	DESIGNED - KLM	REVISED -
	DRAWN - KLM	REVISED -
PLOT SCALE = 100.00000 '/ in.	CHECKED - BMC	REVISED -
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -

SCALE: NONE

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					ROADWAY	ROADWAY	BRIDGE	BRIDGE	BIKE TRAIL
ſ					80% FED / 20% STATE				
	CODE			TOTAL	SN 016-0194	SN 016-0195	SN 016-0194	SN 016-0195	SN 016-0194
		PTFA4							
	NO.	ITEM	UNIT	QUANTITY	0005	0005	0013	0013	0021
				URBAN					
*	20101000	TEMPORARY FENCE	FOOT	310					310
	20101200	TREE ROOT PRUNING	EACH	11					11
-									
	20101300	TREE PRUNING (1 TO 10 INCH DIAMETER)	EACH	6					6
-	20101350	TREE PRUNING (OVER 10 INCH DIAMETER)	EACH	8					8
-									
	20200100	EARTH EXCAVATION	CUYD	30					30
	20201200	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL	CUYD	455					455
-	20201200	REMOVAE AND DISTORED TO STREET MALERIAL	0010	455					433
-	20400800	FURNISHED EXCAVATION	CUYD	375					375
÷									
-	20800150	TRENCH BACKFILL	CUYD	20					20
	21001000	GEOTECHNICAL FABRIC FOR GROUND STABILIZATION	SQYD	1132					1132
_									
*	21101625	TOPSOIL FURNISH AND PLACE, 6"	SQYD	1230					1230
*									
Τ.	25000210	SEEDING, CLASS 2A	ACRE	0.25					0.25
*	25000400	NITO OCCULETOTILIZED NITOICAT	DOUND						8
-	25000400	NITROGEN FERTILIZER NUTRIENT	POUND	8					•
*	25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	8					8
-				-					-
*	25100115	MULCH, METHOD 2	ACRE	0.25					0.25
-									

\* = SPECIALTY ITEM

DESIGNED - KLM REVISED DRAWN - KLM REVISED CHECKED - BMC REVISED PLOT DATE = 4/2/2024 DATE - 04/02/2024 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES SHEET NO. 1 OF 10 SHEETS STA. TO STA.

CONSTRUCTION TYPE CODE

68671539 CHT COO doc	160.000-110-10011000
/dm=(03030)	Collision of the
í	1100
/out-date	District Council Co
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				CONSTRUCTION TYPE CODE					
				ROADWAY	ROADWAY	BRIDGE	BRIDGE	BIKE TRAIL	
CODE			TOTAL	80% FED / 20% STATE SN 016-0194	80% FED / 20% STATE SN 016-0195	80% FED / 20% STATE SN 016-0194	80% FED / 20% STATE SN 016-0195	80% FED / 20% STATE SN 016-0194	
NO.	ПЕМ	UNIT	QUANTITY URBAN	0005	0005	0013	0013	0021	
25200200	SUPPLEMENTAL WATERING	UNIT	330					330	
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	27					27	
28000400	PERIMETER EROSION BARRIER	FOOT	818					818	
28000510	INLET FILTERS	EACH	4					4	
30300001	AGGREGATE SUBGRADE IMPROVEMENT	CUYD	68					68	
31101400	SUBBASE GRANULAR MATERIAL, TYPE B 6"	SQYD	39	39					
35102000	AGGREGATE BASE COURSE, TYPE B 8"	SQYD	905					905	
40600275	BITUMINOUS MATERIALS (PRIME COAT)	POUND	2036					2036	
40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	1296	485	540			271	
40600985	PORTLAND CEMENT CONCRETE SURFACE REMOVAL - BUTT JOINT	SQYD	648	288	360				
40601005	HOT-MIX ASPHALT REPLACEMENT OVER PATCHES	TON	4	4					
40602985	HOT-MIX ASPHALT BINDER COURSE, IL-9.5, N70	TON	44					44	
40604060	HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N50	TON	102					102	
40604062	HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70	TON	275	106	130			39	

AECOM
303 East Wacker Drive, Suite 1400, Chicago, IL 60601 T 1-312-373-7700 F 1-312-373-6800

-	USER NAME = ken.moy	DESIGNED -	KLM	REVISED	-
		DRAWN -	KLM	REVISED	-
	PLOT SCALE = 40.0000 '/ in.	CHECKED -	BMC	REVISED	-
	PLOT DATE = 4/2/2024	DATE -	04/02/2024	REVISED	-

					F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
SUMMARY OF QUANTITIES						15-00131-01-BR	COOK	109	5
							CONTRACT	NO. 6	2W48
	SHEET NO. 2 OF	10 SHEETS	STA.	TO STA.	FED. R	OAD DIST. NO. 1   ILLINOIS FED. A	ID PROJECT		

				ROADWAY	ROADWAY	BRIDGE	BRIDGE	BIKETRAIL
				80% FED / 20% STATE				
CODE			TOTAL	SN 016-0194	SN 016-0195	SN 016-0194	SN 016-0195	SN 016-0194
NO.	ПЕМ	UN <b>i</b> T	QUANTITY	0005	0005	0013	0013	0021
			URBAN					
42001300	PROTECTIVE COAT	SQYD	222	115				107
42400200	PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH	SQ FT	352	352				
44000159	HOT-MIX ASPHALT SURFACE REMOVAL, 2 1/2"	SQYD	139					139
44000500	COMBINATION CURB AND GUTTER REMOVAL	FOOT	1037	223				814
44000600	SIDEWALK REMOVAL	SQ FT	1986	973				1013
44002210	HOT-MIX ASPHALT REMOVAL OVER PATCHES, 2 1/2"	SQYD	35					35
44003100	MEDIAN REMOVAL	SQ FT	105					105
44003510	MEDIAN REMOVAL PARTIAL DEPTH	SQ FT	4501					4501
44003310	MEDIAT CHIOVAL I ANIAL DEI III	3011	4301					4301
44200956	CLASS B PATCHES, TYPE II, 9 INCH	SQYD	35					35
44201298	DOWEL BARS 1 1/4"	EACH	27					27
44213200	SAW CUTS	FOOT	105					105
44213208	TIE BARS 11/4"	EACH	27					27
48203021	HOT-MIX ASPHALT SHOULDERS, 6"	SQYD	59					59
50102400	CONCRETE REMOVAL	CUYD	190.4			41.7	57.0	91.7

CONSTRUCTION TYPE CODE

\* = SPECIALTY ITEM

**AECOM**303 East Wacker Drivo, Sulto 1400, Chicago, IL 60801
11:310-379-3700 E-1-310-379-8900

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

| F.A.U. | SECTION | COUNTY | SHEET | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 1 | NO. 2 | NO. 2 | NO. 3 | NO. 2 | NO. 3 | NO. 2 | NO. 3 |

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				CONSTRUCTION TYPE CODE						
				ROADWAY	ROADWAY	BRIDGE	BRIDGE	BIKE TRAIL		
CODE			TOTAL	80% FED / 20% STATE SN 016-0194	80% FED / 20% STATE SN 016-0195	80% FED / 20% STATE SN 016-0194	80% FED / 20% STATE SN 016-0195	80% FED / 20% STATE SN 016-0194		
NO.	ПЕМ	UNIT	QUANT <b>I</b> TY	0005	0005	0013	0013	0021		
			URBAN							
50157300	PROTECTIVE SHIELD	SQYD	9151			4004	5147			
50300255	CONCRETE SUPERSTRUCTURE	CUYD	519.9			106.1	175.6	238.2		
50300300	PROTECTIVE COAT	SQYD	11401			3738	6075	1588		
50500405	FURNISHING AND ERECTING STRUCTURAL STEEL	POUND	29090			11490	17600			
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	53620			11180	16650	25790		
50800205	REINFURCEMENT BARS, EPOXY COATED	POUND	53620			11180	10000	25790		
50800515	BAR SPUCERS	EACH	504			204	300			
50901720	BICYCLE RAILING	FOOT	789					789		
50901750	PARAPET RAILING	FOOT	1379					1379		
50901750	PARAPEI RAILING	FOOT	1219					13/9		
52000110	PREFORMED JOINT STRIP SEAL	FOOT	605			270	335			
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	140			60	80			
52100520	ANCHOR BOLTS, 1"	EACH	280			120	160			
						120	100			
550A0340	STORM SEWERS, CLASS A, TYPE 2 12"	FOOT	19					19		
55100500	STORM SEWER REMOVAL 12"	FOOT	24					24		
59000200	EPOXY CRACK INJECTION	FOOT	358			208	150			
SPECIALTY ITE	М			<u>I</u>	I	I	<u>I</u>	<u>I</u>		

<b>AECOM</b>
303 East Wacker Drive, Suite 1400, Chicago, IL 60601 T 1-312-373-7700 F 1-312-373-6800

USER NAME = ken.moy	DESIGNED	-	KLM	REVISED	-
	DRAWN	-	KLM	REVISED	-
PLOT SCALE = 40.00000 '/ in.	CHECKED	-	ВМС	REVISED	-
PLOT DATE = 4/2/2024	DATE	-	04/02/2024	REVISED	-

		F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
SUMMARY OF QUANTITIES		3730	15-00131-01-BR	COOK	109	7	
				CONTRACT	NO. 6	2W48	
SHEET NO. 4 OF 10 SHEETS STA.	TO STA.	FED. R	DAD DIST. NO. 1   ILLINOIS FED. AI	D PROJECT			

					ROADWAY	ROADWAY	BRIDGE	BRIDGE	BİKE TRAİL
					80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE
	CODE			TOTAL	SN 016-0194	SN 016-0195	SN 016-0194	SN 016-0195	SN 016-0194
	NO.	ІТЕМ	UNIT	QUANTITY	0005	0005	0013	0013	0021
•		·· <b>-</b> ··	0.1	URBAN					***
				UNDAIN					
	60201330	CATCH BASINS, TYPE A, 4'-DIAMETER, TYPE 23 FRAME AND GRATE	EACH	4					4
	60255500	MANHOLES TO BE ADJUSTED	EACH	1					1
	60500050	REMOVING CATCH BASINS	EACH	4					4
	60620800	CONCRETE MEDIAN, TYPE SB-9.12	SQ FT	48					48
	60624600	CORRUGATED MEDIAN	SQ FT	56					56
*	63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	1					1
*	63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	1					1
	63200310	GUARDRAIL REMOVAL	FOOT	288					288
_									
*	66900200	NON-SPECIAL WASTE DISPOSAL	CUYD	390					390
*	66900530	SOIL DISPOSAL ANALYSIS	EACH	2					2
_ ا									
*	66901001	REGULATED SUBSTANCES PRE-CONSTRUCTION PLAN	L SUM	1					1
_									
*	66901003	REGULATED SUBSTANCES FINAL CONSTRUCTION REPORT	L SUM	1					1
_									
*	66901006	REGULATED SUBSTANCES MONITORING	CAL DA	5					5
	67100100	MOBILIZATION	L SUM	1			0.25	0.50	0.25
		I .	1	1	1	1	I .		,

CONSTRUCTION TYPE CODE

\* = SPECIALTY ITEM

**AECOM**303 East Wacker Prive, Sulto 400, Chicago, IL 60601

USER NAME = ken.moy	DESIGNED	-	KLM	REVISED	-
	DRAWN	-	KLM	REVISED	-
PLOT SCALE = 40.00000 '/ in.	CHECKED	-	ВМС	REVISED	-
PLOT DATE = 4/2/2024	DATE	-	04/02/2024	REVISED	-

				F.A.U. RTE.	SEC <sup>-</sup>	LION	COUNTY	TOTAL	SHEET NO.
SUMMARY	OF QU	ANTITIES		3730	15-0013	1-01-BR	COOK	109	8
							CONTRACT	NO.	52W48
SHEET NO. 5 OF 10	SHEETS	STA.	TO STA.	FED. RO	DAD DIST. NO. 1	ILLINOIS FED. AI	D PROJECT		

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				CONSTRUCTION TYPE CODE						
				ROADWAY	ROADWAY	BRIDGE	BRIDGE	BIKE TRAIL		
0005			TOTAL	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE		
CODE NO.	ПЕМ	UNIT	TOTAL QUANTITY	SN 016-0194 0005	SN 016-0195 0005	SN 016-0194 0013	SN 016-0195 0013	SN 016-0194 0021		
			URBAN							
70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	75	45	30					
<b>70107</b> 025	CHANGEABLE MESSAGE SIGN	CAL DA	60	36	24					
70300100	SHORT TERM PAVEMENT MARKING	FOOT	10963	5395	5568					
70300150	SHORT TERM PAVEMENT MARKING REMOVAL	SQ FT	2029	1220	809					
70307120	TEMPORARY PAVEMENT MARKING - LINE 4"-TYPE IV TAPE	FOOT	44405	26609	17796					
70307130	TEMPORARY PAVEMENT MARKING - LINE 6" - TYPE IV TAPE	FOOT	109	109						
70400100	TEMPORARY CONCRETE BARRIER	FOOT	3187.5	3187.5						
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	6912.5	2587.5	4325.0					
70600250	IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3	EACH	4	2	2					
			_	_	_					
70600350	IMPACT ATTENUATORS, RELOCATE (NON-REDIRECTIVE), TEST LEVEL 3	EACH	4	2	2					
70400500	DEL COATE CION DANIEL ACCEPANTY. THE A	FAOU								
72400500	RELOCATE SIGN PANEL ASSEMBLY - TYPE A	EACH	1					1		
72501000	TERMINAL MARKER - DIRECT APPLIED	EACH	1					1		
12901000	IEMMINAL MARKEN - DIRECT AFFLIED	EAUT	1					1		
78000100	THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS	SQ FT	183	110	73					
		54.1	100							
78000200	THERMOPLASTIC PAVEMENT MARKING - LINE 4"	FOOT	6204	2584	3620					
78000400	THERMOPLASTIC PAVEMENT MARKING - LINE 6"	FOOT	570	435	135					
78000600	THERMOPLASTIC PAVEMENT MARKING - LINE 12"	FOOT	32	32						
78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	1724					1724		
78004635	PREFORMED PLASTIC PAVEMENT MARKING, TYPE D - STANDARD - LINE 7"	FOOT	630			270	360			
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DESIGNED - KLM REVISED DRAWN -KLM REVISED REVISED CHECKED - BMC PLOT DATE = 4/2/2024 - 04/02/2024 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES SHEET NO. 6 OF 10 SHEETS STA. TO STA.

				[	CONSTRUCTION TYPE CODE					
					ROADWAY	ROADWAY	BRIDGE	BIKE TRAIL		
	CODE			TOTAL	80% FED / 20% STATE SN 016-0194	80% FED / 20% STATE SN 016-0195	80% FED / 20% STATE SN 016-0194	80% FED / 20% STATE SN 016-0195	80% FED / 20% STATE SN 016-0194	
	NO.	ITEM	UNIT	QUANTITY	0005	0005	0013	0013	0021	
				URBAN						
*	78009004	MODIFIED URETHANE PAVEMENT MARKING - LINE 4"	FOOT	5102	2228	2874				
	78011040	GROOVING FOR RECESSED PAVEMENT MARKING 8"	FOOT	2551			1114	1437		
*	78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	81	81					
*		L						36		
*	78200006	GUARDRAIL REFLECTORS, TYPE B	EACH	2					2	
*									18	
Ī	78300200	RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	EACH	64			28	36		
Ī										
Ī	78300202	PAVEMENT MARKING REMOVAL - WATER BLASTING	SQ FT	3348	1795	1553				
*	81100605	CONDUIT ATTACHED TO STRUCTURE, 2 " DIA., PVC COATED GALVANIZED STEEL	FOOT	1362			600	762		
*	81300945	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 24"X 24"X 8"	EACH	4			2	2		
Ī										
*	A2002820	TREE, CATALPA SPECIOSA (NORTHERN CATALPA), 2-1/2" CALIPER, BALLED AND BURLAPPED	EACH	5					5	
Ī										
*	A2002916	TREE, CELTIS OCCIDENTALIS (COMMONHACKBERRY), 2" CALIPER, BALLED AND BURLAPPED	EACH	8					8	
ļ										
*	A2005020	TREE, GYMNOCLADUS DIOICUS (KENTUCKY COFFEETREE), 2-1/2" CALIPER, BALLED AND BURLAPPED	EACH	9					9	
-										
*	A2006516	TREE, QUERCUS BICOLOR (SWAMP WHITE OAK), 2" CALIPER, BALLED AND BURLAPPED	EACH	6					6	
ļ										
*	A2008468	TREE, ULMUS AMERICANA PRINCETON (PRINCETON AMERICAN ELM), 2" CALIPER, BALLED AND BURLAPPED	EACH	5					5	
ļ										
*	K0029618	WEED CONTROL, BROADLEAF IN TURF	GALLON	0.50	0.25	0.25				
}	Z0001800	APPROACH SLAB REPAIR (PARTIAL DEPTH)	SQYD	47.0			37.0	10.0		
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\* = SPECIALTY ITEM



DESIGNED - KLM REVISED DRAWN KLM REVISED PLOT SCALE = 40.0000 '/ in. REVISED CHECKED - BMC PLOT DATE = 4/2/2024 - Ø4/Ø2/2Ø24 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES SHEET NO. 7 OF 10 SHEETS STA. TO STA.

				80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE
CODE			TOTAL	SN 016-0194	SN 016-0195	SN 016-0194	SN 016-0195	SN 016-0194
NO.	ITEM	UNIT	QUANTITY	0005	0005	0013	0013	0021
			URBAN					
Z0001899	JACK AND REMOVE EXISTING BEARINGS	EACH	74.0			22.0	52.0	
Z0001903	STRUCTURAL STEEL REMOVAL	POUND	2430				2430	
Z0001905	STRUCTURAL STEEL REPAIR	POUND	430				430	
Z0006016	BRIDGE DECK LATEX CONCRETE OVERLAY, 2 3/4 INCHES	SQYD	7166			3014	4152	
Z0012130	BRIDGE DECK SCARIFICATION 3/4"	SQYD	7528			3376	4152	
Z0012754	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	1229			709	520	
Z0012755	STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES)	SQ FT	1819			1130	689	
Z0013798	CONSTRUCTION LAYOUT	LSUM	1					1
Z0016002	DECK SLAB REPAIR (FULL DEPTH, TYPE II)	SQYD	33.0			22.0	6.0	5.0
-	r r = ";							
Z0018002	DRAINAGE SCUPPERS, DS-11	EACH	4					4
Z0018010	DRAINAGE SCUPPERS, DS-33	EACH	4				4	
Z0018051	DRAINAGE SCUPPERS TO BE ADJUSTED	EACH	2					2
Z0029090	DIAMOND GRINDING (BRIDGE SECTION)	SQ YD	6535			2865	3670	
Z0030850	TEMPORARY INFORMATION SIGNING	SQ FT	102	51	51			
Z0048665	RAILROAD PROTECTIVE LIABILITY INSURANCE	LSUM	1			0.5	0.5	
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ROADWAY

**ROADWAY** 

CONSTRUCTION TYPE CODE

BRIDGE

BRIDGE

**BIKE TRAIL** 

DESIGNED - KLM REVISED DRAWN KLM REVISED CHECKED -ВМС REVISED PLOT DATE = 4/2/2024 - Ø4/Ø2/2Ø24 REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES SHEET NO. 8 OF 10 SHEETS STA. TO STA.

					CONSTRUCTION TYPE CODE					
					ROADWAY	ROADWAY	BRIDGE	BRIDGE	BIKE TRAIL	
					80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	
	CODE NO.	ІТЕМ	UNIT	TOTAL Quantity	SN 016-0194 0005	SN 016-0195 0005	SN 016-0194 0013	SN 016-0195 0013	SN 016-0194 0021	
	NO.	HEIVI	ONII	URBAN	0003	0005	0013	0013	0021	
	Z0055800	RUSTIC RAIL FENCE	FOOT	130					130	
	Z0073200	TEMPORARY SHORING AND CRIBBING	EACH	66			38	28		
*	Z0073510	TEMPORARY TRAFFIC SIGNAL TIMING	EACH	2						
	20073510	IEWPORART IRAFFIC SIGNAL HIWING	EACH	2	1	1				
	X0325748	ACRYLIC COATING	SQYD	171			108	63		
	X0325749	FIBER WRAP	SQ FT	1531			970	561		
	X0326766	CLEAN & RESEAL RELIEF JOINT	FOOT	167			53	114		
*	X0327018	DECORATIVE SIGN POST	EACH	1					1	
	X0327018	DECORATIVE SIGNE OS	LAGII	<u> </u>					1	
*	X1700053	MONUMENT TYPE A FOUNDATION	EACH	1					1	
*	X2010350	TREE REMOVAL, ACRES (SPECIAL)	ACRE	0.25					0.25	
*										
ጥ	X2502014	SEEDING, CLASS 4A (MODIFIED)	ACRE	0.50					0.50	
	X2503000	MAINTENANCE MOWING	ACRE	5	2.5	2.5				
*	X2511630	EROSION CONTROL BLANKET (SPECIAL)	SQYD	1288					1288	
	X4400501	COMBINATION CURB AND GUTTER REMOVAL AND REPLACEMENT LESS THAN OR EQUAL TO 10 FEET	FOOT	5		5				
	V4400503	COMBINATION CURB AND GUTTER REMOVAL AND REPLACEMENT GREATER	FOOT	200	200				100	
	X4400503	THAN 10 FEET	FOOT	326	220				106	
	X4404400	PAVEMENT REMOVAL (SPECIAL)	SQYD	10			8	2		

\* = SPECIALTY ITEM



DESIGNED - KLM REVISED DRAWN - KLM REVISED -CHECKED - BMC REVISED PLOT DATE = 4/2/2024 DATE - 04/02/2024 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES SHEET NO. 9 OF 10 SHEETS STA. TO STA.

					CONSTRUCTION TIPE CODE					
					ROADWAY	ROADWAY	BRIDGE	BRIDGE	BIKE TRAIL	
					80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	80% FED / 20% STATE	
	CODE			TOTAL	SN 016-0194	SN 016-0195	SN 016-0194	SN 016-0195	SN 016-0194	
	NO.	ITEM	UNIT	QUANTITY	0005	0005	0013	0013	0021	
				URBAN						
				ONDAN						
	X5030250	BRIDGE DECK GROOVING (LONGITUDINAL)	SQYD	6769			2937	3832		
-		,	,							
_										
	X5091600	STEEL RAILING REMOVAL	F00T	789					789	
	X5230142	BRIDGE DRAINAGE SYSTEM	LSUM	1			0.5	0.5		
-										
	X6330705	RUB RAIL	FOÖT	75					75	
	X0330703	RODRAIL	1001						73	
٠										
*	X6370048	CONCRETE BARRIER BASE (SPECIAL)	FOOT	242					242	
*	X6370050	CONCRETE BARRIER WALL (SPECIAL)	FOOT	590					590	
-		, ,					_			
-								_	_	
	X6700407	ENGINEER'S FIELD OFFICE, TYPE A (D1)	CAL MO	12			4	5	3	
-										
	X7010216	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	LSUM	1	0.5	0.5				
-										
	X7830050	RAISED REFLECTIVE PAVEMENT MARKER, REFLECTOR REMOVAL	EACH	121	81	40				
	X1000000	INJOED REFERENCE AVENUENT MAINTER, REFEEDON REMOVAE	D.O.I.		51	40				
-										
	X7830052	RAISED REFLECTIVE PAVEMENT MARKER, REFLECTOR REPLACEMENT	EACH	40		40				
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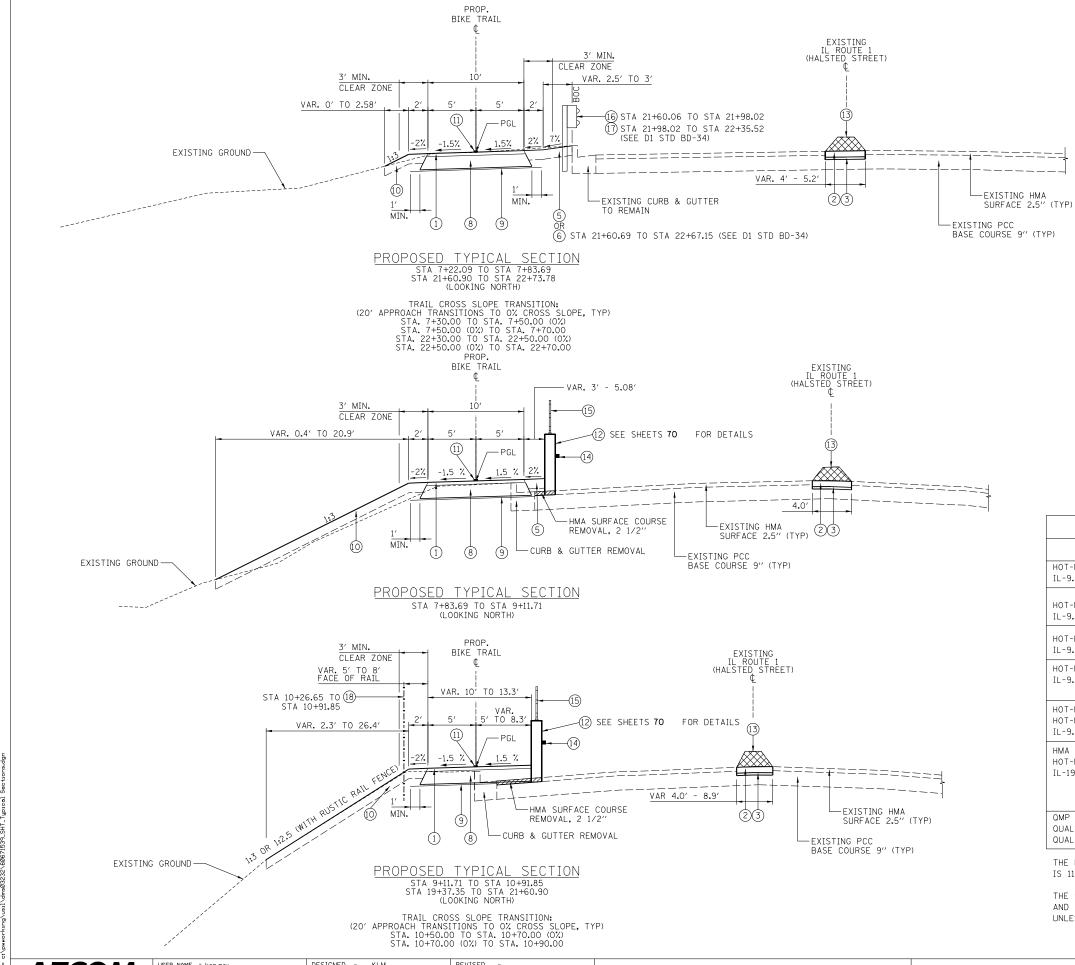
\* = SPECIALTY ITEM

DESIGNED - KLM REVISED DRAWN KLM REVISED REVISED CHECKED - BMC PLOT DATE = 4/2/2024 - 04/02/2024 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES SHEET NO. 10 OF 10 SHEETS STA. TO STA.

CONSTRUCTION TYPE CODE



## LEGEND:

- 1) HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N50, 2"
- (2) HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70, 1 3/4"
- (3) HOT-MIX ASPHALT BINDER COURSE, IL-9.5, N70, 2"
- (4) NOT USED
- (5) TOPSOIL FURNISH AND PLACE, 6", AND SEEDING CLASS 2A
- (6) HOT-MIX ASPHALT SHOULDERS, 6"
- (7) NOT USED
- (8) AGGREGATE BASE COURSE, TYPE B, 8"
- (9) GEOTECHNICAL FABRIC FOR GROUND STABILIZATION
- TOPSOIL FURNISH AND PLACE, 6", EROSION CONTROL BLANKET (SPECIAL), AND SEEDING, CLASS 4A (MODIFIED)
- (11) PAINT PAVEMENT MARKING, LINE 4" YELLOW
- (12) CONCRETE BARRIER WALL (SPECIAL)
- (13) MEDIAN REMOVAL PARTIAL DEPTH
- (14) BARRIER WALL REFLECTORS, TYPE C (CRYSTAL)
- (15) PARAPET RAILING
- (16) TRAFFIC BARRIER TERMINAL, TYPE 6
- (17) TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT
- 18 RUSTIC RAIL FENCE

WHERE UNSUITABLE MATERIAL UNDERLIES THE BIKE TRAIL, SHOULDERS AND/OR EMBANKMENT, AS DETERMINED BY THE ENGINEER, THE SUBGRADE TREATMENT WILL CONSIST OF EXCAVATION OF SUCH UNSUITABLE MATERIAL TO A DEPTH 12" BELOW AGGREGATE BASE COURSE AND PLACEMENT OF 12" OF AGGREGATE SUBGRADE IMPROVEMENT AND A GEOTECHNICAL FABRIC FOR GROUND STABILIZATION.

HMA MIXTURE REQUIREMENTS											
MIXTURE TYPE	AIR VOIDS @ Ndes	USAGE	QMP								
HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N50, 2"	4% @ 50 GYR	BIKE TRAIL SURFACE	QC/QA								
HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70, 1 3/4"	1	HALSTED ST. PAVEMENT RESTORATION AT THE MEDIAN	QC/QA								
HOT-MIX ASPHALT BINDER COURSE, IL-9.5, N70, 2"	4% @ 70 GYR	HALSTED ST. PAVEMENT RESTORATION AT THE MEDIAN	QC/QA								
HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70, 1 3/4"	1	HALSTED ST. PCC APPROACH PAVEMENT SURFACE	QC/QA								
HOT-MIX ASPHALT REPLACEMENT OVER PATCHES: HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70, 2 1/2"	4% @ 70 GYR	HALSTED ST. SURFACE OVER CLASS B PATCHES	QC/QA								
HMA SHOULDERS 6": HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N70, 6"		HALSTED ST. SHOULDER TREATMENT AT TRAFFIC BARRIER TERMINAL TYPE 1 (SPECIAL) TANGENT	QC/QA								

QMP DESIGNATIONS:

QUALITY CONTROL/QUALITY ASSURANCE (QC/QA),

QUALITY CONTROL FOR PERFORMANCE (QCP), OR PAY FOR PERFORMANCE (PFP)

THE UNIT WEIGHT USED TO CALCULATE ALL HMA SURFACE MIXTURE QUANTITIES IS 112 LBS/SQYD/IN.

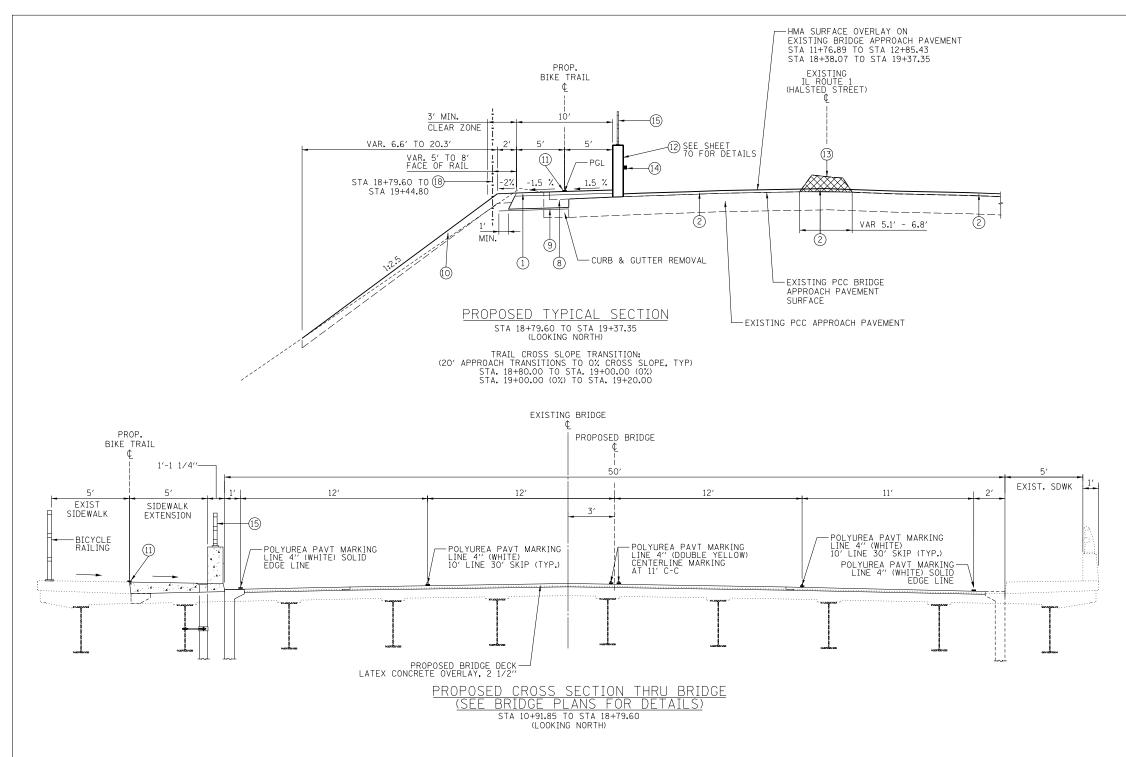
THE "AC TYPE" FOR POLYMERIZED HMA MIXES SHALL BE "SBS/SBR PG 76-22" AND FOR NON-POLYMERIZED HMA THE "AC TYPE" SHALL BE "PG 64-22" UNLESS UNLESS MODIFIED BY RECLAIMED MATERIALS SPECIFICATIONS.

AECOM

USER NAME = ken.moy	DESIGNED - KLM	REVISED -
	DRAWN - KLM	REVISED -
PLOT SCALE = 100.0000 '/ in.	CHECKED - BMC	REVISED -
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -

HALSTE	D ST				RAIL (S <sup>-</sup> Typical		RE NO. 016-0194) DNS	
	SHEET	NO 1	٥E	2	SHEETS	STA	TO STA	

F.A.U. RTE.		SEC	TION			COUNTY	TOTAL SHEETS	SHEE.
3730	1	5-0013	1-01-BR			COOK	109	14
						CONTRACT	NO.	52W48
FED. R	DAD DIST	NO. 1	TLL INDIS	FED.	AID	PROJECT		



	EARTHWORK SCHEDULE													
				EARTH EXCAVATION	EARTH EXCAVATION ADJUSTED FOR SHRINKAGE (15%)	EMBANKMENT	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-) FURNISHED EXCAVATION	UNSUITABLE EXCAVATION (REMOVAL & DISPOSAL OF UNSUITABLE MATERIAL)	UNSUITABLE MATERIAL ADJUSTED FOR SHRINKAGE (25%)	TOPSOIL FURNISH AND PLACE, 6″	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-) (UNSUITABLE MATERIAL)			
	STATION	TO	STATION	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD			
1	07+22.09		10+91.83	7	6	186	-180	216	162	101	61			
2	18+79.60		22+73,78	23	20	214	-194	239	179	104	75			
			TOTAL:	30	26	400	-375	455	341	205	136			
				•										

## LEGEND:

- 1) HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N50, 2"
- 2) HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70, 1 3/4"
- (3) HOT-MIX ASPHALT BINDER COURSE, IL-9.5, N70, 2"
- (4) NOT USED
- (5) TOPSOIL FURNISH AND PLACE, 6", AND SEEDING CLASS 2A
- (6) HOT-MIX ASPHALT SHOULDERS, 6"
- (7) NOT USED
- (8) AGGREGATE BASE COURSE, TYPE B, 8"
- (9) GEOTECHNICAL FABRIC FOR GROUND STABILIZATION
- (1) TOPSOIL FURNISH AND PLACE, 6", EROSION CONTROL BLANKET (SPECIAL), AND SEEDING, CLASS 4A (MODIFIED)
- (1) PAINT PAVEMENT MARKING, LINE 4" YELLOW
- (12) CONCRETE BARRIER WALL (SPECIAL)
- 13 MEDIAN REMOVAL PARTIAL DEPTH
- (14) BARRIER WALL REFLECTORS, TYPE C (CRYSTAL)
- (15) PARAPET RAILING
- (16) TRAFFIC BARRIER TERMINAL, TYPE 6
- 17 TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT
- (18) RUSTIC RAIL FENCE

### NOTE:

WHERE UNSUITABLE MATERIAL UNDERLIES THE BIKE TRAIL, SHOULDERS AND/OR EMBANKMENT, AS DETERMINED BY THE ENGINEER, THE SUBGRADE TREATMENT WILL CONSIST OF EXCAVATION OF SUCH UNSUITABLE MATERIAL TO A DEPTH 12" BELOW AGGREGATE BASE COURSE AND PLACEMENT OF 12" OF AGGREGATE SUBGRADE IMPROVEMENT AND A GEOTECHNICAL FABRIC FOR GROUND STABILIZATION.

<b>AECOM</b>
303 East Wacker Drive, Suite 1400, Chicago, IL 60601
T 1-312-373-7700 F 1-312-373-6800

USER NAME = ken.moy	DESIGNED - KLM	REVISED -
	DRAWN - KLM	REVISED -
PLOT SCALE = 100.0000 '/ in.	CHECKED - BMC	REVISED -
PLOT DATE = 4/2/2024	DATE - Ø4/Ø2/2Ø24	REVISED -

HALST	ED ST				•	TRUCTUR L SECTIO	E NO. 016–0194) NS
	CHEET	NO 1	0.	^	CHEETC	CTA	TO CT4

F.A.U. RTE.			SE	C.	TION			COUNTY	TOTAL SHEETS	SHEET NO.
3730		15	5-00	13	1-01-BR			COOK	109	15
								CONTRACT	NO.	62W48
FED. R	OAD	DIST.	NO.	1	ILLINOIS	FED.	AID	PROJECT		

PROP. CURVE-1 PI STA. = 7+98.56 Δ = 4° 51′ 50″ (RT) D = 11° 27′ 33″

PROP. CURVE-2 PI STA. = 10+47.69 Δ = 5° 06′ 46″ (LT) D = 6° 21′ 58″ R = 900.00' T = 40.18'L = 80.31' E = 0.90'

PROP. CURVE-3 PI STA. = 20+04.67 Δ = 7° 50′ 11′′ (LT) D = 2° 54′ 36′′ R = 1.968.90' T = 134.86'

PROP. CURVE-4 PI STA. = 21+61.40 Δ = 5° 06′ 23″ (RT) D = 11° 27′ 33″

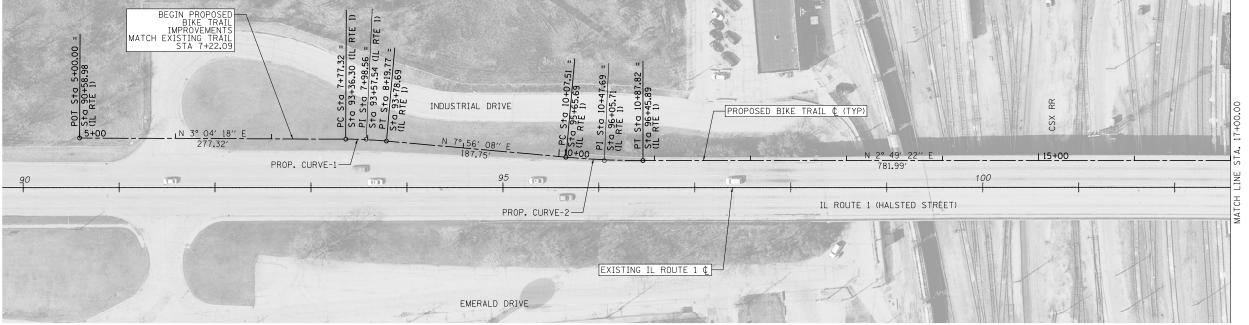
R = 500.00' T = 22.30' L = 44.56'

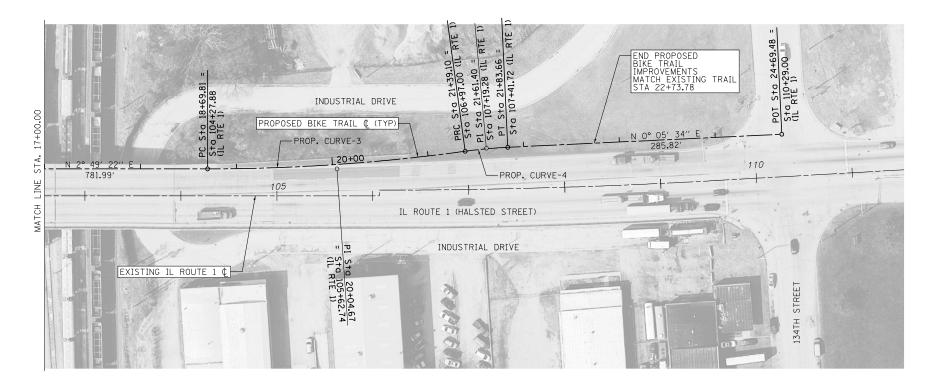
R = 500.00'T = 21.24' L = 42.45' E = 0.45'P.C. STA. = 7+77.32 P.T. STA. = 8+19.77

P.C. STA. = 10+07.51 P.T. STA. = 10+87.82

I = 134.86° L = 269.29° E = 4.61° P.C. STA. = 18+69.81 P.R.C. STA. = 21+39.10

E = 0.50' P.R.C. STA. = 21+39.10 P.T. STA. = 21+83.66





	ALIGNMENT COORDINATES PROPOSED BIKE TRAIL							
POINT	DESCRIPTION	STATION	NORTHING	EASTING				
POT		5+00.00	1,814,501.08	1,173,315.37				
PC	CURVE-1	7+77.32	1,814,778.01	1,173,330.23				
PI	CURVE-1	7+98.56	1,814,799.21	1,173,331.37				
PT	CURVE-1	8+19.77	1,814,820.24	1,173,334.30				
PC	CURVE-2	10+07.51	1,815,006.19	1,173,360.23				
PI	CURVE-2	10+47.69	1,815,045.99	1,173,365.77				
PT	CURVE-2	10+87.82	1,815,086.12	1,173,367.75				
PC	CURVE-3	18+69.81	1,815,867.16	1,173,406.26				
PI	CURVE-3	20+04.67	1,816,001.85	1,173,412.91				
PRC	CURVE-3	21+39.10	1,816,136.19	1,173,401.12				
PRC	CURVE-4	21+39.10	1,816,136.19	1,173,401.12				
PI	CURVE-4	21+61.40	1,816,158.40	1,173,399.17				
PT	CURVE-4	21+83.66	1,816,180.70	1,173,399.17				
POT		24+69.48	1,816,466.52	1,173,399.67				



**→**©→Z

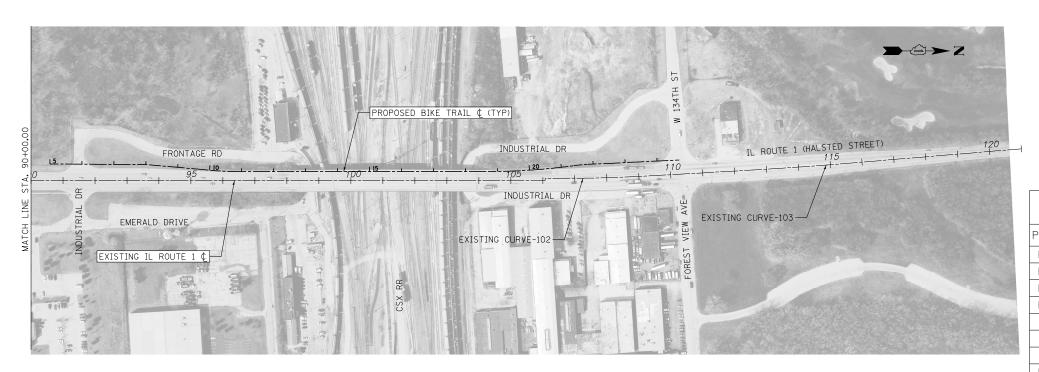


JSER NAME = ken.moy	DESIGNED - KLM	REVISED -	
	DRAWN - KLM	REVISED -	
PLOT SCALE = 100.0000 '/ in.	CHECKED - BMC	REVISED -	
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -	

STATE	: OF	ILLINOIS	
DEPARTMENT	0F	TRANSPORTATION	

							F.A.U. RTE.	SECTION	
STRUCTURE NO. 016-0194				3730 15-0013					
SINUCIONE NO. 010-0134									
SHEET	NO.	1	OF	1	SHEETS	STA.	TO STA.	FED. R	OAD DIST. NO. 1 ILLIN

	F.A.U. SECTION				COUNTY	TOTAL SHEETS	SHEET NO.	
	3730	3730 15-00131-01-BR				COOK	109	16
					$\Box$	CONTRACT	NO. 6	2W48
ı	FED. R	OAD DIST. NO. 1	ILLINOIS	FED.	AII	PROJECT		



ALIGNMENT COORDINATES EXISTING HALSTED ST (IL ROUTE 1)						
POINT	DESCRIPTION	STATION	NORTHING	EASTING		
POT		49+00.00	1,810,340.29	1,173,373.00		
POT		57+00.00	1,811,140.25	1,173,365.39		
POT		57+00.00	1,811,140.20	1,173,360.59		
POT		78+02.44	1,813,242.55	1,173,340.59		
PC	CURVE-101	78+02.44	1,813,242.50	1,173,334.59		
PΙ	CURVE-101	83+08.28	1,813,748.45	1,173,329.78		
PT	CURVE-101	88+13.96	1,814,253.82	1,173,354.74		
POT		100+00.00	1,815,438.42	1,173,413.24		
POT		106+06.92	1,816,044.60	1,173,443.18		
PC	CURVE-102	106+06.92	1,816,044.89	1,173,437.19		
PΙ	CURVE-102	107+83.03	1,816,220.76	1,173,445.87		
РТ	CURVE-102	109+58.99	1,816,396.84	1,173,444.48		
POT		110+00.00	1,816,437.85	1,173,444.15		
PC	CURVE-103	112+57.83	1,816,695.67	1,173,442.11		
ΡI	CURVE-103	114+08.19	1,816,846.04	1,173,440.92		
РТ	CURVE-103	115+59.57	1,816,996.32	1,173,435.79		
POT		121+00.00	1,817,536.43	1,173,417.33		
OTE THE ADOVE DATA IS FOR INFORMATION ONLY						

NOTE: THE ABOVE DATA IS FOR INFORMATION ONLY. THE CONTRACTOR MUST FIELD VERIFY.

SHEET NO.

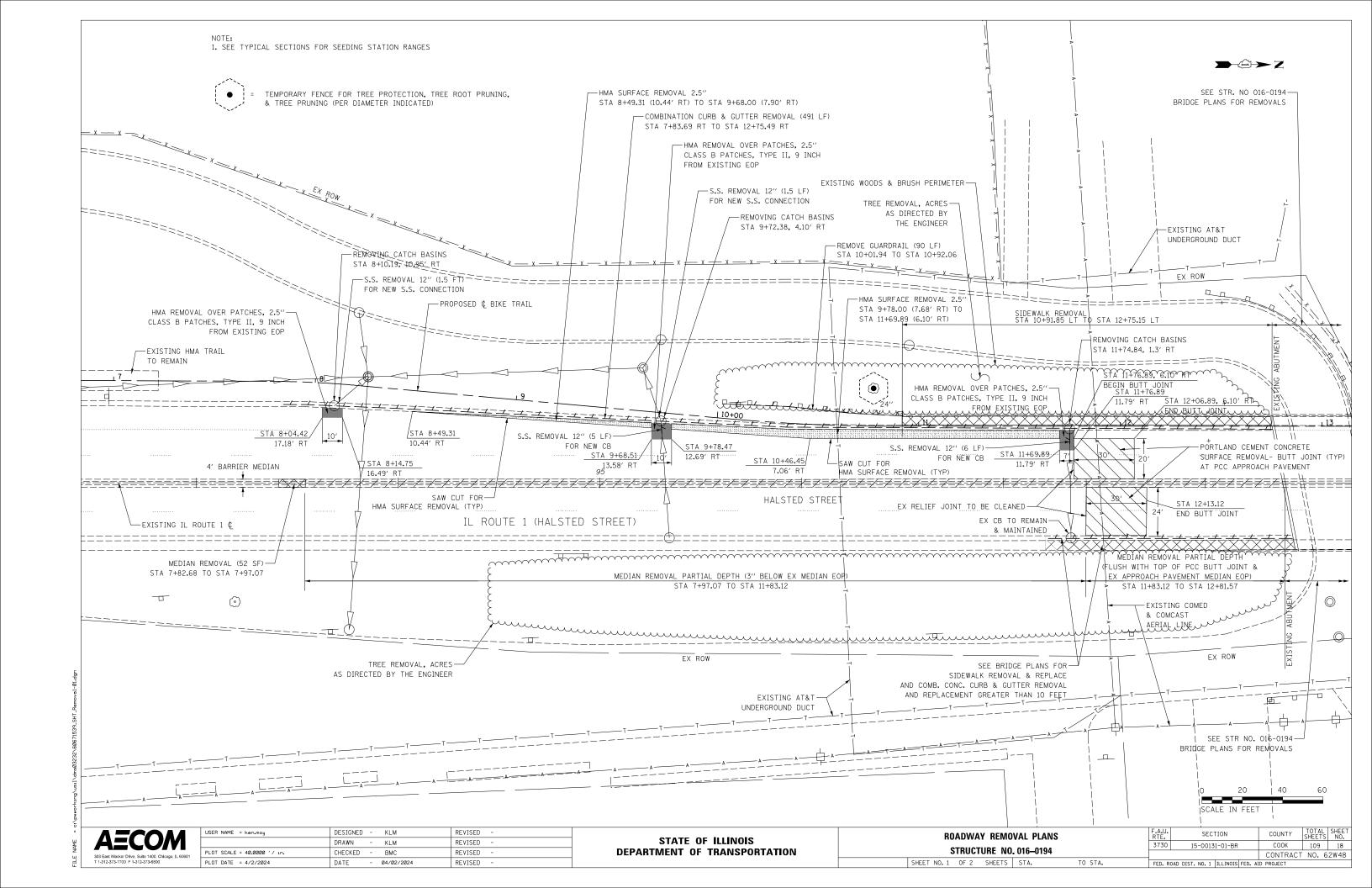


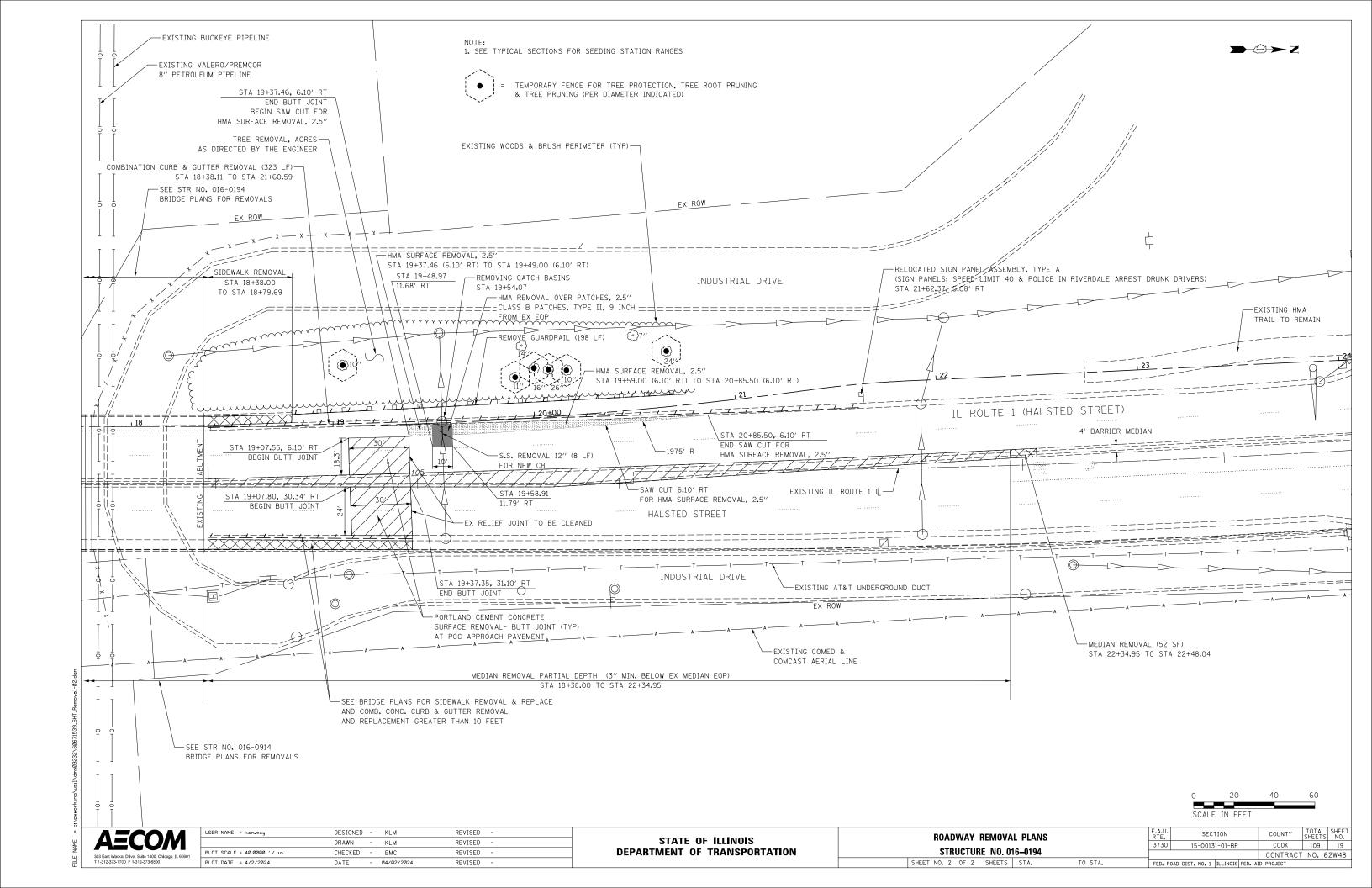
<b>AECOM</b>
303 East Wacker Drive, Suite 1400, Chicago, IL 60601

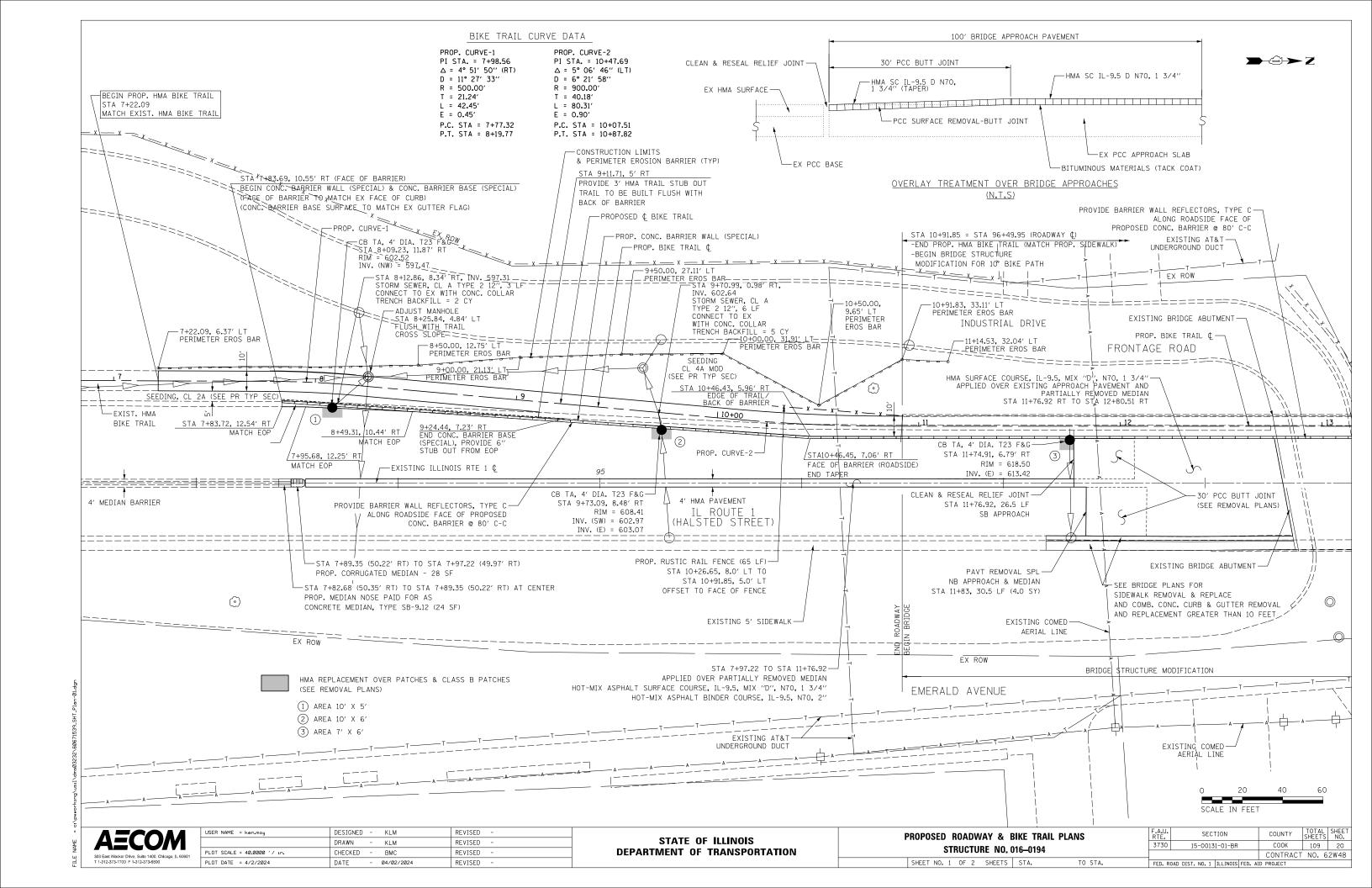
JSER NAME = ken.moy	DESIGNED - KLM	REVISED -	
	DRAWN - KLM	REVISED -	
PLOT SCALE = 300.0000 '/ in.	CHECKED - BMC	REVISED -	
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -	

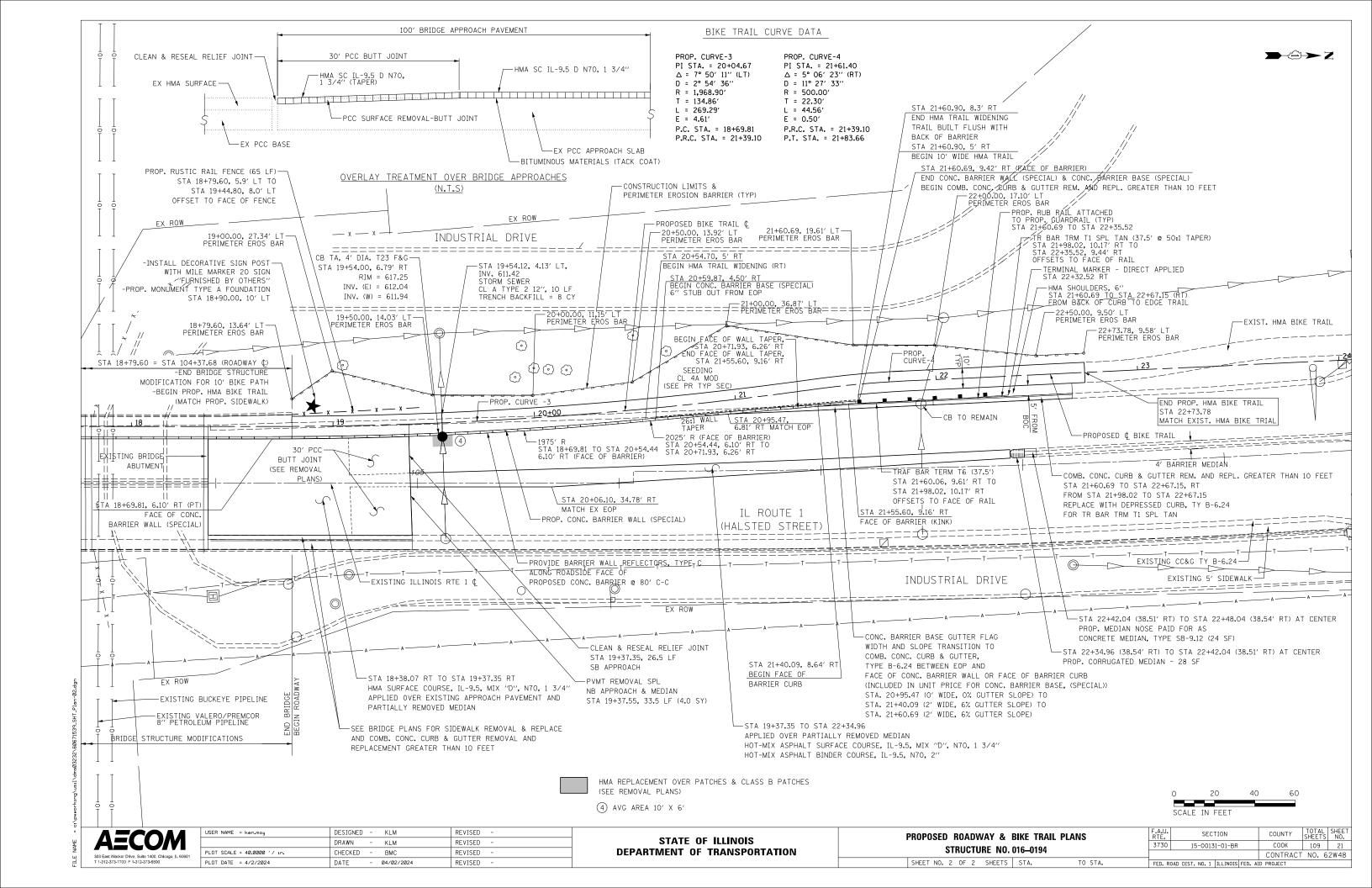
STAT	E OF	- ILLINOIS	
DEPARTMENT	<b>OF</b>	TRANSPORTATION	

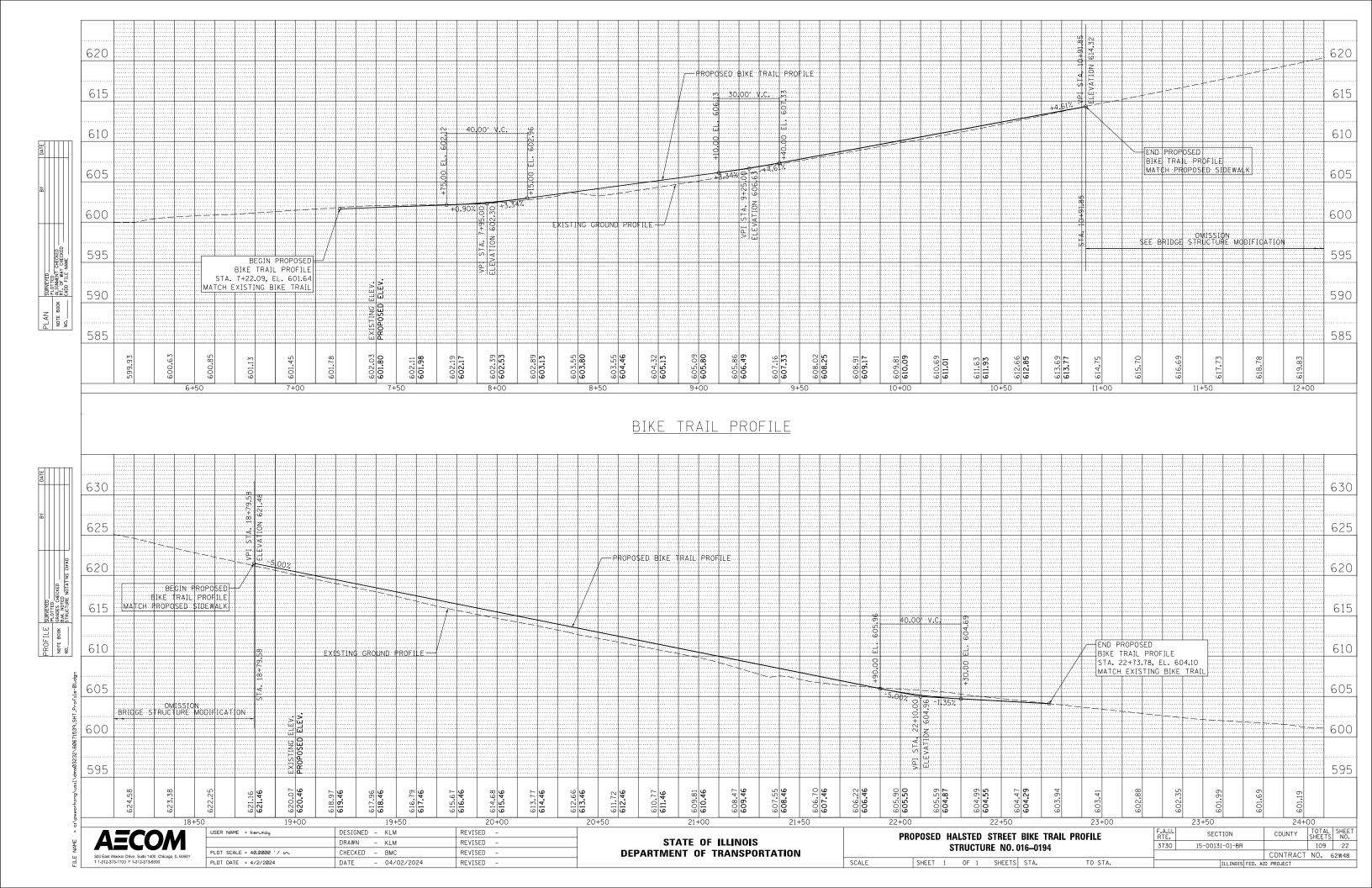
ALIGNMENT AND TIES Ex halsted st (jl rt 1)		F.A.U. RTE.	SEC <sup>-</sup>	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.
		3730	15-0013	1-01-BR		COOK	109	17
,						CONTRACT	NO. 6	2W48
NO. 1 OF 1 SHEETS STA.	TO STA.	FED. R	OAD DIST. NO. 1	ILLINOIS FE	D. AII	PROJECT		











- 3. THE CONTRACTOR SHALL VERIFY LOCATION OF ALL BUILDING ACCESS, COORDINATE WITH BUILDING OWNERS AND LOCAL AUTHORITIES AND PROVIDE FULL ACCESS TO BUSINESSES OR PROPERTIES DURING THEIR NORMAL WORKING HOURS IN ACCORDANCE WITH ADA AND APPLICABLE CODE REQUIREMENTS.
- 4. THE CONTRACTOR SHALL MAINTAIN TRAFFIC ON ALL STREETS EXCEPT WHERE NOTED ON THE PLANS AND PROHIBIT PARKING WITHIN FIFTY (50) FEET OF THE CONSTRUCTION AREA AT ALL TIMES.
- 5. PROVIDE CONTINUOUS TEMPORARY ACCESS TO ALL SIDE STREETS, ALLEYS, DRIVEWAYS, AND PARKING LOTS UNLESS SPECIFICALLY IDENTIFIED ON THE PLANS FOR TEMPORARY CLOSURE. LOTS WITH MORE THAN ONE DRIVEWAY MUST BE STAGED TO KEEP AT LEAST ONE DRIVEWAY OPEN AT ALL TIMES.
- 6. MAINTAIN ACCESS TO FIRE HYDRANTS, BUILDING STANDPIPES AND OTHER EMERGENCY FACILITIES WITHIN THE CONSTRUCTION ZONE.
- 7. SIGNS W21-1 AND W20-7 SHALL BE TAKEN DOWN OR COVERED WHEN THE WORKERS ARE NOT PRESENT FOR MORE THAN 1 HOUR.
- 8. ALL EXISTING PAVEMENT MARKINGS IN CONFLICT WITH THE PAVEMENT MARKING TAPE, TYPE IV (AND/OR MODIFIED URETHANE) USED FOR STAGING SHALL BE REMOVED. THIS WORK SHALL BE PAID FOR AS "PAVEMENT MARKING REMOVAL-WATER BLASTING".
- 9. ALL EXISTING RAISED REFLECTIVE PAVEMENT MARKER REFLECTORS LOCATED WITHIN TEMPORARY LANE CLOSURE TAPERS, LANE SHIFT TAPERS OR IN LOCATIONS THAT CONFLICT WITH THE TEMPORARY PAVEMENT MARKING TAPE USED FOR STAGING SHALL BE REMOVED. THE EXISTING RAISED REFLECTIVE PAVEMENT MARKER REFLECTORS THAT WERE REMOVED SHALL BE RESTORED AFTER THE COMPLETION OF THE STAGING UNLESS OTHERWISE SHOWN ON THE PLANS OR DIRECTED BY THE ENGINEER.
- 10. PAVEMENT MARKING TAPE, TYPE IV SHOWN ON THE PLANS FOR ANY CONSTRUCTION STAGE THAT THE CONTRACTOR PROPOSES TO EXTEND OVER THE WINTER PERIOD SHALL MEAN MODIFIED URETHANE PAVEMENT MARKING AND WILL BE PAID FOR THEIR RESPECTIVE CONTRACT UNIT PRICE.
- 11. THE "ROAD CONSTRUCTION AHEAD" SIGNS SHALL REMAIN INSTALLED UNTIL THE COMPLETION OF THE PROJECT OR WHEN NO ROADWAY HAZARDS REMAIN WITHIN THE WORK ZONE.
- 12. CHANGEABLE MESSAGE SIGNS SHALL BE INSTALLED TWO WEEKS PRIOR TO ALL TRAFFIC STAGE CHANGES ON EACH APPROACH OF THE EFFECTED ROADWAY TO WARN MOTORISTS OF THE UPCOMING EVENT. THE SIGN MESSAGES SHALL BE REVISED TWO WEEKS THEREAFTER WITH MESSAGES WARNING TRAFFIC OF POTENTIAL TRAFFIC DELAYS, QUEUING AND/OR WITH MESSAGES NOTIFYING TRAFFIC TO USE ALTERNATE ROUTES. THE SIGN LOCATIONS AND MESSAGES SHALL BE DETERMINED BY THE ENGINEER.
- 13. TEMPORARY CONCRETE BARRIER SHALL BE INSTALLED IN ACCORDANCE WITH SECTION 704 OF THE IDOT STANDARD SPECIFICATIONS. ALL TEMPORARY CONCRETE BARRIER APPROACH AND DEPARTING END UNITS SHALL BE ANCHORED TO THE PAVEMENT USING SIX ANCHOR PINS AS SHOWN IN IDOT STANDARD 704001. PINNING OF ADDITIONAL BARRIER UNITS WITH THREE ANCHOR PINS ON THE TRAFFIC SIDE HOLES WITHIN THE INSTALLATION SHALL BE REQUIRED WHEN EQUIPMENT, VEHICLES, MATERIALS, FIXED OBJECTS, OR A DROP-OFF IS LOCATED WITHIN 24" BEHIND THE BARRIER. THE 24" OF CLEAR PAVEMENT MEASUREMENT SHALL BE FROM THE BASE OF THE NON-TRAFFIC SIDE OF THE BARRIER. TRAFFIC SIDE PINNED BARRIER SHALL HAVE A MINIMUM OF 6" OF CLEAR PAVEMENT BEHIND THE BARRIER, WHERE BOTH PINNED AND UNPINNED BARRIER UNITS ARE USED IN A CONTINUOUS INSTALLATION, A TRANSITION SHALL BE PROVIDED BETWEEN THEM, THE TRANSITION FROM PINNED TO UNPINNED BARRIER SHALL CONSIST OF TWO ANCHOR PINS INSTALLED IN THE END HOLES ON THE TRAFFIC SIDE OF THE FIRST BARRIER BEYOND THE PINNED SECTION AND ONE ANCHOR PIN INSTALLED IN THE MIDDLE HOLE OF THE TRAFFIC SIDE OF THE SECOND BARRIER BEYOND THE PINNED SECTION. THE THIRD BARRIER BEYOND THE PINNED SECTION SHALL THEN BE UNPINNED.
- 14. ONE SIDE OF THE BRIDGE SIDEWALK SHALL BE KEPT ACCESSIBLE TO PEDESTRIANS AT ALL TIMES.

### SEQUENCE OF CONSTRUCTION NOTES:

- THE WORK AT EACH BRIDGE SHALL BE CONDUCTED IN SEPARATE STAGES AS INDICATED IN THE SUGGESTED MAINTENANCE OF TRAFFIC PLANS.
- 2. WORK ASSOCIATED WITH BRIDGE SN 016-0194 BETWEEN 134TH STREET (NORTH LIMIT) AND 138TH STREET (SOUTH LIMIT) SHALL BE COMPLETED IN STAGES 1A AND 1B BEFORE STARTING THE WORK ASSOCIATED WITH BRIDGE SN 016-0195 (STAGE 2).
- 3. WORK ASSOCIATED WITH BRIDGE SN 016-0195 BETWEEN 138TH STREET (NORTH LIMIT) AND 142ND STREET (SOUTH LIMIT) SHALL BE COMPLETED IN STAGES 2A AND 2B.

### TEMPORARY TRAFFIC SIGNAL TIMING REQUIRMENTS:

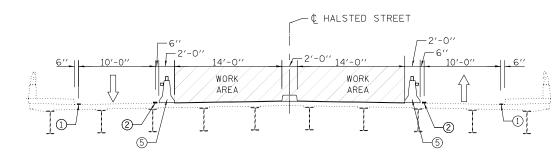
TEMPORARY TRAFFIC SIGNAL TIMING SHALL BE PROVIDED AT THE BELOW INTERSECTIONS ALONG ILLINOIS ROUTE 1 (HALSTED STREET).

INTERSECTION	Z0073510 TEMPORARY TRAFFIC SIGNAL TIMING
	EACH
IL 1 (HALSTED ST) & 134TH ST	1
IL 1 (HALSTED ST) & 138TH ST	1
TOTAL:	2

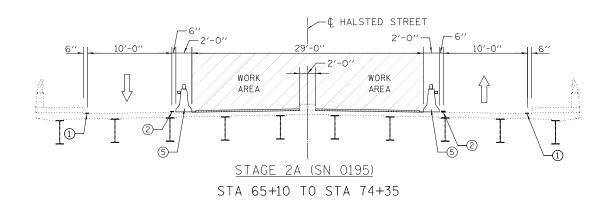
USER NAME = ken.moy	DESIGNED - KLM	REVISED -
	DRAWN - KLM	REVISED -
PLOT SCALE = 100.0000 '/ in.	CHECKED - BMC	REVISED -
PLOT DATE = 5/10/2024	DATE - 04/02/2024	REVISED -

SCALE: NONE

# STAGE 1A (SN 0194) & 2A (SN 0195) TYPICAL ROADWAY SECTION

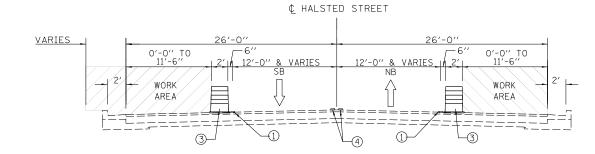


STAGE 1A (SN 0194)
STA 93+44 (STA 7+86 TRAIL) TO STA 108+11 (STA 22+52 TRAIL)



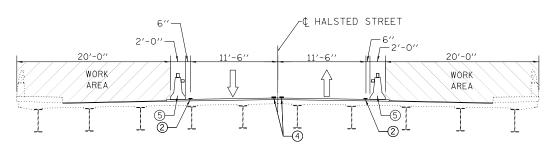
# LEGEND:

- 1 TEMPORARY PAVEMENT MARKING LINE 4" TYPE IV TAPE WHITE
- 2 TEMPORARY PAVEMENT MARKING LINE 4" TYPE IV TAPE YELLOW
- 3 DRUMS/TYPE II BARRACADES OR TEMPORARY CONCRETE BARRIER WALL
- 4 TEMPORARY PAVEMENT MARKING LINE 4" TYPE IV TAPE (DOUBLE YELLOW)
- (5) TEMPORARY CONC. BARRIER & TYPE C REFLECTORS PER STANDARD 704001

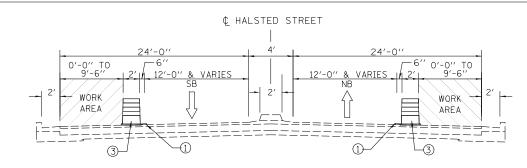


# STAGE 1B (SN 0194) TYPICAL ROADWAY SECTION

STA 92+12 TO STA 97+33 (STA 11+67 TRAIL) STA 108+35 (STA 22+77 TRAIL) TO STA 115+20

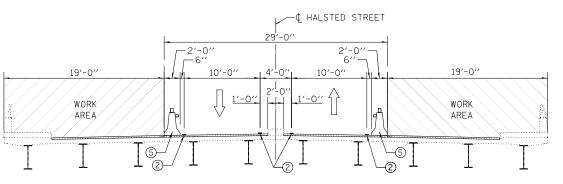


STAGE 1B (SN 0194)
STA STA 97+33 (STA 11+67 TRAIL) TO STA 108+35 (STA 22+77 TRAIL)



# STAGE 2B (SN 0195) TYPICAL ROADWAY SECTION

STA 61+10 TO STA 65+10 STA 74+35 TO STA 78+38



<u>STAGE 2B (SN 0195)</u> STA 65+10 TO STA 74+35

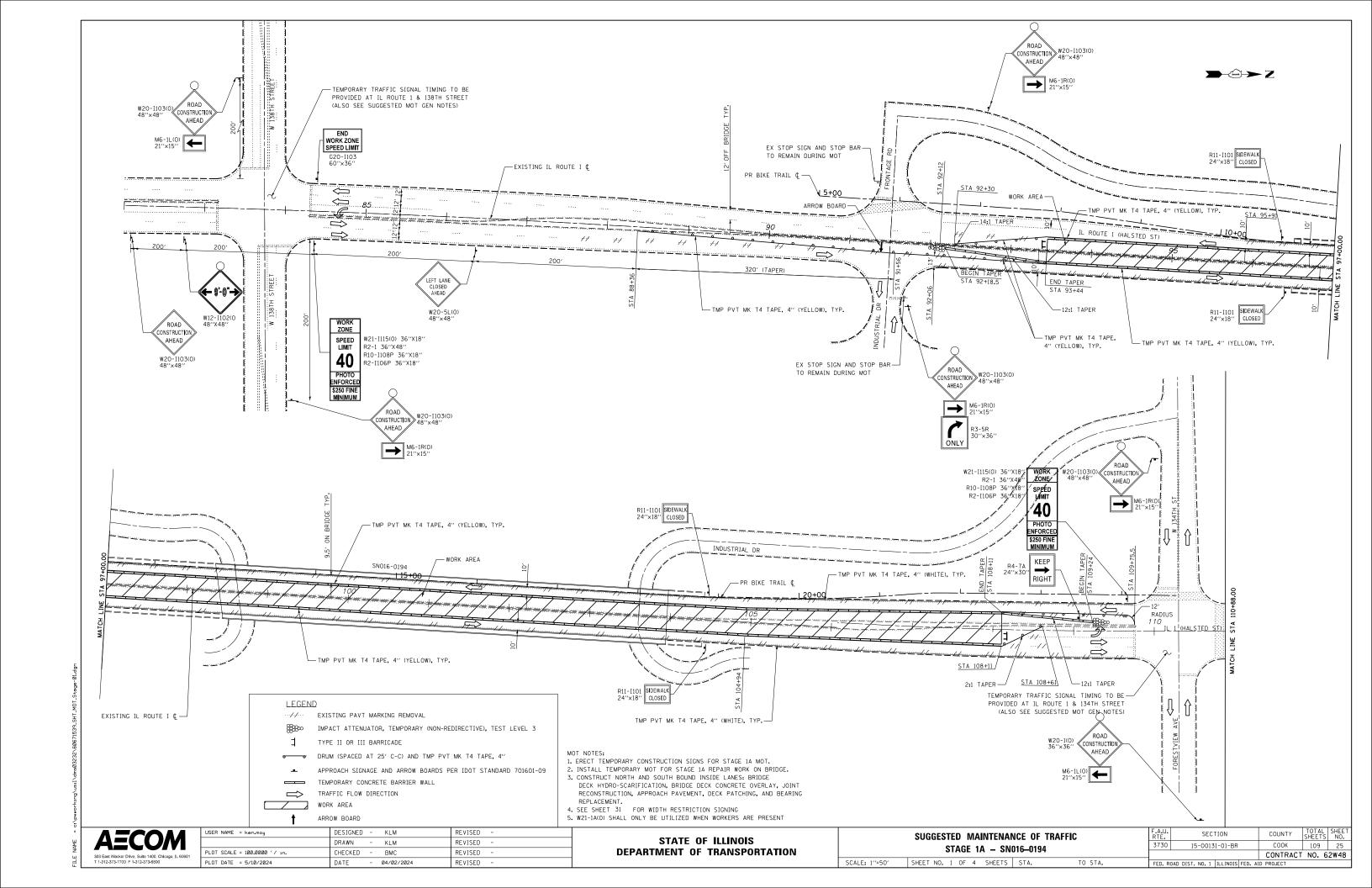


USER NAME = ken.moy	DESIGNED - KLM	REVISED -
	DRAWN - KLM	REVISED -
PLOT SCALE = 100.0000 '/ in.	CHECKED - BMC	REVISED -
PLOT DATE = 5/10/2024	DATE - 04/02/2024	REVISED -

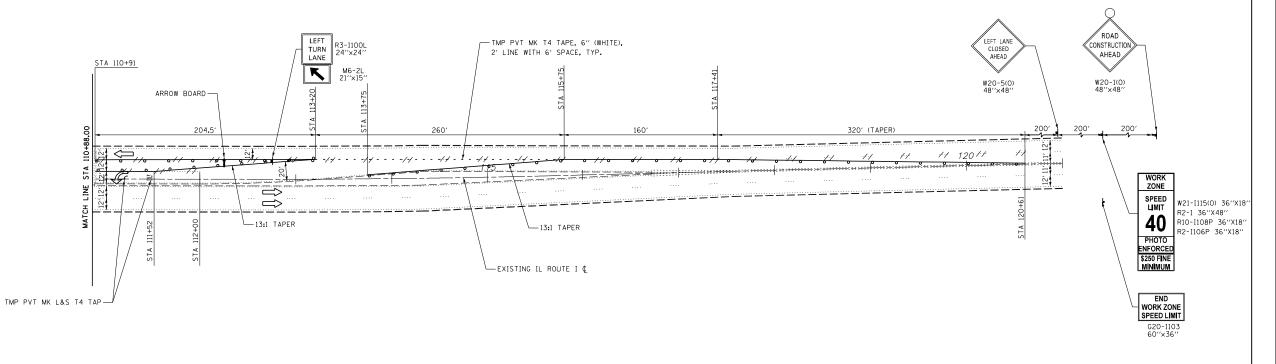
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SCALE: NTS

	SUGGESTED MAINTENANCE OF TRAFFIC				F.A.U RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.		
STAGING SECTIONS					3730	15-00131-01-BR	соок	109	24		
	STAULING SECTIONS								CONTRACT	「 NO. €	2W48
	SHEET	1	OF 1	SHEETS	STA.	TO STA.		ILLINOIS FED. AI	D PROJECT		







LEGEND

..././.. EXISTING PAVT MARKING REMOVAL

IMPACT ATTENUATOR, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3

TYPE II OR III BARRICADE

DRUM (SPACED AT 25' C-C) AND TMP PVT MK T4 TAPE, 4"

APPROACH SIGNAGE AND ARROW BOARDS PER IDOT STANDARD 701601-09

TEMPORARY CONCRETE BARRIER WALL 

TRAFFIC FLOW DIRECTION

 $\Longrightarrow$ WORK AREA ARROW BOARD

MOT NOTES:

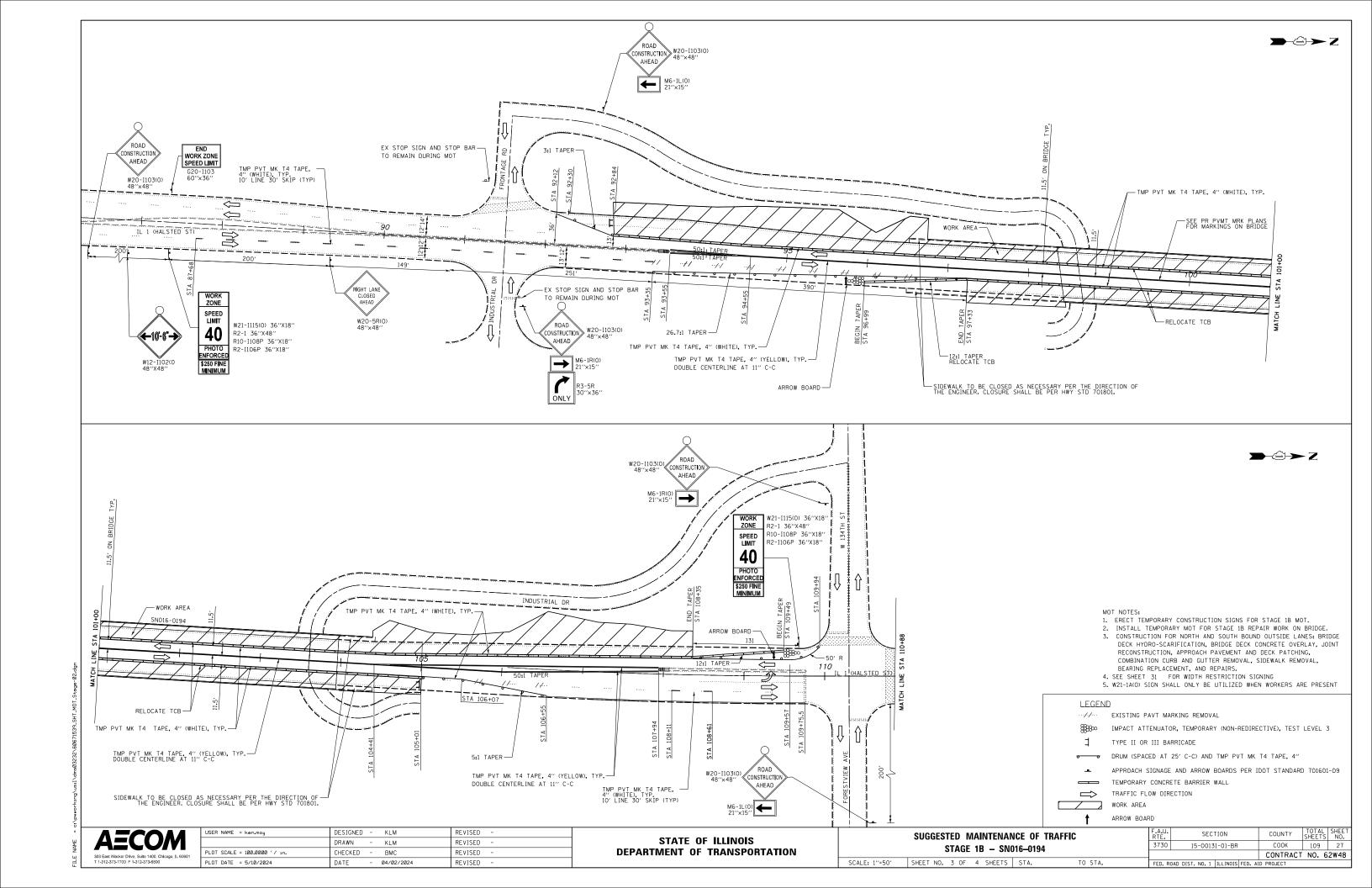
1. ERECT TEMPORARY CONSTRUCTION SIGNS FOR STAGE 1A MOT.

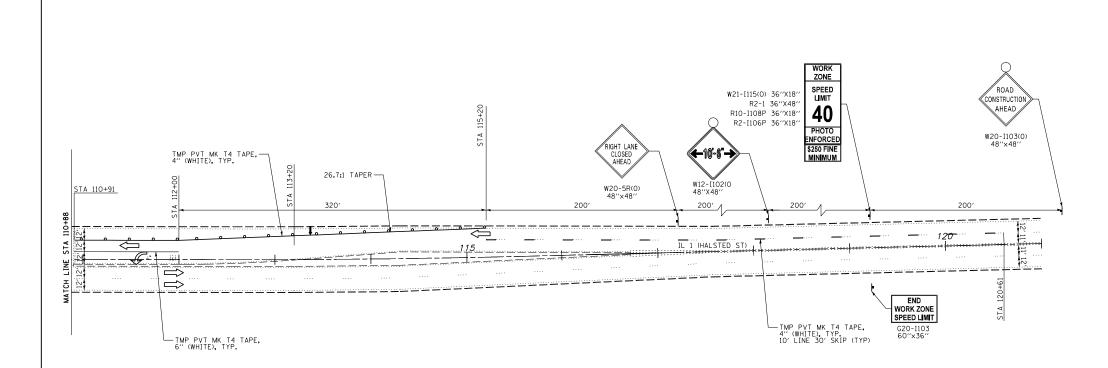
2. INSTALL TEMPORARY MOT FOR STAGE 1A REPAIR WORK ON BRIDGE.

3. CONSTRUCT NORTH AND SOUTH BOUND INSIDE LANGES BRIDGE DECK HYDRO-SCARIFICATION, BRIDGE DECK CONCRETE OVERLAY, JOINT RECONSTRUCTION, APPROACH PAVEMENT, DECK PATCHING, AND BEARING REPLACEMENT.
4. SEE SHEET 31 FOR WIDTH RESTRICTION SIGNING
5. W21-1A(O) SHALL ONLY BE UTILIZED WHEN WORKERS ARE PRESENT

USER NAME = ken.moy	DESIGNED	-	KLM	REVISED -
	DRAWN	-	KLM	REVISED -
PLOT SCALE = 100.0000 '/ in.	CHECKED	-	ВМС	REVISED -
PLOT DATE = 5/10/2024	DATE	-	04/02/2024	REVISED -

SUGGESTED MAINTENANCE OF TRAFFIC					SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STACE	E 1A – SNO	116_010/		3730	15-00131-01-BR	СООК	109	26
STAGE	. IA - SIN	J10-0134				CONTRACT	T NO. 6	2W48
SCALE: 1"=50" SHEET NO. 2 OF	4 SHEETS	STA.	TO STA.	FED. RC	DAD DIST. NO. 1 ILLINOIS FED.	AID PROJECT		





LEGEND

 $\Rightarrow$ 

.../-/... EXISTING PAVT MARKING REMOVAL

IMPACT ATTENUATOR, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3 TYPE II OR III BARRICADE

DRUM (SPACED AT 25' C-C) AND TMP PVT MK T4 TAPE, 4"

APPROACH SIGNAGE AND ARROW BOARDS PER IDOT STANDARD 701601-09 TEMPORARY CONCRETE BARRIER WALL

TRAFFIC FLOW DIRECTION

WORK AREA

ARROW BOARD

- 1. ERECT TEMPORARY CONSTRUCTION SIGNS FOR STAGE 1B MOT.
  2. INSTALL TEMPORARY MOT FOR STAGE 1B REPAIR WORK ON BRIDGE.
  3. CONSTRUCTION FOR NORTH AND SOUTH BOUND OUTSIDE LANES: BRIDGE
- DECK HYDRO-SCARIFICATION, BRIDGE DECK CONCRETE OVERLAY, JOINT RECONSTRUCTION, APPROACH PAVEMENT AND DECK PATCHING, COMBINATION CURB AND GUTTER REMOVAL, SIDEWALK REMOVAL, BEARING REPLACEMENT, AND REPAIRS.
- 4. SEE SHEET 31 FOR WIDTH RESTRICTION SIGNING
- 5. W21-1A(O) SIGN SHALL ONLY BE UTILIZED WHEN WORKERS ARE PRESENT

**→**©→Z



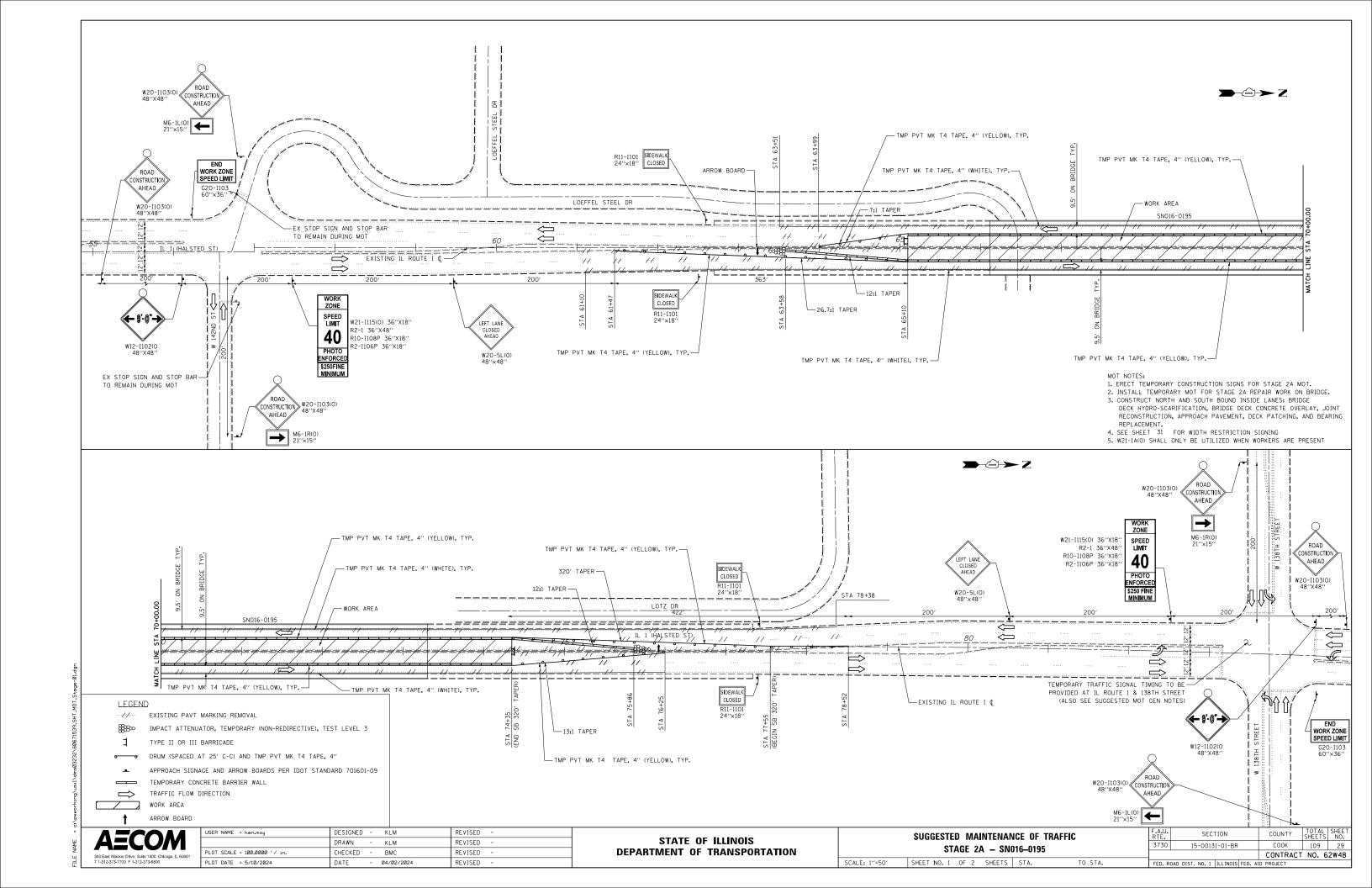
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	DRAWN -	KLM	REVISED -
PLOT SCALE = 100.0000 '/ in.	CHECKED -	ВМС	REVISED -
PLOT DATE = 5/10/2024	DATE -	04/02/2024	REVISED -

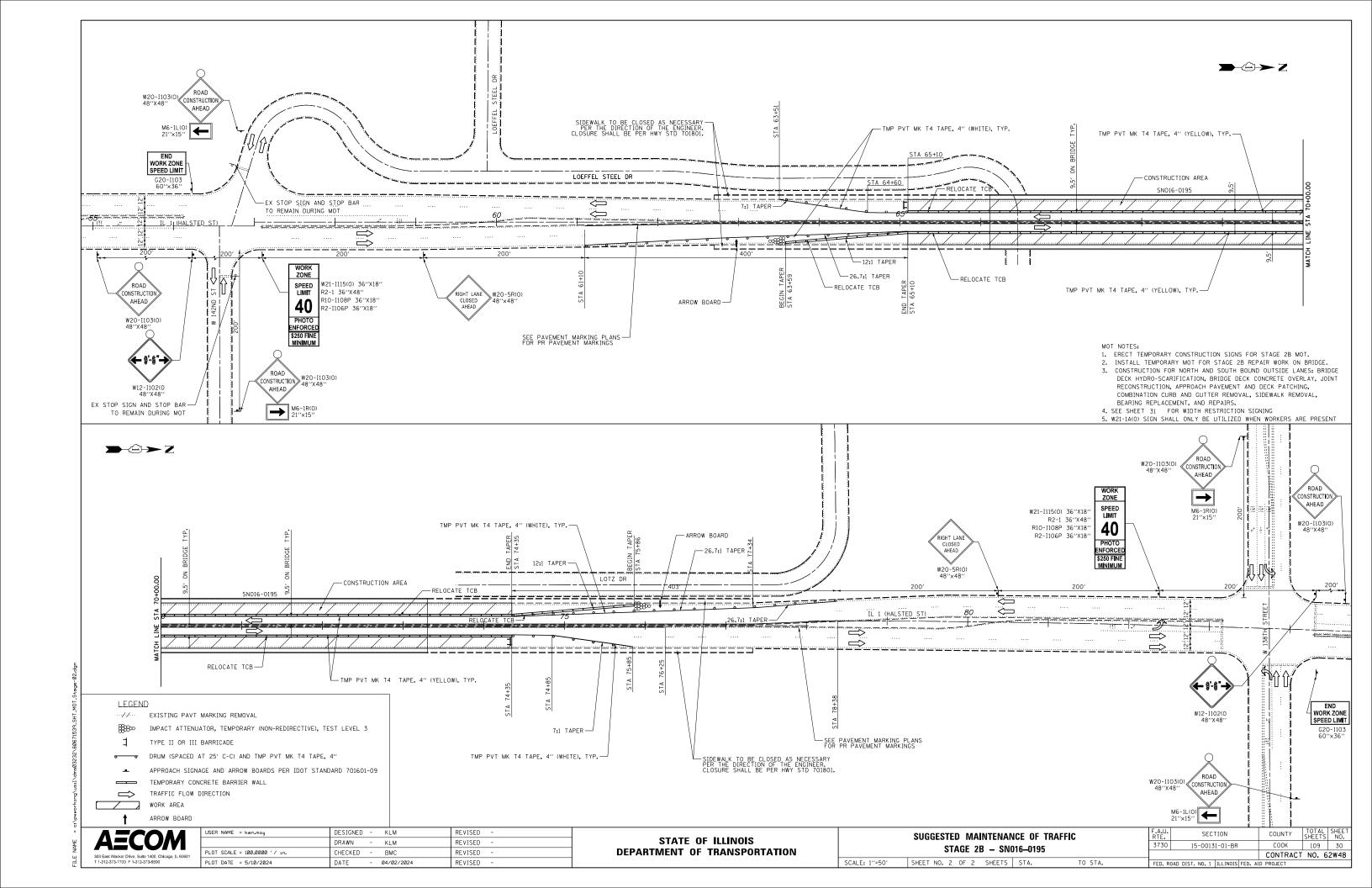
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

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STAGE 1B - SN016	3730	15-00131-01-BR	
STAGE ID - SIVIIO	-0134		
ET NO. 4 OF 4 CUEETS C	T. TO CT.		

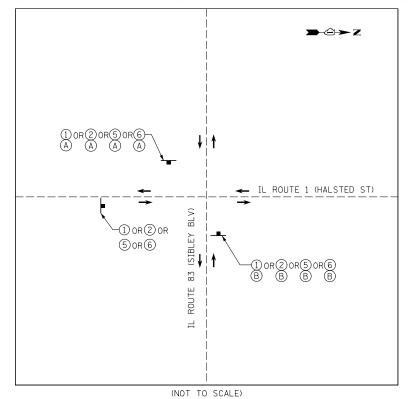
COUNTY TOTAL SHEET NO.

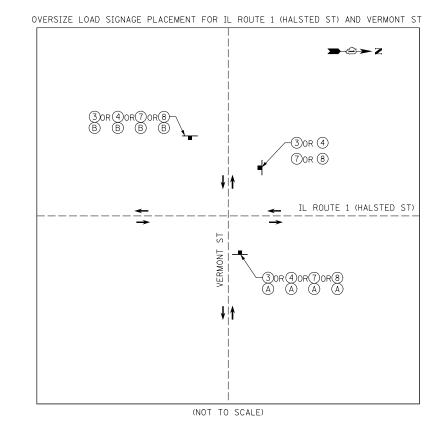
COOK 109 28 SUG CONTRACT NO. 62W48 SCALE: 1"=50" SHEET NO. 4 OF 4 SHEETS STA. FED. ROAD DIST. NO. 1 | ILLINOIS | FED. AID PROJECT





OVERSIZE LOAD SIGNAGE PLACEMENT FOR IL ROUTE 1 (HALSTED ST) AND IL ROUTE 83 (SIBLEY BLV)





STAGE 1A (SN 0194) MAX WIDTH **1.50** MILES AHEAD W12-I103(0)-48

STAGE 1B (SN 0194) MAX WIDTH 10'-6" **1.50** MILES AHEAD W12-I103(0)-48

STAGE 1A (SN 0194) MAX WIDTH 9'-0" **1.25** MILES AHEAD W12-I103(0)-48

STAGE 2A (SN 0195)

MAX WIDTH

9'-0"

**1.25** MILES

AHEAD

W12-I103(0)-48

STAGE 1B (SN 0194) MAX WIDTH 10'-6" 4 **1.25** MILES AHEAD W12-I103(0)-48

STAGE 2B (SN 0195) MAX WIDTH 9'-6" **1.25** MILES

> AHEAD W12-I103(0)-48

STAGE 2A (SN 0195) MAX WIDTH 9'-0" **1.00** MILES AHEAD

W12-I103(0)-48

6 **1.00** MILES

W12-I103(0)-48

MAX WIDTH AHEAD

STAGE 2B (SN 0195)

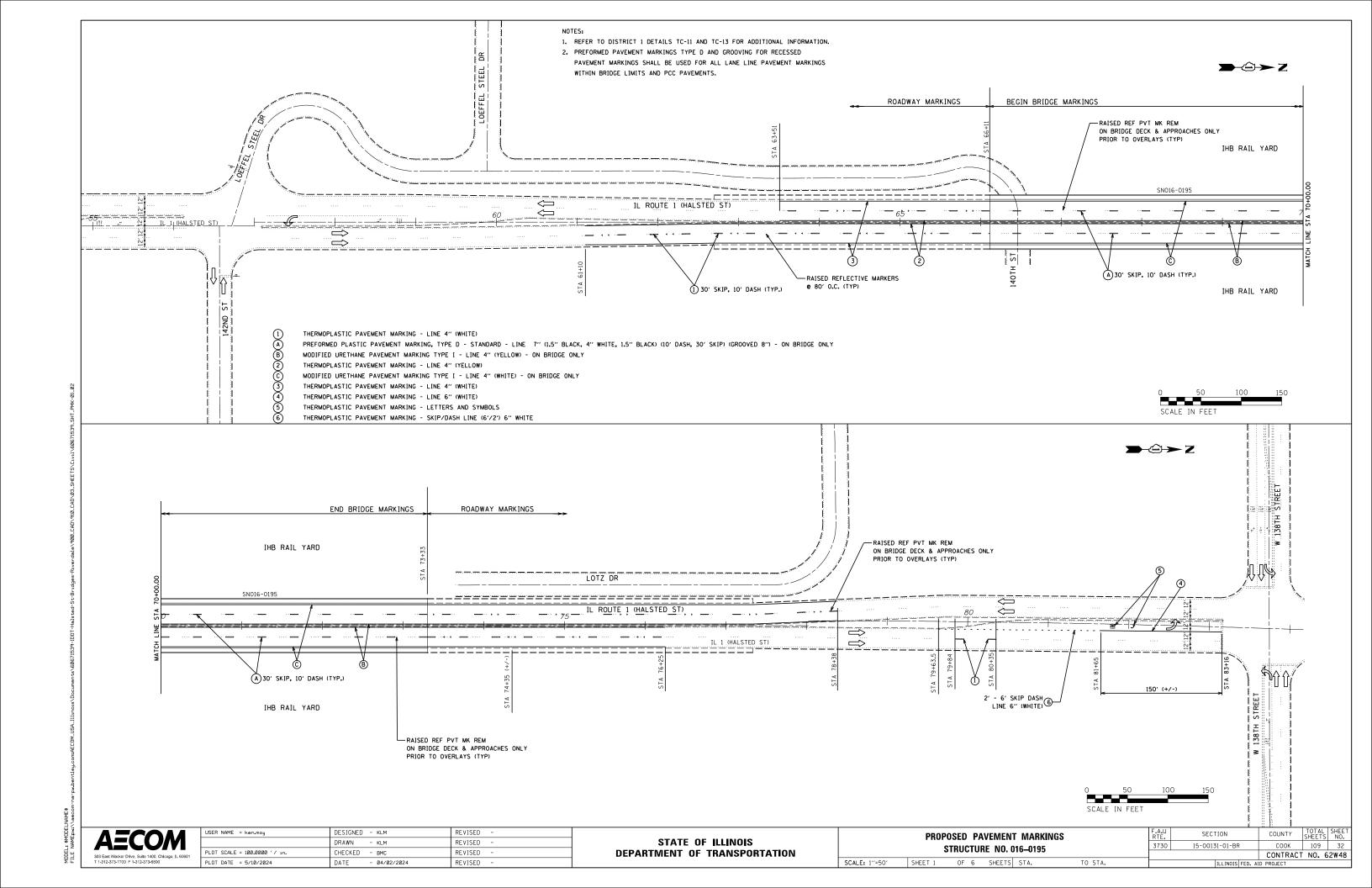
(7)

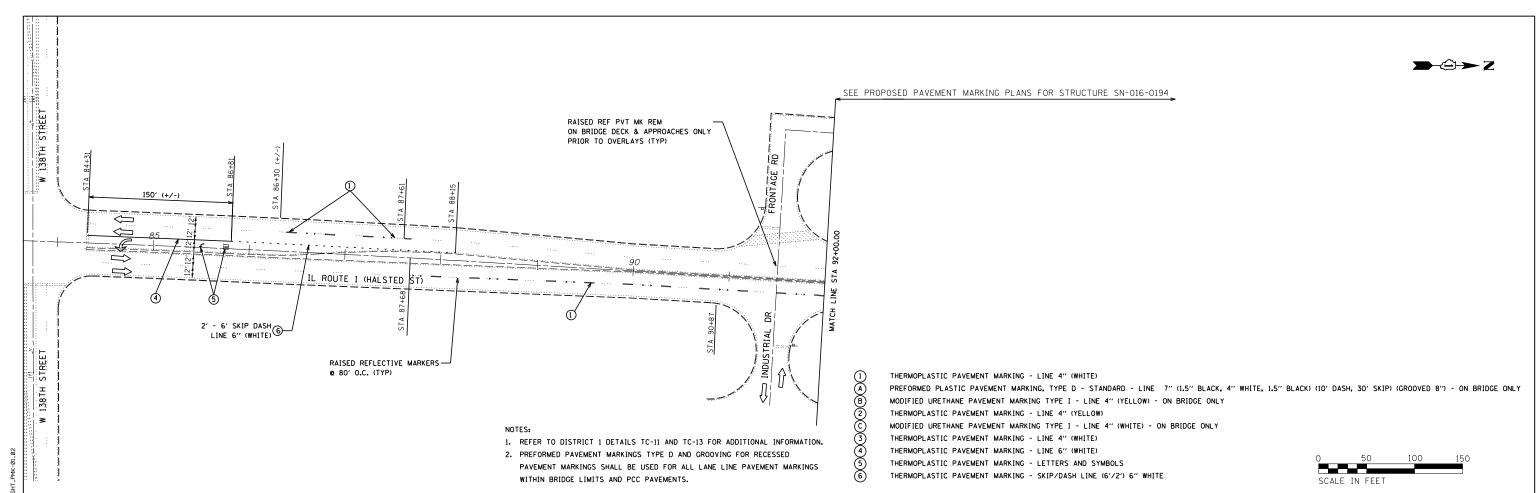
1. OVERSIDED LOAD SIGNAGE SHALL BE ERECTED FOR EACH RESPECTIVE CONSTRUCTION STAGE ONLY.
2. SIGNAGE SHALL BE PLACED 200 FEET IN ADVANCE OF THE INTERSECTION.
3. STAGE 1 = WORK ASSOCIATED WITH SN 016-0194
4. STAGE 2 = WORK ASSOCIATED WITH SN 016-0195
5. SEE SUGGESTED STAGES OF CONSTRUCTION AND TRACETOR OF THE STAGES OF CONSTRUCTION AND TRACETOR OF THE STAGES OF STAGE

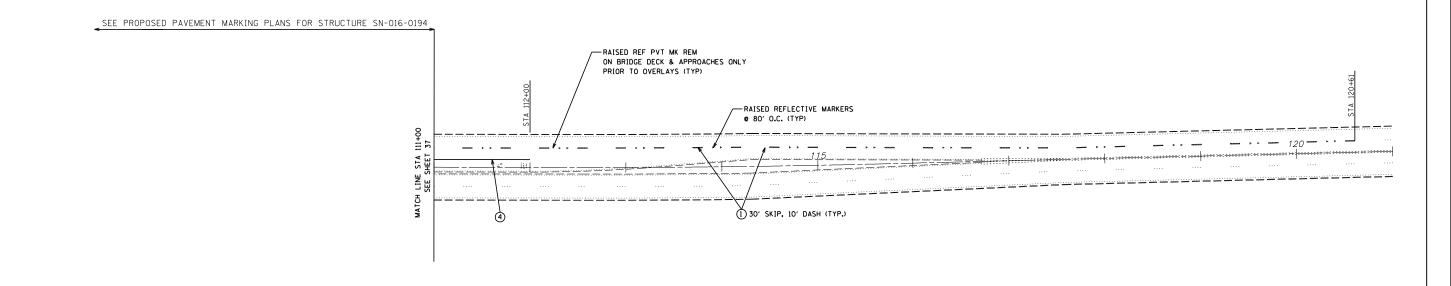
TRAFFIC CONTROL PLANS FOR STAGING DETAILS.

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	DRAWN -		KLM	REVISED	-
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PLOT DATE = 5/10/2024	DATE -	- 1	04/02/2024	REVISED	-

	F.A.U. RTE.	SECTIO	ON	COUNTY	TOTAL SHEETS	SHEET NO.
OVERSIZED LOAD SIGNAGE PLACEMENT DIAGRAM	3730	15-00131-	01-BR	соок	109	31
				CONTRAC	T NO. 6	2W48
SCALE: 1" = NONE   SHEET NO. 1 OF 1 SHEETS   STA. TO STA.	FED. R	OAD DIST. NO. 1 IL	LINOIS FED. A	ID PROJECT		





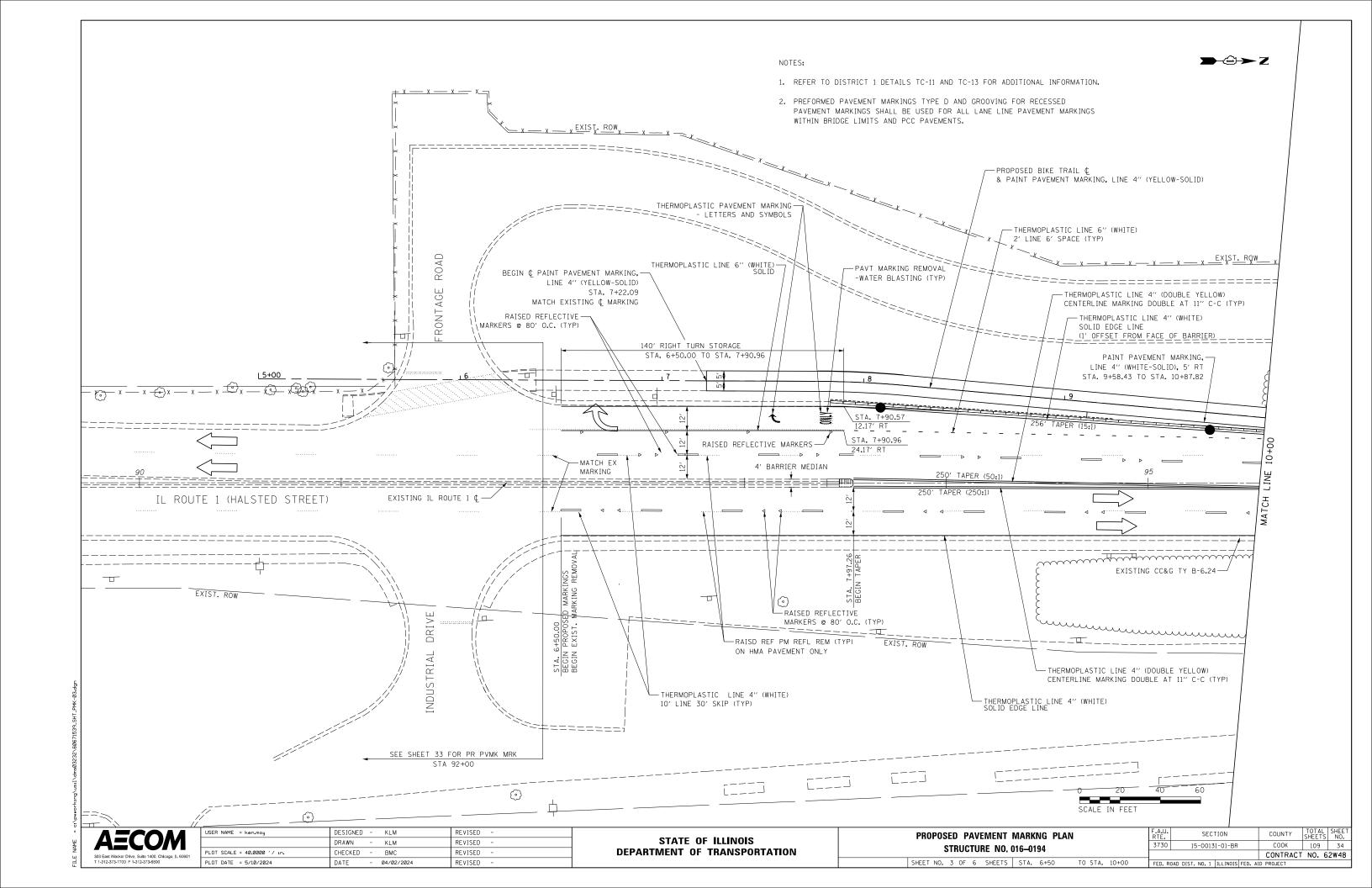


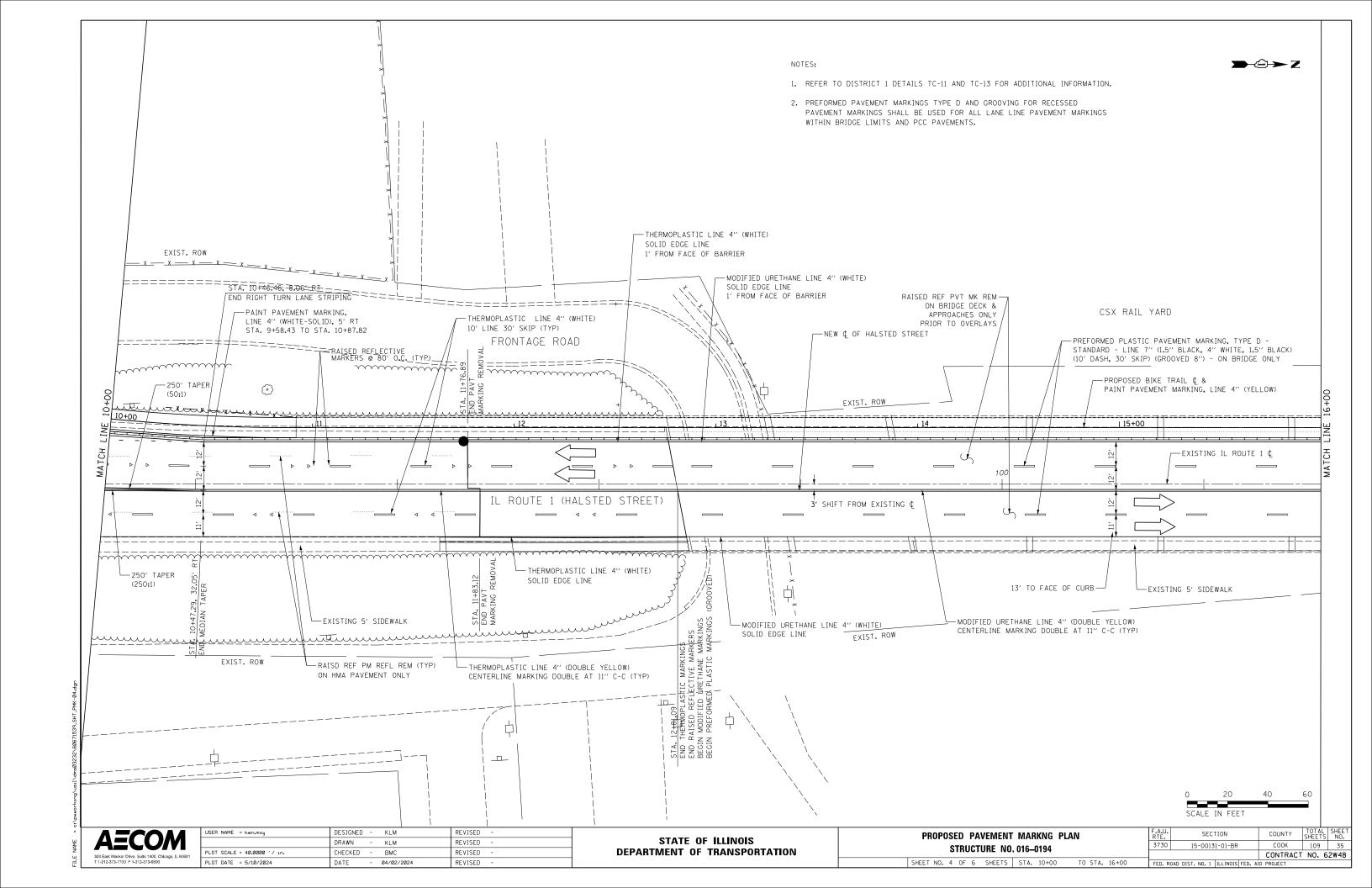
**AECOM**303 East Wacker Drive, Saite 1400, Chicago, IL 60601
11-312-373-7700, F1-312-373-8800

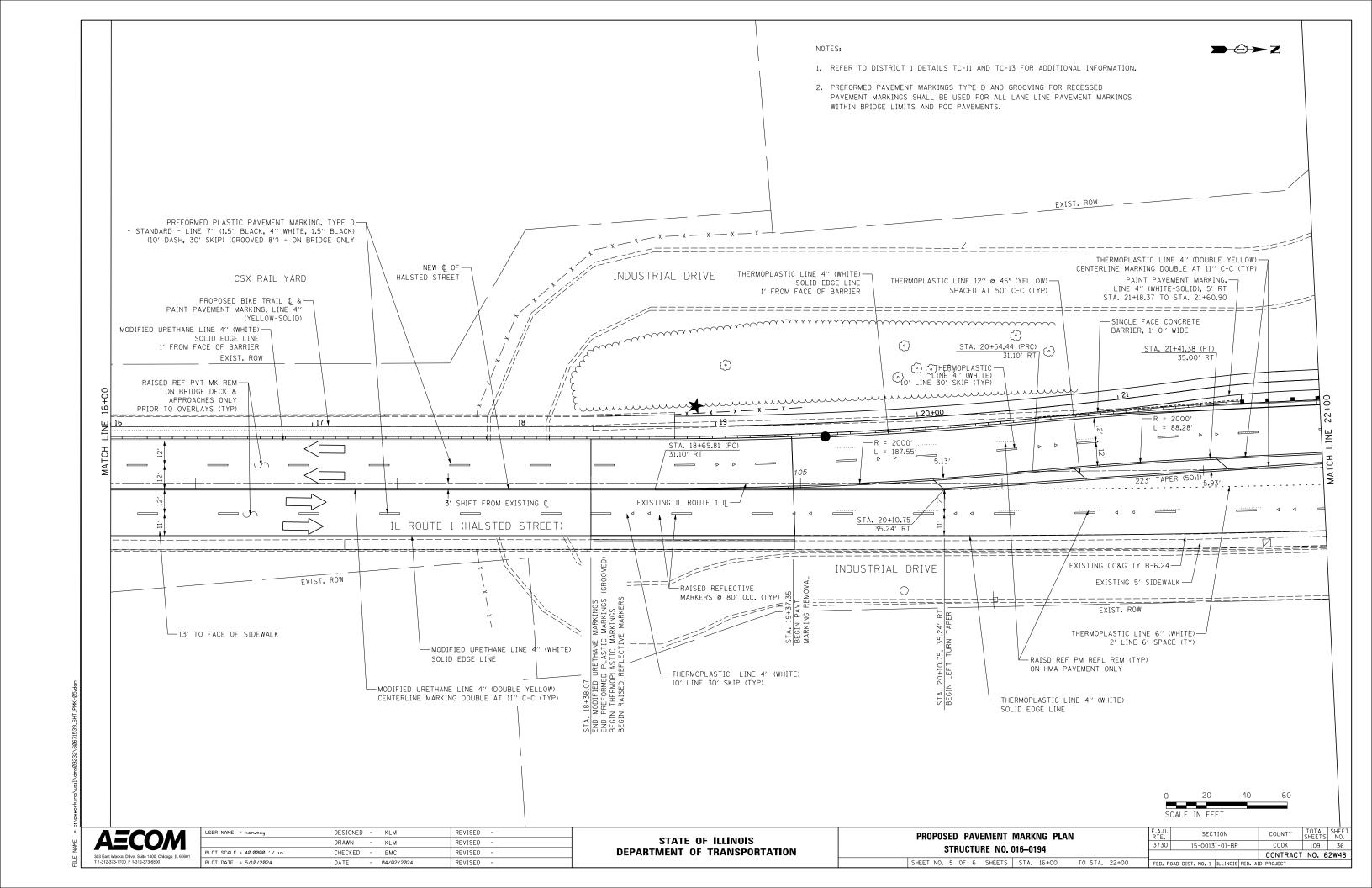
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	DRAWN - KLM	REVISED -	
PLOT SCALE = 100.0000 ' / in.	CHECKED - BMC	REVISED -	
PLOT DATE = 5/10/2024	DATE - 04/02/2024	REVISED -	

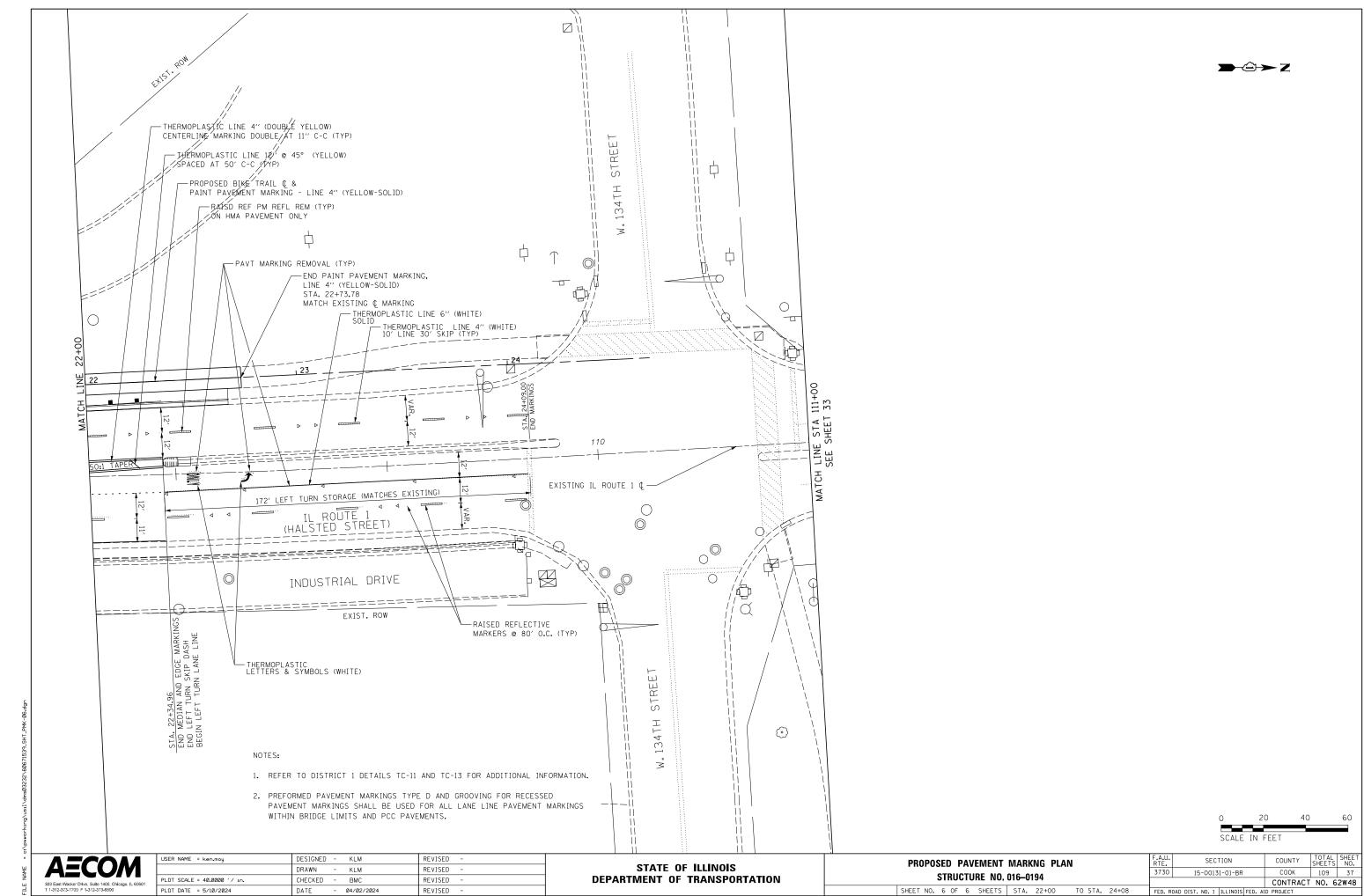
STATE 0	OF ILLINOIS
DEPARTMENT OF	F TRANSPORTATION

PROPOSED PAVEMENT MARKINGS					F.A.U RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
	STRUCTURE NO. 016-0194					15-00131-01-BF	?	COOK	109	33
	31110	CIOIL NO.	710-0134					CONTRACT	T NO. 6	2W48
SCALE: 1"=50"	SHEET 2 OF	6 SHEETS	STA.	TO STA.		ILLINOIS FED. AID PROJECT				









Existing Structure:
Existing Structure No. 016-0195 was built in 1927 as SBI 1 Section 3262-VB.
In 1979, under SBI-1 Section 3262-VB, the superstructure was replaced.
Existing Structure is a twelve span reinforced concrete slab on steel beams between closed abutments, 722'-0" bk. to bk. abutments, 68'-0" out to out deck. Traffic to be staged during construction.

# <u>DESIGN SPECIFICATIONS</u>

(NEW CONSTRUCTION)
2002 AASHTO Standar Specifications
for Highway Bridges

# DESIGN STRESSES

#### FIELD UNITS (NEW CONSTRUCTION)

f'c = 3,500 psi

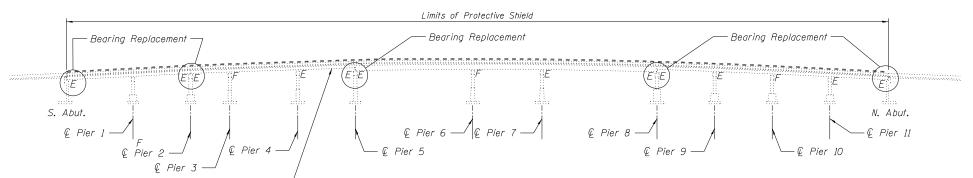
f'c = 4,000 psi (Superstructure & Appr. Slab)

fy = 60,000 psi (Reinforcement)

fy = 50,000 psi (Structural Steel)

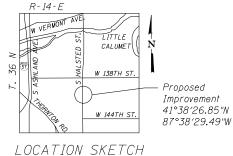
# SCOPE OF WORK

- 1. Remove existing expansion joints at the abutments and Piers 2, 5 and 8 and install new preformed joint strip seals.
- 2. Scarify existing bridge deck surface  $\frac{3}{4}$ ".
- 3. Perform concrete repairs to bridge deck and approach slab.
- 4. Replace drainage scuppers and construct new drainage system at Piers 3 and 8.
- 5. Place  $2^3_4$ " latex concrete overlay on bridge deck and perform  $^l_4$ " diamond grinding of new overlay.
- 6. Jack and remove existing expansion bearings at the abutments and at Piers 2, 5 and 8 and replace with elastomeric bearings.
- 7. Perform steel repairs to beam ends at Piers 2 and 8.
- . Replace steel diaphragms at over Pier 8.
- 9. Construct concrete encasement at Piers 2, 5 & 8.
- 10. Perform substructure repairs.
- 11. Apply protective coat to inside and top faces of existing parapets, sidewalks, medians, new concrete at expansion joints and latex overlay.
- 12. Apply  $1^{3}_{4}$ " HMA resurfacing to approach slabs.



Conduit Attached to Structure, 2" Dia., PVC Coated Galvanized Steel to be clamped paralled to the west side of Beam 1 flange and connected to new Junction Box attached to the west side of structure at each end of the bridge for future traffic signal use (See Plan below and Conduit Beam Clamped Installation Detail on Sheet No. SA-2)

#### ELEVATION JUN BX SS AS, JUN BX SS AS, 24" X 24" X 8" (1 Each) 24" X 24" X 8" (1 Each) to be attached to bridge to be attached to bridge Back Existing Abubtments parapet or as directed by parapet or as directed by 7 50<u>'-0"</u> 59′-6" 100′-0" 102'-0" 60'-0" 50′-0" 52′-6" 50′-0" 39′-0" 59'-0" 50′-0" 50'-0" the Engineer for future the Engineer for future \\\_Span 5 traffic signal use Span 1 Span 2 Span 3 Span Span 9 Span 12 Span 4 Span 6 Span 10 Span 11 traffic signal use Ŧ Bk. of N. Abut. Bk. of S. Abut. Sta. 73+33.04 Sta. 66+11.04 - Ç S. Halsted St. & Stage Const. Line EX ROW Pier 7 € Pier 1 © Pier 8 # € Pier 9-€ Pier 10 © Pier 3—



AMISH T. BHATT 081-006249

I hereby certify that these plans were prepared by me or under my direct personal supervision and that I am a duly licensed structural engineer under the laws of the State of Illinois.

Amish T. Bhatt DATE: 2-29-2024

AMISH T. BHATT S.E. License #081-006249

LICENSE EXP. 11-30-2024

Applies to Sheet SA-1 thru SA-14 and SA-16 thru SA-23.

AECOM

303 EAST WACKER DRIVE, SUITE 1400
CHICAGO, IL GOGOI-5276
PHONE, 13122 373-7700 FAM, (1)22 373-6800

USER NAME = ken.moy	DESIGNED -	RJL/SEA	REVISED -
	CHECKED -	ATB	REVISED -
PLOT SCALE = N.T.S.	DRAWN -	JJE/KAD	REVISED -
PLOT DATE = 4/2/2024	CHECKED -	ATB	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PLAN

GENERAL PLAN & ELEVATION STRUCTURE NO. 016-0195 SHEET NO. SA-1 OF SA-23 SHEETS

F.A.U. RTE	SECT	ΠON		COUNTY	TOTAL SHEETS	SHEE NO.
3730	15-00131-01-BR		соок	109	38	
				CONTRAC	T NO. 62	N48
		ILLINOIS	FED. AI	D PROJECT		

# GENERAL NOTES

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Reinforcement bars designated (E) shall be epoxy coated.

Existing reinforcement bars extending into the new construction shall be cleaned and straightened prior to incorporating into new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved embedded reinforcement or Mechanical Bar Splicer System. Cost included with Concrete Removal.

All structural steel shall conform to AASHTO Classification M-270 Grade 50, unless otherwise noted.

Fasteners shall be ASTM F 3125 Grade A325 Type 1, mechanically galvanized bolts. Bolts  $^{3}_{4}$ "  $\phi$ , open holes  $^{13}_{16}$  "  $\phi$ , unless otherwise noted.

Prior to pouring the new concrete deck or overlay, all heavy or loose rust, loose mill scale and other loose and potentially detrimental foreign material shall be removed from the surfaces in contact with concrete, Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost shall be included with Concrete Removal.

Up to  $^{l}_{4}$ " may be ground off the bridge deck. Bridge Deck Grooving shall be completed only after Diamond Grinding (Bridge Section) is completed.

Existing raised reflective pavement markers on the bridge deck and approach slab shall be removed and replaced. See Proposed Pavement Marking Plans for location of proposed markers on the bridge structure.

Cost of removal and re-installation of all structural steel members necessary to complete the work as detailed on the plans and as specified in the Special Provisions shall be included with Furnishing and Erecting Structural Steel.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

All new structural steel and bearing assemblies shall be hot-dip galvanized. See Special Provisions for Hot Dip Galvanizing for Structural Steel.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.

No free fall deck drains will be permitted in the spans over the tracks.

Substructure repairs and bearing replacement shall be performed under staged construction when no live load is present.

The Contractor is advised that the existing structure may contain members that are in deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the existing structure when developing construction procedures. An existing structure information package will be provided by the Department to the Contractor upon request.

# INDEX OF SHEETS

SA - 1	General Plan & Elevation
SA - 2	Index of Sheets, General Notes & Bill of Materials
SA - 3	Stage Construction Details
SA - 4	Temporary Concrete Barrier

SA - 5 Approach Slab & Deck Repairs Existing Expansion Joint Removal

SA - 7 Expansion Joint Replacement at South Abutment

SA - 8 Expansion Joint Replacement at Pier 2 SA - 9 Expansion Joint Replacement at Pier 5

Expansion Joint Replacement at Pier 8 SA - 10 Expansion Joint Replacement at North Abutment SA - 11

Beam Encasement Cross Section Piers 2, 5 & 8 SA - 12 SA - 13 Preformed Joint Strip Seal

Preformed Joint Strip Seal SA - 14

SA - 15 Ream Fnd Renairs

SA - 16 Steel Diaphragm Replacement Drainage Scupper, DS-33

SA - 18 Drainage System

SA - 19 Type I Bearing Details SA-20 Type I Bearing Details

SA-21 Abutment Repairs

SA-22 Pier Repairs

SA - 23 Bar Splicer Assembly and Mechanical Splicer Details

# TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTA
Bituminous Materials (Tack Coat)	Pound	540		540
Portland Cement Concrete Surface Removal - Butt Joint	Sq. Yd.	360		360
Hot-Mix Asphalt Surface Course, IL-9.5 Mix "D", N70	Ton	130		130
Concrete Removal	Cu. Yd.	57.0		57.0
Protective Shield	Sq. Yd.	5,147		5,147
Concrete Superstructure	Cu. Yd.	175.6		175.6
Protective Coat	Sq. Yd.	6,075		6,07.
Furnishing and Erecting Structural Steel	Pound	17,600		17,60
Reinforcement Bars, Epoxy Coated	Pound	16,650		16,65
Bar Splicers	Each	300		300
Preformed Joint Strip Seal	Foot	335		335
Elastomeric Bearing Assembly, Type I	Each	80		80
Anchor Bolts, 1"	Each	160		160
Epoxy Crack Injection	Foot		150	150
Fiber Wrap	Sq. Ft.		561	561
Clean and Reseal Relief Joint	Foot	114		114
Bridge Drainage System	L. Sum		0.5	0.5
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	3,832		3,83
Pavement Removal (Special)	Sq. Yd.	2		2
Approach Slab Repair (Partial Depth)	Sq. Yd.	10		10
Jack and Remove Existing Bearings	Each	52		52
Structural Steel Removal	Pound	2,430		2,43
Structural Steel Repair	Pound	430		430
Combination Curb and Gutter Removal and Replacement Less Than or Equal to 10 Feet	Foot	5		5
Bridge Deck Latex Concrete Overlay, 2 <sup>3</sup> 4"	Sa. Yd.	4.152		4,15
Bridge Deck Scarification, $\frac{3}{4}$ "	Sa. Yd.	4.152		4,15
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	93	427	520
Structural Repair of Concrete (Depth Greater than 5 inches)	Sq. Ft.		689	689
Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	6		6
Drainage Scuppers, DS-33	Each	4		4
Diamond Grinding (Bridge Section)	Sq. Yd.	3,670		3.67
Temporary Shoring and Cribbing	Each	28		28
Conduit Attached to Structure, 2" Dia., PVC Coated Galvanized Steel	Foot	762		762
Junction Box, Stainless Steel, Attached to Structure, 24" X 24" X 8"	Each	2		2
Acrylic Coating	Sq. Yd.	_	63	63

<sup>\*</sup> Quantity includes repairs to median and sidewalk. See Sheet SA-5.

# Conduit Beam Clamp Installation Detail Exist. Bridge Deck-Exist, Steel Girder PVC Coated -PVC Coated Conduit Clamp Conduit Beam Clamp See Note 1 N.T.S. See Note 3 PVCC Conduit See Note 2

# NOTES:

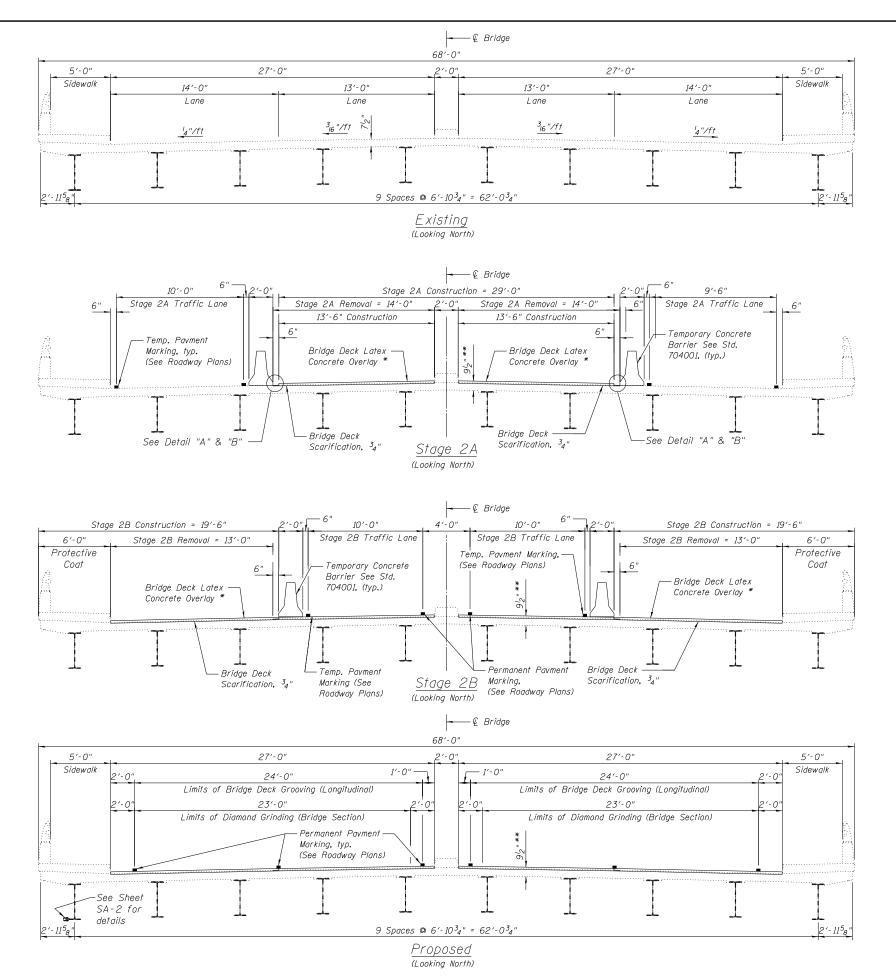
- 1. All mounting hardware for the PVCC RGC conduit must be PVC coated.
- 2. Secure the conduit with PVC coated conduit clamps or conduit beam clamps as shown at 5'-0" intervals for laterals and within 2'-0" maximum from any junction box, flexible conduit, or change in direction. All PVC coated conduit clamps or beam clamps will be included with the cost of the "conduit attached to structure" pay item.
- 3. All conduit attached to structure shall be PVC coated rigid steel conduit (PVCC RGC) typical.

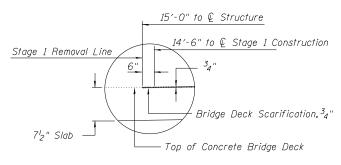
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	CHECKED - ATB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - JJE/KAD	REVISED -
PLOT DATE = 5/10/2024	CHECKED - ATB	REVISED -

INDEX OF SHEETS, GENERAL NOTES & BILL OF MATERIALS STRUCTURE NO. 016-0195
SHEET NO. SA-2 OF SA-23 SHEETS

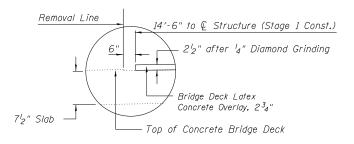
LU.	SECT	ΠON		COUNTY	TOTAL SHEETS	SHEET NO.
30 15-00131-01-BR		соок	109	39		
·		CONTRAC	T NO. 62	W48		
		ILLINOIS	FED. AII	D PROJECT		

<sup>\*\*</sup> See Temporary Shoring and Cribbing notes on sheet SA-21 & SA-22.





<u>DETAIL A (Showing Removal) @ West Side</u> (East Side Similar)



<u>DETAIL B (Showing Proposed)</u> @ West Side (East Side Similar)

- \* Place  $2\frac{3}{4}$ " latex concrete overlay on bridge deck. Final overlay thickness shall be  $2\frac{1}{2}$ " after  $\frac{1}{4}$ " Diamond Grinding, (See Special Provisions.)
- \*\* Prior to grinding

<b>AECOM</b>
303 EAST WACKER DRIVE, SUITE 1400 CHICAGO, IL 60601-5276 PHONE: (312) 373-7700 FAX: (312) 373-6800

USER NAME = ken.moy	DESIGNED -	RJL/SEA	REVISED -
	CHECKED -	MCC	REVISED -
PLOT SCALE = N.T.S.	DRAWN -	JJE/KAD	REVISED -
PLOT DATE = 5/10/2024	CHECKED -	MCC	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

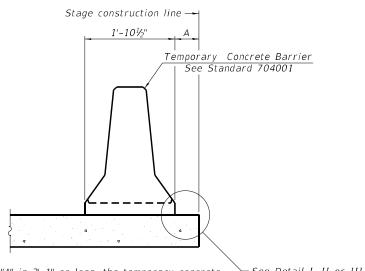
STAGE CONSTRUCTION DETAILS
STRUCTURE NO. 016-0195

SHEET NO. SA-3 OF SA-23 SHEETS

 
 FAU. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

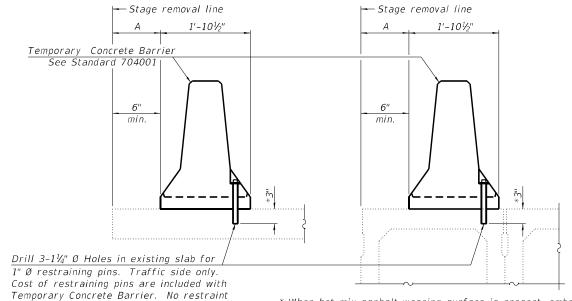
 3730
 15-00131-01-BR
 COOK
 109
 40

 CONTRACT NO. 62W48



- See Detail I, II or III When "A" is 3'-1" or less, the temporary concrete barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

## NEW SLAB OR NEW DECK BEAM



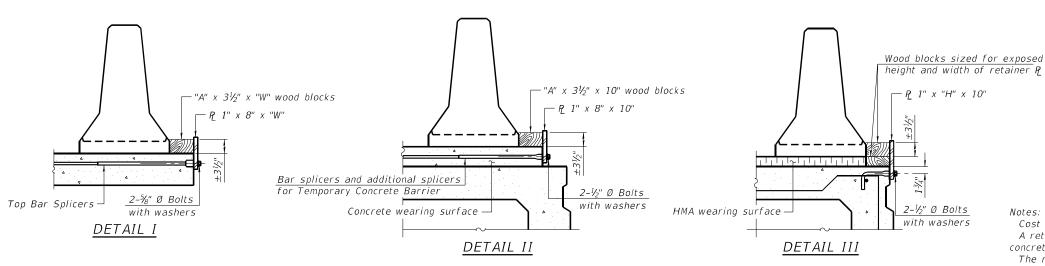
\* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

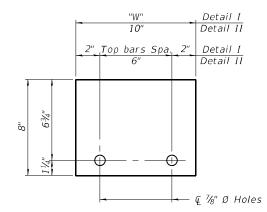
# EXISTING DECK BEAM

# SECTIONS THRU SLAB OR DECK BEAM

is required when "A" is greater than 3'-1".

EXISTING SLAB

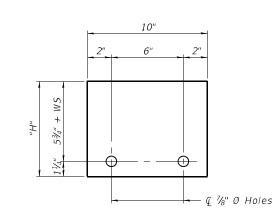




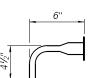
RAILING	CRITERIA
11/11/11/0	

NCHRP 350 Test Level	3
Railing Weight (plf)	440

10-12-2021



STEEL RETAINER P 1" x "H" x 10" (Detail III)



RESTRAINING PIN

# BAR SPLICER FOR #4 BAR - DETAIL III

#### Notes:

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

1x8 UNC

US Std.  $1\frac{1}{16}$ " I.D. x  $2\frac{1}{2}$ " 0.D. x approx. 8 gauge thick washer

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than  $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

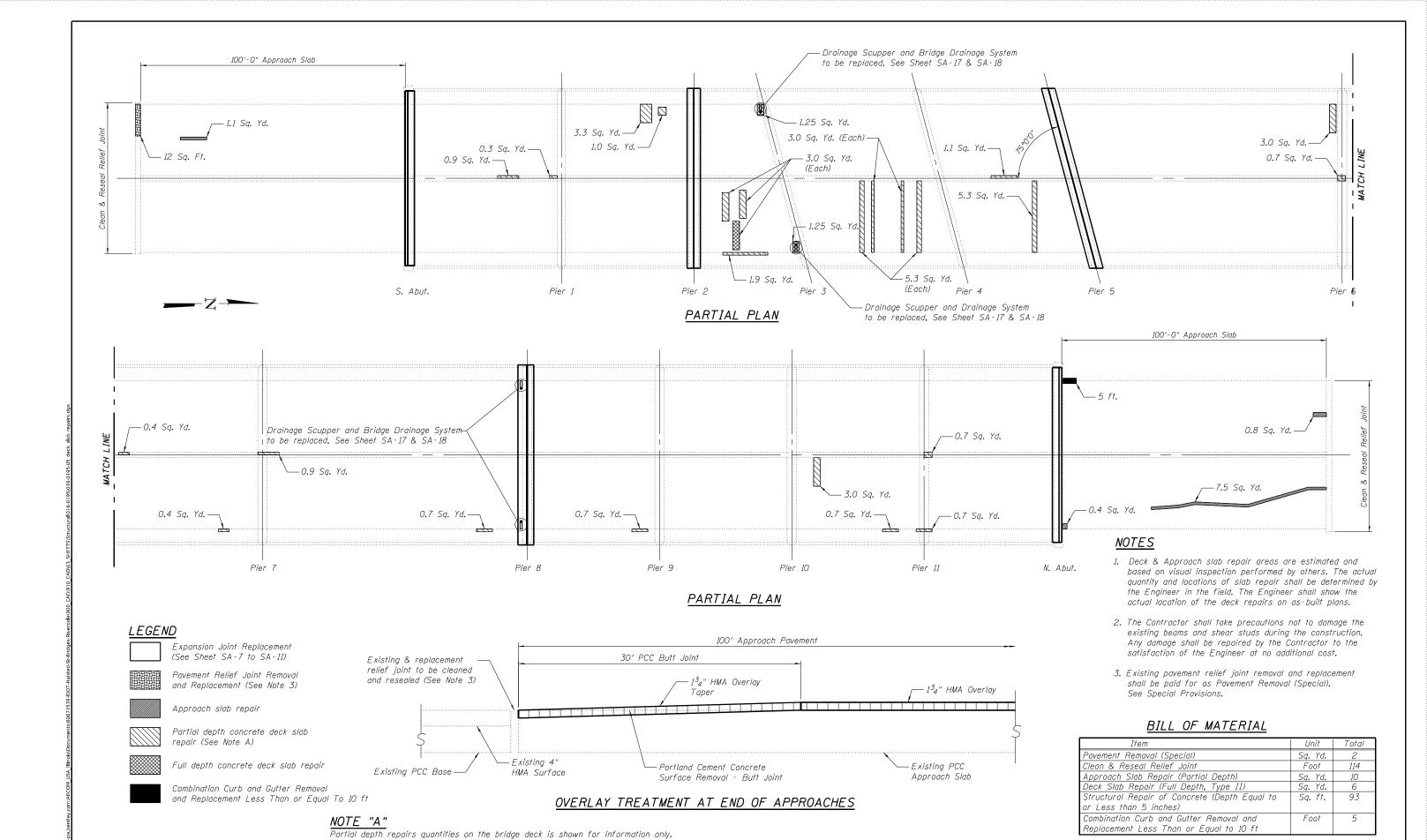
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	CHECKED - ATB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - MCC	REVISED -
PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)

TEMPORARY CONCRETE BARRIER STRUCTURE NO. 016-0195					
SHEET NO. SA-4 OF SA-23 SHEETS					

A.U. TE	SECT	TION		COUNTY	TOTAL SHEETS	SHEET NO.
730	15-00131-0	)1-BR		соок	109	41
				CONTRAC	T NO. 62	W48
		ILLINOIS	FED AL	D PROJECT		



AECOM
303 EAST WACKER DRIVE, SUITE 1400
CHICAGO, IL GOGGI-5276
PHONE: 1322 373-7700 FAX: (3)23 373-6800

 USER NAME
 = BhattA
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 RJL/SEA
 REVISED

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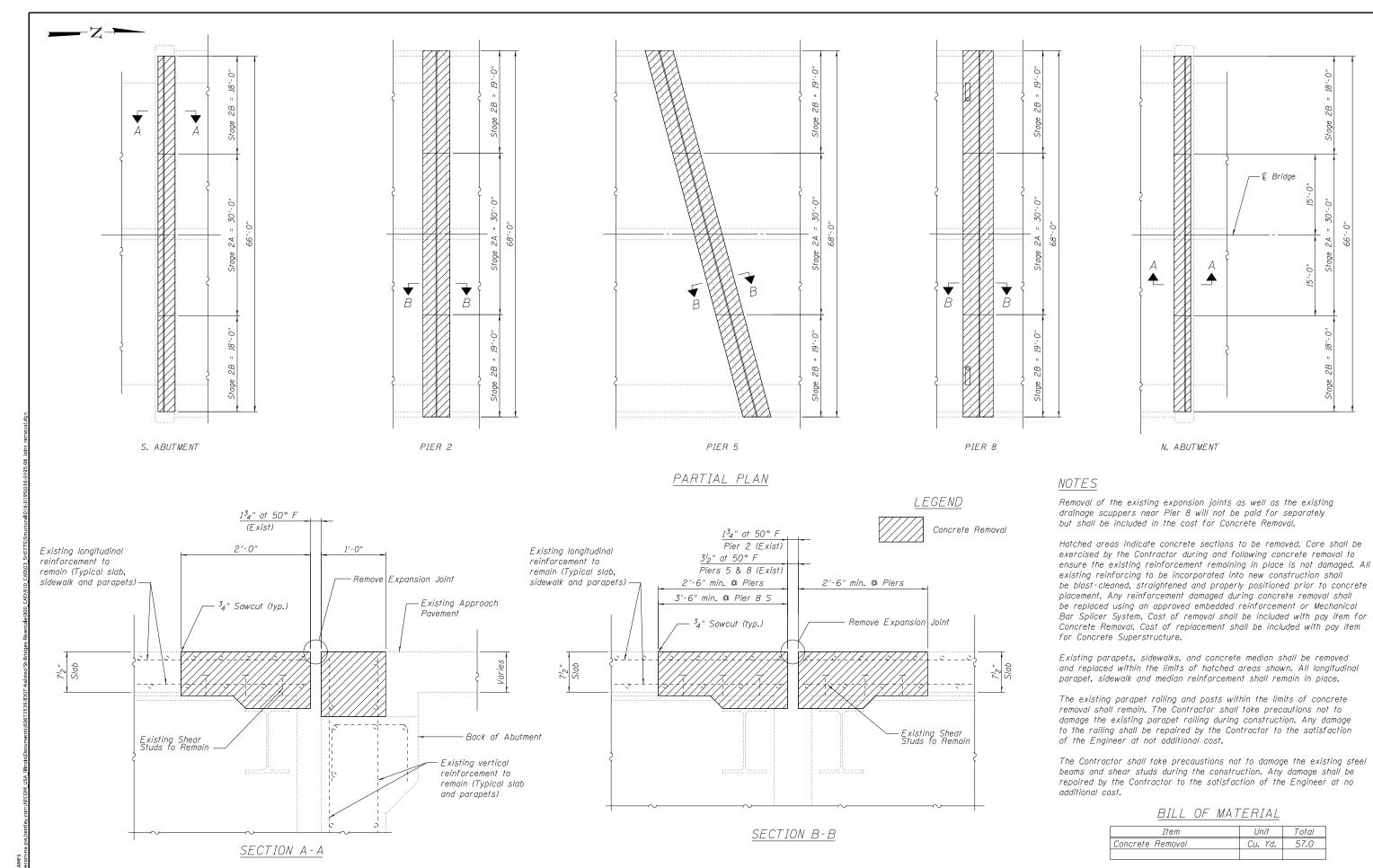
 PLOT SCALE
 = N.T.S.
 DRAWN
 JJE/KAD
 REVISED

 PLOT DATE
 = 5/10/2024
 CHECKED
 ATB
 REVISED

Formed concrete repairs to the bridge median and sidewalks shall be paid for as Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

APPROACH SLAB & DECK REPAIRS STRUCTURE NO. 016-0195 SHEET NO. SA-5 OF SA-23 SHEETS



AECOM

33 EAST WACKER DRIVE, SUITE 1400
11CAGO, ILL GOGO1-5276

 USER NAME
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 PLOT SCALE
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 DRAWN
 JJE/KAD
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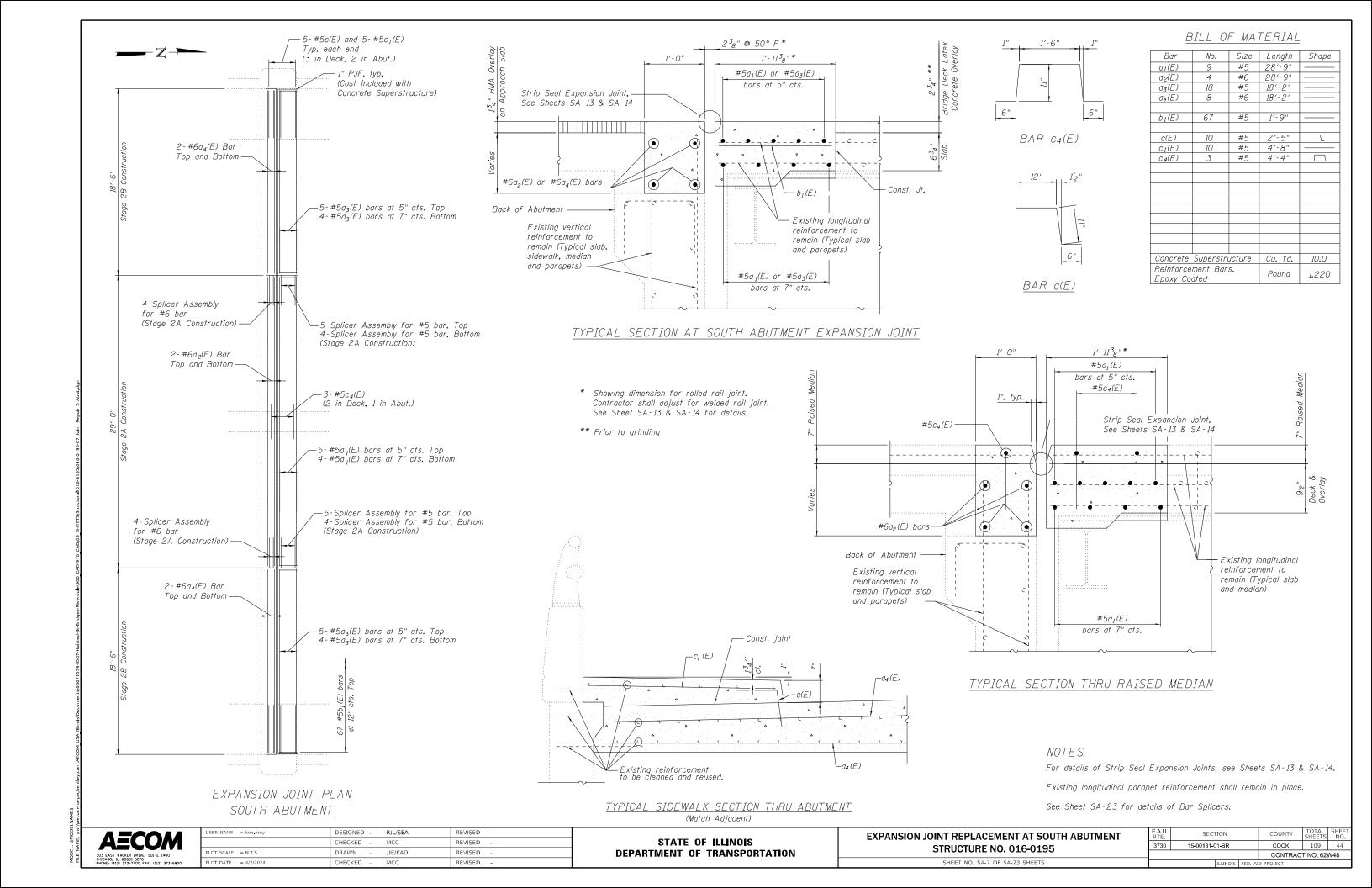
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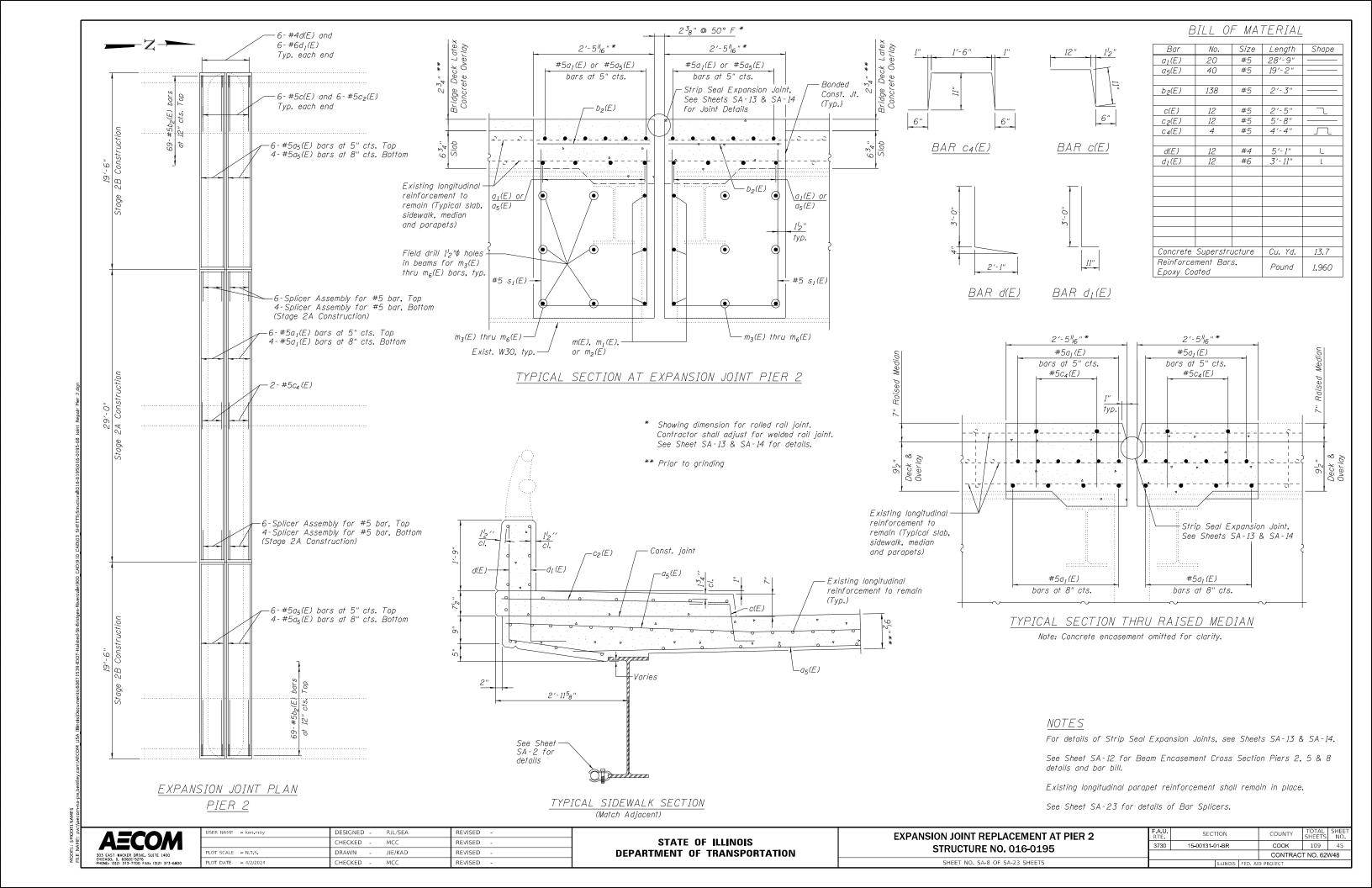
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

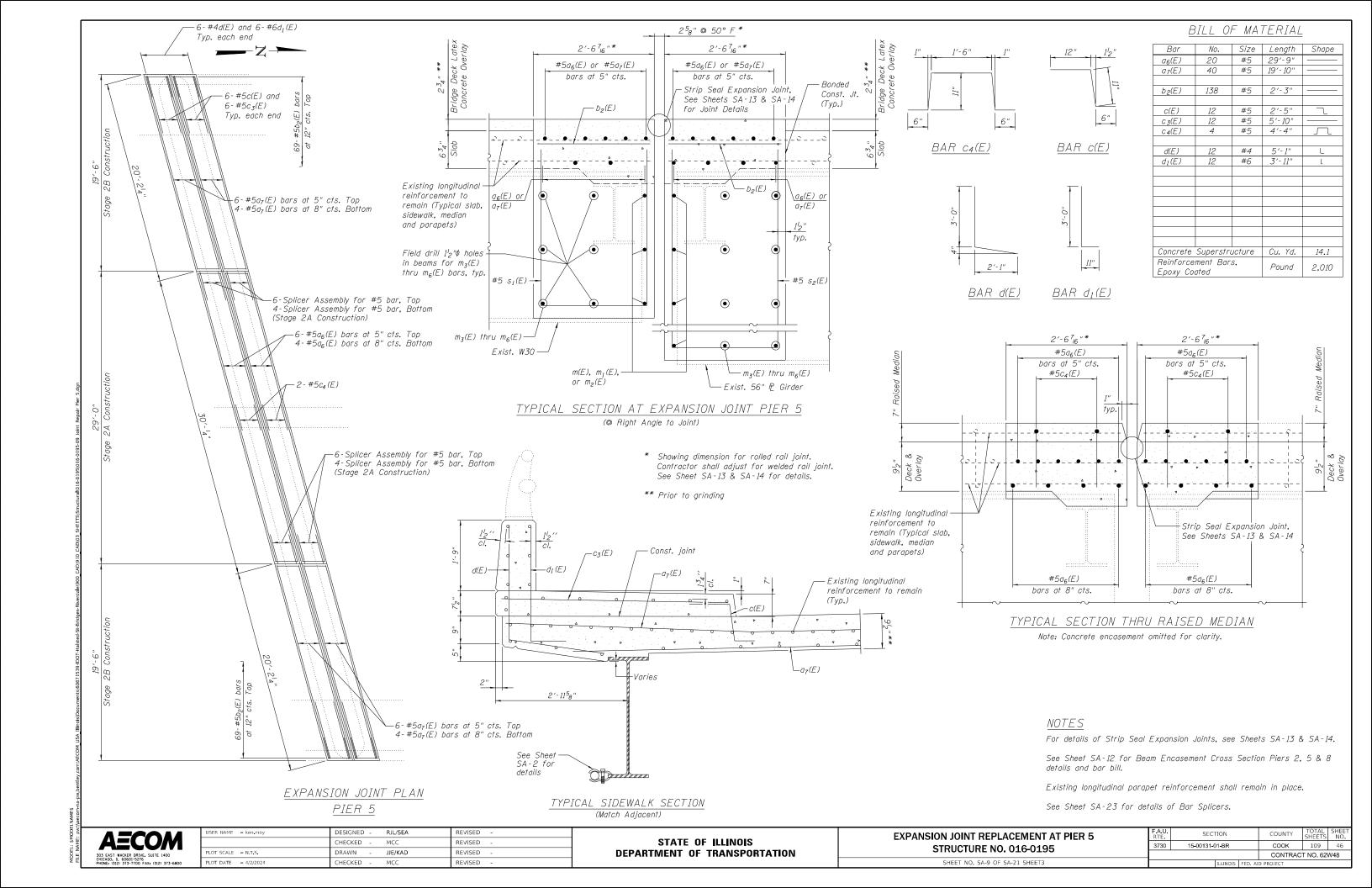
EXISTING EXPANSION JOINT REMOVAL STRUCTURE NO. 016-0195

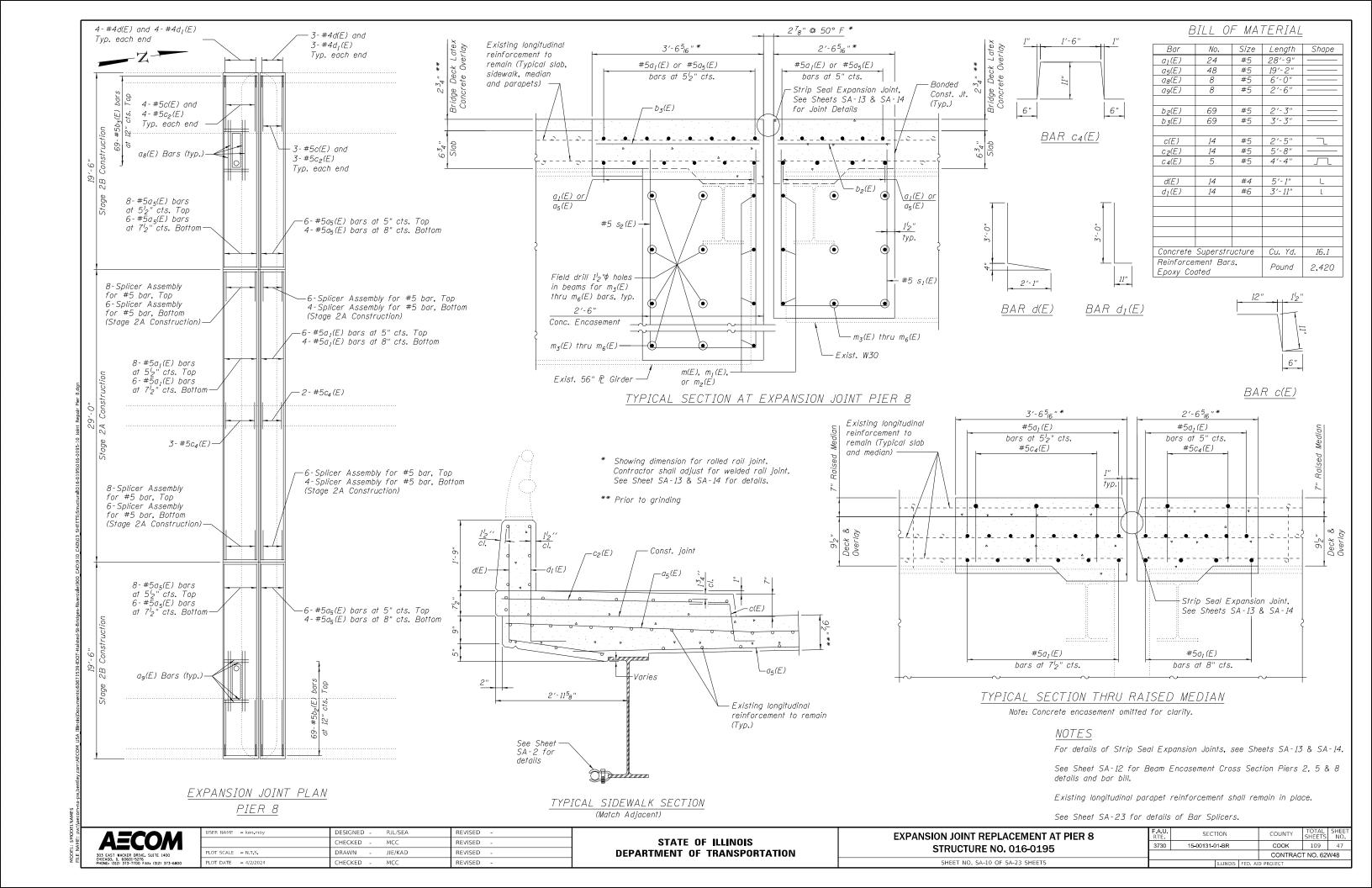
SHEET NO. SA-6 OF SA-23 SHEETS

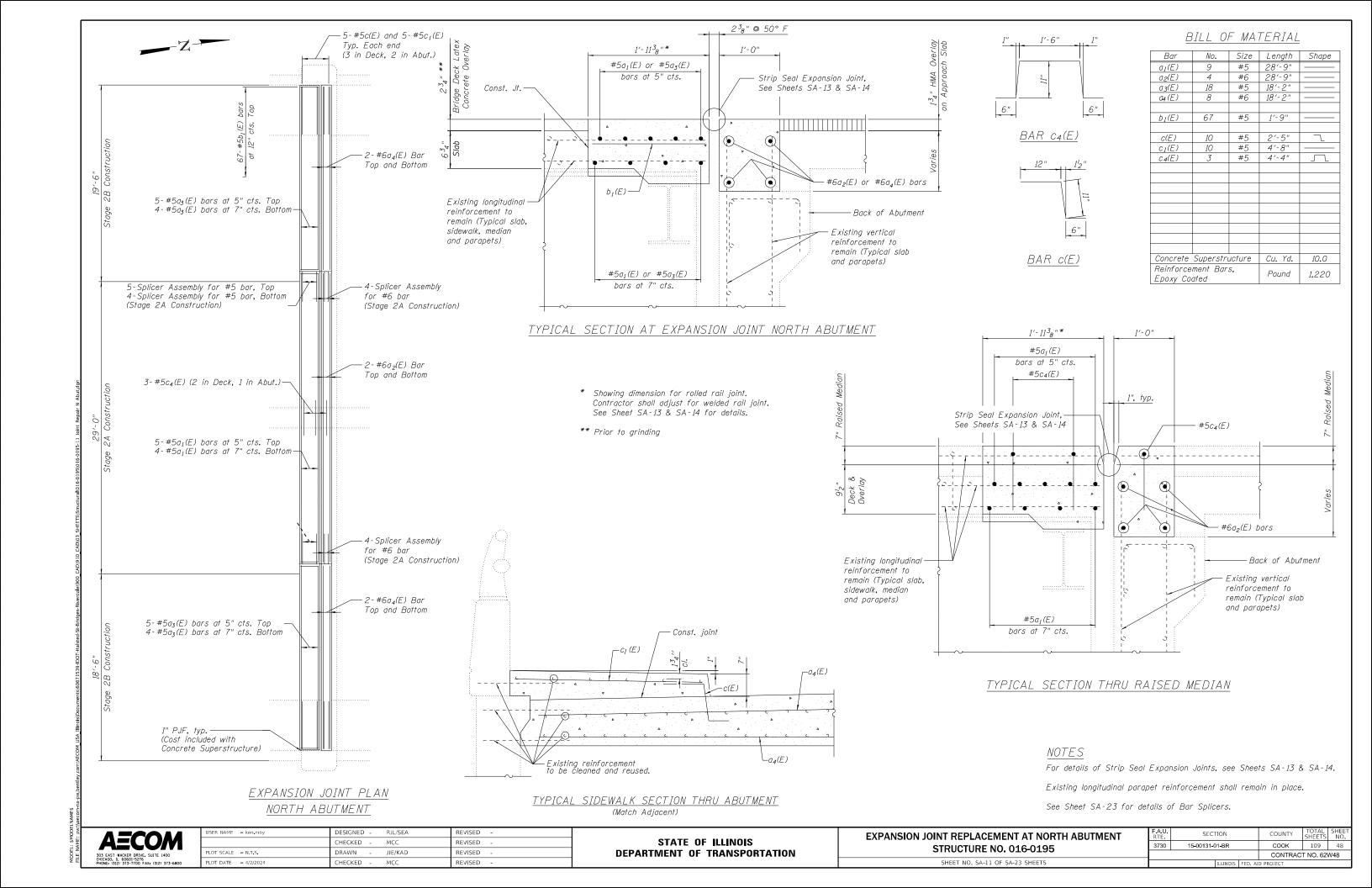
F.A.U. RTE	SECT	ΠON		COUNTY	TOTAL SHEETS	SHEET NO.
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				CONTRAC	T NO. 62	N48
		ILLINOIS.	FFD, AI	D PROJECT		

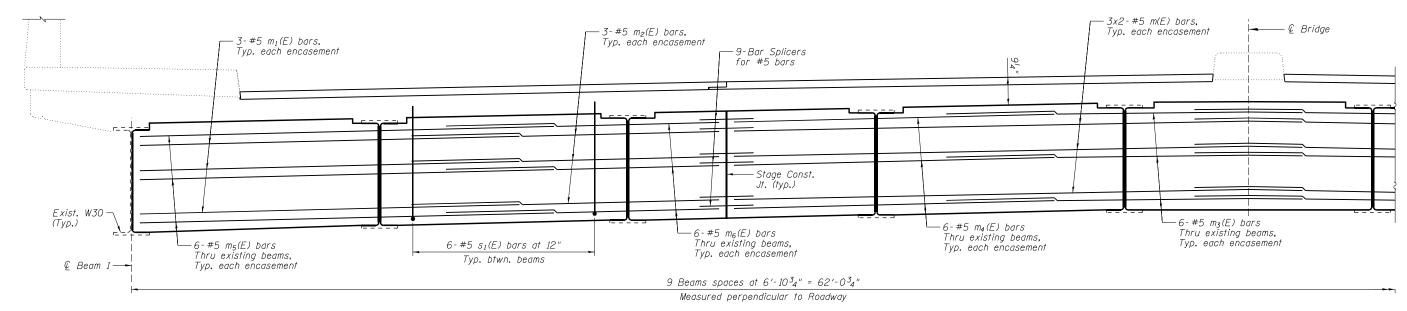






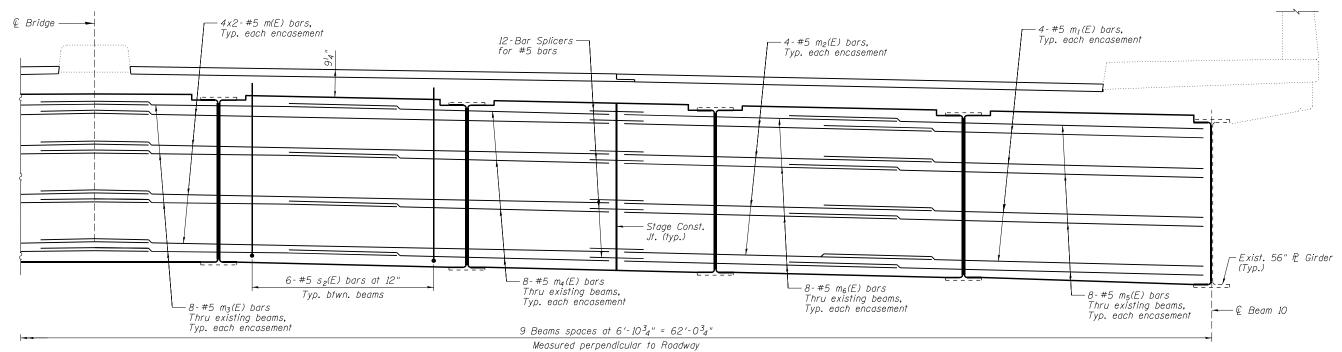






# <u>SECTION AT PIERS 2, 5 & 8 - SPANS 2, 3, 5 & 9</u>

(Looking North, Showing West Side; Opp. side similar)



# SECTION AT PIERS 5 & 8 - SPANS 6 & 8

(Looking North, Showing West Side; Opp. side similar)

# MINIMUM BAR LAP

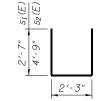
#5 bar = 3'-0"

# NOTES:

See Sheets SA-3 for Stage Construction Details.

See Sheets SA-8 thru SA-10 for typical section thru expansion joint and beam encasement.

Clean beam ends of all loose rust and debris prior to pouring encasement concrete.



BARS  $s_1(E)$  &  $s_2(E)$ 

# <u>SUPERSTRUCTURE</u> BILL OF MATERIAL

Bar	No.	Size	Length	Shape
m(E)	40	#5	16'-4''	
$m_1(E)$	40	#5	8'-11''	
$m_2(E)$	40	#5	10'-11''	
m3(E)	80	#5	10′-5′′	
m4(E)	80	#5	9'-5"	
m5 (E)	80	#5	11'-11''	
m <sub>6</sub> (E)	80	#5	7'-11''	
s1(E)	216	#5	7′-5′′	
s2 (E)	108	#5	11'-9''	
Concret	e Supers	Cu. Yd.	111.7	
Reinford Epoxy (	cement B Coated	Pound	7,820	

<b>AECOM</b>
303 EAST WACKER DRIVE, SUITE 1400 CHICAGO, IL 60601-5276
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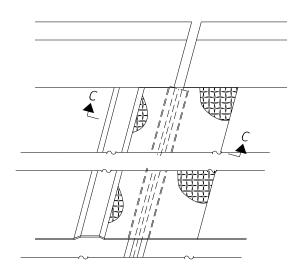
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	CHECKED - ATB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - MCC	REVISED -
PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

BEAM ENCASEMENT CROSS SECTION PIERS 2, 5 & 8 STRUCTURE NO. 016-0195	
SHEET NO. SA-12 OF SA-23 SHEETS	

LU. E	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
30	15-00131-01-BR		соок	109	49
			CONTRAC	T NO. 62	N48
	ILLINOIS	FED. All	D PROJECT		

# SECTION AT RAISED SIDEWALK

\*\*\* Prior to grinding



# PLAN AT RAISED SIDEWALK

(For skews ≤ 30°) See Sheet SA-14 for Section C-C Notes:

The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

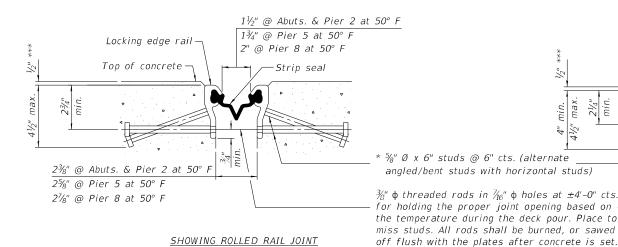
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

The top surface of sidewalk sliding plates shall have a raised pattern according to ASTM A786.

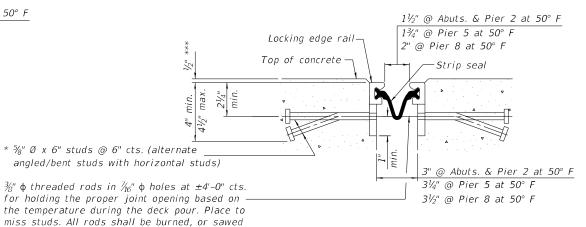
Cost of sidewalk sliding plates, embedded plates, anchorage studs, and expansion anchors included with Preformed Joint Strip Seal.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required.

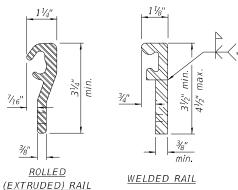


SECTION A-A

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

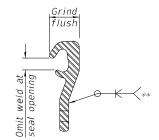


SHOWING WELDED RAIL JOINT



# LOCKING EDGE RAILS

\*\* Back gouge not required if complete joint penetration is verified by mock-up.



# LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

## BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	335

**AECOM** 

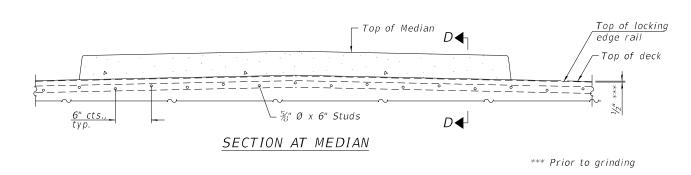
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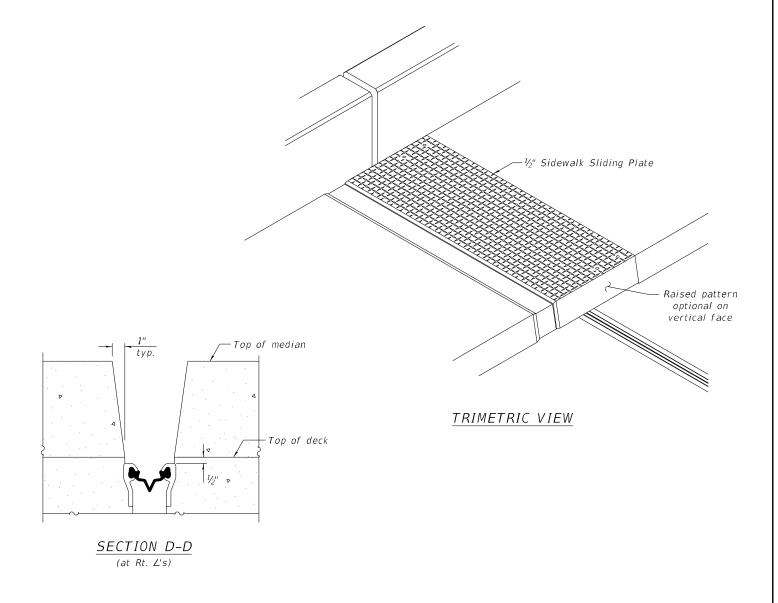
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

(Sheet 1 of 2) PREFORMED JOINT STRIP SEAL **STRUCTURE NO. 016-0195** 

SECTION COUNTY 3730 15-00131-01-BR COOK 109 50 CONTRACT NO. 62W48

SHEET NO. SA-13 OF SA-23 SHEETS





 USER NAME
 = ken.moy
 DESIGNED
 RL/SEA
 REVISED

 CHECKED
 MCC
 REVISED

 PLOT SCALE
 = N.T.S.
 DRAWN
 MCC
 REVISED

 PLOT DATE
 = 4/2/2024
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 ATB
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

(Sheet 2 of 2)

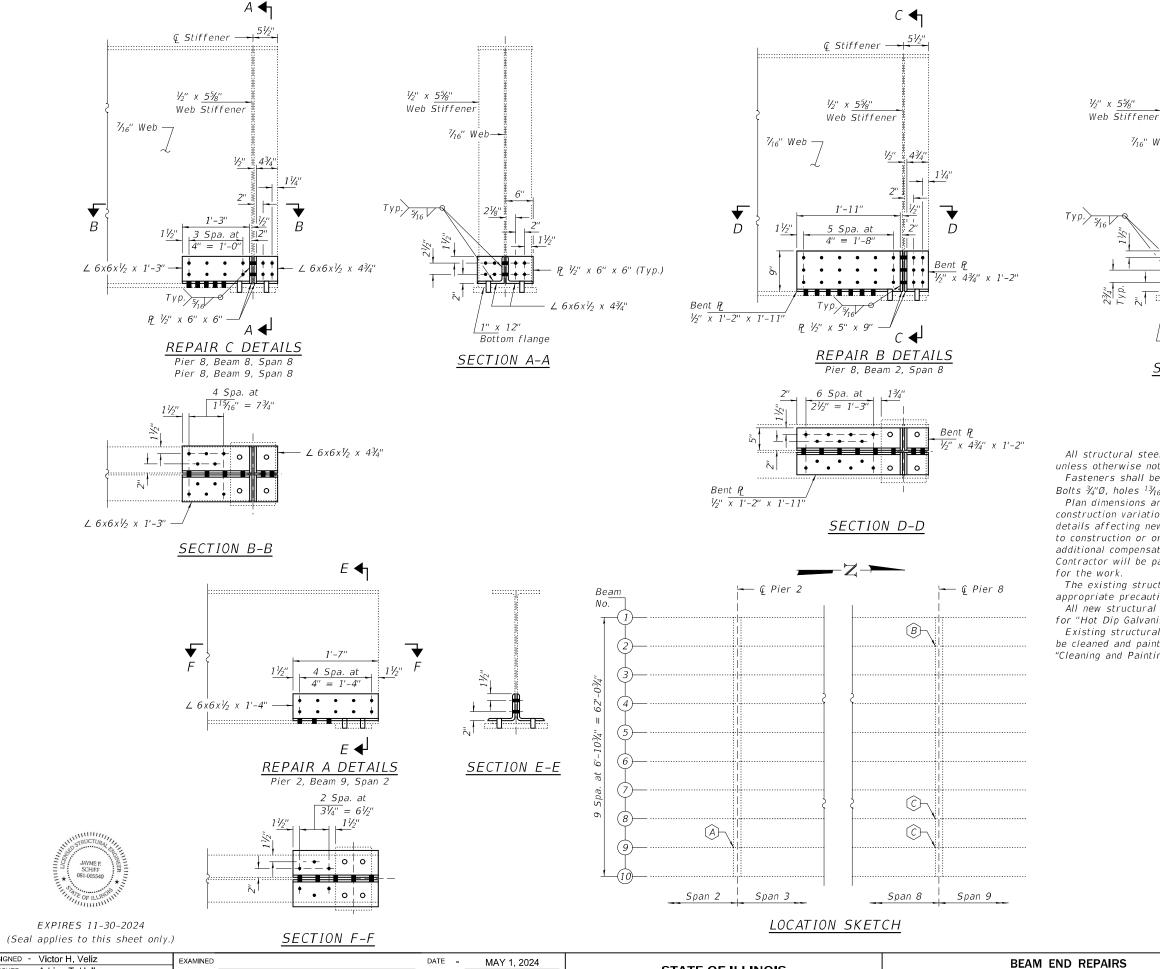
PREFORMED JOINT STRIP SEAL
STRUCTURE NO. 016-0195

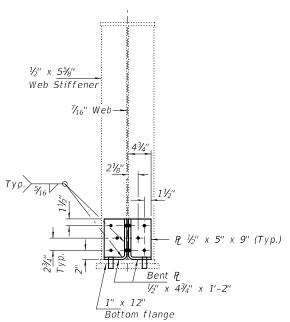
SHEET NO. SA-14 OF SA-23 SHEETS

 
 F.A.U. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

 3730
 15-00131-01-BR
 COOK
 109
 51

 CONTRACT NO. 62W48





# SECTION C-C

# GENERAL NOTES

All structural steel shall conform to AASHTO Classification M-270 Gr. 36, unless otherwise noted.

Fasteners shall be ASTM F 3125 Grade 325 Type 1 mechanically galvanized bolts. Bolts  $\frac{3}{4}$ "Ø, holes  $\frac{13}{16}$ "Ø, unless otherwise noted.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

All new structural steel shall be hot dip galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".

Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the special provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

# LEGEND

- HO

Holes to be field drilled in existing steel using new steel as template.

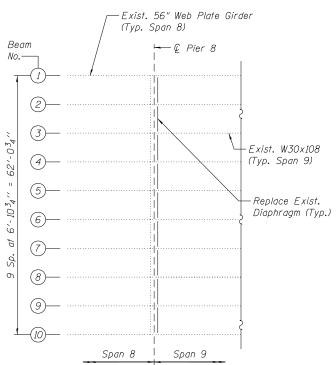
O - H

Holes to be field drilled in new steel using existing holes as template.

# BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Structural Steel Repair	Pound	430

DESIGNED - Victor H. Veliz	EXAMINED	DATE -	MAY 1, 2024	CTATE OF ILLINOIS	BEAM END REPAIRS	RTE	SECTION	COUNTY	SHEETS NO.
CHECKED - Adrian T. Halloway	ENGINEER O	STRUCTURAL SERVICES		STATE OF ILLINOIS	SN 016-0195	3730	15-00131-01-BR	соок	109 52
DRAWN - daburdell	PASSED	REVISED	-	DEPARTMENT OF TRANSPORTATION	314 010-0133			CONTRAC	T NO. 62W48
CHECKED - VHV ATH	ENGINEER OF I	RIDGES AND STRUCTURES REVISED	-		SHEET NO. SA-15 OF SA-23 SHEETS		ILLINOIS FED. A	AID PROJECT	



LOCATION SKETCH

Remove and Replace
Exist. <sup>3</sup><sub>4</sub>" H.S. Bolts,
typ. (See Note I)

Exist. <sup>3</sup><sub>4</sub>" Bent R
to remain, typ.

Exist. W30x108
to remain, typ.

DIAPHRAGM REPLACEMENT

# NOTES:

1.  $^{15}$ <sub>16</sub>" holes in bottom flange of new diaphragm shall be field drilled to match holes in existing Bent  $P_c$ .

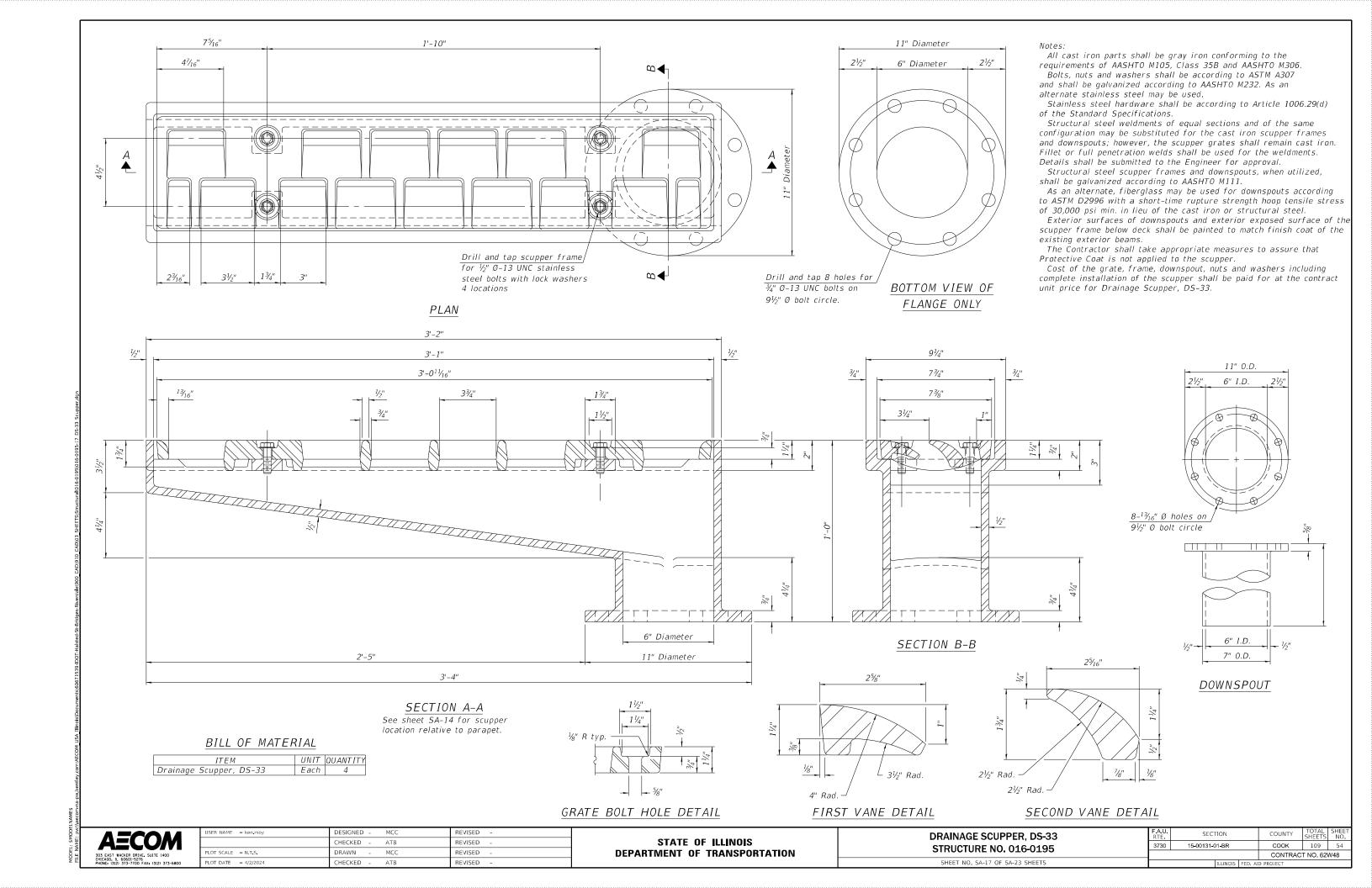
# BILL OF MATERIAL

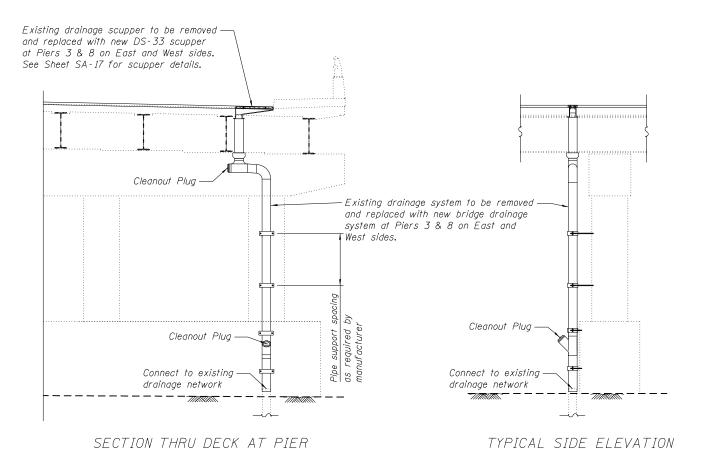
ITEM	UNIT	QUANTITY
Structural Steel Removal	Pound	2,430
Furnishing and Erecting Structural Steel	Pound	2,430

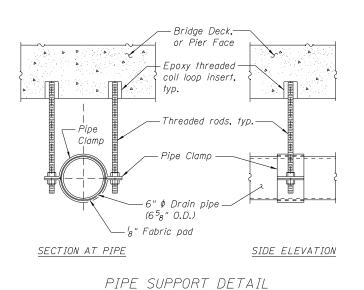
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	CHECKED - ATB	REVISED -	
PLOT SCALE = N.T.S.	DRAWN - MCC	REVISED -	
PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -	

STEEL DIAPHRAGM REPLACEMENT STRUCTURE NO. 016-0195
CHEET NO. CA 16 OF CA 22 CHEETC

A.U. TE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
730	15-00131-01-BR			соок	109	53
			CONTRAC	T NO. 62	N48	
		ILLINOIS	FED. AI	D PROJECT		







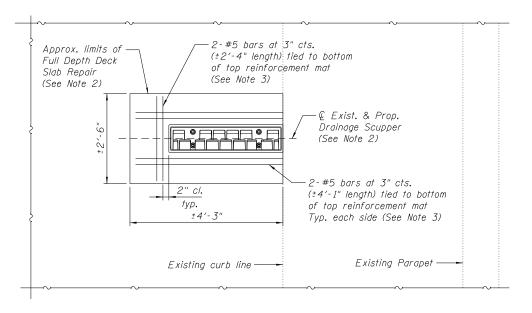
(at Pier 3 & Pier 8)

# NOTES

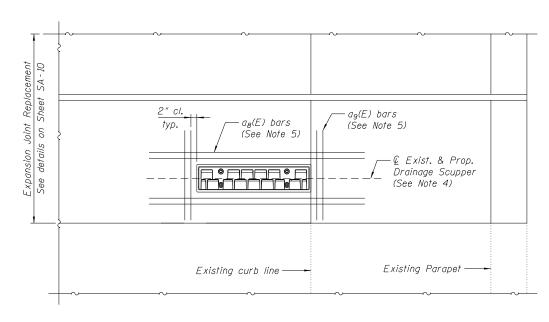
1. Drainage scupper top elevation shall be recessed to accommodate diamond grinding. Place scupper 1/4" below the top of slab.

(at Pier 3 & Pier 8)

- 2. The removal of the existing scuppers near Pier 3 shall not be paid for separately but shall be included with Deck Slab Repair (Full Depth, Type II). See SA-5 for scupper location and deck removal limits.
- 3. The additional reinforcement bars shown at the Pier 3 scupper replacement shall not be paid for separately but shall be included in the cost of Deck Slab Repair (Full Depth, Type II).
- 4. The removal of the existing scuppers near Pier 8 shall not be paid for separately but shall be included in the concrete removal for the expansion joint. See SA-6 for concrete removal limits.
- 5. Reinforcement bars for drainage scuppers near Pier 8 included with expansion joint replacement. See Sheet SA-10 for bar list and quantity.
- 6. Remove and install new bridge drainage system at Pier 3 and Pier 8. Cost shall be included with Bridge Drainage System.



TOP PLAN AT DRAINAGE SCUPPER NEAR PIER 3



TOP PLAN AT DRAINAGE SCUPPER NEAR PIER 8

# BILL OF MATERIAL

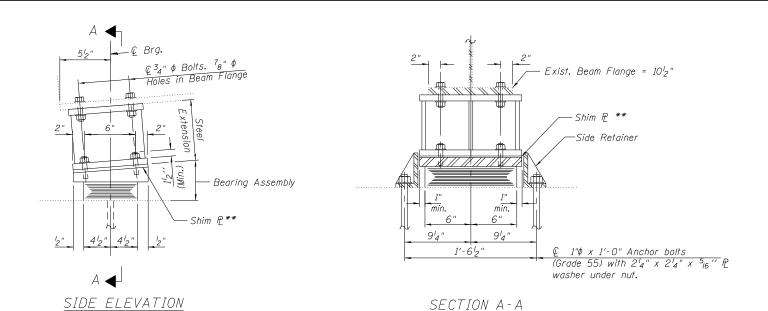
Item	Unit	Total
Bridge Drainage System	L. Sum	0.5

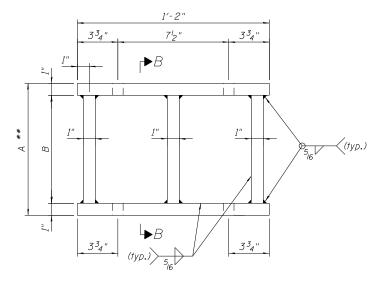
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PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

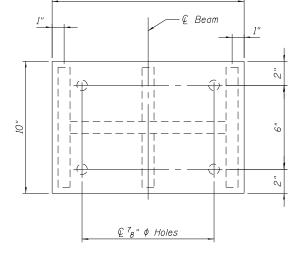
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

DRAINAGE SYSTEM **STRUCTURE NO. 016-0195** SHEET NO. SA-18 OF SA-23 SHEETS

SECTION COUNTY 3730 15-00131-01-BR COOK 109 55 CONTRACT NO. 62W48







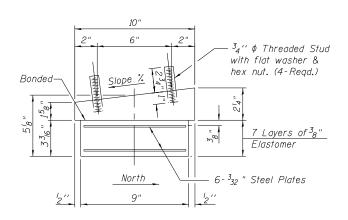
# ELEVATION STEEL EXTENSION

(Looking North or South)

# PLAN STEEL EXTENSION

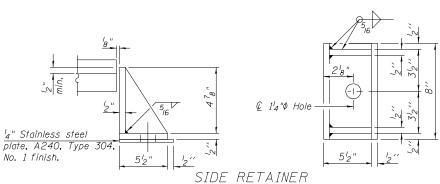
# TYPE I ELASTOMERIC EXP. BRG.

North & South Abutments, Pier 2 South, and Pier 2 North



# BEARING ASSEMBLY

Shim plates shall not be placed under Bearing Assembly.



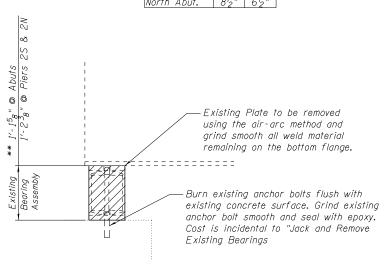
Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

# BEARING DIMENSIONS

Location	Slope @ P.G.L.
South Abut.	5.21%
Pier 2 South	5.21%
Pier 2 North	5.21%
North Abut.	-5.21%

# STEEL EXTENSION DIMENSIONS

Location	Α	В
South Abut.	812"	6½"
Pier 2 South	91/4"	74"
Pier 2 North	934"	734"
North Abut	812"	6/2"



# EXISTING BEARING REMOVAL DETAIL

\*\* Existing Bearings @ Pier 2N have an  $^{\prime\prime}_{16}$ " shim  $^{\prime\prime}_{2}$  for Beams 1-5 and  $^{7}_{16}$ " for Beams 6-10. An allowance of  $\frac{1}{2}$ " has been included in Dimension N.

#### Notes:

Existing bearing height (from top of the bearing seat to bottom of girder flange) information obtained from available existing plans is used to compute proposed steel extension height. Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions.

Anchor bolts shall be ASTM F1554 all-thread of the grade(s) and diameter(s) specified.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Cost of side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.

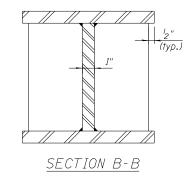
Two  $_8^{l}$ " in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details. Cost included with Elastomeric Bearing Assembly, Type I.

The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50. New steel extensions, shim plates and connection bolts are included with Furnishing and Erecting Structural Steel.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless and equivalent temporary means of lateral restrain is used.

\_\_ 9¼" to € Beam

7<sup>3</sup>₄" to **©** Beam



# REACTION TABLE (KIPS)

Туре	S. Abut.	Pier 2S	Pier 2N	N. Abut.
R DL	30.7	30.6	31.1	27.0
R LL	40.7	40.7 38.1 38.		39.5
R IMP	11.0	10.8	11.6	11.2
R Total	82.4	79.5	81.3	77.7

Minimum jack capcity required = 45 Tons

# BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	40
Anchor Bolts, 1"	Each	80
Furnishing and Erecting Structural Steel	Pound	6,840

(Sheet 1 of 2)

rNew 1" Ø

anchor bolt

**TYPE I BEARING DETAILS STRUCTURE NO. 016-0195** 

Exist. 14" \$

anchor bolt

LOCATIONS OF ANCHOR BOLTS

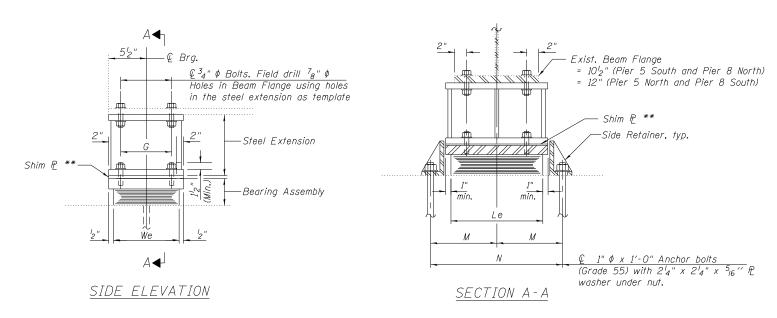
SECTION COUNTY 3730 15-00131-01-BR COOK 109 56 CONTRACT NO. 62W48

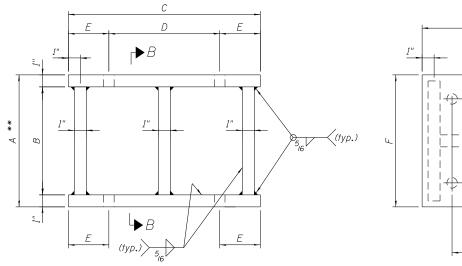
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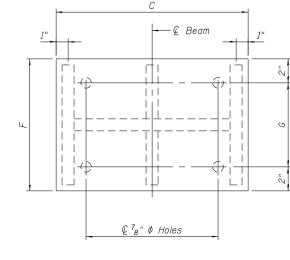
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	CHECKED - ATB	REVISED -
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PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

SHEET NO. SA-19 OF SA-23 SHEETS







# ELEVATION STEEL EXTENSION

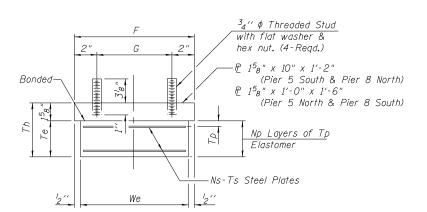
(Looking North or South)

# PLAN STEEL EXTENSION

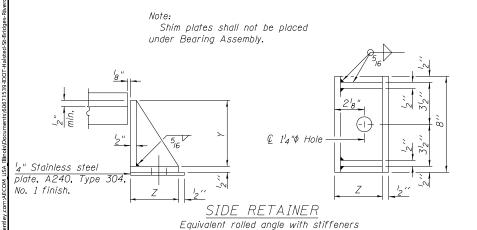
# TYPE I ELASTOMERIC EXP. BRG.

Pier 5 South, Pier 5 North Pier 8 South, and Pier 8 North

\*\* Provide one additional  $^38$ " shim  $^{12}$ for beam line 6 at Pier 5 South.



# BEARING ASSEMBLY



will be allowed in lieu of welded plates.

# BEARING DIMENSIONS

	Location	Le	We	Np	Τρ	Ns	Ts	Те	Th	Υ	Ζ	М	N
Ţ	Pier 5 South	12"	9"	7	38"	6	332 "	3 <sup>3</sup> 16"	4 <sup>13</sup> 16 "	4 <sup>5</sup> 8"	5 <sup>1</sup> 2"	94"	1'-612"
,	Pier 5 North	16"	11"	6	2"	5	8"	3 <sup>5</sup> 8"	514"	5"	6 <sup>1</sup> / <sub>4</sub> "	1114"	1'-10 <sup>1</sup> 2"
Į,	Pier 8 South	16"	11"	6	2"	5	8"	3 <sup>5</sup> 8"	54"	5"	64"	114"	1'-1012"
,	Pier 8 North	12"	9"	7	38"	6	332 "	3316"	4 <sup>13</sup> 16 "	4 <sup>5</sup> 8"	5½"	94"	1'-6'2"

# STEEL EXTENSION DIMENSIONS

A B C D E F G

1'-23g" @ Piers 5S & 8N 1'-2" @ Piers 5N & 8S	Pier 5 South $9^9_{16}$ " $7^9_{16}$ " $1' - 2$ " $6^1_{2}$ " $3^3_{4}$ " $10$ " $6$ " Pier 5 North $8^3_{4}$ " $6^3_{2}$ " $1' - 6$ " $8$ " $5$ " $12$ " $8$ " Pier 8 South $8^3_{4}$ " $6^3_{4}$ " $1' - 6$ " $8$ " $5$ " $12$ " $8$ " Pier 8 North $9^9_{16}$ " $7^9_{16}$ " $1' - 2$ " $6^1_{2}$ " $3^3_{4}$ " $10$ " $6$ " $10$ "
** Bearing Assemby  © £ Brg.	Existing Plate to be removed using the air-arc method and grind smooth all weld material remaining on the bottom flange.  Burn existing anchor bolts flush with existing concrete surface. Grind existing anchor bolt smooth and seal with epoxy. Cost is incidental to "Jack and Remove Existing Bearings".

# EXISTING BEARING REMOVAL DETAIL

Prior to jacking, the Contractor shall verify that hold-down rods installed at North and South abuments during construction have been removed according to original details.

Existing bearing height (from top of the bearing seat to bottom of girder flange) information obtained from available existing plans is used to compute proposed steel extension height. Prior to ordering any material, the Contractor shall verify in the field all bearing heights and shim thickness dimensions.

Anchor bolts shall be ASTM F1554 all-thread of the grade(s)

and diameter(s) specified.

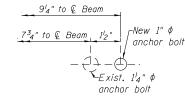
Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Cost of side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.

Two  $^{l}_{8}$ " in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details. Cost included with Elastomeric Bearing Assembly, Type I.

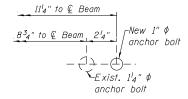
The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50. Existing bearing at beam line 6 at Pier 5 South has an additional  $^3{}_8$ " shim  $^4{}_8$ . Cost of this shim plate is included with Furnishing and Erecting Structural Steel.

New steel extensions, shim plates and connection bolts are included with Furnishing and Erecting Structural Steel. Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.



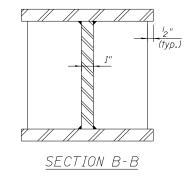
# LOCATIONS OF ANCHOR BOLTS

Pier 5 South & Pier 8 North



# LOCATIONS OF ANCHOR BOLTS

Pier 5 North & Pier 8 South



# REACTION TABLE (KIPS)

Туре	Pier 5S	Pier 5N	Pier 8S	Pier 8N
R DL	32.5	76.3	71.3	32.9
R LL	40.0	43.7	43.9	39.4
R IMP	10.8	9,6	9.7	11.2
R Total	83.3	129.6	124.9	83.5

Minimum jack capacity required

= 45 Tons (Pier 5S & Pier 8N) = 80 Tons (Pier 5N & Pier 8S)

# BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	40
Anchor Bolts, 1"	Each	80
Furnishing and Erecting Structural Steel	Pound	8,320

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USER NAME = ken.moy	DESIGNED - MCC	REVISED -
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PLOT SCALE = N.T.S.	DRAWN - MCC	REVISED -
PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

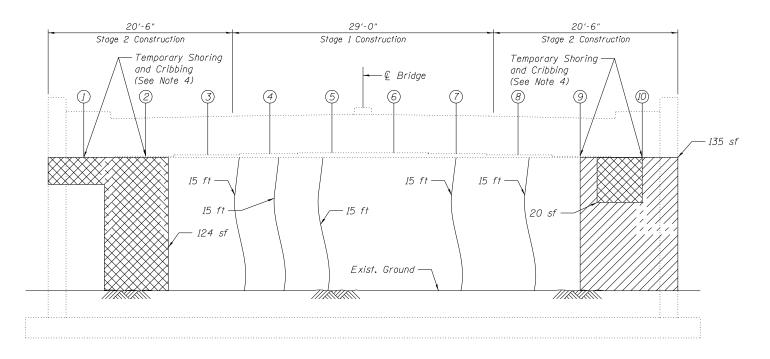
STATE OF ILLINOIS

(Sheet 2 of 2) **TYPE I BEARING DETAILS STRUCTURE NO. 016-0195** 

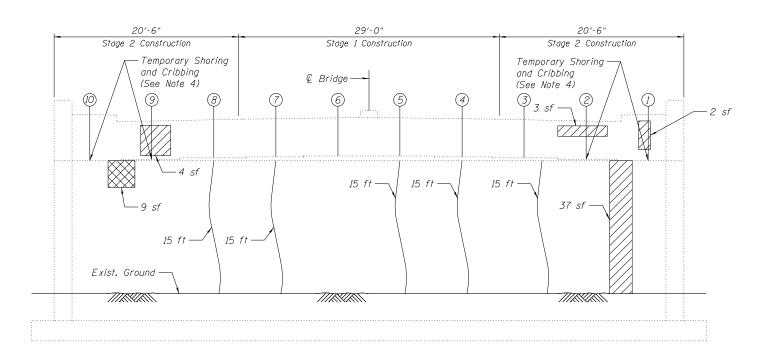
SHEET NO. SA-20 OF SA-23 SHEETS

F.A.U. RTE	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
3730	15-00131-01-BR		соок	109	57	
				CONTRAC	T NO. 62	N48
		ILLINOIS.	FFD, AI	D PROJECT		

**DEPARTMENT OF TRANSPORTATION** 



# NORTH ABUTMENT



# SOUTH ABUTMENT

# NOTES:

- Contractor shall be responsible for removing and reattaching any utility that falls within any substructure repair.
- Substructure repairs shall be performed when no live load is present over the area of repair work.
- 3. Concrete repair areas shown based on visual inspection. The actual sizes and locations of repairs shall be determined by the Engineer.
- 4. Girder lines 1, 2, 9, and 10 shall be temporarily supported off the North and South Abutments during the concrete repairs below the bearing seats. The temporary supports shall be paid for as "Temporary Shoring and Cribbing". The existing bearing removal at these locations shall not be paid for separately but shall be included in the unit cost of "Temporary Shoring and Cribbing".
- 5. For bearing reactions, see SA-19 and SA-20.

# LEGEND

Si (D

Structural Repair Of Concrete (Depth equal to or less than 5 inches)



Structural Repair Of Concrete (Depth greater than 5 inches)



— Epoxy Crack Injection

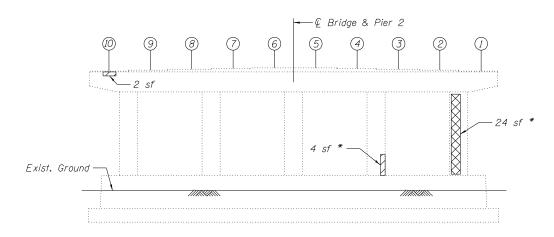
# BILL OF MATERIAL

Item	Unit	Total
Structural Boogin of Congrete (Booth Equal	• • • • • • • • • • • • • • • • • • • •	7 07 07
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	181
Structural Repair of Concrete (Depth Greater than 5 inches)	Sq. Ft.	153
Epoxy Crack Injection	Foot	150
Temporary Shoring and Cribbing	Each	8

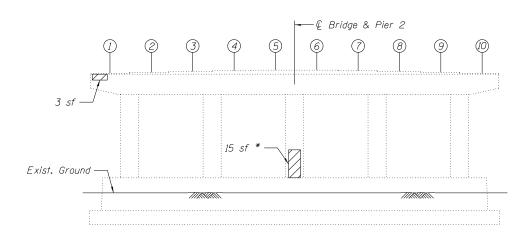


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<u>PIER 2</u> (North Face)

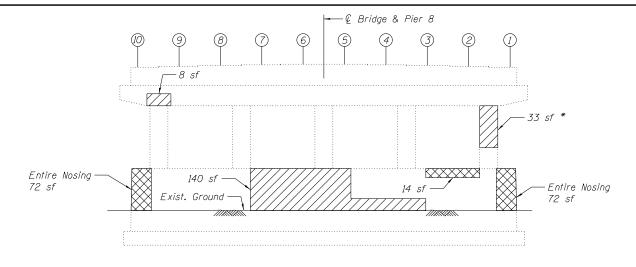


PIER 2

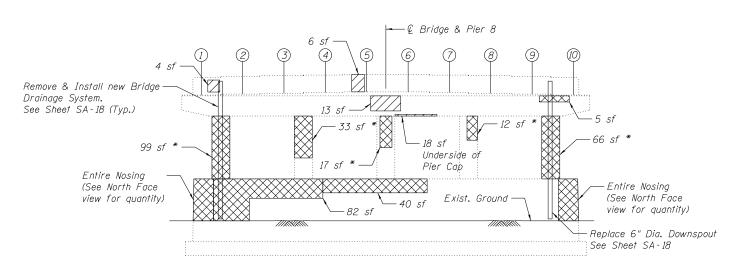
(South Face)

NOTES:

- 1. Existing cribbing is installed near the existing bearings between the concrete pier cap and diaphragms near beams 1, 2, 3, 8 and 10 at Pier 8, Span 9 side. Contractor shall monitor Pier 8 existing bearings and existing cribbing pending bearing replacement.
- 2. Contractor shall be responsible for removing and reattaching any utility that falls within any substructure repair.
- 3. Substructure repairs shall be performed when no live load is present over the area of repair work.
- 4. Concrete repair areas shown based on visual inspection. The actual sizes and locations of repairs shall be determined by the Engineer.
- 5. All girder lines shall be temporarily supported off of Pier 8 during the concrete repairs to the pier cap and columns below. The temporary supports shall be paid for as "Temporary Shoring and Cribbing". The existing bearing removal at these location shall not be paid for separately but shall be included in the unit cost of "Temporary Shoring and Cribbina".
- 6. For bearing reactions, see SA-19 and SA-20.
- 7. Overhead concrete repairs shall be performed using Shotcrete. See Special Provisions.



<u>PIER 8</u> (North Face)



<u>PIER 8</u> (South Face)

\* Fiber Wrap shall be applied to the circumferance of the pier column after structural repair of concrete. See Special Provisions.

# LEGEND



Structural Repair Of Concrete
(Depth equal to or less than 5 inches)



Structural Repair Of Concrete (Depth greater than 5 inches)

# BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	246
Structural Repair of Concrete (Depth Greater than 5 inches)	Sq. Ft.	536
Fiber Wrap	Sq. Ft.	561
Temporary Shoring and Cribbing	Each	20
Acrylic Coating	Sq. Yd.	63

AECOM
303 EAST WACKER DRIVE, SUITE 1400
DHICAGO, IL 60601-5276

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIER REPAIRS
STRUCTURE NO. 016-0195
SHEET NO. SA-22 OF SA-23 SHEETS

 
 FA.U. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

 3730
 15-00131-01-BR
 COOK
 109
 59

 CONTRACT NO. 62W48

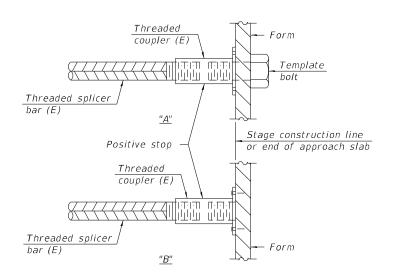
# STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

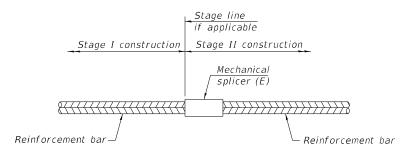
Location	Bar	No. assemblies	Minimum
LUCALIUII	size	required	lap length
South Abutment	#6	8	3'-7"
South Abutment	#5	18	3'-6"
Pier 2	#5	40	3'-6"
Pier 5	#5	40	3'-6"
Pier 8	#5	48	3'-6"
North Abutment	#6	8	3'-7"
North Abutment	#5	18	3'-6"
Beam Encasement	#5	120	3'-0"



# INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



# STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements

for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

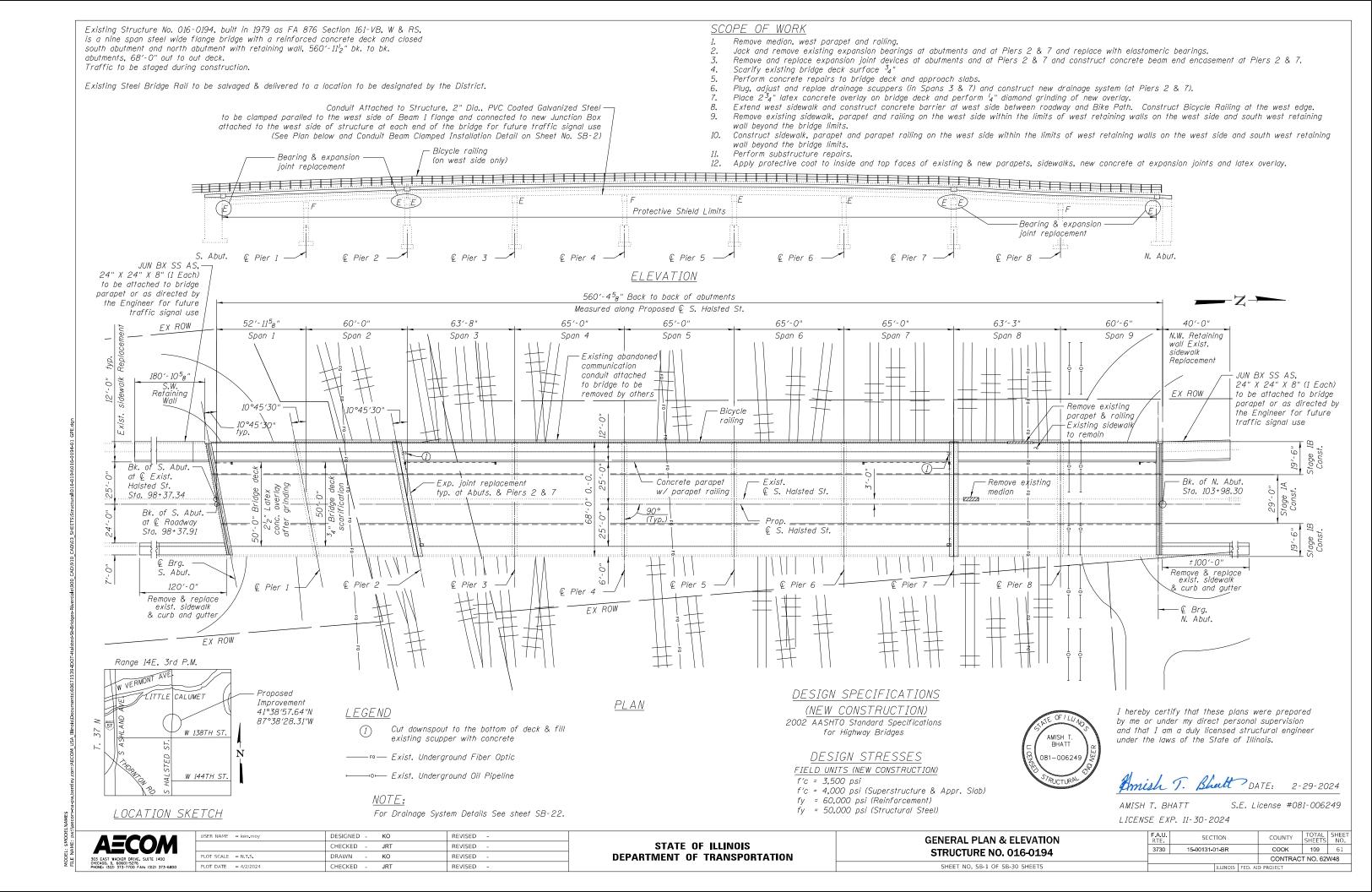
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ILLINOIS FED. AI			D PROJECT			

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**DEPARTMENT OF TRANSPORTATION** 



# GENERAL NOTES

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Reinforcement bars designated (E) shall be epoxy coated.

Existing reinforcement bars extending into the new construction shall be cleaned and straightened prior to incorporating into new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved embedded reinforcement or Mechanical Bar Splicer System. Cost included with Concrete Removal.

Cut existing vertical reinforcement flush with top of west sidewalk at the parapet and top of the deck at the median during removal of west parapet and median and coat the bar ends with epoxy. Level the existing concrete surface smooth with an approved cement grout after removal of the west parapet. The cost of this work is included with Concrete Removal,

All structural steel shall conform to AASHTO Classification M-270 Grade 50. unless otherwise noted.

Fasteners shall be ASTM F 3125 Grade A325 Type 1, mechanically galvanized bolts. Bolts  $\frac{3}{4}$ "  $\phi$ , open holes  $\frac{13}{16}$ "  $\phi$ , unless otherwise noted.

Prior to pouring the new concrete deck or overlay, all heavy or loose rust, loose mill scale and other loose and potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included with Concrete Removal.

Bridge Deck Grooving shall be completed only after Diamond Grinding (Bridge Section) is completed.

Existing raised reflective pavement markers on the bridge deck and approach slab shall be removed and replaced. See Proposed Pacement Marking Plans for location of proposed markers on the bridge structure.

Cost of removal and re-installation of all structural steel members necessary to complete the work as detailed on the plans and as specified in the Special Provisions shall be included with Furnishing and Erecting Structural Steel.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

All new structural steel and bearing assemblies shall be hot-dip galvanized. See Special Provisions for Hot Dip Galvanizing for Structural Steel.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.

No free fall deck drains will be permitted in the spans over the tracks.

Substructure repairs and bearing replacement shall be performed under staged construction when no live load is present.

The Contractor is advised that the existing structure may contain members that are in deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the existing structure when developing construction procedures. An existing structure information package will be provided by the Department to the Contractor upon request.

# INDEX OF SHEETS

SB-29 Piers 7 & 8 Repairs

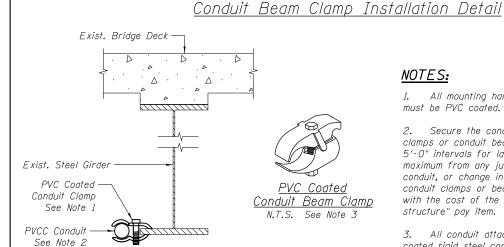
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SB-1	General Plan & Elevation
SB-2	Index of Sheets, General Notes & Bill of Materials
SB-3	Stage Construction Details 1
SB-4	Stage Construction Details 2
SB-5	Temporary Concrete Barrier
SB-6	Superstructure Plan
SB-7	Superstructure Sections
SB-8	Superstructure Details 1
SB-9	Superstructure Details 2
SB-10	Superstructure Details 3
SB-11	Bicycle Railing & Parapet Railing
SB-12	Existing Expansion Joint Removal
SB-13	Expansion Joint Replacement at South Abutment
SB-14	Expansion Joint Replacement at Pier 2
SB-15	Expansion Joint Replacement at Pier 7
SB-16	Expansion Joint Replacement at North Abutment
SB-17	Beam Encasement Cross Section Piers 2 & 7
SB - 18	Preformed Joint Strip Seal
SB-19	Preformed Joint Strip Seal
SB-20	Adjusting Existing Scupper
SB-21	Drainage Scupper, DS-11
SB-22	Drainage System
SB-23	Type I Bearing Details
SB-24	Type I Bearing Details
SB-25	Abutment Repairs
SB-26	Piers 1 & 2 Repairs
SB-27	Piers 3 & 4 Repairs
SB-28	Piers 5 & 6 Repairs

SB-30 Bar Splicer Assembly and Mechanical Splicer Details

# TOTAL BILL OF MATERIAL

DAY ITEMS	UNIT	BRIDGE REPAIR		BIKE PATH		TOTALS
PAY ITEMS		SUPER	SUB	SUPER SUB		
Subbase Granular Material, Type B 6"	Sq. Yd.	39				39
Portland Cement Concrete Sidewalk 5 Inch	Sq. Ft.	352				352
Sidewalk Removal	Sq. Ft.			973		973
Concrete Removal	Cu. Yd.	41.7		91.7		133.4
Protective Shield	Sq. Yd.	4,004				4,004
Concrete Superstructure	Cu. Yd.	106.1		238.2		344.3
Protective Coat	Sq. Yd.	3,738		1,588		5,326
Furnishing and Erecting Structural Steel	Pound	11,490				11,490
Reinforcement Bars, Epoxy Coated	Pound	11,180		25,790		36,970
Bar Splicers	Each	204				204
Bicycle Railing	Foot			789		789
Parapet Railing	Foot			1,379		1,379
Preformed Joint Strip Seal	Foot	270				270
Elastomeric Bearing Assembly, Type I	Each	60				60
Anchor Bolts, 1"	Each	120				120
Epoxy Crack Sealing	Foot		208			208
Fiber Wrap	Sq. Ft.		970			970
Steel Railing Removal	Foot			789		789
Bridge Drainage System	L Sum	0.5				0.5
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	2,937				2,937
Approach Slab Repair (Partial Depth)	Sq. Yd.	37				37
Jack and Remove Existing Bearings	Each	22				22
Combination Curb and Gutter Removal and Replacement Greater Than 10 Feet	Foot	220				220
Bridge Deck Latex Concrete Overlay, 2 3/4 Inches	Sq. Yd.	3,014				3,014
Bridge Deck Scarification 3/4"	Sq. Yd.	3,376				3,376
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.		709			709
Structural Repair of Concrete (Depth Greater Than 5 Inches)	Sq. Ft.		1,130			1,130
Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	22		5		27
Drainage Scuppers, DS-11	Each					4
Drainage Scuppers to Be Adjusted	Each			4		2
Diamond Grinding (Bridge Section)	Sa. Yd.	2,865		2		2,865
Temporary Shoring and Cribbing	Each	38				38
Conduit Attached to Structure, 2" Dia., PVC Coated Galvanized Steel	Foot	600				600
Junction Box, Stainless Steel, Attached to Structure, 24" X 24" X 8"	Each	2				2
Acrylic Coating	Sq. Yd.		108			108

<sup>\*</sup> See Temporary Shring and Cribbing notes on Sheet SB-24 thru. SB-26



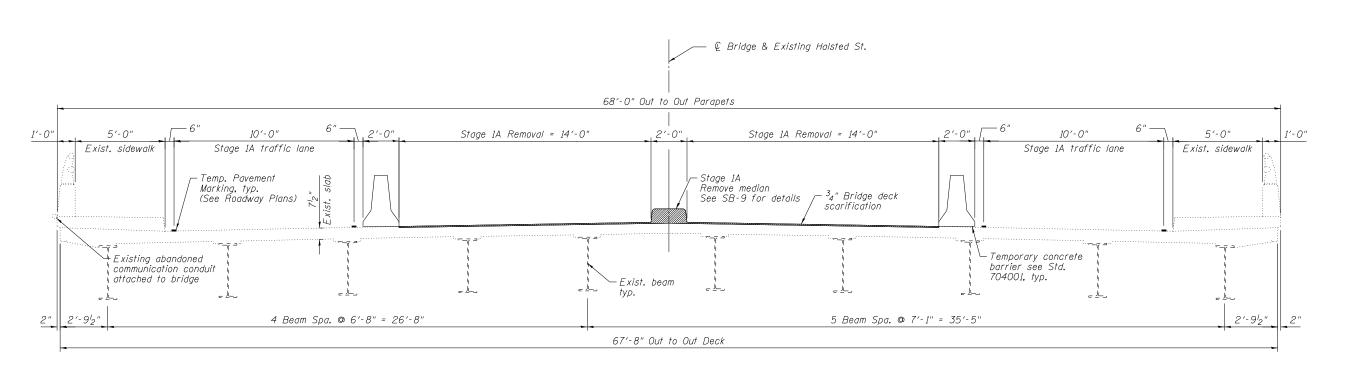
# NOTES:

- 1. All mounting hardware for the PVCC RGC conduit must be PVC coated.
- 2. Secure the conduit with PVC coated conduit clamps or conduit beam clamps as shown at 5'-0" intervals for laterals and within 2'-0" maximum from any junction box, flexible conduit, or change in direction. All PVC coated conduit clamps or beam clamps will be included with the cost of the "conduit attached to structure" pay item.
- 3. All conduit attached to structure shall be PVC coated rigid steel conduit (PVCC RGC) typical.

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PLOT SCALE = N.T.S.	DRAWN - KO	REVISED -	
PLOT DATE = 5/10/2024	CHECKED - MCC	REVISED -	

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  INDEX OF SHEETS, GENERAL NOTES & BILL OF MATERIAL **STRUCTURE NO. 016-0194** SHEET NO. SB-2 OF SB-30 SHEETS

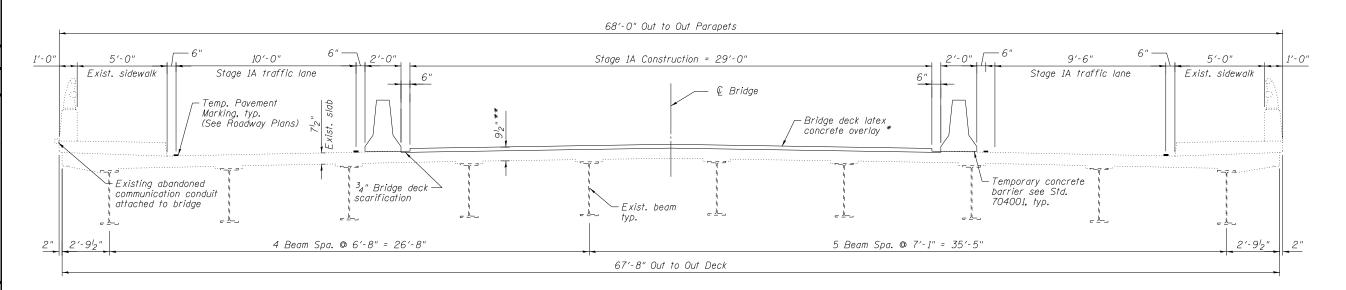
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3730	15-00131-01-BR			соок	109	62
				CONTRAC	T NO. 62	W48
ILLINOIS FED, AI				D PROJECT		



STAGE 1A REMOVAL AND TRAFFIC (Looking North)

<u>LEGEND</u>





STAGE 1A CONSTRUCTION AND TRAFFIC (Looking North)

\* Place  $2^3_4$ " latex concrete overlay on bridge deck. Final overlay thickness shall be  $2^l_2$ " after  $^l_4$ " Diamond Grinding. (See Special Provisions.)

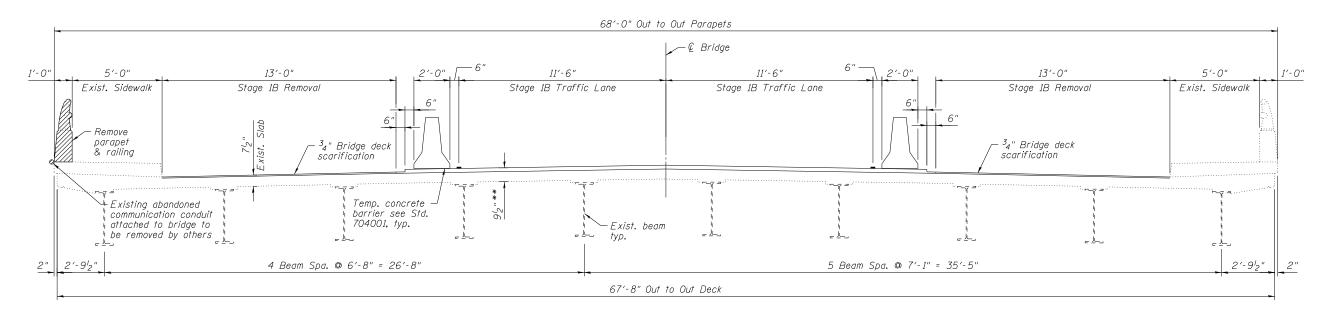
\*\* Prior to grinding

AECOM
303 EAST WACKER DRIVE, SUITE 1400
CHICAGO, II. 60601-5276
PHOBE: 13/23 733-7700 FAX: (3)22 373-6800

USER NAME = ken.moy	DESIGNED - NPP	REVISED -
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STAGE CONSTRUCTION DETAILS 1 STRUCTURE NO. 016-0194
SHEET NO. SR-3 OF SR-30 SHEETS

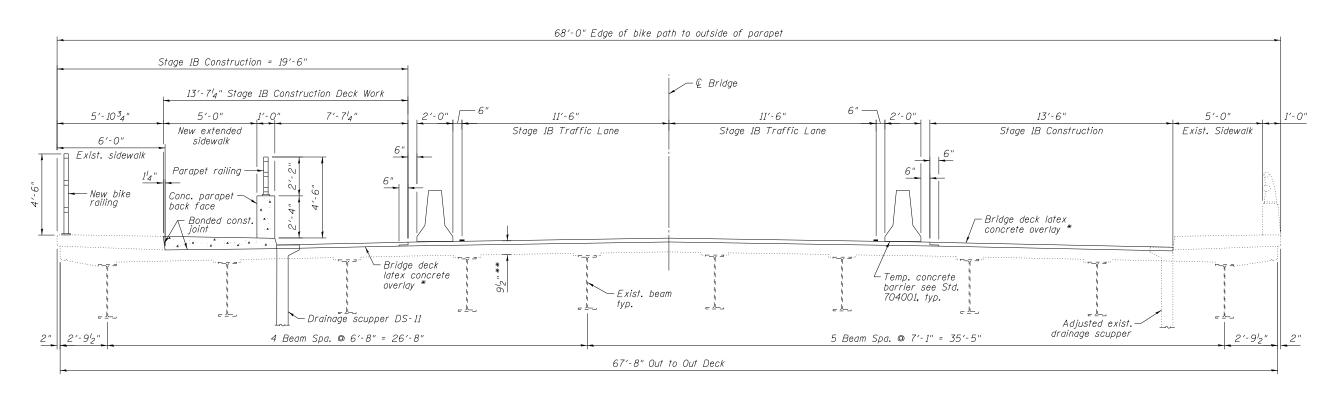
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3730	15-00131-01-BR			соок	109	63
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STAGE 1B REMOVAL AND TRAFFIC (Looking North)

<u>LEGEND</u>

Indicates removal



STAGE 1B CONSTRUCTION AND TRAFFIC (Looking North)

\* Place  $2^3_4$ " latex concrete overlay on bridge deck. Final overlay thickness shall be  $2^l_2$ " after  $^l_4$ " Diamond Grinding. (See Special Provisions.)

\*\* Prior to grinding

<b>AECOM</b>
303 EAST WACKER DRIVE, SUITE 1400 CHICAGO, IL 60601-5276 PHONE: (312) 373-7700 FAX: (312) 373-6800

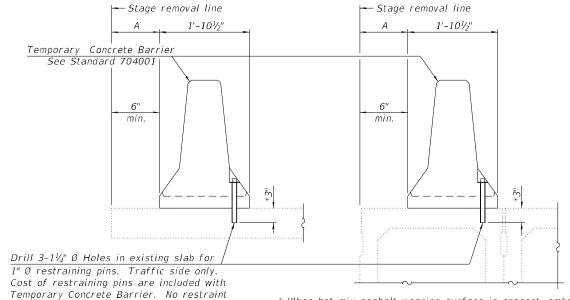
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PLOT DATE = 5/10/2024	CHECKED - MCC	REVISED -

STAGE CONSTRUCTION DETAILS 2						
<b>STRUCTURE NO. 016-0194</b>						
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				CONTRAC	T NO. 62	N48
		ILLINOIS	FED. AI	D PROJECT		

barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

NEW SLAB OR NEW DECK BEAM



\* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

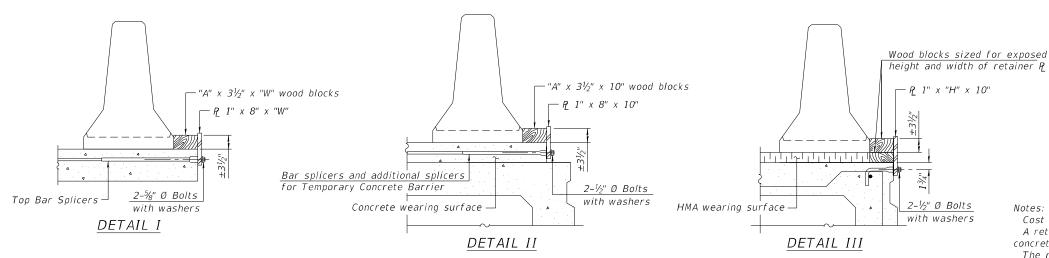
EXISTING DECK BEAM

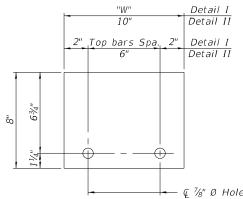
# 1x8 UNC US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 gauge thick washer RESTRAINING PIN

# SECTIONS THRU SLAB OR DECK BEAM

is required when "A" is greater than 3'-1".

EXISTING SLAB

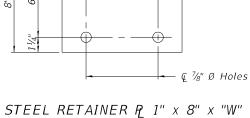




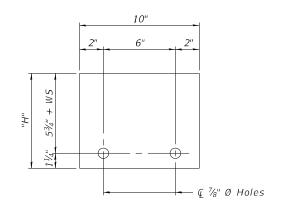
# RAILING CRITERIA

NCHRP 350 Test Level Railing Weight (plf)

10-12-2021



(Detail I and II)



STEEL RETAINER P 1" x "H" x 10" (Detail III)

# BAR SPLICER FOR #4 BAR - DETAIL III

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than  $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

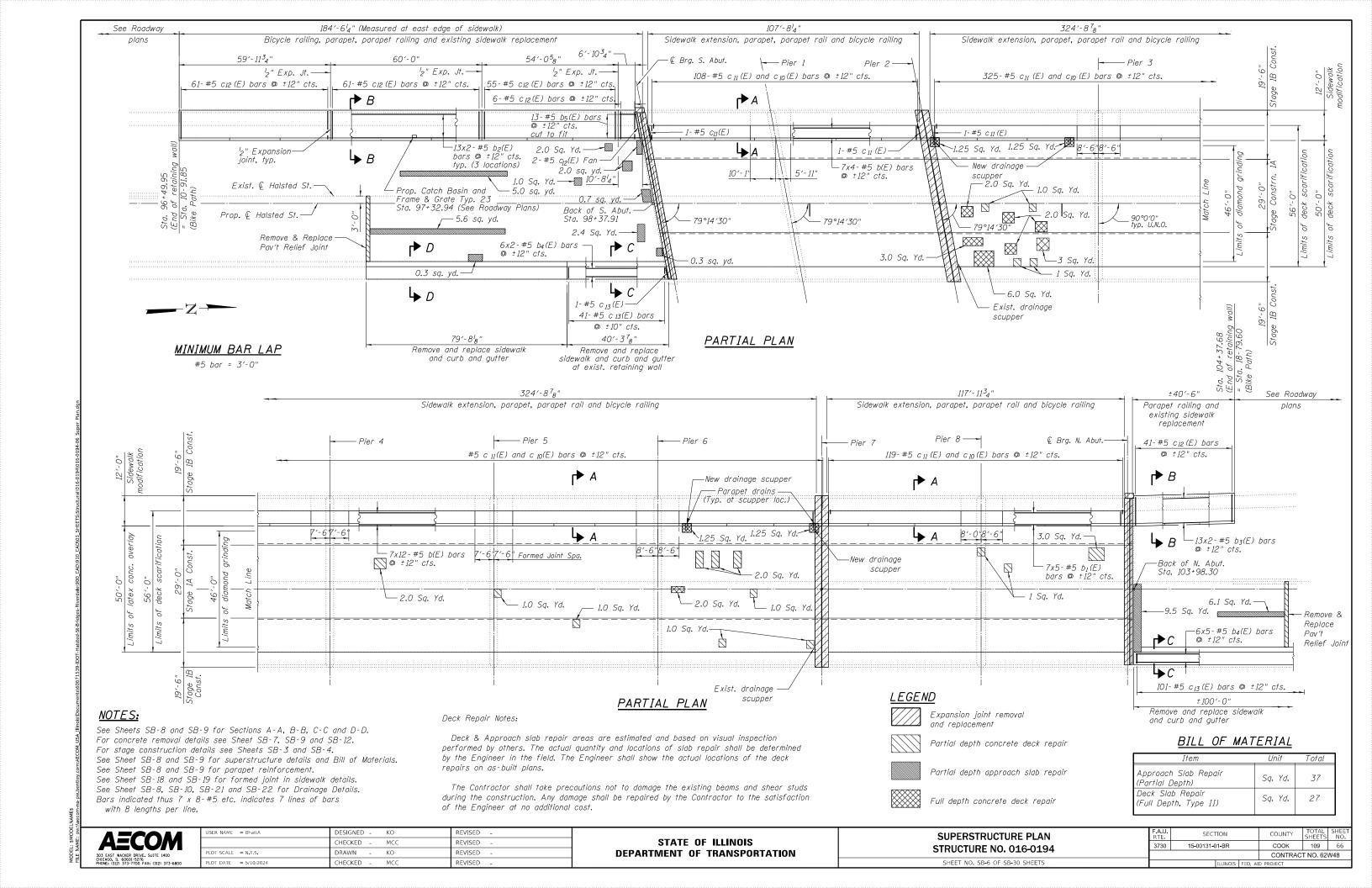
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PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

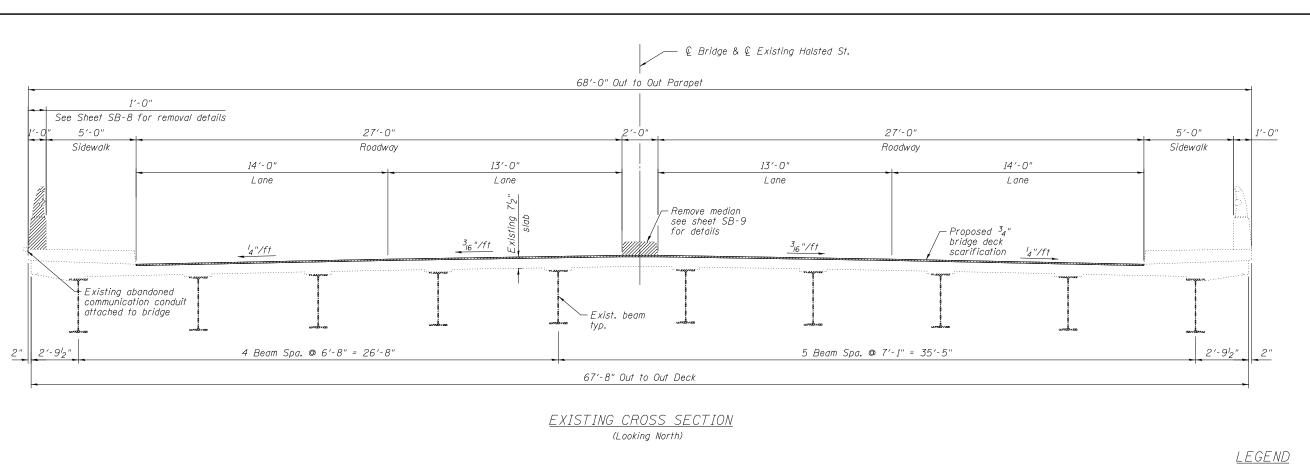
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

TEMPORARY CONCRETE BARRIER	
STRUCTURE NO. 016-0194	

F.A.U. SECTION		COUNTY	TOTAL SHEETS	SHEET NO.			
	3730 15-00131-01-BR		соок	109	65		
			CONTRACT NO. 62W48				
			ILLINOIS	FED. AI	D PROJECT		

SHEET NO. SB-5 OF SB-30 SHEETS







SECTION

15-00131-01-BR

3730

SUPERSTRUCTURE SECTIONS

STRUCTURE NO. 016-0194

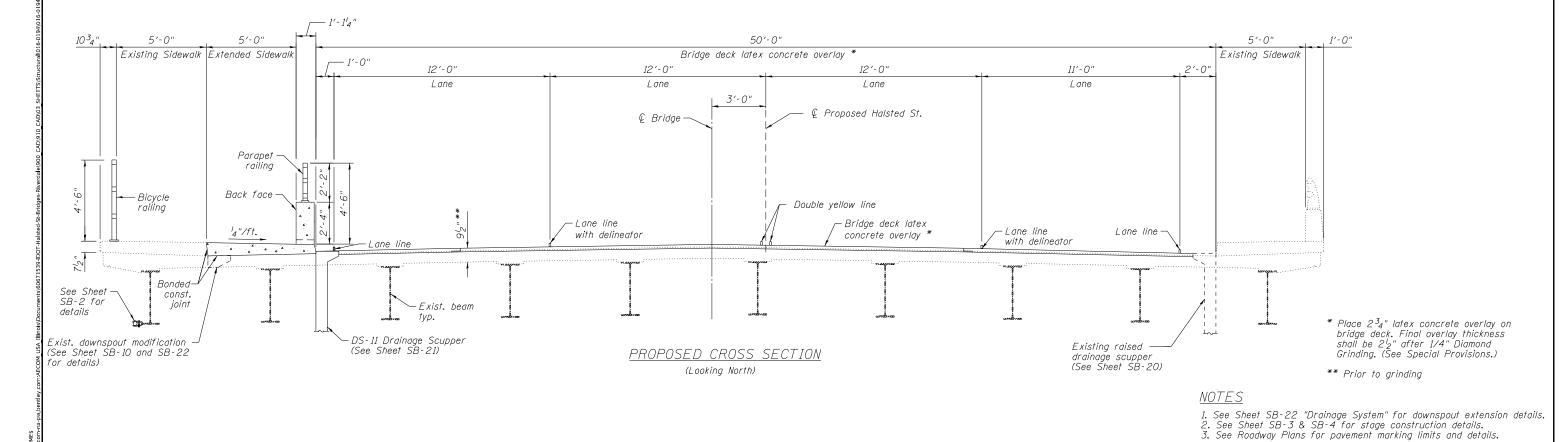
SHEET NO. SB-7 OF SB-30 SHEETS

COUNTY

соок

CONTRACT NO. 62W48

109 67



STATE OF ILLINOIS

**DEPARTMENT OF TRANSPORTATION** 

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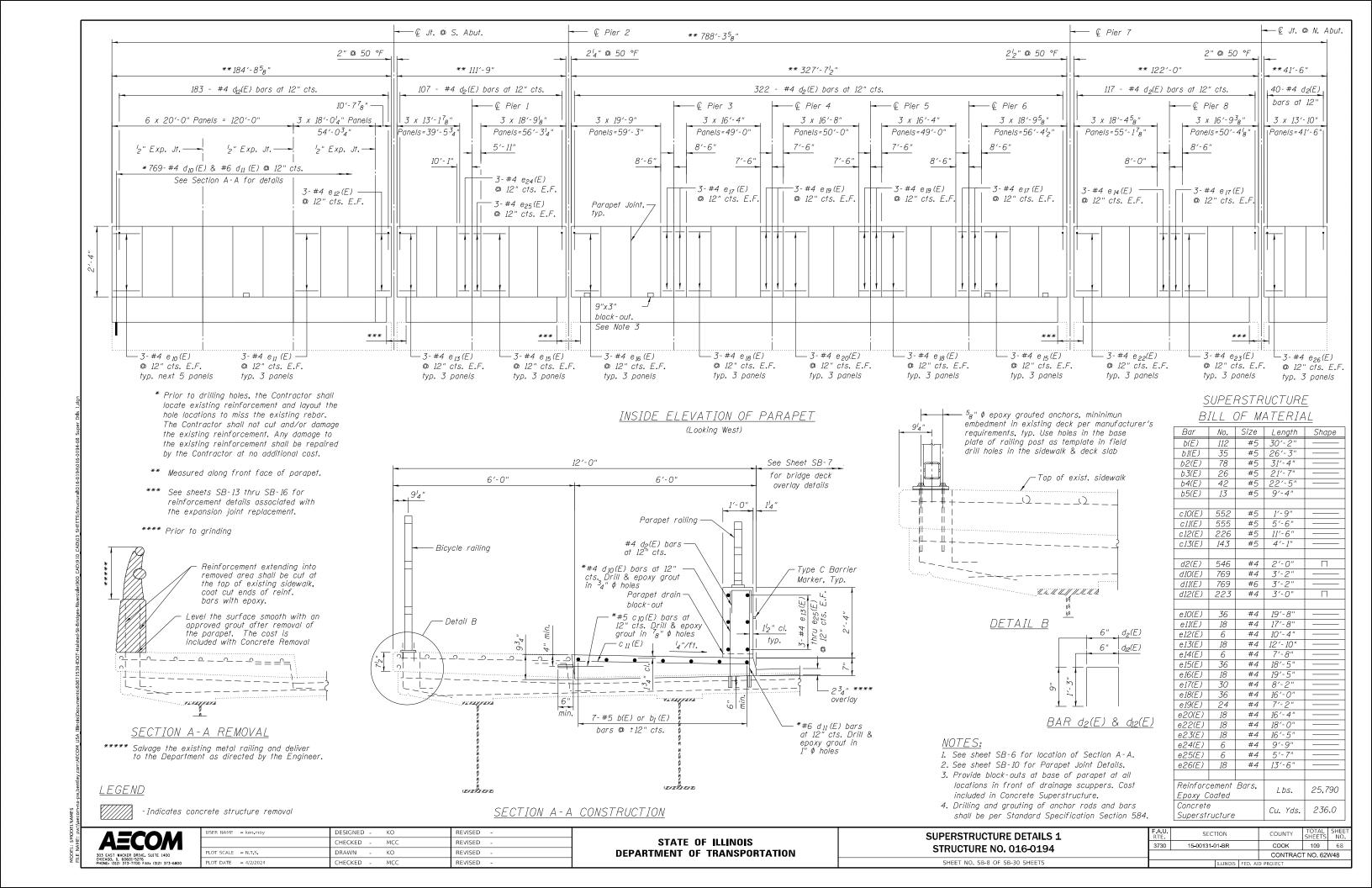
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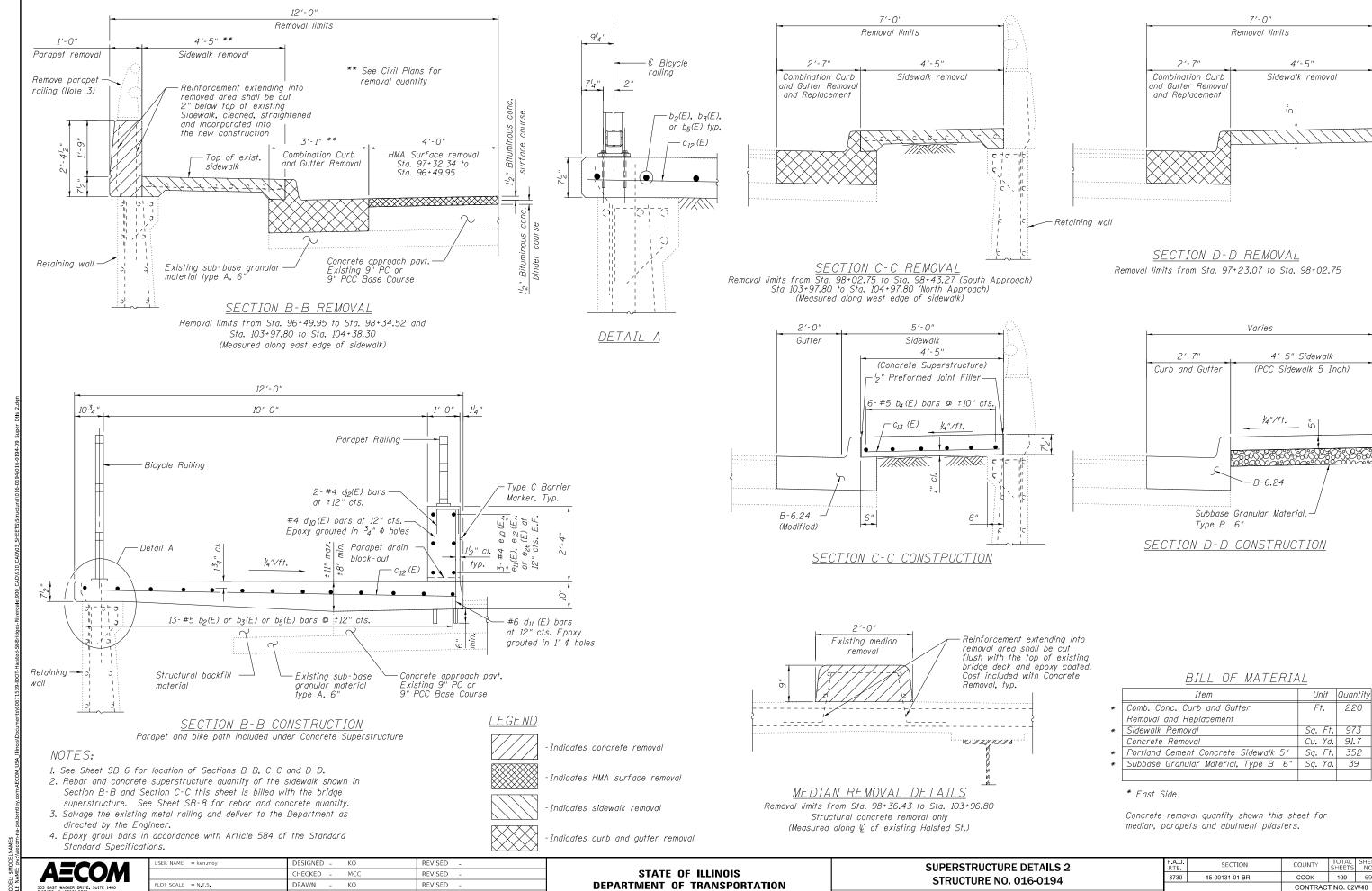
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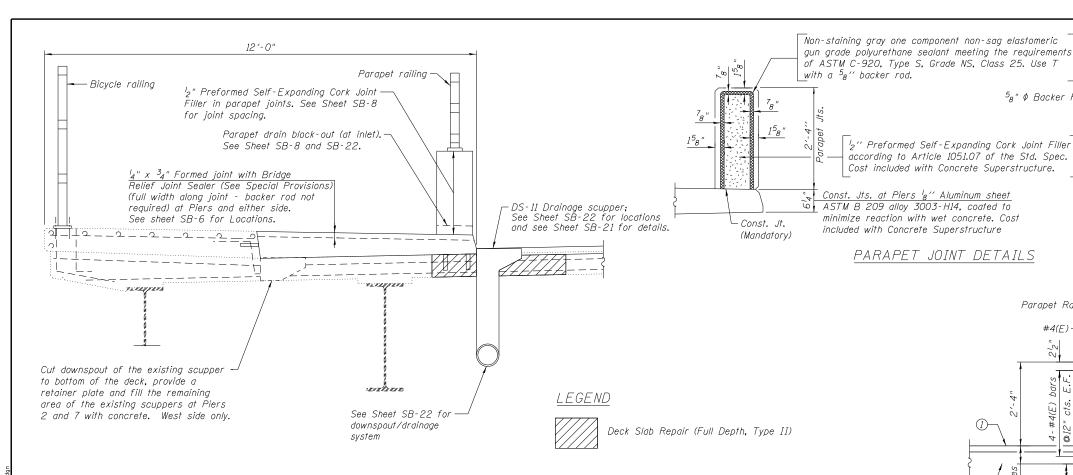
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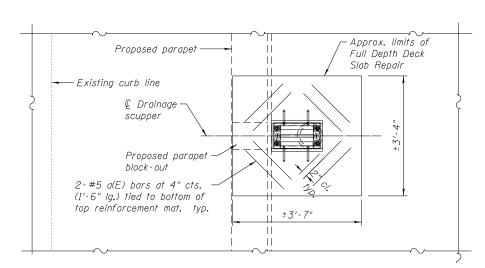


SHEET NO. SB-9 OF SB-30 SHEETS

CHECKED



# SECTION THRU PARAPET



# TOP PLAN AT DRAINAGE SCUPPER

### Notes:

Deck removal and replacement for drainage scupper installation shall be included with Deck Slab Repair (Full Depth, Type II). See Sheet SB-8 for quantity.

The cost for reinforcement bars for the drainage scuppers shall be included in the cost for Drainage Scuppers, DS-11.

Drainage scupper top elevation shall be recessed to accomodate diamond grinding. Place drainage scupper  ${}^{l}_{4}$ " below the top of slab.

# LEGEND

- HMA Surface Course (Prop. Bike Path)
- Aggregate Base Course (Prop. Bike Path)
- #6 (E) Bars are epoxy grouted in 1" \$\phi\$ holes.
- \*\* #5 (E) Bars are epoxy grouted in <sup>3</sup>4" \$\phi\$ holes

# CONCRETE BARRIER WALL (SPECIAL)

typ.

Sta. 9+24.44 to Sta. 10+91.85 Sta. 18+79.60 to Sta. 20+59.87

1'-0"

Type C Barrier Marker, Typ.

<sup>5</sup><sub>8</sub>" ¢ Backer Rod

Parapet Rail #4(E)

Vertical Sawcut -

Pavement Removal

Varies (5'-0" Max. min. 1'-7<sup>1</sup><sub>4</sub>")

Payment Limits for Concrete

Barrier Base (Special)

12" Preformed Self-Expanding Cork Joint Filler

according to Article 1051.07 of the Std. Spec. Cost included with Concrete Superstructure.

PARAPET JOINT DETAILS

# Parapet Rail #4(E) Type C Barrier #5 d(E) -Marker, Typ. #6 d(E) -Dowel Bars #8 @ 24" cts. $1_2^{\prime}$ c/. Exist. 2<sup>1</sup><sub>2</sub>" Binder/Surface or CC 6 B-6.24 Existing PCC Base Course 9" 4-#4(E) bars Тур. @18" cts.

Exist. 21/2" Binder/Surf.

or Drop HMA Surf. 1<sup>3</sup>4"

#6 (E) bars at 12" cts.\*

#5 (E) bars at 12" cts.\*\*

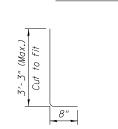
(Sta. 18+79,58 to Sta. 19+43.31

— Existing PCC Base Course 9" or exist. PCC Approach Pavt.

(Sta. 18+79.58 to Sta.19+43.31)

Reinforcement bars and dowel bars are to be provided per detail and shall be included in the contract cost per foot for "Concrete Barrier Wall (Special) & "Concrete Barrier Base (Special)".

Epoxy grout bars in accordance with Article 584 of the Standard Specifications. See roadway plans for "Concrete Barrier Wall (Special)" stationing, additional details and quantities.



BAR #4(E)

CONCRETE BARRIER WALL (SPECIAL)

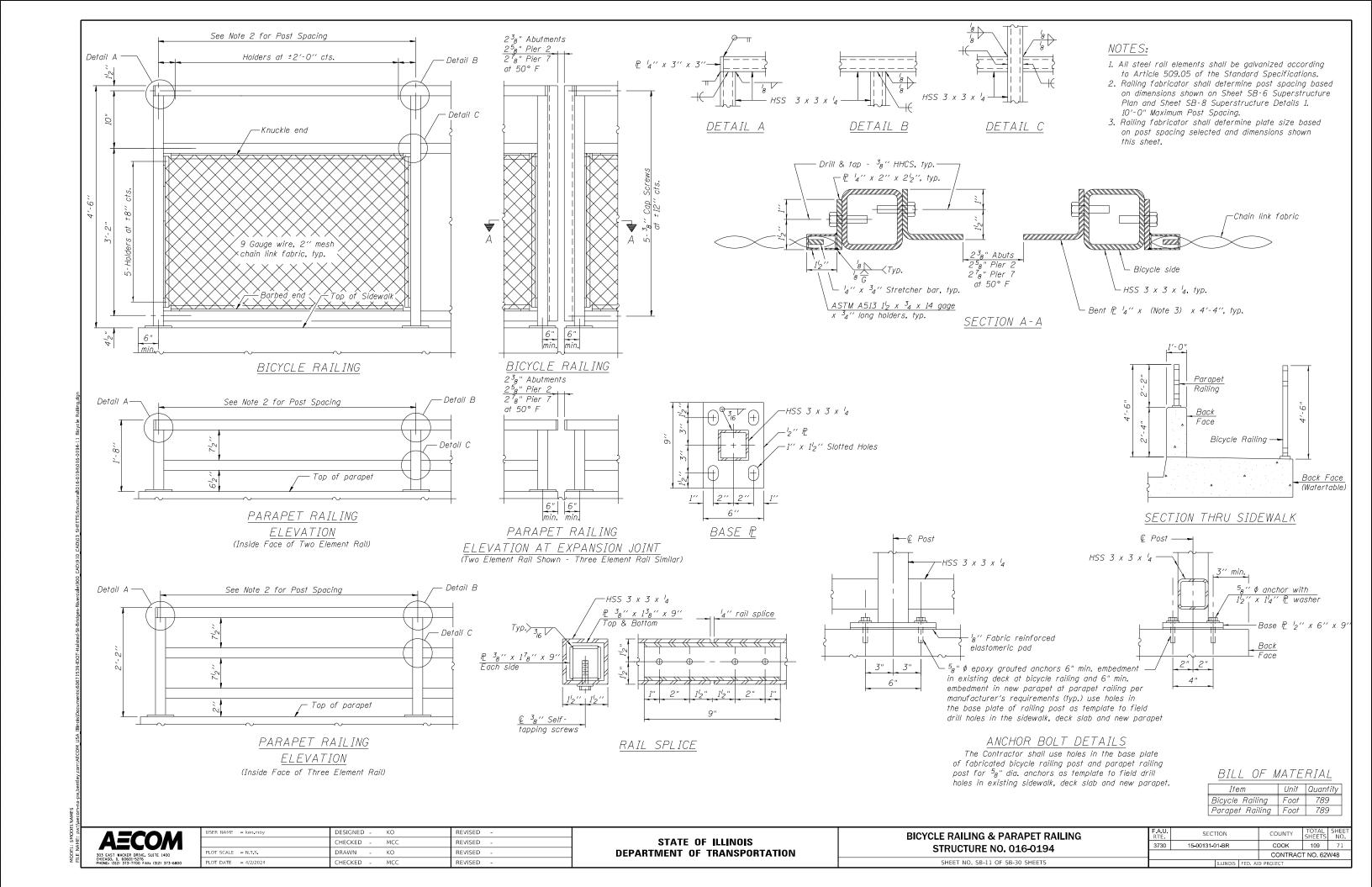
(Stationing based on bike path layout) Sta. 7+83.69 to Sta. 9+24.44 Sta. 20+59.87 to Sta. 21+60.69

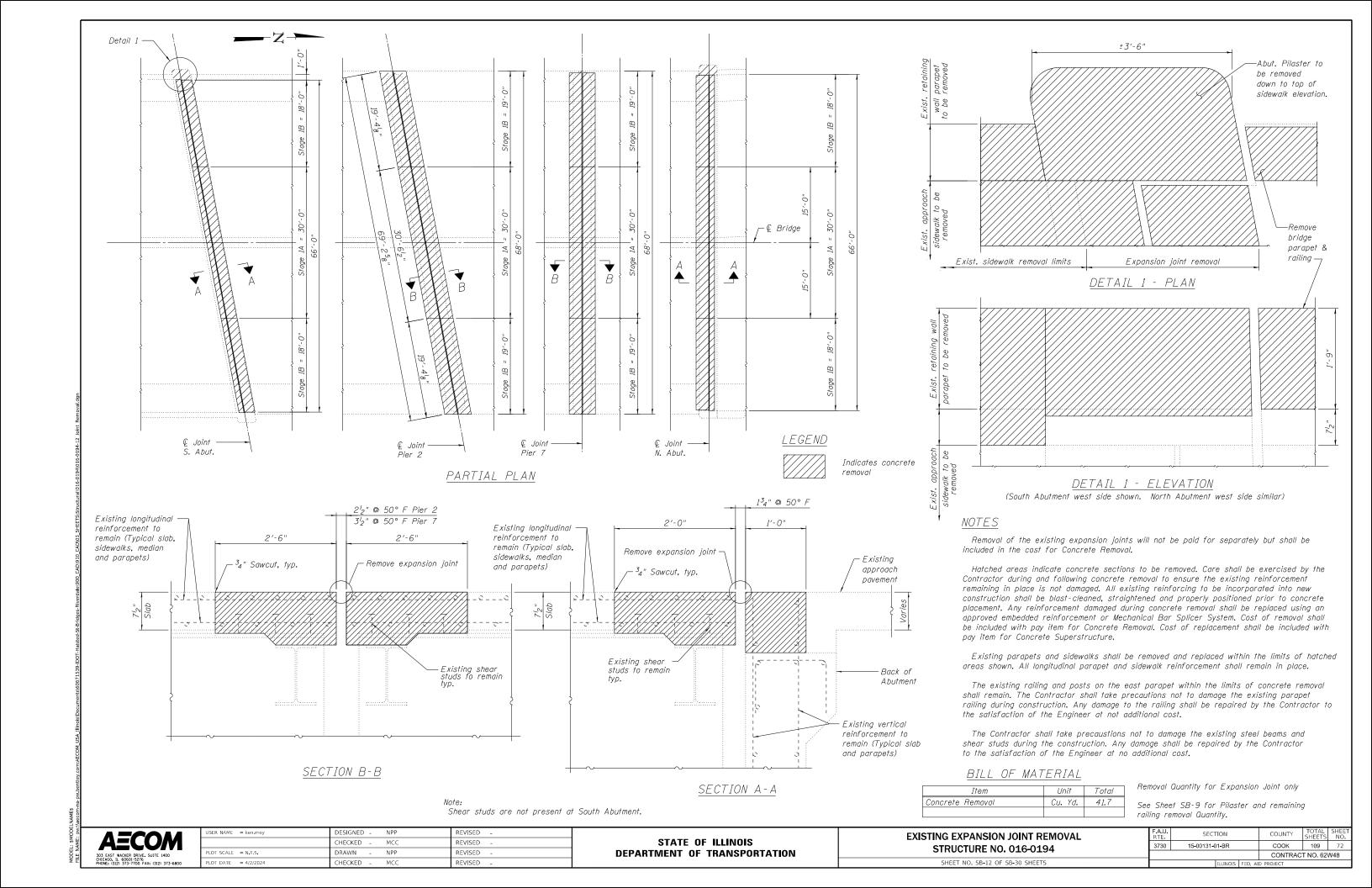
BARS #5 d(E) & #6 d(E)

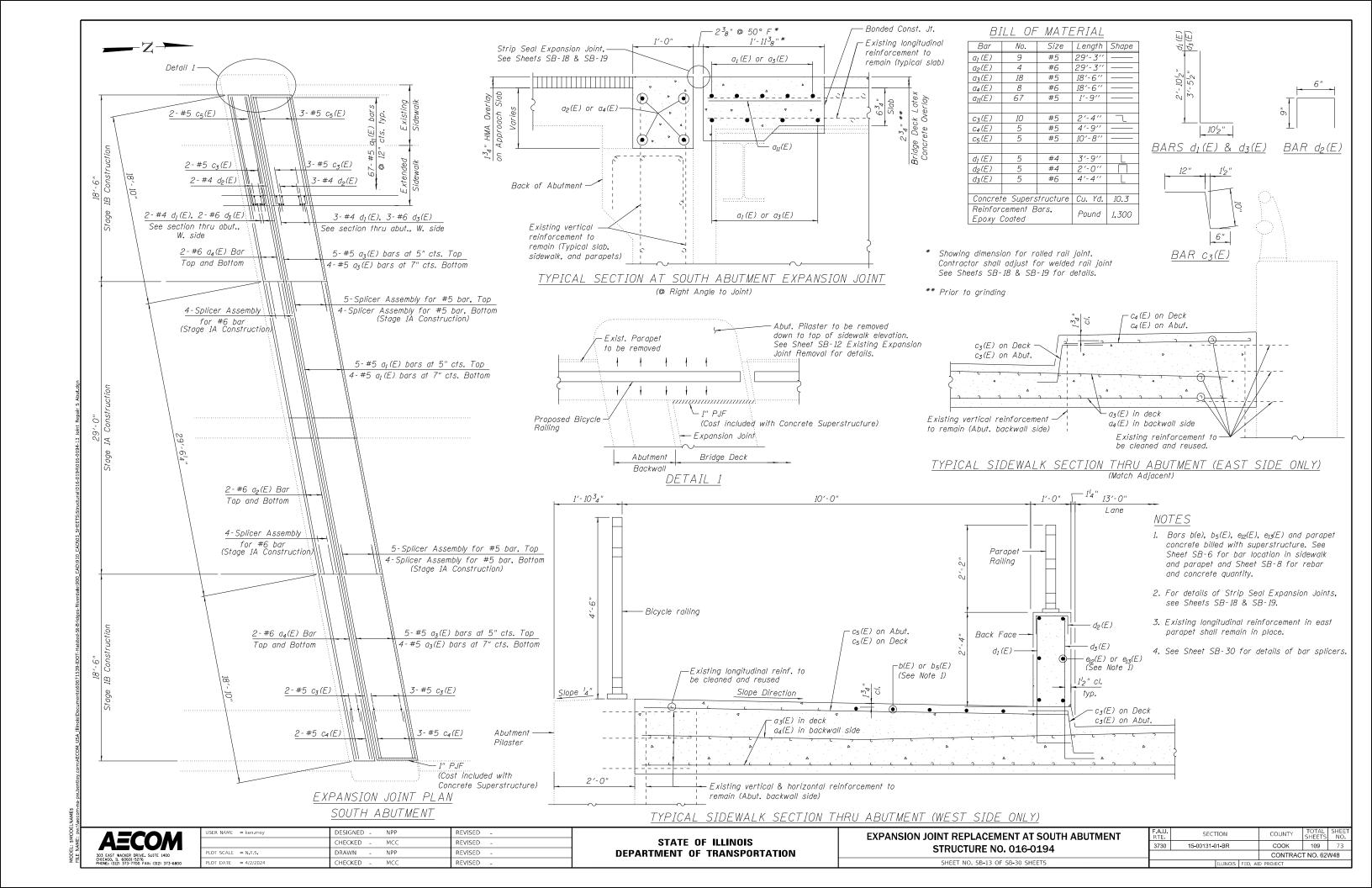


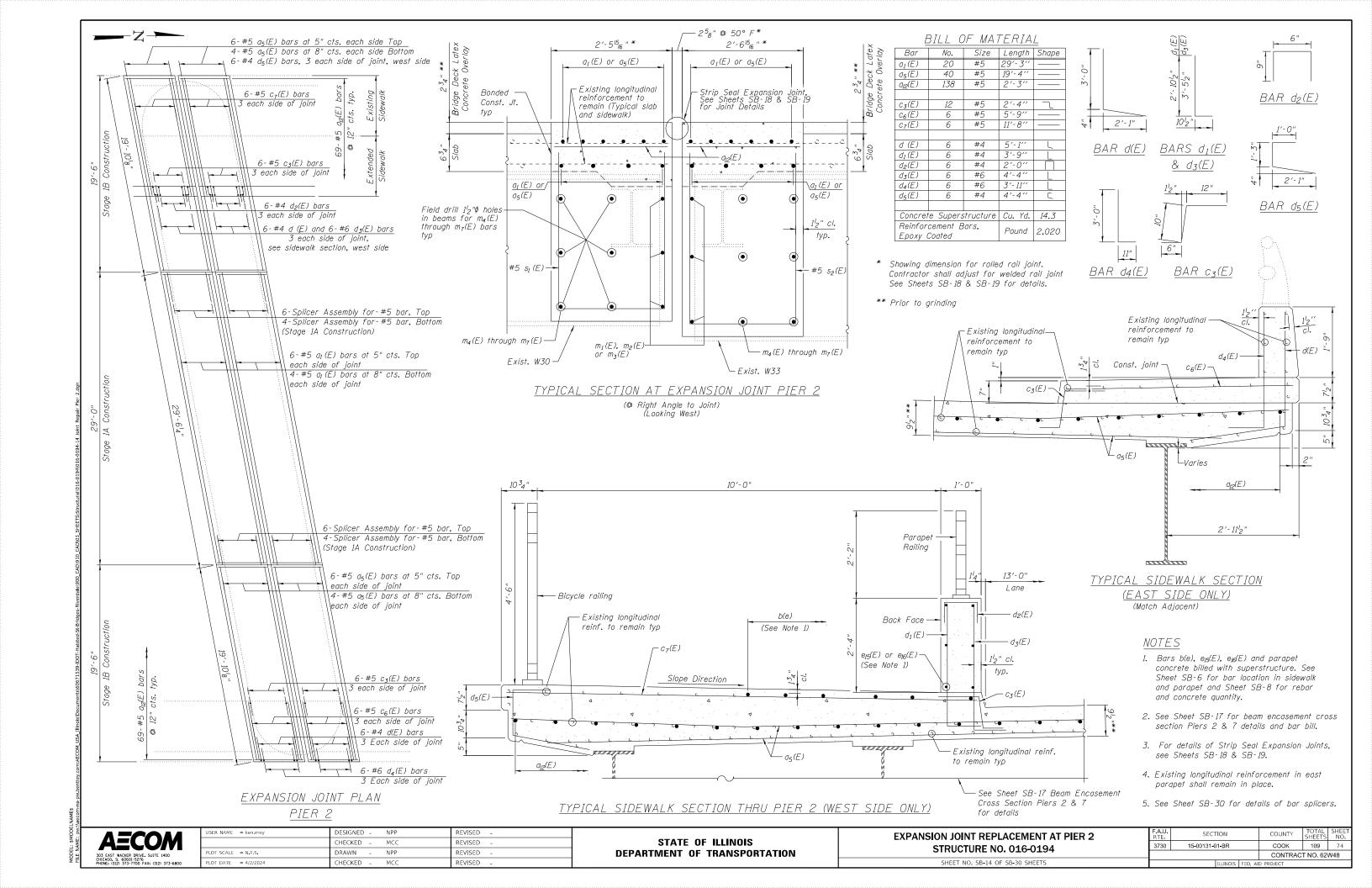
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PLOT DATE = 4/2/2024	CHECKED - MCC	REVISED -

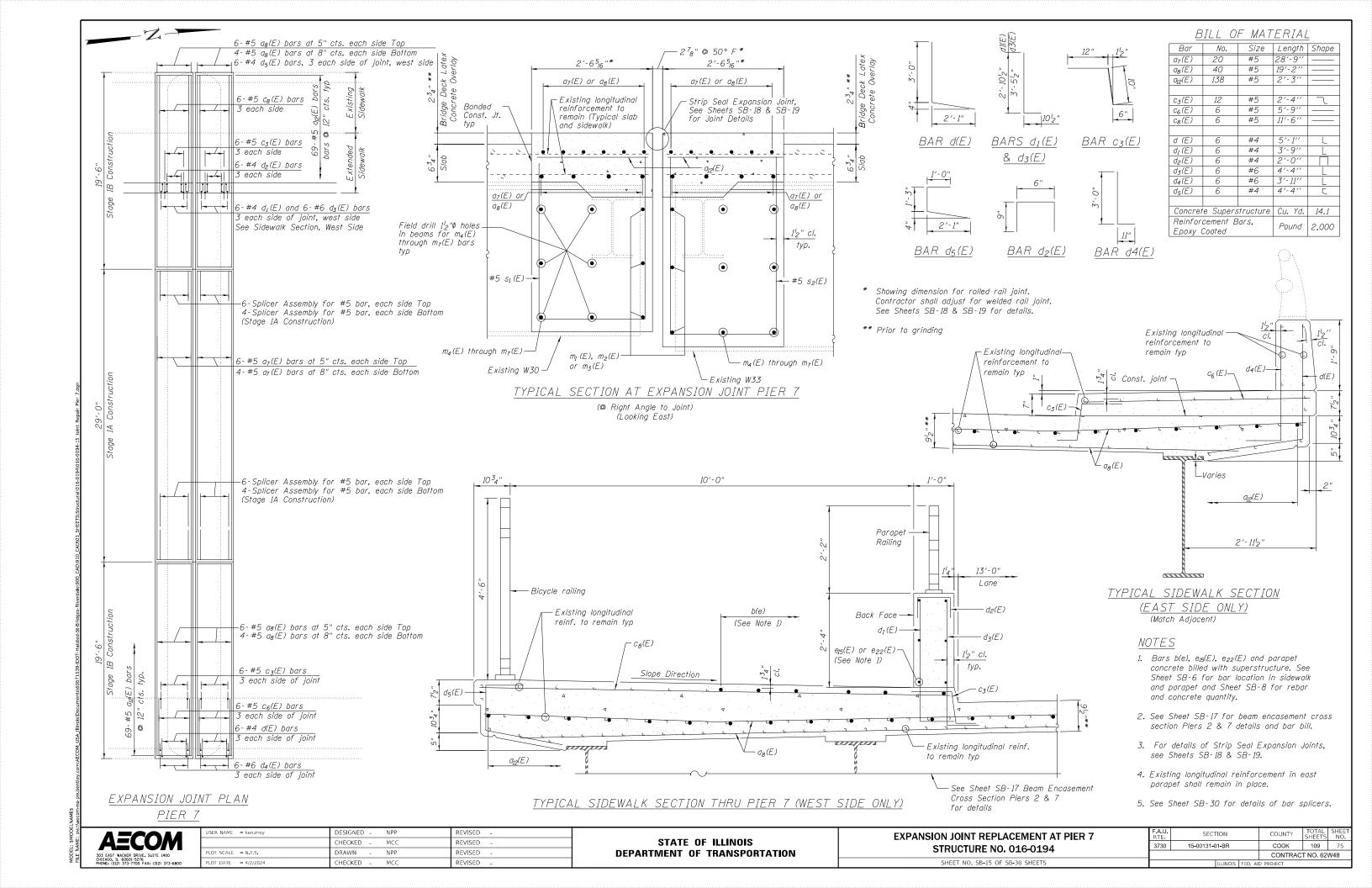
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		CONTRACT NO. 62W48			
ILLINOIS FED AID PROJECT					

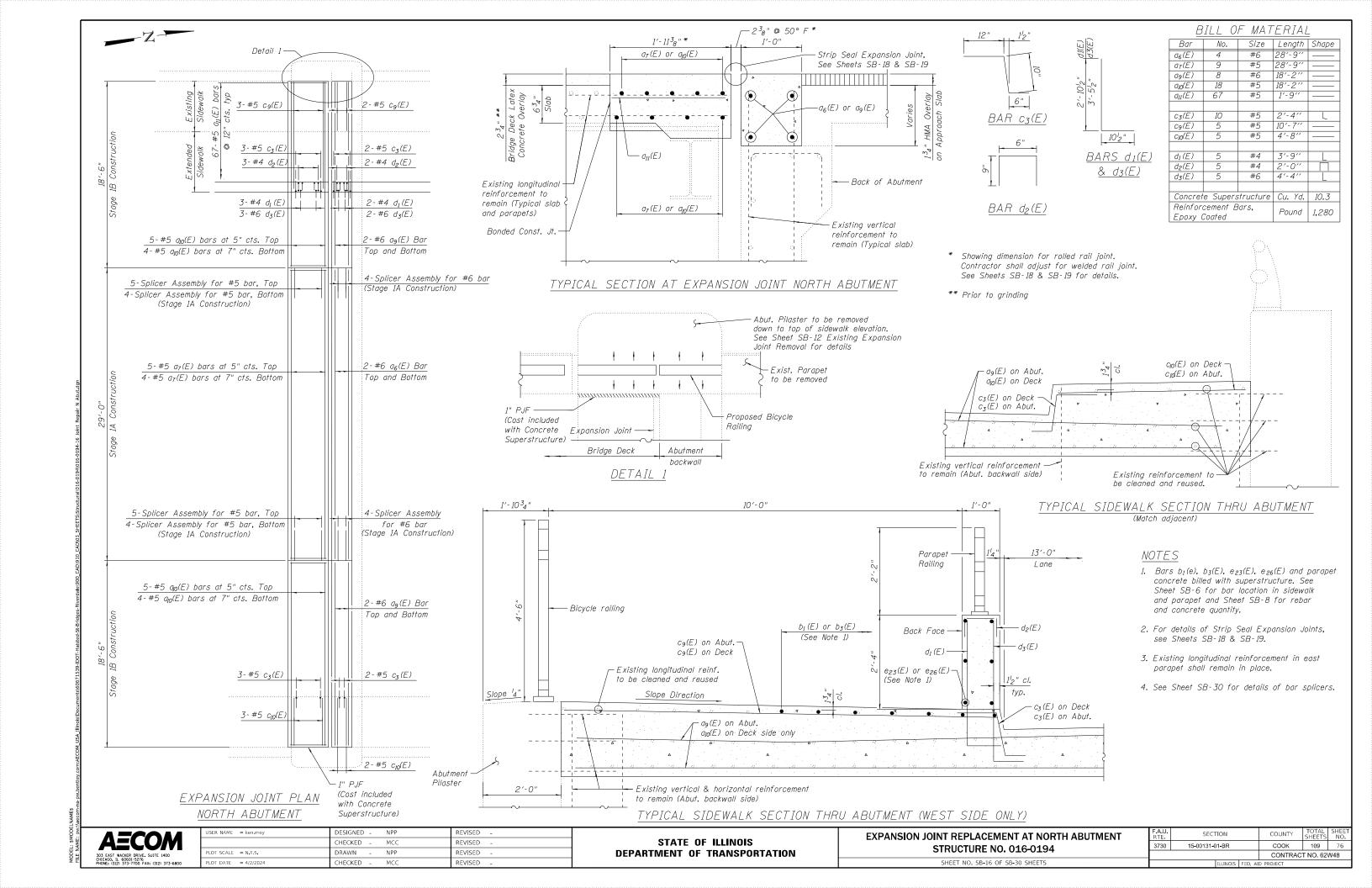


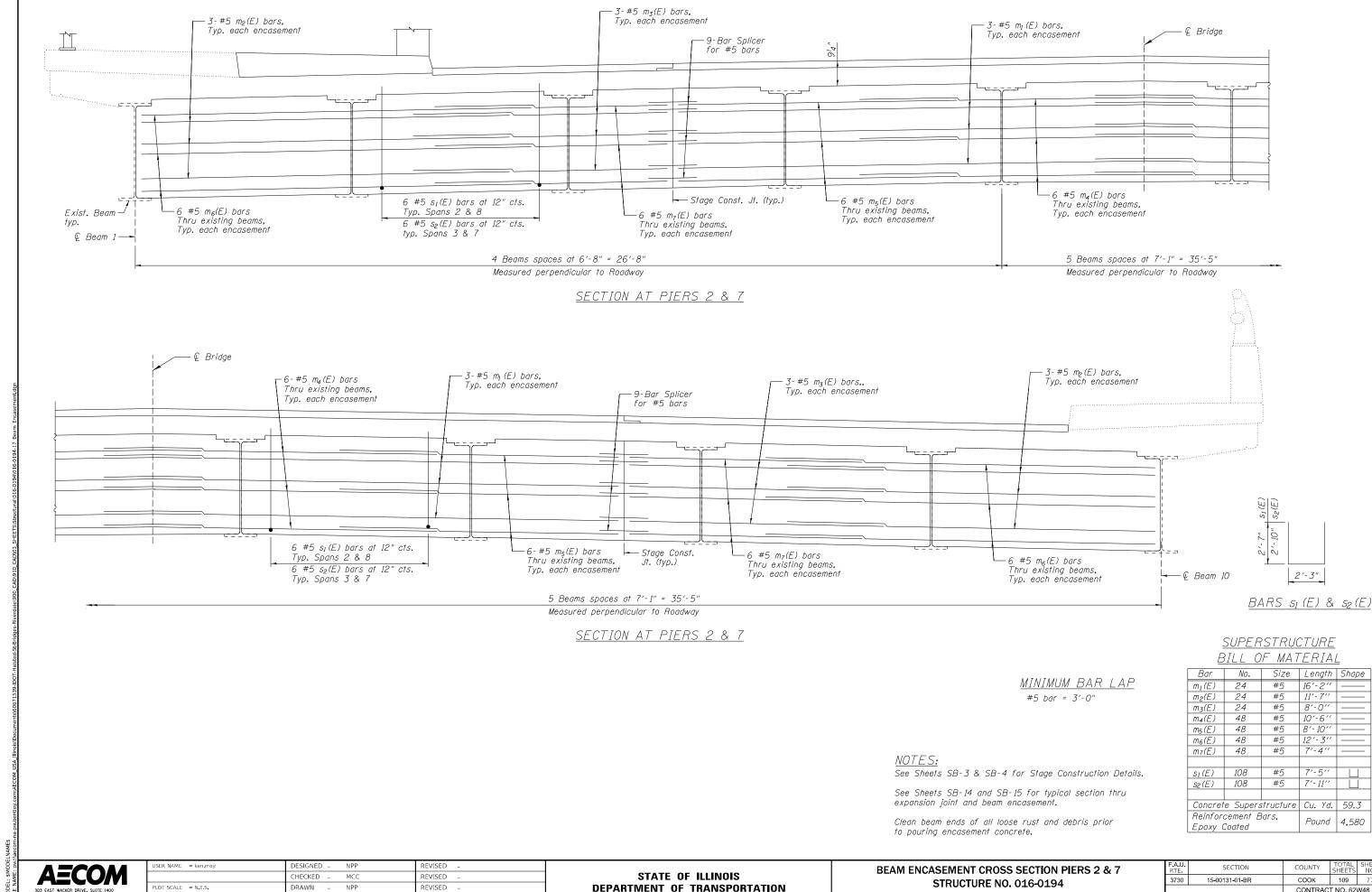












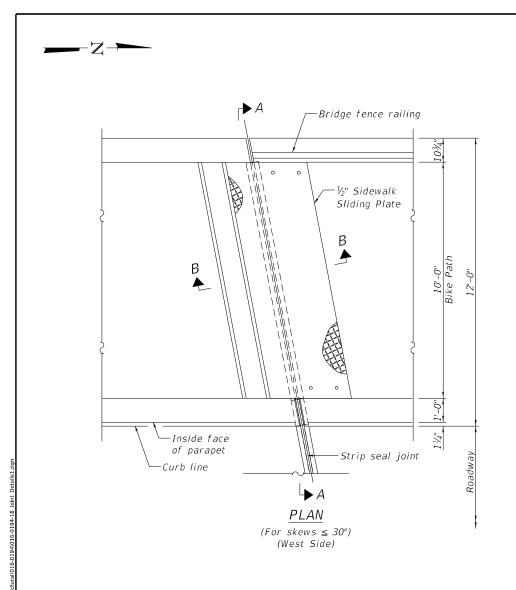
CONTRACT NO. 62W48

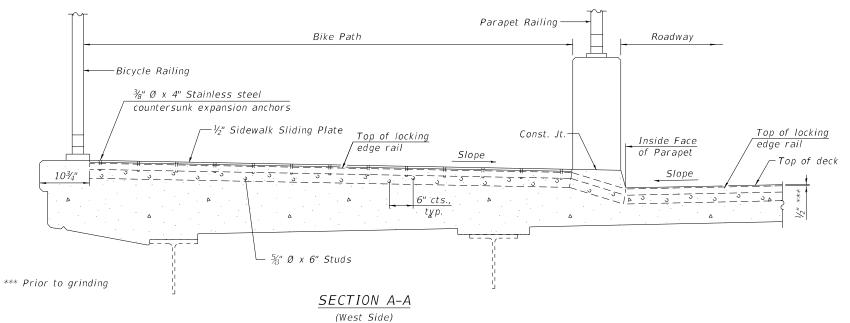
SHEET NO. SB-17 OF SB-30 SHEETS

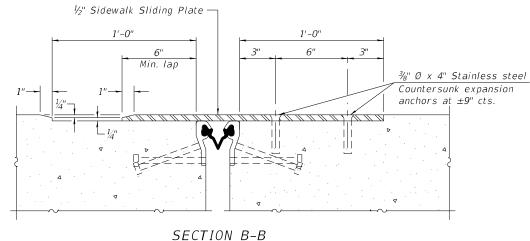
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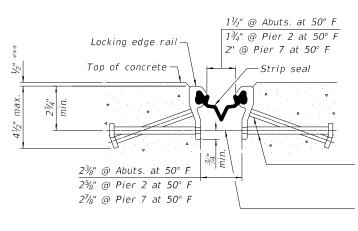
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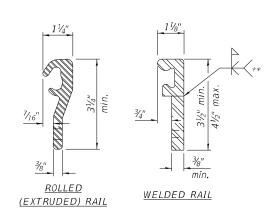


SHOWING ROLLED RAIL JOINT

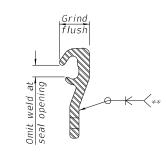
angled/bent studs with horizontal studs)  $\frac{3}{6}$ "  $\phi$  threaded rods in  $\frac{1}{16}$ "  $\phi$  holes at  $\pm 4$ '-0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

 $1\frac{1}{2}$ " @ Abuts. at 50° F 1¾" @ Pier 2 at 50° F Locking edge rail-@ Pier 7 at 50° F Top of concrete Strip seal \*  $\frac{5}{8}$ " Ø x 6" studs @ 6" cts. (alternate 3" @ Abuts. at 50° F 3¼" @ Pier 2 at 50° F 3½" @ Pier 7 at 50° F

SHOWING WELDED RAIL JOINT



LOCKING EDGE RAILS \*\* Back gouge not required if complete joint penetration is verified by mock-up.



#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

#### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	270

#### SECTION A-A

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

(Sheet 1 of 2)

PREFORMED JOINT STRIP SEAL **STRUCTURE NO. 016-0194** 

SECTION COUNTY 3730 15-00131-01-BR COOK 109 CONTRACT NO. 62W48

**AECOM** 

DESIGNED -REVISED CHECKED ATB REVISED DRAWN REVISED REVISED LOT DATE = 4/2/2024 CHECKED -ATB

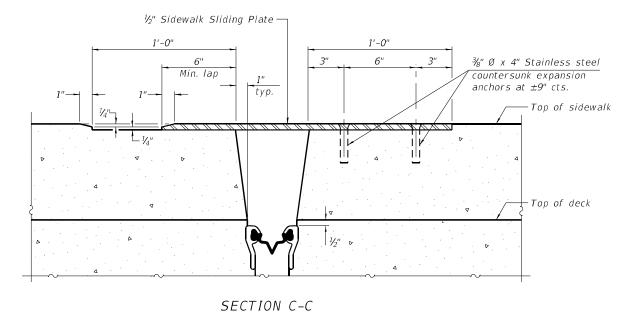
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

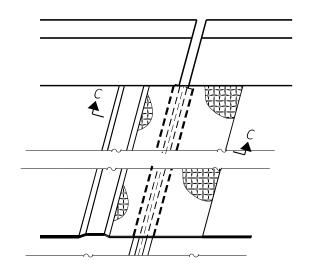
SHEET NO. SB-18 OF SB-30 SHEETS

#### SECTION AT RAISED SIDEWALK - EAST SIDE

(Looking South)

\*\*\* Prior to grinding





PLAN AT RAISED SIDEWALK

(For skews ≤ 30°)

The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

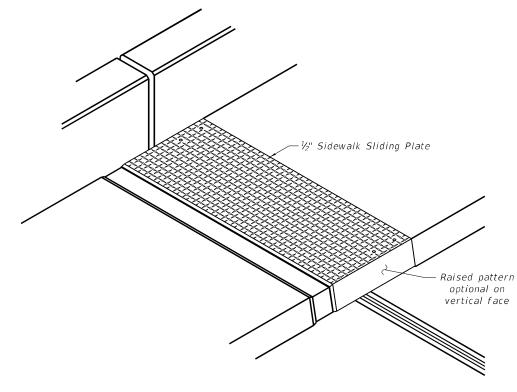
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

The top surface of sidewalk sliding plates shall have a raised pattern according to ASTM A786.

Cost of sidewalk sliding plates, embedded plates, anchorage studs, and expansion anchors included with Preformed Joint Strip Seal.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required.



TRIMETRIC VIEW

USER NAME = ken.moy	DESIGNED - RJL/SE	A REVISED	-
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PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

(Sheet 2 of 2)
PREFORMED JOINT STRIP SEAL
<b>STRUCTURE NO. 016-0194</b>

SECTION 3730 15-00131-01-BR COOK 109 79 CONTRACT NO. 62W48



All structural steel shall conform to AASHTO Classification M-270 Gr. 36.

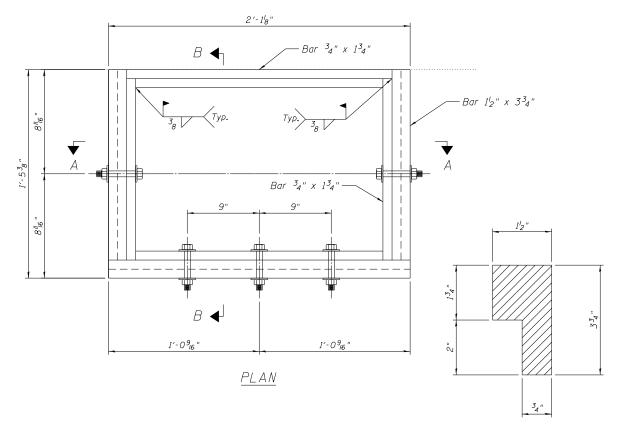
The adjusting scupper ring shall be galvanized. Bolts shall be  $^{\prime}_{2}$  ' $\phi$ , AASHTO M164 Type I, mechanically galvanized.

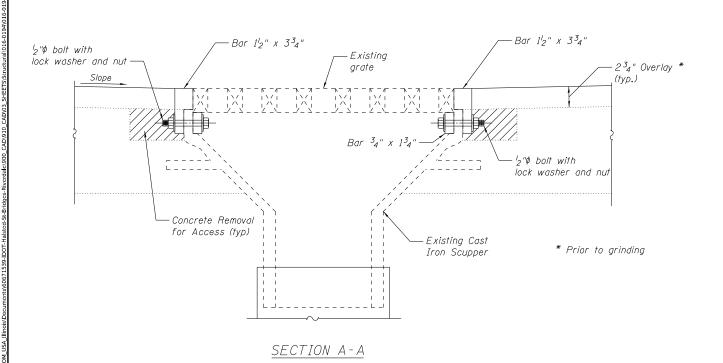
Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

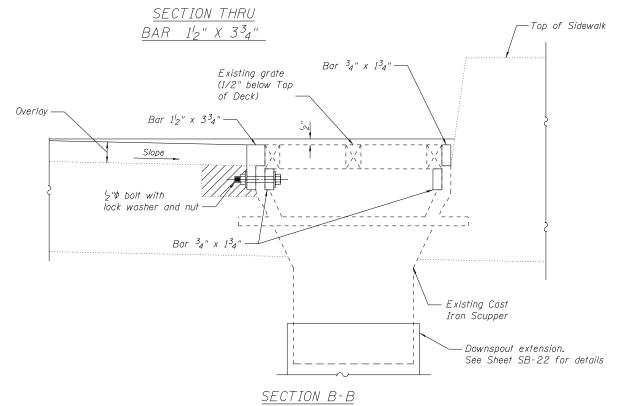
The contractor shall ensure that no damage is done to existing grates to be reused. Shop plans for proposed adjusting scupper ring shall be submitted for approval prior to fabrication.

Cost of all labor and materials necessary to remove existing grates, clean existing scuppers, install adjusting scupper rings and reinstalling grates is included in the cost per unit each for Drainage Scuppers To Be Adjusted.

Existing Scuppers are located near Piers 2 and 7 at east Sidewalk. See Sheet SB-22 for details of downspout extension.







#### BILL OF MATERIAL

		Unit	Total		
Drainage	Scuppers	Each	2		



±2′-0"

Slope

±2′-0"

Slope

Overlay 12'

Slope Overlay

SLOPING PLAN

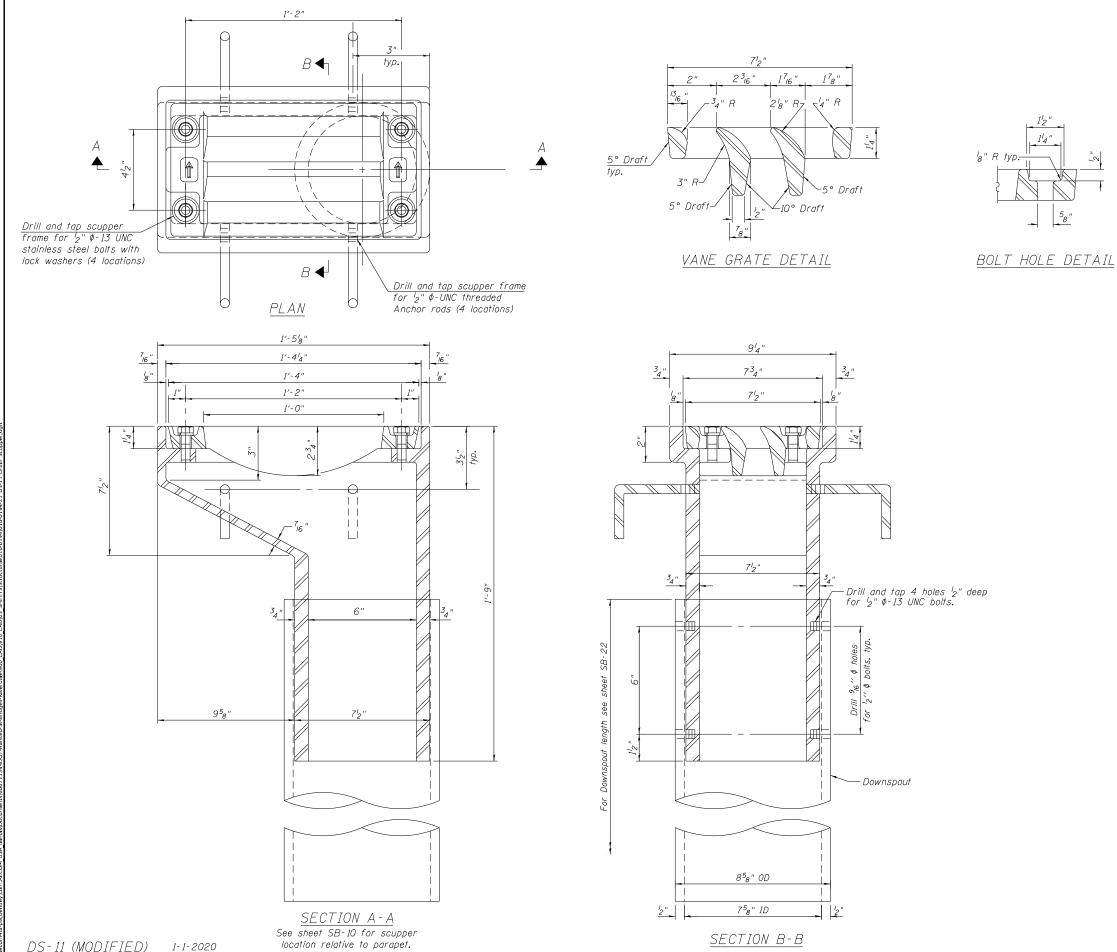
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  ADJUSTING EXISTING DRAINAGE SCUPPER **STRUCTURE NO. 016-0194** SHEET NO. SB-20 OF SB-30 SHEETS

LU. E.				COUNTY	TOTAL SHEETS	SHEET NO.	
30	30 15-00131-01-BR			соок	109	80	
					CONTRAC	T NO. 62	N48
		ILLINOIS	FED	ΑI	D PROJECT		



Notes

All cast iron parts shall be gray iron conforming to the requirements of AASHTO M105, Class 35B and AASHTO M306.

Bolts, anchor rods, nuts and washers shall be according to ASTM A307 and shall be galvanized according to AASHTO M232. As an alternate, stainless steel may be used.

Stainless steel hardware shall be according to Article 1006.29(d) of the Standard Specifications.

Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frames and downspouts; however, the scupper grates shall remain cast iron. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval.

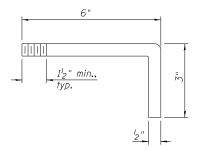
Structural steel scupper frames and downspouts, when utilized, shall be galvanized according to AASHTO M111.

As an alternate, fiberglass may be used for downspouts according to ASTM D2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. in lieu of the cast iron or structural steel.

Exterior surfaces of downspouts and exterior exposed surface of the scupper frame below deck shall be painted to match the finish coat of the exterior steel beams.

The Contractor shall take appropriate measures to assure that  $\mbox{\it Protective Coat}$  is not applied to the scupper.

Cost of the grate, frame, downspout, anchor rods, nuts and washers including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-11.



ANCHOR ROD DETAIL

#### BILL OF MATERIAL

	UNIT	QUANTITY		
Drainage So	cuppers,	DS-11	Each	4

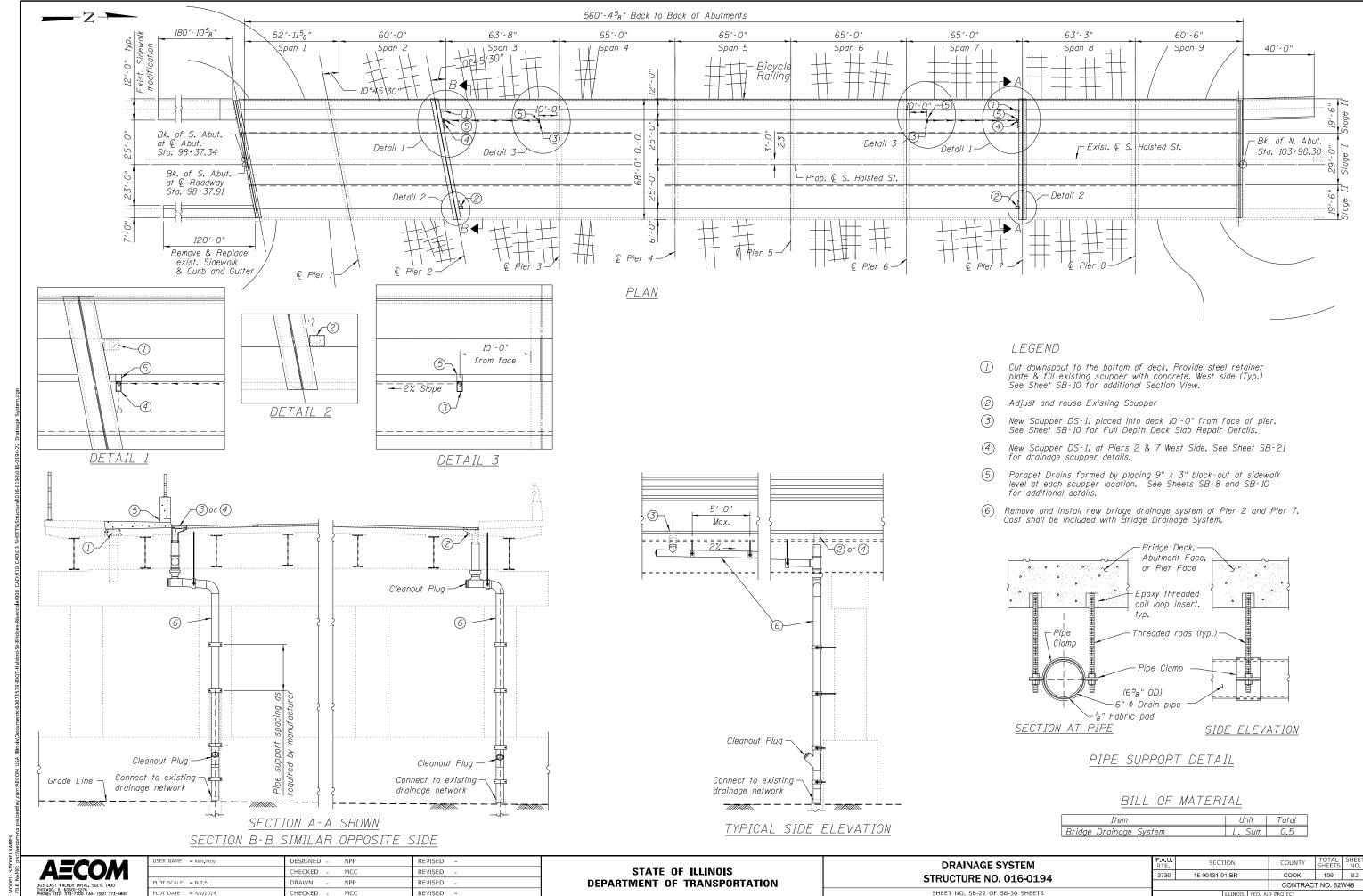
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303 EAST WACKER DRIVE, SUITE 1400
CHICAGO, IL 60601-5276

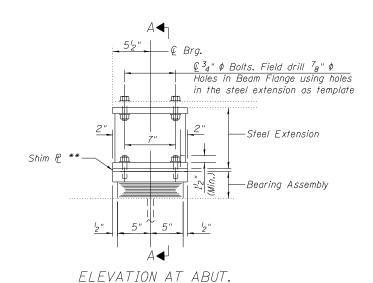
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DRAINAGE SCUPPER, DS-11 STRUCTURE NO. 016-0194 SHEET NO. SB-21 OF SB-30 SHEETS  
 FAU. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS
 NO.

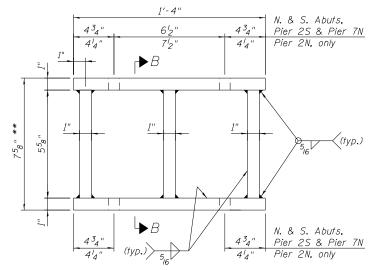
 3730
 15-00131-01-BR
 COOK
 109
 81

 CONTRACT NO. 62W48





Exist. Beam Flange =  $10^{1}_{2}$ " (North & South Abutments, Pier 2 South, and Pier 7 North) = 11<sup>1</sup>2" (Pier 2 North) - Shim P \*\* -Side Retainer, typ. 104' 1'-8'2' 1" ∮ x 1'-0" Anchor bolts Grade 55) with 214" x 214" x 516" P. washer under nut. SECTION A-A



ELEVATION STEEL EXTENSION (Looking North or South)

 $Q^{7}_{8}$ "  $\phi$  Holes

PLAN STEEL EXTENSION

1'-4"

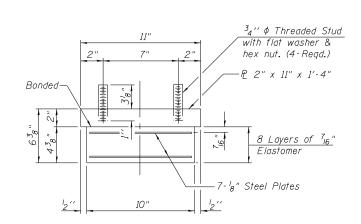
 $\pm 1.1$ 

<del>-</del>— € Beam

#### TYPE I ELASTOMERIC EXP. BRG.

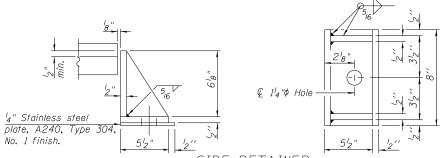
North & South Abutments, Pier 2 South, Pier 2 North, and Pier 7 North

\*\* Provide one additional  $^38$ " shim Pfor beam line 6 at North Abutment, and Pier 7 North.

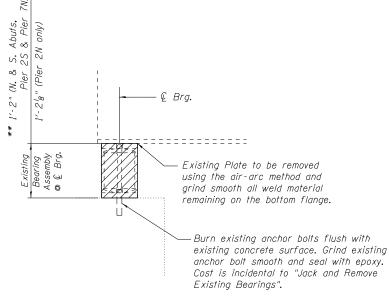


#### BEARING ASSEMBLY

Shim plates shall not be placed under Bearing Assembly.

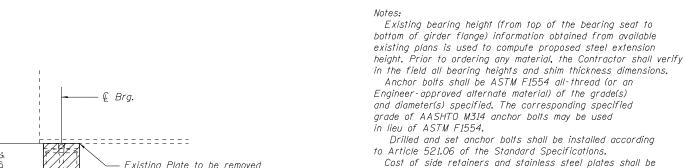


SIDE RETAINER Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



#### EXISTING BEARING REMOVAL DETAIL

Prior to jacking, the Contractor shall verify that hold-down rods installed at North and South abuments during construction have been removed according to original details.



Cost of side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.

Two 18" in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details. Cost included with Elastomeric Bearing Assembly, Type I.

The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50. Existing bearing at beam line 6 at Pier 7 North and N. Abut. has additional  ${}^3\!8$ " shim P. Cost of this shim plate is included in Furnishing and Erecting Structural Steel. New steel extensions, shim plates and connection bolts are included with Furnishing and Erecting Structural Steel. Anchor bolts and side retainers at all supports shall be

installed as each member is erected unless an equivalent

temporary means of lateral restraint is used.

(typ.) SECTION B-B

#### REACTION TABLE (KIPS)

Туре	S. Abut.	Pier 2S	Pier 2N	Pier 7N	N. Abut.	
R DL	53.3	62.0	65.7	65.4	60.2	
R LL	40.7	43.0	43.2	43.3	42.4	
R IMP	11.6	11.6	<i>11</i> <b>.</b> 5	11.6	11.6	
R Total	105.6	116.6	120.4	120.3	114.2	

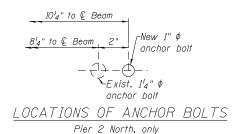
Note:

Minimum Jack Capacity 75 Tons.

#### 10<sup>1</sup>4" to @ Beam \_New 1" Φ 734" to € Beam, 21/2" anchor bolt Exist. 14" \$ anchor bolt

LOCATIONS OF ANCHOR BOLTS

North & South Abutments, Pier 2 South and Pier 7 North



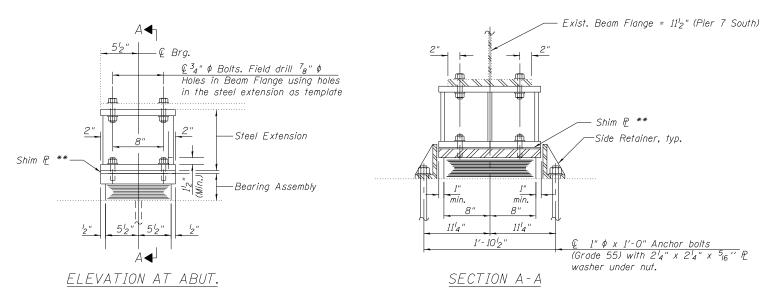
#### BILL OF MATERIAL

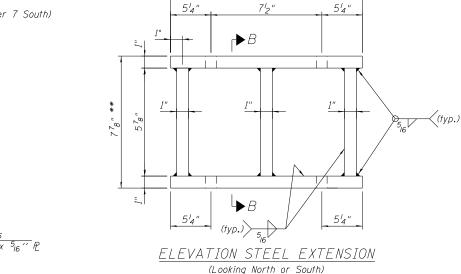
Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	50
Anchor Bolts, 1"	Each	100
Furnishing and Erecting Structural Steel	Pound	9,260

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	CHECKED -	-	ATB	REVISED	-
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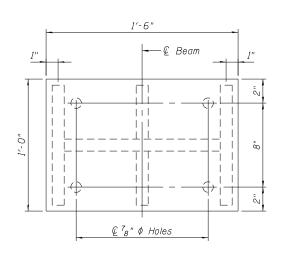
(Sheet 1 of 2)	
TYPE I BEARING DETAILS	
STRUCTURE NO. 016-0194	
SHEET NO. SB-23 OF SB-30 SHEETS	

A.U. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRAC	T NO. 62	N48	
ILLINOIS FED. AI			D PROJECT			





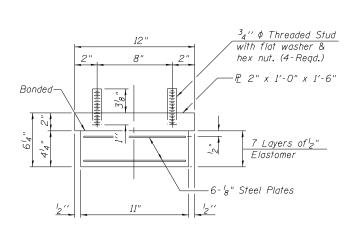
1'-6"



PLAN STEEL EXTENSION

## TYPE I ELASTOMERIC EXP. BRG. Pier 7 South

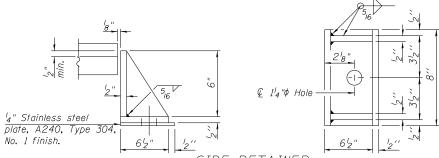
\*\* Provide one additional  ${}^3_8$ " shim  ${}^3_8$  for beam line 6 at Pier 7 South.



#### BEARING ASSEMBLY

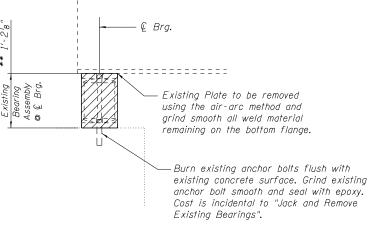
Note:

Shim plates shall not be placed under Bearing Assembly.



SIDE RETAINER

Equivalent rolled angle with stiffeners
will be allowed in lieu of welded plates.



#### EXISTING BEARING REMOVAL DETAIL

Note:

Prior to jacking, the Contractor shall verify that hold-down rods installed at North and South abuments during construction have been removed according to original details.

Existing bearing height (from top of the bearing seat to bottom of girder flange) information obtained from available existing plans is used to compute proposed steel extension height. Prior to ordering any material, the Contractor shall verify in the field all bearing heights and shim thickness dimensions. Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used

in lieu of ASTM F1554.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

to Article 521.06 of the Standard Specifications.

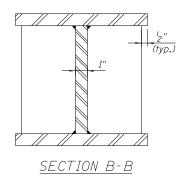
Cost of side retainers and stainless steel plates shall be included in the cost of Elastomeric Bearing Assembly, Type I.

Two  $^{\prime}_{8}$ " in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details. Cost included with Elastomeric Bearing Assembly, Type I.

The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50.

Existing bearing at beam line 6 at Pier 7 South has additional <sup>3</sup>8" shim P. Cost of this shim plate is included in Furnishing and Erecting Structural Steel.

New steel extensions, shim plates and connection bolts are included with Furnishing and Erecting Structural Steel.
Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.



#### REACTION TABLE (KIPS)

Туре	Pier 7S
R DL	67.4
R LL	43.3
R IMP	11.5
R Total	122.2

Note:

Minimum Jack Capacity 75 Tons.

#### 

#### BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	10
Anchor Bolts, 1"	Each	20
Furnishing and Erecting Structural Steel	Pound	2,230

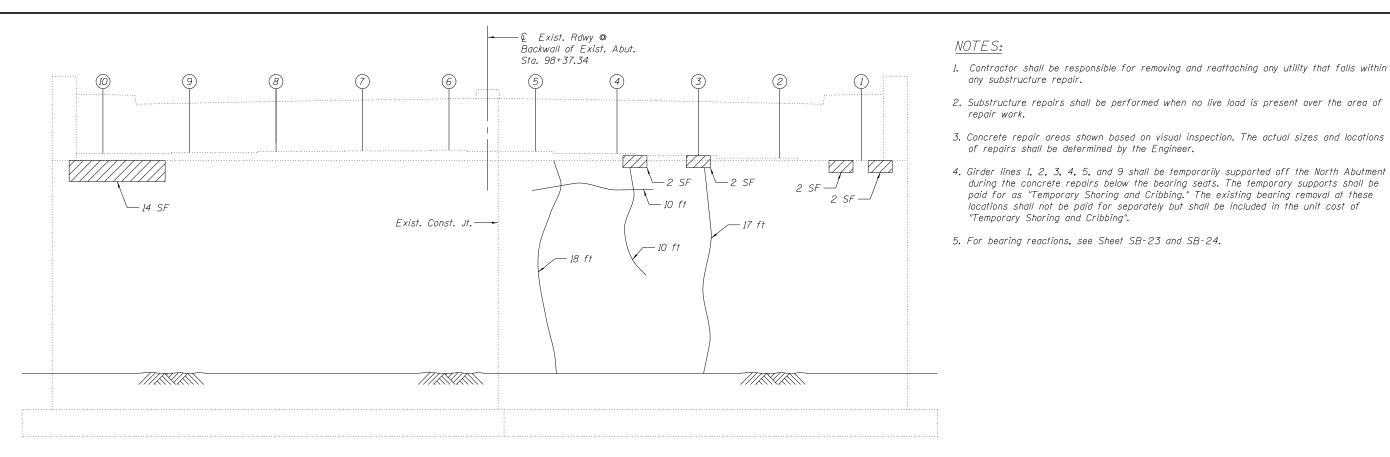
(Sheet 2 of 2)



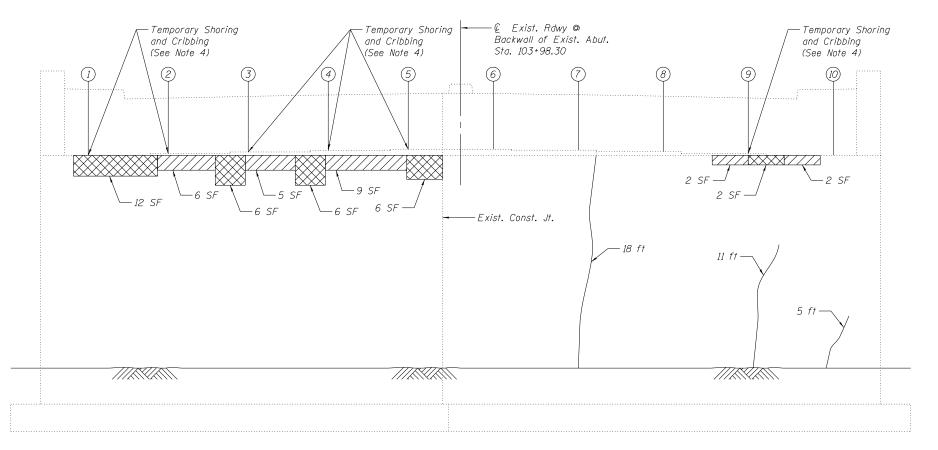
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	CHECKED - ATB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - MCC	REVISED -
PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

TYPE I BEARING DETAILS STRUCTURE NO. 016-0194	
SHEET NO. SB-24 OF SB-30 SHEETS	

A.U. TE	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
730	15-00131-0	1-BR		соок	109	84
				CONTRAC	T NO. 62	N48
		ILLIMOIS.	EED AL	D PROJECT		



#### SOUTH ABUTMENT



#### NORTH ABUTMENT

## **AECOM**

DESIGNED -REVISED -CHECKED MCC REVISED DRAWN NPP REVISED LOT DATE = 4/2/2024 REVISED CHECKED -

#### STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

#### **ABUTMENT REPAIRS** STRUCTURE NO. 016-0194 SHEET NO. SB-25 OF SB-30 SHEETS

#### SECTION COUNTY 3730 15-00131-01-BR COOK 109 85 CONTRACT NO. 62W48

LEGEND

any substructure repair.

of repairs shall be determined by the Engineer.

"Temporary Shoring and Cribbing".

during the concrete repairs below the bearing seats. The temporary supports shall be paid for as "Temporary Shoring and Cribbing." The existing bearing removal at these locations shall not be paid for separately but shall be included in the unit cost of

repair work.

Structural Repair Of Concrete (Depth equal to or less than 5 inches)

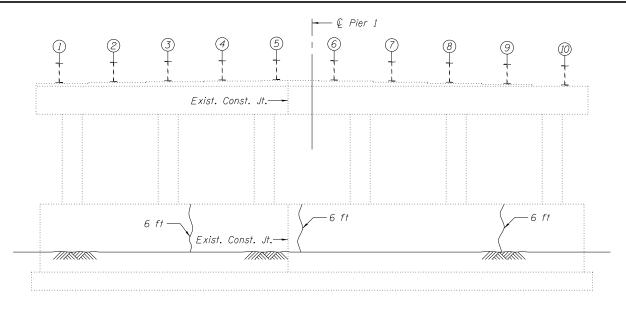


Structural Repair Of Concrete (Depth greater than 5 inches)

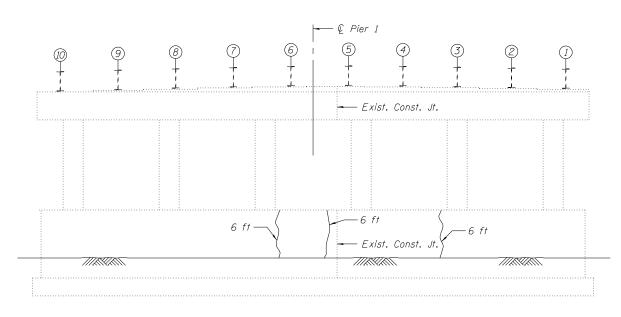
Epoxy Crack Injection

#### BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	46
Structural Repair of Concrete (Depth Greater than 5 inches)	Sq. Ft.	32
Epoxy Crack Injection	Foot	89
Temporary Shoring and Cribbing	Each	6



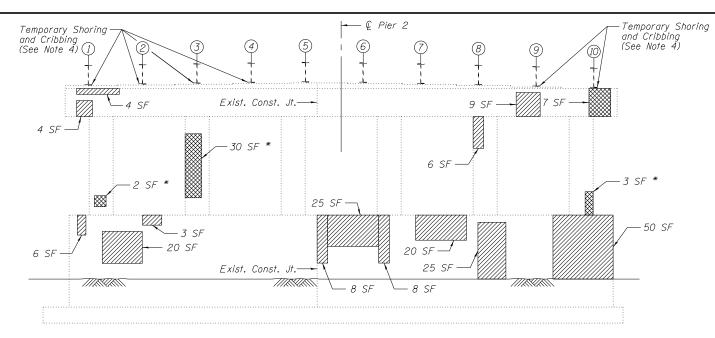
<u>PIER 1</u> (South Face)



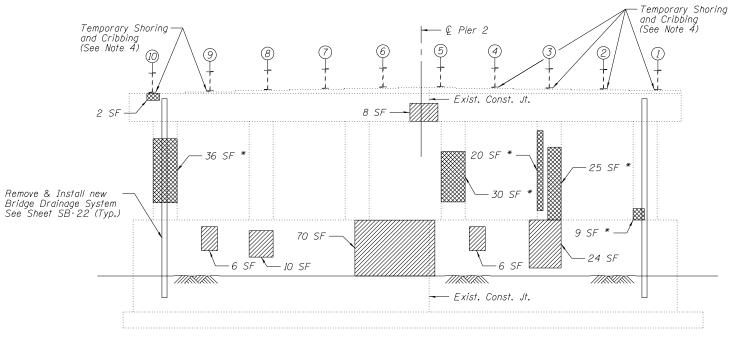
<u>PIER 1</u> (North Face)

#### NOTES:

- 1. Contractor shall be responsible for removing and reattaching any utility that falls within any substructure repair.
- Substructure repairs shall be performed when no live load is present over the area of repair work.
- 3. Concrete repair areas shown based on visual inspection. The actual sizes and locations of repairs shall be determined by the Engineer.
- 4. Girder lines 1, 2, 3, 4, 9 and 10 shall be temporarily supported off the pier during the concrete repairs to the pier cap and columns below. The temporary supports shall be paid for as "Temporary Shoring and Cribbing." The existing bearing removal at these locations shall not be paid for separately but shall be included in the unit cost of "Temporary Shoring and Cribbing".
- 5. For bearing reactions, see Sheet SB-23 and SB-24.



<u>PIER 2</u> (South Face)



<u>PIER 2</u> (North Face)

\* Fiber Wrap shall be applied to the circumferance of the pier column after structural repair of concrete. See Special Provisions.

#### LEGEND



Structural Repair Of Concrete (Depth equal to or less than 5 inches)



Structural Repair Of Concrete (Depth greater than 5 inches)



— Epoxy Crack Injection

#### BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	312
Structural Repair of Concrete (Depth Greater than 5 inches)	Sq. Ft.	164
Epoxy Crack Injection	Foot	36
Fiber Wrap	Sq. Ft.	288
Temporary Shoring and Cribbing	Each	12
Acrylic Coating	Sq. Yd.	32

**AECOM** 

 USER NAME
 = ken.moy
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 REVISED

 PLOT SCALE
 = N.T.S.
 DRAWN
 NPP
 REVISED

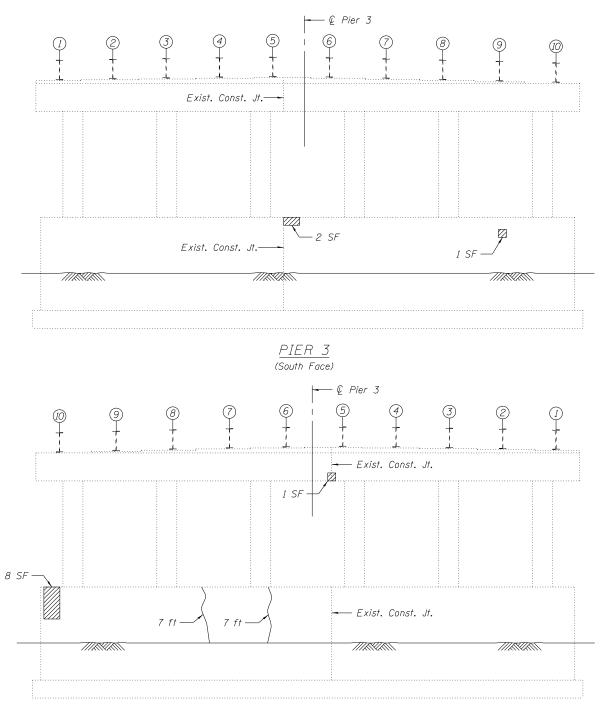
 PLOT DATE
 = 4/2/2024
 CHECKED
 MCC
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIERS 1 & 2 REPAIRS STRUCTURE NO. 016-0194 SHEET NO. 5B-26 OF SB-30 SHEETS  
 F.A.U. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL NO
 SHEETS NO
 SHEETS NO

 3730
 15-00131-01-BR
 COOK
 109
 86

 CONTRACT NO. 62W48



<u>PIER 3</u> (North Face)

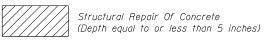
## Exist. Const. Jt. 6 ft — Exist. Const. Jt. /// PIER 4 (South Face) ├--- <u>€</u> Pier 4 6 - Exist. Const. Jt. Exist. Const. Jt. 12 SF -///85//85 ///88//88 PIER 4 (North Face)

(5)

#### NOTES:

- Contractor shall be responsible for removing and reattaching any utility that falls within any substructure repair.
- 2. Concrete repair areas shown based on visual inspection. The actual sizes and locations of repairs shall be determined by the Engineer.
- 3. Substructure repairs shall be performed when no live load is present over the area of repair work.

#### LEGEND



Epoxy Crack Injection

#### BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	45
Epoxy Crack Injection	Foot	20

AECOM

303 EAST WACKER DRIVE. SUITE 1400
CHICAGO. IL. GOGGUI-SZIG.
PHONE: 1312 373-7500

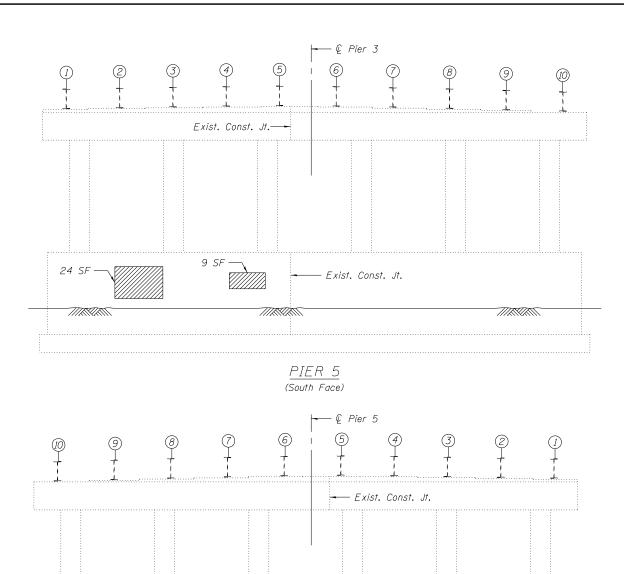
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PLOT SCALE = N.T.S.	DRAWN - MCC	REVISED -
PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIERS 3 & 4 REPAIRS STRUCTURE NO. 016-0194 SHEET NO. SB-27 OF SB-30 SHEETS  
 FAU. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

 3730
 15-00131-01-BR
 COOK
 109
 87

 CONTRACT NO. 62W48

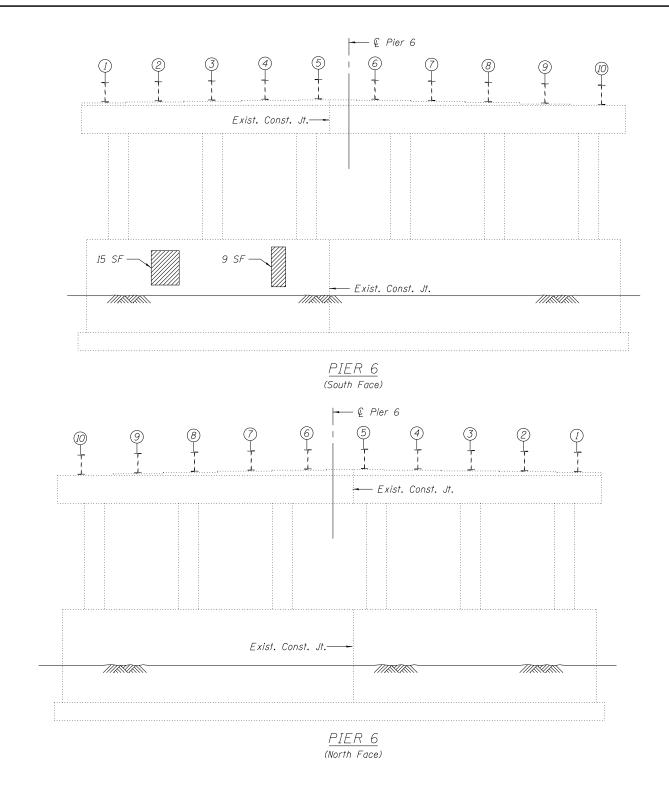


<u>PIER 5</u> (North Face)

Exist. Const. Jt.—

20 SF —

///XV/X



#### NOTES:

 Contractor shall be responsible for removing and reattaching any utility that falls within any substructure repair.

///XV/X

- Concrete repair areas shown based on visual inspection. The actual sizes and locations of repairs shall be determined by the Engineer.
- 3. Substructure repairs shall be performed when no live load is present over the area of repair work.

#### LEGEND



#### BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	101

AECOM
303 EAST WACKER DRIVE, SUITE 1400
CHICAGO, II. 60601-5276
PHOME: 1312 373-700 FAX: (312) 373-6800

USER NAME = ken moy	DESIGNED - MCC	REVISED -
	CHECKED - ATB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - MCC	REVISED -
PLOT DATE = 4/2/2024	CHECKED - ATB	REVISED -

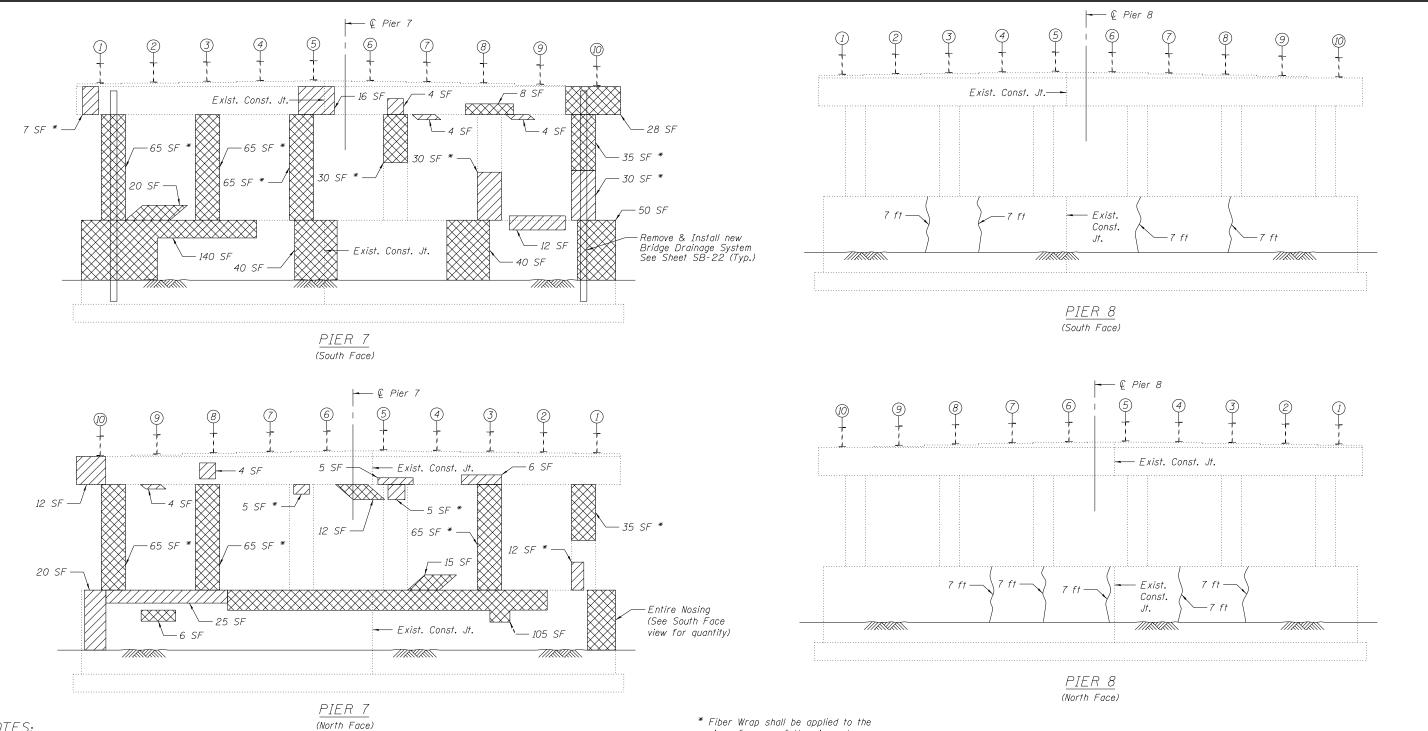
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

-24 SF

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PIERS 5 & 6 REPAIRS
STRUCTURE NO. 016-0194
CHEET NO CD 20 OF CD 20 CHEETC

F.A.U. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
3730	15-00131-01-BR			соок	109	88
				CONTRAC	T NO. 62'	W48
ILLINOIS FED. AI				D PROJECT		



NOTES:

- 1. Contractor shall be responsible for removing and reattaching any utility that falls within any substructure repair.
- 2. Substructure repairs shall be performed when no live load is present over the area of repair work.
- 3. Concrete repair areas shown based on visual inspection. The actual sizes and locations of repairs shall be determined by the Engineer.
- 4. All girder lines shall be temporarily supported off of Pier 7 during the concrete repairs to the pier cap and columns below. The temporary supports shall be paid for as "Temporary Shoring and Cribbing." The existing bearing removal at these locations shall not be paid for separately but shall be included in the unit cost of "Temporary Shoring and Cribbing".
- 5. For bearing reactions, see Sheet SB-23 and SB-24.
- 6. Overhead concrete repairs shall be performed using Shotcrete. See Special Provisions.

\* Fiber Wrap shall be applied to the circumferance of the pier column after structural repair of concrete. See Special Provisions.

#### LEGEND



Structural Repair Of Concrete (Depth equal to or less than 5 inches)



Structural Repair Of Concrete (Depth greater than 5 inches)



Epoxy Crack Injection

#### BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth Equal to or Less than 5 inches)	Sq. Ft.	205
Structural Repair of Concrete (Depth Greater than 5 inches)	Sq. Ft.	934
Epoxy Crack Injection	Foot	63
Fiber Wrap	Sq. Ft.	682
Temporary Shoring and Cribbing	Each	20
Acrylic Coating	Sq. Yd.	76

USER NAME = ken.moy	DESIGNED - NPP	REVISED -
	CHECKED - MCC	REVISED -
PLOT SCALE = N.T.S.	DRAWN - NPP	REVISED -
PLOT DATE = 4/2/2024	CHECKED - MCC	REVISED -

STATE OF ILLINOIS

PIERS 7 & 8 REPAIRS **STRUCTURE NO. 016-0194** SHEET NO. SB-29 OF SB-30 SHEETS

A.U. TE	SECT	ПОП		COUNTY	TOTAL SHEETS	SHEET NO.			
730	15-00131-01-BR		соок	109	89				
			CONTRACT NO. 62W48						

**DEPARTMENT OF TRANSPORTATION** 

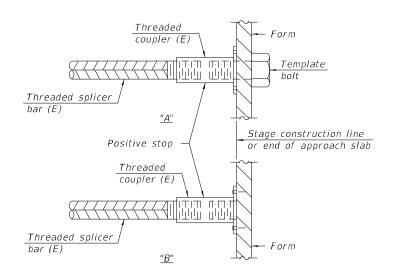
#### STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

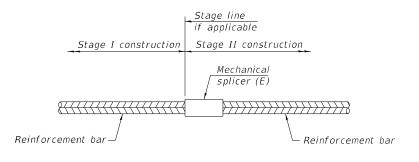
		I	
Location	Bar	No. assemblies	Minimum
LUCALIUII	size	required	lap length
South Abutment	#6	8	3'-7"
South Abutment	#5	18	3'-6"
North Abutment	#6	8	3'-7"
North Abutment	#5	18	3'-6"
Pier 2	#5	40	3'-6"
Pier 7	#5	40	3'-6"
Beam Encasement	#5	72	3'-0"



#### INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



#### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements

for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

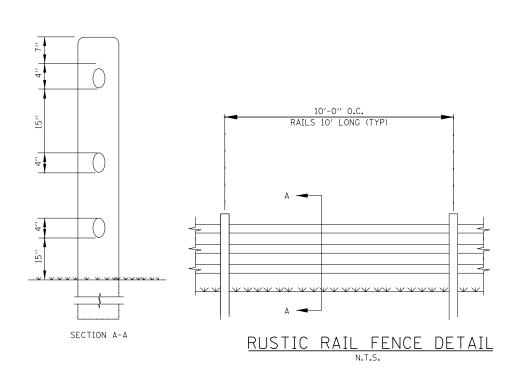
1-1-2020

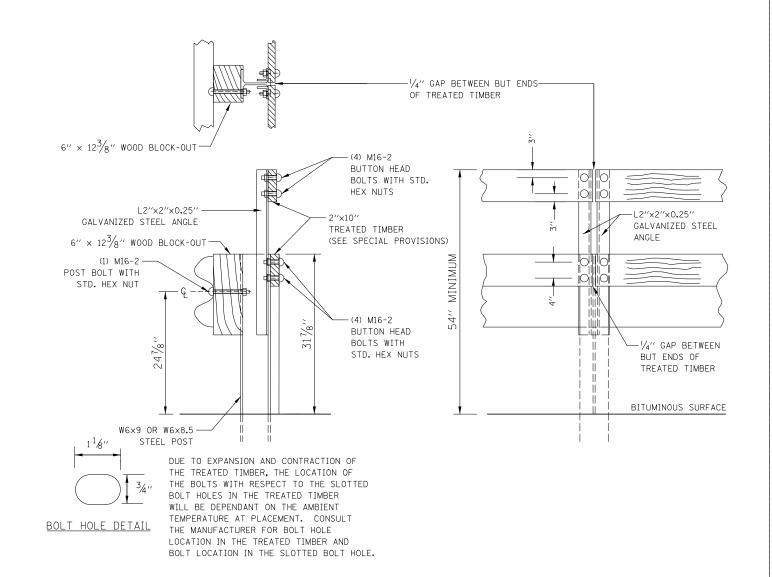
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PLOT DATE = 4/2/2024	CHECKED - MCC	REVISED -

A.U. TE	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
730	15-00131-0	1-BR		соок	109	90
			CONTRAC	T NO. 62	N48	
ILLINOIS FED. AID PROJECT						

BSD-1

**DEPARTMENT OF TRANSPORTATION** 





# RUB RAIL DETAIL BIKE PATH GUARDRAIL ADJUSTMENT

SHEET NO.

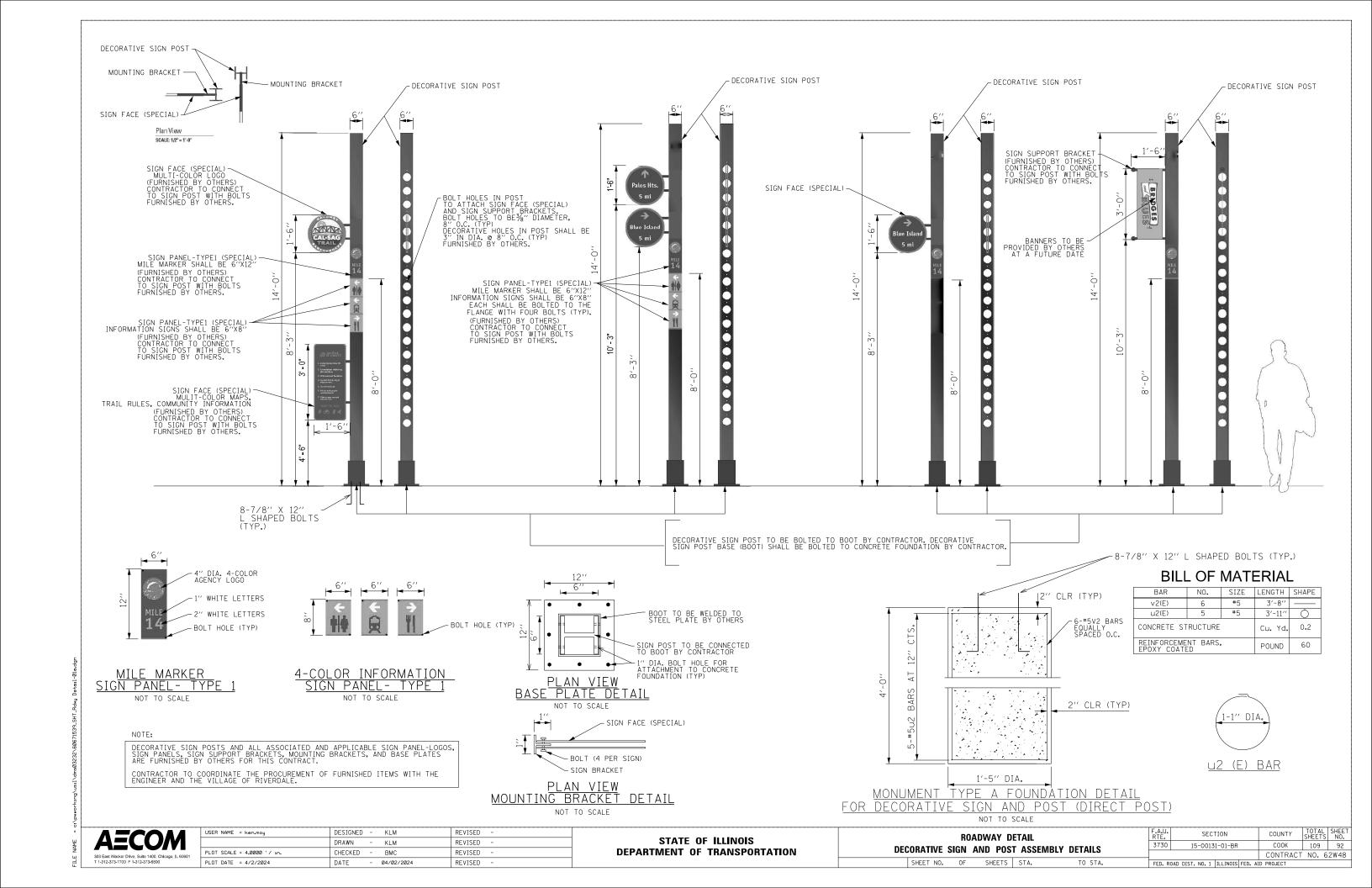
**AECOM**303 East Wacker Drive, Sulte 1400. Chicago, IL 60601
T1-312-373-7700 F1-312-373-8800

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	DRAWN	-	KLM	REVISED	-
PLOT SCALE = 20.00000 '/ in.	CHECKED	-	BMC	REVISED	-
PLOT DATE = 4/2/2024	DATE	-	04/02/2024	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

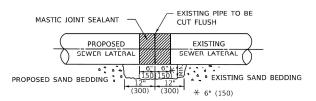
ROADWAY DETAIL			F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.	
MISCELLANEOUS		3730	30 15-00131-01-BR		COOK	109	91		
MII2CELLAINEUU3						CONTRACT	NO. 6	2W48	
OF	SHEETS	STA.	TO STA.	FED. RO	DAD DIST. NO. 1   ILLINOIS FE	D. AII	PROJECT		

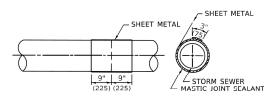
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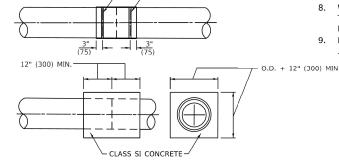


#### **DETAIL** "A"

LATERAL CONNECTION TO EXISTING SEWER OF 27" (675) OR SMALLER





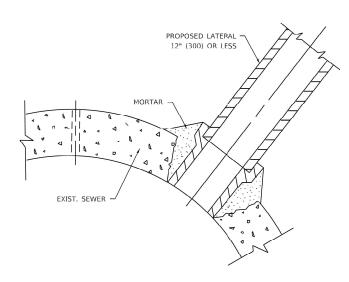


METAL BINDING

**DETAIL "B"**CLASS SI CONCRETE COLLAR

#### **CONSTRUCTION SEQUENCE**

- CUT THE EXISTING END OF THE PIPE SO AS TO PRESENT A FLUSH BUTT JOINT. BRUSH AND CLEAN ALL PIPES.
- APPLY THE MASTIC JOINT SEALANT TO THE FIRST 6" (150) OF EACH PIPE.
- BUTT THE PIPES TOGETHER LEAVING A MINIMUM OF 12' x 6' (300 x 150) DEEP EXCAVATION UNDER AND AROUND EACH PIPE END.
- 4. CUT A PIECE OF SHEET METAL GAGE NO. 19 1.1 (0.0418) 18" (450) WIDE BY THE OUTSIDE CIRCUMFERANCE OF THE PIPE PLUS 3" (75) LONG.
- 5. WRAP THE SHEET METAL AROUND THE PIPES, 9" (225) ON EACH SIDE OF THE JOINT, STARTING AT THE TOP OF THE PIPE.
- 6. LAP THE SHEET METAL AT LEAST 3" (75) AT THE TOP OF THE PIPE AND PLACE THE MASTIC JOINT SEALANT BETWEEN THE LAP.
- 7. PLACE TWO METAL BANDS AROUND THE SHEET METAL AND TIGHTEN.
- 8. WIPE OFF ANY EXCESS MASTIC JOINT SEALANT THAT OOZES OUT FROM BETWEEN THE SHEET METAL AND THE PIPES.
- PLACE CLASS SI CONCRETE AROUND THE JOINT.



#### DETAIL "C"

PROPOSED LATERAL
CONNECTION TO EXISTING SEWER
OF 30" (750) OR LARGER

#### NOTES:

#### **MATERIAL**

MATERIAL USED FOR THE TEE OR WYE SECTION SHALL BE COMPATIBLE WITH THE FXISTING STORM SEWER OR THE PROPOSED STORM SEWER.

#### **CONSTRUCTION METHODS**

- I. THIS WORK SHALL BE CONSTRUCTED IN CONFORMANCE WITH THE APPLICABLE PORTIONS OF SECTION 550 OF THE STANDARD SPECIFICATIONS.
- II. CONNECTION TO AN EXISTING STORM SEWER SHALL BE BY EITHER OF THE FOLLOWING METHODS:

  A) PROPOSED STORM SEWER CONNECTION TO EXISTING SEWER OF 27" (675) OR SMALLER SEE DETAIL "A" AND "B".
  - B) PROPOSED STORM SEWER CONNECTION TO EXISTING SEWER OF 30" (750) OR LARGER SEE DETAIL "C".

IF THE EXISTING SEWER PIPE IS CRACKED, BROKEN OR OTHERWISE DAMAGED BY THE CONTRACTOR IN MAKING THE CIRCULAR OPENING, THE CONTRACTOR SHALL REPLACE THAT SECTION OF PIPE WITH PIPE EQUAL AND SIMILAR IN ALL RESPECTS TO THE PIPE IN THE EXISTING SEWER, IN A CAREFUL WORKMANLIKE MANNER, WITHOUT EXTRA COMPENSATION.

#### GENERAL

- 1. CARE MUST BE TAKEN TO PREVENT DEBRIS FROM ENTERING THE SEWER.
  ALL DEBRIS WHICH ENTERS THE SEWER MUST BE REMOVED. THE SEWER MUST
  BE LEFT CLEAN AND UNOBSTRUCTED UPON COMPLETION OF THE CONTRACT.
- 2. CARE MUSI BE TAKEN TO PREVENT ANY PART OF THE NEW PIPE CONNECTION FROM PROJECTING INTO THE EXISTING SEWER.

#### **BASIS OF PAYMENT**

- 1. TEE OR WYE CONNECTIONS SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE EACH FOR STORM SEWER TEE OR WYE OF THE TYPE AND SIZE SPECIFIED IN THE PLANS, THIS PRICE SHALL INCLUDE ALL EXCAVATION OF THE TRENCH, REMOVAL OF THE EXISTING STORM SEWER, FURNISHING AND INSTALLING THE SPECIFIED TEE OR WYE SECTION, FURNISHING AND INSTALLING THE REQUIRED CONCRETE COLLAR, AND ALL OTHER MATERIAL NECESSARY TO COMPLETE THIS WORK AS SHOWN AND SPECIFIED.
- 2. REMOVAL AND REINSTALLATION OF EXISTING STORM SEWER ADJACENT TO THE PROPOSED TEE OR WYE SECTION, FOR THE PURPOSE OF FACILITATING THE INSTALLATION OF THE TEE OR WYE SECTION, WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE WORK.
- 3. TRENCH BACKFILL, EXCAVATION IN ROCK AND REMOVAL AND REPLACEMENT OF UNSUITABLE MATERIAL BELOW PLAN BEDDING GRADE WILL BE PAID FOR SEPARATELY.
- 4. CONCRETE COLLAR FOR CONNECTING A PROPOSED STORM SEWER TO AN EXISTING STORM SEWER WILL NOT BE PAID PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF THE PROPOSED STORM SEWER.

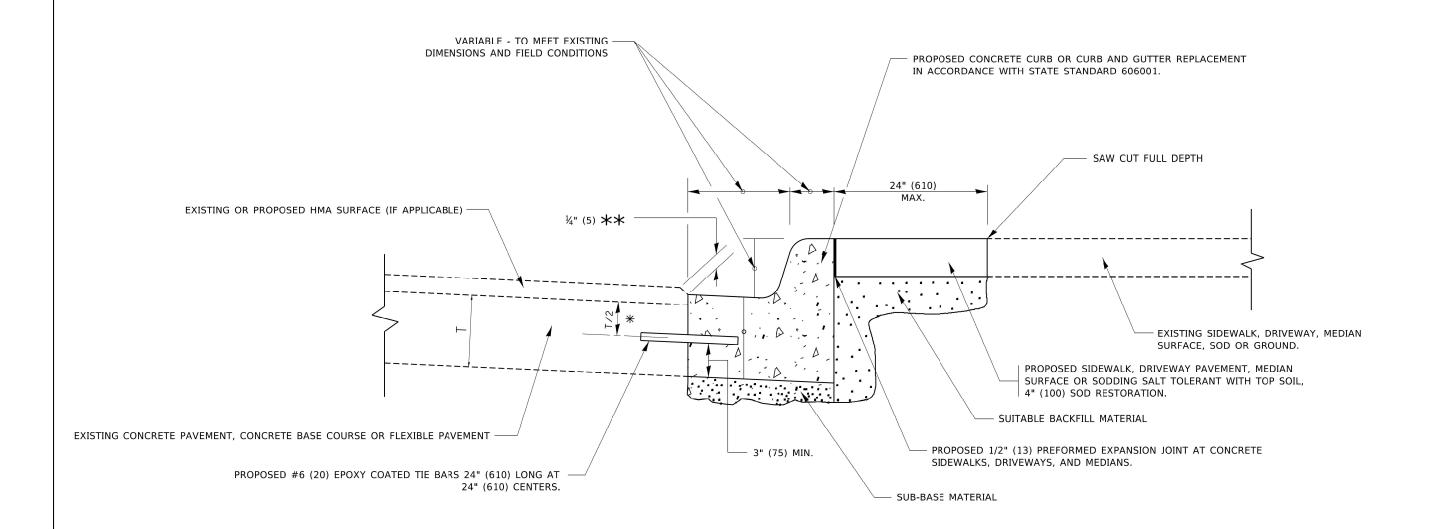
\* ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.



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	DRAWN	-	KLM	REVISED	-
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PLOT DATE = 4/2/2024	DATE	-	04/02/2024	REVISED	-

D1 STANDARD – DETAIL OF STORM SEWER CONNECTION TO EXISTING SEWER								
	SHEET NO.	OF	SHEETS	STA.	TO STA			

F.A.U. RTE.	SECT	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEE NO.	
3730	15-0013	1-01-BR		Т	COOK	109	93
	BD-	-07	Т	CONTRACT	NO. 6	2W48	
FED. R	OAD DIST. NO. 1	ILLINOIS	FED.	AID	PROJECT		



- 💥 3" (75) MINIMUM FROM TOP AND BOTTOM OF THE CONCRETE PAVEMENT OR BASE COURSE.
- $\star\star$  IF THE FINAL SURFACE OF THE PAVEMENT IS CONCRETE, THE GUTTER IS TO BE FLUSH WITH THE PAVEMENT.

## **CURB OR CURB AND GUTTER REMOVAL AND REPLACEMENT**

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

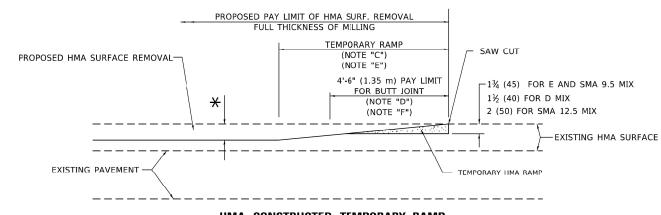


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PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -

D1	STANI	DARD	– CI	IRB OR (	CURB AND	F.A.U. RTE.	SEC	COUNTY	TOTAL SHEETS	S		
	REMOVAL AND REPLACEMENT							15-00131-01-BR		соок	109	
								BD-24 CONTRACT				
	SHEET	NO.	OF	SHEETS	STA.	TO STA.	FED. F	ROAD DIST. NO. 1	ILLINOIS FED. A	ID PROJECT		

(FOR BUTT JOINT AND HMA TAPER SEE DETAIL BELOW)

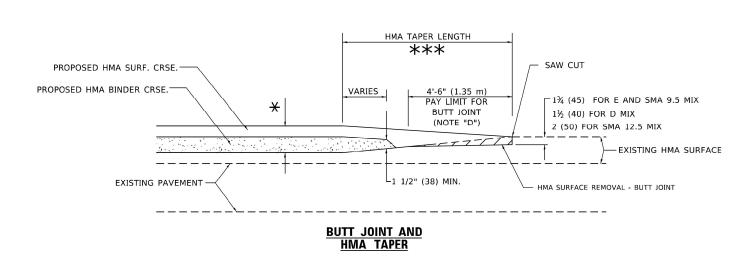
#### OPTION 1



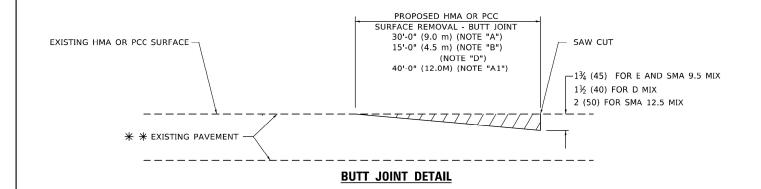
HMA CONSTRUCTED TEMPORARY RAMP (FOR BUTT JOINT AND HMA TAPER SEE DETAIL BELOW)

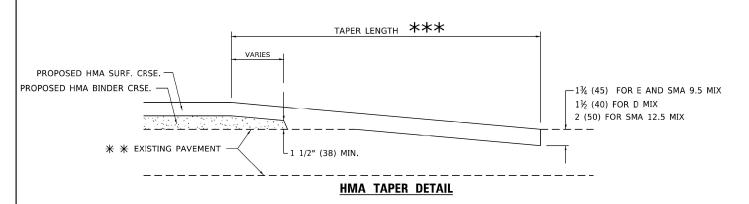
OPTION 2

### TYPICAL TEMPORARY RAMP



## TYPICAL BUTT JOINT AND HMA TAPER FOR MILLING AND RESURFACING





# TYPICAL BUTT JOINT AND HMA TAPER FOR RESURFACING ONLY

\*\* PC CONCRETE, HMA OR HMA RESURFACED PAVEMENT.

#### **GENERAL NOTES**

- A. MAINLINE ARTERIAL ROADWAYS AND MAJOR SIDE ROADS.
- A1. INTERSTATES
- B. MINOR SIDE ROADS
- C. THE TEMP. RAMP SHALL BE CONSTRUCTED IMMEDIATELY UPON REMOVAL OF THE EXISTING HMA SURFACE.
- D. THE BUTT JOINT SHALL BE CONSTRUCTED IMMEDIATELY PRIOR TO PLACING THE PROPOSED HMA COURSES.
- E. TAPER THE TEMP. RAMP AT A RATE OF 3' 4" (1.02m) PER 1 INCH (25 mm) OF MILLING THICKNESS.
- \*\*\* 20'-0" (6.1 m) PER 1 (25) RESURFACING (NOTE "A") 10'-0" (3.0 m) PER 1 (25) RESURFACING (NOTE "B")

#### **BASIS OF PAYMENT**

- 1. THE BUTT JOINT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR "HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT" OR FOR "PORTLAND CEMENT CONCRETE SURFACE REMOVAL- BUTT JOINT".
- 2. THE TEMPORARY RAMP AND SAW CUT SHALL BE INCLUDED IN THE UNIT COST FOR HMA OR PCC SURFACE REMOVAL-BUTT JOINT

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

109 95

CONTRACT NO. 62W48



DESIGNED - KLM	REVISED -
DRAWN - KLM	REVISED -
CHECKED - BMC	REVISED -
DATE - 04/02/2024	REVISED -
	DRAWN - KLM CHECKED - BMC

D1 STANDARD - BUTT JOINT AND							F.A.U. SECT		TION			COUNTY		
DI STANDARD - BUTT JUINT AND HMA TAPER DETAILS						3730		15-	0013	1-01-BR			COOK	
		IIIVIA	TAPEN DE	IAILS			BD-32			CONTRA				
SHEET	NO.	OF	SHEETS	STA.	TO	STA.	FED.	ROAD	DIST. N	0.1	ILLINOIS	FED.	AID	PROJECT

#### SECTION A-A

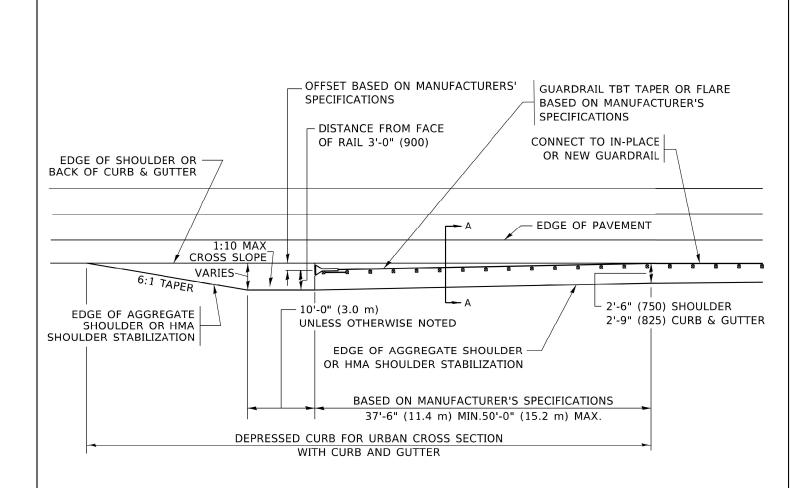
#### NOTES:

- 1. THE AGGREGATE SHOULDER, 10 (250) OR HMA SHOULDER, 6 (150) (IF REQUIRED) SHALL EXTEND UNDER THE TRAFFIC BARRIER TERMINAL.
- 2. "EXISTING" GUARDRAIL REFERS TO CONNECTING TERMINAL SECTION TO GUARD RAILING PRIOR TO THE MIDWEST GUARDRAIL SYSTEM.
- 3. THE CONTRACTOR SHALL VERIFY THE TYPE/HEIGHT OF GUARDRAIL IN-PLACE BEFORE ORDERING THE NEW TERMINAL SECTION. COST INCLUDED WITH THE COST OF THE TERMINAL. THE EXISTING GUARDRAIL HEIGHT SHALL TRANSISTION TO MATCH THE NEW TERMINAL END SECTION AND SHALL BE PAID FOR AS VERTICAL ADJUSTMENT OF EXISTING GUADRAIL.

DETAILS FOR STEEL PLATE BEAM

GUARD RAIL ADJACENT TO CURB AND GUTTER

[FOR ROADWAY SPEED 35 MPH (60 kmh) TO 45 MPH (70 kmh)]



# DEPRESSED CURB AND GUTTER AND SHOULDER TREATMENT AT TBT TY. 1 SPL.

AGGREGATE SHOULDER, 10 (250) WILL BE PAID ACCORDING TO SECTION 481.

HMA SHOULDERS 6 (150) (IF REQUIRED) WILL BE PAID ACCORDING TO SECTION 482.

COMB. CONC. C&G, STEEL PLATE BEAM GUARD RAIL AND TRAFFIC BARRIER TERMINAL, OF THE TYPE SPECIFIED WILL BE PAID FOR SEPARATELY.

TBT = TRAFFIC BARRIER TERMINAL
ALL DIMENSIONS ARE IN INCHES (MILLIMETERS)
UNLESS OTHERWISE SHOWN.



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

D1 STANDARD — DETAILS FOR DEPRESSEC CURB & GUTTER
AND SHOULDER TREATMENT AT TBT TY. 1 SPL.

NONE SHEET NO. OF SHEETS STA. TO STA.

## TYPICAL BENCHING DETAIL FOR EMBANKMENT

#### **GENERAL NOTES**

- 1. CONSTRUCT SUCCEEDING BENCH CUTS AND EMBANKMENT PLACEMENT AND COMPACTION FROM BOTTOM TO TOP IN STAIRSTEP FASHION.
- 2. EXISTING FORESLOPE PREPARED IN ACCORDANCE WITH ARTICLE 205.03 OF THE STANDARD SPECIFICATIONS.
- 3. BENCH CUT EXISTING SLOPE TYPICAL FOR EACH STEP.
- 4. TRIM TO FINAL SLOPE.
- EQUAL 8-INCH (200) LIFTS OF EMBANKMENT COMPACTED IN ACCORDANCE WITH ARTICLE 205.05 OF THE STANDARD SPECIFICATIONS.

#### **BASIS OF PAYMENT**

 EXCAVATION OF BENCH CUTS WITHIN EXISTING EMBANKMENT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC METER OR CUBIC YARD FOR "EARTH EXCAVATION". THIS PRICE WILL INCLUDE ALL LABOR AND MATERIAL, NO ADDITIONAL COMPENSATION WILL BE ALLOWED.

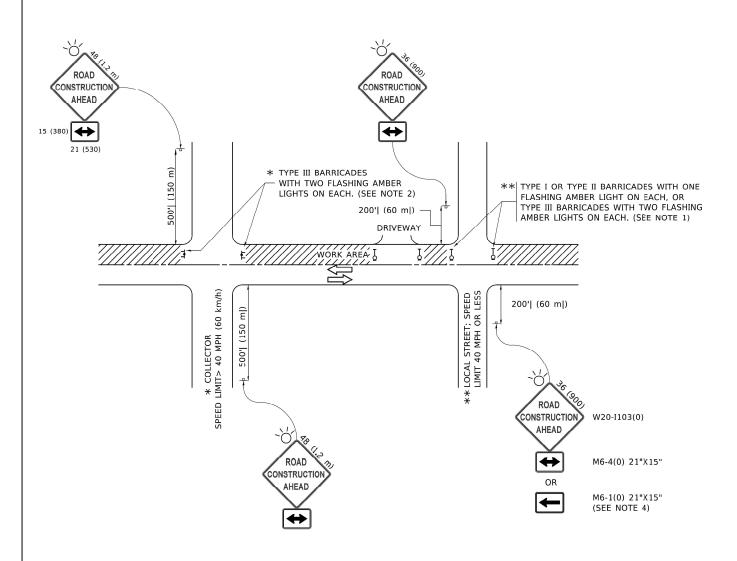
ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.



USER NAME = ken.moy	DESIGNED - KLM	REVISED -	
	DRAWN - KLM	REVISED -	
PLOT SCALE = 2.0000 '/ in.	CHECKED - BMC	REVISED -	
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -	

SCALE: NONE

D1 S	TAN	IDARD	– BENC	HING DE	TAIL	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FOR EMBANKMENT WIDENING							15-00131-01-BR	COOK	109	97
- 11	on i	LIVIDA	INIZIVILIVI	VVIDEIVIII	<b></b>	BD-51 CONTRACT NO. 62W				2W48
SHEET N	0.	OF	SHEETS	STA.	TO STA.	FED. RO	DAD DIST, NO. 1 ILLINOIS FED. AI	D PROJECT		



#### NOTES:

- 1. SIDE ROAD WITH A SPEED LIMIT OF 40 MPH (60 km/h) OR LESS AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEER:
- a) ONE "ROAD CONSTRUCTION AHEAD" SIGN 36 x 36 (900x900) WITH A FLASHER MOUNTED ON IT APPROXIMATELY 200' (60 m) IN ADVANCE OF THE MAIN ROUTE.
- b) THE CLOSED PORTICN OF THE MAIN ROUTE SHALL BE PROTECTED BY BLOCKING WITH TYPE I, TYPE II OR TYPE III BARRICADES, 1/3 OF THE CROSS SECTION OF THE CLOSED PORTION.
- 2. SIDE ROAD WITH A SPEED LIMIT GREATER THAN 40 MPH (60 km/h) AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEER:
  - a) ONE "ROAD CONSTRUCTION AHEAD" SIGN 48 x 48 (1.2 m x 1.2 m) WITH A FLASHER MOUNTED ON IT APPROXIMATELY 500' (150 m) IN ADVANCE OF THE MAIN FOURTE
- THE CLOSED PORTICN OF THE MAIN ROUTE SHALL BE PROTECTED BY
  b) BLOCKING WITH TYPE III BARRICADES, 1/2 OF THE CROSS SECTION
  OF THE CLOSED PORTION.
- 3. CONES MAY BE SUBSTITUTED FOR BARRICADES OR DRUMS AT HALF THE SPACING DURING DAY OPERATIONS. CONES SHALL BE A MINIMUM OF 28 (710)
  IN HEIGHT
- WHEN THE SIDE ROAD LIES BETWEEN THE BEGINNING OF THE MAINLINE
  4. SIGNING AND THE WORK ZONE, A SINGLE HEADED ARROW (M6-1) SHALL
  BE USED IN LIEU OF THE DOUBLE HEADED ARROW (M6-4).

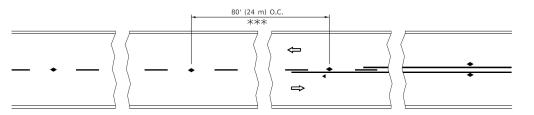
- 5. WHEN WORK IS BEING PERFORMED ON A SIDE ROAD OR DRIVEWAY, FOLLOW THE APPLICABLE STANDARD(S). THE DIRECTIONAL ARROW (M6-1 OR M6-4) SHALL BE COVERED OR REMOVED WHEN NO LONGER CONSISTENT WITH THE TRAFFIC CONTROL SET-UP.
- 6. ADVANCE WARNING SIGNS ARE TO BE OMITTED ON DRIVEWAYS UNLESS OTHERWISE SPECIFIED IN THE PLANS OR BY THE ENGINEER
- 7. THE TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS SHALL BE INCLUDED IN THE COST OF SPECIFIED TRAFFIC CONTROL STANDARDS OR ITEMS.

All dimensions are in inches (millimeters) unless otherwise shown.



USER NAME = ken.moy	DESIGNED - KLM	REVISED -
	DRAWN - KLM	REVISED -
PLOT SCALE = 2.0000 '/ in.	CHECKED - BMC	REVISED -
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -

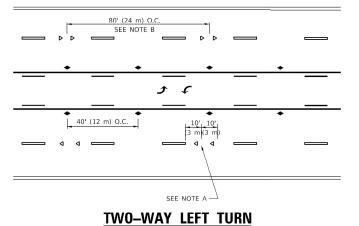
F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHE
3730	15-00131-01-BR		COOK	109	98
	TC-10		CONTRACT	NO. 6	2W4
FED. R	DAD DIST. NO. 1   ILLINOIS FEE	, AI	D PROJECT		



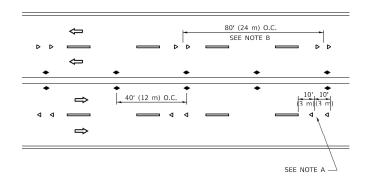
\*\*\* REDUCE TO 40' (12 m) O.C. ON CURVES WITH POSTED OR ADVISORY SPEED 45 M.P.H. (70 km/h) OR LESS.

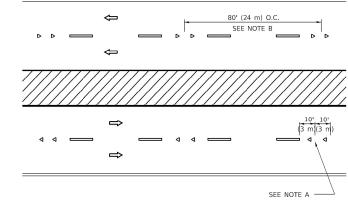
# 3 @ 40' (12 m) O.C. $\Rightarrow$ LANE REDUCTION TRANSITION

SEE FIGURE 3B-14 MUTCD



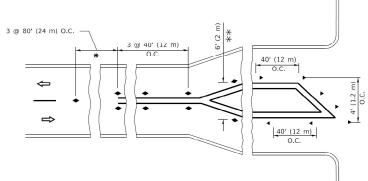
#### TW0-LANE/TW0-WAY

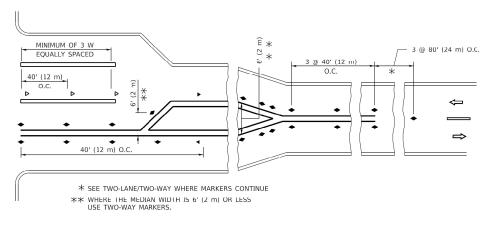




#### MULTI-LANE/UNDIVIDED







#### **TURN LANES**

#### **GENERAL NOTES**

- 1. MARKERS USED WITH DASHED LINES SHALL BE CENTERED IN THE GAP BETWEEN SEGMENTS.
- 2. MARKERS USED ADJACENT TO SOLID LINES SHALL BE OFFSET 2 TO 3 (50 TO 75) TOWARD TRAFFIC AS SHOWN.
- 3. MARKERS THROUGH TANGENTS LESS THAN 500' (150 m) IN LENGTH BETWEEN CURVES SHALL BE INSTALLED AT THE
- 4. MARKERS ARE TO BE USED ADJACENT TO BOTH SOLID WHITE LINES IN DUAL LEFT TURN LANES

#### LANE MARKER NOTES

- A. USE DOUBLE LANE LINE MARKERS SPACED AS SHOWN.
- B. REDUCE TO 40' (12 m) O.C. ON CURVES WHERE ADVISORY SPEEDS ARE 10 M.P.H (20 km/h) LOWER THAN POSTED SPEEDS.

#### **DESIGN NOTES**

- 1. DOUBLE LANE LINE MARKERS SHALL BE USED UNLESS SPECIFIED OTHERWISE
- 2. EXCEPT AS SHOWN ON THE LANE REDUCTION TRANSITION AND FREEWAY EXIT RAMP DETAIL, MARKERS ARE NOT TO BE SPECIFIED ON RIGHT EDGE LINES.
- 3. THE EXACT MARKER LIMITS, SPACING, AND COLOR SHALL BE INCLUDED IN
- 4. MARKERS SHOULD NOT BE USED ALONGSIDE CURBS EXCEPT FOR EXTREMELY INVOLVED.

All dimensions are in inches (millimeters) unless otherwise shown.

DESIGNED - KLM REVISED USER NAME = ken.moy DRAWN KLM REVISED CHECKED ВМС REVISED PLOT DATE = 4/2/2024 DATE - 04/02/2024 REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

D1 STANDARD - TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS (SNOW-PLOW RESISTANT) SHEET NO. OF SHEETS STA.

SECTION COUNTY COOK 15-00131-01-BR 109 99 TC-11 CONTRACT NO. 62W48

SYMBOLS

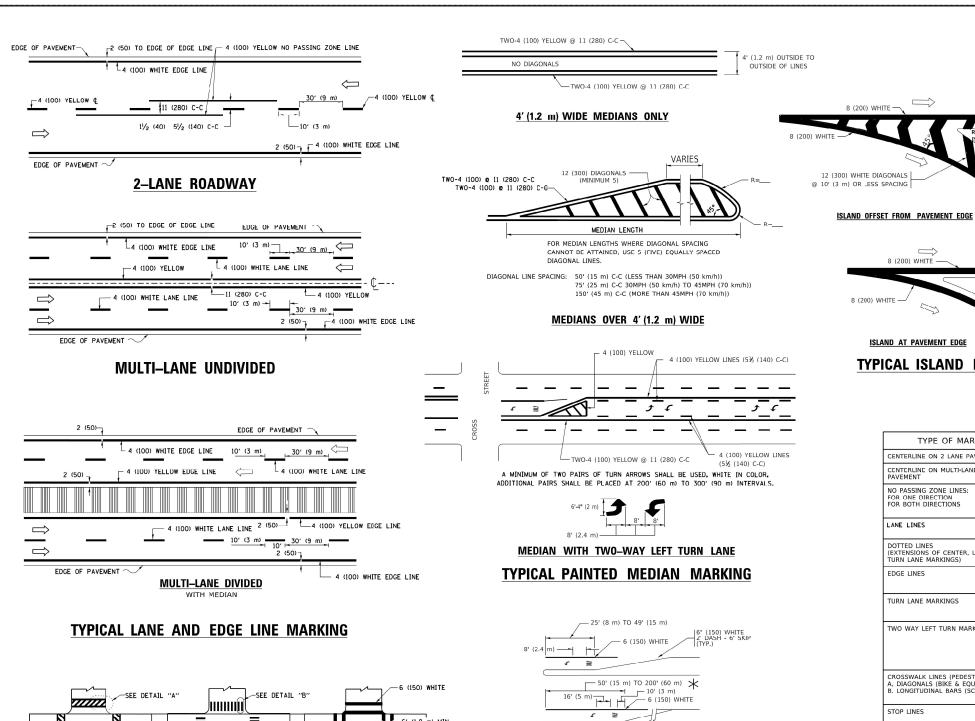
ONE-WAY AMBER MARKER

TWO-WAY AMBER MARKER

ONE-WAY CRYSTAL MARKER (W/O)

YELLOW STRIPE

WHITE STRIPE



# PEDESTRIAN 2' (600) -12 (300) WHITE DETAIL "B"

#### TYPICAL CROSSWALK MARKING

-6 (150) WHITE

DETAIL "A"

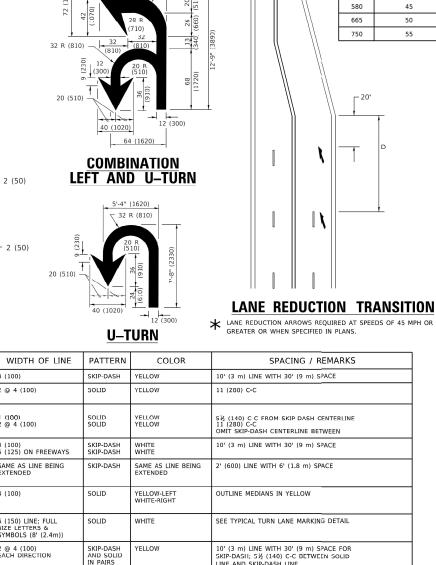
 $m{\star}$  MARKINGS SHALL BE INSTALLED PARALLEL TO THE CENTERLINE OF THE ROAD WHICH IT CROSSES

# OVER 200' (60 m) 6 (150) WHITE

 $\star$  TURN LANES IN EXCESS OF 400' (120 m) IN LENGTH MAY HAVE AN ADDITIONAL SET OF ARROW - "ONLY" INSTALLED MIDWAY BETWEEN THE OTHER TWO SETS OF

#### TYPICAL LEFT (OR RIGHT) TURN LANE

#### TYPICAL TURN LANE MARKING



D(FT)

345

425

SPEED LIMIT

CENTERLINE ON 2 LANE PAVEMENT	4 (100)	SKIP-DASH	YELLOW	10' (3 m) LINE WITH 30' (9 m) SPACE
CENTERLINE ON MULTI-LANE UNDIVIDED PAVEMENT	2 @ 4 (100)	SOLID	YELLOW	11 (280) C-C
NO PASSING ZONE LINES: FOR ONE DIRECTION FOR BOTH DIRECTIONS	4 (100) 2 @ 4 (100)	SOLID SOLID	YELLOW YELLOW	SK (140) C C FROM SKIP DASH CENTERLINE 11 (280) C-C OMIT SKIP-DASH CENTERLINE BETWEEN
LANE LINES	4 (100) 5 (125) ON FREEWAYS	SKIP-DASH SKIP-DASH	WHITE WHITE	10' (3 m) LINE WITH 30' (9 m) SPACE
DOTTED LINES (EXTENSIONS OF CENTER, LANE OR TURN LANE MARKINGS)	SAME AS LINE BEING EXTENDED	SKIP-DASH	SAME AS LINE BEING EXTENDED	2' (600) LINE WITH 6' (1.8 m) SPACE
EDGE LINES	4 (100)	SOLID	YELLOW-LEFT WHITE-RIGHT	OUTLINE MEDIANS IN YELLOW
TURN LANE MARKINGS	6 (150) LINE; FULL SIZE LETTERS & SYMBOLS (8' (2.4m))	SOLID	WHITE	SEE TYPICAL TURN LANE MARKING DETAIL
TWO WAY LEFT TURN MARKING	2 @ 4 (100) EACH DIRECTION 8' (2.4m) LEFT ARROW	SKIP-DASH AND SOLID IN PAIRS	YELLOW	10' (3 m) LINE WITH 30' (9 m) SPACE FOR SKIP-DASI; 5½ (140) C-C DETWEEN SOLID LINE AND SKIP-DASH LINE SEE TYPICAL TWO-WAY LEFT TURN MARKING DETAIL
CROSSWALK LINES (PEDESTRIAN) A. DIAGONALS (BIKE & EQUESTRIAN) B. LONGITUDINAL BARS (SCHOOL)	2 @ 6 (150) 12 (300) @ 45° 12 (300) @ 90°	SOLID SOLID SOLID	WHITE WHITE WHITE	NOT LESS THAN 6' (1.8 m) APART 2' (600) APART 2' (500) APART SEE TYPICAL CROSSWALK MARKING DETAILS.
STOP LINES	24 (600)	SOLID	WHITE	PLACE 4" (1.2 m) IN ADVANCE OF AND PARALLEL TO CROSSWALK, IF PRESENT. OTHERWISE, PLACE AT DESIRED STOPPING POINT: PARALLEL TO CROSSROAD CENTERLINE, WHERE POSSIBLE
PAINTED MEDIANS	2 @ 4 (100) WITH 12 (300) DIAGONALS @ 45° NO DIAGONALS USED FOR 4' (1.2 m) WIDE MEDIANS	SOLID	YELLOW: TWO WAY TRAFFIC WHITE: ONE WAY TRAFFIC	11 (280) C-C FOR THE DOUBLE LINE SEE TYPICAL PAINTED MEDIAN MARKING.
GORE MARKING AND CHANNELIZING LINES	8 (200) WITH 12 (300) DIAGONALS @ 45°	SOLID	WHITE	DIAGONALS: 15' (4.5 m) C-C (LESS THAN 30MPH (50 km/h)) 20' (6 m) C-C 30MPH (50 km/h) TO 45MPH (70 km/h)) 30' (9 m) C-C (OVER 45MPH (70 km/h))
RAILROAD CROSSING	24 (600) TRANSVERSE LINES; "RR" IS 6' (1.8 m) LETTERS; 16 (400) LINE FOR "X"	SOLID	WHITE	SEE STATE STANDARD 780001 AREA OF: "R"=3.6 SQ. FT. (0.33 m PEACH "X"=54.0 SQ. FT. (5.0 m P
SHOULDER DIAGONALS (REQUIRED FOR SHOULDERS > 8')	12 (300) @ 45°	SOLID	WHITE - RIGHT YELLOW - LEFT	50' (15 m) C-C (LESS THAN 30MPH (50 km/h)) 75' (25 m) C-C (30 MPH (50 km/h) TO 45MPH (70 km/h)) 150' (45 m) C-C (OVER 45MPH (70 km/h))
U TURN ARROW	SEE DETAIL	SOLID	WHITE	16.3 SF
2 ARROW COMBINATION LEFT AND U TURN	SEE DETAIL	SOLID	WHITE	30.4 SF

FOR FURTHER DETAILS ON PAVEMENT MARKING REFER TO STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND STATE STANDARD 780001.

All dimensions are in inches (millimeters) unless otherwise shown.



/////// BICYCLE & EQUESTRIAN

JSER NAME = ken.moy	DESIGNED - K	LM	REVISED	-
	DRAWN - K	LM	REVISED	-
PLOT SCALE = 2.0000 '/ in.	CHECKED - B	вмс	REVISED	-
PLOT DATE = 4/2/2024	DATE - Ø4	/02/2024	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

						F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
D1 :	D1 STANDARD – TYPICAL PAVEMENT MARKINGS						15-00131-01-BR	COOK	109	100
							TC-13 CONTRACT NO. 62			
	SHEET NO.	OF	SHEETS	STA.	TO STA.	FED. RO.	AD DIST, NO. 1 ILLINOIS FED. AI	D PROJECT		

SCALE: NONE

8 (200) WHITE -

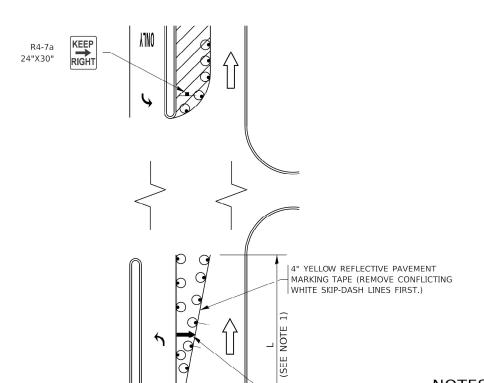
**ISLAND AT PAVEMENT EDGE** 

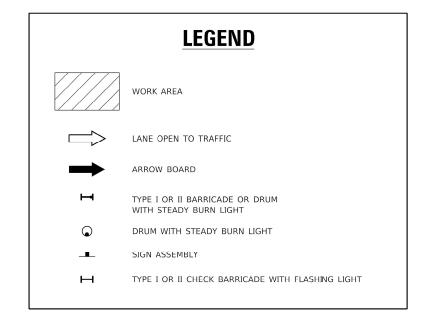
TYPICAL ISLAND MARKING

TYPE OF MARKING

RAISED

## TURN BAY ENTRANCE AT START OF LANE CLOSURE TAPER

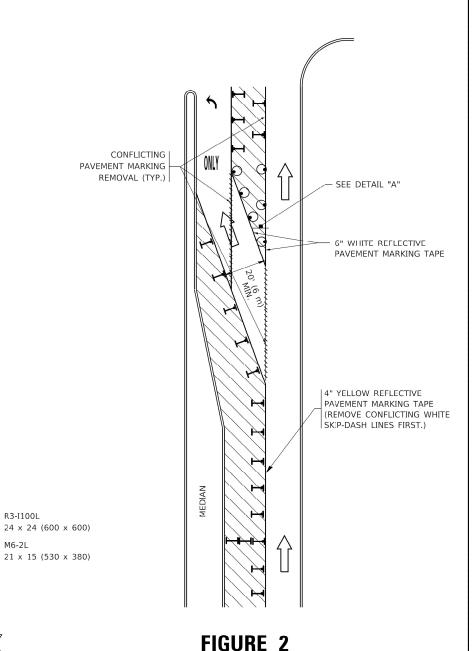




#### NOTES:

- 1. A) WHEN "L" IS ≤ THE STORAGE LENGTH OF THE TURN LANE (AS SHOWN IN FIG. 1), USE FIGURE 1.
  - B) WHEN "L" IS > THE STORAGE LENGTH OF THE TURN LANE OR THE TURN LANE IS WITHIN THE LANE CLOSURE, USE FIGURE 2.
- 2. CONES MAY BE SUBSTITUTED FOR BARRICADES OR DRUMS AT HALF THE SPACING DURING DAY OPERATIONS. CONES SHALL BE A MINIMUM OF 28 (710) IN HEIGHT.
- 3. LIGHTS WILL NOT BE REQUIRED ON BARRICADES OR DRUMS FOR DAY OPERATIONS. ALL LIGHTS SHALL BE MONODIRECTIONAL.
- 4. REFLECTIVE TEMPORARY PAVEMENT MARKINGS SHALL BE PLACED THROUGHOUT THE BARRICADED AREAS OF EACH TURN BAY AS SHOWN WHERE THE CLOSURE TIME IS GREATER THAN FOURTEEN (14) DAYS.
- 5. THIS APPLICATION ALSO APPLIES WHEN WORK IS BEING PERFORMED IN THE RIGHT LANE(S) AND THE RIGHT TURN BAY IS TO REMAIN OPEN. UNDER THIS CONDITION, "RIGHT TURN LANE" R3-I100R 24 x 24 (600 x 600) AND M6-2R 21 x 15 (530 x 380) SHALL BE USED.
- 6. THESE CONTROLS SHALL SUPPLEMENT MAINLINE TRAFFIC CONTROL FOR LANE CLOSURES.
- 7. THE SIGNS SHALL BE MOUNTED ABOVE THE BARRICADES/DRUMS ON SEPARATE SIGN SUPPORTS THAT MEET NCHRP 350 OR MASH PREQUIREMENTS.
- 8. TRAFFIC CONTROL AND PROTECTION AT TURN BAYS (TO REMAIN OPEN TO TRAFFIC) SHALL BE INCLUDED IN THE COST OF SPECIFIED TRAFFIC CONTROL STANDARDS OR ITEMS.

## **TURN BAY ENTRANCE** WITHIN A LANE CLOSURE



**DETAIL A** 

M6-2L

TURN

LANE

All dimensions are in inches (millimeters) unless otherwise shown



SEE DETAIL "A"

USER NAME = ken.moy	DESIGNED - KLM	REVISED -
	DRAWN - KLM	REVISED -
PLOT SCALE = 2.0000 '/ in.	CHECKED - BMC	REVISED -
PLOT DATE = 4/2/2024	DATE - 04/02/2024	REVISED -

FIGURE 1

- ARROW BOARD

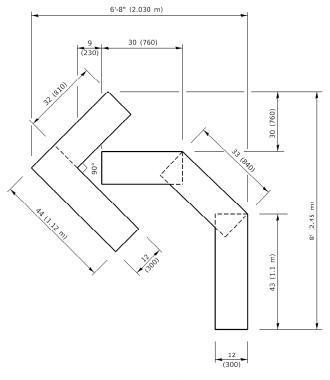
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

D1 STANDARD — TRAFFIC CONTROL AND PROTECTION AT TURN BAYS (TO REMAIN OPEN TO TRAFFIC)  SCALE: NONE SHEET NO. OF SHEETS STA. TO STA.					<del>-</del>
SCALE: NONE	SHEET NO.	OF	SHEETS	STA.	TO STA.

STABILIZE SIGN SUPPORT WITH

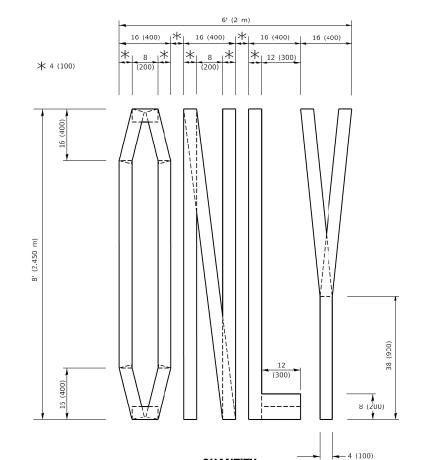
SANDBAGS AS

SECTION COUNTY COOK 15-00131-01-BR 109 101 TC-14 CONTRACT NO. 62W48 FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT



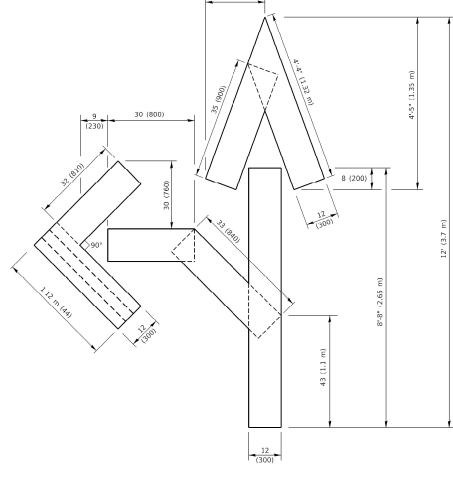
#### QUANTITY

4 (100) LINE = 45.5 ft. (13.9 m) 15.2 sq. ft. (1.41 sq. m)



4 (100) LINE = 64.1 ft. (19.5 m) 21.4 sq. ft. (1.99 sq. m)

QUANTITY

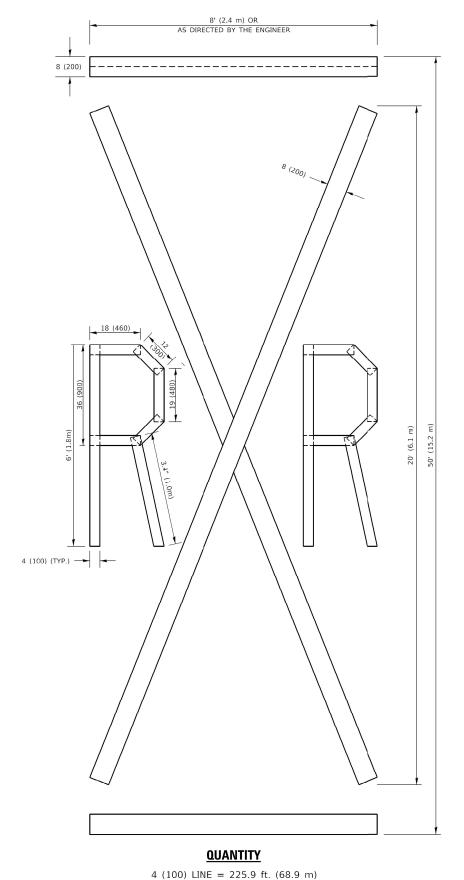


#### QUANTITY

4 (100) LINE = 82.5 ft. (25.1 m) 27.5 sq. ft. (2.53 sq. m)

#### NOTE:

ALL QUANTITIES OF PLACEMENT ARE REPRESENTED IN LINEAR FEET OF 4" LINES TO MATCH THE 4" TEMPORARY TAPE PAY ITEM AND REPRESENTS THE TOTAL QUANTITY OF 4" TAPE REQUIRED.



4 (100) LINE = 225.9 ft. (68.9 m) 75.3 sq. ft. (6.99 sq. m)

All dimensions are in inches (millimeters) unless otherwise shown.

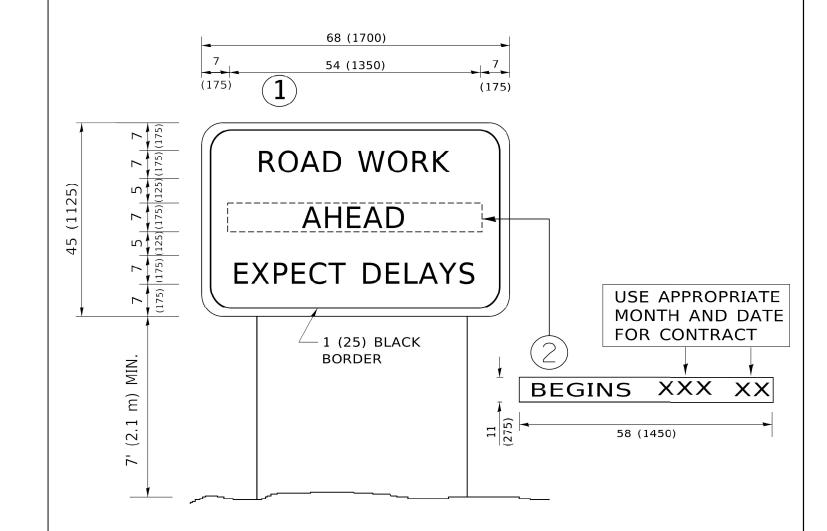
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT



USER NAME = ken.moy	DESIGNED	-	KLM	REVISED	-
	DRAWN	-	KLM	REVISED	-
PLOT SCALE = 2.0000 '/ in.	CHECKED	-	BMC	REVISED	-
PLOT DATE = 4/2/2024	DATE	-	04/02/2024	REVISED	-

D1 STANDARD — SHORT TERM PAVEMENT MARKING LETTERS AND SYMBOLS  SCALE: NONE SHEET NO. OF SHEETS STA. TO STA.										
	SCALE: NONE	SHEET NO	. OF	SHEETS	STA.	TO STA.				

Ŀ.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
30	15-00131-01-BR	COOK	109	102
	TC-16	CONTRACT	NO. 6	2W48



### NOTES:

- 1. USE BLACK LETTERING ON ORANGE BACKGROUND.
- 2. ERECT SIGNS IN ADVANCE OF THE LOCATION FOR THE "ROAD CONSTRUCTION AHEAD" SIGN AT LOCATIONS AS DIRECTED BY THE ENGINEER.
- 3. ERECT SIGN 1 WITH INSTALLED PANEL 2 ONE WEEK PRIOR TO THE START OF CONSTRUCTION.
- 4. REMOVE PANEL 2 SOON AFTER THE START OF CONSTRUCTION.
- 5. SEE SPECIAL PROVISION FOR "TEMPORARY INFORMATION SIGNING" FOR ADDITIONAL INFORMATION.
- 6. ONE SIGN ASSEMBLY EQUALS 25.70 SQ. FT. (2.3 SQ. M.)

SCALE: NONE

7. SHALL BE PAID FOR AS TEMPORARY INFORMATION SIGNING.

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.



USER NAME = ken.moy	DESIGNED	-	KLM	REVISED	-
	DRAWN	-	KLM	REVISED	-
PLOT SCALE = 2.0000 '/ in.	CHECKED	-	ВМС	REVISED	-
PLOT DATE = 4/2/2024	DATE	-	04/02/2024	REVISED	-

D1 STANDARD - ARTERIAL ROAD						F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
INFORMATION SIGN					3730	15-00131-01-BR	COOK	109	103	
		IIVI OII	INATION	SIGIV			TC-22	CONTRACT	NO. 6	2W48
	SHEET NO.	OF	SHEETS	STA.	TO STA.	FED. RO	DAD DIST. NO. 1 ILLINOIS FED. A	D PROJECT		

