PGL & B ROADWAY

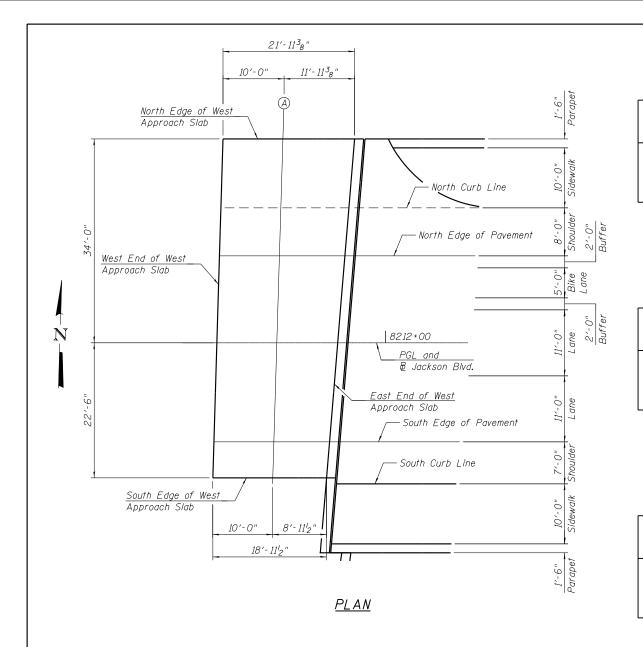
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Jackson G1	8240+30.88	0.00	594.34	594.33
1 A 1 B 1 C 1 D 1 E 1 F 1 G	8240+40.88 8240+50.88 8240+60.88 8240+70.88 8240+80.88 8240+90.88 8241+00.88	0.00 0.00 0.00 0.00 0.00 0.00	594.18 594.03 593.87 593.72 593.56 593.39 593.12	594.20 594.07 593.93 593.77 593.60 593.41 593.13
CL Brg. Pier R1	8241+10.88	0.00	592.75	592.75
2 A 2 B 2 C 2 D 2 E 2 F 2 G	8241+20.88 8241+30.88 8241+40.88 8241+50.88 8241+60.88 8241+70.88 8241+80.88	0.00 0.00 0.00 0.00 0.00 0.00	592.27 591.69 591.01 590.24 589.47 588.69 587.91	592.29 591.73 591.07 590.32 589.54 588.75 587.95
CL Brg. N. Abut.	8241+90.88	0.00	587.14	587.14
Bk. N. Abut.	8241+94.38	0.00	586.87	586.87

<u>GIRDER R7</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Jackson G1	8240+30.88	0.63	594.31	594.31
1 A 1 B 1 C 1 D 1 E 1 F 1 G	8240+40.88 8240+50.88 8240+60.88 8240+70.88 8240+80.88 8240+90.88 8241+00.88	0.63 0.63 0.63 0.63 0.63 0.63	594.16 594.01 593.85 593.70 593.54 593.37 593.10	594.18 594.05 593.90 593.75 593.58 593.39 593.11
CL Brg. Pier R1	8241+10.88	0.63	592.73	592.73
2A 2B 2C 2D 2E 2F 2G	8241+20.88 8241+30.88 8241+40.88 8241+50.88 8241+60.88 8241+70.88 8241+80.88	0.63 0.63 0.63 0.63 0.63 0.63	592.26 591.68 590.99 590.23 589.45 588.67 587.90	592.27 591.71 591.05 590.30 589.52 588.73 587.93
CL Brg. N. Abut.	8241+90.88	0.63	587.12	587.12
Bk. N. Abut.	8241+94.38	0.63	586.85	586.85

USER NAME = wjcolletti	DESIGNED	JM	REVISED	Г
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JM	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

TOP OF SLAB ELEVATIONS 3 — RAMP STRUCTURE NO.016—1702	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHE NO
	1422	2014-015R&B-R	COOK	825	40
31110C1011L 140.010-1702			CONTRACT	NO. 6	0X9
SHEET NO. S2-16 OF S2-80 SHEETS		ILLINOIS FED. A	ID PROJECT		



NORTH EDGE OF WEST APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevations
W. End West Appr. Slab	8211+72.92	-34.00	595.43
A	8211+82.92	-34.00	595.92
E. End West Appr. Slab	8211+94.86	-34.00	596.43

PGL & B ROADWAY

Location	Station	Offset	Theoretical Grade Elevations				
W. End West Appr. Slab A E. End West Appr. Slab	8211+71.90 8211+81.90 8211+92.05	0.00 0.00 0.00	595.91 596.40 596.85				

NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
W. End West Appr. Slab	8211+72.57	-22.50	595.59
A	8211+82.57	-22.50	596.08
E. End West Appr. Slab	8211+93.91	-22.50	596.57

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End West Appr. Slab	8211+71.41	16.50	595.63
A	8211+81.41	16.50	596.12
E. End West Appr. Slab	8211+90.68	16.50	596.53

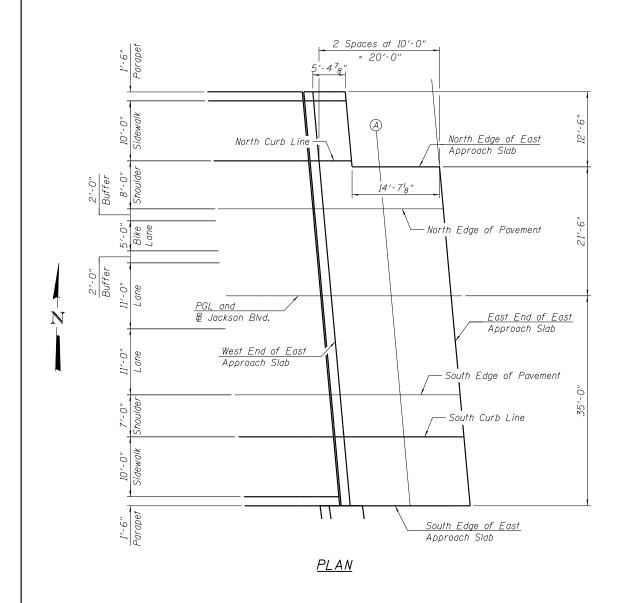
NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End West Appr. Slab	8211+72.33	-14.50	595.71
A	8211+82.33	-14.50	596.19
E. End West Appr. Slab	8211+93.25	-14.50	596.67

SOUTH EDGE OF WEST APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevations
W. End West Appr. Slab	8211+71.23	22.50	595.53
A	8211+81.23	22.50	596.02
E. End West Appr. Slab	8211+90.18	22.50	596.42

	USER NAME = wjcolletti	DESIGNED	JM	REVISED	
		CHECKED	WJC	REVISED	
>	PLOT SCALE = NTS	DRAWN	JM	REVISED	
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	



NORTH EDGE OF EAST APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevations
W. End East Appr. Slab	8214+56.38	-34.00	592.82
A	8214+67.50	-21.50	592.82
E. End East Appr. Slab	8214+77.50	-21.50	592.69

PGL & B ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
W. End East Appr. Slab	8214+59.44	0.00	593.29
A	8214+69.44	0.00	593.13
E. End East Appr. Slab	8214+79.44	0.00	593.01

NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
W. End East Appr. Slab	8214+57.41	-22.50	592.98

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End East Appr. Slab	8214+60.93	16.50	593.01
A	8214+70.93	16.50	592.85
E. End East Appr. Slab	8214+80.93	16.50	592.70

NORTH EDGE OF PAVEMENT

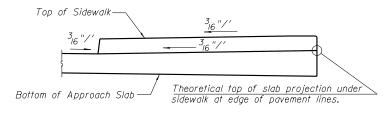
Location	Station	Offset	Theoretical Grade Elevations
W. End East Appr. Slab	8214+58.14	-14.50	593.09
A	8214+68.14	-14.50	592.92
E. End East Appr. Slab	8214+78.14	-14.50	592.80

SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations
W. End East Appr. Slab	8214+61.56	23.50	592.89
A	8214+71.56	23.50	592.73
E. End East Appr. Slab	8214+81.56	23.50	592.57

Theoretical top of slab projection under sidewalk at edge of pavement lines.

LOCATION OF ELEVATION UNDER NORTH SIDEWALK



LOCATION OF ELEVATION UNDER SOUTH SIDEWALK

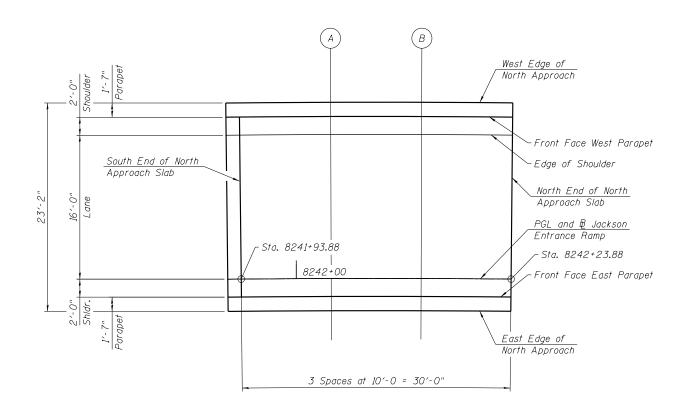
SOUTH EDGE OF EAST APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevations
W. End East Appr. Slab A E. End East Appr. Slab	8214+62.60 8214+72.60 8214+82.60	35.00 35.00 35.00	593.05 592.90 592.73



USER NAME = wjcolletti	DESIGNED	JM	REVISED	
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JM	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

TOP OF EAST APPROACH SLAB ELEVATIONS	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-1702	1422	2014-015R&B-R	COOK	825	403
0111001011L 140: 010-1702			CONTRACT	NO. 6	0X94
SHEET NO. S2-18 OF S2-80 SHEETS		TILL INOTS FED. AT	D PROJECT		



PLAN

WEST EDGE OF NORTH APPROACH

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Slab A B	8241+93.88 8242+03.88 8242+13.88	-19.58 -19.58 -19.58	587.41 586.64 585.86
N. End North Appr. Slab	8242+23.88	-19.58	585.09

FRONT FACE WEST PARAPET

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Slab A B	8241+93.88 8242+03.88 8242+13.88	-18.00 -18.00 -18.00	587.37 586.60 585.82
N. End North Appr. Slab	8242+23.88	-18.00	585.04

EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Slab A B N. End North Appr. Slab	8241+93.88	-16.00	587.32
	8242+03.88	-16.00	586.54
	8242+13.88	-16.00	585.77
	8242+23.88	-16.00	584.99

PGL & B ROADWAY

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Slab A B	8241+93.88 8242+03.88 8242+13.88	0.00 0.00 0.00	586.91 586.13 585.35
N. End North Appr. Slab	8242+23.88	0.00	584.58

FRONT FACE EAST PARAPET

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Slab	8241+93.88 8242+03.88	2.00	586.85 586.08
N. End North Appr. Slab	8242+13.88 8242+23.88	2.00 2.00	585.30 584.52

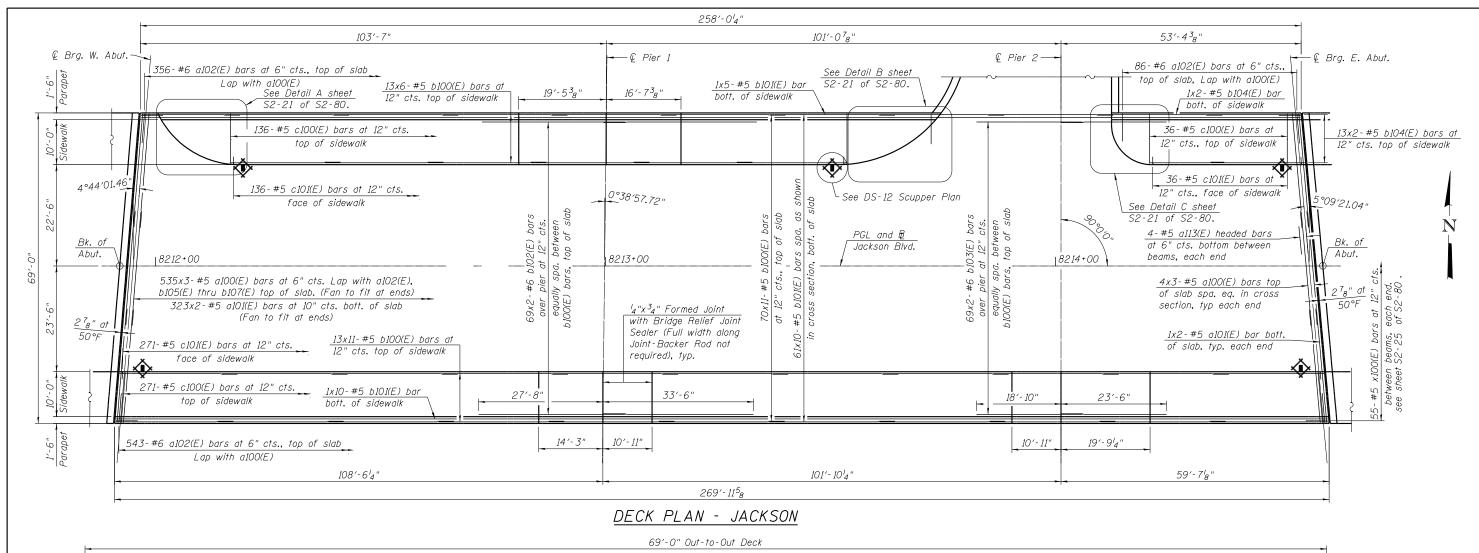
EAST EDGE OF NORTH APPROACH

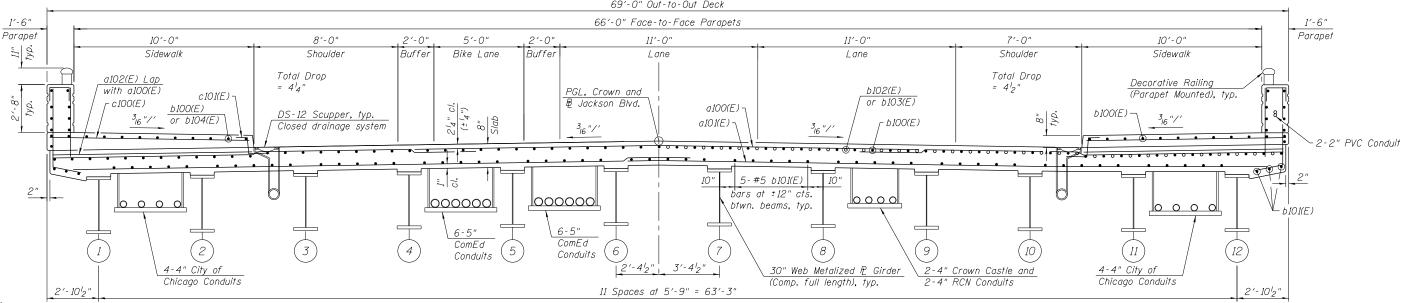
Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Slab A B	8241+93.88 8242+03.88 8242+13.88	3.58 3.58 3.58	586.85 586.08 585.30
N. End North Appr. Slab	8242+23.88	3.58	584.52



USER NAME = wjcolletti	DESIGNED	JM	REVISED	
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JM	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

TOP OF NORTH APPROACH SLAB ELEVATIONS	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	
STRUCTURE NO 016_1702	1422	2014-015R&B-R	COOK	825	404
0111001011L NO: 010-1702			CONTRACT	NO. 6	0X94
TOP OF NORTH APPROACH SLAB ELEVATIONS STRUCTURE NO. 016-1702 SHEET NO. 52-19 OF 52-80 SHEETS		ILL INOIS FED. AT	D PROJECT		





Notes:

Bars indicated thus 13x8-#5 etc. indicates 13 lines of bars with 8 lengths per line.

Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on Sheet S2-31 of S2-80.

See Sheet S2-25 of S2-80 for Bill of Material.

See Sheet S2-23 and S2-24 of S2-80 for parapet reinforcement. For Scupper locations see Sheet S2-04 of S2-80.

For Scupper Plan, see Sheet S2-21 of S2-80.

NEAR MIDSPAN

<u>CROSS SECTION - JACKSON</u>

(Looking East)

Details for the proposed City of Chicago conduit support system are shown on the CDOT Lighting Plans. The concrete inserts will be provided by the contractor. The installation location of the concrete inserts will be determined in the field by the contractor. There is no separate payment for the placement of the inserts. Cost of the work involved in installing the inserts included with Concrete Superstructure.

Proposed conduit support systems for the utilities are shown for information only. The concrete inserts will be provided to the Contractor by the utility companies. The Contractor is responsible for placing inserts per layout details and direction from the utility companies. The utility companies may elect to provide support to Contractor for final insert placement in advance of pouring concrete. There is no separate payment for the placement of inserts. The work involved in placing inserts is included in the cost of Concrete Superstructure.

NEAR PIER

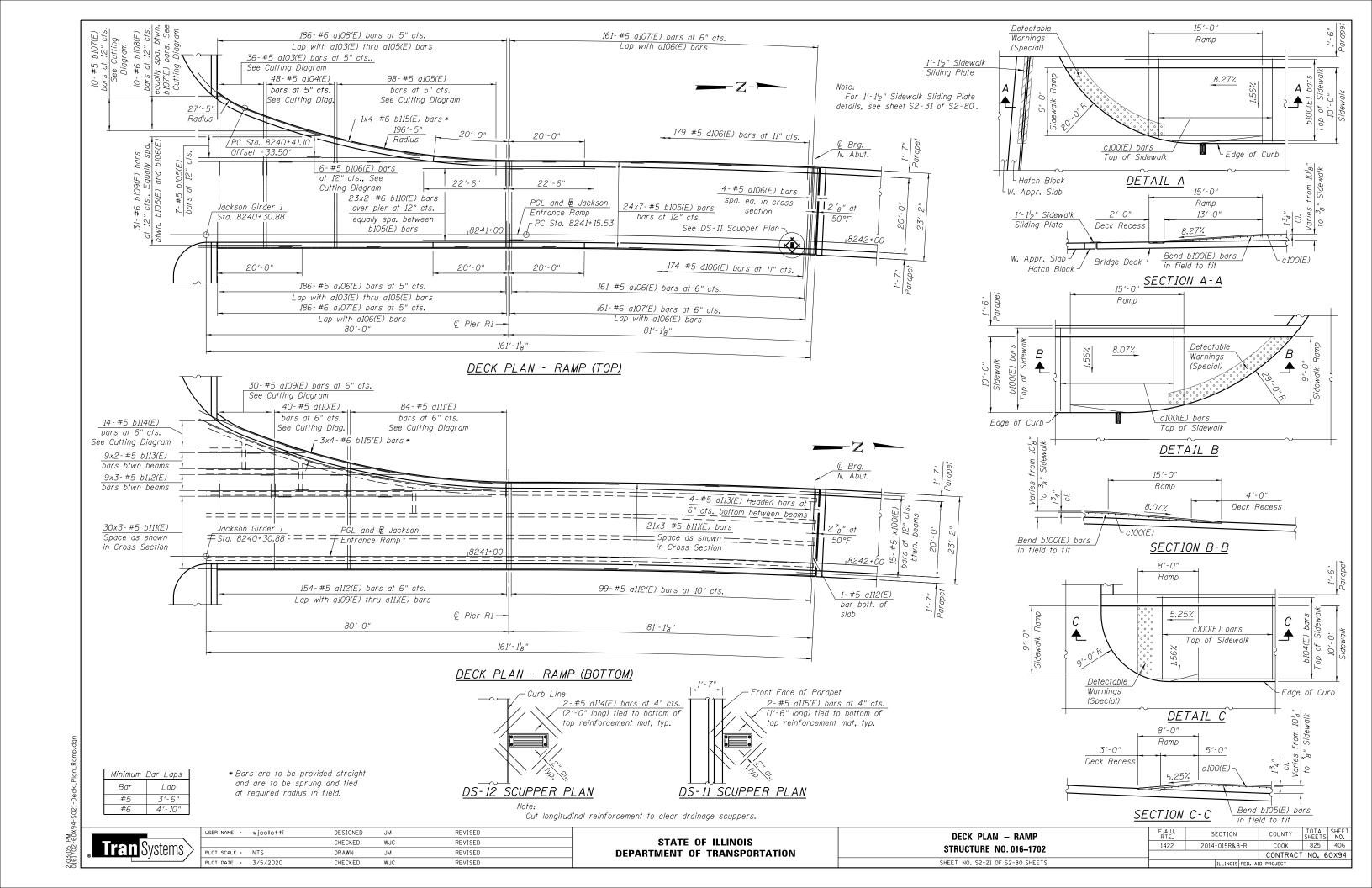
Minimum .	Bar Laps
Bar	Lap
#5	3′-6"
#6	4′-10"

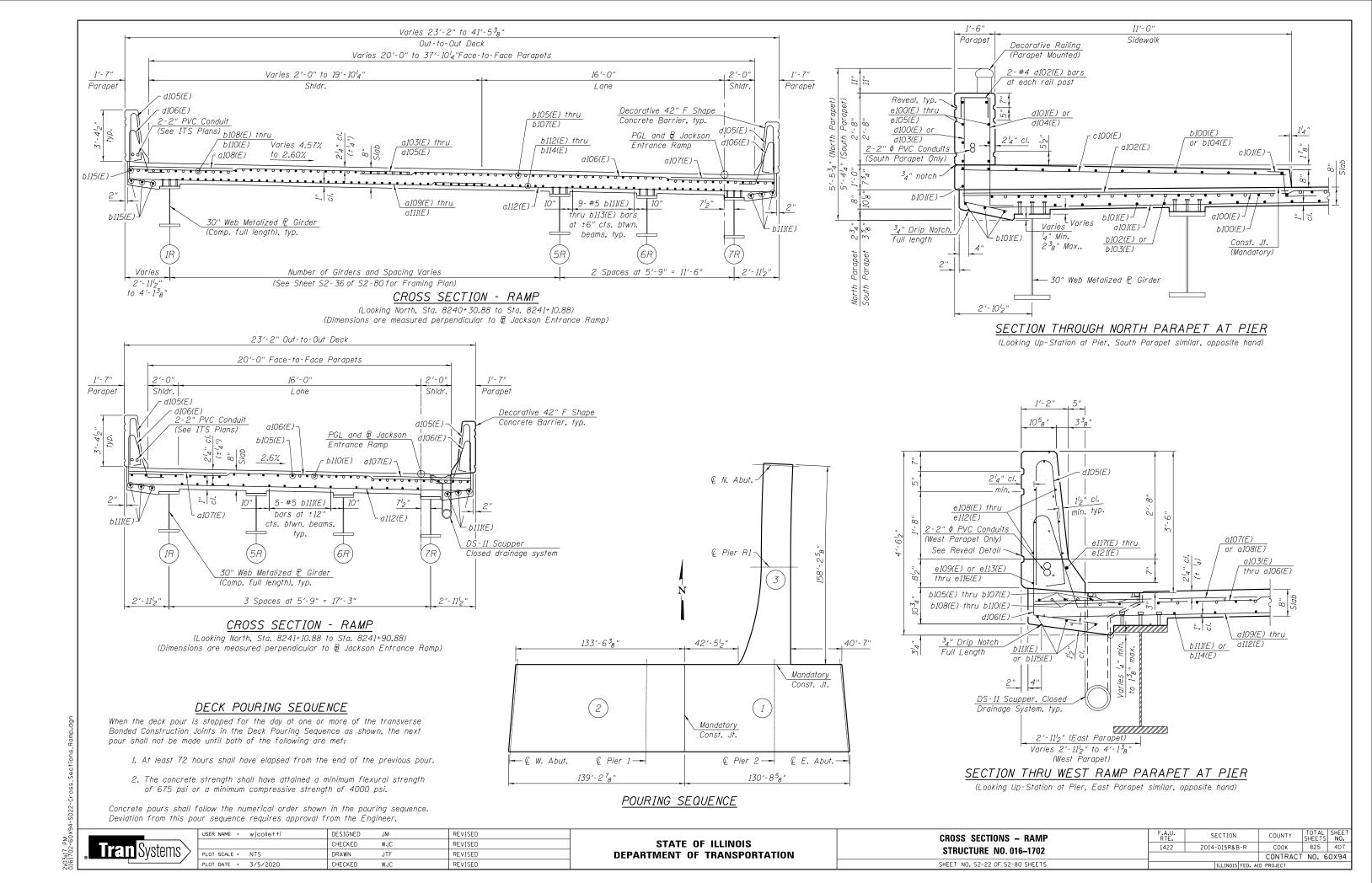


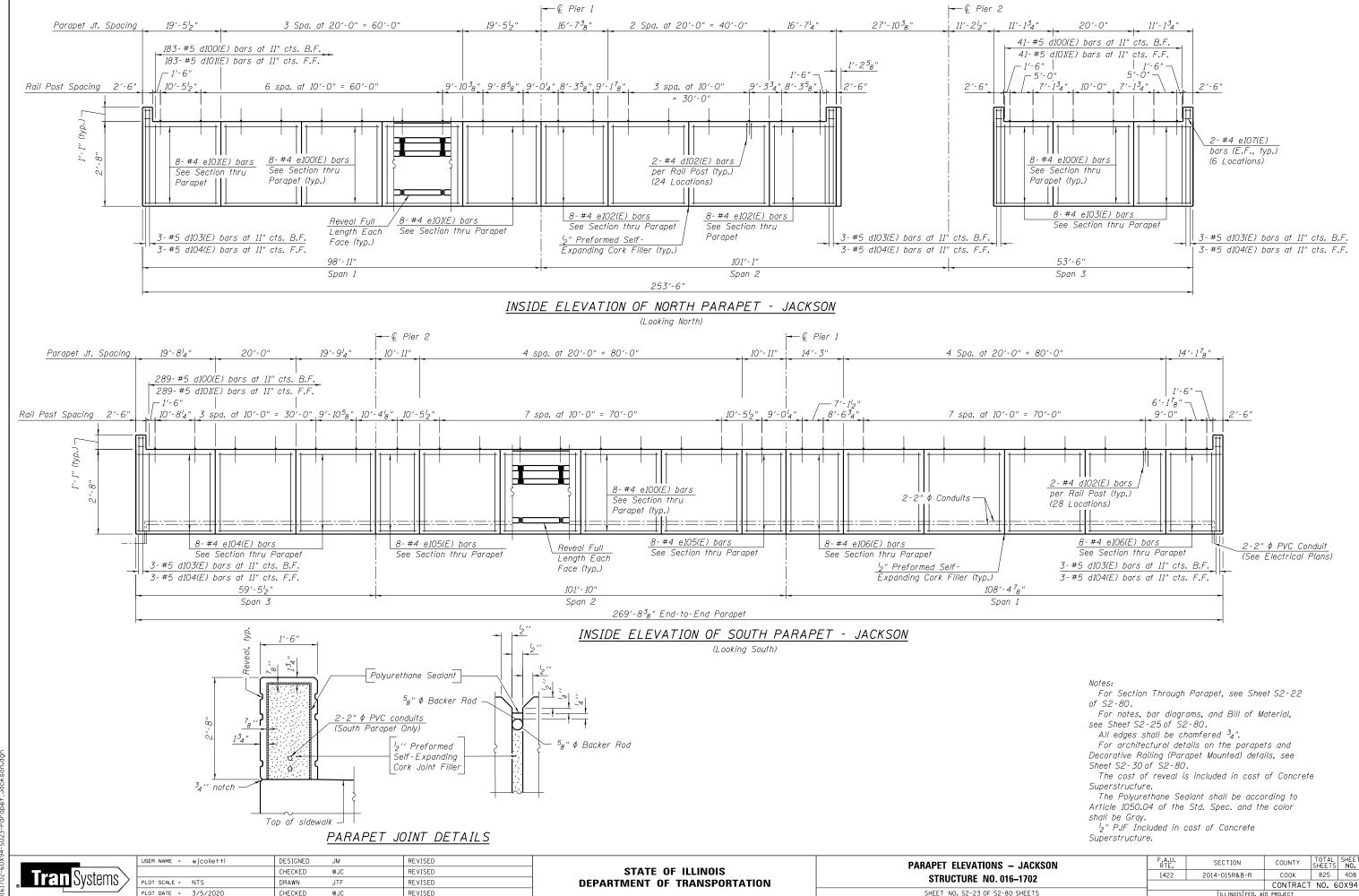
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		CHECKED	WJC	REVISED
>	PLOT SCALE = NTS	DRAWN	JTF	REVISED
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

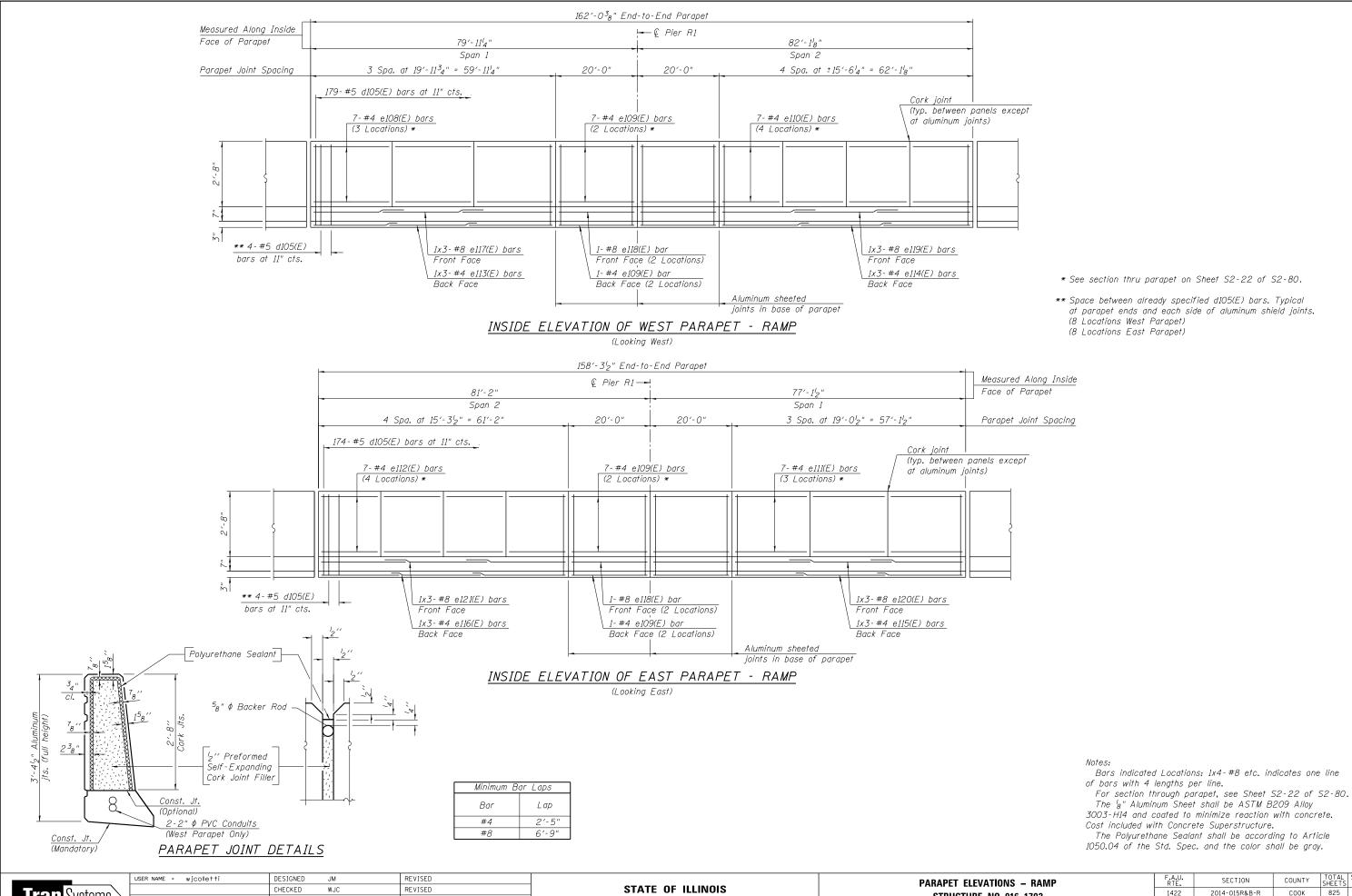
DECK PLAN AND CROSS SECTION – JACKSON	F.A.U. RTE.	SECTION	COUNTY	TOTAL	
STRUCTURE NO. 016-1702	1422	2014-015R&B-R	COOK	825	405
0111001011E 110:010-1702			CONTRACT	NO.	60X94
SHEET NO. S2-20 OF S2-80 SHEETS		ILL INOIS FED. A	D. PROJECT		







2:03:27 PM 0161702-60x94-5023-Parabet Jacks



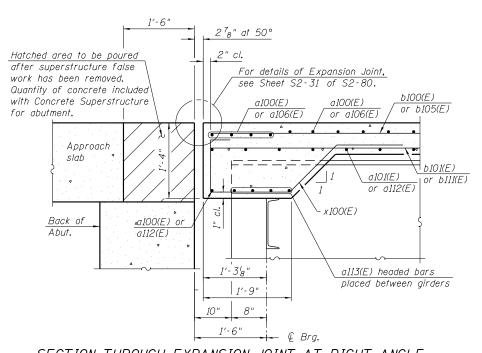
Tran Systems

DRAWN REVISED PLOT DATE = 3/5/2020 CHECKED WJC REVISED

DEPARTMENT OF TRANSPORTATION

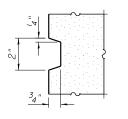
STRUCTURE NO. 016-1702 SHEET NO. S2-24 OF S2-80 SHEETS

SECTION COUNTY 1422 2014-015R&B-R COOK 825 409 CONTRACT NO. 60X94

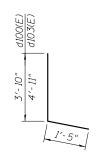


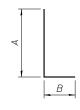
SECTION THROUGH EXPANSION JOINT AT RIGHT ANGLE

(Looking North, West joint shown, East and North joints similar)



REVEAL DETAIL



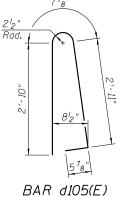


BAR d101(E) AND d104(E)

<u>BAR</u>	d100(E)
AND	d103(E)

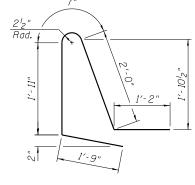
Bar	Α	В
d101(E)	3'-10"	10"
d104(E)	4'-11"	10"



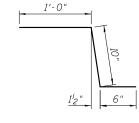




BAR d102(E)



BAR d106(E)



BAR c101(E)

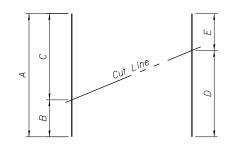
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a100(E)	1629	#5	25′-3"	
a101(E)	650	#5	36′-0"	
a102(E)	985	#6	6′-8"	
a103(E)	18	#5	38′-11"	
a104(E)	24	#5	26′-0"	
a105(E)	49	#5	15′-0"	
a106(E)	351	#5	22′-6"	
a107(E)	508	#6	6′-6"	
a108(E)	186	#6	7′-0"	
a109(E)	15	#5	36′-4"	
a110(E)	20	#5	23′-5"	
a111(E)	42	#5	12'-4"	
a112(E)	254	#5	22'-6"	
a113(E)	100	#5	5′-5"	<u> </u>
a114(E)	40	#5	2'-0"	
a115(F)	8	#5	1'-6"	

_			
	II		

BAR x100(E)

1'-6"



BAR CUTTING DIAGRAM

Order bars full length

Bar	Α	В	С	D	Ε
a103(E)	38′-11"	22'-11"	16'-0"	19′-7"	19'-4"
a104(E)	26'-0"	<i>15′-10"</i>	10'-2"	13′-0"	13'-0"
a105(E)	15'-0"	10'-1"	4'-11"	7′-6"	7′-6"
a109(E)	36′-4"	14'-9"	21'-7"	18′-1"	18′-3"
a110(E)	23'-5"	8'-10"	14'-7"	11'-8"	11'-9"
a111(E)	12'-4"	3'-7"	8'-9"	6'-2"	6'-2"
b106(E)	54'-3"	11'-4"	42'-11"	27'-8"	26'-7"
b107(E)	38′-3"	8'-3"	30′-0"	17′-10"	20′-5"
b108(E)	36′-9"	7′-7"	29'-2"	17'-0"	19′-9"
b114(E)	29'-5"	6'-2"	23'-3"	14'-1"	15′-4"

Bar	No.	Size	Length	Shape
b100(E)	991	#5	27'-9"	
b101(E)	625	#5	30'-2"	
b102(E)	138	#6	33'-0"	
b103(E)	138		23'-7"	
		#6	20'-0"	
b104(E)	28	#5		
b105(E)	175	#5	26′-9"	
b106(E)	3	#5	54′-3"	
b107(E)	5	#5	38'-3"	
b108(E)	5	#6	36′-9"	
b109(E)	31	#6	29'-5"	
b110(E)	46	#6	24'-11"	
			30'-0"	
b111(E)	153	#5		
b112(E)	27	#5	28′-0"	
b113(E)	18	#5	24'-2"	
b114(E)	7	#5	29'-5"	
b115(E)	16	#6	28'-5"	
D110(E)	10			
c100(E)	443	#6	11'-2"	
		#5		
c101(E)	443	#5	2'-4"	
d100(E)	513	#5	5′-3"	
d101(E)	513	#5	4'-8"	Ì
d102(E)	104	#4	2'-7"	-
d102(E)	18		6'-4"	
		#5	5'-9"	<u> </u>
d104(E)	18	#5		L
d105(E)	417	#5	6′-11"	/J
d106(E)	353	#5	7′-5"	
e100(E)	120	#4	19′-8"	
e101(E)	16	#4	19'-2"	
e102(E)	16	#4	16'-3"	
e103(E)	16	#4	10'-11"	
e104(E)	16	#4	19′-5"	
e105(E)	16	#4	10'-7"	
e106(E)	16	#4	13'-11"	
e107(E)	24	#4	2'-2"	
e108(E)	21	#4	19'-7"	
e109(E)	32	#4	19'-8"	
			15'-3"	
e110(E)	28	#4		
e111(E)	21	#4	18′-9"	
e112(E)	28	#4	15′-0"	
e113(E)	3	#4	21'-6"	
e114(E)	3	#4	22'-3"	
e115(E)	3	#4	20'-7"	
e116(E)	3	#4	21'-11"	
	3		24'-5"	
e117(E)		#8		
e118(E)	4	#8	19'-8"	
e119(E)	3	#8	25′-2"	
e120(E)	3	#8	23′-6"	
e121(E)	3	#8	24'-10"	
x100(E)	125	#5	6′-5"	
/100(L)	165	"."		
Doinfair	most C			
Reinforce		ars,	Pound	202,690
Ероху Со	ated		. 55.76	,
Concrete			Cu va	816 1
Superstru	icture		Cu. Yd.	846.1
	rotective Coat			2,765
Bridge De		ovina	Sq. Yd. Sq. Yd.	1,422
			Jy. 10.	19166
Detectable	E WALLUL	iys	Sq. Ft.	92
(Special)				
Bridge De		oving	Sq. Yd.	402
(Longitudi	'nal)		Jy. 14.	702
(Edity) admin				

No. Size Length Shape

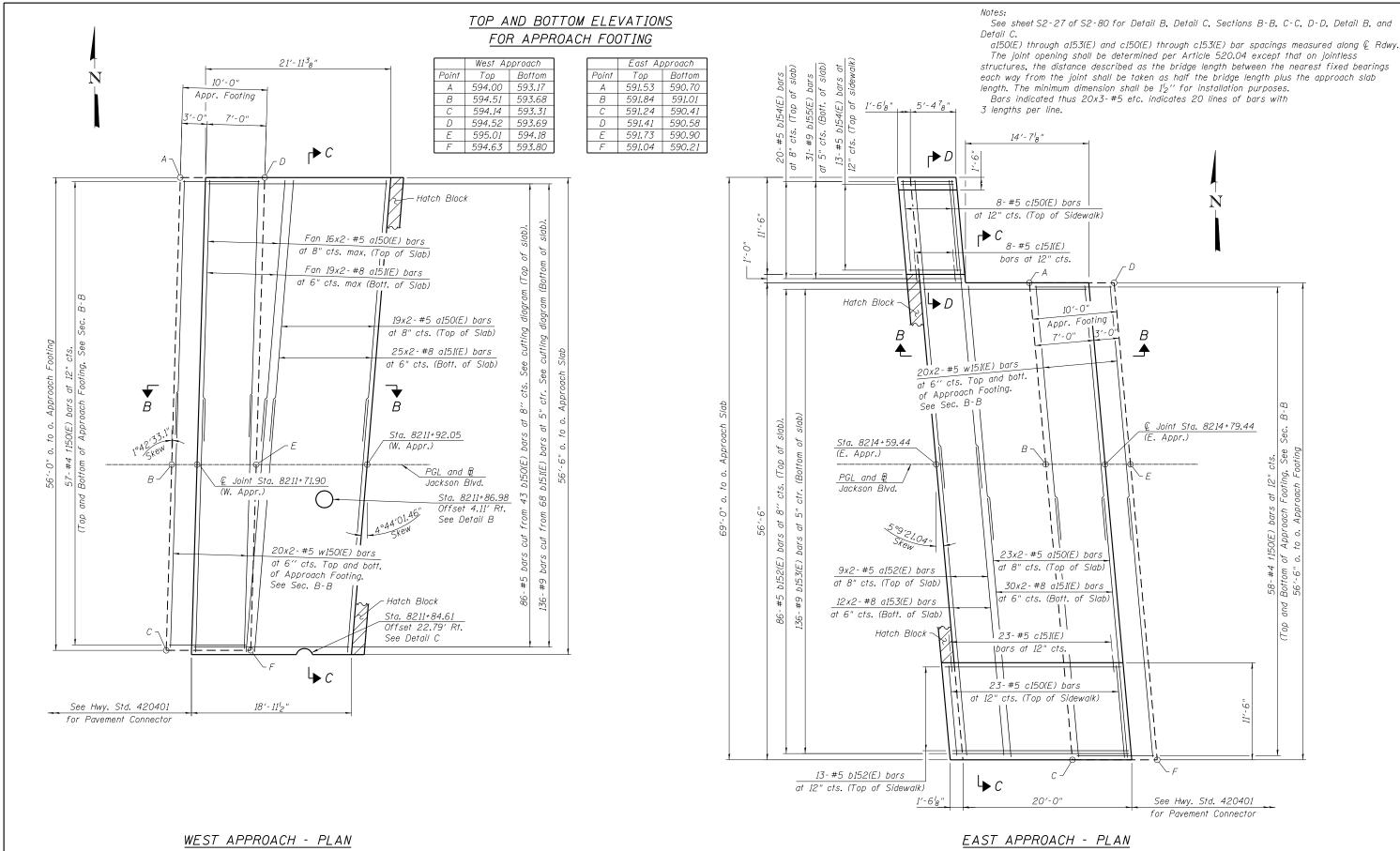
Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated.



USER NAME = wjcolletti	DESIGNED	JM	REVISED	
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JM	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

SUPERSTRUCTURE DETAILS						
STR	UCT	TURE	NO	016	-1702	
SHEET	NΟ	\$2-25	ΛF	S2-80	SHEETS	

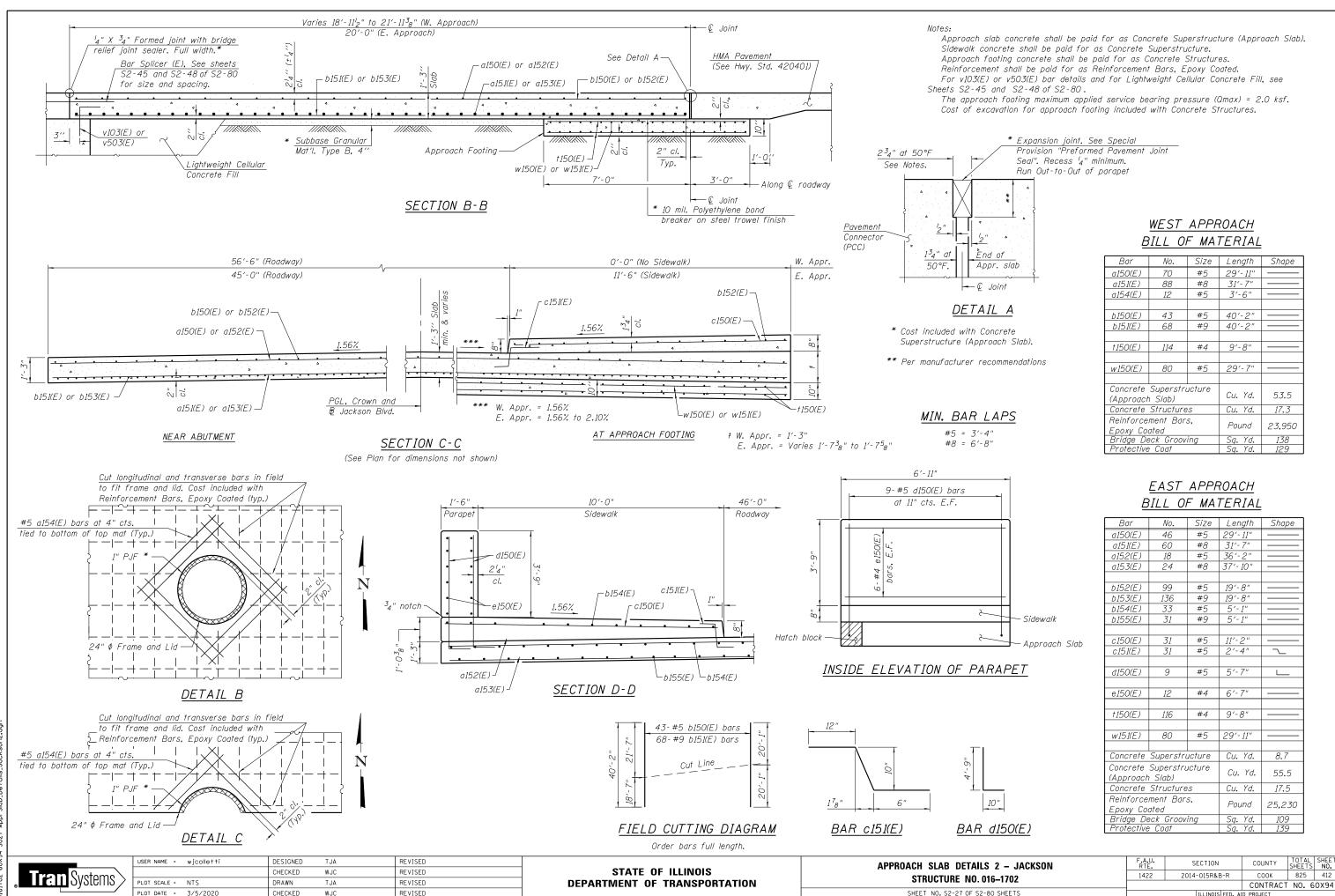
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1422	2014-015R&B-R	COOK	825	410
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. AI	D PROJECT		





	USER NAME = wjcolletti	DESIGNED	TJA	REVISED	
		CHECKED	WJC	REVISED	
•	PLOT SCALE = NTS	DRAWN	TJA	REVISED	
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	
					-

APPROACH SLAB DETAILS 1 – JACKSON	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-1702	1422	2014-015R&B-R	COOK	825	411
SINUCIONE NO. 010-1702			CONTRACT	NO. 6	0X94
SHEET NO. S2-26 OF S2-80 SHEETS	ILLINOIS FED. AID		D PROJECT		



53.5

23,950

55.5

25,230

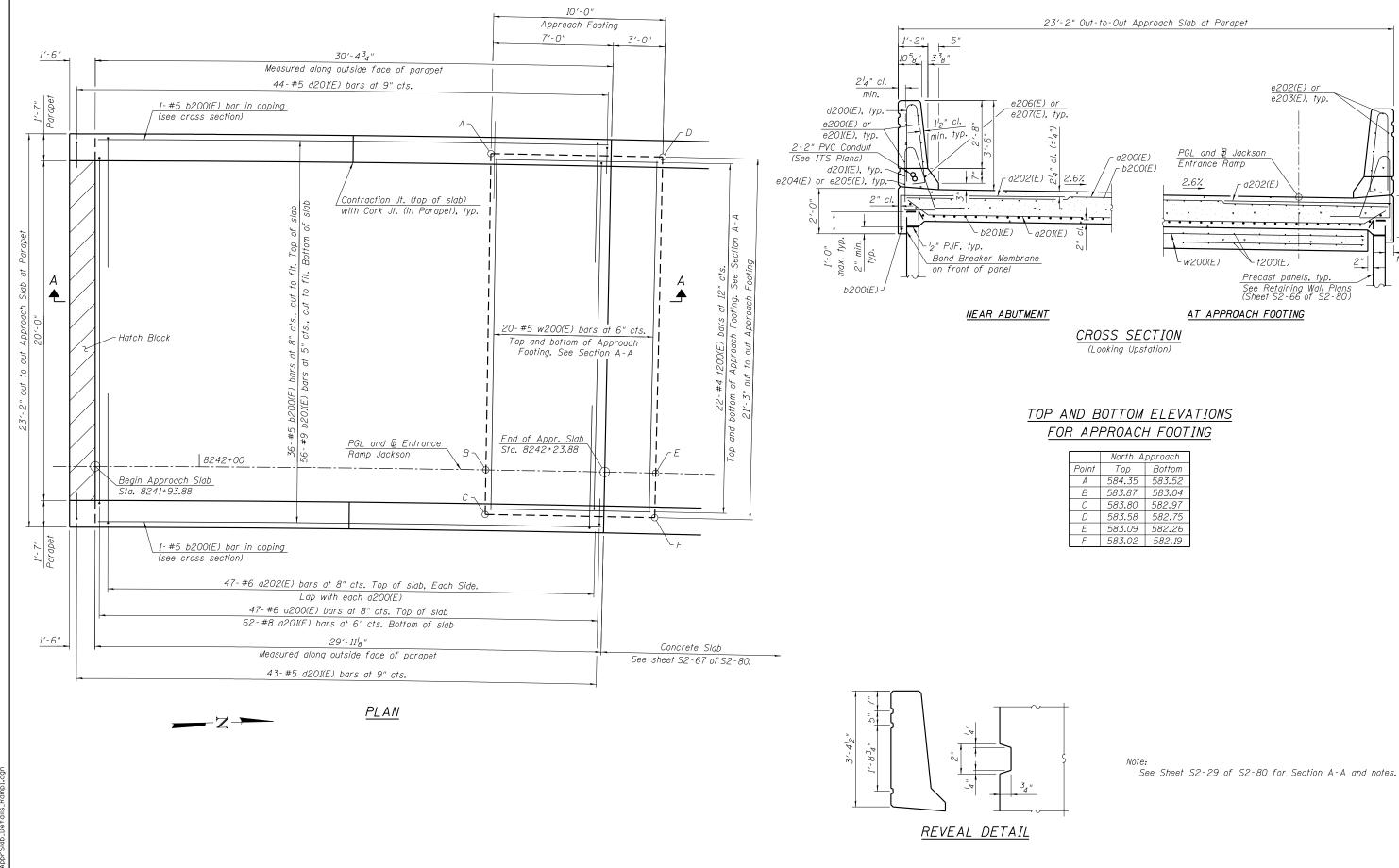
825 412

CHECKED

WJC

REVISED

PLOT DATE = 3/5/2020



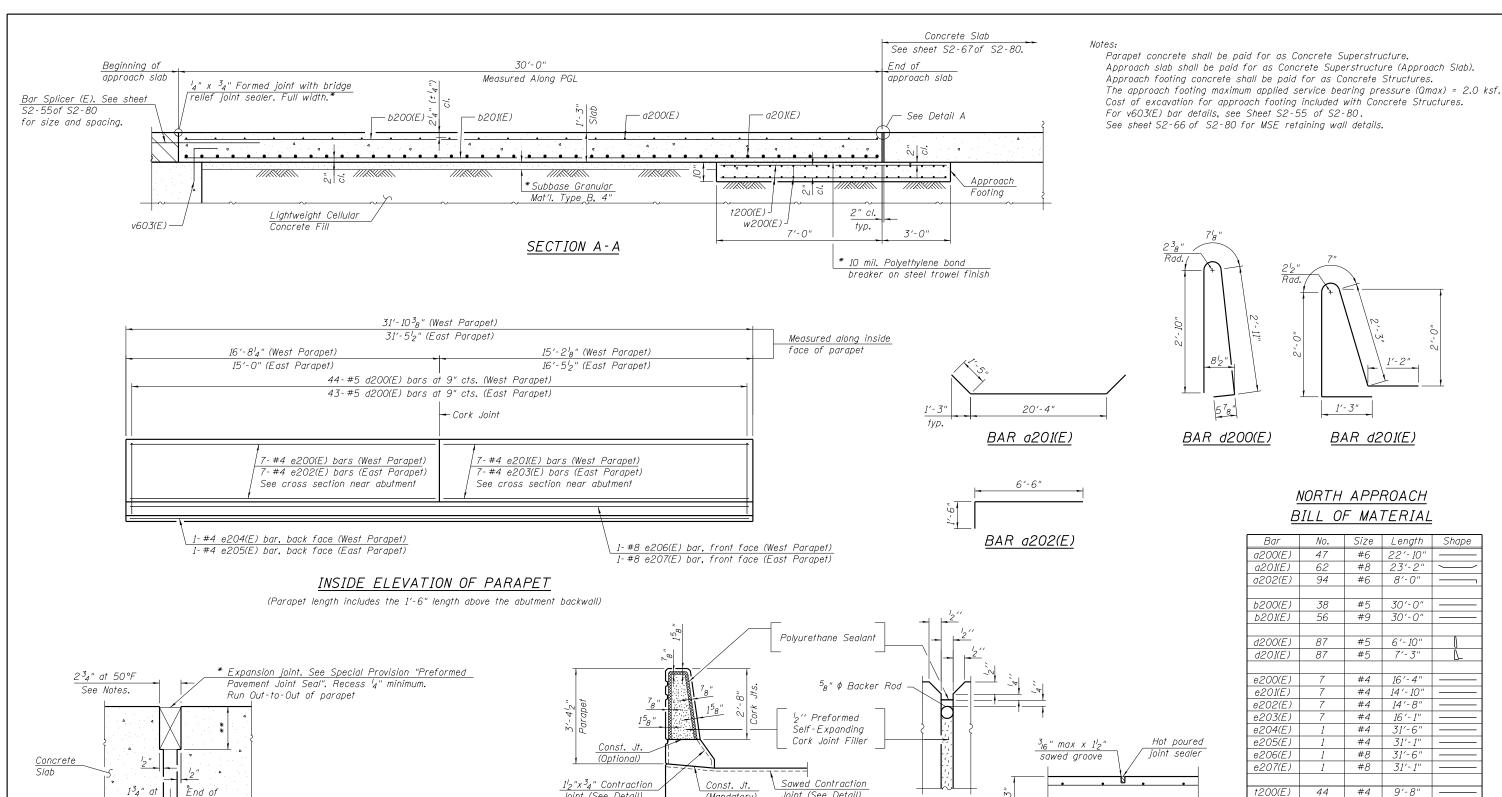
Tran Systems

	user name = wjcolletti	DESIGNED	TJA	REVISED	
		CHECKED	WJC	REVISED	
>	PLOT SCALE = NTS	DRAWN	TJA	REVISED	
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

APPROACH SLAB DETAILS 1 - RAMP					
STRUCTURE NO. 016-1702					
SHEET NO. S2-28 OF S2-80 SHEETS					

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1422	2014-015R&B-R	соок	825	413
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. A	ID PROJECT		



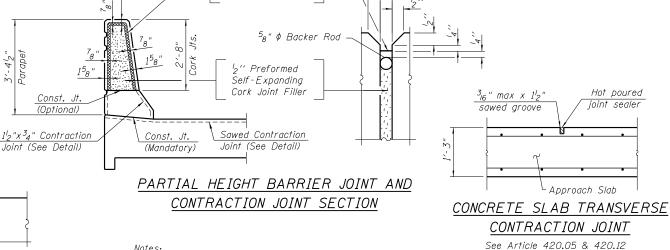
DETAIL A

→ © Joint

Appr. slab

- * Cost included with Concrete Superstructure (Approach Slab).
- ** Per manufacturer recommendations

50°F



PARAPET TRANSVERSE

CONTRACTION JOINT

The Polyurethane Sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.

 $^{l}_{2}$ " PJF is included in the cost of Concrete Superstructure

Bar	No.	Size	Length	Shape
a200(E)	47	#6	22′-10"	
a201(E)	62	#8	23'-2"	
a202(E)	94	#6	8′-0"	
b200(E)	38	#5	30'-0"	
b201(E)	56	#9	30'-0"	
d200(E)	87	#5	6'-10"	n
d201(E)	87	#5	7'-3"	<u></u>
000(5)	7	# 4	107.4"	
e200(E)	7	#4	16'-4"	
e201(E)		#4	14 '- 10"	
e202(E)	7	#4	14'-8"	
e203(E)	7	#4	16 '- 1"	
e204(E)	1	#4	31'-6"	
e205(E)	1	#4	31'-1"	
e206(E)	1	#8	31′-6"	
e207(E)	1	#8	31'-1"	
†200(E)	44	#4	9′-8"	
w200(E)	40	#5	20'-11"	
Concrete			Cu. Yd.	6.6
Concrete		ucture	Cu. Yd.	9.1
Protective	: Coat		Sq. Yd.	103
Concrete (Approact		uctures	Cu. Yd.	32.4
Reinforce Epoxy Co	ment Bar	S,	Pound	16,420
Bridge De (Longitudi	eck Groov	ing	Sq. Yd.	70

Tran Systems

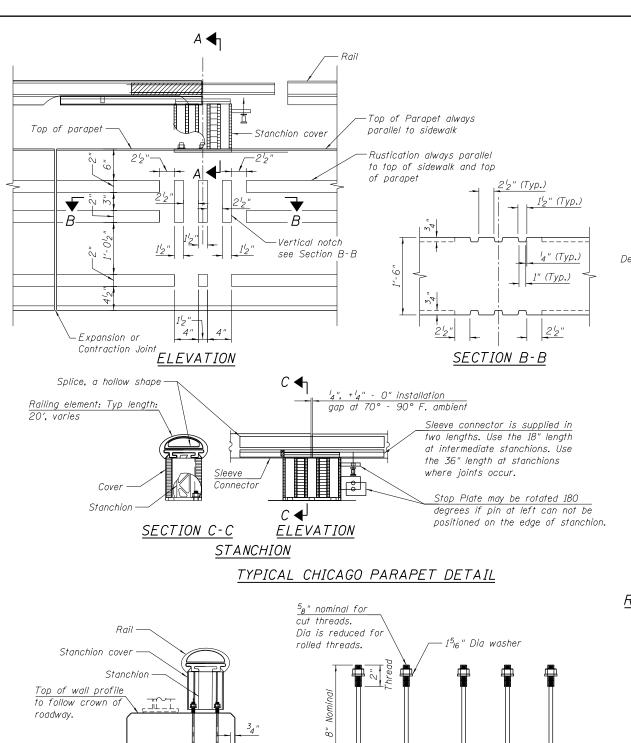
	USER NAME = wjcolletti	DESIGNED	TJA	REVISED
		CHECKED	WJC	REVISED
>	PLOT SCALE = NTS	DRAWN	TJA	REVISED
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED

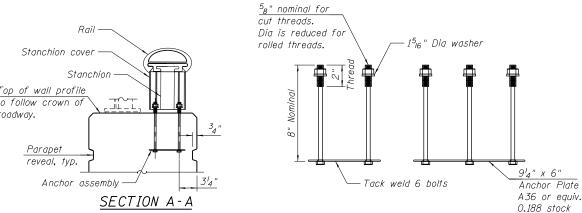
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

	APPROACH SLAB DETAILS 2 - RAMP					
STRUCTURE NO. 016-1702						
	CHEET NO COLOR OF COLOR CHEETS					

of the Standard Specifications

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1422	2014-015R&B-R	COOK	825	414
		CONTRACT	NO. 6	0X94
	ILL INOIS FED. AT	D PROJECT		



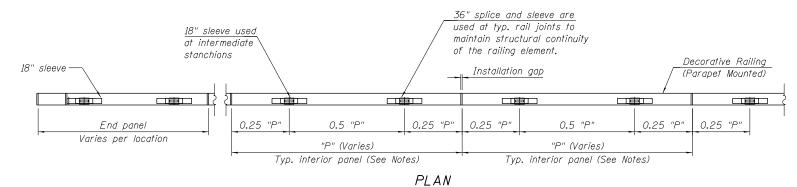


NOTES:

1. All fasteners to meet, or exceed, ASTM A307 Grade C strength requirements.

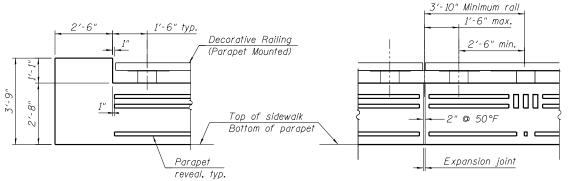
ANCHOR ASSEMBLY

- 2. Galvanize per Article 509.05 of the Standard Specifications after fabrication.
- 3. The size and position of parapet reinforcing must be consistent with capture of the anchor assembly. See Sheet S2-23 of S2-80 for rebar details.

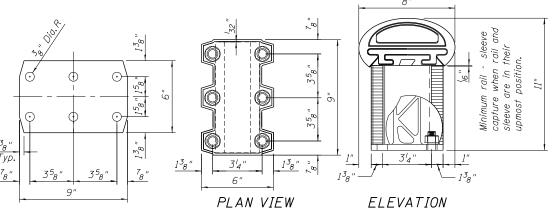


Rail element, typ. (Length = "P") 0.25 "P" 0.25 "P" 0.5 "P" 0.25 "P" 0.25 "P" 0.5 "P" 0.25 "P" Decorative Railing Stanchion Detail 1 spacing 🔠 (Parapet Mounted) - € Joint − © Joint "P" (Varies) "P" (Varies) Fnd panel Varies per location Typ. interior panel (See Notes) Typ. interior panel (See Notes) ELEVATION

RAIL DETAIL - STANCHION LOCATION AND SPACING



RAIL DETAIL 1 - AT END OF PARAPET RAIL DETAIL 2 - AT EXPANSION JOINT



NOTES:

- 1. Elastomeric pad for stanchion made from 1/16" thick stock.
- 2.One required per stanchion.

When walls without rail are adjacent to Chicago wall with rail, their traffic face, or the terminus of their traffic face, must be in the same plane as Chicago wall with rail.

Rustication may vary at terminal ends and is subject to site conditions and site approval. In all other situations, the middle 2.5" x 1.5" rustication is aligned with the center of the stanchion.

Wall details above show that portion of the wall above the gutter break, the substructure is not shown. Note that substructure(s) depth variations could significantly alter the required top of wall

Field cutting of rail elements is acceptable. The cut edge will no longer be anodized. Saw cut only, flame cut not allowed.

End caps shall be used at all rail terminals. Railing system to be produced using extruded aluminum that can be clear anodized.

Alloy selection is based upon the above color requirement and the engineering sufficiency analysis which must be supplied by the Contractor.

Physical appearance to be equal to "Valentine Urban Systems - Chicago wall with rail".

For panel layout, overall stanchion location, spacing and details, see Sheet S2-23 of S2-80.



Decorative Railing (Parapet Mounted)

BILL OF MATERIAL

user name = wjcolletti	DESIGNED	WJC	REVISED
	CHECKED	MDS	REVISED
PLOT SCALE = NTS	DRAWN	JTF	REVISED
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED

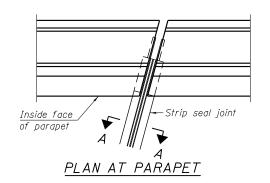
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** **DECORATIVE RAILING, PARAPET MOUNTED** STRUCTURE NO. 016-1702 SHEET NO. S2-30 OF S2-80 SHEETS

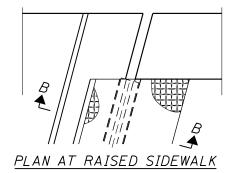
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1422	2014-015R&B-R	COOK	825	415
		CONTRACT	NO. 6	0X94
	ILL INOIS FED AT	D PROJECT		

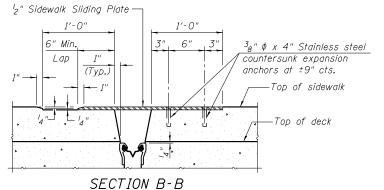
ELASTOMERIC PAD

STANCHION COVER NOTES:

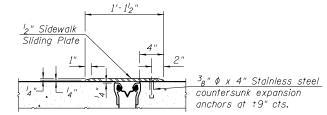
- 1. Cover is shown superimposed over stanchion with anchors in place.
- 2. The stanchion cover is a non-structural element, serving an aesthetic function. It rests on the flange of the stanchion, without fasteners and is captured in place by the rail and stanchion.





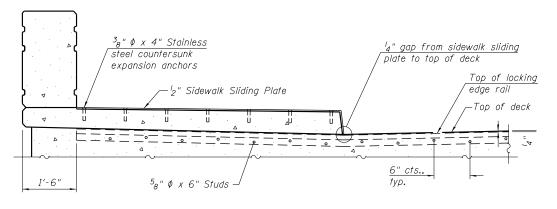


(At northeast, southeast, and southwest corner of bridge)



SECTION THRU SIDEWALK SLIDING PLATE

(At northwest corner of bridge)



Notes:

The strip seal shall be made continuous and shall have a minimum thickness of ${}^{l}_{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

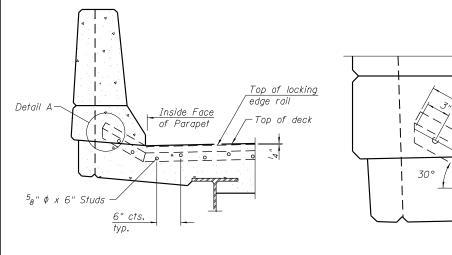
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

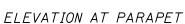
The Maximum space between locking edge rail segments shall be 3 le" and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

The top surface of sidewalk sliding plates shall have a raised pattern according to ASTM A786.

Cost of sidewalk sliding plates and anchorage studs included with Preformed Joint Strip Seal.

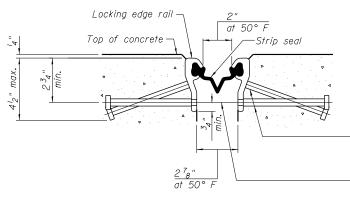
The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



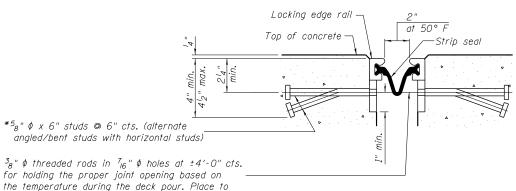


DETAIL A

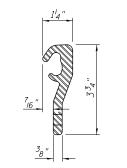
ELEVATION AT RAISED SIDEWALK



SHOWING ROLLED RAIL JOINT



SHOWING WELDED RAIL JOINT

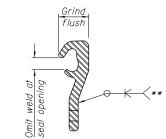


<u>ROLLED</u> EXTRUDED RAIL

WELDED RAIL

LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	156

SECTION A-A

miss studs. All rods shall be burned, or sawed

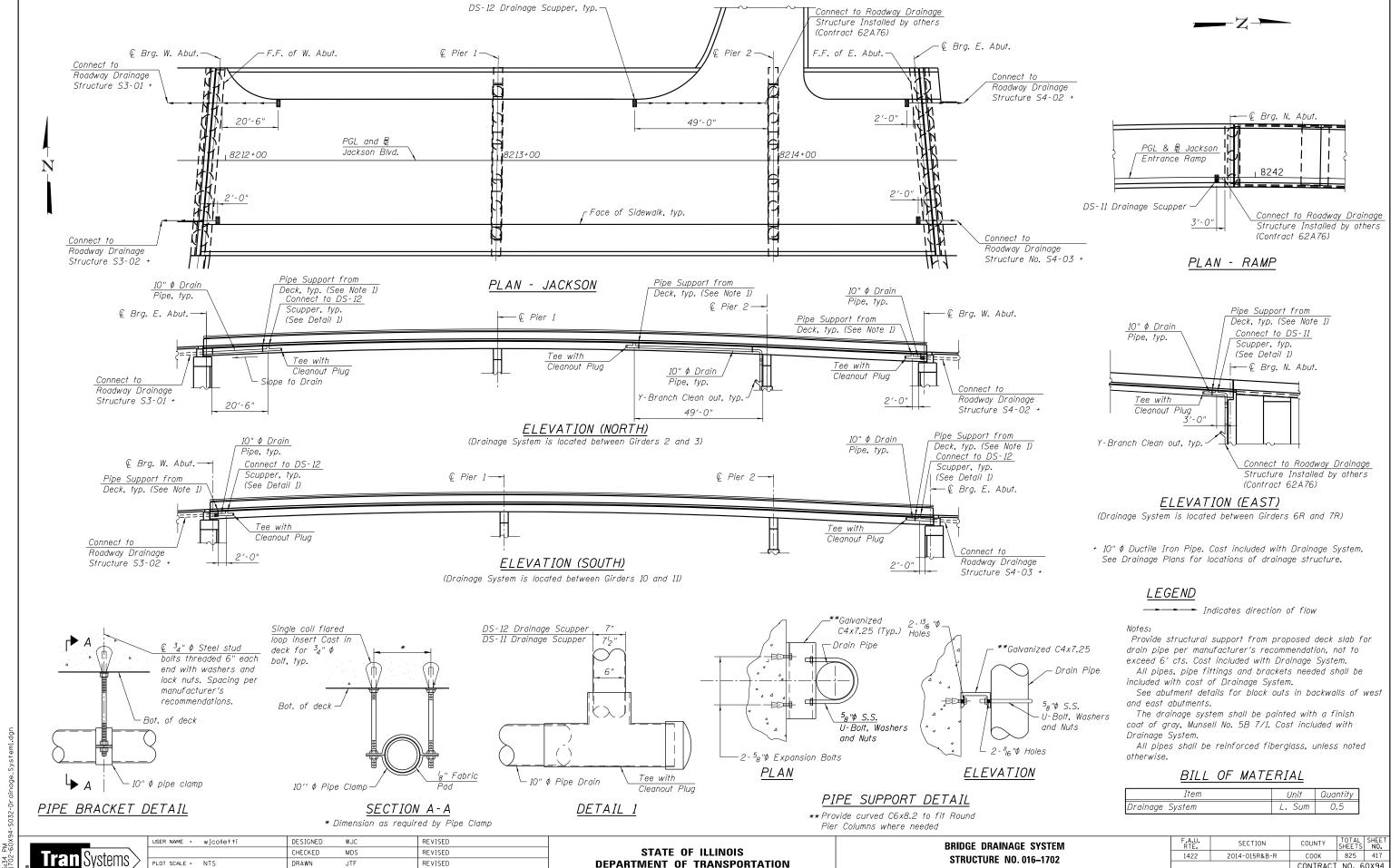
off flush with the plates after concrete is set.

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

USER NAME = wjcolletti	DESIGNED	WJC	REVISED
	CHECKED	TLR	REVISED
PLOT SCALE = NTS	DRAWN	JTF	REVISED
PLOT DATE = 3/5/2020	CHECKED	TLR	REVISED

F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
1422	2014-015R&B-R		COOK	825	416
			CONTRACT	NO. 6	0X94
	ILLINOIS	FED. A	D PROJECT		

Tran Systems



DEPARTMENT OF TRANSPORTATION

STRUCTURE NO. 016-1702

SHEET NO. S2-32 OF S2-80 SHEETS

CONTRACT NO. 60X94

DRAWN

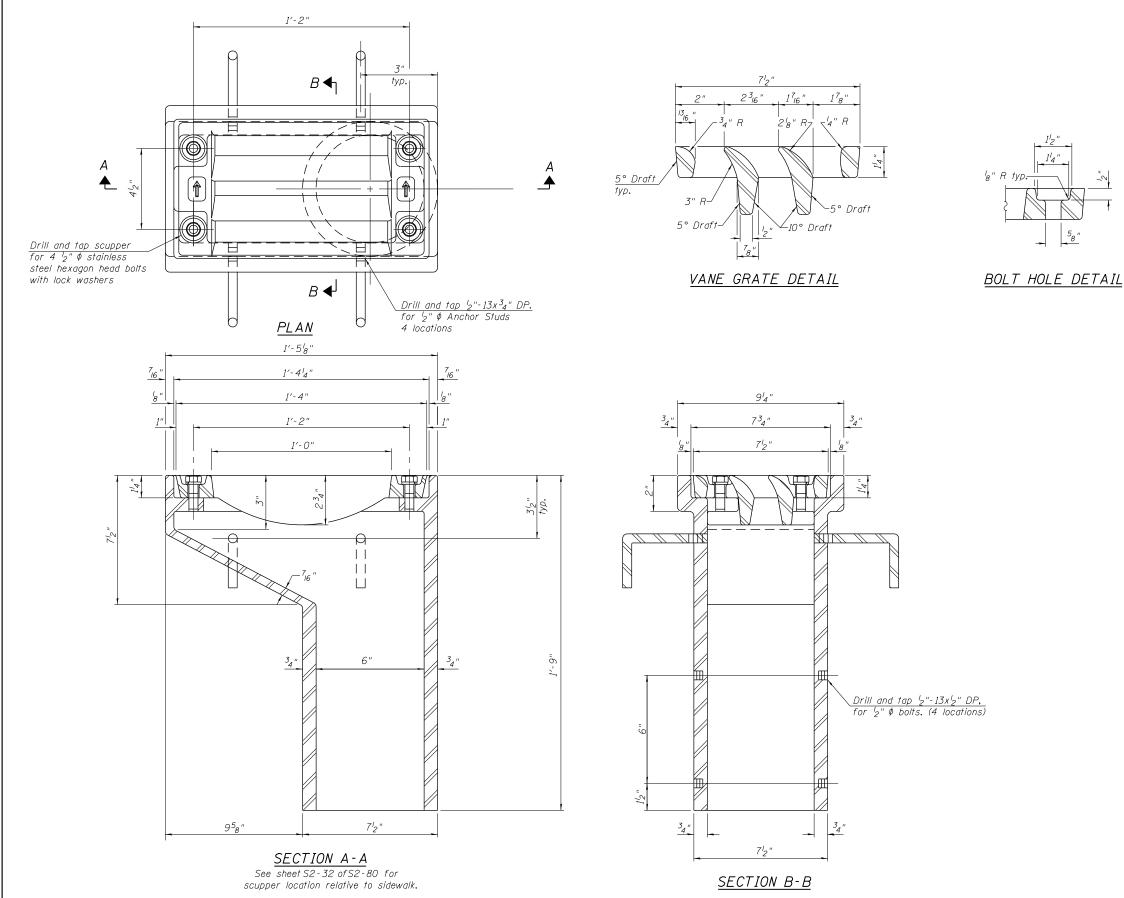
CHECKED

WJC

PLOT DATE = 3/5/2020

REVISED

REVISED





All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.

Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.

Downspouts located on the exterior side of a painted steel fascia beam shall be painted with the finish coat of gray, Munsell No. 5B 7/1. Cost included with Drainage System.

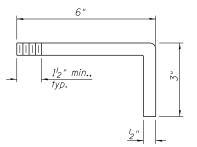
As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.

Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO MIII.

The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.

Cost of the Grate, Frame, Downspout, Anchor Studs, Field Welding, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-11.

Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.



ANCHOR STUD DETAIL

BILL OF MATERIAL

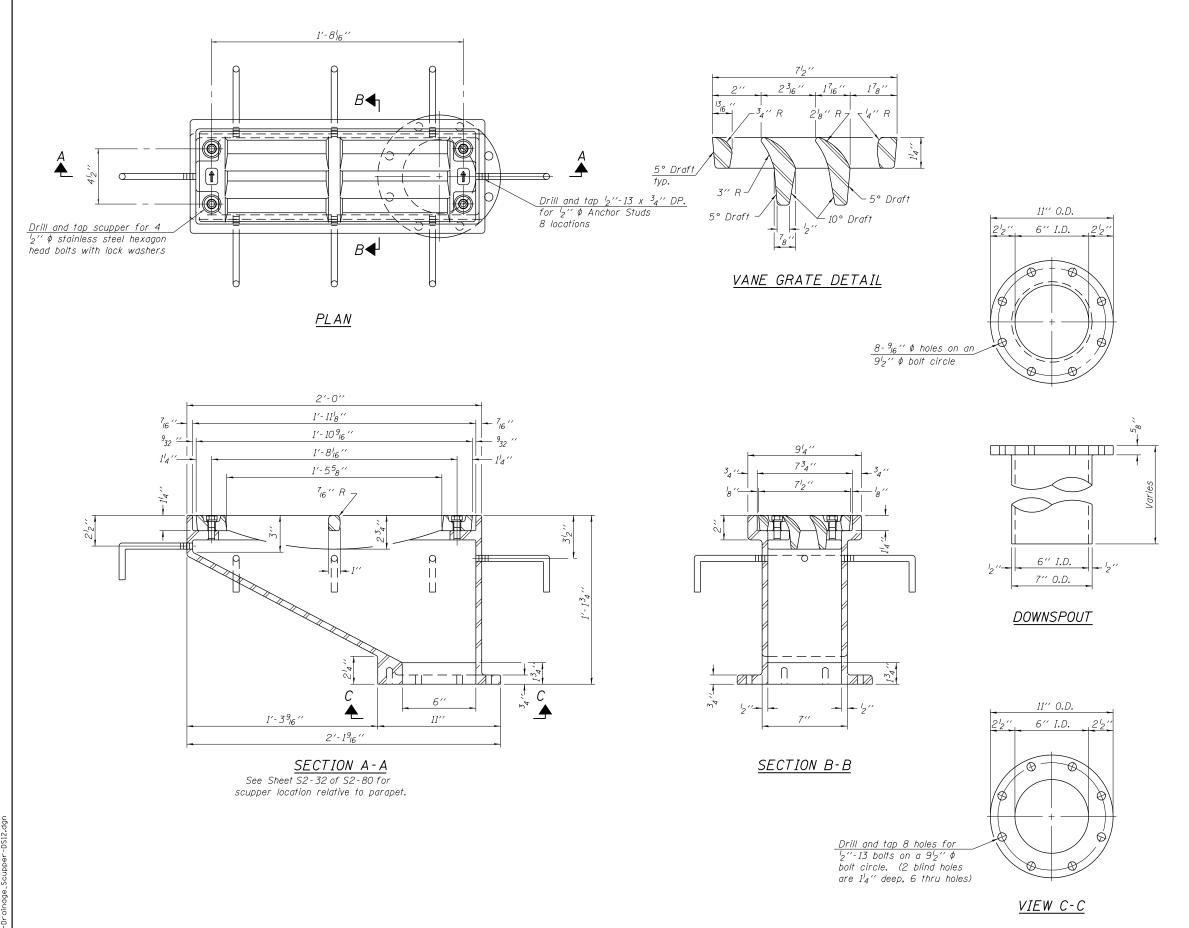
Item	Unit	Quantity
Drainage Scuppers, DS-11	Each	1



USER NAME = wjcolletti	DESIGNED	MJC	REVISED
	CHECKED	MDS	REVISED
PLOT SCALE = NTS	DRAWN	JTF	REVISED
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED

DRAI	NAGE	SCU	PPER,	DS-1	I
STR	UCTUR	E NO	0. 016	-1702	
SHEET	NO S2-	-33 OF	S2-80	SHEETS	

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHE NO
1422	2014-015R&B-R	соок	825	418
		CONTRACT	NO. 6	0X9
	ILLINOIS FED. AI	ID PROJECT		





All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.

Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.

Downspouts located on the exterior side of a painted steel fascia beam shall be painted with the finish coat specified for the exterior side of the fascia beam.

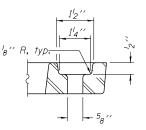
As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.

Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M111.

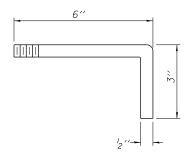
The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.

Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-12.

Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.



BOLT HOLE DETAIL



ANCHOR STUD DETAIL

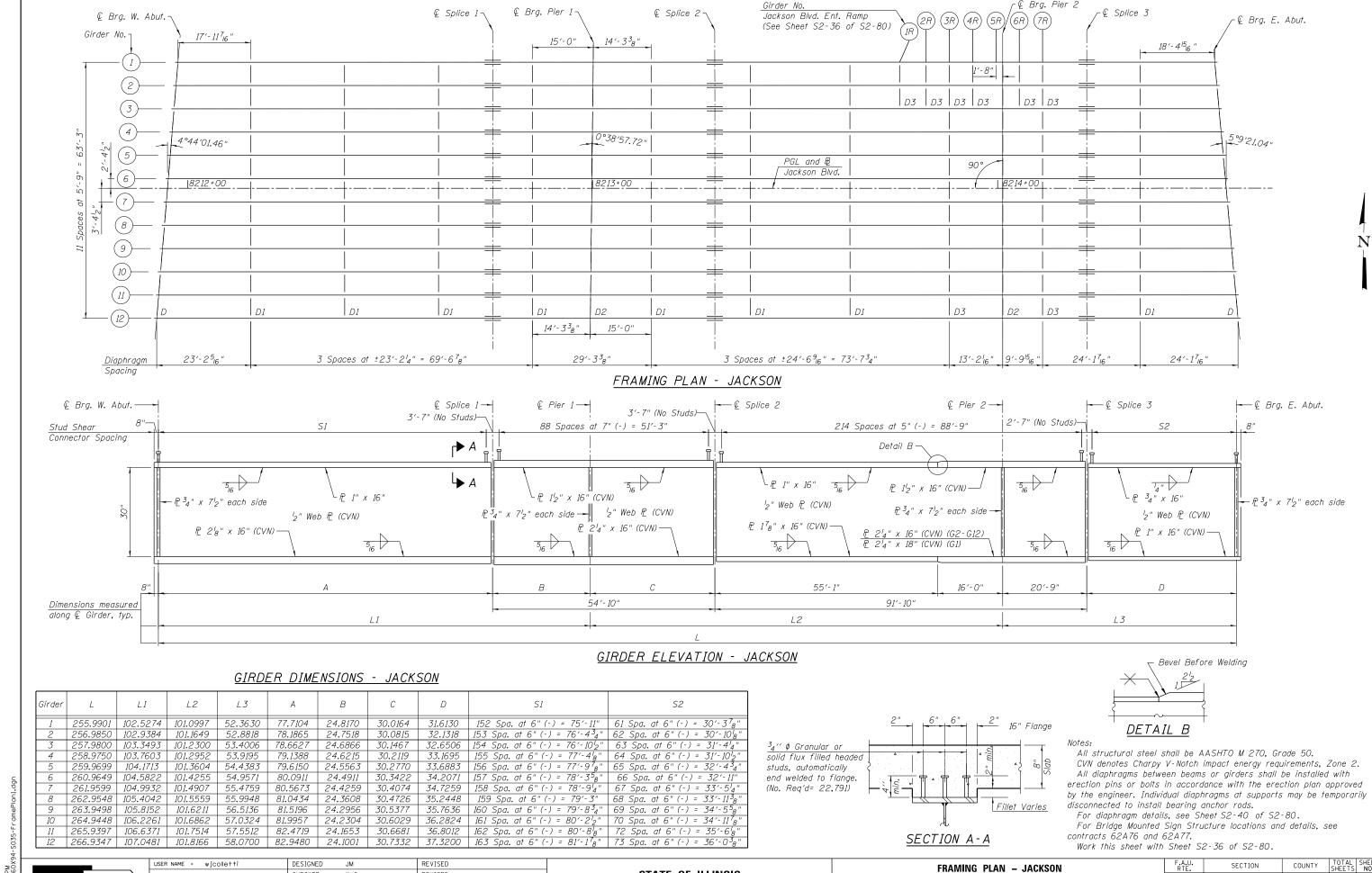
BILL OF MATERIAL

Item	Unit	Quantity
Drainage Scuppers, DS-12	Each	5



USER NAME = wjcolletti	DESIGNED	WJC	REVISED	
	CHECKED	MDS	REVISED	
PLOT SCALE = NTS	DRAWN	JTF	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
1422	2014-015R&B-R	соок	825	419
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. A	ID PROJECT		



STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

2014-015R&B-R

STRUCTURE NO. 016-1702

SHEET NO. S2-35 OF S2-80 SHEETS

COOK

825 420

CONTRACT NO. 60X94

Tran Systems

PLOT DATE = 3/5/2020

CHECKED

CHECKED

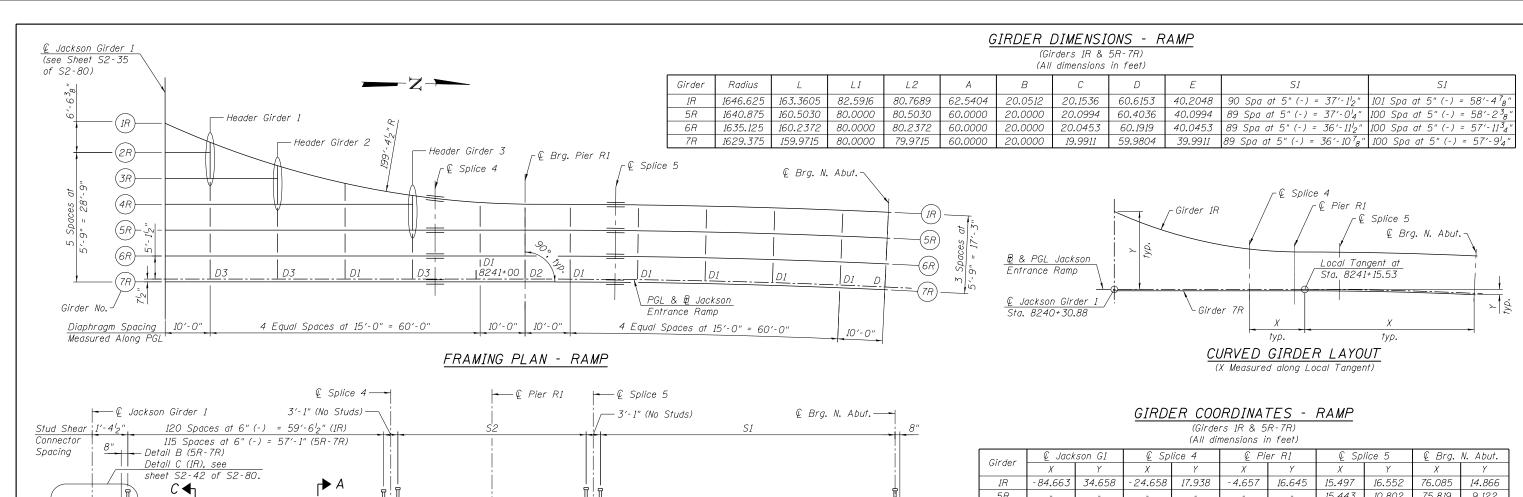
WJC

WJC

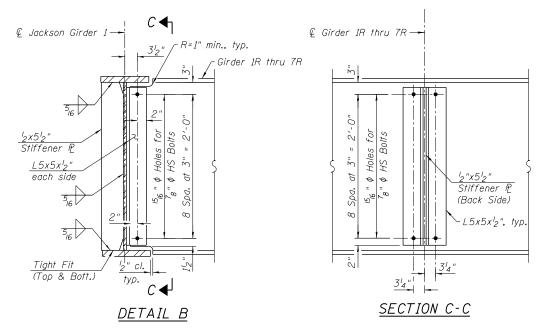
REVISED

REVISED

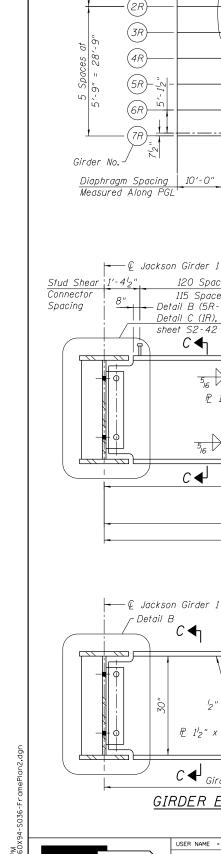
REVISED

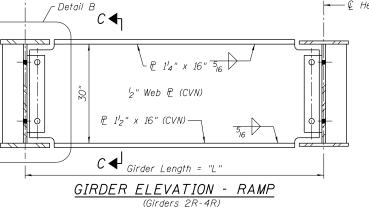


5R 15.443 10.802 75**.**819 9.122 *15.388* 5.053 75.554 3.379 6R 75.288 7R 15.334 -0.697 -2.365



Work this sheet with Sheet S2-35 of S2-80. For diaphragm details, see Sheet S2-40 of S2-80. For Section A-A, see Sheet S2-35 of S2-80.





P 14" x 16"

C◀

5" Web ₱ (CVN)

P2 1/2" x 16" (CVN) -

L1

— © Header Girder Clip 1" Horizontal x 2½" Vertical Top & Bottom GIRDER DIMENSIONS - RAMP Brg. Stiffener (Girders 2R-4R) (All dimensions in feet) Mill Stiffener 16 to bear 11 11 11 11 10.0000 3R 25,0000 **SECTION** SECTION 55,0000 4R AT PIER AT ABUTMENT

-P2 1'4" x 16"

2" Web P (CVN)

-P2 1/2" x 16" (CVN)

P 24" x 16" (CVN)

½" Web | P2 (CVN)

Girder Length = "L"

GIRDER ELEVATION - RAMP (Girders 1R & 5R-7R)

 P_{4}^{3} " x 7_{2}^{l} " each side -

5/6

BEARING STIFFENER DETAILS

 P_{4}^{3} " x 7_{2}^{l} " each side -

P 1/2" x 16" (CVN) -

* Use '4" Weld for E. Abut.

	USER NAME = wjcolletti	DESIGNED JM	REVISED
Van Customs		CHECKED WJC	REVISED
ran Systems > 1	PLOT SCALE = NTS	DRAWN JTF	REVISED
	PLOT DATE = 3/5/2020	CHECKED WJC	REVISED

 $\downarrow A$

STATE OI	F ILLINOIS
DEPARTMENT OF	TRANSPORTATION

FRAMING PLAN - RAMP STRUCTURE NO. 016-1702 F.A.U. RECTION COUNTY SHEETS NO. 1422 2014-015R&B-R COOK 825 421						
STRUCTURE NU. 016-1/02		F.A.U. RTE.	SECTION	COUNTY		
STHOUTONE NO. 010-1702	STRUCTURE NO 016_1702	1422	2014-015R&B-R	COOK	825	421
CONTRACT NO. 60X94	31110C1011L 140.010-1702			CONTRACT	NO. 6	0X94
SHEET NO. S2-36 OF S2-80 SHEETS ILLINOIS FED. AID PROJECT	SHEET NO. S2-36 OF S2-80 SHEETS		ILLINOIS FED. AI	D PROJECT		

INTERIOR	GIRDE	R 8 MOMENT	TABLE	JACKSON (HL	-93 LOADIN	G)
		0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.7 Sp. 3
$I_{\mathcal{S}}$	(in ⁴)	12,388	<i>15,912</i>	11,731	<i>15,912</i>	7,714
$I_c(n)$	(in ⁴)	35,267	-	32,471	-	21,375
$I_c(3n)$	(in4)	24,211	-	22,618	-	15,541
$I_c(cr)$	(in4)	-	19,179	-	<i>19,179</i>	-
Ss	(in ³)	988	829	895	829	5 <i>31</i>
Sc(n)	(in ³)	1,347	-	1,216	-	749
Sc(3n)	(in ³)	1,233	-	1,115	-	686
Sc(cr)	(in ³)	-	1,057	-	<i>1</i> ,057	-
DC1	(k/')	0.85	0.88	0.84	0.88	0.77
M DC1	('k)	697	1,094	285	517	108
DC2 **	(k/')	0.32	0.32	0.32	0.32	0.32
M DC2	('k)	267	370	112	188	49
DW	(k/')	0.19	0.19	0.19	0.19	0.19
Mow	('k)	161	226	72	116	28
LLDF		0.32	0.35	0.36	0.42	0.34
M4 + IM	('k)	746	829	620	711	368
Mu (Strength I)	('k)	2752	3620	1689	2298	884
$\phi_f M_D$	('k)	5799	-	5 <i>31</i> 6	-	3582
fs DC1	(ksi)	8.46	<i>15.85</i>	3.82	7.48	2.45
f _s DC2	(ksi)	2.60	4.20	1.20	2.13	0.86
f _s DW	(ksi)	1.57	2.57	0.77	1.31	0.49
fs (4+IM)	(ksi)	6.65	9.42	6.12	8. 07	5.90
fs (Service II)	(ksi)	21.27	<i>34.85</i>	13.75	21.42	11.48
0.95R _h F _{yf}	(ksi)	47.50	47.50	47.50	47.50	47.50
fs (Total)(Strength I)	(ksi)	-	45.38	-	28.11	-
$\phi_f F_n$	(ksi)	-	50.00	-	50.00	-
Vf	(k)	<i>38.33</i>	-	24.37	-	21.66

INT. GIA	RDER 8	REACTION 7	TABLE - JA	CKSON (HL-9	3 LOADING)	
		W. Abut.	Pier 1	Pier 2	E. Abut.	
LLDF		0.43	0.48	0.55	0.46	1
0CF		1.02	-	-	1.02	**;
RDCI	(k)	34.7	105.5	71.2	13.2	1
R _{DC2}	(k)	12.1	35.9	24.8	5.1	1
Row	(k)	7.5	22.0	<i>15.2</i>	3.1	1
R4 + IM	(k)	48.8	99.8	90.2	40.9	
R Total	(k)	103.1	263.2	201.5	62.3]

EXTERIOR	GIRDE	R 1 MOMENT	TABLE	JACKSON (HL	-93 LOADIN	G)
		0.4 Sp. 1	Pier 1	0.7 Sp. 2	Pier 2	0.7 Sp. 3
$I_{\mathcal{S}}$	(in ⁴)	12,388	15,912	11,731	<i>16,679</i>	7,714
$I_c(n)$	(in ⁴)	35,267	-	32,471	-	21,375
$I_c(3n)$	(in4)	24,211	-	22,618	-	15,541
$I_c(cr)$	(in4)	-	19,179	-	20,170	-
Ss	(in ³)	988	829	895	836	5 <i>31</i>
Sc(n)	(in ³)	1,347	-	1,216	-	749
Sc(3n)	(in ³)	1,233	-	1,115	-	686
Sc(cr)	(in ³)	-	1,057	-	1,065	-
Sxc	(in ³)	717	1175	708	1299	563
DC1	(k/')	0.85	0.88	0.84	0.90	0.77
M DC1	('k)	630	1,114	411	965	35
DC2 **	(k/')	0.32	0.32	0.32	0.32	0.32
M DC2	('k)	254	364	122	280	38
DW	(k/')	0.19	0.19	0.19	0.19	0.19
Mow	('k)	148	231	104	266	9
LLDF		0.30	0.32	0.47	0.45	0.22
M4 + IM	('k)	685	718	549	777	195
f! (Strength I)	(ksi)	0.48	0.11	1.38	<i>8.</i> 56	0.70
Mu + 1/3 fi Sxc	('k)	2534	3454	1809	3624	456
$\phi_f M_D$	('k)	5799	-	53 <i>1</i> 6	-	3582
fs DC1	(ksi)	7.66	16.14	5 . 51	13.86	0.79
f _s DC2	(ksi)	2.47	4.13	1.31	<i>3.1</i> 6	0.67
fs DW	(ksi)	1.44	2.62	1.11	3.00	0 . 15
fs (4+IM)	(ksi)	6.10	8. 15	5.42	8.76	3.12
fı (Service II)	(ksi)	0.04	0.04	0.14	3.37	0.16
$f_s + f_{2}$ (Service II)	(ksi)	19.51	33.50	15.05	33.09	5.75
0.95R _h F _{yf}	(ksi)	47.50	47,50	47.50	47,50	47,50
$f_s + f_{1/3}$ (Total)(Strength I)	(ksi)	-	43.57	-	43.95	-
$\phi_f F_n$	(ksi)	-	50.00	-	50.00	-
Vf	(k)	12.07	11.07	6.49	6.12	6.19

EXT. GIRL	DER 1	REACTION	TABLE - JA	CKSON (HL-9	3 LOADING)	
		W. Abut.	Pier 1	Pier 2	E. Abut.	
LLDF		0.28	0.35	0.83	0.18	
0CF		1.02	-	-	1.02	*
R DC1	(k)	32.7	106.6	198.5	8.5	
RDC2	(k)	12.2	35.9	65.8	4.1	
Row	(k)	7.4	22.3	70.1	2.0	
R4 + IM	(k)	30.5	72.0	149.4	15.6	
RTotal	(k)	82.9	236.9	483.7	30.2	

- * Points shown indicate location of maximum moment within each beam segment
- ** Load allowance includes 0.025 k/' for duct banks.
- *** Obtuse Correction Factor is included with Live Load Distribution Factors shown in Table

- Is. Ss: Non-composite moment of inertia and section modulus of the steel section used for computing fs (Total-Strength I, and Service II) due to non-composite dead loads (in.4 and in.3).
- $I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing $f_s(Total-Strength\ I,\ and\ Service\ II)$ in uncracked sections due to short-term composite live loads (in.⁴ and in.³).
- $I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing $f_s(Total-Strength\ I$, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.4 and in.3).
- Ic(cr), Sc(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in. 4 and in. 3).
 - Sxc: Section modulus about the major axis of section to the controlling flange, tension or compression, taken as yield moment with respect to the controlling flange over the yield strength of the controlling flange (in.³).
 - DC1: Un-factored non-composite dead load (kips/ft.).
 - Mpci: Un-factored moment due to non-composite dead load (kip-ft.).
 - DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 - MDc2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 - DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 - Mow: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 - M½ IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- M_U (Strength 1): Factored design moment (kip-ft.). 1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M \pm + IM
 - $\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).
 - fs DCI: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

 MDCI / Snc
 - fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
 - $M_{DC2}/S_c(3n)$ or $M_{DC2}/S_c(cr)$ as applicable.
 - fs DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

 Mpw / Sc(3n) or Mpw / Sc(cr) as applicable.
 - f_s (4+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
 - $M_{4} + IM / S_{c}(n)$ or $M_{DW} / S_{c}(cr)$ as applicable.
- f_s (Service II): Sum of stresses as computed below (ksi). f_{sDCI} + f_{sDC2} + f_{sDW} + 1.3 f_{s} (ξ + im)
- $0.95R_hF_yf$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
- f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi). $1.25~(f_{sDCl}+~f_{sDC2})~+~1.5~f_{sDW}+~1.75~f_{s}~(\cline{t}_{s}~+~IM)$
 - ₱rFn: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
 - V_f : Maximum factored shear range in span computed according to Article 6.10.10.
 - Vf: Maximum factored shear range in span computed according to Article 6.10.10.
 - LLDF : Live Load Distribution Factor
 - OCF : Obtuse Correction Factor



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		CHECKED	WJC	REVISED
>	PLOT SCALE = NTS	DRAWN	JTF	REVISED
	PLOT DATE = 3/12/2020	CHECKED	WJC	REVISED

EXTERIOR GI	RDER			
		0.4 Sp. 1	Pier R1	0.6 Sp. 2
$I_{\mathcal{S}}$	(in ⁴)	11,896	15,912	11,896
$I_c(n)$	(in4)	28,635	-	28,635
$I_c(3n)$	(in4)	20,775	-	20,775
$I_c(cr)$	(in4)	-	19,223	-
Ss	(in ³)	773	1,094	773
Sc(n)	(in³)	1,026	-	1,026
Sc(3n)	(in³)	940	-	940
Sc(cr)	(in³)	-	1,174	-
Sxc	(in³)	64	96	64
DC1	(k/')	0.83	0.89	0.83
M DC1	('k)	363	863	338
DC2 *	(k/')	0.24	0.29	0.29
M DC2	('k)	90	295	146
DW	(k/')	0.26	0.25	0.25
Mow	('k)	99	237	106
LLDF		0.39	0.62	0.47
M4 + IM	('k)	588	1,034	827
fe (Strength I)	(ksi)	13.95	20.77	3.12
Mu + 1/3 ft Sxc	('k)	1768	3668	2217
$\phi_f M_D$	('k)	4739	-	4739
fs DC1	(ksi)	5.64	9.47	5.25
f _s DC2	(ksi)	1.14	3.01	1.87
f _s DW	(ksi)	1.26	2.42	1.35
f_s (4+IM)	(ksi)	6.88	10.58	9.67
fı (Service II)	(ksi)	4.67	7.87	0.73
$f_s + f'/_2$ (Service II)	(ksi)	19.32	32.59	21.40
0.95R _h F _{yf}	(ksi)	47.50	47.50	47.50
$f_s + f_{1/3}$	(ksi)	-	44.67	-
(Total)(Strength I)	(KS1)			
$\phi_f F_n$	(ksi)	-	46.65	-
Vf	(k)	29.85	40.66	30.75

EXTERIOR GIRDER 1R					
REACTION TABLE - RAMP					
	Pier R1	N. Abut			
LLDF	0.70	0.61			
0CF	-	1.00			
RDCI (k)	103.4	24.7			
R_{DC2} (k)	44.6	12.5			
Row (k)	29.5	7.2			
RL+IM (k)	123.1	67.5			
RTotal (k)	300.7	111.8			

INT. GIRDER 4R MON	1ENT T	ABLE - RAMP
		0.6 Span 1
$I_{\mathcal{S}}$	(in ⁴)	11,896
Is Ss	(in ³)	773
DC1	(k/')	0.83
M DC1	('k)	415
DC2	(k/')	0.24
M DC2	('k)	73
DW	(k/')	0.25
Mow	('k)	115
LLDF		0.37
M4 + IM	('k)	434
Mu (Strength I)	('k)	1543
$\phi_f M_D$	('k)	2066
fs DC1	(ksi)	6.45
f _s DC2	(ksi)	1.14
f _s DW	(ksi)	1.79
f _s (4+IM)	(ksi)	6.74
fs (Service II)	(ksi)	18.13
0.95R _h Fyf	(ksi)	47.50
fs (Total)(Strength I)	(ksi)	-
$\phi_f F_n$	(ksi)	-
Vf	(k)	22.61

INTERIOR GIA	RDER			AMP
		0.4 Sp. 1	Pier R1	0.6 Sp. 2
I_s	(in ⁴)	11,896	15,912	11,896
$I_c(n)$	(in4)	28,527	-	28,527
$I_c(3n)$	(in4)	20,685	-	20,685
Ic(cr)	(in4)	-	19,179	-
Ss	(in ³)	773	829	773
Sc(n)	(in³)	1,025	-	1,025
Sc(3n)	(in³)	939	-	939
Sc(cr)	(in³)	-	1,103	-
Sxc	(in³)	64	64	64
DC1	(k/')	0.83	0.88	0.83
M DC1	('k)	398	827	344
DC2*	(k/')	0.24	0.29	0.29
M DC2	('k)	71	204	120
DW	(k/')	0.27	0.25	0.25
Mow	('k)	106	243	108
LLDF		0.20	0.37	0.35
M& + IM	('k)	324	681	564
f, (Strength I)	(ksi)	0.61	1.10	2.43
Mu + 1/3 fi Sxc	('k)	1313	2846	1733
$\phi_f M_D$	('k)	4724	-	4724
f _s DC1	(ksi)	6.18	11.97	5.34
f _s DC2	(ksi)	0.90	2.22	1.53
f _s DW	(ksi)	1.35	2.64	1.37
fs (4+IM)	(ksi)	3.80	7.41	6.60
f, (Service II)	(ksi)	0.03	0.32	0.56
$f_s + f_{2}$ (Service II)	(ksi)	13.39	26.63	17.12
0.95R _h F _{yf}	(ksi)	47.50	47.50	47.50
$f_s + f_{3}$	(40:)	-	35.04	-
(Total)(Strength I)	(ksi)			
$\phi_f F_n$	(ksi)	-	50.00	-
Vf	(k)	27.72	33.11	26.64
			_	

INTERIOR GIRDER 5R						
REACTION TABLE - RAMP						
		Pier R1	N. Abut			
LLDF		0.43	0.40			
0CF		-	1.00			
Roci	(k)	94.5	24.6			
R DC2	(k)	17.5	5 . 4			
Row	(k)	29.6	7.7			
R4 + IM	(k)	75.1	40.5			
RTotal	(k)	216.7	78 . 2			

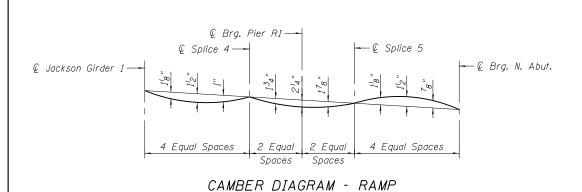
EXTERIOR GI	RDER	7R MOMENT		
		0.4 Sp. 1	Pier R1	0.6 Sp. 2
I_s	(in ⁴)	11,896	15,912	11,896
$I_c(n)$	(in ⁴)	28,635	-	28,635
$I_c(3n)$	(in4)	20,775	-	20,775
Ic(cr)	(in4)	-	19,223	-
Ss	(in ³)	773	829	773
Sc(n)	(in ³)	1,026	-	1,026
Sc(3n)	(in ³)	940	-	940
Sc(cr)	(in ³)	-	1,107	-
Sxc	(in ³)	64	64	64
DC1	(k/')	0.83	0.89	0.83
M DC1	('k)	316	688	352
DC2 *	(k/')	0.24	0.29	0.29
M DC2	('k)	89	255	144
DW	(k/')	0.27	0.25	0.25
Mow	('k)	74	182	109
LLDF		0.37	0.53	0.51
M4 + IM	('k)	591	972	812
f! (Strength I)	(ksi)	0.57	2.08	2.26
Mu + 1/3 ft Sxc	('k)	<i>1</i> 652	3156	2210
$\phi_f M_D$	('k)	4739	-	4739
fs DC1	(ksi)	4.91	9.96	5.47
f _s DC2	(ksi)	1.13	2.76	1.84
f _s DW	(ksi)	0.94	1.97	1.40
fs (4+IM)	(ksi)	6.91	10.54	9.50
fı (Service II)	(ksi)	0.02	0.93	0.61
$f_s + f_{2}$ (Service II)	(ksi)	15.98	28.87	21.35
0.95R _h F _y f	(ksi)	47.50	47.50	47.50
$f_s + \frac{f_l}{3}$ (Total)(Strength I)	(ksi)	-	38.01	-
$\phi_f F_n$	(ksi)	-	50.00	-
Vf	(k)	31.78	37.32	32.19

EXTERIOR GIRDER 7R						
REACTION TABLE - RAMP						
Pier R1 N. Abut						
LLDF	0.63	0.65				
0CF	-	1.00				
RDC1 (k	82.5	24.1				
RDC2 (k	37.6	12.2				
Row (k	19.7	7.1				
R4 + IM (K	111.0	66.3				
R Total (k	250.8	109.6				

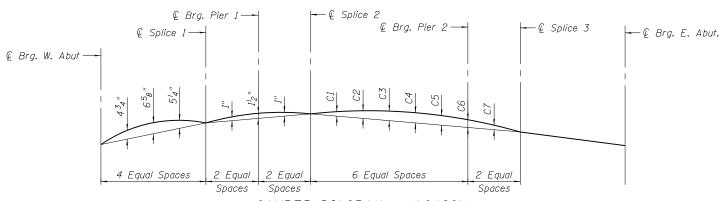
* Points shown indicate location of maximum moment within each beam segment.



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PLOT SCALE = NTS	DRAWN	JTF	REVISED
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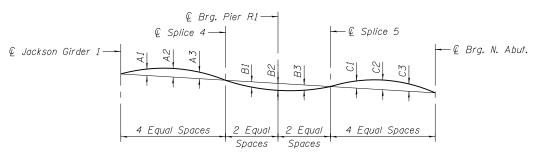
(Girder 1R)



CAMBER DIAGRAM - JACKSON

CAMBER TABLE - JACKSON

Girder	C1	C2	C3	C4	C5	C6	C7
,	75 "	47 "	45 "	0.3 "	1.3 11	111	.3 //
1	2-8	4 8	4-8	2-4	1-4	I	-8
2	31/2"	4 ³ 4"	412"	2 ⁵ 8"	134"	1"	38"
3	3 ³ 8"	4 ⁵ 8"	41/4"	21/2"	134"	1"	38"
4-12	33 ₈ "	4 ³ 8"	41/8"	2 ³ 8"	1 ⁵ 8"	78"	38"



CAMBER DIAGRAM - RAMP (Girders 5R, 6R & 7R)

CAMBER TABLE - RAMP

(Girders 5R, 6R & 7R)

Girde.	r A1	A2	A3	B1	B2	B3	C1	C2	C3
5R	34"	11/8"	⁷ 8"	17 ₈ "	21/2"	178"	18"	1/2"	⁷ 8"
6R	3 ₄ "	18"	⁷ 8"	17 ₈ "	21/2"	1 ⁷ 8"	18"	1 ¹ 2"	⁷ 8"
7R	34"	18"	78"	178"	21/2"	1 ⁷ 8"	1 ¹ 8"	1/2"	78"

TOP OF WEB ELEVATIONS - RAMP

(Girders 1R, 5R, 6R & 7R)

♀ Pier

592.15

592.00 7R 593.44 592.49 591.85 590.80 586.27

Girder & Jackson & Splice G1 4

1R 595.17 593.00

5R 593.98 592.80 6R 593.70 592.64

├- @ Header 4 Equal Spaces

CAMBER DIAGRAM - RAMP

(Girders 2R, 3R & 4R)

CAMBER TABLE - RAMP

(Girders 2R, 3R & 4R)

Girder	A1	A2	A 3
2R	0"	0"	0"
3R	0"	0"	0"
4R	58"	34"	58"

TOP OF WEB ELEVATIONS - RAMP

(Girders 2R, 3R & 4R)

Girder	€ Jackson G1	⊈ Header Beam
- 0.0	504.04	E04 E0
2R	594.84	<i>594.59</i>
3R	594.55	593.99
4R	594.26	593.09

TOP OF WEB ELEVATIONS - JACKSON

(For fabrication use only)

that rapheation and only							
Girder	© Brg. W. Abut.	€ Splice 1	ℚ Brg. Pier 1	€ Splice 2	€ Brg. Pier 2	© Splice 3	€ Brg. E. Abut.
1	595.77	597.37	597,20	596.72	593.89	592.95	592.13
2	595,84	597.47	597.29	596.79	593.98	593.05	592.20
3	595.91	597.56	597.38	596.87	594.07	593 . 14	592,28
4	595.98	597.66	597.47	596.96	594 . 15	593.23	592.36
5	596.05	597.75	597.56	597.04	594.24	593.33	592.44
6	596.12	597.84	597.65	597.13	594.33	593.42	592.52
7	596.09	597.83	597.64	597.12	594.31	593.40	592.49
8	595.98	597.74	597.55	597.03	594.22	593.31	592.39
9	595.87	597.65	597.46	596.93	594.14	593.22	592.29
10	595.76	597.56	597.37	596.84	594.05	593.13	592.19
11	595.69	597.52	597.33	596.80	594.00	593.09	592.14
12	595.67	597.52	597.33	596.80	594.00	593.09	592.13

Tran Systems

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	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JTF	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	
				•

591.09 590.94

592.31 591.23

€ N. Abut

586.72

586.57

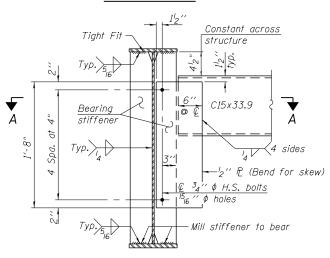
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

STRUCTURAL STEEL DETAILS 3		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-1702	1422	2014-015R&B-R	COOK	825	424
STRUCTURE NO. 010-1702			CONTRACT	NO. 6	0X94
SHEET NO. S2-39 OF S2-80 SHEETS		ILLINOIS FED. AI	D PROJECT		

ALONG JACKSON

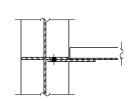
ALONG RAMP

SECTION A-A

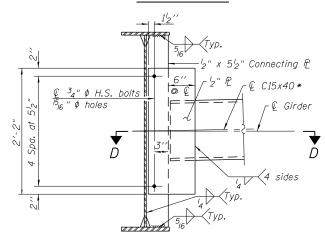


END DIAPHRAGM D

(25 Required)

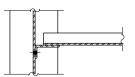


SECTION D-D

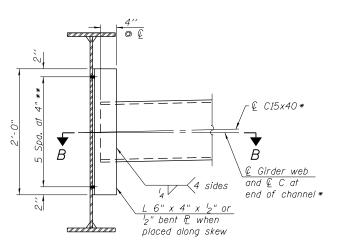


INTERIOR DIAPHRAGM D3

(39 Required)

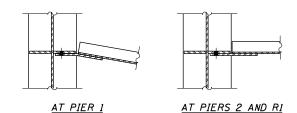


SECTION B-B

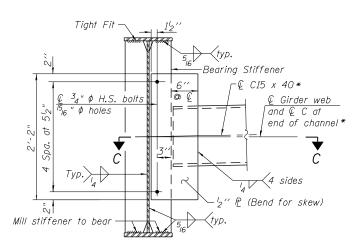


INTERIOR DIAPHRAGM D1

(110 Required)



SECTION C-C



INTERIOR DIAPHRAGM D2

(25 Required)

Not

All structural steel shall be AASHTO M 270, Grade 50. Two hardened washers required for each set of oversized holes.

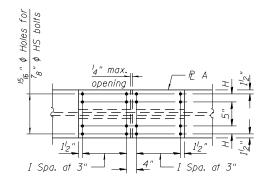
* Alternate channels C15x50 are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.

** 3/4" \$\phi\$ HS bolts, \$\frac{15}{16}\$" \$\phi\$ holes

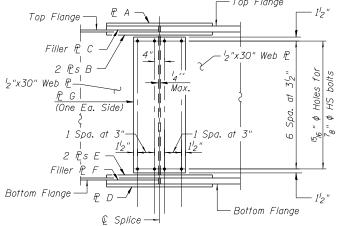


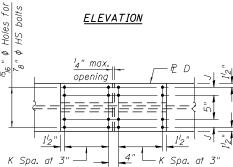
USER NAME = wjcolletti	DESIGNED	JM	REVISED
	CHECKED	WJC	REVISED
PLOT SCALE = NTS	DRAWN	JTF	REVISED
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED

A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
422	2014-015R&B-R	COOK	825	425
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. AI	D PROJECT		



PLAN - TOP FLANGE





PLAN - BOTTOM FLANGE

FIELD SPLICE DETAIL

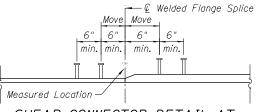
(44 Required)

© Field Splice Move Measured Move Location 3" 6" min. min. min.

SHEAR CONNECTOR DETAIL AT SPLICE AND FLANGE TRANSITIONS

DO NOT place shear connectors on splice plates.

Move row of studs to 3" beyond nearest edge of splice plate from measured location.



SHEAR CONNECTOR DETAIL AT WELDED FLANGE SPLICE TRANSITIONS

Do not place shear connectors on welded splice.

Move row of studs to 6" beyond nearest edge of flange transition from measured location.

TABLE OF FIELD SPLICE DATA

Splice	P _C A	PB	Filler ₽ C	PC D	PE E	Filler & F	PC G	Н	I	J	Κ
1	⁵ 8" x 1'-4" x 3'-1"	³ ₄ " x 7" x 3'-1"	¹ 2" x 1'-4" x 1'-6 ³ 8"	1'4" x 1'-4" x 4'-7"	1 ³ 8" x 7" x 4'-7"	1 ₈ " x 1'-4" x 2'-3 ³ ₈ "	¹ 2" x 2'-0" x 1'-1"	4"	5	4"	8
2	⁵ 8" x 1'-4" x 3'-1"	³ 4" x 7" x 3'-1"	¹ ₂ " x 1'-4" x 1'-6 ³ ₈ "	1'8" x 1'-4" x 4'-1"	1 ¹ 4" x 7" x 4'-1"	3 ₈ " x 1'-4" x 2'-03 ₈ "	¹ 2" x 2'-0" x 1'-1"	4"	5	4"	7
3	¹ 2" x 1'-4" x 2'-1"	⁵ 8" x 7" x 2'-1"	3 ₄ " x 1'-4" x 1'-03 ₈ "	⁵ 8" x 1'-4" x 3'-1"	3 ₄ " x 7" x 3'-1"	14" x 1'-4" x 1'-6 ³ 8"	¹ 2" x 2'-0" x 1'-1"	4"	3	4"	5
4	³ 4" x 1'-4" x 2'-7"	⁷ 8" x 7" x 2'-7"	'4" x 1'-4" x 1'-3 ³ 8"	⁷ 8" x 1'-4" x 3'-7"	1" x 7" x 3'-7"	³ ₄ " x 1'-4" x 1'-9 ³ ₈ "	¹ 2" x 2'-0" x 1'-1"	4"	4	4"	6
5	³ 4" x 1'-4" x 2'-7"	⁷ 8" x 7" x 2'-7"	1 ₄ " x 1'-4" x 1'-3 ³ 8"	⁷ 8" x 1'-4" x 3'-7"	1" x 7" x 3'-7"	3 ₄ " x 1'-4" x 1'-9 ³ 8"	¹ 2" x 2'-0" x 1'-1"	4"	4	4"	6

Notes:

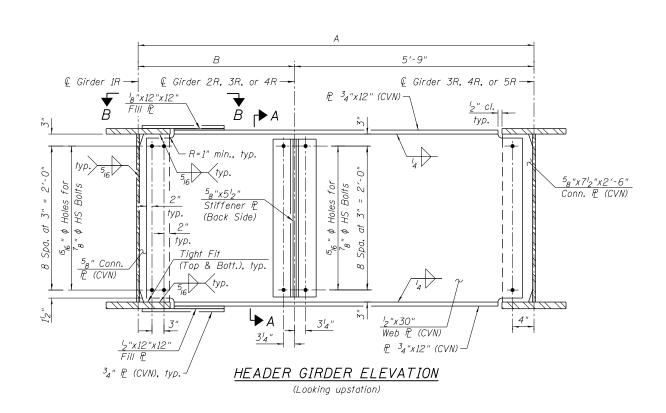
All splice plates, except filler plates, shall meet CVN.
CVN denotes Charpy V-Notch impact energy requirements, Zone 2.
All structural steel, except filler plates, shall be AASHTO
M 270 Grade 50. Filler Plates may be AASHTO M 270 Grade 36.

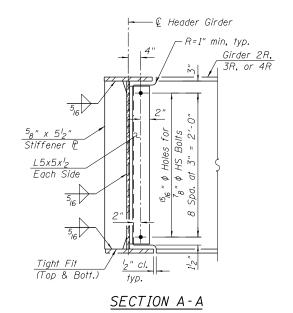


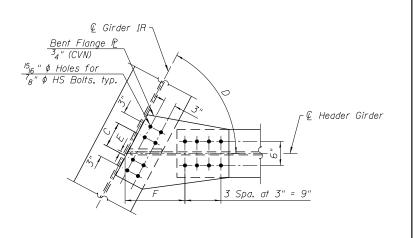
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	CHECKED	WJC	REVISED
PLOT SCALE = NTS	DRAWN	JTF	REVISED
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED

STRUCTURAL STEEL DETAILS 5 STRUCTURE NO. 016–1702	
CHEET NO COLAI OF COLAO CHEETS	-

F.A.U. RTE.	S	ECTION			COUNTY	TOTAL SHEETS	SHE
1422	2014	-015R&E	3-R		COOK	825	42
					CONTRACT	NO. 6	0X9
		ILLINOIS	FED.	ΑI	D PROJECT		



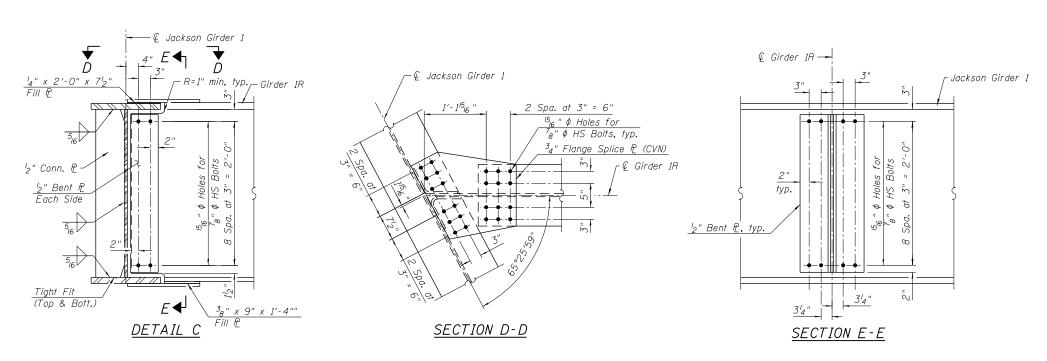




SECTION B-B

HEADER GIRDER TABLE

Header Girder	Longit Left	tudinal G. Center		Girder A	Spacing B	Angle, D	С	Ε	F	Connection Plate	Top & Bott, Flange Splice Plate	
1	1R	2R	3R	8'-012"	2'-31/2"	68°64′01"	6"	1"	1'-1"	⁵ 8" X 8" X 2′-6"	3 ₄ " x 1'-5" x 2'-1"	
2	1R	3R	4R	8'-6 ⁷ 8"	2'-978"	73°07′53"	5½"	1 ⁵ 16 "	1'-0 ³ 16"	⁵ 8" X 7 ¹ 2" X 2'-6"	³ ₄ " x 1'-4" x 2'-0"	
3	1R	4R	5R	7'-8'2"	1'-11'2"	81°58′09"	5"	1 ⁹ 16"	10 ¹⁵ 16 "	⁵ 8" X 7 ¹ 2" X 2'-6"	3 ₄ " x 1'-3" x 1'-10"	



Notos

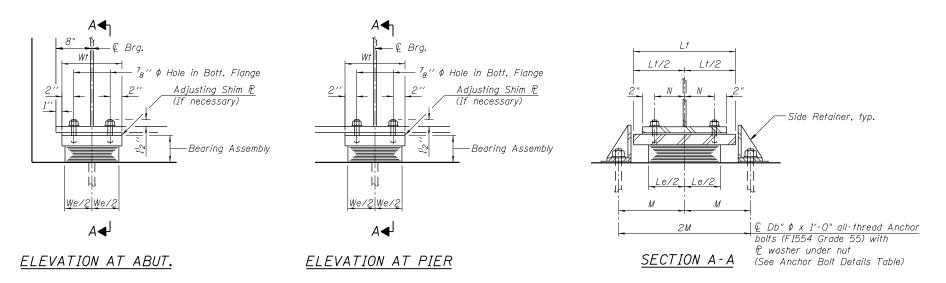
All structural steel shall be AASHTO M 270 Grade 50. CVN denotes Charpy V-Notch impact energy requirements, Zone 2.



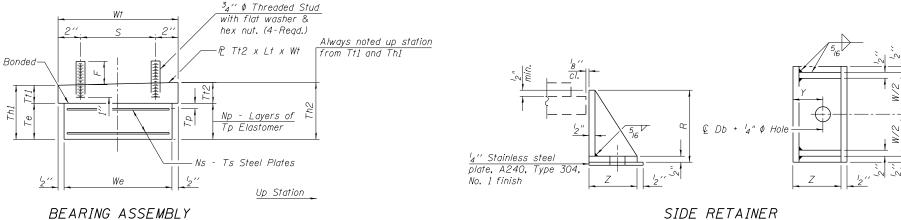
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PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED

STRUCTURAL STEEL DETAILS 6 STRUCTURE NO. 016-1702	
CHEET NO S2-42 OF S2-80 SHEETS	_

RTE.	ECTION			COUNTY	SHEETS	NO.	
1422	2014	-015R&E	3-R		COOK	825	427
			CONTRACT	NO. 6	0X94		
		ILLINOIS	FED.	ΑI	ID PROJECT		



TYPE I ELASTOMERIC EXP. BRG.



Note: Shim plates shall not be placed under Bearing Assembly.

SIDE RETAINER Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

Notes

Anchor bolts shall be ASTM F1554, Grade 55, all-thread (or an Engineer-approved alternate material) of the diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly. Type I.

Beams shall be braced for stability during erection and remain braced until deck is poured and cured.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

Two 1/8 in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

All bearing plates, side retainers, anchor bolts, nuts, washers and pintles shall be galvanized according to AASHTO M111 or M232 as applicable.

The structural steel plates of the bearing assembly shall conform to the requirements of AASHTO M270, Grade 50.

ANCHOR BOLT DETAILS

Bolt Dia. x	Plate Washer
Length**	Traine Wallingt
⁵ 8" x 12"	1 ³ 4"x1 ³ 4"x ⁵ 16"
1" x 12"	2 ¹ ₄ "x2 ¹ ₄ "x ⁵ ₁₆ "

^{**}Length shown is minimum required embedment length.

FILL PLATE THICKNESS TABLE

Location	G1R	G5R	G6R	G7R	G1	G2	G3	G4	G5	G6	G7	G8	G9	G10	G11	G12
W. Abut	-	-	-	-	-	-	-	-	-	-	38''	-	-	-	4"	-
Pier 1	-	-	-	-	-	-	-	-	-	18''	-	-	-	38''	-	-
E. Abut	-	-	-	-	-	-	-	-	-	-	2"	8''	-	-	4"	¹ 8''
N. Abut	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

EXPANSION BEARING DIMENSIONS TABLE

		Elastomer				Top Bearing Plate							Anchor	Side Retainer								
Brg. Location	We (in)	Le (in)	Te (in)	Np	Tp (in)	Ns	Ts (in)	T†1 (in)	T†2 (in)	Wt (in)	Lt (in)	N (in)	S (in)	F (in)	Th1 (in)	Th2 (in)	Bolt Dia. Db (in)	M (in)	R (in)	Y (in)	W (in)	Z (in)
W. Abut	12''	18′′	5½′′	7	916 ′′	6	3 ₁₆ ′′	1/2′′	216''	13′′	20′′	6"	9′′	334''	6 ⁹ 16′′	7/8′′	58′′	11 ⁷ 8 ′′	712''	134''	10′′	714''
Pier 1	18′′	24''	39 ₁₆ ′′	4	34''	3	3 ₁₆ ′′	212"	214''	19′′	26′′	6"	<i>15′′</i>	378''	616''	5 ¹³ 16 ''	1''	15½′′	612"	218''	19''	10 ³ 8 ′′
E. Abut	7''	12′′	1 ⁵ 16 ′′	3	38′′	2	332 ′′	1"16''	1'2''	8′′	16′′	6''	4′′	2 ⁵ 8′′	3''	21316 "	58′′	978′′	314''	134''	8′′	414''
N. Abut	10′′	14′′	21/16''	5	7 ₁₆ ′′	4	8''	23 ₈ ''	1'2''	11''	16′′	6"	7''	318''	516′′	4 ³ 16 ′′	58′′	97 _{8′′}	5½″	134''	8′′	5½′′

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	40
Anchor Bolts, ⁵ 8"	Each	56
Anchor Bolts, 1"	Each	24

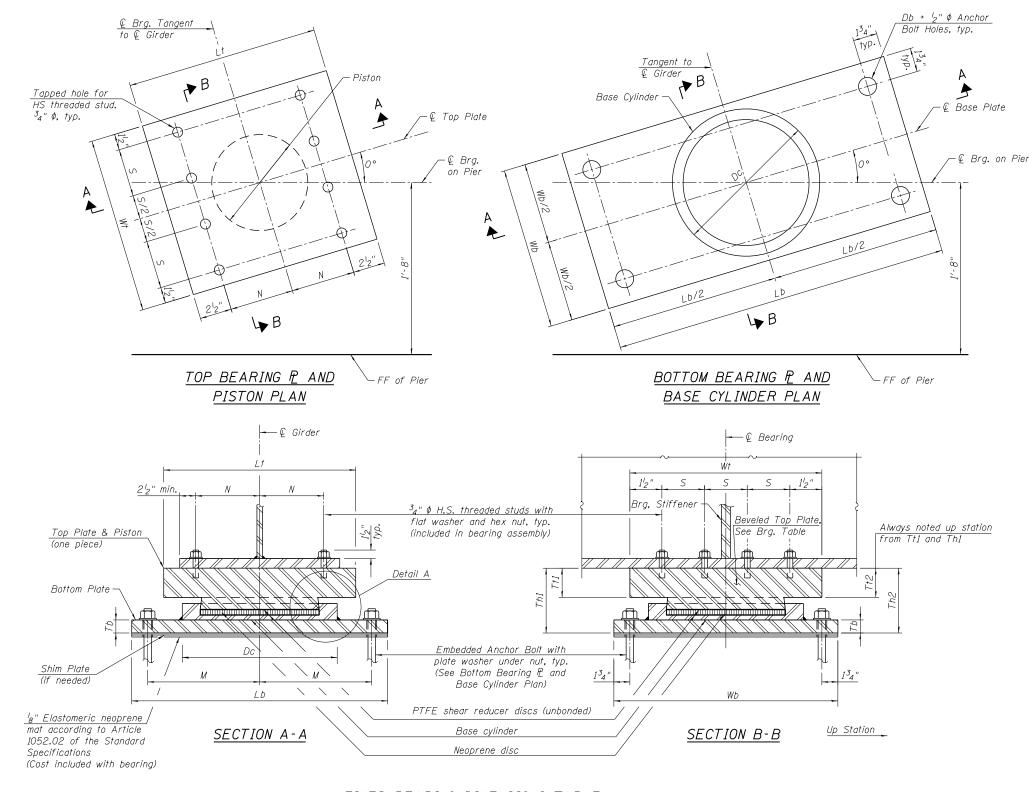


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	CHECKED	TLR	REVISED
PLOT SCALE = NTS	DRAWN	TLR	REVISED
PLOT DATE = 3/5/2020	CHECKED	CG	REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

EXPANSION BEARING DETAILS	F.A.U. RTE.		
STRUCTURE NO. 016-1702	1422	201	
0111001011E 140:010-1702			
SHEET NO. S2-43 OF S2-80 SHEETS			

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1422	2014-015R&B-R	COOK	825	428
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. AI	ID PROJECT		



FIXED BEARING DIMENSIONS TABLE

	Vertical	Lateral			Bott	om Bearin	g Plate	Top Bearing Plate								Anchor	
Brg. Location	Design Load (kips)	Design Load (kips)	HLMR Size (kips)	Dc (in)	Tb (in)	Lb (in)	Wb (in)	Tt1 (in)	Tt2 (in)	Lt (in)	Wt (in)	N (in)	S (in)	Th1 (in)	Th2 (in)	Bolt Dia. Db (in)	M (in)
Pier 2-Girders 2-12	187.0	89.0	200	11/4′′	1/8′′	2034''	11/4′′	216''	1'2''	11/4′′	11/4′′	318''	234"	6 ¹⁵ 16 ′′	6 ³ 8′′	58′′	8 ⁵ 8′′
Pier 2-Girder 1	458.0	264.0	500	17½''	1''	24'4''	17 ¹ 2′′	316''	214"	17 ¹ 2′′	17 ¹ 2′′	64''	41316 "	97 _{16′′}	8 ⁵ 8′′	1''	10 ³ 8 ′′
Pier R1	280.0	104.0	300	13½′′	1''	214''	13½''	23 ₈ "	134''	13½′′	13½′′	414''	312"	778''	714''	34''	87 _{8′′}

Not

The Structural Steel for the top & bottom bearing plates shall be AASHTO M270 Grade 50.

Top & bottom plates, threaded studs, washers & shim plates are included in the cost of the Bearings.

Anchor bolts for bearings shall be placed in holes drilled in the concrete through holes in the bottom bearing plate after members are in place.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Two $^{\prime}_{8}$ in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

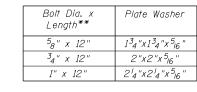
All (embedded and separate) bearing plates, anchor bolts, nuts, washers, pintles, and threaded studs shall be galvanized according to AASHTO M111 or M232 as applicable.

If base cylinder is recessed into the bottom bearing plate, the thickness of the bottom plate shall be Tb plus the depth of the recess.

All HLMR bearings shall be designed to carry minimum Factored Ultimate (Strength) Design Rotation of 0.02 radians. See Special Provision.

Anchor bolts shall be ASTM F1554, Grade 55 all-thread (or an Engineer-approved alternate material) of the diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

ANCHOR BOLT DETAILS



**Length shown is minimum required embedment length.

<u>DETAIL A</u>

Brass seal,

FILL PLATE THICKNESS TABLE

Weld may be omitted if

into bott. brg. plate

base cylinder is recessed

Location	G1R	G5R	G6R	G7R	G1	G2	G3	G4	G5	G6	G7	G8	G9	G10	G11	G12
Pier 2	-	-	-	-	-	-	-	-	-	8''	-	-	-	2"	-	-
Pier R1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

BILL OF MATERIAL

Item	Unit	Total
High Load Multi-Rotational Bearings, Fixed - 200K	Each	11
High Load Multi-Rotational Bearings, Fixed - 300K	Each	4
High Load Multi-Rotational Bearings, Fixed - 500K	Each	1
Anchor Bolts, ⁵ 8"	Each	44
Anchor Bolts, 3 ₄ "	Each	16
Anchor Bolts, 1"	Each	4

Tran Systems

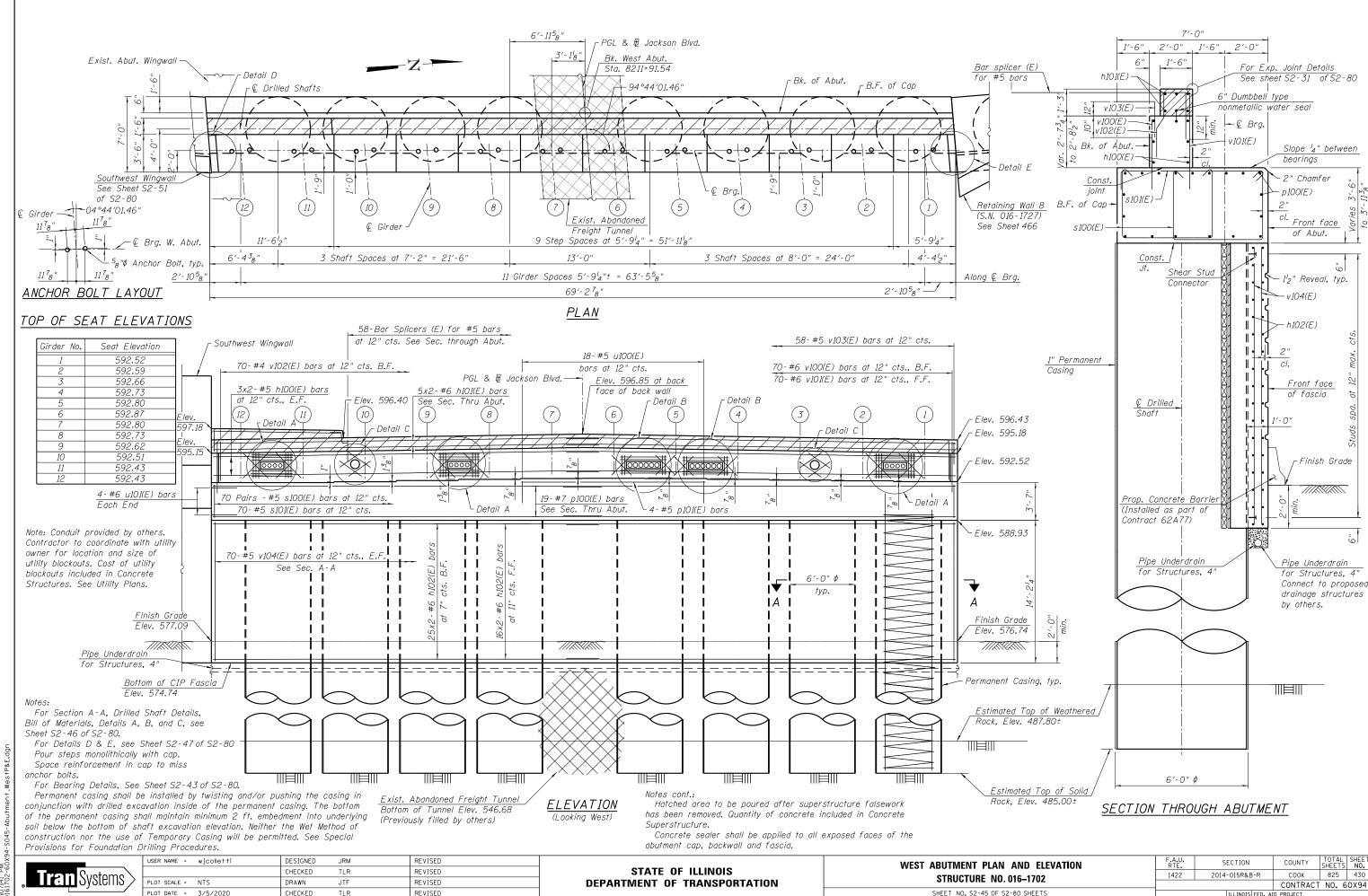
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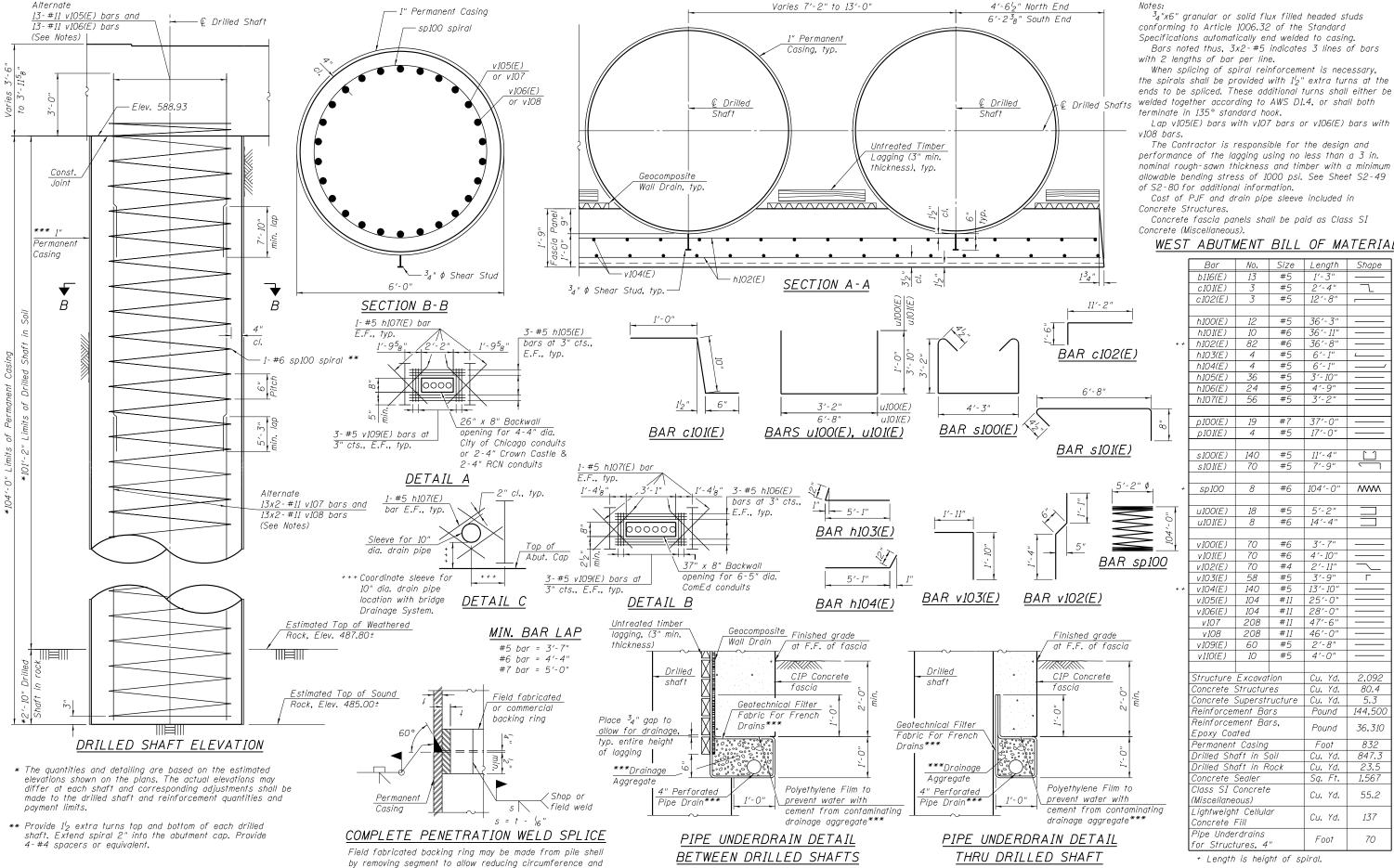
FIXED	BEARING	DETAILS
STRUC	TURE NO.	016-1702
SHEET NO	\$2-44 NF \$	2-80 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE'
1422	2014-015R&B-R	соок	825	429
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. A	ID PROJECT		

02-60x94-S044-Bearing_Details2.dgn

0x94-S044-Bear





	USER NAME = wjcolletti	DESIGNED	JRM	REVISED
		CHECKED	TLR	REVISED
>	PLOT SCALE = NTS	DRAWN	JTF	REVISED

CHECKED

TLR

vertically rejoin with partial joint penetration weld.

REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** WEST ABUTMENT DETAILS STRUCTURE NO. 016-1702 SHEET NO. S2-46 OF S2-80 SHEETS

++ Shown for information only, Cost included with Class SI Concrete (Miscellaneous). F.A.U. RTE. SECTION COUNTY 2014-015R&B-R COOK 825 431

Size

#5

#5

#5

#6

#6

#5

#5 #5

#5

#5

#5

#5

#6

#5

#6

#6

#4

#5

#5

#11

#11 #11 47'-6"

#5

#5

#6

Length

12'-8"

36′-11"

3'-10"

17'-0"

11'-4"

104′-0"

14'-4"

4'-10"

2'-11"

13′-10"

#11 46'-0"

25'-0"

2'-8"

4'-0"

Cu. Yd.

Cu. Yd.

Cu. Yd.

Pound

Pound

Foot

Cu. Yd.

Sa. Ft.

Cu. Yd.

Cu. Yd.

Foot

www

2,092

5.3

144,500

36,310

832

847.3

23.5

1,567

55**.**2

137

70

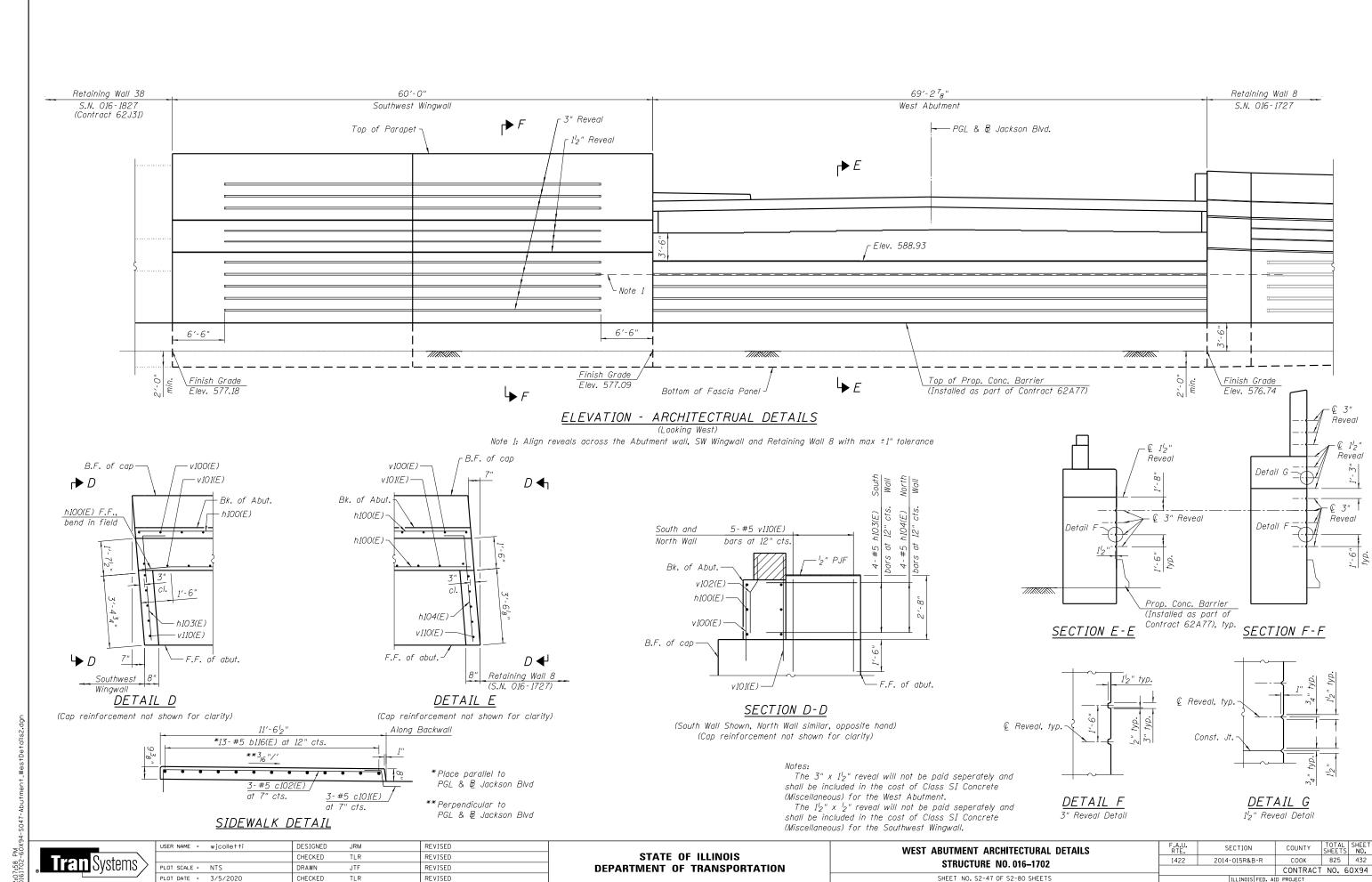
#5 3'-2"

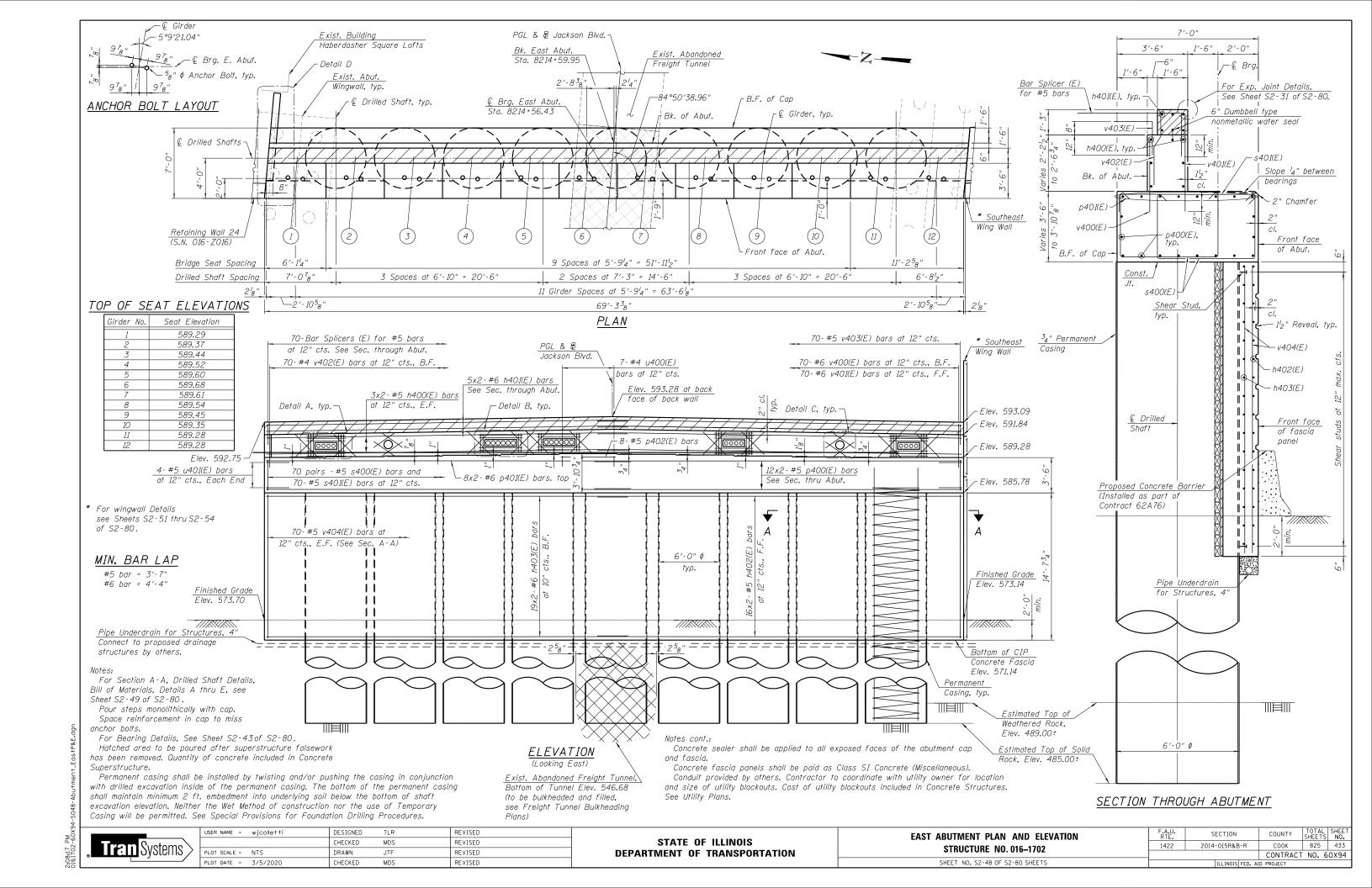
*** Included in the cost of Pipe Underdrains for Structures, 4'

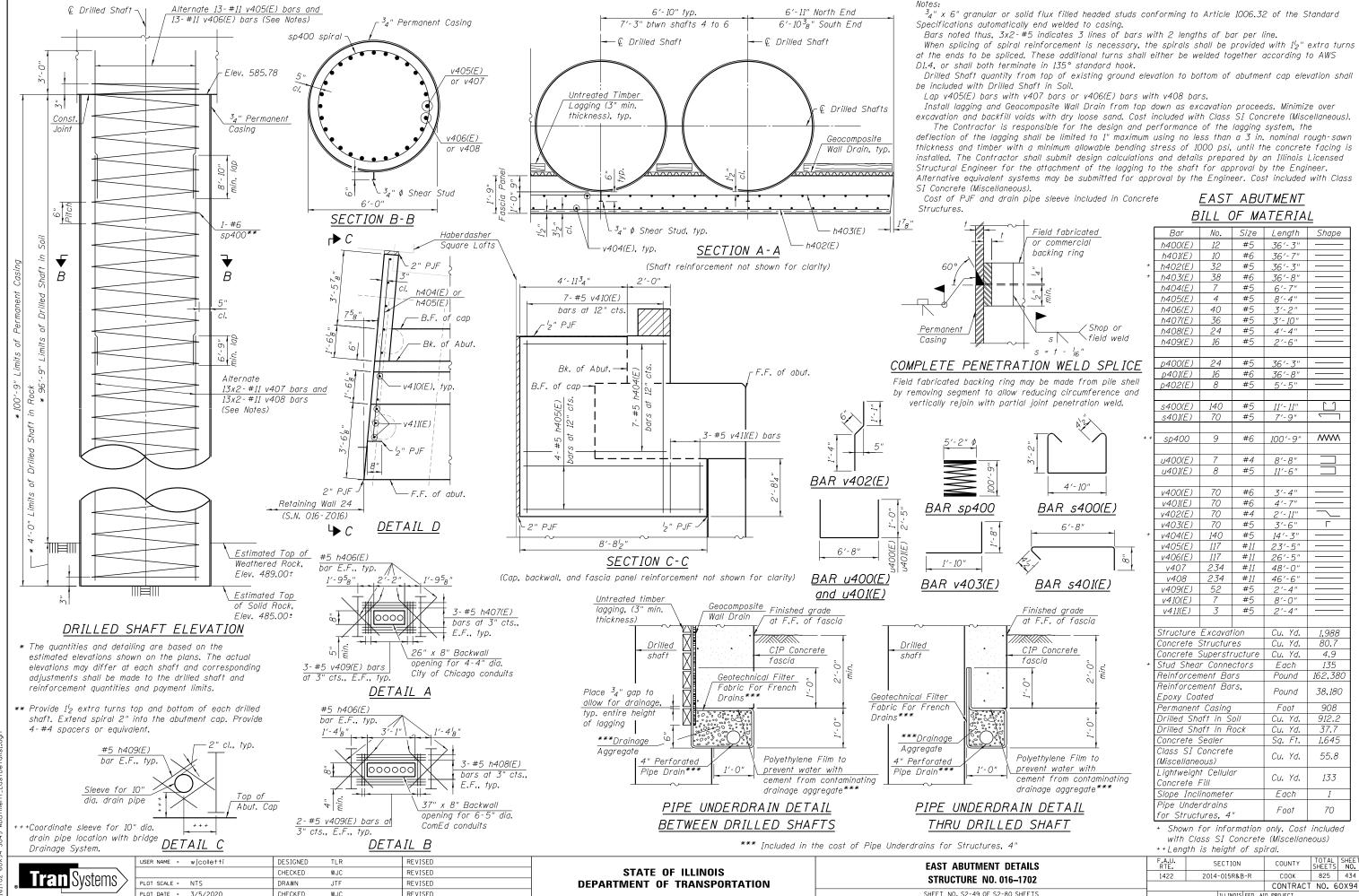
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1422 CONTRACT NO. 60X94

Tran Systems







WWW

4.9

135

162,380

38,180

908

912.2

37,7

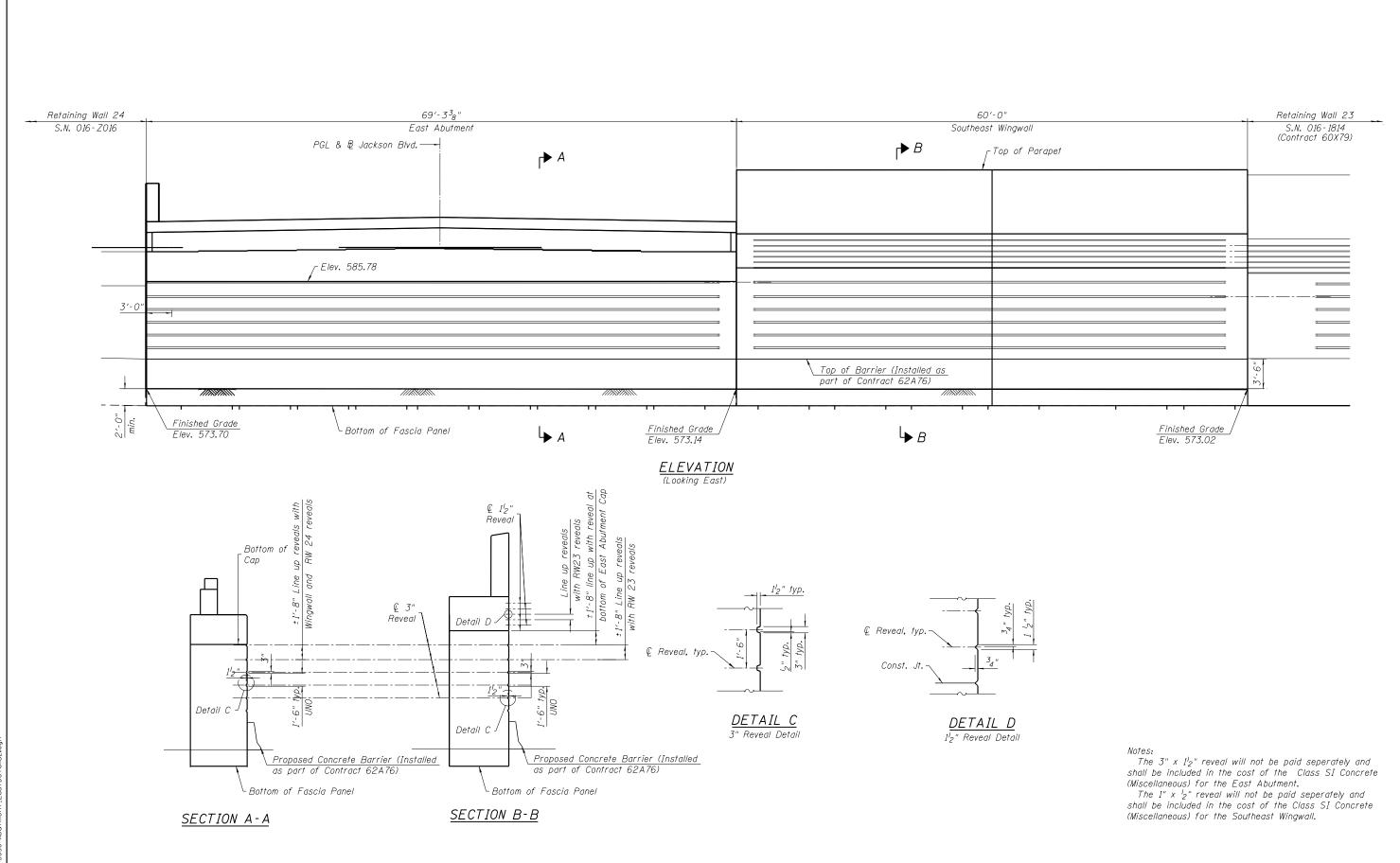
1,645

55.8

133

70

825 434



Tran Systems

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

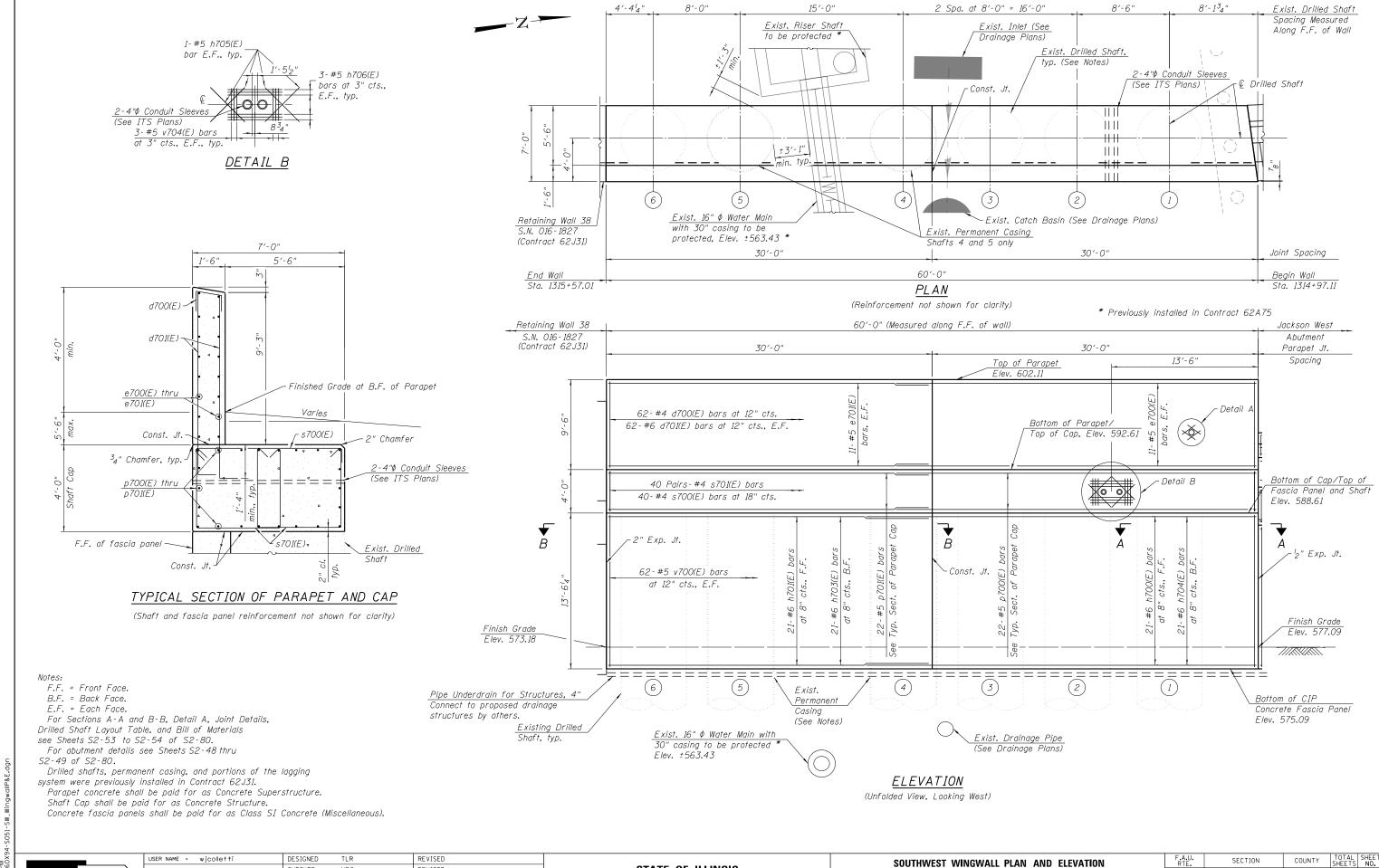
EAST ABUTMENT ARCHITECTURAL DETAILS
STRUCTURE NO. 016–1702

SHEET NO. S2-50 OF S2-80 SHEETS

F,A,U. SECTION COUNTY TOTAL SHEET NO.

1422 2014-015R&B-R COOK 825 435

CONTRACT NO. 60X94

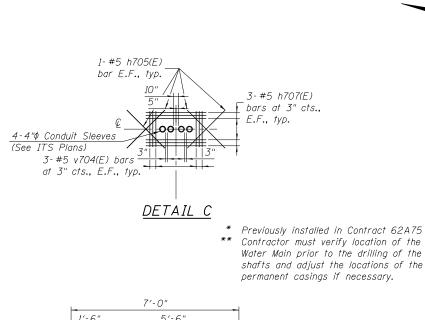


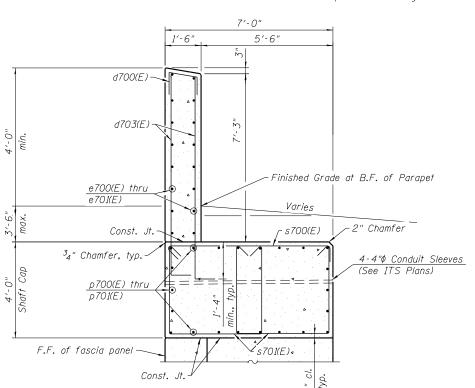
Tran Systems

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PLOT SCALE =	NTS	DRAWN	TLR	REVISED
PLOT DATE =	3/5/2020	CHECKED	MDS	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

THWEST WINGWALL PLAN AND ELEVATION		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
STRUCTURE NO. 016-1702	1422	2014-015R&B-F	R COOK	825	436	
3111001011L NO. 010-1702		·		CONTRACT NO. 60X94		
SHEET NO. S2-51 OF S2-80 SHEETS		ILLINOIS FED. AID PROJECT				





TYPICAL SECTION OF PARAPET AND CAP

(Shaft and fascia panel reinforcement not shown for clarity)

F.F. = Front Face.

B.F. = Back Face.

E.F. = Each Face.

For Sections A-A and B-B, Detail A, Joint Details, and Bill of Materials see Sheets S2-53 to S2-54 of S2-80.

For abutment details see Sheets S2-48 thru S2-50 of S2-80.

Permanent casing shall be installed by twisting and/or pushing the casing in conjunction with drilled excavation inside of the permanent casing. The bottom of the permanent casing shall maintain minimum 2 ft. embedment into underlying soil below the bottom of shaft excavation elevation. Neither the Wet Method of construction nor the use of Temporary Casing will be permitted. See Special Provisions for Foundation Drilling Procedures.

Parapet concrete shall be paid for as Concrete Superstructure.

Shaft Cap shall be paid for as Concrete Structure.

Concrete fascia panels shall be paid for as Class SI Concrete (Miscellaneous).

SHAFT LAYOUT TABLE

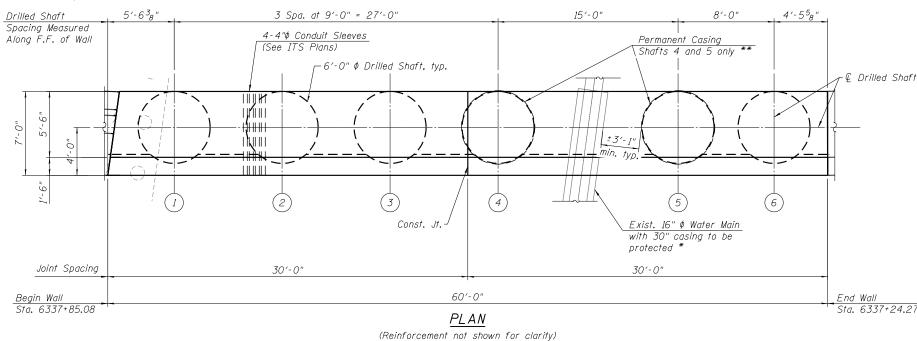
Shaft No.	Station	Offset
1	6337+79.55	22.18′ Rt.
2	6337+70.40	22.30' Rt.
3	6337+61.25	22.37' Rt.
4	6337+52.10	22.37′ Rt.
5	6337+36 . 85	22.24′ Rt.
6	6337+28.72	22.10' Rt.

SE WINGWALL DRILLED

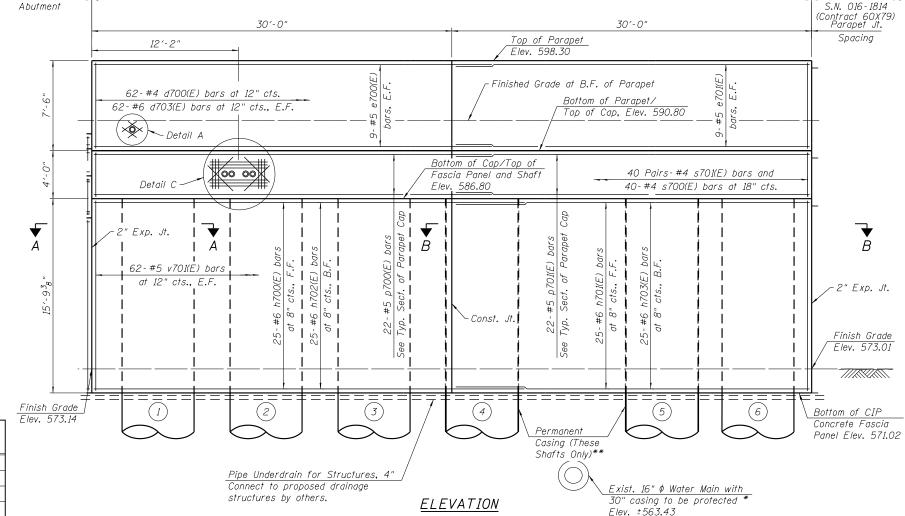
←Z.**—**

Jackson East

Shaft No.	Station	Offset		
1	6337+79.55	22.18' Rt.		
2	6337+70.40	22.30′ Rt.		
3	6337+61.25	22.37′ Rt.		
4	6337+52.10	22.37' Rt.		
5	6337+36 . 85	22.24′ Rt.		
6	6337+28.72	22.10' Rt.		



60'-0" (Measured along F.F. of wall)



(Unfolded View, Looking East)



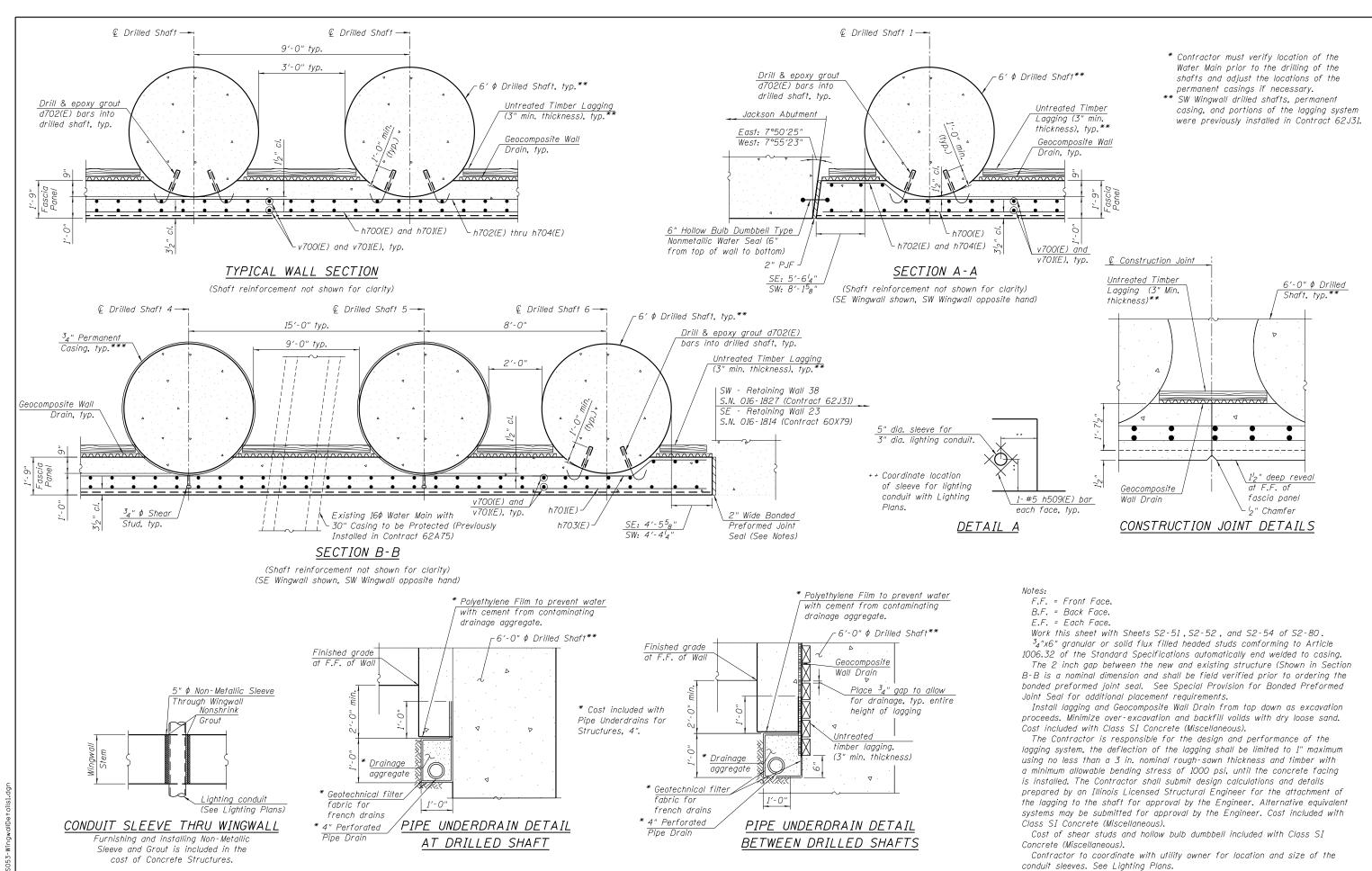
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PLOT DATE = 3/5/2020	CHECKED	MDS	REVISED	

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SOUTHEAST WINGWALL PLAN AND ELEVATION	
STRUCTURE NO. 016-1702	
3111001011L NO. 010-1702	
SHEET NO. S2-52 OF S2-80 SHEETS	Г

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE'
1422	2014-015R&B-R	COOK	825	437
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. A	ID PROJECT		

Retaining Wall 23



, **Tran** Systems

 USER NAME
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 DESIGNED
 TLR
 REVISED

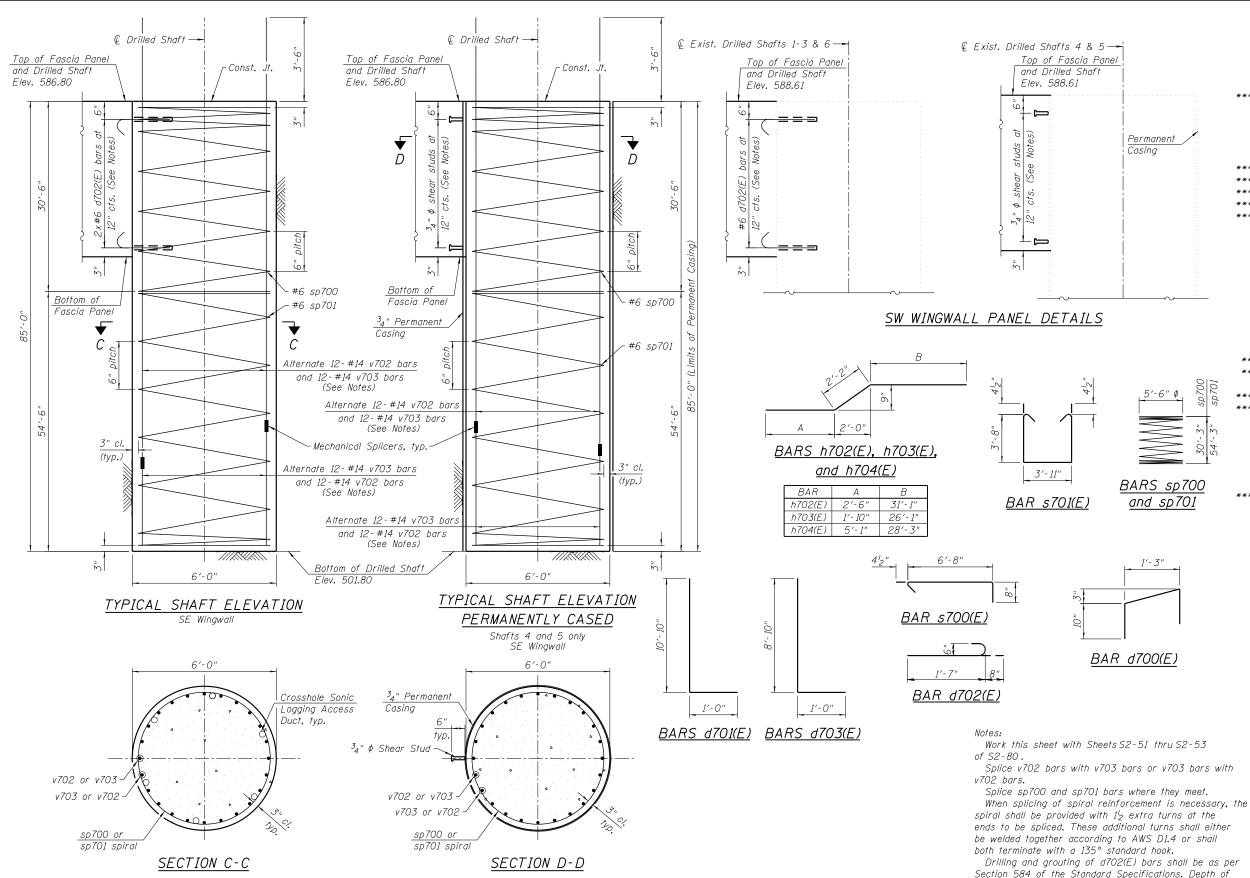
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 PLOT SCALE =
 NTS
 DRAWN
 JTF
 REVISED

 PLOT DATE =
 3/6/2020
 CHECKED
 MDS
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WINGWALL DETAILS 1
STRUCTURE NO. 016–1702
SHEET NO. S2-53 OF S2-80 SHEETS



<u>WINGWALLS</u> BILL OF MATERIAL

	Bar	No.	Size	Length	Shape
	d700(E)	124	#4	3'-0"	
	d701(E)	124	#6	11'-10"	
**	d702(E)	120	#6	2'-3"	
	d703(E)	124	#6	9'-10"	
	e700(E)	20	#5	33'-2"	
	e701(E)	20	#5	29′-8"	
**	h700(E)	46	#6	33′-10"	
**	h701(E)	46	#6	29'-8"	
**	h702(E)	25	#6	35′-9"	
**	h703(E)	46	#6	30'-1"	
**	h704(E)	21	#6	35′-6"	
	h705(E)	16	#5	3'-0"	
	h706(E)	12	#5	4'-10"	
	h707(E)	12	#5	4'-0"	
	h708(E)	16	#5	2'-6"	
	777 00(2)	10			
	p700(E)	44	#5	33'-2"	
	p701(E)	44	#5	29'-8"	
	prone.				
	s700(E)	80	#5	7′-8.5"	
	s701(E)	160	#5	12'-0"	<u>r1</u>
	OTOILLY	100		12 0	
**	sp700	6	#6	30′-3"	MM
**	sp701	6	#6	54'-3"	/WX
	0,0101		0	3, 3	7000
**	v700(E)	124	#5	13'-2"	
**	v701(E)	124	#5	15'-5"	
	v702	144	#14	42'-2"	
	v703	144	#14	46'-1"	
	v704(E)	24	#5	2'-6"	
			_		170
	Structure E			Cu. Yd.	130
	Concrete S			Cu. Yd.	124.5 55.9
w.w.	Concrete S			Cu. Yd.	
**	Stud Shear			Each Pound	60 124,480
	Reinforcem			Pouria	124,400
	Reinforcem		5,	Pound	11,740
	Epoxy Coat			C4	
	Permanent	tasing ft in Co	.71	Foot	170
	Drilled Shar		'11	Cu. Yd.	534.1
	Concrete S			Sq. Ft.	3,918
	Class SI Co			Cu. Yd.	100.7
	(Miscellaneo				
	Crosshole Sonic Logging			Foot	340
	Access Ducts				
	Crosshole Sonic Logging			Each	1
	Testing				
	Lightweight			Cu. Yd.	431
	Concrete F.		loint		
	Bonded Pre		JOINT	Foot	55
	Sealer, 2 I	IICII	`~~		
	Pipe Under		Ur.	Foot	120
	Structures	4			

- ** Length is height of spiral
- *** Shown for information only. Cost included with Class SI Concrete (Miscellaneous).

Minimum B	ar Laps
Bar	Lap
#5	3'-2"
#6	3′-10"

• Tran Systems

USER NAME = wjcolletti	DESIGNED	TLR	REVISED
	CHECKED	MDS	REVISED
PLOT SCALE = NTS	DRAWN	JTF	REVISED
PLOT DATE = 3/5/2020	CHECKED	MDS	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

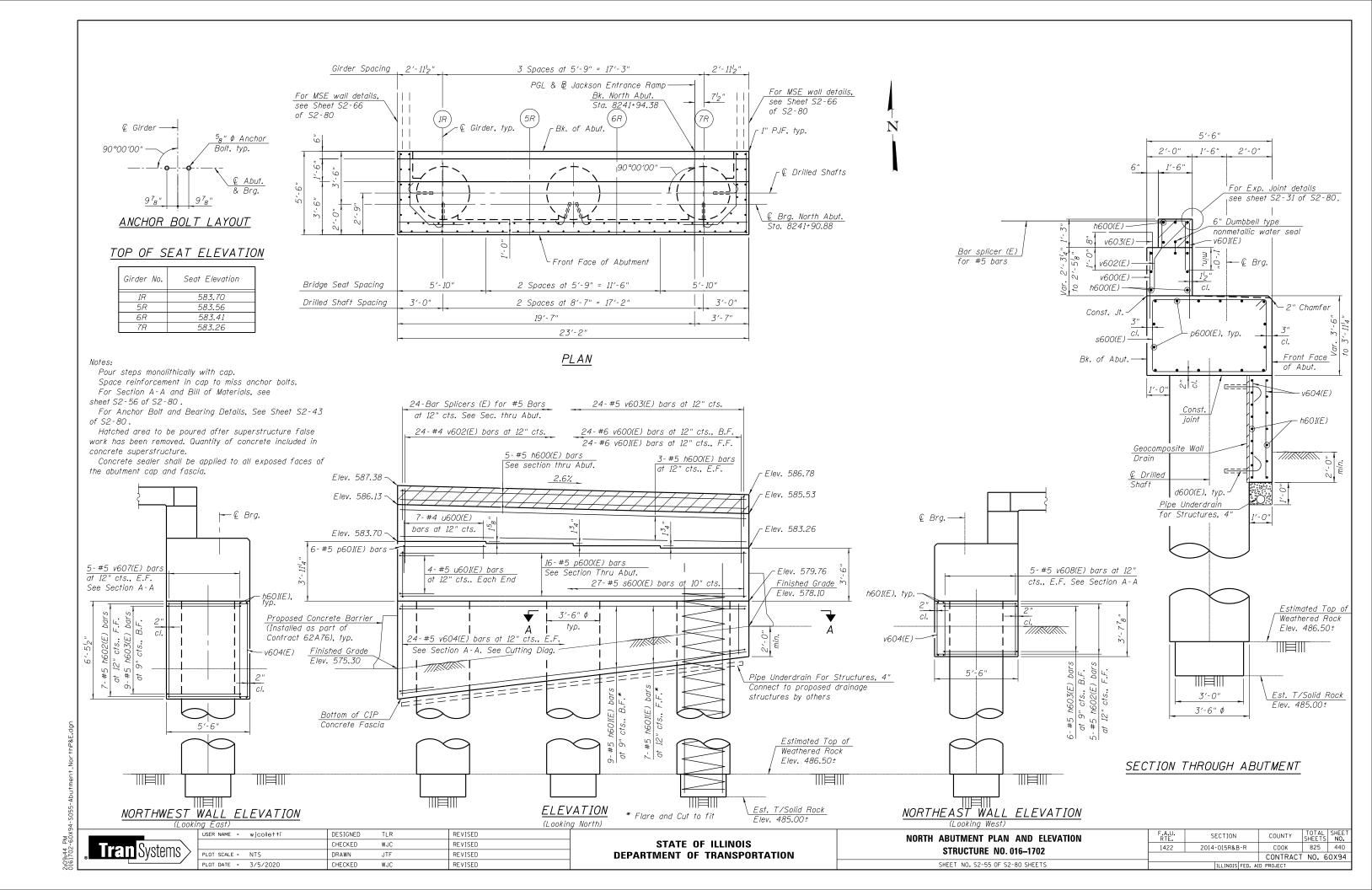
		DETAILS 2 NO. 016–1702	
SHEET	NO. S2-54	OF S2-80 SHEETS	

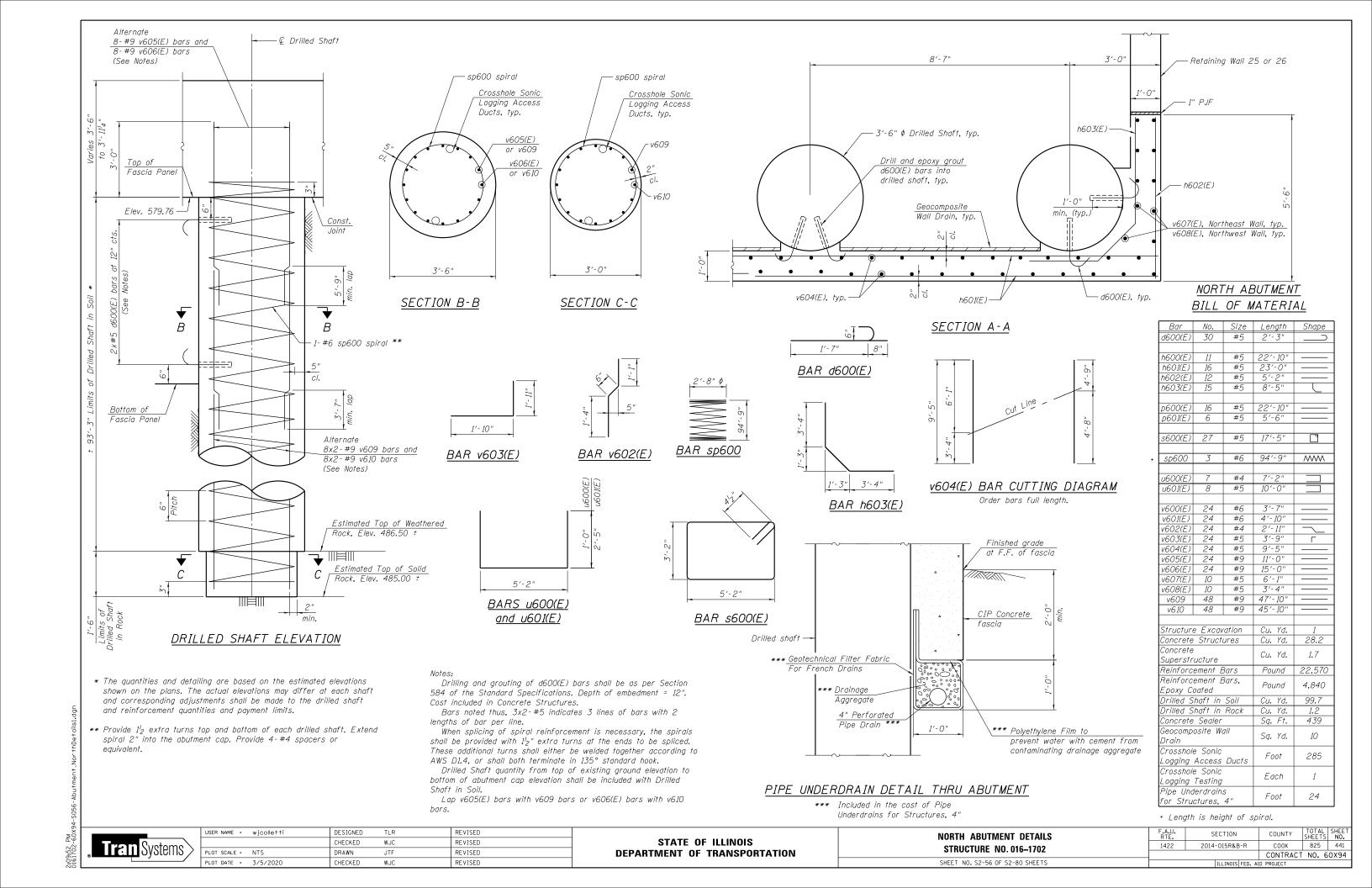
(Miscellaneous).

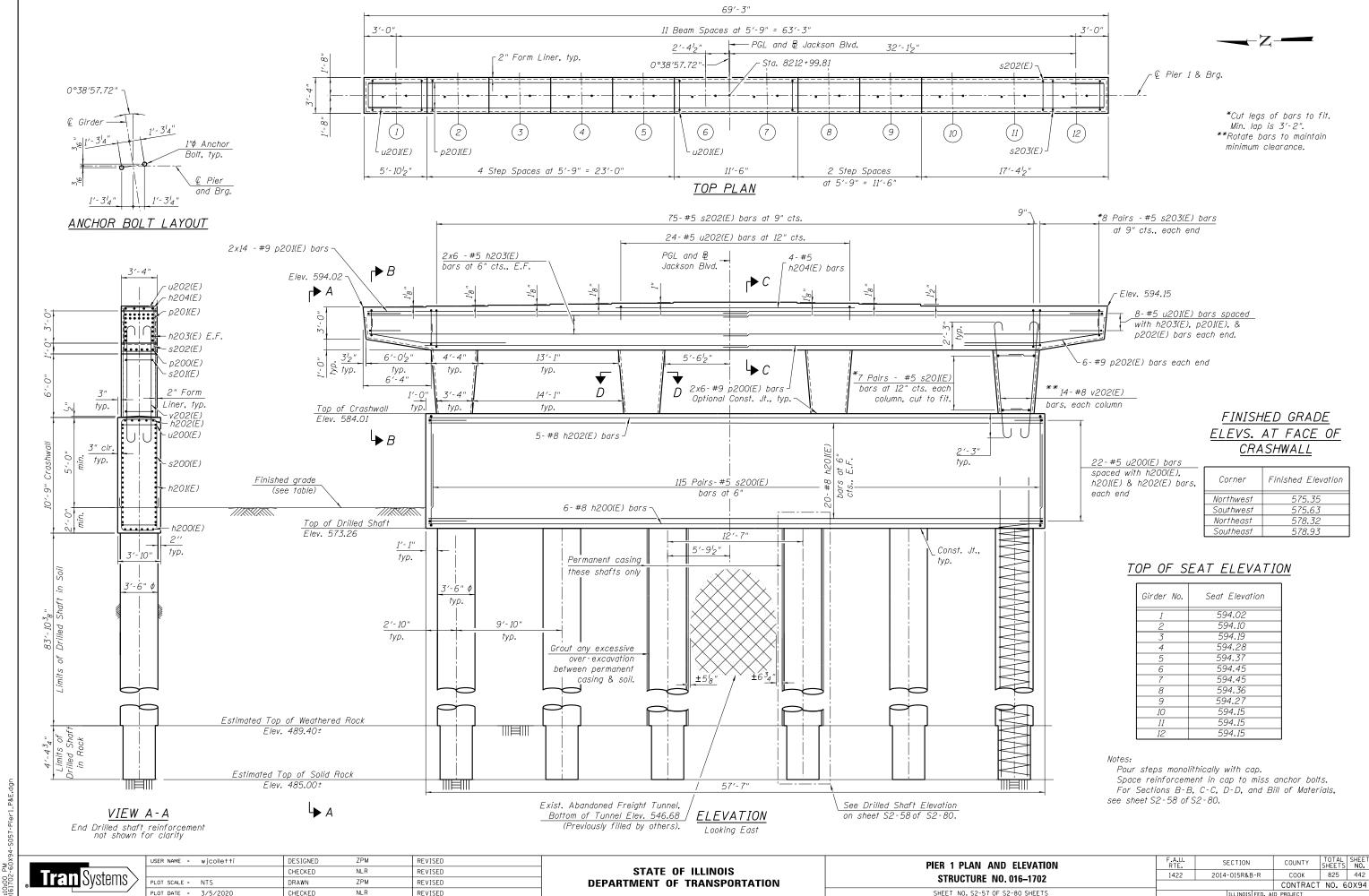
embedment = 12". Contractor shall position d702(E) bars to miss shaft rebar. Cost included in Class SI Concrete

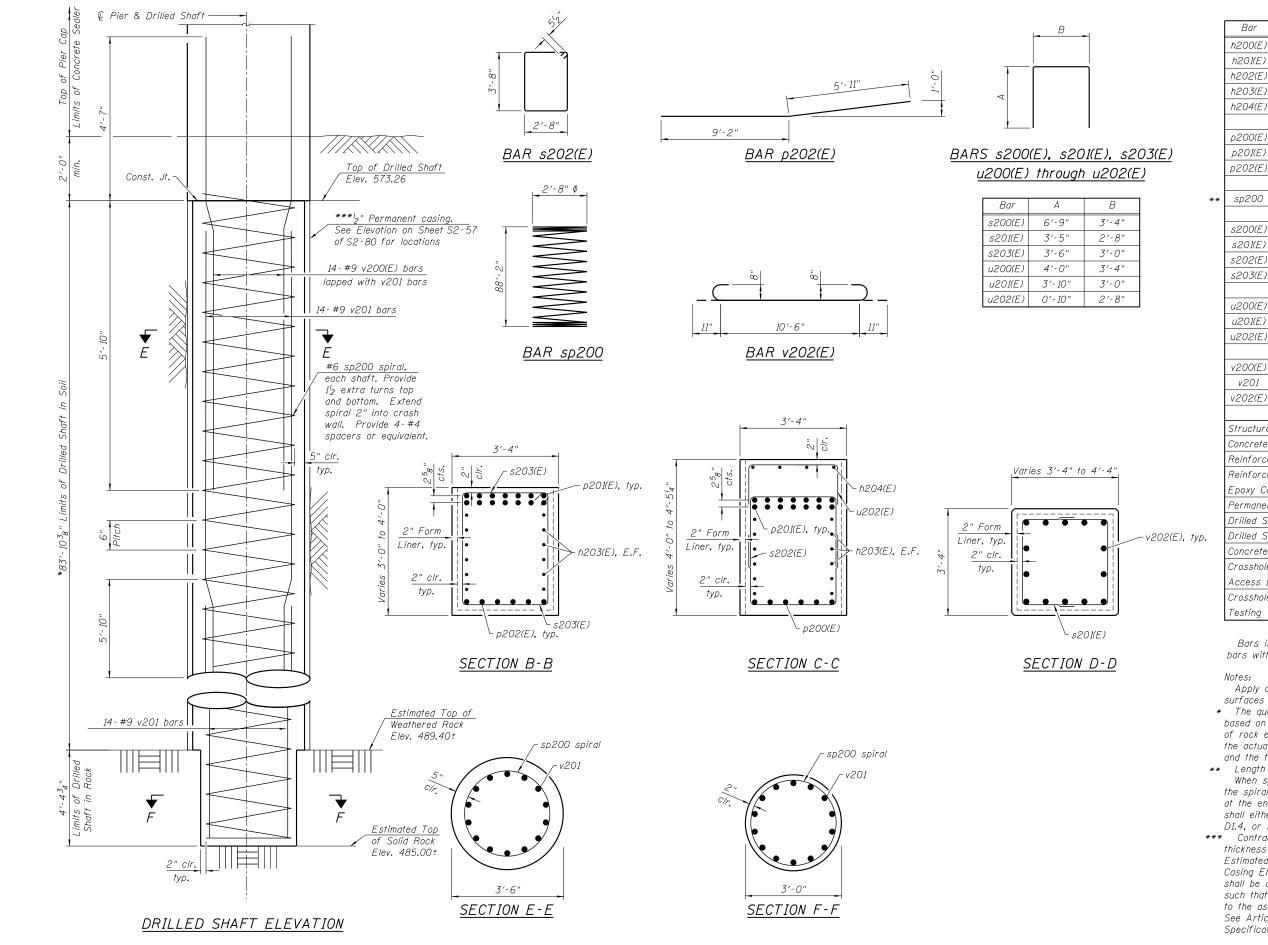
 34 " x 6" granular or solid flux filled headed studs conforming to Article 1006.32 of the Standard Specifications automatically end welded to casing.

F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
1422	2014-015R&B-R		COOK	825	439
·			CONTRACT	NO. 6	0X9
	ILLINOIS	FED. AI	D PROJECT		









BILL OF MATERIAL

	Bar	No.	Size	Length	Shape
	h200(E)	6	#8	57′-1"	
	h201(E)	40	#8	57′-1"	
	h202(E)	5	#8	57′-1"	
	h203(E)	24	#5	35′-11"	
	h204(E)	4	#5	22′-8"	
	p200(E)	12	#9	32′-6"	
	p201(E)	28	#9	39′-8"	
	p202(E)	12	#9	15′-1"	
ŧ	sp200	6	#6	88′-2"	1
	s200(E)	230	#5	16′-9"	П
	s201(E)	56	#5	9′-6″	П
	s202(E)	75	#5	13′-7"	
	s203(E)	32	#5	10'-0"	П
	u200(E)	44	#5	11'-4"	П
	u201(E)	16	#5	10′-8"	
	u202(E)	24	#5	4'-4"	П
	v200(E)	84	#9	10′-5″	
	v201	168	#9	46′-11"	
	v202(E)	56	#8	12'-4"	J
	Structure	Excavat	ion	Cu. Yd.	95
	Concrete	Structur	es	Cu. Yd.	133.8
	Reinforce	ement Bai	rs	Pound	40,370
	Reinforce	ement Bai	rs,	Pound	29,270
	Ероху Сс	Epoxy Coated		Found	29,270
	Permanent Casing			Foot	168
	Drilled Shaft in Soil		oil	Cu. Yd.	179.3
	Drilled Shaft in Rock		ock	Cu. Yd.	7.0
	Concrete	Concrete Sealer		Sq. Ft.	2654
	Crosshole Sonic Logging		ogging	Foot	E 70
	Access Ducts			F 001	530
	Crosshole	Sonic L	ogging	Each	,
	Testing			Lucii	1

Bars indicated thus 1x15 etc., indicates 1 line of bars with 15 lengths per line.

Apply concrete sealer to all exposed concrete surfaces of the pier.

- * The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.
- ** Length is height of spiral.

When splicing spiral reinforcement is necessary, the spirals shall be provided with 1/2" extra turns at the ends to be spliced. These additional turns shall either be welded together according to AWS D1.4, or shall both terminate in 135° standard hook.

*** Contractor may need to increase the casing thickness to withstand the installation process. The Estimated Top of Rock/Bottom of Permanent Casing Elevation is shown. The limits of casing shall be adjusted as necessary, and as approved, such that the actual installed casing length extends to the as-encountered to if rock at each shaft. See Article 516.06(d) of the Standard Specifications.

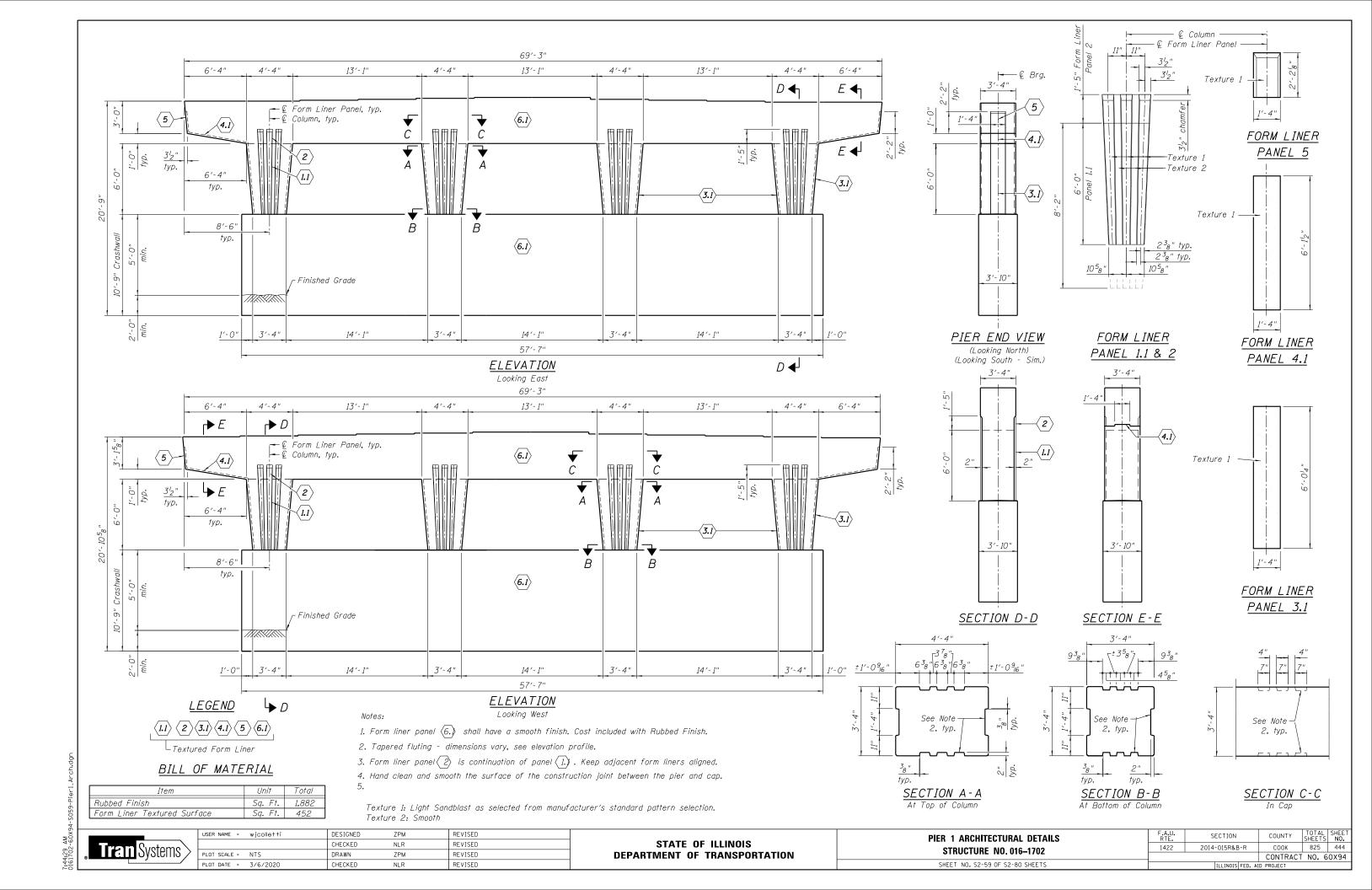


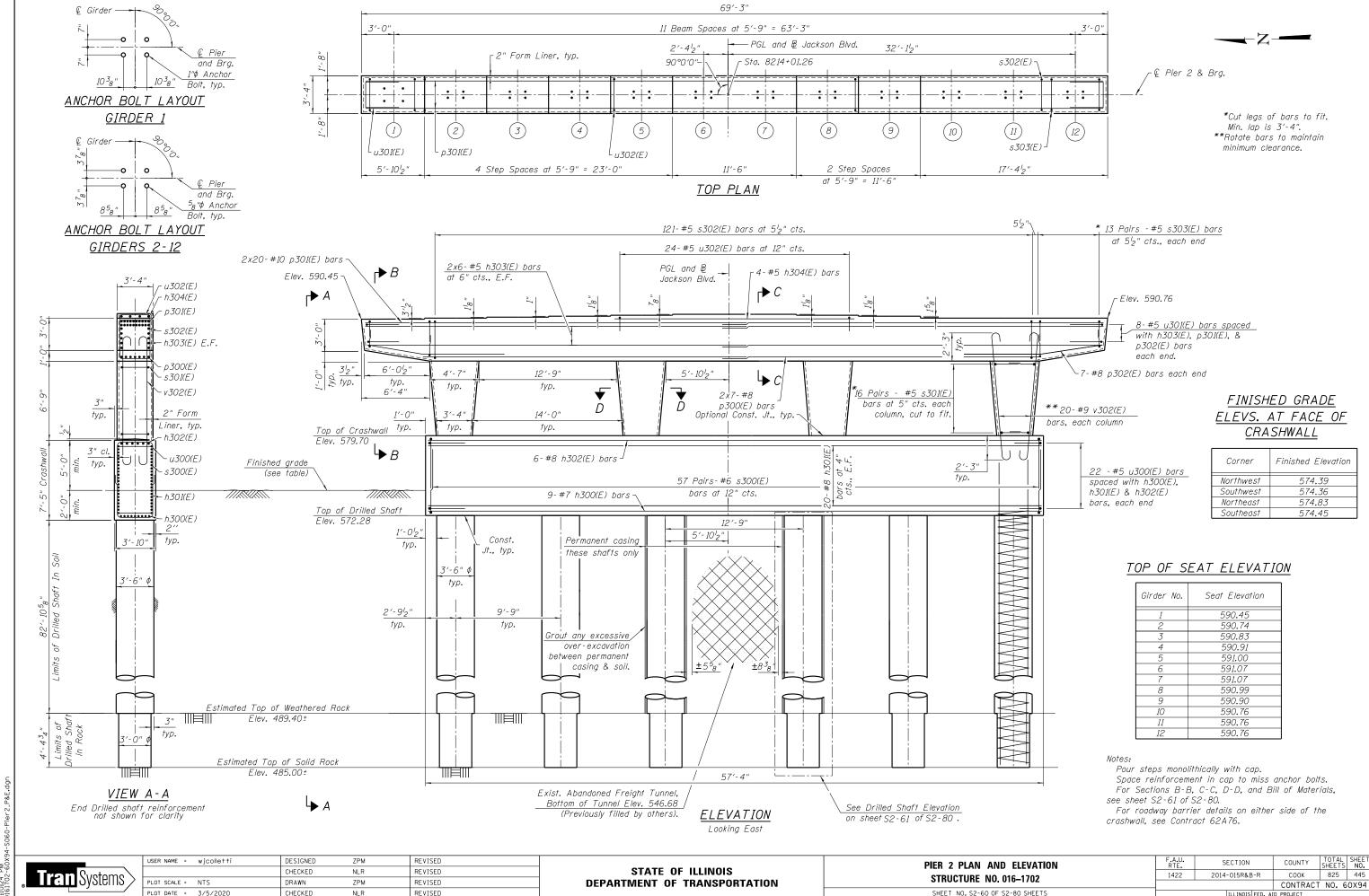
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		CHECKED	NLR	REVISED
PLOT SCALE =	NTS	DRAWN	ZPM	REVISED
PLOT DATE =	3/5/2020	CHECKED	NLR	REVISED

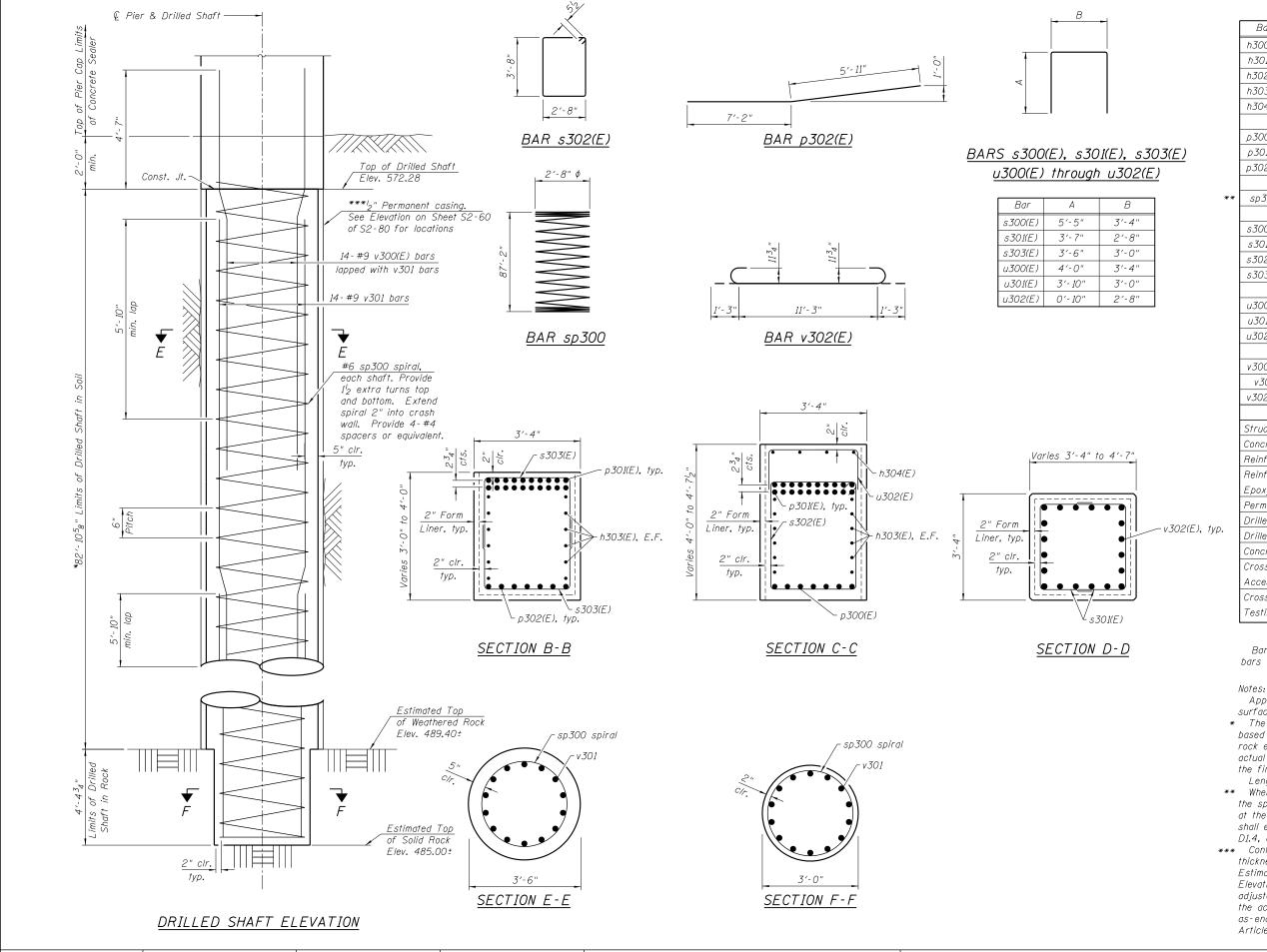
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

	ΡI	ER 1	DETAILS	
STR	UCT	ΓURE	NO. 016-1702	
CHEET	NO	CO EO	OF CO BO CHEFTS	

F.A.U. RTE.	SI	ECTION		COUNTY	TOTAL SHEETS	SHEET NO.
1422	1422 2014-015R&B-R			COOK	825	443
CONTRACT NO. 60X94						0X94
ILLINOIS FED. AID PROJECT						







BILL OF MATERIAL

	Bar	No.	Size	Length	Shape
	h300(E)	9	#7	56′-10"	
	h301(E)	40	#8	56′-10"	
	h302(E)	6	#8	56′-10"	
	h303(E)	24	#5	35′-11"	
	h304(E)	4	#5	22′-8"	
	p300(E)	14	#8	31′-6"	
	p301(E)	40	#10	40′-3"	
	p302(E)	14	#8	13′-1"	
	sp300	6	#6	87′-2"	1
	s300(E)	114	#6	14 '- 1"	П
	s301(E)	128	#5	9′-11"	П
	s302(E)	121	#5	13′-7"	
	s303(E)	52	#5	10'-0"	П
	u300(E)	44	#5	11'-4"	П
	u301(E)	16	#5	10′-8"	П
	u302(E)	24	#5	4'-4"	П
	v300(E)	84	#9	10′-5"	
	v301	168	#9	46′-5"	
	v302(E)	80	#9	13′-9"	
	Structure	Excavat	ion	Cu. Yd.	39
	Concrete	Structur	es	Cu. Yd.	108.7
	Reinforce	ement Ba	rs	Pound	39,930
	Reinforce	ement Ba	rs,	Pound	32,930
	Ероху Сс	pated		, 50,75	32,330
	Permanent Casing			Foot	166
	Drilled Shaft in Soil			Cu. Yd.	177.2
	Drilled Shaft in Rock			Cu. Yd.	7.0
	Concrete	Sealer		Sq. Ft.	2,299
Crosshole Sonic Logging				524	
	Access [Ducts		Foot	327
	Crosshole	Sonic L	ogging		,
	Testing			Each	1

Bars indicated thus 1x15 etc., indicates 1 line of bars with 15 lengths per line.

Apply concrete sealer to all exposed concrete surfaces of the pier.

- * The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.
- Length is height of spiral.
- ** When splicing spiral reinforcement is necessary, the spirals shall be provided with 11#2" extra turns at the ends to be spliced. These additional turns shall either be welded together according to AWS D1.4, or shall both terminate in 130° standard hook.
- *** Contractor may need to increase the casing thickness to withstand the installation process. The Estimated Top of Rock/Bottom of Permanent Casing Elevation is shown. The limits of casing shall be adjusted as necessary, and as approved, such that the actual installed casing length extends to the as-encountered to if rock at each shaft. See Article 516.06(d) of the Standard Specifications.

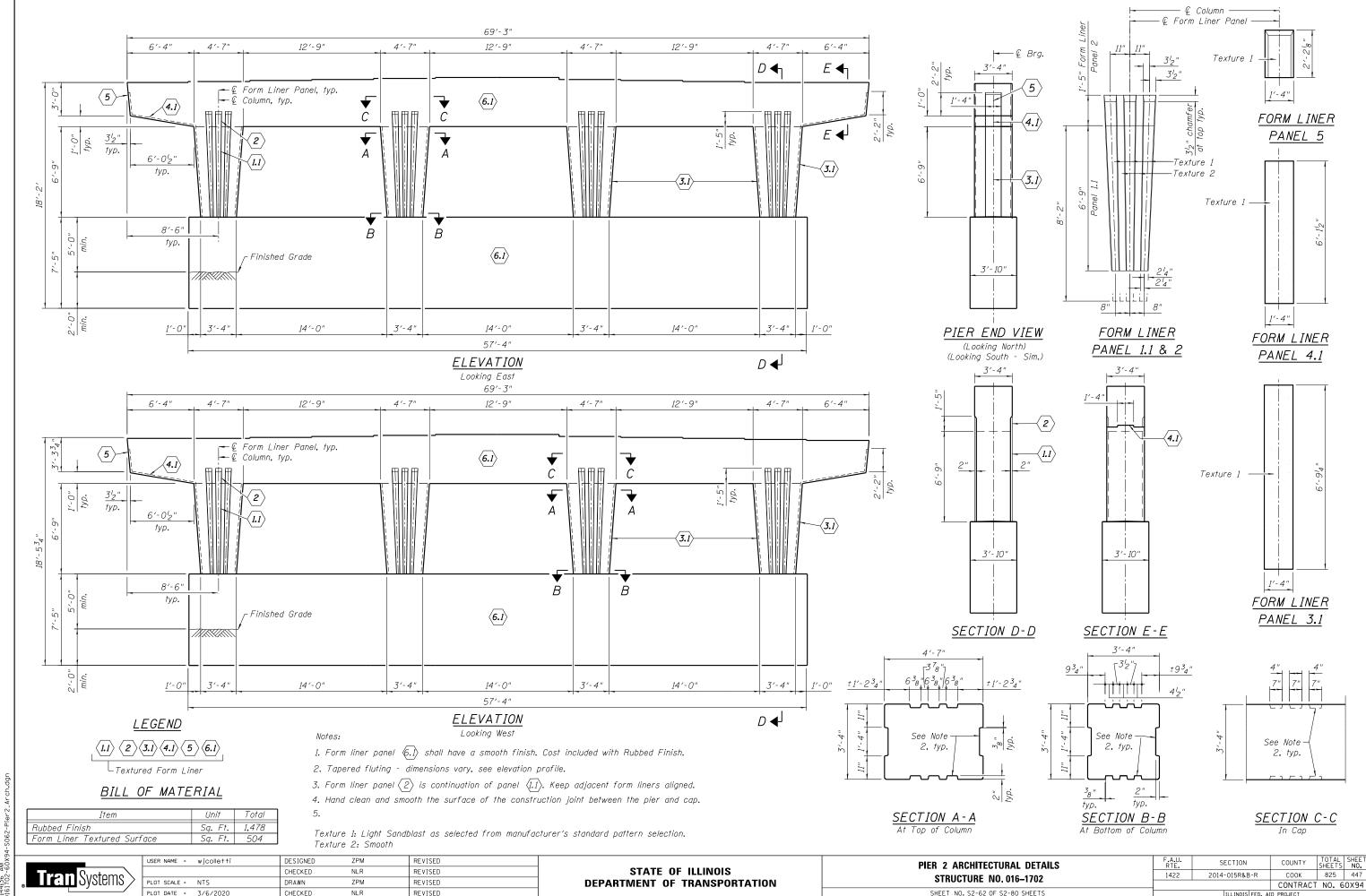


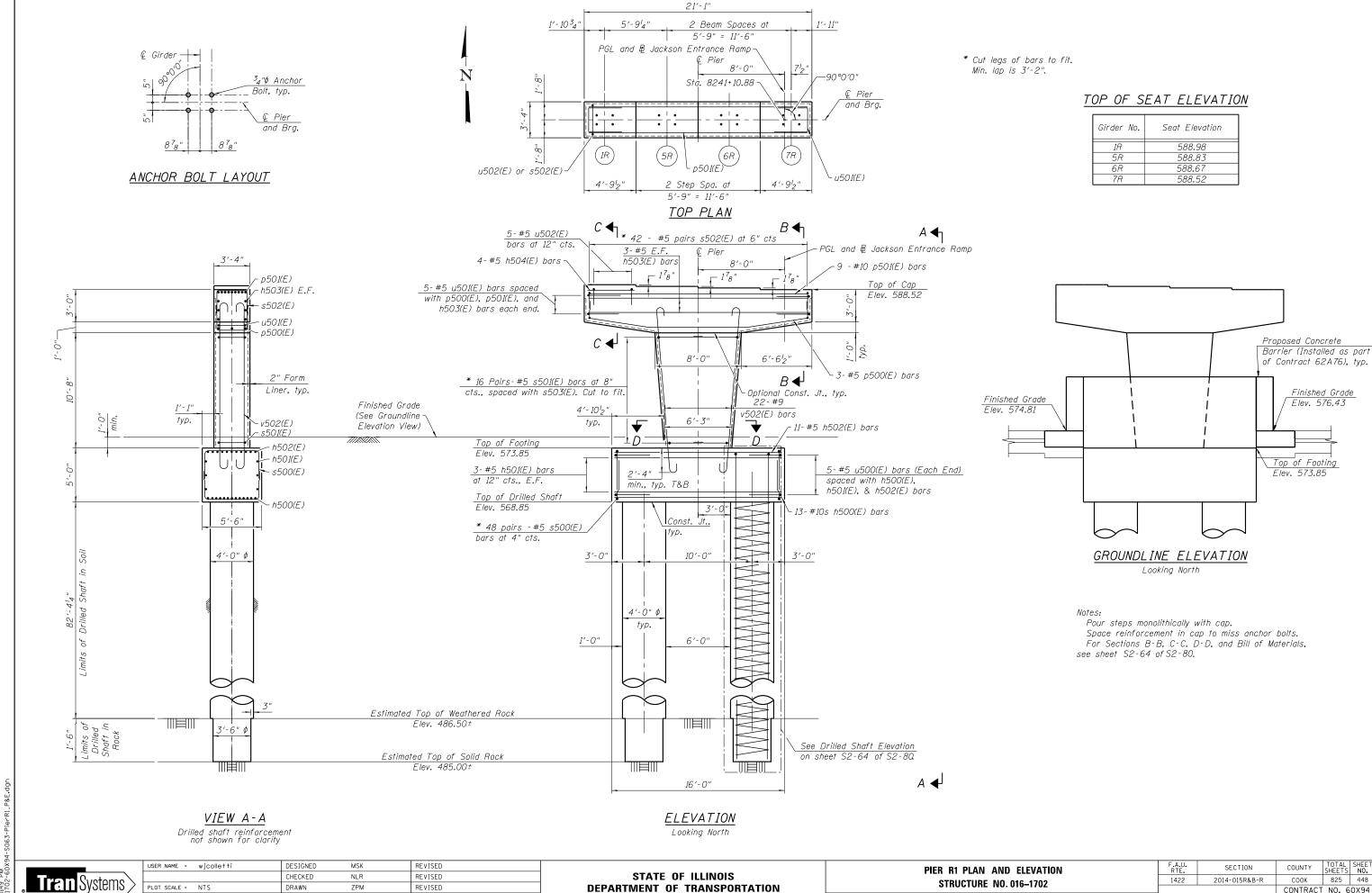
USER NAME = wjcolletti	DESIGNED	ZPM	REVISED
	CHECKED	NLR	REVISED
PLOT SCALE = NTS	DRAWN	ZPM	REVISED
PLOT DATE = 3/5/2020	CHECKED	NLR	REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION PIER 2 DETAILS
STRUCTURE NO. 016–1702

SHEET NO. \$2-61 OF \$2-80 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1422	2014-015R&B-R	COOK	825	446
		CONTRACT	NO. 6	0X94
	ILLINOIS FED. AI	ID PROJECT		





DEPARTMENT OF TRANSPORTATION

SHEET NO. S2-63 OF S2-80 SHEETS

CONTRACT NO. 60X94

DRAWN

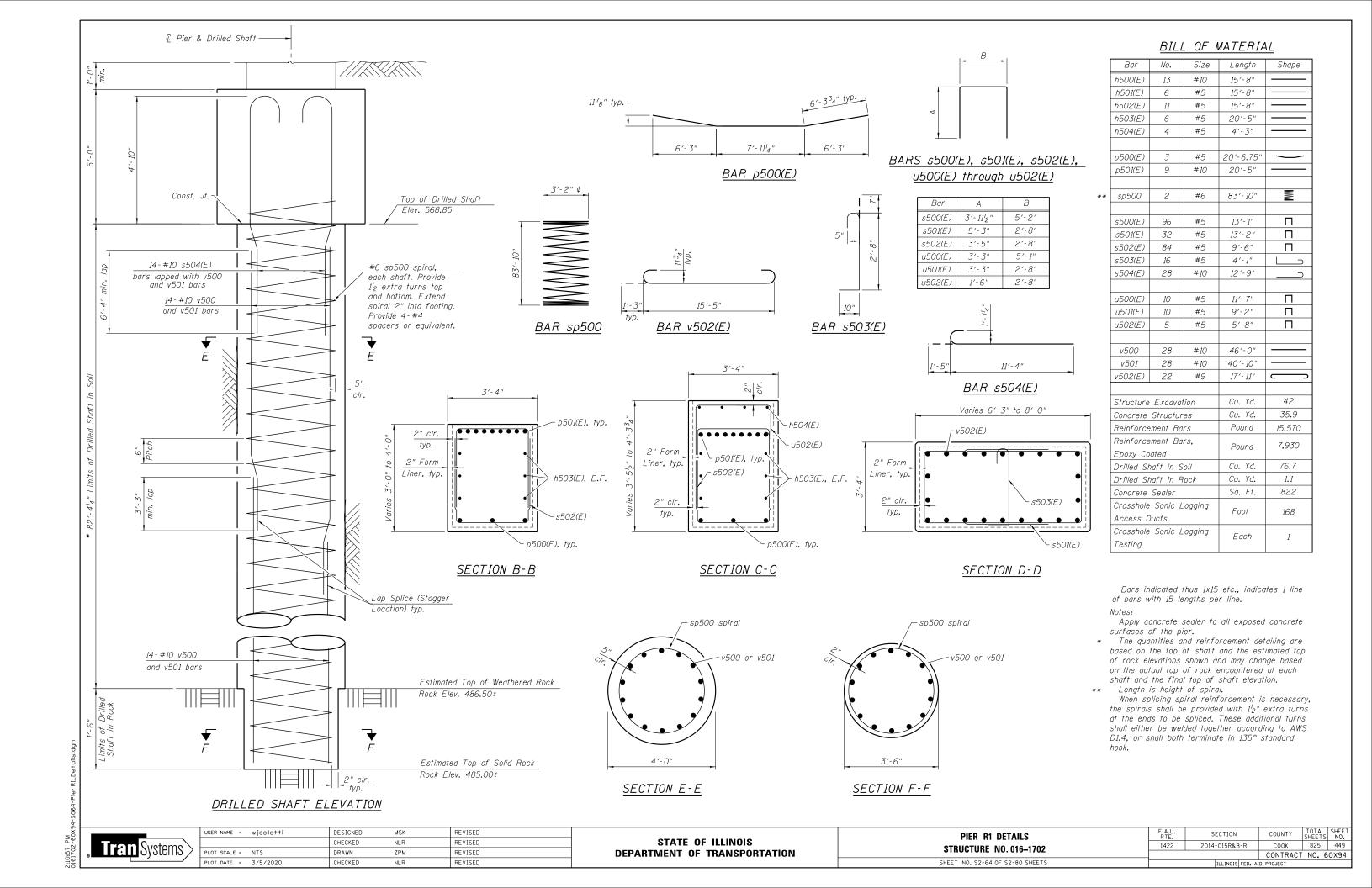
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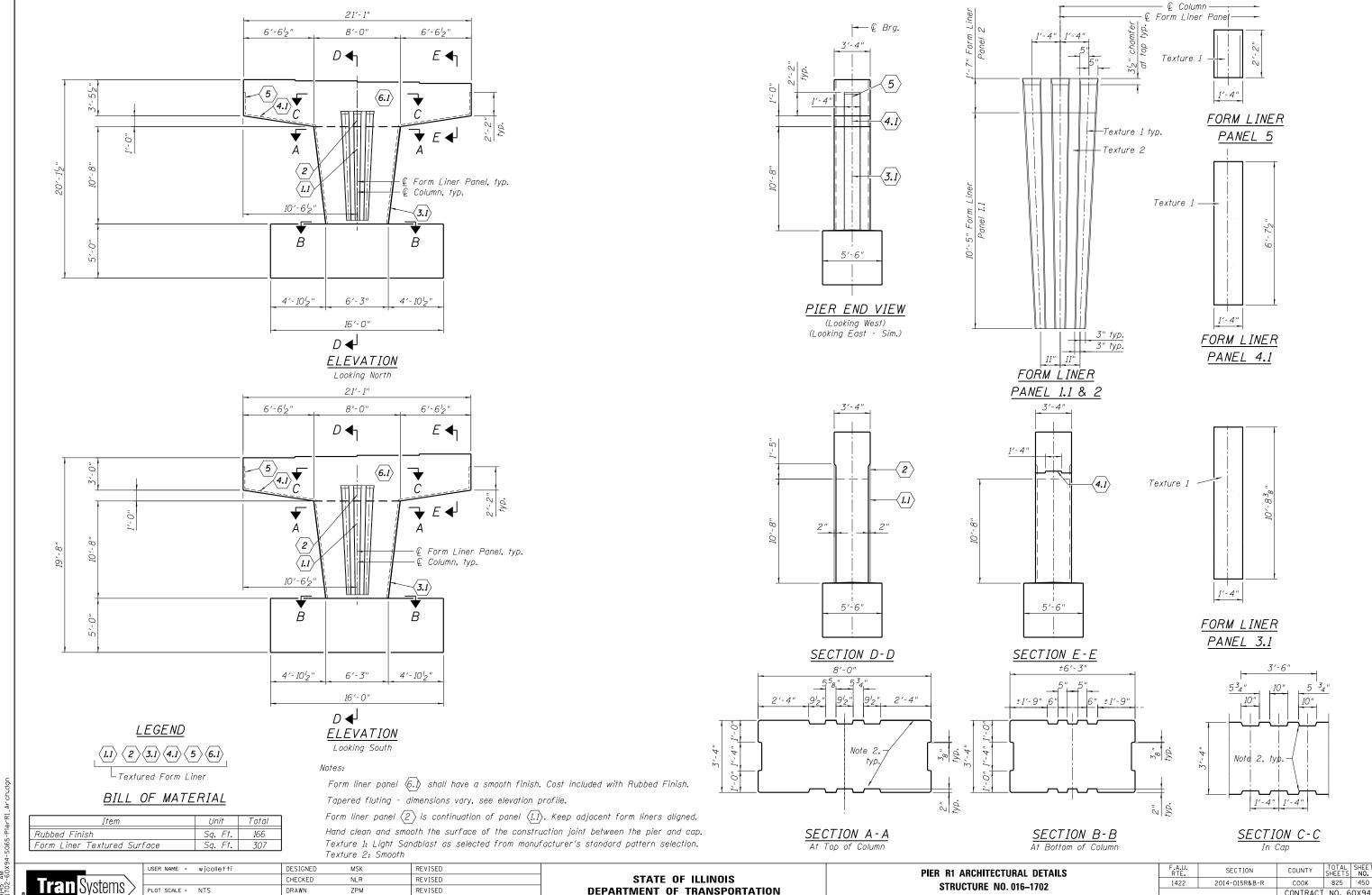
PLOT DATE = 3/5/2020

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DEPARTMENT OF TRANSPORTATION

SHEET NO. S2-65 OF S2-80 SHEETS

CONTRACT NO. 60X94

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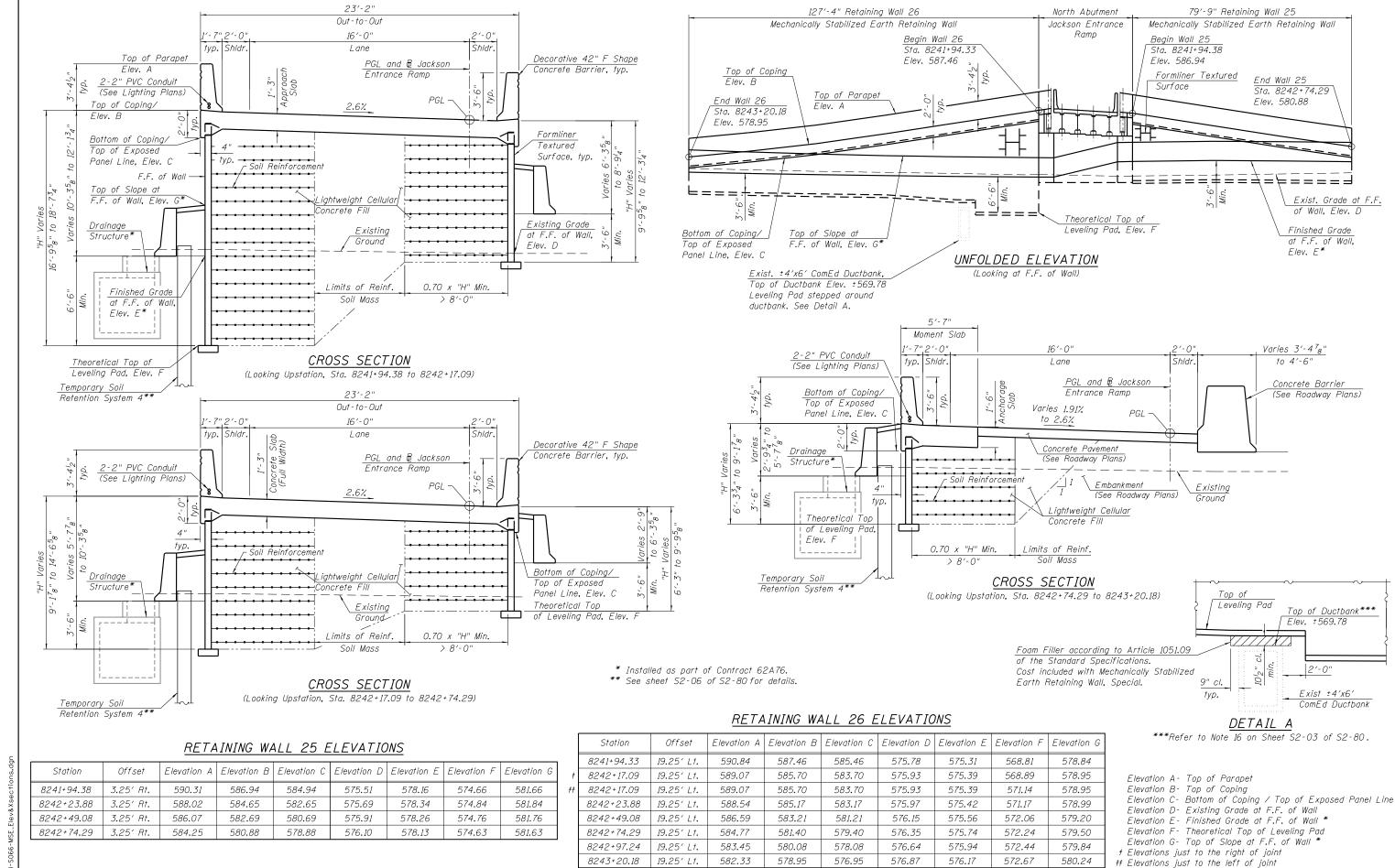
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PLOT DATE = 3/6/2020

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STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

SECTION

2014-015R&B-R

1422

MSE WALL ELEVATION AND CROSS SECTIONS

STRUCTURE NO. 016-1702

SHEET NO. S2-66 OF S2-80 SHEETS

COUNTY

COOK

825 451

CONTRACT NO. 60X94

Tran Systems

USER NAME = wjcolletti

PLOT DATE = 3/5/2020

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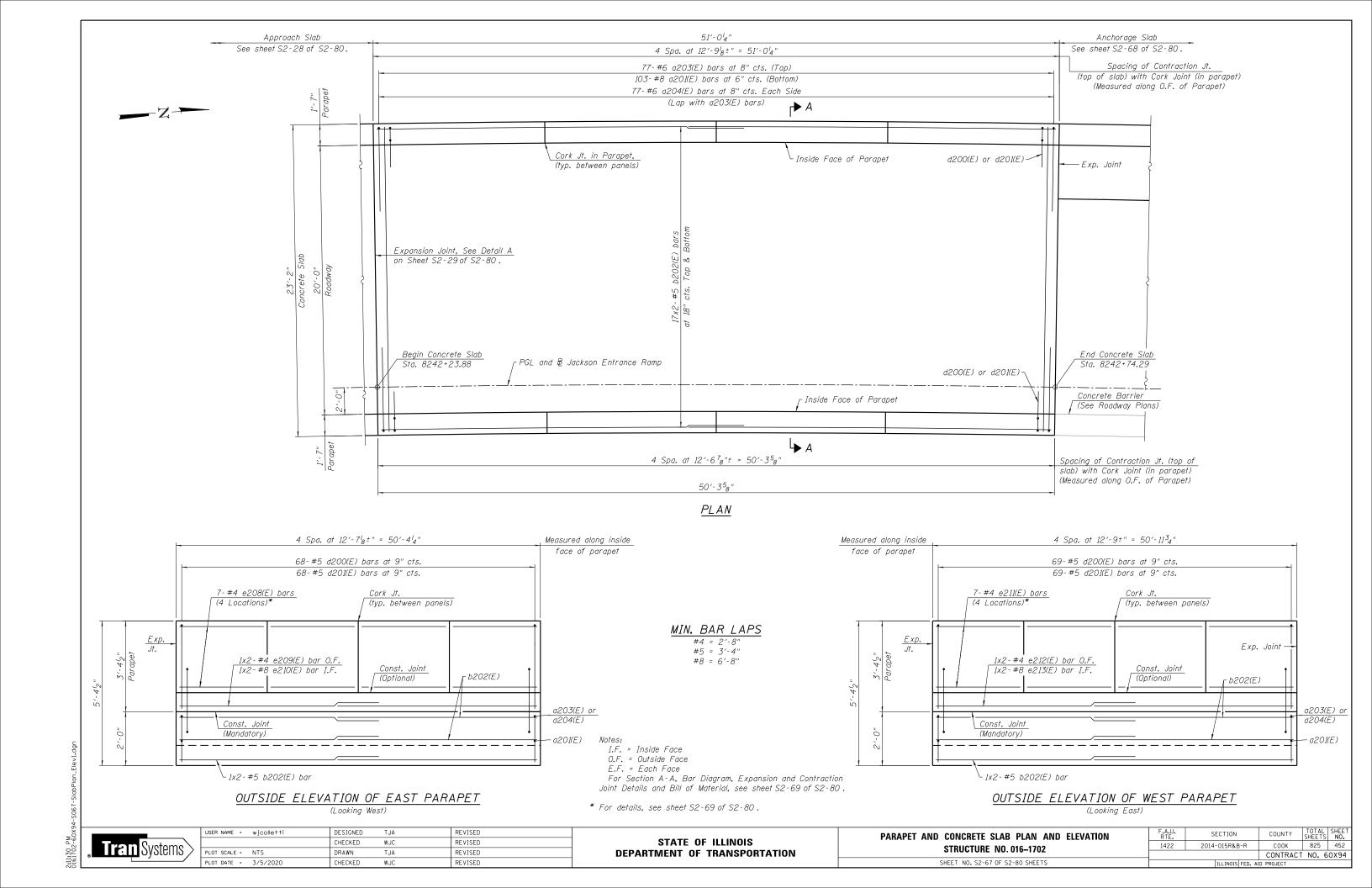
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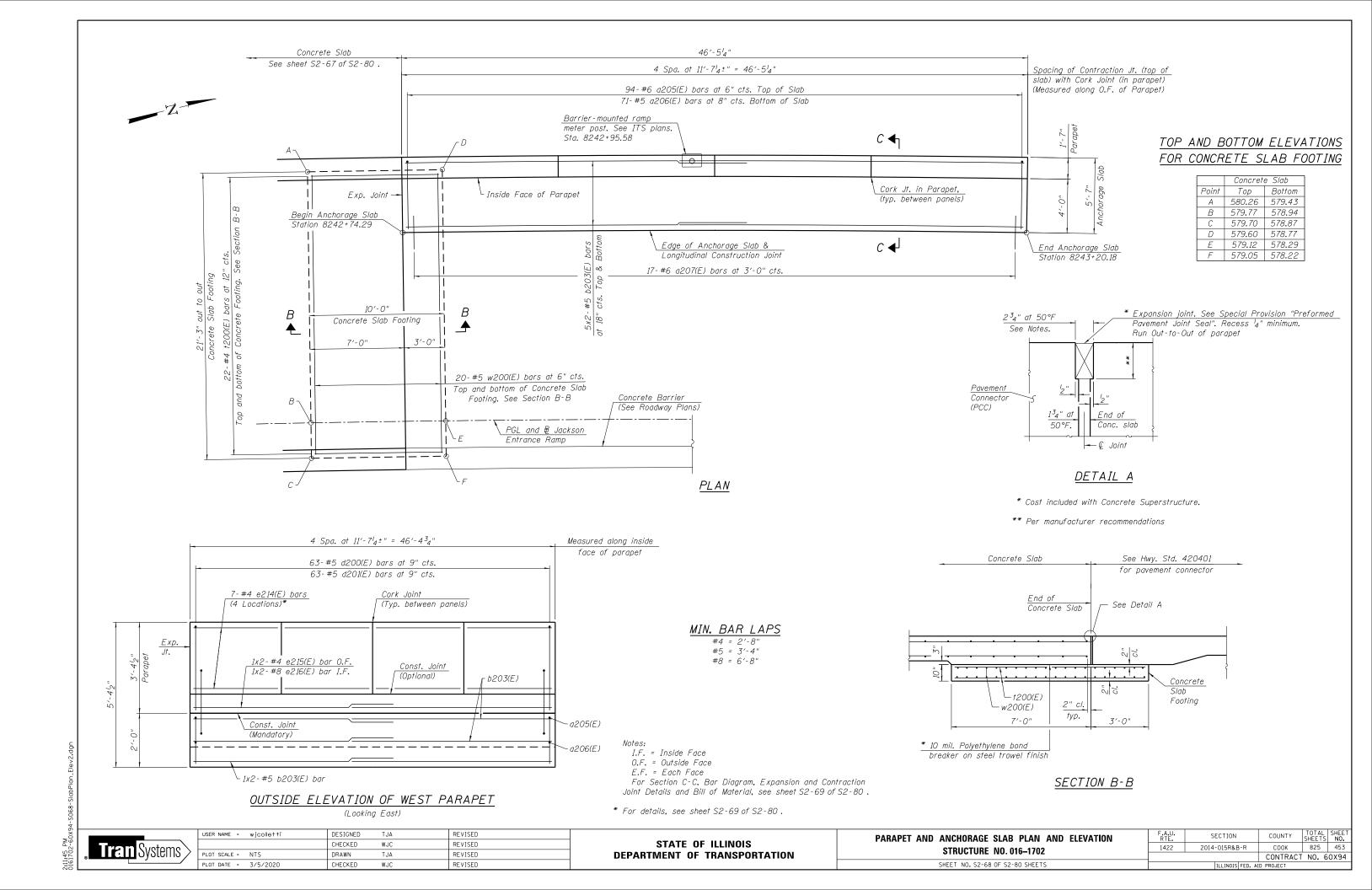
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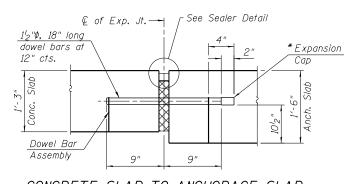
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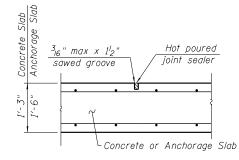




CONCRETE SLAB TO ANCHORAGE SLAB TRANSVERSE EXPANSION JOINT

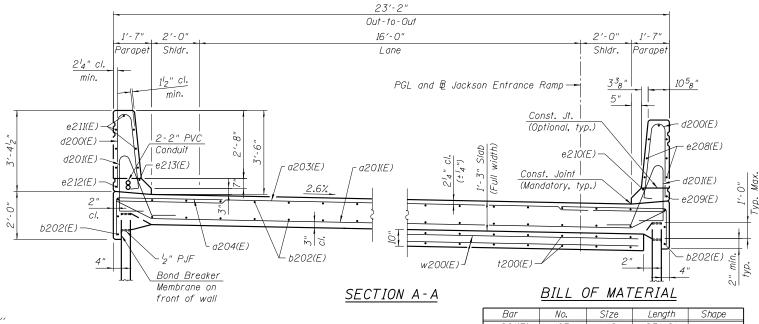
Expansion Joint Filler, Sealer and Dowel Bars included in cost of Concrete Superstructure.

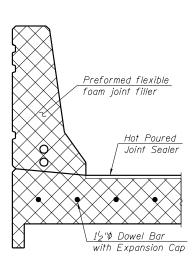
 st Expansion caps shall be installed on the exposed end of each dowel bar once header has been removed and the joint filler material has been installed,



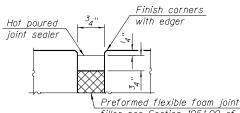
TRANSVERSE CONTRACTION JOINT

See Article 420.05 & 420.12 of the Standard Specifications



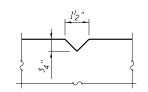


TRANSVERSE EXPANSION JOINT SECTION

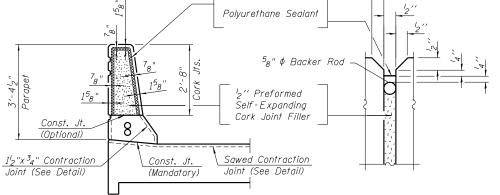


filler per Section 1051.09 of Standard Specifications

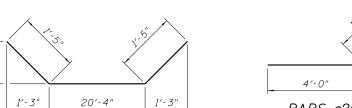
SEALER DETAIL



PARAPET TRANSVERSE CONTRACTION JOINT



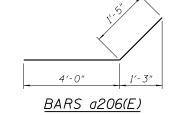
PARTIAL HEIGHT BARRIER JOINT AND CONTRACTION JOINT SECTION

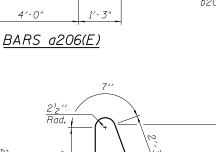


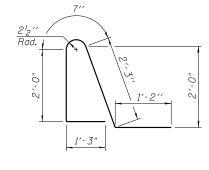
2 ³8′′ Rad.

BARS a201(E)

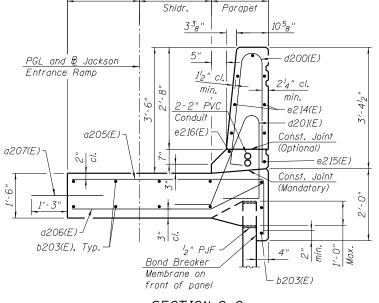
REVEAL DETAIL











5'-7" Anchorage Slab

2'-0"

1'-7"

2'-0"

SECTION	<u></u>	\sim
<u> 3L C / 10/V</u>	<u></u>	<u></u>

6'-6"	ı
5′-3"	
	1,-6"
	-

BARS a204(E) and a205(E)

Bar	No.	Size	Length	Shape
a201(E)	103	#8	23'-2"	
a203(E)	77	#6	22'-10"	
a204(E)	154	#6	8'-0"	Г
a205(E)	94	#6	6'-9"	Г
a206(E)	71	#5	5′-5"	
a207(E)	17	#6	2'-6"	
b202(E)	72	#5	27'-0"	
b203(E)	22	#5	24'-9"	
d200(E)	200	#5	6′-10"]
d201(E)	200	#5	7′-3"	
e208(E)	28	#4	12'-3"	
e209(E)	2	#4	26'-4"	
e210(E)	2	#8	28'-4"	
e211(E)	28	#4	12'-4"	
e212(E)	2	#4	26′-8"	
e213(E)	2	#8	28'-8"	
e214(E)	28	#4	11'-3"	
e215(E)	2	#4	24'-5"	
e216(E)	2	#8	26′-5"	
†200(E)	44	#4	9′-8"	
w200(E)	40	#5	20'-11"	
Structure i			Cu. Yd.	333
Concrete S			Cu. Yd.	6.6
Concrete S		ture	Cu. Yd.	90.6
Protective			Sq. Yd.	210
Reinforcem			Pound	20,190
Epoxy Coa			6 5	1 711
Concrete Sealer			Sq. Ft.	1,311
Lightweight Concrete			Cu. Yd.	746
Cellular Fill Bridge Deck Grooving			C V/	177
_	-	7	Sq. Yd.	133
(Longitudin		1 =41	C - E'	1.755
Mechanicall	,		Sq. Ft.	1,755
Retaining V	vaii, Speci	ai .		

Notes:

All edges shall be chamfered $\frac{3}{4}$ inches.

Protective coat shall be applied to the parapet top and interior vertical surface above ground line and top face of concrete and anchorage slab.

Bars indicated thus 3x4-#5 etc. indicates 3 lines of bars with 4 lengths per line.

See Sheet S2-03 of S2-80 for additional notes for MSE wall suppliers.

The Polyurethane Sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray. 1/2" PJF is included in the cost of Concrete Superstructure

COUNTY

COOK 825 454

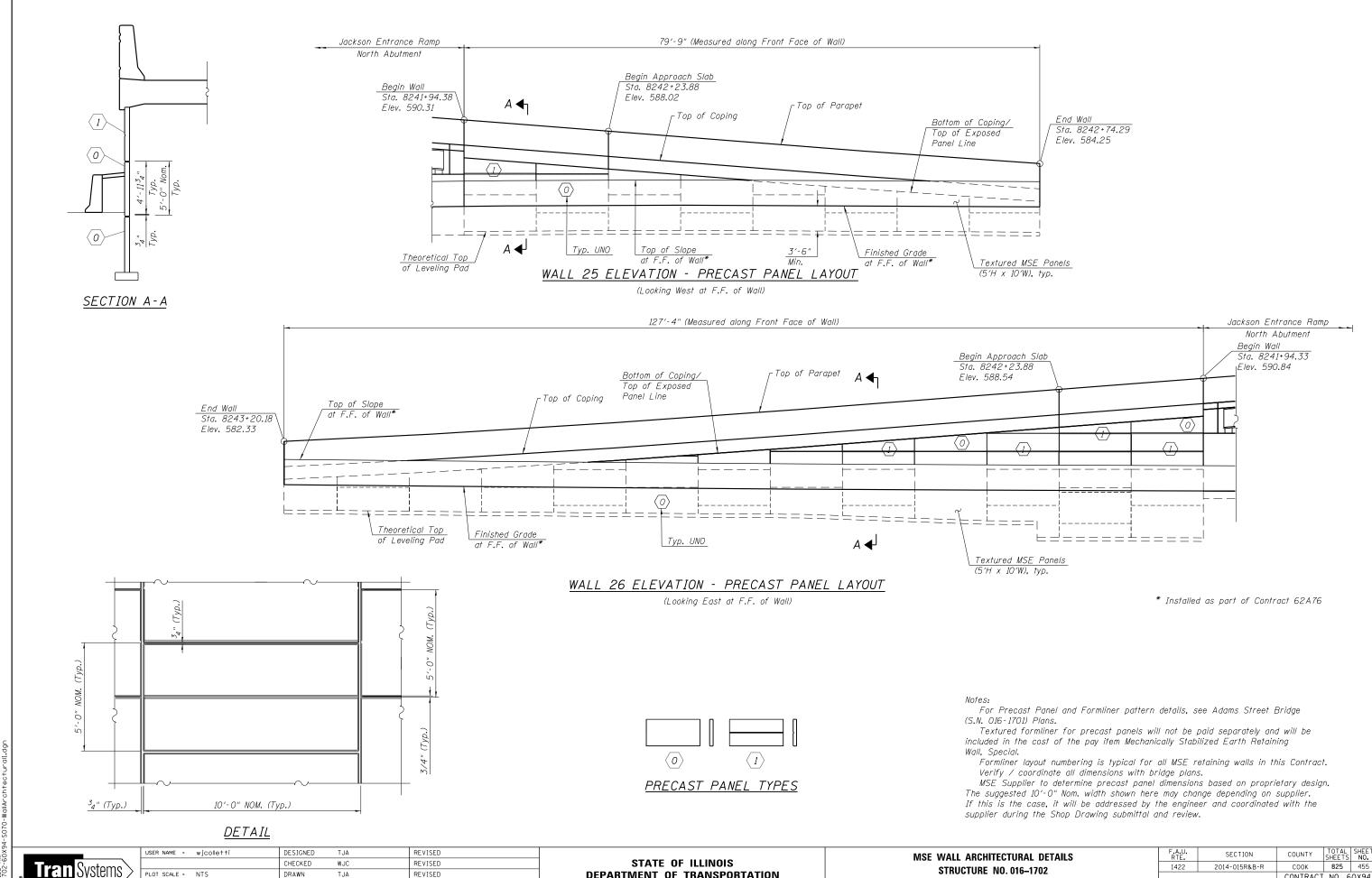
CONTRACT NO. 60X94



	USER NAME =	wjcolletti	DESIGNED	TJA	REVISED
			CHECKED	WJC	REVISED
	PLOT SCALE =	NTS	DRAWN	TJA	REVISED
	PLOT DATE =	3/5/2020	CHECKED	WJC	REVISED
7					

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

2		000: 0: 00::0:
PARAPET, CONCRETE, AND ANCHORAGE SLAB DETAILS	F.A.U. RTE.	SECTION
STRUCTURE NO. 016-1702	1422	2014-015R&B-
SHEET NO S2-69 OF S2-80 SHEETS		TI I INOIS I



DEPARTMENT OF TRANSPORTATION

STRUCTURE NO. 016-1702

SHEET NO. S2-70 OF S2-80 SHEETS

CONTRACT NO. 60X94

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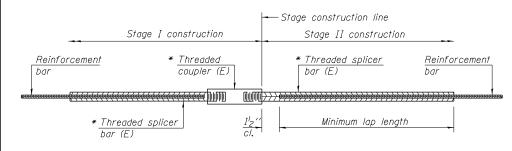
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PLOT DATE = 3/6/2020

TJA

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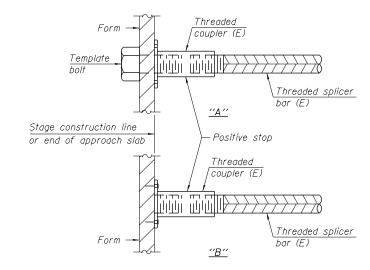


STANDARD BAR SPLICER ASSEMBLY

Threaded splicer bar length = min. lap length + 1_2^{l} " + thread length

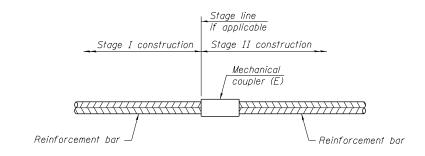
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Minimum lap length



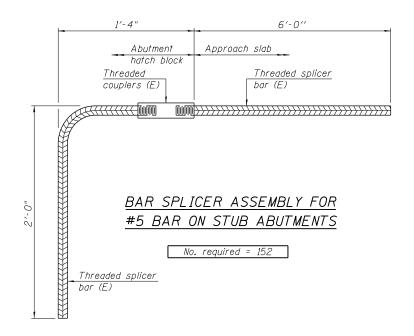
INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar	No. assemblies
Locarion	size	required
SE Wingwall Drilled Shaft 1	14	24
SE Wingwall Drilled Shaft 2	14	24
SE Wingwall Drilled Shaft 3	14	24
SE Wingwall Drilled Shaft 4	14	24
SE Wingwall Drilled Shaft 5	14	24
SE Wingwall Drilled Shaft 6	14	24



Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

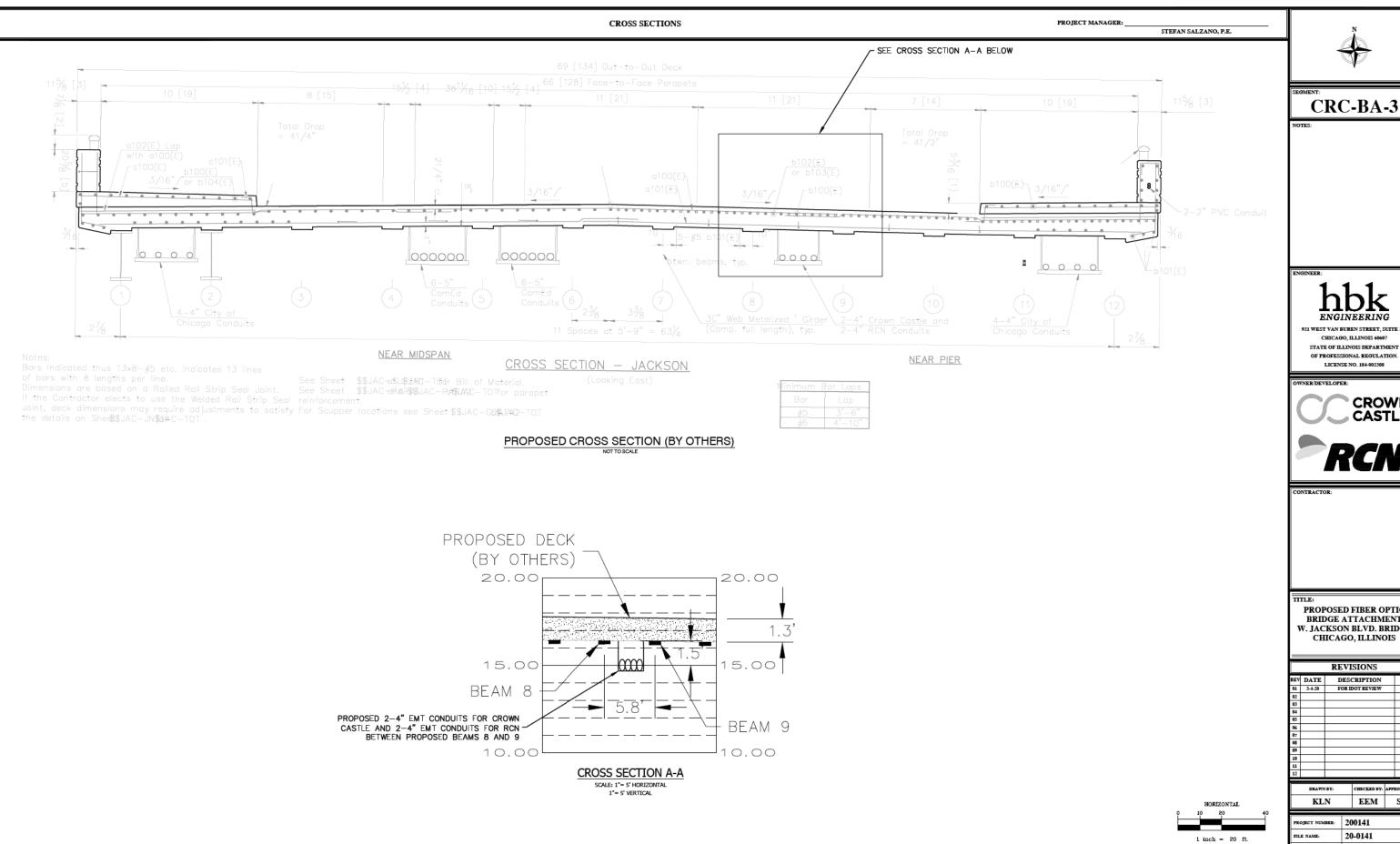
Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.



	USER NAME = wjcolletti	DESIGNED	WJC	REVISED	
		CHECKED	MDS	REVISED	
<i>></i> [PLOT SCALE = NTS	DRAWN	JTF	REVISED	
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS	F.A.U. RTE.	SI	ECTION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-1702	1422	2014-	-015R&B-	R	соок	825	456
31110010HL 140.010-1702					CONTRACT	NO. 6	0X94
SHEET NO. S2-71 OF S2-80 SHEETS			ILLINOIS F	ED. AID	PROJECT		



921 WEST VAN BUREN STREET, SUITE 100 CHICAGO, ILLINOIS 60607 STATE OF ILLINOIS DEPARTMENT OF PROFESSIONAL REGULATION. LICENSE NO. 184-002308





PROPOSED FIBER OPTIC BRIDGE ATTACHMENT W. JACKSON BLVD. BRIDGE CHICAGO, ILLINOIS

REVISIONS										
REV	DATE	DESC	CRIPTION		BY					
01	3-4-20	FOR I	OOT REVIEW		KLN					
02										
03										
04										
05										
96										
07										
80										
09										
10										
11										
12										
	DRAWN	BY:	CHECKED BY:	APPI	ROVED BY:					
KLN			EEM		SS					

KLN		EEM	SS
PROJECT NUMBER:	20	0141	
FILE NAME:	20	-0141	
OUC NUMBER:	N	/A.	
DATE DRAWN:	03	3-04-2020	
SCALE:	A	S SHOW	N

S2-72 OF S2-80 (457 OF 825)

1 inch = 5 ft.



CRC-BA-3



CHICAGO, ILLINOIS 6060

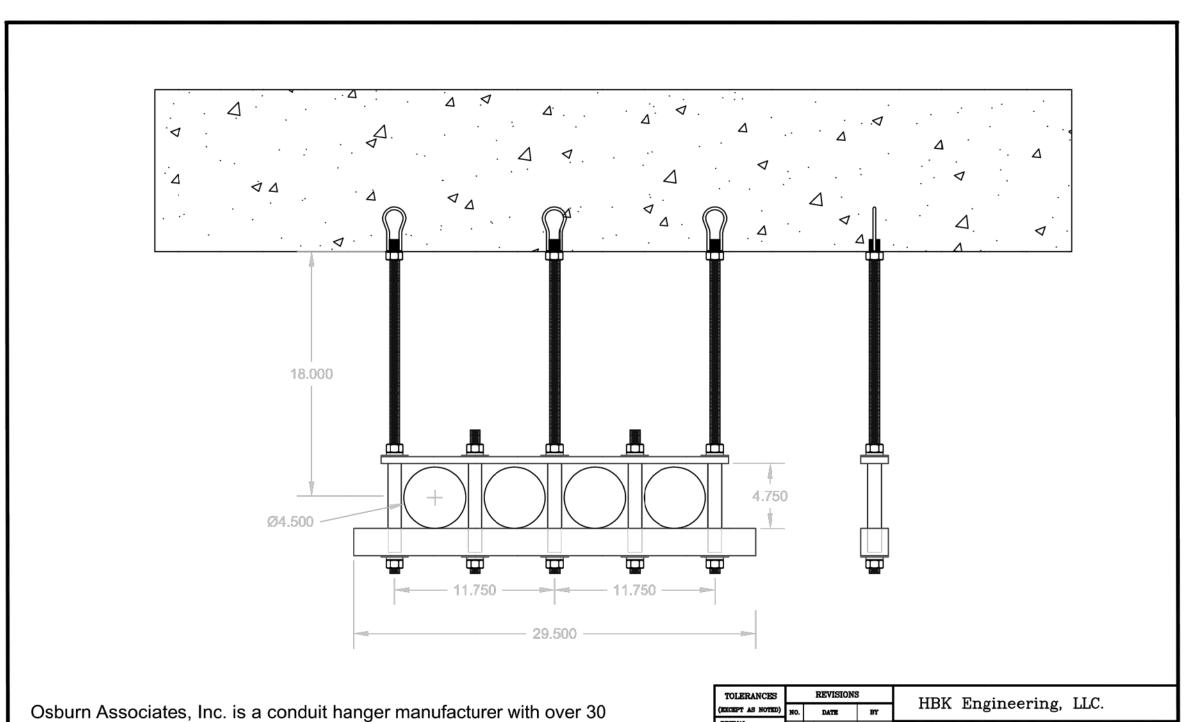




	REVISIONS													
REV	DATE	DATE DESCRIPTION												
01	3-4-20	FOR IDOT REVIEW	KLN											
02														
03														
04														
05														
06														
07														
80														
09														
10														
11														
12														

DRAWN BY:		CHECKED BY:	APPROVED BY
KLN		EEM	SS
PROJECT NUMBER:	20	00141	
FILE NAME:	20)-0141	
OUC NUMBER:	N	/ A	
DATE DRAWN:	03	3-04-2020	
SCALE:	N	.T.S.	

PROPOSED FIBER OPTIC BRIDGE ATTACHMENT W. JACKSON BLVD. BRIDGE



Osburn Associates, Inc. is a conduit hanger manufacturer with over 30 years experience. We are supplying these drawings for expansion joint layouts. If these need to be stamped by an engineer, we are unable to provide that service.

TOLERANCES		REVISIONS	3	прк г	HDV Engineering IIC								
(EXCEPT AS NOTED)	NO.	DATE	BY	HDK EII	HBK Engineering, LLC.								
DECIMAL	1			OSBU	RN ASSOCIATE	S INC.							
FRACTIONAL	2				P.O. Box 912 Logan, Ohio 43	138							
+ 1/16"	8			DRAWN BY	SCALE	MATERIAL							
ANGULAR	4			CHK,D	DATE 2/20/20	DRAWING NO.							
	5			TRACED	APP'D	1							



EGMENT:

CRC-BA-3

NOTES

ENGINEER:



921 WEST VAN BUREN STREET, SUITE CHICAGO, ILLINOIS 60607 STATE OF ILLINOIS DEPARTMENT OF PROFESSIONAL REGULATION. LICENSE NO. 184-002308

OWNER/DEVELOPER:





CONTRACT

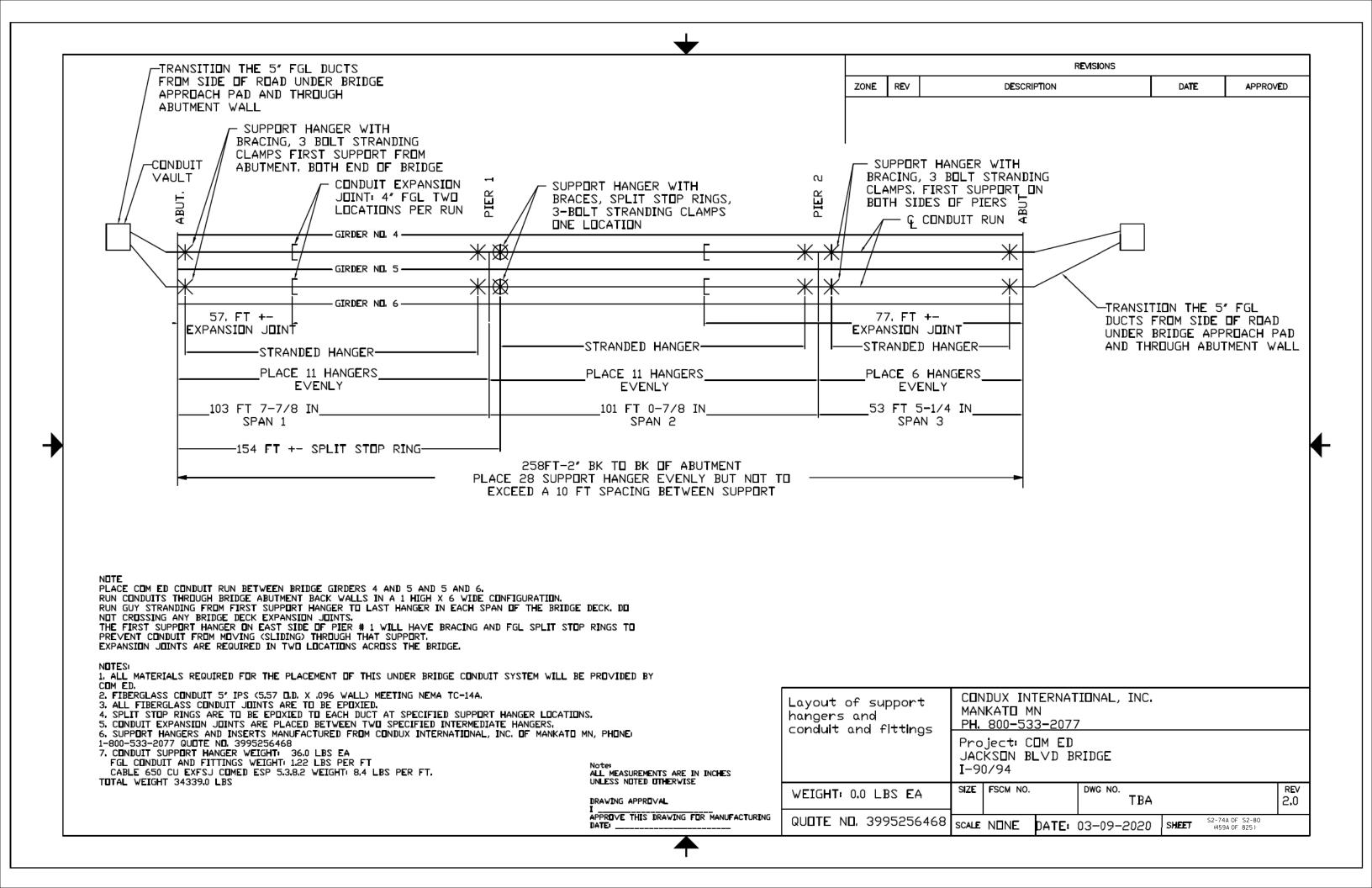
TITLE:

PROPOSED FIBER OPTIC BRIDGE ATTACHMENT W. JACKSON BLVD. BRIDGE CHICAGO, ILLINOIS

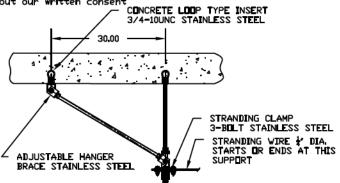
	REVISIONS													
REV	REV DATE DESCRIPTION													
01	3-4-20	FOR IDOT REVIEW	KLN											
02														
03														
04														
05														
06														
07														
80														
09														
10														
11														
12														

KLN		EEM	SS S
PROJECT NUMBER:	20	00141	
FILE NAME:	20)-0141	
OUC NUMBER:	N	/A.	
DATE DRAWN:	03	3-04-2020	
SCALE:	N	.T.S.	

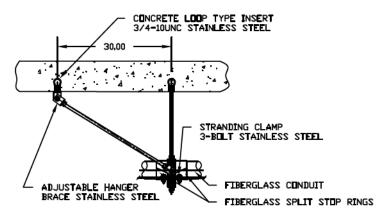
S2-74 OF S2-80 (459 OF 825)



This arawing is the property of Condux International, Inc. and the information thereon is to be treated as confidential. It is not to be used, copied or alsolosed to outside parties without our written consent



STANDARD SUPPORT HANGER WITH BRACING, 3-BOLT STRANDING CLAMPS. LOCATIONS FIRST HANGER INSIDE EACH ABUTMENT AND FIRST HANGER ON BOTH SIDE OF PIERS



STANDARD SUPPORT HANGER WITH BRACING, FGL SPLIT STOP RINGS AND 3-BOLT CLAMPS 1 LOCATION
 ZONE
 REV
 DESCRIPTION
 DATE
 APPROVED

BILL OF MATERIAL

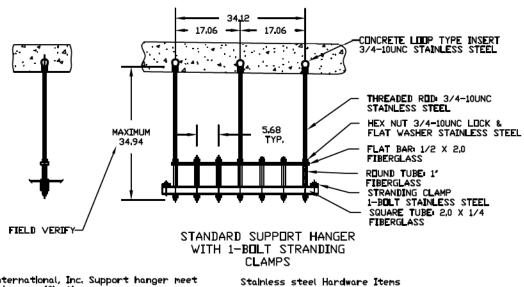
ITEM NO.	PART NO.	DESCRIPTION	QTY	UNIT
1	TBA	STANDARD CENDUTT SUPPERT MANGER 1 MICH X 6 VIDE FIBERGLASS AND STADLESS STEEL, EPENDING FER TVELVE 5' FGL DUCTS THREADED REDS THREE @ 3/4-LOUNC X 36.00 LENG	56	EA.
2	08610242	HANGER BRACE ADJUSTABLE: 42 INCH STAINLESS STEEL	24	EA.
3	08409990	CONCRETE INSERT: 3/4-10 LOOP TYPE, STAINLESS STEEL	192	EA.
4	08558300	CONCRETE INSERT SETTING PLUG 3/4-10	192	EΑ
5				
6	08408902	STRANDING VIRE 1/4 X 350 FT., STAINLESS STEEL	4	EA.
7	08409404	GUY STRAND CLAMP: 3-BOLT, STAINLESS STEEL	24	EA.
8	08409504	GUY STRAND CLAMP: 1-BOLT, STAINLESS STEEL	88	EA.
9	08460053	CONDUIT FIBERGLASS: 5' IPS, MV (5.57 DLD. X .096 VALL) MEETING NEMA TC-14A	3360	FT.
10	08460153	CONDUIT STOP COUPLING 5' IPS MV	24	EA.
11	08460453	CONDUIT EXPANSION JOINT O-RING TYPE 5' IPS MV	24	EA.
12	08460953	CONDUIT SPLIT STOP RING 5' IPS MV	24	EA.
13	08461553	CONDUIT ADAPTER: 5' IPS MV TO 5' GRC	24	EA.
14	08463402	CONDUIT EPOXY ADHESIVE CARTRIDGE	35	EA.
15	02288990	CONDUIT EPDXY ADHESIVE GUN	1	EA.

General Construction, Hanger and Conduit Notes
1.0 Recommended spacing between Support is 10 foot.
2.0 Support Hanger Material shall be manufactured using 316 stainless steel and fiberglass components.
3.0 Conduit is 5 inch Fiberglass with minimum wall thickness of .096 inch meeting NEMA TC-14A Specs.

4.0 Conduit joints shall be positive locking adhesive bonded bell and spigot.
5.0 Conduit expansion joints shall be sliding sleeve with provision for 8

inch of travel.

6.0 Bridge abutments must have a block out or be sleeved to allow
the fiberglass conduit to pass through. After conduit is placed
through abutment seal up opening with state approved sealant.



Condux International, Inc. Support hanger meet the following specifications
Fiberglass items
Flat Barı ½ × 2.0
Round Tube: 1.0° 0.0. .105 wall
Square Tube: 2 × 2 × 1/4
Fiberglass representation with polyester resin with

Fiberglass reinforced with polyester resin with surface vell for better weathering, resin shall contains u.v. inhibitor. Fiberglass is made with continuous strand mat and uni-directional roving, gray in color

gray in color
Tensile Strength (ASTM D 638) 30,000 PSI
Tensile Modules (ASTM D 638) 2,3E6 PSI
Flexural Strength (ASTM D 790) 30,000 PSI
Flexural Modules (ASTM D 790) 2,3E6 PSI
Compressive Strength (ASTM D 695 20,000 PSI
Compressive Modules 1.4E6 PSI
Yield shear strength 2000 PSI
Barcol hardness 50
Dielectric strength (ASTM D 149) 200 VPM Min.

Threaded Rod
Threaded rod meets (ASTM/ASME B1.1)
(ASTM A307 Grade A) (Tensile Strength 60,000 PSI)
Hexnut
Hexnut meets (ANSI/ASME B18.2.2)
Materiali 316 Stainless steel (ASTM F594)
Flatwasher
Flatwasher meets (ANSI/ASME B18.22.1)
Materiali 316 stainless steel (ASTM F436)
Lockwasher
Lockwasher meets (ANSI/ASME B18.21.1)
Materiali 316 Stainless steel (ASTM F436)
Stranding Items
Stranding Clamps (1-Bolt & 3-Bolt)
Materiali 1/4 x 1.5 (316 Stainless Steel)
Stranding Wire
Cablei ½/ Dia (7 X 19 Steel Aircraft)
304 Stainless Steel
Bracing

Adjustable hanger attachment brackets Materiali angle 2.5 x 2.5 x .25 (316 Stainlss steel)

69.00 **—** 34.50 34,50 17.06 --17.06 17.06 -17.06 44 .4 .4 MAXIMUM MAXIMUM 34.94 34.94 FIELD VERIFY-FIELD VERIFY NO PART OF THE CONDUIT SUPPORT HANGER CAN EXTEND BELOW THE BRIDGE GIRDER

CONDUIT SUPPORT
HANGER DETAIL AND
BILL OF MATERIALS

EXTENDED RODS MAY NEED TO BE ALTERED

CONDUX INTERNATIONAL, INC. MANKATO MN PH. 800-533-2077

DWG NO.

Project: Com Ed JACKSON BLVD BRIDGE I-90/94

WEIGHT: 0.0 LBS EA

QUOTE NO. 3995256468 SCALE 1/2

SIZE FSCM NO.

TBA

DATE: 03-09-2020 SHEET S2-74B OF 8. 459B OF 8.

REV

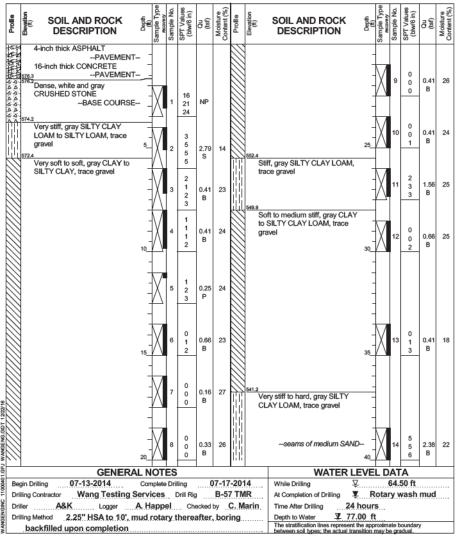
DRAWING APPROVAL

APPROVE THIS DRAWING FOR MANUFACTURING

Note: ALL MEASUREMENTS ARE IN INCHES UNLESS NOTED OTHERWISE

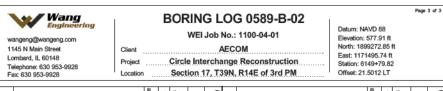
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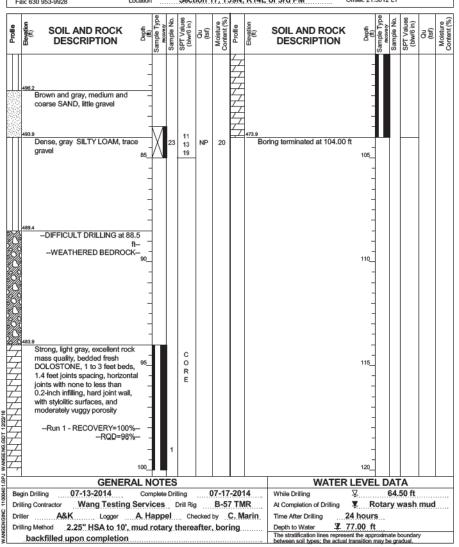




\ ₩₩ Wang	BORING LOG 0589-B-02		Page 2 of
Engineering	DOMINO 200 0303-D-02	Datum: NAVD 88	
wangeng@wangeng.com	WEI Job No.: 1100-04-01	Elevation: 577.91 ft	
145 N Main Street	Client AECOM	North: 1899272.85 ft	
ombard, IL 60148 elephone: 630 953-9928	Project Circle Interchange Reconstruction	East: 1171495.74 ft Station: 6149+79.82	
eiepnone: 630 953-9928 Fax: 630 953-9928	Location Section 17, T39N, R14E of 3rd PM	Offset: 21.5012 LT	

Second S		none: 630 953-9928 30 953-9928	Location								of 3rd PM	Station: 6' Offset: 21			?		
15 5 5 5 5 5 5 5 5 5	Profile Elevation	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)			Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
gravel 16 5 7 2.30 22 22 23 22 24 23 24 22 24 22 24 24			45	X	15			13	0.00	Q 611.2		-		19	4	NΡ	13
GENERAL NOTES Begin Drilling 07-13-2014 Complete Drilling 07-17-2014 Orilling Contractor Wang Testing Services Drill Rig B-57 TMR Driller A&K Logger A Happel Chekedby C. Marin Time After Drilling 24 hours Dental Rig D			50	X	16	7		22		gra	vel	70_	X	20	14	NP	14
GRAVEL 18 3 NP 13 Dense, gray SiLTY LOAM, trace gravel		2 Medium dense, grav SILTY	55	X	17	5		23		gra	ivel	-	X	21	21	NP	13
Drilling Contractor Wang Testing Services Drill Rig B-57 TMR At Completion of Drilling ▼ Rotary wash mud Drilling Method 2.25" HSA to 10", mud rotary thereafter, boring Depth to Water ▼ 77.00 ft	1,4570 WANGEING,GEUT 1,222/10	LOAM, trace gravel	-	ОТЕ		5 5	NP	13	0	GF 498.9 De	nse, gray SILTY LOAM	80_	X L D		19 21	NP	21
Driller A&K Logger A, Happel Checked by C, Marin Time After Drilling 24 hours Drilling Method 2.25" HSA to 10", mud rotary thereafter, boring Depth to Water ₹ 77.00 ft	5	Orilling 07-13-2014	Comp	olete	Drill	ling					While Drilling	₹	!	64.	50 ft		
packfilled upon completion	Driller Drilling	A&K Logger	A. Ha	ppe	el	Che	ecked	by .	C. N	larin.	Time After Drilling Depth to Water	24 hour 77.00 f	s				

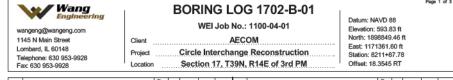


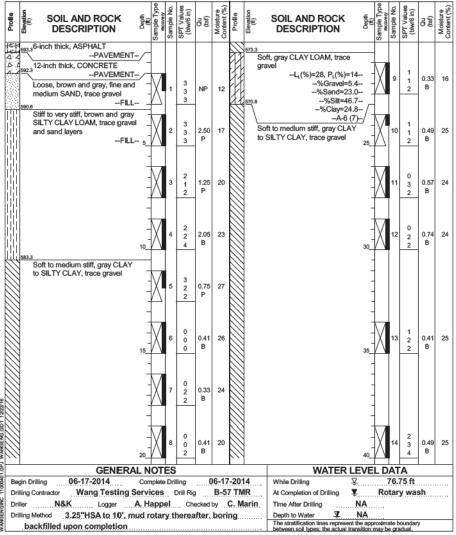


Boring Log 0589-B-02 station and offset along & Jackson Blvd. is: Sta. 8313+34.94, Offset 61.17' Rt.



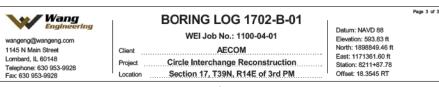
USER NAME = wjcolletti	DESIGNED	TLR	REVISED	
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JTF	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

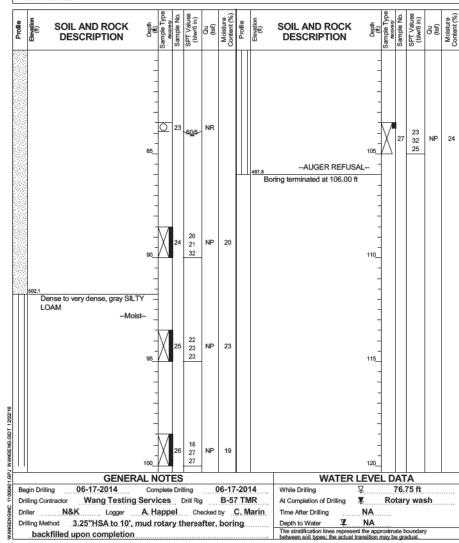




₩ Wang		BORING LOG 1702-B-01	Po	age 2
Engineering		BORING LOG 1702-B-01	Datum: NAVD 88	
wangeng@wangeng.com		WEI Job No.: 1100-04-01	Elevation: 593.83 ft	
1145 N Main Street	Client	AECOM	North: 1898849.46 ft	
Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9928	Project Location	Circle Interchange Reconstruction Section 17, T39N, R14E of 3rd PM	East: 1171361.60 ft Station: 8211+87.78 Offset: 18.3545 RT	

	elephone: 630 953-9928 ax: 630 953-9928	Location	5	Secti	on 1	7, T3	39N,	R14E	of 3rd PM	Offset: 18				
Profile	SOIL AND R		Sample No.	SPT Values (blw/6 in)	Ou (Ist)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROO DESCRIPTION		Sample Type	Sample No.	(hw/6 in)	Moisture Content (%)
	Stiff, gray SILTY CLA SILTY LOAM, trace of									- - -				
		45	15	3 4 6	1.39 B	17				65_	X	19	5 7 14	5 22
			16	2 4 3	1.39 B	16			dium stiff to very stiff, f AY, trace gravel	gray -	X	20	5 7 8	1 24
	Soft to stiff, gray SIL1 trace gravel	Y CLAY,	17	4 6 6	0.49 B	21				75_	X	21	3 4 0.8 5 B	2 30
MANGENGINC 11000/01/6PJ WANGENS/GDT 12/22/18		60	18	5 9 17	1.00 P	22		gra	dium dense to very de y, fine SAND and SIL` ilnations		X	22	10 7 NF	22
01.GP.		SENERAL NOT								R LEVE				
90 Be	egin Drilling 06-17-20					6-17 B. 5			While Drilling		7			
S D	illing Contractor Wang iller N&K	Testing Services A. Happ		_					At Completion of Drilling Time After Drilling	g <u>¥</u> NA	RO	ary	wash	
Dr Big		to 10', mud rotar							Depth to Water	Z NA				
W AN	backfilled upon con								The stratification lines rep between soil types; the ar	present the app ctual transition	roxima may be	te bou gradu	ndary al.	

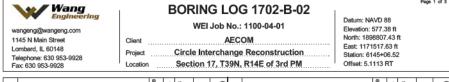


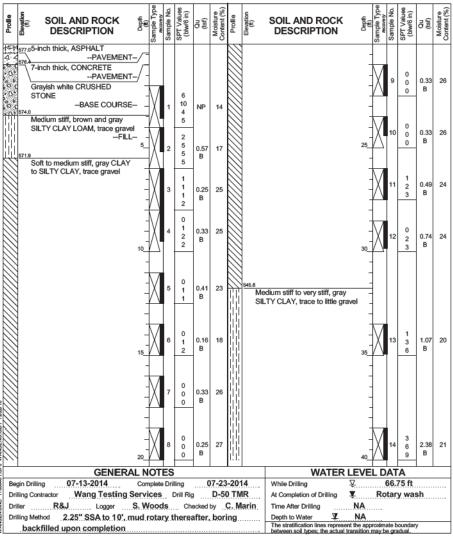


Boring Log 1702-B-01 station and offset are measured along & Jackson Blvd.

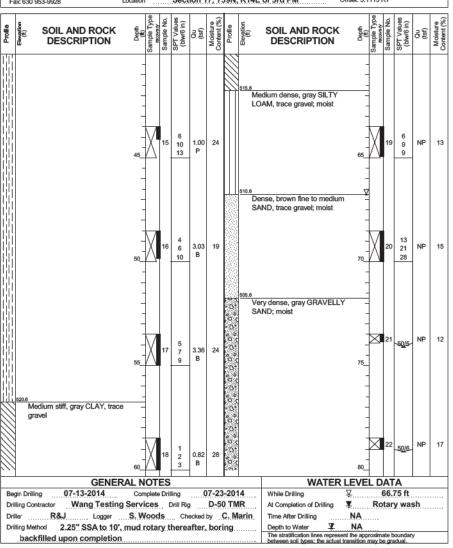


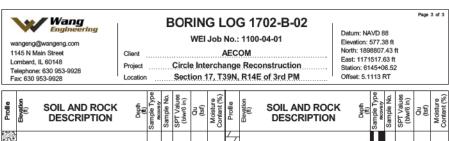
USER NAME = wjcolletti	DESIGNED	TLR	REVISED	
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JTF	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

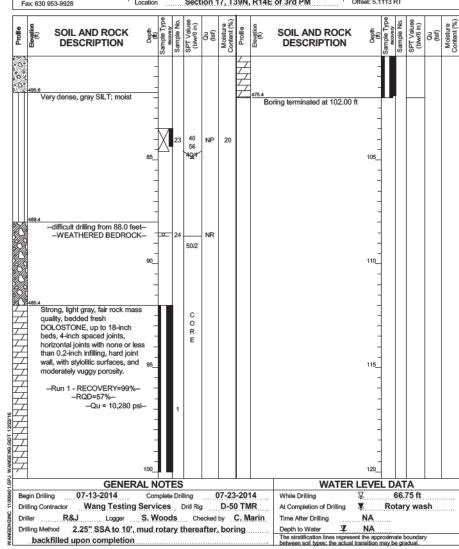










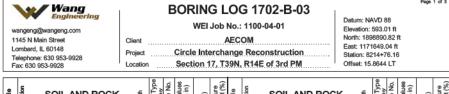


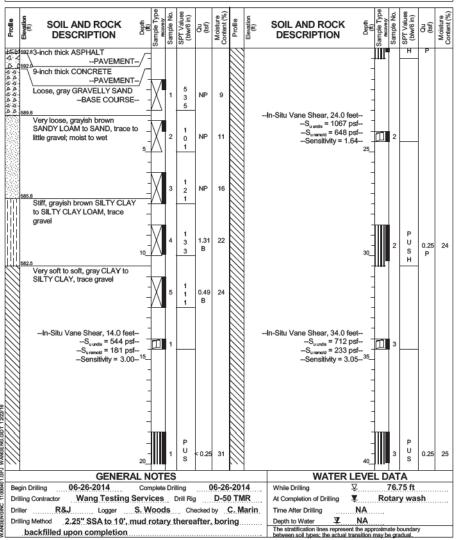
Boring Log 1702-B-02 station and offset along ₱ Jackson Blvd. is: Sta. 8213+42.72, Offset 63.99' Rt.

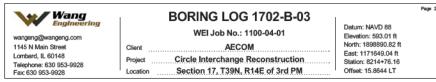


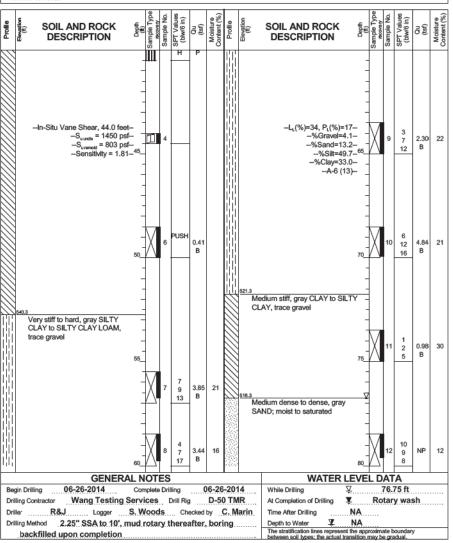
	USER NAME = wjcolletti	DESIGNED	TLR	REVISED	Г
		CHECKED	WJC	REVISED	
•	PLOT SCALE = NTS	DRAWN	JTF	REVISED	
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

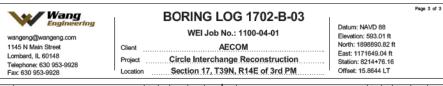
BORING LOGS 3	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
STRUCTURE NO. 016-1702	1422	2014-015R&B-R	COOK	825	462
31110C1011E 1NO. 010-1702			CONTRACT	NO. 6	50X94
SHEET NO. S2-77 OF S2-80 SHEETS		ILLINOIS FED. AI	D PROJECT		

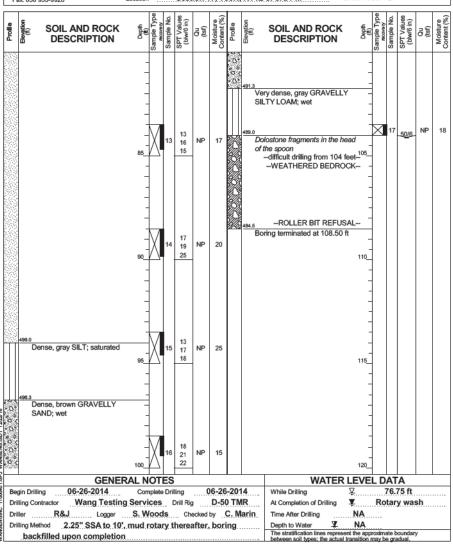








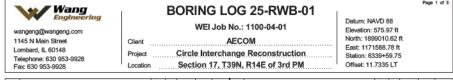




Boring Log 1702-B-03 station and offset are measured along & Jackson Blvd.

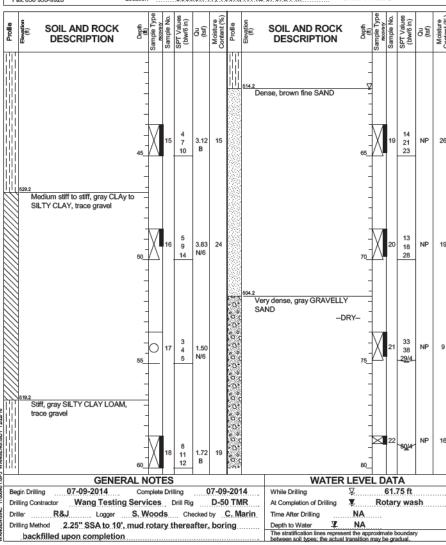


USER NAME = wjcolletti	DESIGNED	TLR	REVISED	
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JTF	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	



SOIL AND ROCK SE SE SE SE SE SE SE S																		
### Page 10-inch thick, CONCRETE ### Pa	Profile	I	DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)		Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
### 310-inch thick, CONCRETE _PAVEMENT _PA	1111	575.5	inch thick, ASPHALT	_ /-						///			-					
Medium dense, gray CRUSHED - STONE - BASE COURSE -FILL - STONE - BASE COURSE -FILL - STONE - S	11511	_{574.6} 10	inch thick, CONCRETE	~ _									1	\/		0		
Modum stiff, gray SILTY CLAY LOAM, trace gravel	44	M		~ 1	V	,		ND	e				-	X	9	0		25
Modum stiff, gray SILTY CLAY LOAM, trace gravel	44	ST		- 1	\setminus	Ι,			Ů									
FILL— 5				$\overline{}$. /								+	1				
Soft to medium stiff, gray CLAY to SiLTY CLAY, trace gravel 10	Hili			- 1	XΙ	2	2		18				1	Х	10	1		25
to SILTY CLAY, trace gravel 1	lili	570.5		5_/	\triangle		2	P					25	/ \		2	В	
3 1 0 0.57 23				\exists									7					
12 0 0.57 25 12 0 0.57 25 12 0 0.57 25 13 0 0.41 19 15 0 0.41 19 15 0 0.41 19 15 0 0.41 19 15 0 0.45 1			ole i i ole i i, dato grato.		X	3			23					Х	11			25
10				7	/\		2	В					7	/\		2	В	
10				1									1					
10				-	V	4		0.57	24				\dashv	V	12		0.57	25
S 2 0.74 24 13 0 0.41 19 15 1 18 19 19 10 10 10 19 10 10				10/	$^{\prime}$								30_	$/\backslash$				
S 2 0.74 24 13 0 0.41 19 15 1 18 19 19 10 10 10 10 10 10				\dashv									-					
GENERAL NOTES Begin Drilling 07-09-2014. Complete Drilling 07-09-2014. Drilling Contractor Wang Testing Services. Drill Rig D-50 TMR. Driller R&J. Logger S. Woods. Checketby C. Martin Drilling NA. Drilling Method 2.25" SSA to 10", mud rotary thereafter, boring Depth to Water Y. NA.				7	V	_		0.74	24				4					
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling Contractor Wang Testing Services Drill Rg D-50 TMR Driller R&J Logger S. Woods Checked by C. Martin Driller NA Drilling NA Depth to Water Y NA				1	Λ	Ĭ,			24				_					
18				\dashv									4					
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling Various of Drilling National Property of S. Woods Checked by C. Marin Drilling National Property of S. Woods Checked by C. Marin Drilling National Property Natio				_	/		0						1	\/		0		
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 Driller R&J Logger S. Woods Checket by C. Marin Drilling Method 2.25" SSA to 10", mud rotary thereafter, boring Depth to Water Y. NA				15	X	6			24				35	Ň	13			19
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling Vang Testing Services Drill Rig Drilling Contractor Wang Testing Services Drill Rig D-50 TMR In Time After Drilling NA Drilling Method 2.25" SSA to 10", mud rotary thereafter, boring Depth to Water Y NA				-									30_					
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling Van Generator Wang Testing Services Drill Rg D-50 TMR Driller R&J Logger S. Woods Checked by C. Martin Time After Drilling NA Drilling Method 2.25" SSA to 10", mud rotary thereafter, boring Depth to Water Y NA				+	. /		,						+					
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling V 61.75 ft Driller R&J Logger S. Woods Checked by C. Martin Driller R&J Logger S. Woods Checked by C. Martin Driller R&J Logger S. Woods Checked by C. Martin Driller Drilling NA Driller NA				7	X	7	2		24				\dashv					
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling Contractor Wang Testing Services Drill Rig D-50 TMR. Drilling Method 2.25" SA to 10', mud. rotary, thereafter, boring Depth to Water Y. NA The stratification lines represent the approximate boundary				Í	٧		-			ĿĿ	CL	AY LOAM, trace gravel						
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling Contractor Wang Testing Services Drill Rig D-50 TMR. Drilling Method 2.25":SA to 10', mud. rotary, thereafter, boring Depth to Water Y. NA Drilling Method 2.25":SA to 10', mud. rotary, thereafter, boring Depth to Water Y. NA The stratification lines represent the approximate boundary				1									7					
GENERAL NOTES GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling □ 07-09-2014 While Drilling □ 07-09-2014 Complete Drilling □ 07-09-2014 While Drilling □ □ 07-09-2014 At Completion of Drilling □ □ 07-09-2014 At Completion □ 07-09-2014 At Completion of Drilling □ 07-09-2014 At Com				4	Х	8	2		25					χ	14	10		16
GENERAL NOTES Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 While Drilling ♀ 61.75 ft Driller R&J Logger S. Woods Checked by C. Marin Drilling Method 2.25" SSA to 10', mud rotary, thereafter, boring backfilled upon completion GENERAL NOTES WATER LEVEL DATA While Drilling ♀ 61.75 ft At Completion of Drilling ▼ Rotary wash Time After Drilling NA Depth to Water ▼ NA The stratification lines represent the approximate boundary				20_/	\triangle			В		1111			40	/ \			В	
Begin Drilling 07-09-2014 Complete Drilling 07-09-2014 White Drilling	-					_												
Drilling Contractor Wang Testing Services Drill Rig D-50 TMR Driller R&J Logger S, Woods Checked by C, Marin Drilling Method 2.25" SA to 10', mud. rotary, thereafter, boring Depth to Water ▼ NA The stratification lines represent the approximate boundary	Beg	_																
Drillier R&J Logger S, Woods Checked by C, Marin Time After Drilling NA	Dril																	
backfilled upon completion Depth to Water	Dril																	
packfilled upon completion	Dril	-		ud.re	otai	ry t	herea	after,	bori	ng.								
Detween soil types; the actual transition may be gradual.	backfilled upon completion									between soil types; the actual	ransition n	nay b	e gra	dual.	,			

Wang Wang		BORING LOG 25-RWB-01		Page 2 of
Engineering		BOIGHO EGG 20-IGGB-01	Datum: NAVD 88	
wangeng@wangeng.com		WEI Job No.: 1100-04-01	Elevation: 575.97 ft	
1145 N Main Street	Client	AECOM	North: 1899010.62 ft	
Lombard, IL 60148 Telephone: 630 953-9928	Project	Circle Interchange Reconstruction	East: 1171588.78 ft Station: 6339+59.75	
Fax: 630 953-9928	Location	Section 17, T39N, R14E of 3rd PM	Offset: 11.7335 LT	





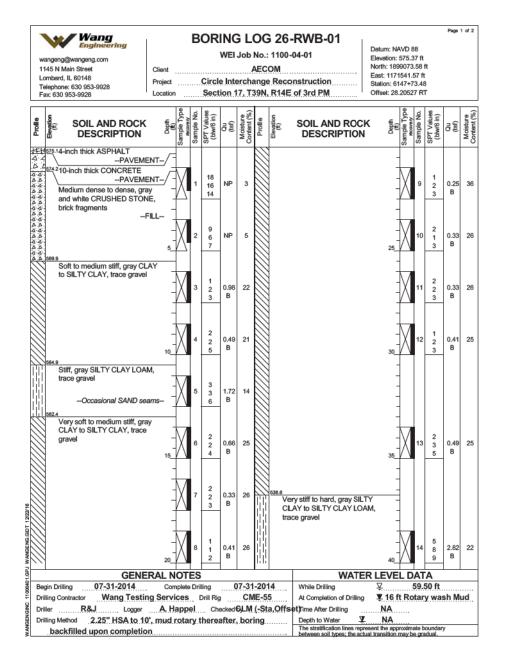
Fax	phone: 630 953-9928 : 630 953-9928	Location			Sect	ion 1	17, T	39N	R14E	of 3rd PM	Offset: 11	.7335	LT			
Profile	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROC DESCRIPTION		Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
O.		-														
0 °																
2 49	Very dense, gray SILT to SIL	TY –														
	LOAM, trace gravel	RY-														
		85	X	23	33 43 2 <u>4/</u> 3	NP	17									
		_														
		-														
		_														
		-														
		-														
	DOLLED DE DECLE		0	24	50/2	NR										
48	Boring terminated at 89.50 ft				30/2											
		90_														
		_														
		-														
		_														
		_														
		-														
		95_														
		-														
		_														
		-														
		_														
		_														
		400														
	CENE	100_		_						MATE	D E\/E	_				
Rogin	Drilling 07-09-2014	RAL N	nplete			(07-09	-20	14	While Drilling	R LEVE			75 ft		
_	ng Contractor Wang Testir									At Completion of Drilling	T			y wa	sh	
Drille	r R&J Logger	S. W	000	ls	Ch	ecked	by .			Time After Drilling	NA.					
	ng Method 2.25" SSA to 10			-				_		Depth to Water The stratification lines rep		orcodim	ate h	ounda	v	
J	backfilled upon completio	n								The stratification lines repr between soil types; the act	ual transition	may b	e gra	adual.	y	

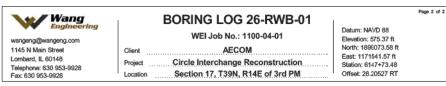
Boring Log 25-RWB-01 station and offset along B Jackson Entrance Ramp is: Sta. 8214+37.23, Offset 8.26' Rt.

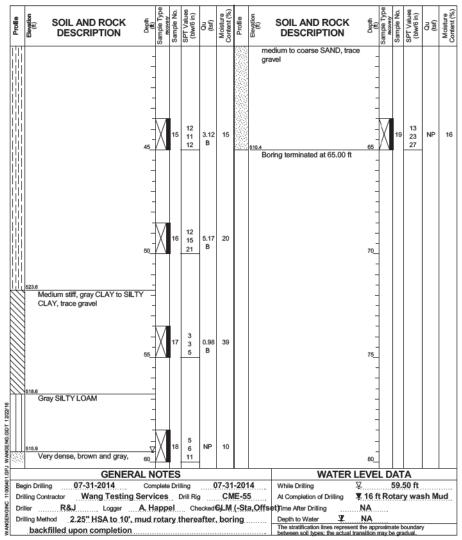
Tran Systems

	USER NAME = wjcolletti	DESIGNED	TLR	REVISED	Г
		CHECKED	WJC	REVISED	
•	PLOT SCALE = NTS	DRAWN	JTF	REVISED	
	PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

BORING LOGS 5	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-1702	1422	1422 2014-015R&B-R		825	464
31110C1011L 140.010-1702			CONTRACT	NO. 6	0X94
SHEET NO. S2-79 OF S2-80 SHEETS		ILLINOIS FED. A	ID PROJECT		





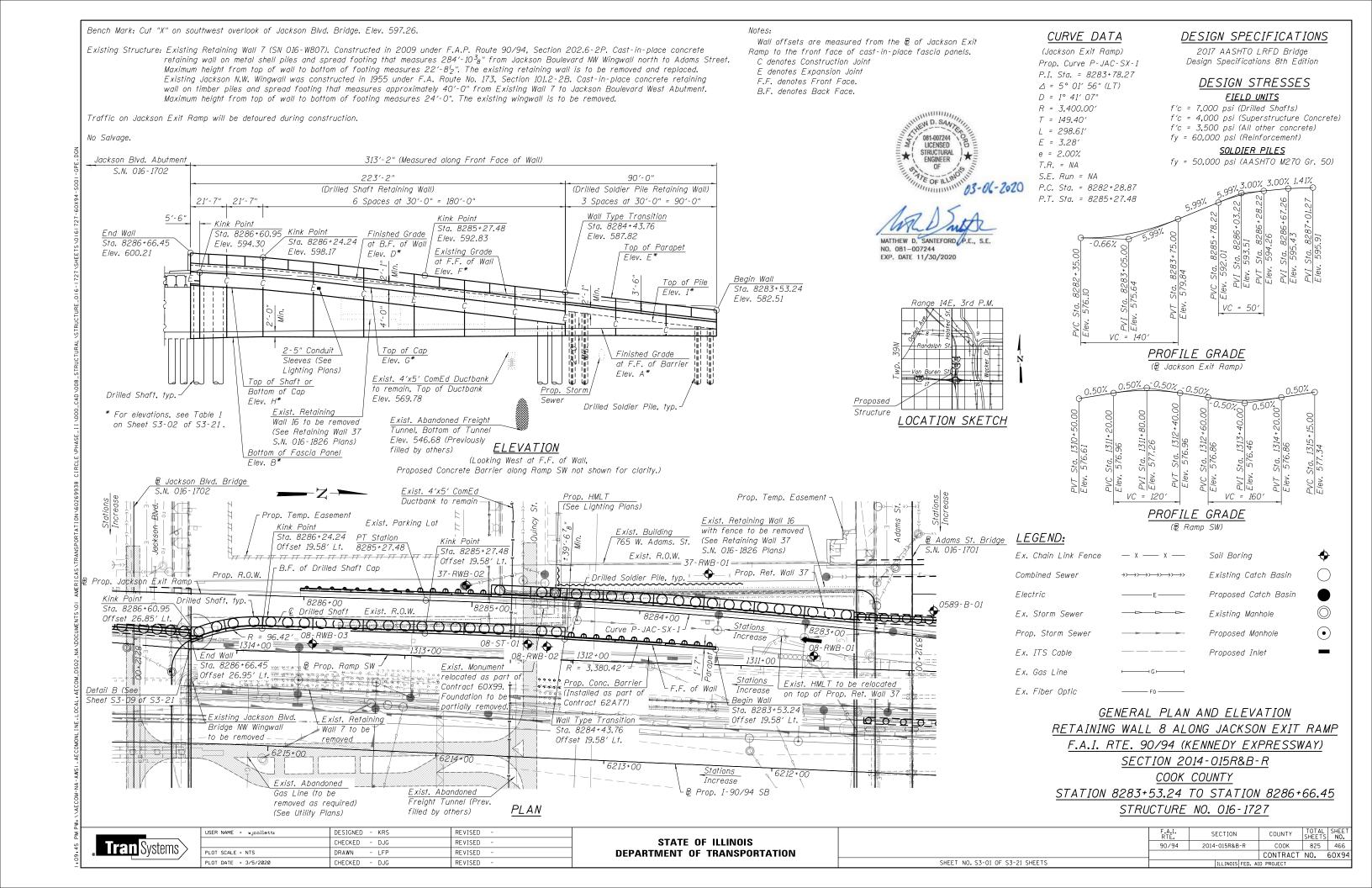


Boring Log 26-RWB-01 station and offset along & Jackson Entrance Ramp is: Sta. 8241+97.24, Offset 39.44' Lt.

Tran Systems

USER NAME = wjcolletti	DESIGNED	TLR	REVISED	
	CHECKED	WJC	REVISED	
PLOT SCALE = NTS	DRAWN	JTF	REVISED	
PLOT DATE = 3/5/2020	CHECKED	WJC	REVISED	

BORING LOGS 6	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE'
STRUCTURE NO. 016-1702	1422	2014-015R&B-R	COOK	825	465
3111001011E 140.010=1702			CONTRACT	NO. 6	0X94
SHEET NO. S2-80 OF S2-80 SHEETS	ILLINOIS FED. AID PROJECT				



GENERAL NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. The Contractor shall exercise extreme caution during construction to make certain that construction activities, live load surcharge and other loads applied to the structures will not have detrimental effects on the adjacent building foundations. Any damage during construction shall be repaired by the Contractor at his expense and no charge to the department. Driving piles and temporary sheet piling is not allowed.
- 3. The Contractor shall provide vibration and displacement monitoring at the locations specified in the Special Provisions for Construction Vibration Monitoring and Monitoring Adjacent Structures, to ensure that removal/construction activities in the vicinity of the structures do not have detrimental effects on building foundations. No additional compensation shall be provided to the Contractor for alternative means and methods, or additional precautionary measures, required during removal/ construction activities to satisfy these requirements. See Contract Special Provisions for details.
- Drilled shaft construction above existing grade shall not be paid separately but shall be included with Drilled Shaft In Soil.
- 5. Slipforming of parapets is not allowed.
- The Contractor shall field verify locations of existing underground utilities. The
 Contractor shall take precautions to protect existing utilities during construction of
 the wall. Any damage to the existing utilities shall be the responsibility of the
 Contractor.
- 7. Concrete for the Drilled Shafts shall be in accordance with Section 516 of Standard Specifications and shall have the minimum compressive strength of 7,000 psi prior to excavation in front of shafts and installation of lagging system.
- 8. For drilled shaft locations where permanent casing is required as shown on the plans, the casing will be paid for under Permanent Casing. If Contractor elects to use permanent casing for ease of construction, the casing will not be paid for separately and is included in Drilled Shaft In Soil.
- Wall to be built along straight chords between construction and expansion joints.
 Concrete Sealer shall be applied to the exposed top, front, and back faces of the parapet, and to the exposed front faces of cap and fascia panels.
- 11. Limited groundwater elevation data is available in the boring logs. In addition, groundwater may also be present in deeper granular layers. The groundwater may rise in the shafts to an elevation above the top of granular layers. The Contractor shall consider this information when choosing construction methods. The Contractor will not be compensated for issues related to the groundwater elevation.
- 12. The Contractor shall take all necessary precautions not to contaminate groundwater during the drilled shaft construction operation. Contractor is responsible for the proper containment and disposal of the contaminated groundwater and spoils resulting from the Contractor's means and methods. No additional cost will be paid for this effort.
- 13. Due to the squeeze potential of the clay soils, the use of temporary casing will be required to properly construct the shafts. Casing may be pulled or remain in place, as determined by the Contractor at no cost to the Department.
- 14. The Contractor shall coordinate the construction of the proposed structure with the construction of the Proposed Jackson Blvd. Bridge and Proposed Retaining Wall 37. See MOT plan sheets and special provisions, including the Available Work Areas and Sequencing Requirements special provision, for additional construction and coordination requirements.
- 15. The Contractor shall provide a method to assure the soldier piles achieve at least the plan tip elevations. The soldier pile locations and elevations shall meet the tolerances provided in the Special Provisions. Any additional measures required to satisfy the construction tolerances will not be paid for separately but shall be included in Drilling and Setting Soldier Piles (In Soil).
- Soldier piles shall be cleaned and given one shop coat of Inorganic Zinc Rich Primer. Cost included with Furnishing Soldier Piles (W Section).
- 17. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be the cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

SUGGESTED CONSTRUCTION SEQUENCE

- 1. Construct Retaining Wall 37 drilled shafts and soldier piles.
- Remove portion of Existing Retaining Wall 16 that is in conflict with the proposed drilled shafts and excavate in front of Retaining Wall 37 to install timber lagging for Wall 37.
- 3. Drill shafts and install soldier piles.
- 4. Remove portion of Existing Retaining Wall 7 in front of the proposed wall and excavate to install timber lagging.
- 5. Construct concrete fascia panels, cap, and parapet.

TABLE 1 - WALL ELEVATIONS

	Station	Offset	Elevation A	Elevation B	Elevation C	Elevation D	Elevation E	Elevation F	Elevation G	Elevation H	Elevation I	Wall Type
Ī	8283+53.24	19.58′ Lt.	576.78	574.78	579.76	579.01	582.51	583.57	-	-	576.92	Drilled
	8283+83.41	19.58′ Lt.	576.90	574.90	580.06	580.70	584.20	584.02	-	-	578.62	Soldier
	8284+13.59	19.58′ Lt.	576.95	574.95	580.33	582.51	586.01	584.95	-	-	580.43	Pile
#	8284+43.76	19.58′ Lt.	576.93	574.93	580.35	584.32	587.82	586.20	-	-	582.23	Wall
##	8284+43,76	19.58′ Lt.	576.93	574.93	580.35	584.32	587.82	586.20	582.23	578.23	-	
	8284+73.93	19.58′ Lt.	576.83	574.83	580.28	586.12	589.62	587.11	584.04	580.04	-	
	8285+04.11	19.58′ Lt.	576.68	<i>574.68</i>	580.17	587.93	591.43	588.31	585.85	581.85	-	
[8285+27.48	19.58′ Lt.	576.59	<i>574.59</i>	580.05	589.33	592.83	589.11	587.25	583.25	-	
	8285+34.24	19.58′ Lt.	576.57	574.57	580.02	589.74	593.24	589.37	587.65	583.65	-	Drilled
	8285+64.24	19.58′ Lt.	576.51	57 4. 51	579.87	591.54	595.04	590.61	589.45	585.45	-	Shafts
	8285+94.24	19.58′ Lt.	576.51	<i>574.51</i>	579.93	593.26	596.76	592.33	591.17	587.17	-	Wall
	8286+24.24	19.58′ Lt.	576.57	574.57	580.11	594.67	598.17	593.15	592.59	588.59	-	
	8286+45.65	21.99′ Lt.	576.64	574 . 64	580.25	595.57	599.07	593.75	593.49	589.49	-	
	8286+60.95	26.85′ Lt.	576.71	574.71	580,28	596.38	599,88	594.23	594.30	590.30	-	
	8286+66.45	26.95′ Lt.	576,74	574.74	580.28	596.71	600.21	593.98	594.62	590.62	-	

Elevation A - Finished Grade at Front Face of Barrier*

Elevation B - Bottom of Fascia Panel

Elevation C - Top of Slope at Front Face of Wall* Elevation D - Finished Grade at Back Face of Wall

Elevation E - Top of Parapet

Elevation F - Existing Grade at Front Face of Wall

Elevation G - Top of Cap Elevation H - Top of Shaft/ Bottom of Cap Elevation I - Top of Pile

Elevations just to the right of joint

Elevations just to the left of joint

* Installed as part of Contract 62A77

TOTAL BILL OF MATERIAL

Item	Unit	Total Quantity	
Removal of Existing Structures No. 2	Each	1	
Structure Excavation	Cu. Yd.	922	
Concrete Structures	Cu. Yd.	231.5	
Concrete Superstructure	Cu. Yd.	72.4	
Stud Shear Connectors	Each	170	
Reinforcement Bars	Pound	459,810	
Reinforcement Bars, Epoxy Coated	Pound	24,850	
Mechanical Splicers	Each	504	
Name Plates	Each	1	
Permanent Casing	Foot	180	
Drilled Shaft In Soil	Cu. Yd.	1,979.3	
Furnishing Soldier Piles (W Section)	Foot	563	
Drilling and Setting Soldier Piles (In Soil)	Cu. Ft.	4,564	
Untreated Timber Lagging	Sq. Ft.	390	
Concrete Structures (Retaining Wall)	Cu. Yd.	28.6	
Concrete Sealer	Sq. Ft.	5,031	
Geocomposite Wall Drain	Sq. Yd.	32	
Crosshole Sonic Logging Access Ducts	Foot	1,890	
Crosshole Sonic Logging Testing	Each	5	
Class SI Concrete (Miscellaneous)	Cu. Yd.	128.6	
Pipe Underdrain for Structures 4"	Foot	314	

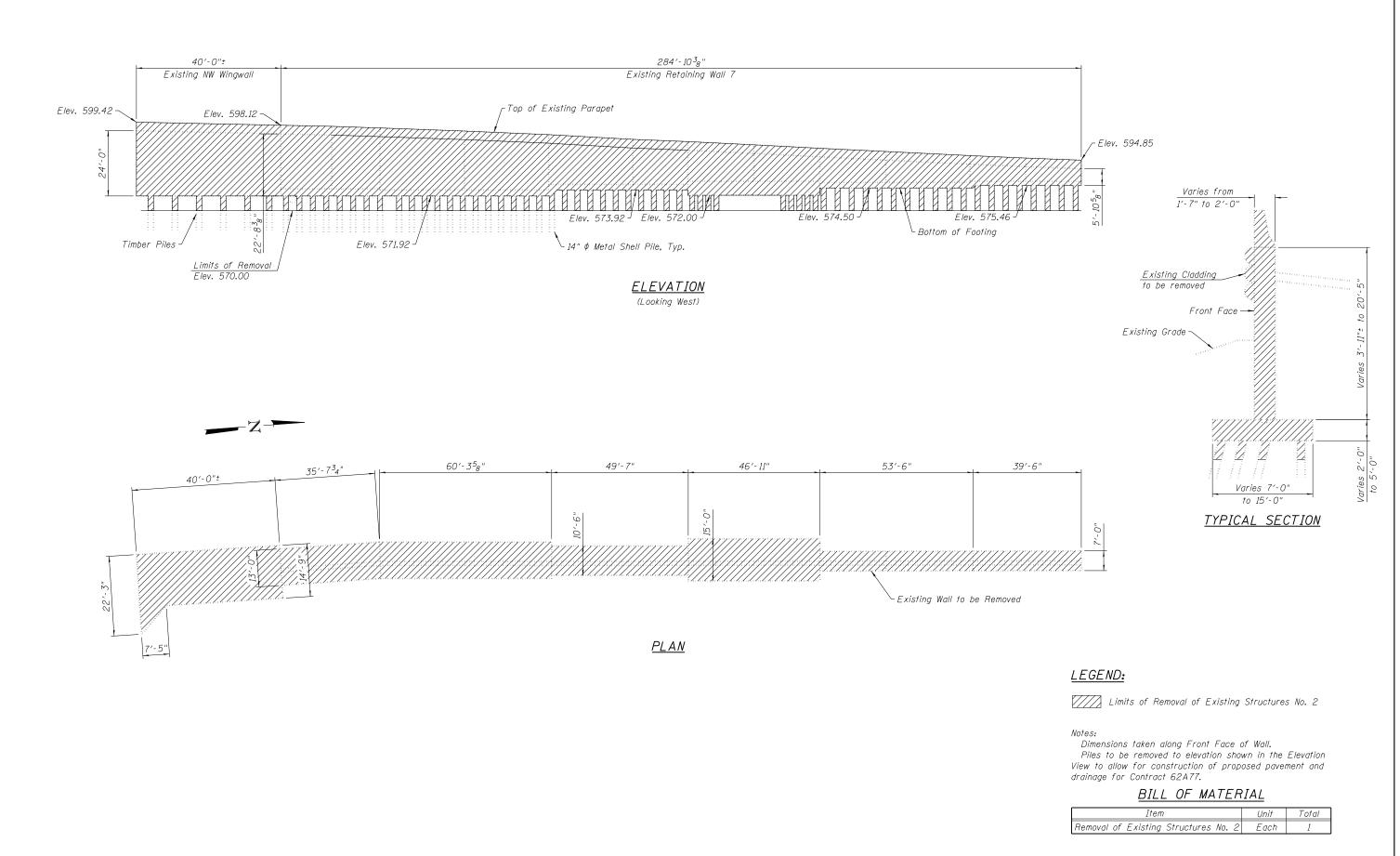
STATION 8283+53.24 TO 8286+66.45 BUILT 20__ BY STATE OF ILLINOIS F.A.I. RTE. 90/94 SEC. 2014-015R&B-R LOADING HL-93 STR. NO. 016-1727

NAME PLATE
See Std. 515001

INDEX OF SHEETS

S3-01 General Plan and Elevation

S3-02	General Data
S3-03	Existing Wall 7 Removal
S3-04	Plan and Elevation 1
S3-05	Plan and Elevation 2
S3-06	Plan and Elevation 3
S3-07	Plan and Elevation 4
S3-08	Drilled Shaft Wall Sections and Details 1
S3-09	Drilled Shaft Wall Sections and Details 2
S3-10	Drilled Shaft Wall Sections and Details 3
S3-11	Drilled Soldier Pile Wall Sections and Details 1
S3-12	Drilled Soldier Pile Wall Sections and Details 2
S3-13	Architectural Details
S3-14	Bar Splicer Assembly and Mechanical Splicer Details
S3-15	Boring Logs 1
S3-16	Boring Logs 2
S3-17	Boring Logs 3
S3-18	Boring Logs 4
S3-19	Boring Logs 5
S3-20	Boring Logs 6
S3-21	Boring Logs 7



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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

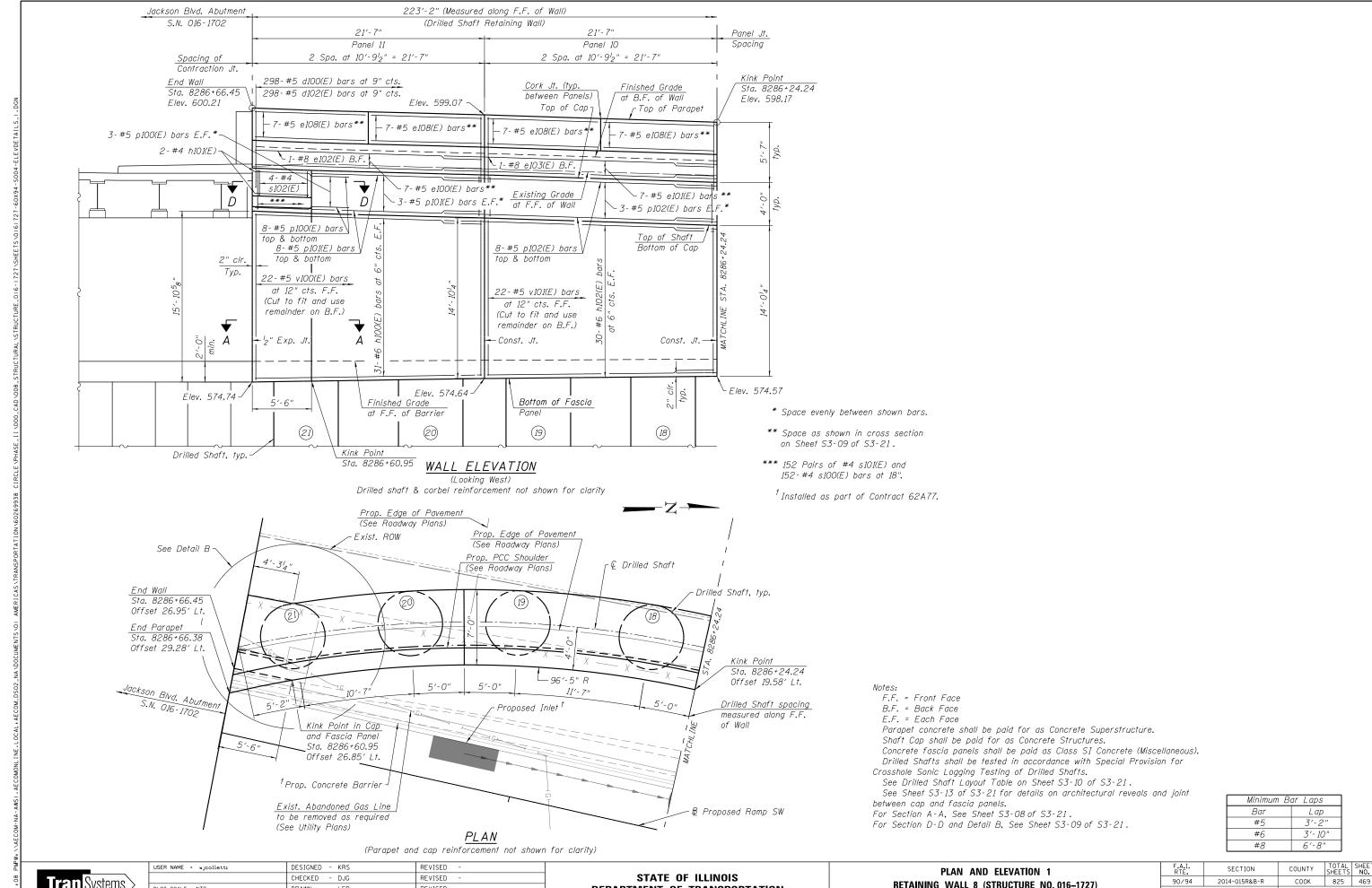
EXISTING WALL 7 REMOVAL
RETAINING WALL 8 (STRUCTURE NO. 016–1727)

SHEET NO. S3-03 OF S3-21 SHEETS

F.A.I. SECTION COUNTY TOTAL SHEETS NO.

90/94 2014-015R&B-R COOK 825 468

CONTRACT NO. 60X94



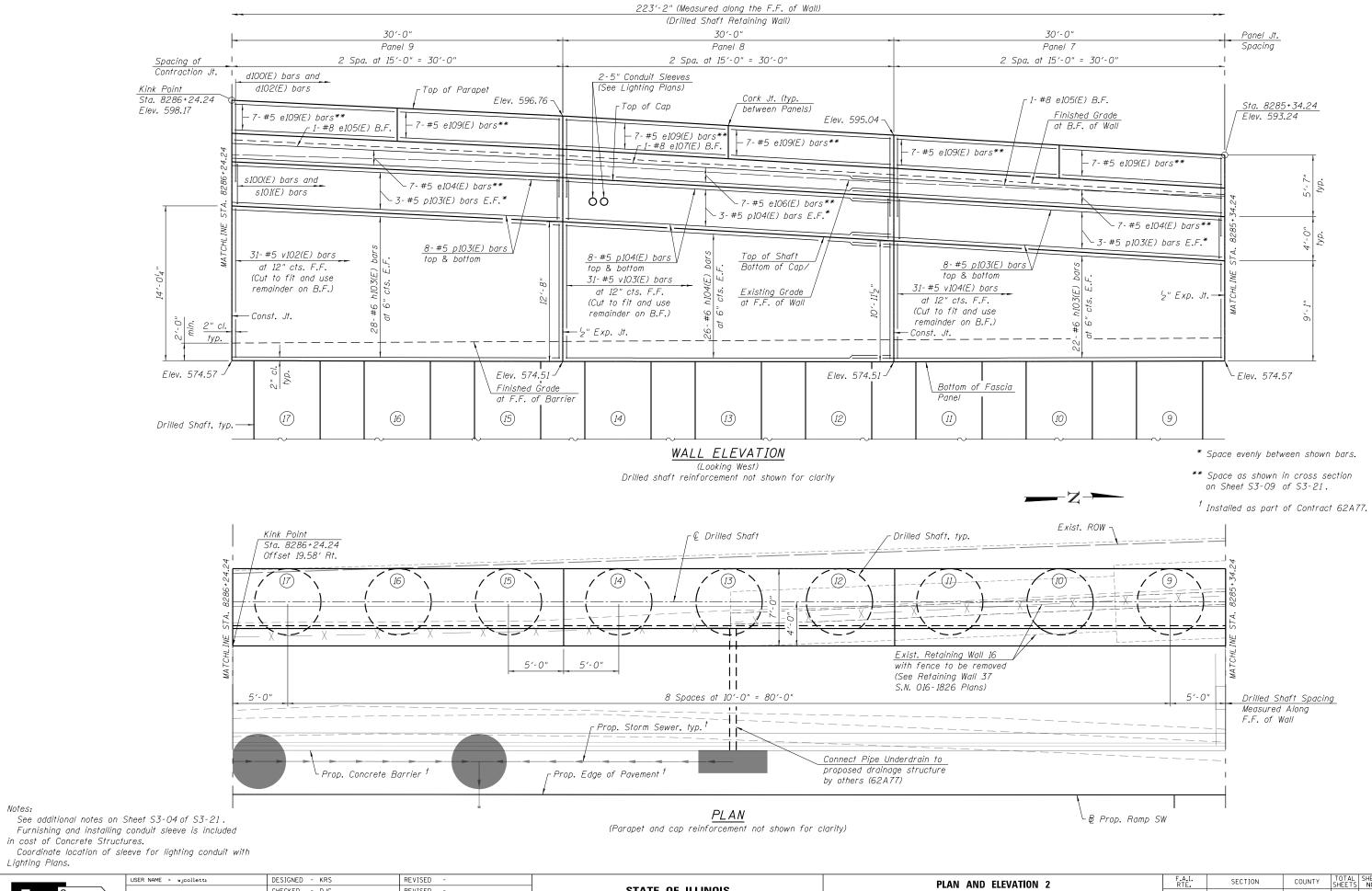
Tran Systems

REVISED CHECKED - DJG PLOT DATE = 3/5/2020 REVISED

DEPARTMENT OF TRANSPORTATION

RETAINING WALL 8 (STRUCTURE NO. 016-1727) SHEET NO. S3-04 OF S3-21 SHEETS

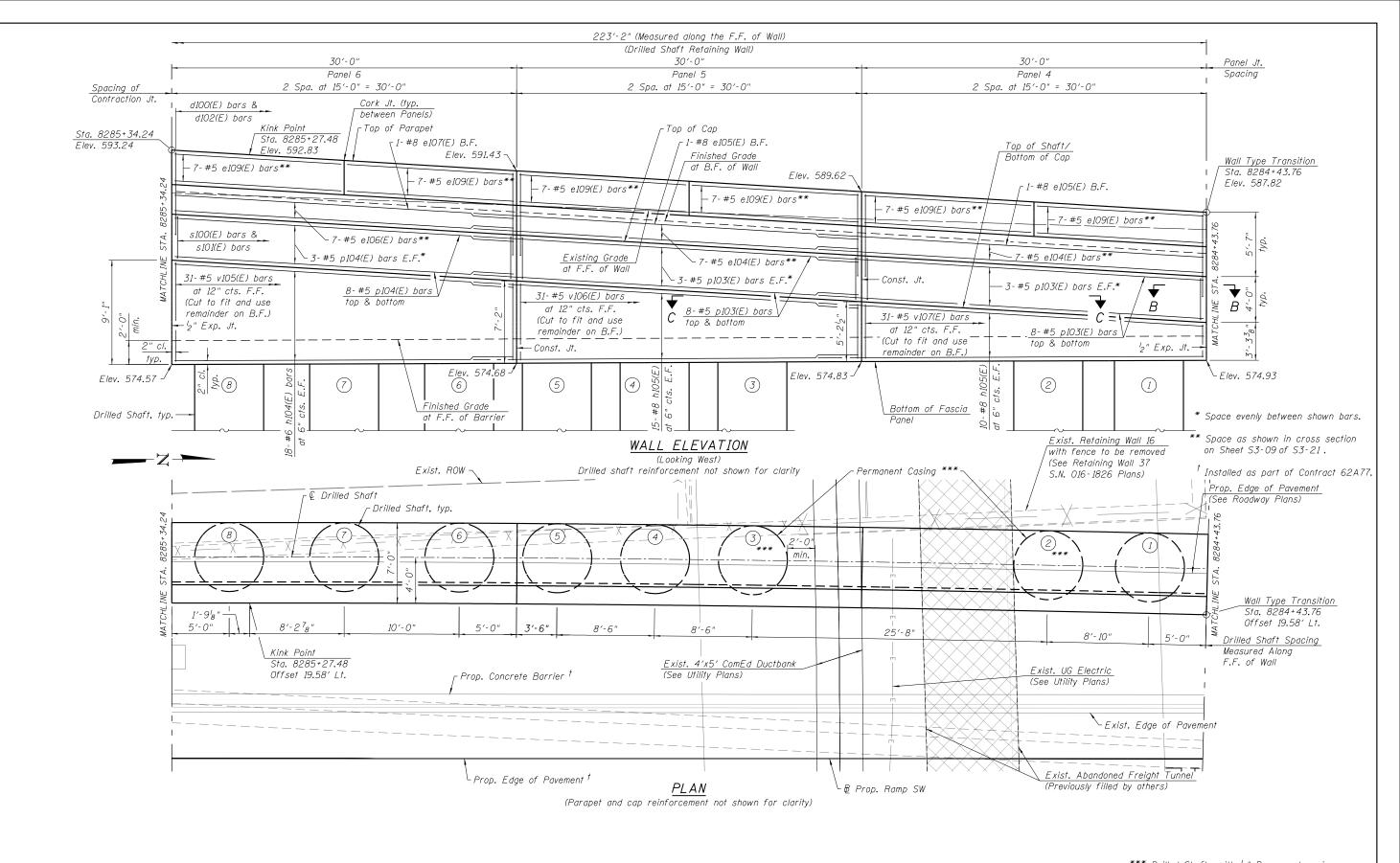
COOK 825 469 CONTRACT NO. 60X94



• Tran Systems

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PLAN AND ELEVATION 2
RETAINING WALL 8 (STRUCTURE NO. 016–1727)
SHEET NO. S3-05 OF S3-21 SHEETS



Notes

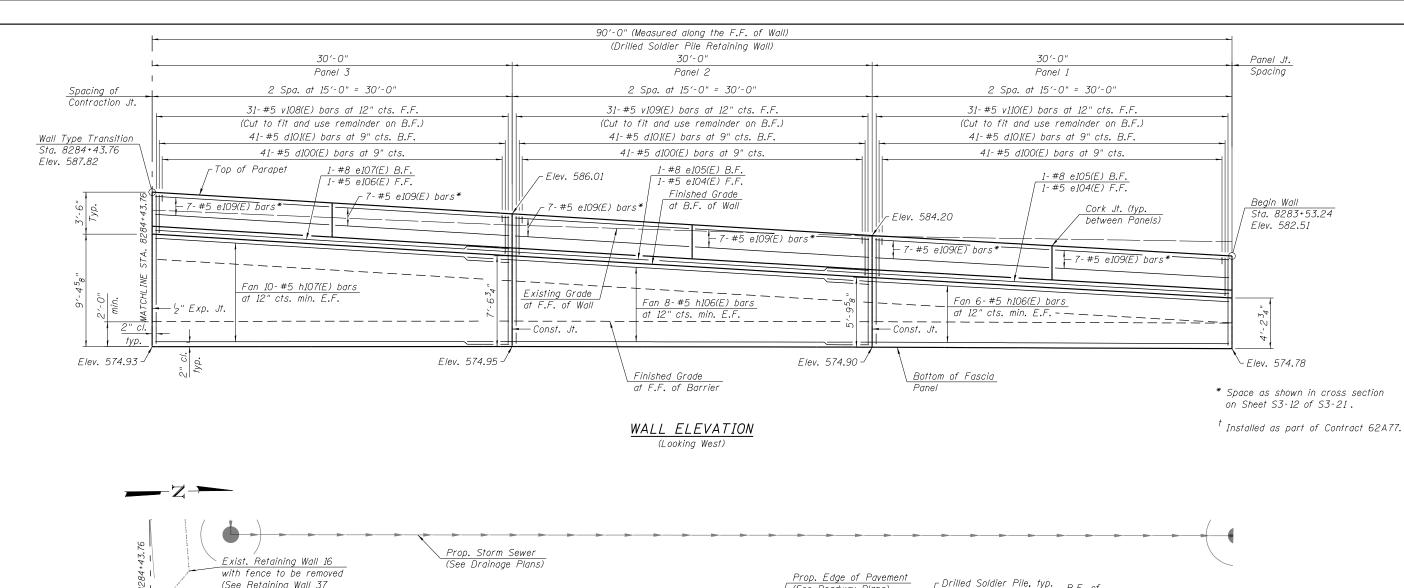
See additional notes on Sheet S3-04 of S3-21. For Sections B-B and C-C, see sheet S3-08 of S3-21. *** Drilled Shafts with '2" Permanent casing required. Contractor must verify location of the Abandoned Tunnel and ComEd prior to drilling of the shafts and adjust the locations of the permanent casing as necessary subject to approval of the Engineer.

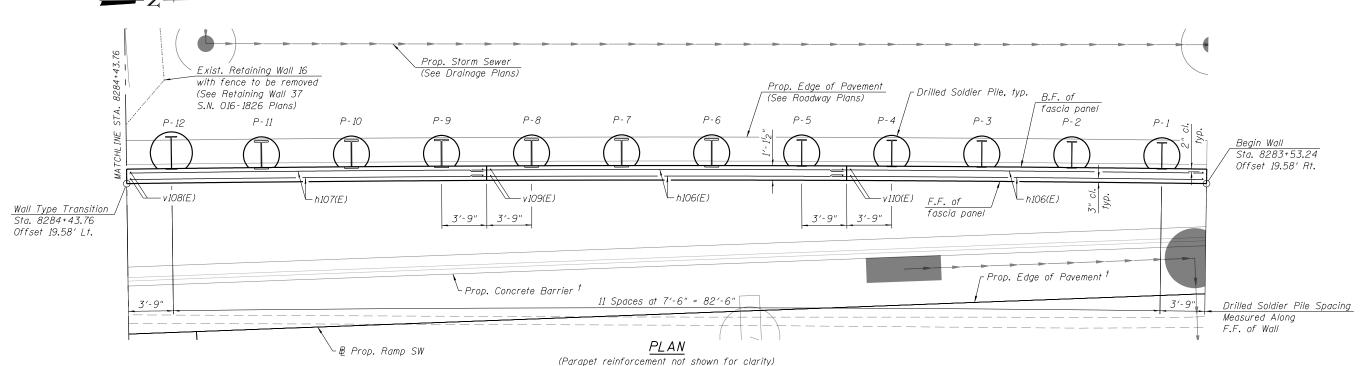


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	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - LFP	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -

PLAN AND ELEVATION 3
RETAINING WALL 8 (STRUCTURE NO. 016-1727)
CUEFT NO CZ OC OF CZ O1 CUEFTC

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2014-015R&B-R	соок	825	471
		CONTRACT	NO.	60X94
	ILLINOIS FED. A	ID PROJECT		





Notes:

F.F. = Front Face.

B.F. = Back Face.

E.F. = Each Face.

For soldier pile wall cross sections and details, see Sheet S3-11 of S3-21. For soldier pile layout, sections and details and Bill of Material, see Sheet S3-12 of S3-21.

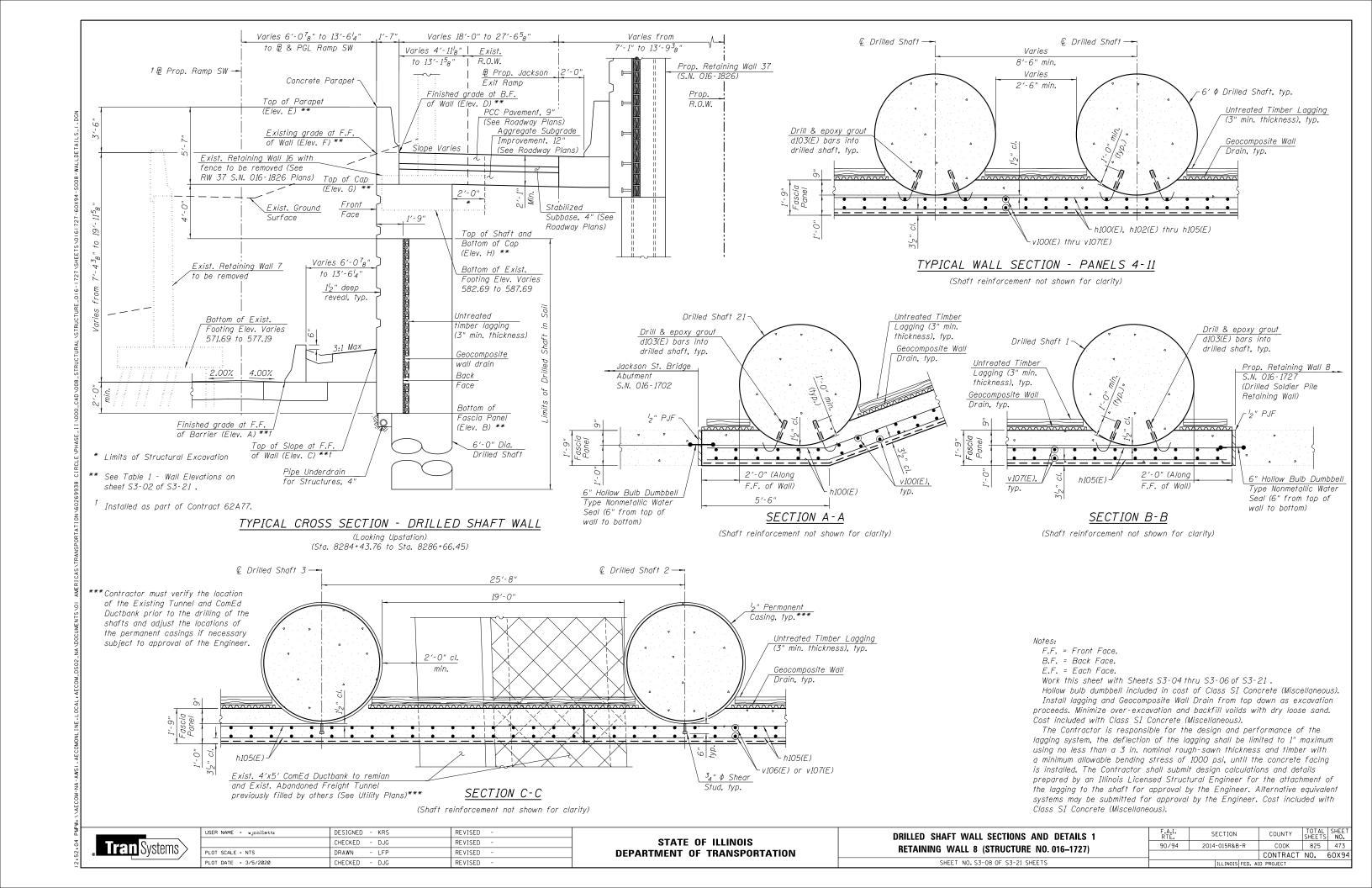
Concrete fascia panels shall be paid as Concrete Structures (Retaining Wall).

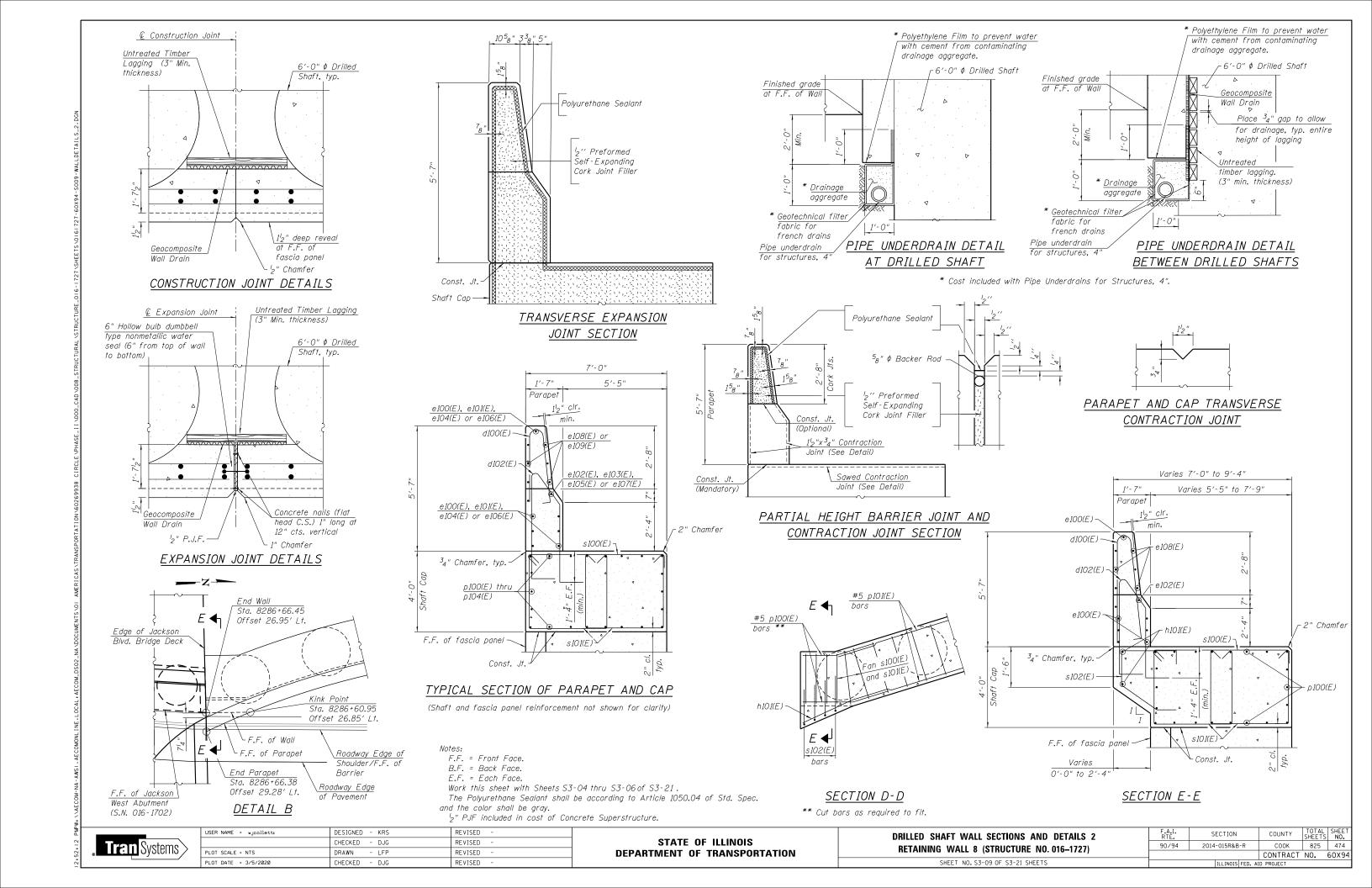
0	Tran Systems

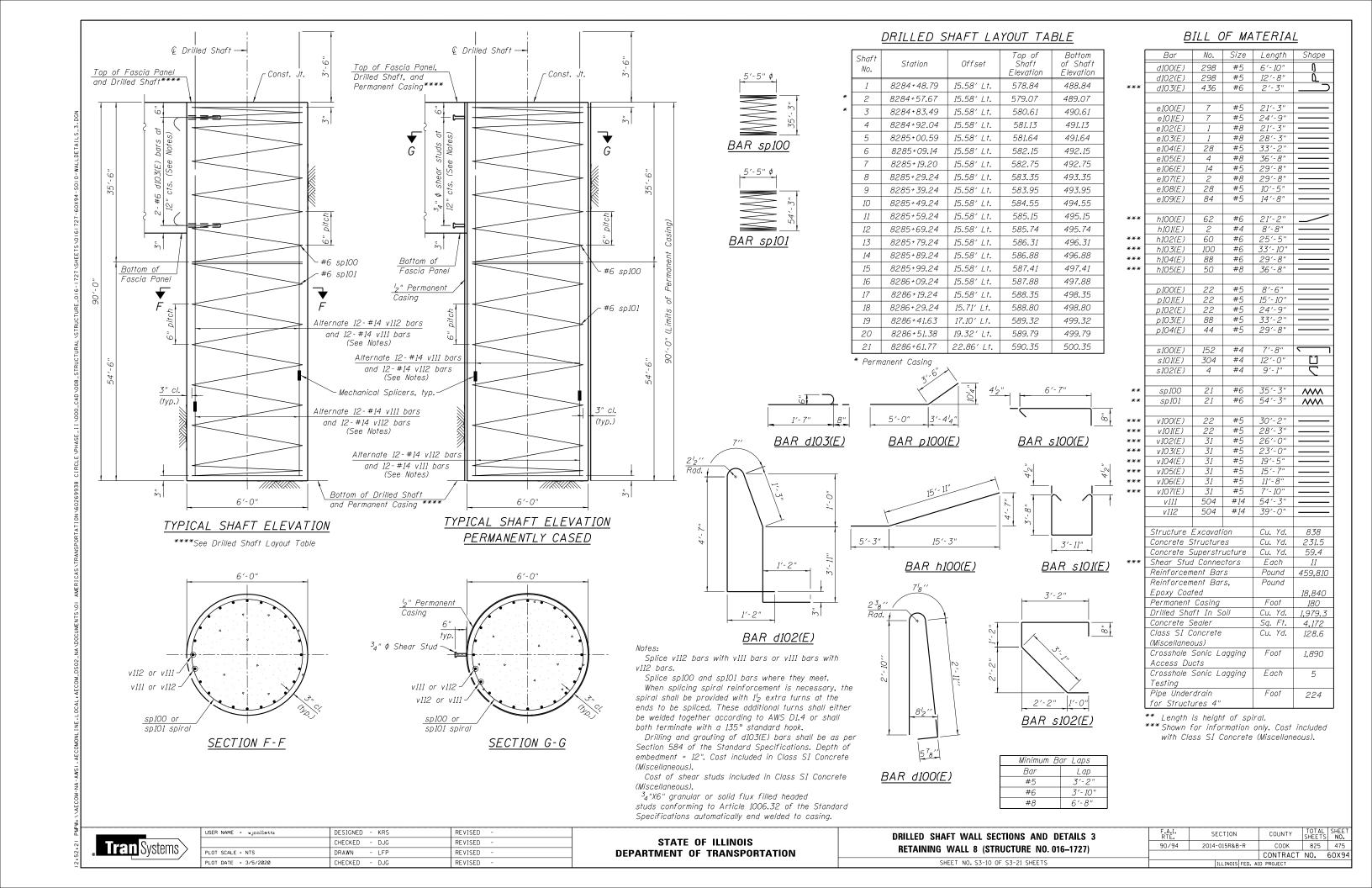
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USER NAME = wjcollett:	DESIGNED - KRS	REVISED -	
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PLOT SCALE = NTS	DRAWN - LFP	REVISED -	
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -	
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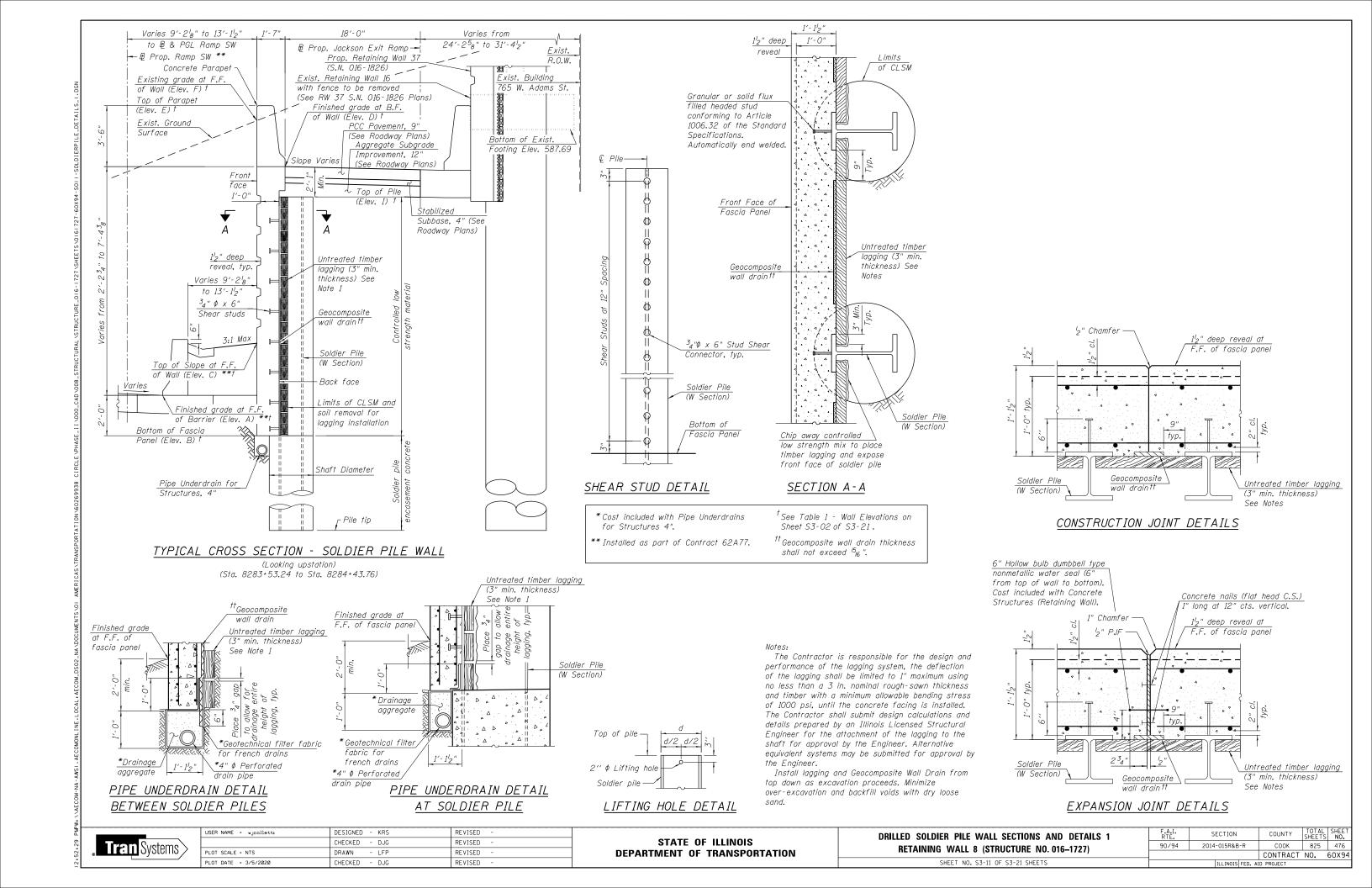
	PLAN	AND ELI	EVATION	4
RETAINING	WALL	8 (STRU	JCTURE	NO. 016–1727)
	SHEET NO	S3-07 0F	\$3-21 SHE	FTS

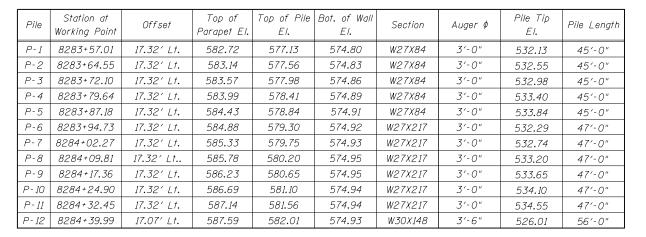
F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
90/94	2014-015R&B-R		СООК	825	472
			CONTRACT	NO.	60X9
	ILLINOIS FED	. AI	D PROJECT		

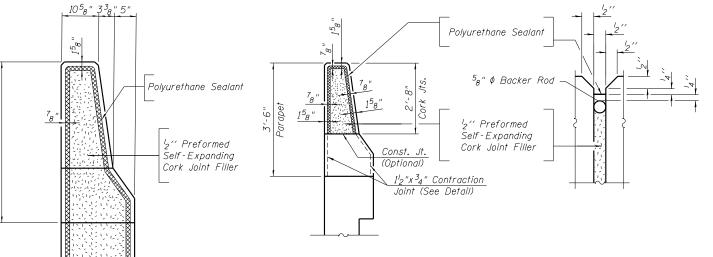












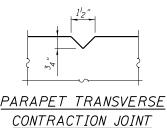
PARTIAL HEIGHT BARRIER JOINT AND CONTRACTION JOINT SECTION

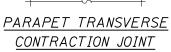
BILL OF MATERIAL

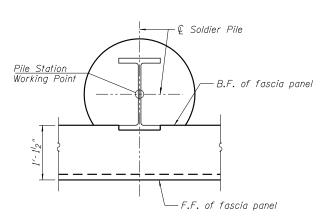
Bar	No.	Size	Length	Shape
d100(E)	123	#5	6′-10"	Ŋ
d101(E)	123	#5	10'-0"	Ŋ
e104(E)	2	#5	33′-2"	
e105(E)	2	#8	36′-8"	
e106(E)	1	#5	29′-8"	
e107(E)	1	#8	29′-8"	
e109(E)	42	#5	14'-8"	
h106(E)	20	#5	29′-8"	
h107(E)	28	#5	33′-2"	
v108(E)	31	#5	16′-3"	
v109(E)	31	#5	12′-9"	
v110(E)	31	#5	9'-4"	
Structure	Excavat	ion	Cu. Yd.	84
Concrete	Superstr	ucture	Cu. Yd.	13.0
Stud She	ar Conne	ctors	Each	170
Reinforcement Bars,		·s,	Pound	6,010
Epoxy Coated				
	Furnishing Soldier Piles		Foot	563
(W Section)				
	Drilling and Setting		Cu. Ft.	4,564
	Soldier Piles (In Soil)			
	d Timber Lagging		Sq. Ft.	390
Concrete Structures		Cu. Yd.	28.6	
	Retaining Wall)			
Concrete			Sq. Ft.	859
Geocompo		Drain	Sq. Yd.	32
Pipe Unde			Foot	90
for Struc	tures 4"			

 $\frac{1}{2}$ " PJF included in cost of Concrete Superstructure.

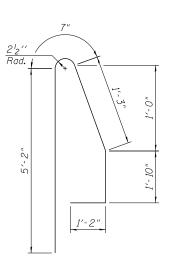
TRANSVERSE EXPANSION JOINT SECTION







SOLDIER PILE WORKING POINT



BAR d101(E)

Minimum Bo	ar Laps
Bar	Lap
#5	3'-2"
#8	6′-8"

. Tran Systems

e109(E)

e105(E) or e107(E)

e104(E) or e106(E)

F.F. of

fascia panel

reveal, typ.

h106(E) and

 $^{3}_{4}$ " $\phi \times 6$ " Shear studs, typ.

<u>v108(E) thru</u> v110(E)

h107(E)

d100(E)

d101(E)

typ.

B.F. of parapet

Bonded

" сі., Тур.

Soldier Pile (W Section)

Wall drain

Untreated

timber lagging

B.F. of fascia panel

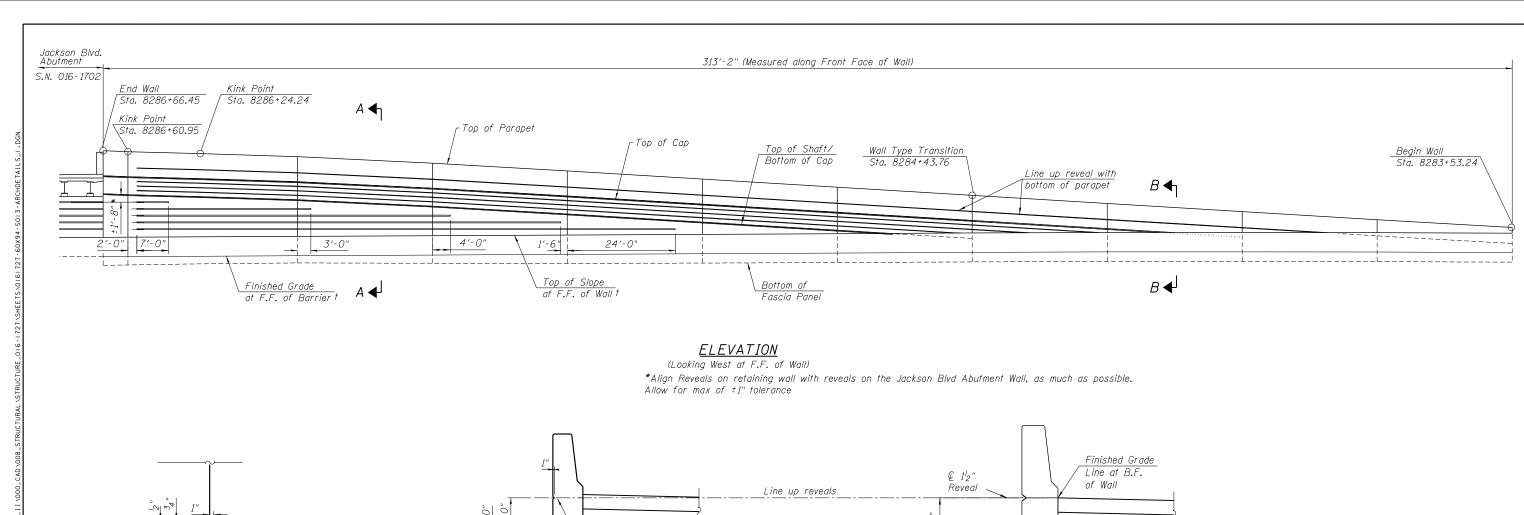
Const. Jt.

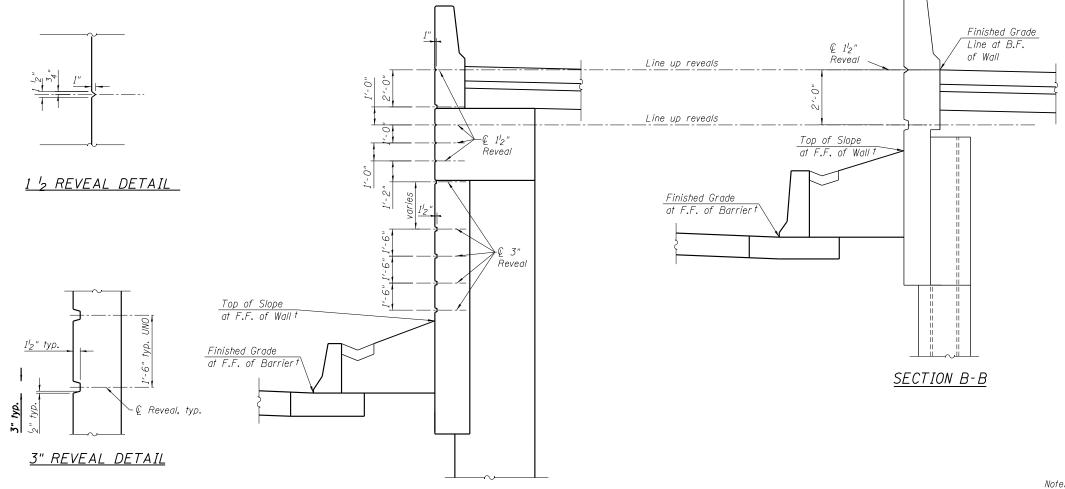
USER NAME = wjcolletti	DESIGNED - KRS	REVISED -
	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - LFP	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -

TYPICAL SOLDIER PILE WALL CROSS SECTION

DRILLED SOLDIER PILE WALL SECTIONS AND DETAILS 2 RETAINING WALL 8 (STRUCTURE NO. 016–1727)
SHEET NO. S3-12 OF S3-21 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2014-015R&B-R	соок	825	477
		CONTRACT	NO.	60X94
	ILLINOIS FED. AI	ID PROJECT		





SECTION A-A

t Installed as part of Contract 62A77.

Coordinate / verify all dimensions with structural drawings. Reveals will not be paid separately and shall be included in the cost of pay item Class SI Concrete (Miscellaneous), Concrete Structures, and Concrete Structures (Retaining Wall).



USER NAME = wjcolletti	DESIGNED - KRS	REVISED -
	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - LFP	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -

ARCHITECTURAL DETAILS	
RETAINING WALL 8 (STRUCTURE NO. 016–1727)	
CHEET NO CZ 17 OF CZ 21 CHEETC	_

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2014-015R&B-R	соок	825	478
		CONTRACT	NO.	60X94
	ILL INOIS FED. A	ID PROJECT		

STANDARD BAR SPLICER ASSEMBLY

		Minim	num Lap Len	gths		
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6

Table 1: Black bar, 0.8 Class C

Table 2: Black bar, Top bar lap, 0.8 Class C

Table 3: Epoxy bar, 0.8 Class C

Table 4: Epoxy bar, Top bar lap, 0.8 Class C

Table 5: Epoxy bar, Class C

Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

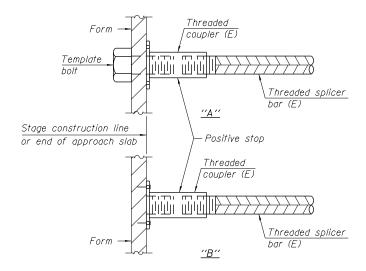
Location	Bar size	No. assemblies required	Table for minimum lap length

<u>NOTES</u>

Splicer bars shall be deformed $\overline{\text{with thre}}$ and have a minimum 60 ksi yield strength.

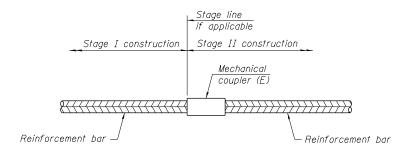
All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.



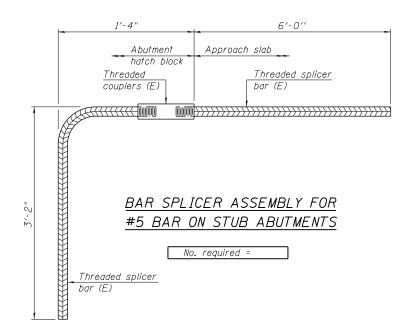
INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

No. assemblies required
24
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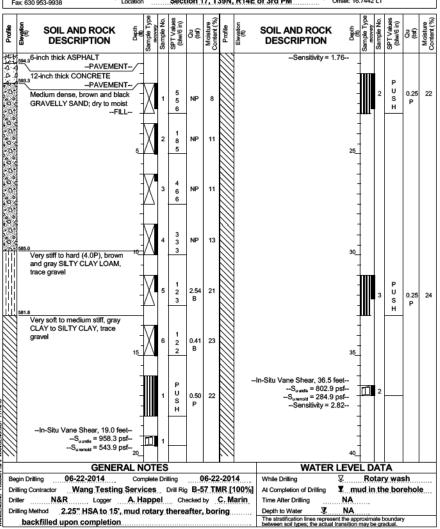


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	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - AJD	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -

LY AND MECHANICAL SPLICER DETAILS	BAR SPLICER ASSEMBLY
ALL 8 (STRUCTURE NO. 016–1727)	RETAINING WALL
NO S3-14 OF S3-21 SHEETS	SHEET NO

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
90/94	2014-015R&B	-R	соок	825	479
			CONTRACT	NO.	60X9
	ILLINOIS	FED. A	ID PROJECT		

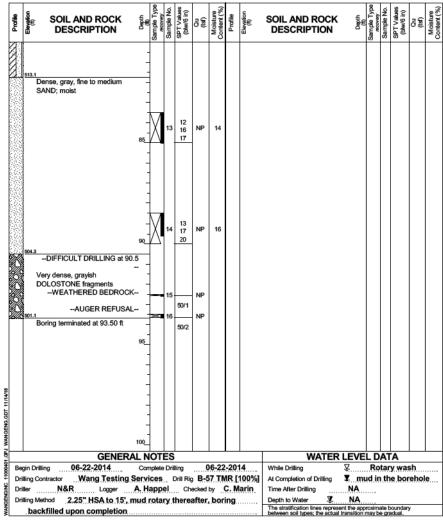




Wang Engineering	1	BOF	RIN	G L	00	G 058	9-B-01				Page	2 of 3
wangeng@wangeng.com			WEI			1100-04	I - 01	Datum: N Elevation: North: 18	594.8	2 ft		
1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9938	Project	Sect		Byr		nterchar	nge f 3rd PM	East: 117 Station: 8 Offset: 16	1345.8 311+8	0 ft 5.85		
SOIL AND ROCE DESCRIPTION		SPT Values (blw/6 in)	Qu (tst)	Moisture Content (%)	Profile		SOIL AND ROC DESCRIPTION		Sample Type recovery	SPT Values (blw/6 in)	Qu (tsf)	Moisture
	- -	Р					%Clay	t=55.6 /=23.2- A-6 (8)				

Fax	630 953-9938 Loc	ation			Ject	ion .	A 4. A 5	JJIN	1.IN.19E.	or sru PW	. Oliset.	. 10.7	172				
Profile	SOIL AND ROCK DESCRIPTION	Depth (#)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Og (ES)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND RO		Ompth (ff)	Selliple Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture
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<u>\</u> 54	2.3	_]	Ш	5	s	0.25 P	2/	l¦i¦i				1					
i(ii	Very stiff to hard, gray SILTY CLAY to SILTY CLAY LOAM,	Į	Ш		Н			慠				+					
lil -	trace gravel	+	\ /		4					L _L (%)=37, I		1	/	ı	4		
		1	χ	7	6	2.13 B	17				ravel=0.5- Sand=1.9-	1)	X	11	5	1.72 B	2
		55_/	/ \		10	Р.		Hili		-%	Silt=63.3	75_/	٧	!	6	•	
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	1 (9/)=00 D (9/)=47	1	_					0				1					
	-L _L (%)=28, P _L (%)=15- -%Gravel=4.8-		V	8	5 8	2.95	16	7	515.8 Me	dium dense, gray L0	DAM:	\		12	12 10	NP	1
i¦il	%Sand=16.4-	60_/	\wedge		9	В			mo			80 <u>/</u>	\setminus		10		"
	GENERA	L NO	ОТ	ES			L	_		WA'	TER LE	/EL	D	ΑT	Α		
Begin	Drilling 06-22-2014	Com		_		0	6-22	-20	14	While Drilling	₹		Ro	tary	y was		
	ng Contractor Wang Testing S									At Completion of Dril			l in	the	e bor	ehol	е
Driller	r N&R Logger A ng Method 2,25" HSA to 15', m									Time After Drilling Depth to Water	<u>₹</u> N.						
	backfilled upon completion			-	nere:	arter.	LOOK	mg.		The stratification lines between soil types; the			xima	ate bo	oundary	у	
	www.man.upan.vampanan									Detween soil types; the	actual transit	ion ma	sy De	gra	auai.		





Notes:

Boring Log 0589-B-01 Station and Offset along ₺ Jackson Exit Ramp are: Sta. 8282+22.07, Offset 15.66′ Rt.

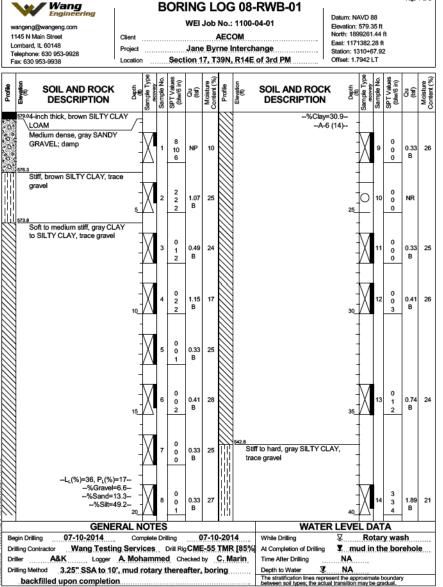
COUNTY

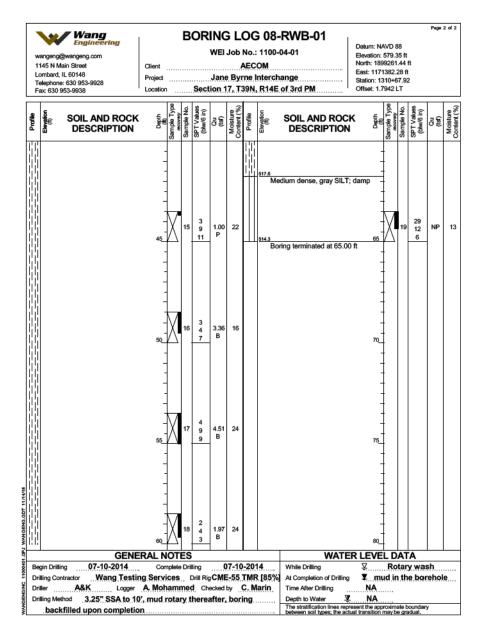
COOK 825 480

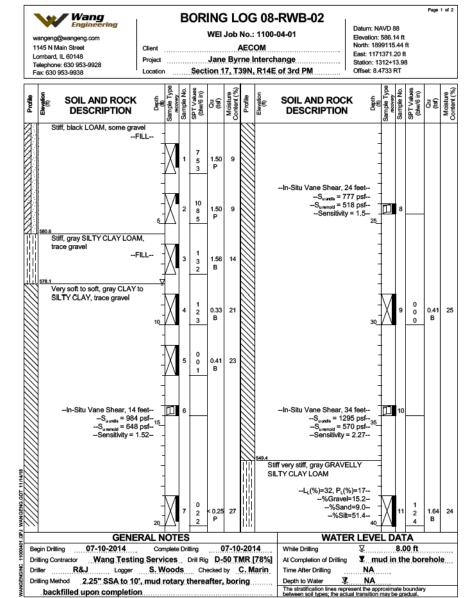
CONTRACT NO. 60X94



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PLOT SCALE = NTS	DRAWN - LFP	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -



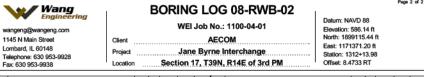




Boring Log 08-RWB-01 Station and Offset along & Jackson Exit Ramp are: Sta. 8283+04.84, Offset 27.79' Lt. Boring Log 08-RWB-02 Station and Offset along # Jackson Exit Ramp are: Sta. 8284+52.34, Offset 23.85' Lt.

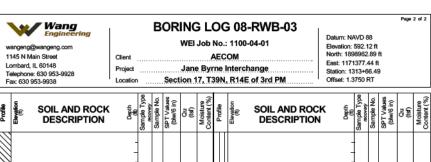


	USER NAME = wjcollett:	DESIGNED - KRS CHECKED - DJG	REVISED - REVISED -	STATE OF ILLINOIS	
>	PLOT SCALE = NTS	DRAWN - LFP	REVISED -	DEPARTMENT OF TRANSPORTATION	
	PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -		



		ne: 630 953-9928 953-9938	Location		Sec				, R14E of 3rd PM	Station: 13 Offset: 8.4		98		
Profile	Elevation (f.)	SOIL AND ROCK DESCRIPTION	Depth	Sample Lype recovery	SPT Values (blw/6 in)	on (jsg)	Moisture Content (%)	Profile	SOIL AND ROODESCRIPTION		Sample Type recovery Sample No	SPT Values (blw/6 in)	Qu (tst)	Moisture Content (%)
		%Clay=2 A-6 (524.4 Medium stiff, gray CLAY	-				
			-	1	2 0	0.66	20			- - -	\ \ 1	- 4	0.74	42
	539.4		45 <u>/</u> - -	V	3	В			821.1 Boring terminated at 65.0	65 0 ft	/ \	7	В	
	С	ery stiff to hard, gray SILTY LAY LOAM to SILTY LOAM ace gravel		/	4					-				
			50 <u></u>	<u>`</u> \∎¹	3 8	3.94 B	18			70 <u> </u>				
			-	4						- - - -				
			55_	1	4 8 11	4.35 B	15			75 <u> </u>				
11/14/18			-							- - -				
WANGENGING 11000401.0PJ WANGENG.GDT 11/14/18			60	7	5 9 14	4.51 B	18			- - 80_				
9			RAL NO							R LEVE				
1 Pe	gin Drill illing Co	ling 07-10-2014 ontractor Wang Testin	-		Orilling)7-10 50 TI			<u></u> ▼		00 ft he bo		
SE Dr	illing Co iller	R&J Logger								NA NA	est.001.1	ile DO	GIIO	e
Dr SEN	illing Me								Depth to Water	Z NA				
Š	bac	kfilled upon completion	n						The stratification lines re between soil types; the a	resent the app tual transition	roximate may be o	boundar radual.	У	

DESCRIPTION	Page 1 of	t	12 ft 2.89 f 44 ft 66.49	592.1 98962 1377.4 313+6	Datum: No Elevation: North: 189 East: 117' Station: 1: Offset: 1.3	-RWB-03 04-01 lange of 3rd PM	No.: 1100 NECOM ne Interd	Job By	WEI			Client Project	Wang Engineering ngeng@wangeng.com 5 N Main Street hard, IL 60148 sphone: 630 953-9928 630 953-9938	11 Lo Te
	(tsf) Moisture	SPT Values (blw/6 in)	Sample No.	Sample Type	(the		Profile Elevation (ft)	Moisture Content (%)	Qu (tsf)	SPT Values (blw/6 in)	Sample Type recovery Sample No.	Depth (#)	DESCRIPTION	
Marie Mari).25 : B	0	9	X	- - - -			6	NP	18	1	NT-/ VEL -	PAVEME 99.912-inch thick CONCRETEPAVEME Dense, brown SANDY GRA'BASE COUR	4 4
Soft to medium stiff, gray CLAY to SiLTY CLAY, trace gravel	0.25 : P	o <	10	X	25_			4	NP	7	2	· †	medium SAND, trace gravel	
Soft to medium stiff, gray CLAY to SiLTY CLAY, trace gravel 10).49 : B	1	11	X	-			5	NP		3	1		
S 1 0.82 16).66 B	ō	12	X	30_			25		1	4	. ⊣ .		
15					- - - -			16		1	5	1		
GENERAL NOTES GENERAL NOTES Begin Drilling 07-10-2014. Complete Drilling 07-10-2014. While Drilling V Rotary wash Drilling Contractor Wang Testing Services. Drill Rig D-50 TMR [78%] At Completion of Drilling V mud in the bore!).57 : B	0	13	X	35 <u> </u>			23		ő	6	15_/		
GENERAL NOTES Begin Drilling 07-10-2014 Complete Drilling 07-10-2014 While Drilling V Rotary wash Drilling Contractor Wang Testing Services Drill Rig D-50 TMR [78%] At Completion of Drilling V mud in the borel					- - -			25		0	7	-		
Begin Drilling 07-10-2014 Complete Drilling 07-10-2014 While Drilling ▼ Rotary wash Drilling Contractor Wang Testing Services Drill Rig D-50 TMR [78%] At Completion of Drilling ▼ mud in the borel).41 B	0 2	14	X	40_			26		0 2	8	20_/		
Drilling Method 2.25", SSA to 10", mud rotary thereafter, boring Depth to Water		was	tary	Ro	Ţ ▼ mi	While Drilling At Completion of Drilling Time After Drilling	//R [78% C. Marin	50 T	D- ecked	ling Drill Rig	es (Comp ng Servic S. We	n Drilling 07-10-2014 ng Contractor Wang Testin r R&J Logger	Dri



Profile	SOIL AND ROCK DESCRIPTION	Depth (ff)	Sample hype secovery Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth	Sample Type recovery	Sample No. SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		45	15	0 1 3	0.66 B	25		trac	y stiff, gray SILTY CLAY, se gravel ing terminated at 65.00 ft		X	19 5 8 14	3.94 B	21
	10.4	50	16	1 2 3	0.57 B	24				70_				
	Stiff, gray SILTY CLAY, trace gravel	55	17	2 4 6	1.88 B	22				- - - 75_				
MOI GPJ WANGENG.GDT 11/14/18	Medium dense, gray SILTY LOAM, trace gravel GENER.		18 DTES		NP 0	14	1-20	14	WATER While Drilling			ATA ary wa	ech	
Drillin Drillin	ng Contractor Wang Testing r R&J Logger ng Method 2.25" SSA to 10", backfilled upon completion	Servic S. Wo	es ods	Drill Rig	D- ecked	50 TI	MR C. N	[78%] Marin	At Completion of Drilling Time After Drilling Depth to Water The stratification lines repressibetween soil types; the actual	¥ mi NA NA	ıd in	the bo	rehol	е

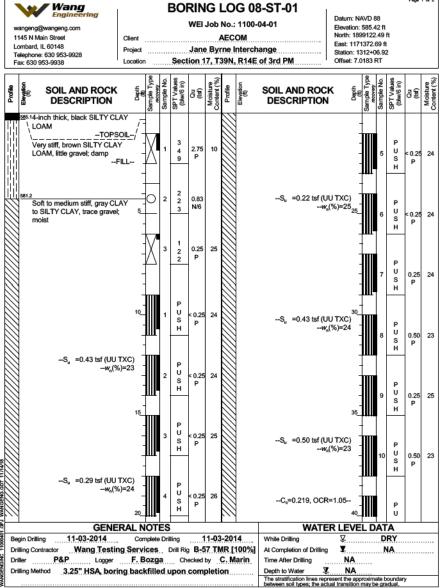
Notes: Boring Log O8-RWB-O2 Station and Offset along ₤ Jackson Exit Ramp are: Sta. 8284+52.34, Offset 23.85' Lt.

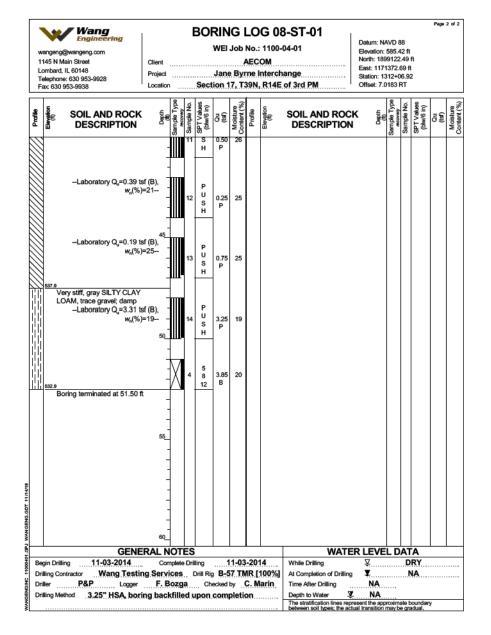
Boring Log O8-RWB-03 Station and
Offset along & Jackson Exit Ramp are:
Sta. 8286+05.39, Offset 31.68' Lt.

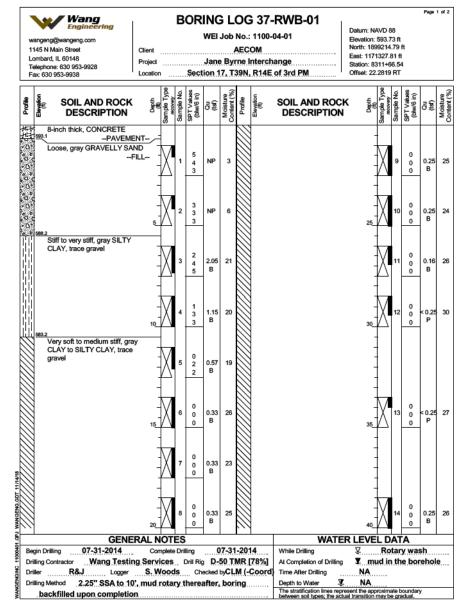


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PLOT SCALE = NTS	DRAWN - LFP	REVISED -	
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -	

BORING LOGS 3	F.A.I. RTE.	SECTION
RETAINING WALL 8 (STRUCTURE NO. 016-1727)	90/94	2014-015R&B-R
TILIAMING WALL & STROUTONE NO. 010-1727		
CHEET NO CZ 17 OF CZ 01 CHEETC		





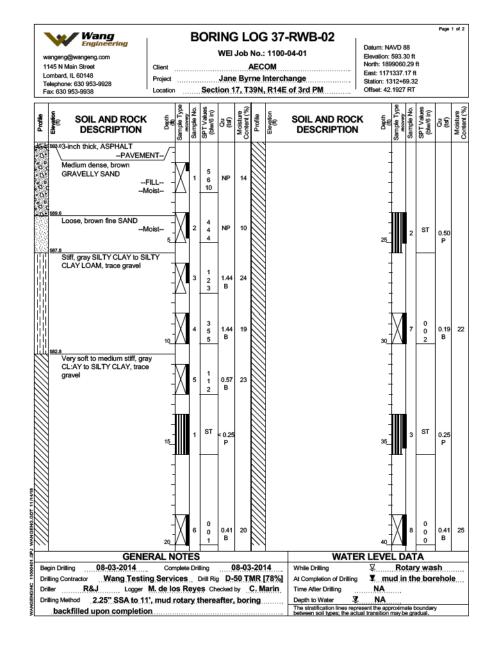


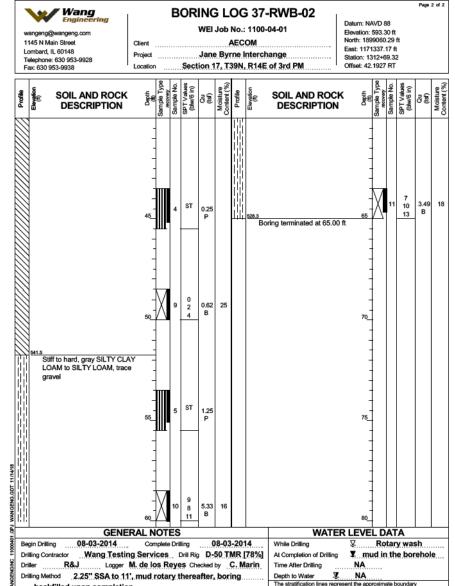
Notes:

Boring Log 08-ST-01 Station and Offset along ₤ Jackson Exit Ramp are: Sta. 8284+45.20, Offset 25.14' Lt. Boring Log 37-RWB-01 Station and Offset along ₤ Jackson Exit Ramp are: Sta. 8283+54.87, Offset 23.63' Rt.



DESIGNED - KRS	REVISED -
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DRAWN - LFP	REVISED -
CHECKED - DJG	REVISED -
	CHECKED - DJG DRAWN - LFP





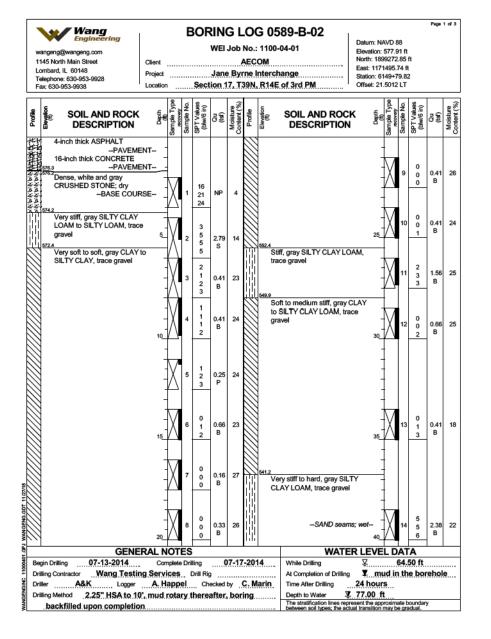
Notes.

Boring Log 37-RWB-01 Station and Offset along & Jackson Exit Ramp are: Sta. 8283+54.87. Offset 23.63' Rt. Boring Log 37-RWB-02 Station and Offset along & Jackson Exit Ramp are: Sta. 8285+08.25, Offset 9.13' Rt.



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PLOT SCALE = NTS	DRAWN - LFP	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -

backfilled upon completion



11 Lo	145 Nort ombard, elephone	Wang Engineering @wangeng.com th Main Street IL 60148 e: 630-953-9928 953-9938	Client Project Location				WEI	Job By	No AE	G 0589-B-02 .: 1100-04-01 COM Interchange	Datum: No Elevation: North: 188 East: 117 Station: 6 Offset: 21	577.9 99272 1495.	91 ft 2.85 .74 f 79.8	ft	Page	2 of 3
Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (#)	Sample Type	Sample No.	SPT Values (blw/6 in)	gg (gg)	Moisture Content (%)	Profile	SOIL AND ROCI DESCRIPTION		Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture
			45	X	15	4 5 7	5.25 B	13		Brown SANDY GRAVEL; saturated 511.2 Dense, gray SILTY LOAM, gravet; moist	65	X	19	4 4 5 5	NP	13
			50_ -	X	16	5 7 9	2.30 B	22		506.2 Dense, gray SANDY LOAI	70_ - - - - - - -	X	20	12 14 22	NP	14
	<u>521.2</u>		- - - - 55_ -	X	17	3 5 6	2.54 B	23		gravel; wet	75_	X	21	17 21 21	NP	13
	LC	edium dense, gray SILTY)AM, trace gravel; damp to oist	60	X	18	3 5 5	NP	13	2 0 0 0	Brown and gray, SANDY GRAVEL; saturated	, trace	X	22	13 19 21	NP	21

WATER LEVEL DATA

Time After Drilling
Depth to Water

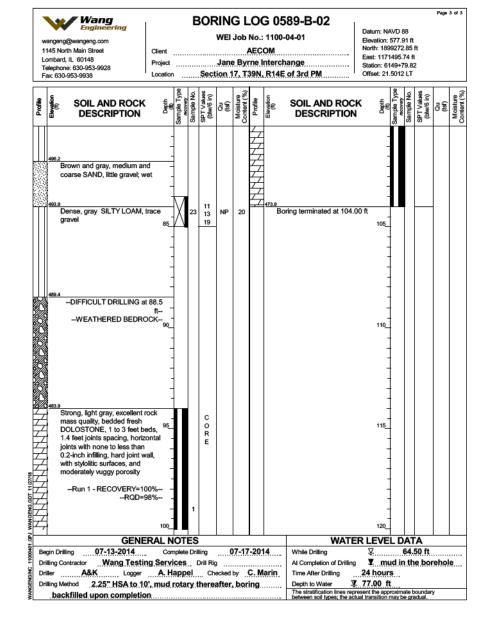
24 hours

77,00 ft

While Drilling

Depth to Water The stratification line

64.50 ft



Notes:

Boring Log 0589-B-02 Station and Offset along \(\mathbb{B} \) Jackson Exit Ramp are: Sta. 8282+84.63, Offset 140.11' Lt.

COUNTY

COOK 825 485

CONTRACT NO. 60X94



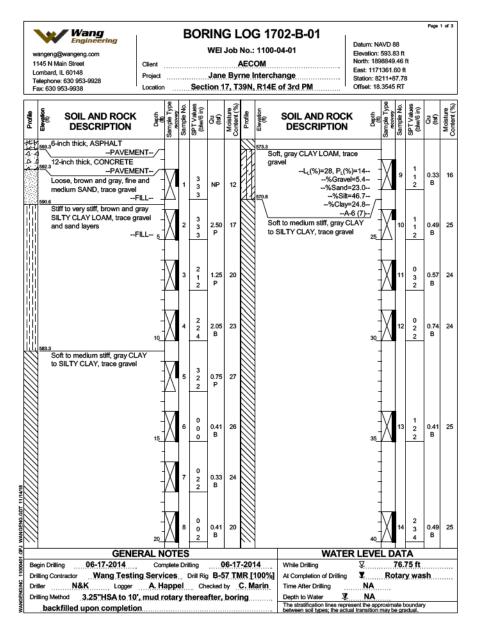
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	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - LFP	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -

GENERAL NOTES

Begin Drilling 07-13-2014 Complete Drilling 07-17-2014

Drilling Contractor Wang Testing Services Drill Rig
Driller A&K Logger A. Happel Checked by C. Marin

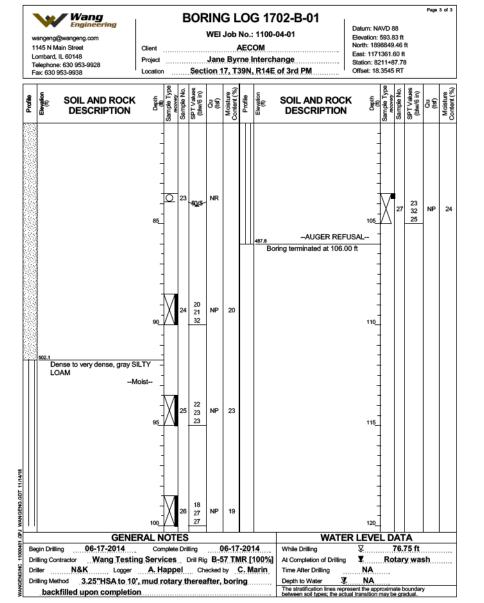
Drilling Method 2.25". HSA to 10', mud rotary thereafter, boring.



SOIL AND ROCK DESCRIPTION	Depth (#)	Sample No.	in (in	3							
		S S	Chw/6 in) Qu (tsf)	Moisture Content (%	Profile	SOIL AND ROC DESCRIPTION		Sample Type recovery Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture
892.1 Stiff, gray SILTY CLAY LO/ SILTY LOAM, trace gravel	AM to -	15	3 4 6 B	9 17		527.1	- - - - 65_ -	1	5 7 14	0.75 P	222
	50 50	16	2 4 1.36 3 B	9 16		Medium stiff to very stiff, g CLAY, trace gravel	ray - - - - - 70_ - -	2	5 7 8	2.21 B	24
trace gravel	55	17	4 6 0.48 6 B	9 21		517.1 Medium dense to very der	75_ - - - -	2	3 1 4 5	0.82 B	30
	60	18	5 9 1.00 17	0 22		gray, fine SAND and SILT laminations	-Moist - - 80_	2	11	NP	22
GENI Begin Drilling 06-17-2014	ERAL NOT Complet		ng	06-1	7-20	WATE 14 While Drilling	R LEVE				

¥ NA

Depth to Water The stratification line



Notes:

Boring Log 1702-B-01 Station and Offset along ₺ Jackson Exit Ramp are: Sta. 8287+18.90, Offset 16.41' Lt.

COUNTY

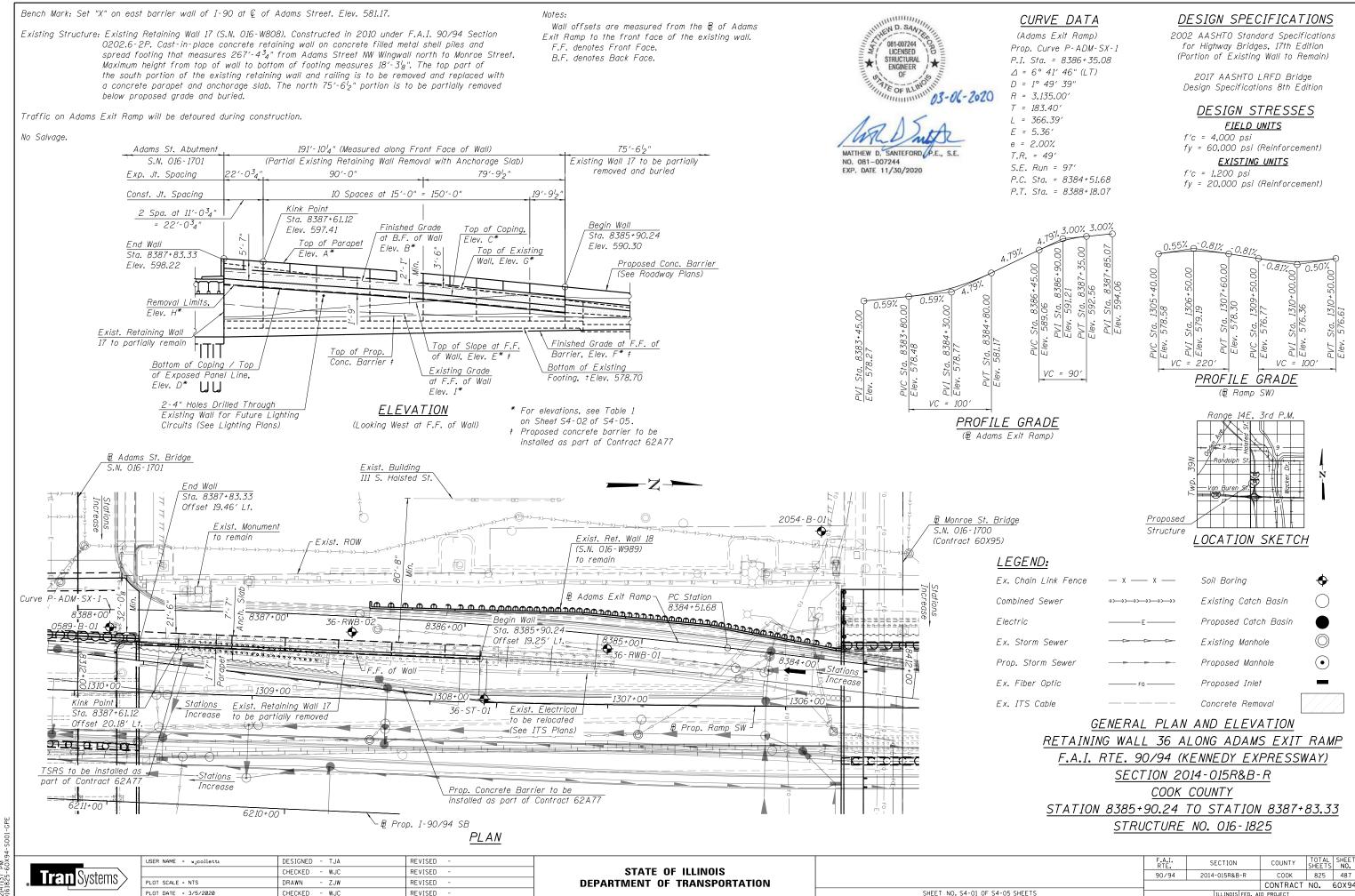
COOK 825 486

CONTRACT NO. 60X94



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	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - LFP	REVISED -
PLOT DATE = 3/5/2020	CHECKED - DJG	REVISED -

Drilling Method 3.25"HSA to 10', mud rotary thereafter, boring



GENERAL NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price hid for the work.
- 3. Concrete Sealer shall be applied to exposed front face surfaces of the anchorage slab, parapet, and existing front face of retaining wall. Protective Coat shall be applied to the top and interior face of parapet above groundline.
- 4. Slipforming of parapets is not allowed.
- 5. The Contractor shall field verify locations of existing underground utilities. The Contractor shall take all precautions to protect existing utilities during construction of the wall. Any damage to the existing utilities shall be responsibility of The Contractor.
- 6. The Contractor shall exercise extreme caution during construction to make certain that construction activities, live load surcharge, and other loads applied to the structures will not have detrimental effects on the adjacent monument and building foundations. Any damage during construction shall be repaired by the Contractor at his expense and no charge to the department. Driving piles and temporary sheet piling is not allowed.
- 7. The Contractor shall provide vibration and displacement monitoring at the locations specified in the Special Provision for Construction Vibration Monitoring and Monitoring Adjacent Structures, to ensure that removal/construction activities in the vicinity of the structures do not have detrimental effects on building foundations. No additional compensation shall be provided to the Contractor for alternative means and methods, or additional precautionary measures, required during removal/ construction activities to satisfy these requirements. See Contract Special Provisions for details.
- 8. Wall repair locations are approximate and were determined from field inspection performed at the time of plan preparation. The necessary adjustments based on current field conditions will be made at time of construction. Such variations shall not be cause for additional compensation for a change in the scope of work. However, the Contractor will be paid for the actual quantity furnished at the unit price bid for the work.
- 9. The Contractor shall take precautions not to damage existing retaining wall during the construction. Any damage to the existing retaining wall shall be repaired by the Contractor at no additional cost.

SUGGESTED CONSTRUCTION SEQUENCE

- 1. Locate existing utilities that are to remain. Contractor to coordinate any required improvements to or removals of existing utilities with utility owner(s). See Utility Location Plans and ITS Plans.
- 2. Remove railing, cladding, and portions of Existing Wall 17.
- 3. Repair cracks on the front face of existing wall to remain.
- 4. Construct Anchorage Slab and Parapet.
- 5. Install Roadway pavement (See Roadway Plans).
- 6. No portions of the wall shall be compromised by excavation for other elements of work, including the West abutment of structure 016-1701, under the contract, and adjacent structures.

TABLE 1 - WALL ELEVATIONS

Station	Offset	Elevation A	Elevation B	Elevation C	Elevation D	Elevation E	Elevation F	Elevation G	Elevation H	Elevation I
8385+90.24	19.25′ Lt.	590.30	586.80	584.72	582.97	587.48	578.04	586.58	583.09	581.79
8386+10.14	19.84′ Lt.	591.26	587.76	585.68	583.93	588,24	577.88	587.74	584.06	581.84
8386+40.33	20.48′ Lt.	592.72	589.22	587.14	585,39	587.18	577.59	589.51	585,51	581.92
8386+70.53	20.84′ Lt.	594.11	590.61	588.53	586.78	586.12	<i>577.30</i>	591.28	586.90	582.61
8387+00.73	20.91′ Lt.	595.31	591.81	589.73	587.98	585.05	577.01	592.69	588,10	583.18
8387+30.93	20.69' Lt.	596.33	592.83	590.74	588.99	583.99	576.71	593.75	589.12	583.49
8387+61.12	20.18' Lt.	597.41	593.91	591.83	590.08	582.11	576.42	595.60	590.20	584.51
8387+61.12	19.35′ Lt.	597.41	593.91	591.83	590.08	582.11	576.42	595.60	590.20	584.51
8387+83.33	19.46′ Lt.	598.22	594.72	592 . 64	590.89	581.76	576.22	596.96	591 . 01	585.50

Elevation A - Top of Parapet

Elevation B - Finished Grade at B.F. of Wall

Elevation C - Top of Coping

Elevation D - Bottom of Coping / Top of Exposed Wall Line Elevation E - Top of Slope at F.F. of Wall ***

Elevation F - Finished Grade at F.F. of Barrier ***

Elevation G - Top of Existing Wall Elevation H - Removal Limits

Elevation I - Existing Grade at F.F. of Wall

* Elevations just to the right of joint

** Elevations just to the left of joint

STATION 8385+90.24 TO 8387+83.33 BUILT 20__ BY STATE OF ILLINOIS F.A.I. RTE. 90/94 SEC. 2014-015R&B-R LOADING HL-93 STR. NO. 016-1825

NAME PLATE See Std. 515001

TOTAL BILL OF MATERIAL

Item	Unit	Total Quantity
Concrete Removal	Cu. Yd.	59
Structure Excavation	Cu. Yd.	393
Concrete Superstructure	Cu. Yd.	122.7
Protective Coat	Sq. Yd.	97
Reinforcement Bars, Epoxy Coated	Pound	15,100
Name Plates	Each	1
Concrete Sealer	Sq. Ft.	2,665
Epoxy Crack Injection	Foot	78
Steel Railing Removal	Foot	268
Removal Of Ornamental Cladding	Foot	238

INDEX OF SHEETS

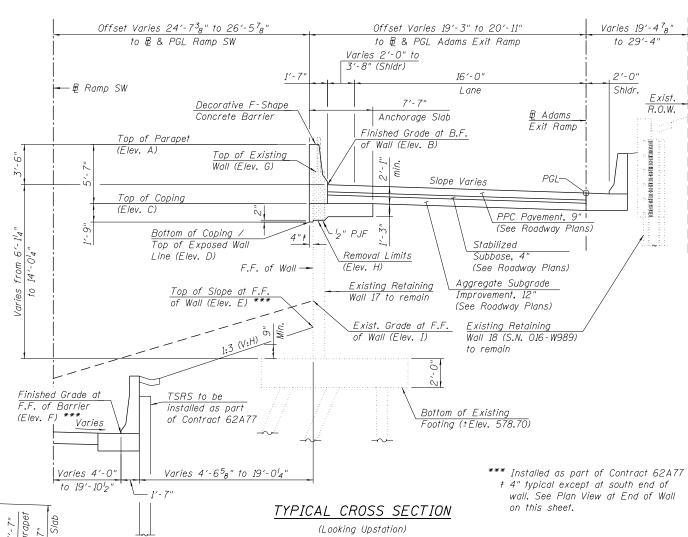
S4-01 General Plan and Elevation

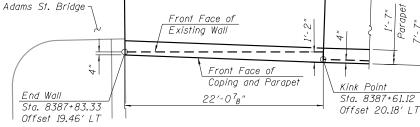
54-02 General Data

S4-03 Repair and Removal Plans

54-04 Parapet and Anchorage Slab Plan and Elevation Parapet and Anchorage Slab Details S4-05

S4-06 Architectural Details





PLAN VIEW AT END OF WALL

USER NAME = wjcolletti DESIGNED - TJA REVISED CHECKED - WJC REVISED **Tran** Systems` - ZJW REVISED PLOT DATE = 3/5/2020 CHECKED - WJC REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

GENERAL DATA RETAINING WALL 36 (STRUCTURE NO. 016-1825) SHEET NO. S4-02 OF S4-05 SHEETS

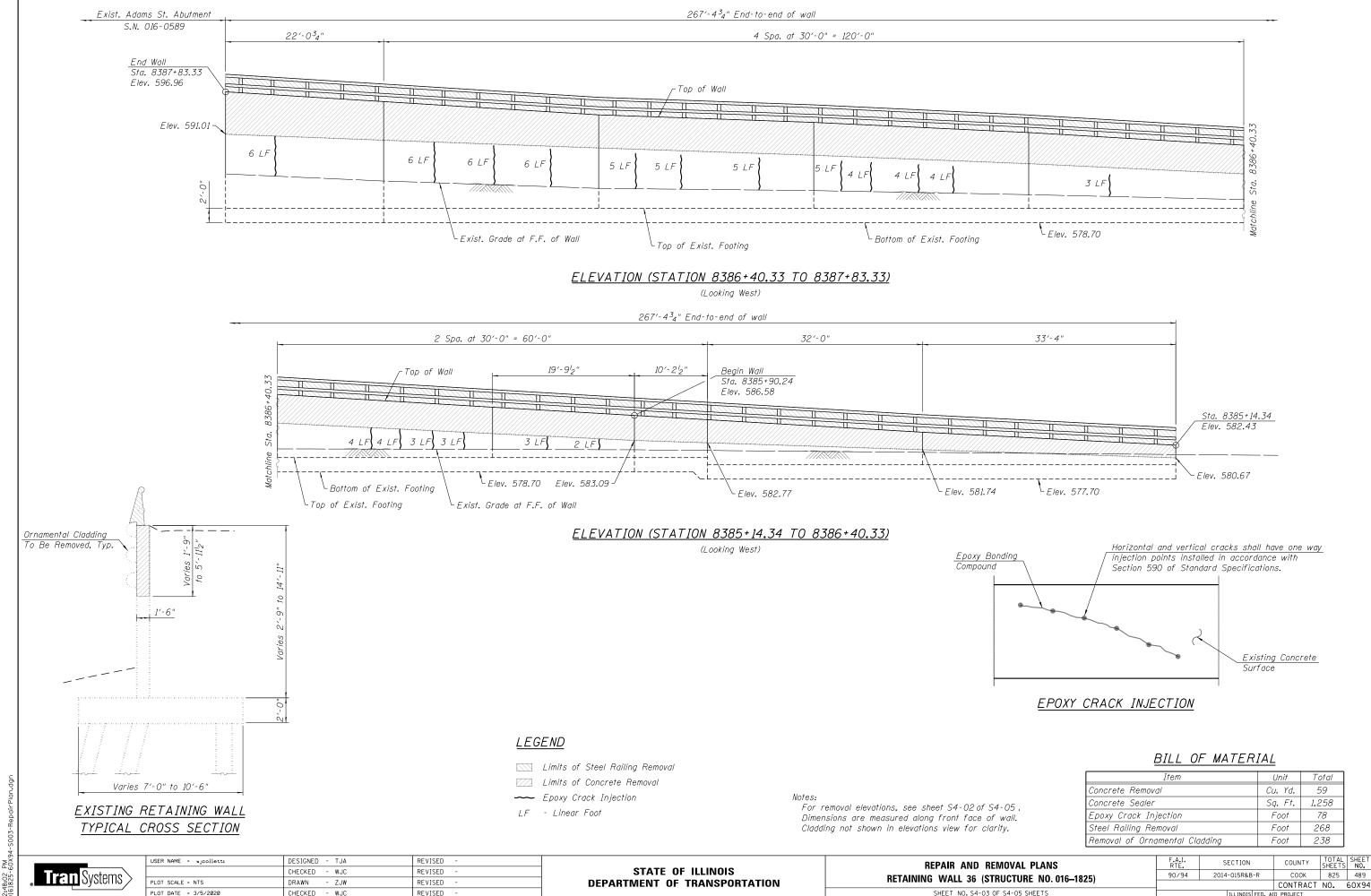
COUNTY 90/94 2014-015R&B-R COOK 825 488 CONTRACT NO. 60X94

LEGEND:

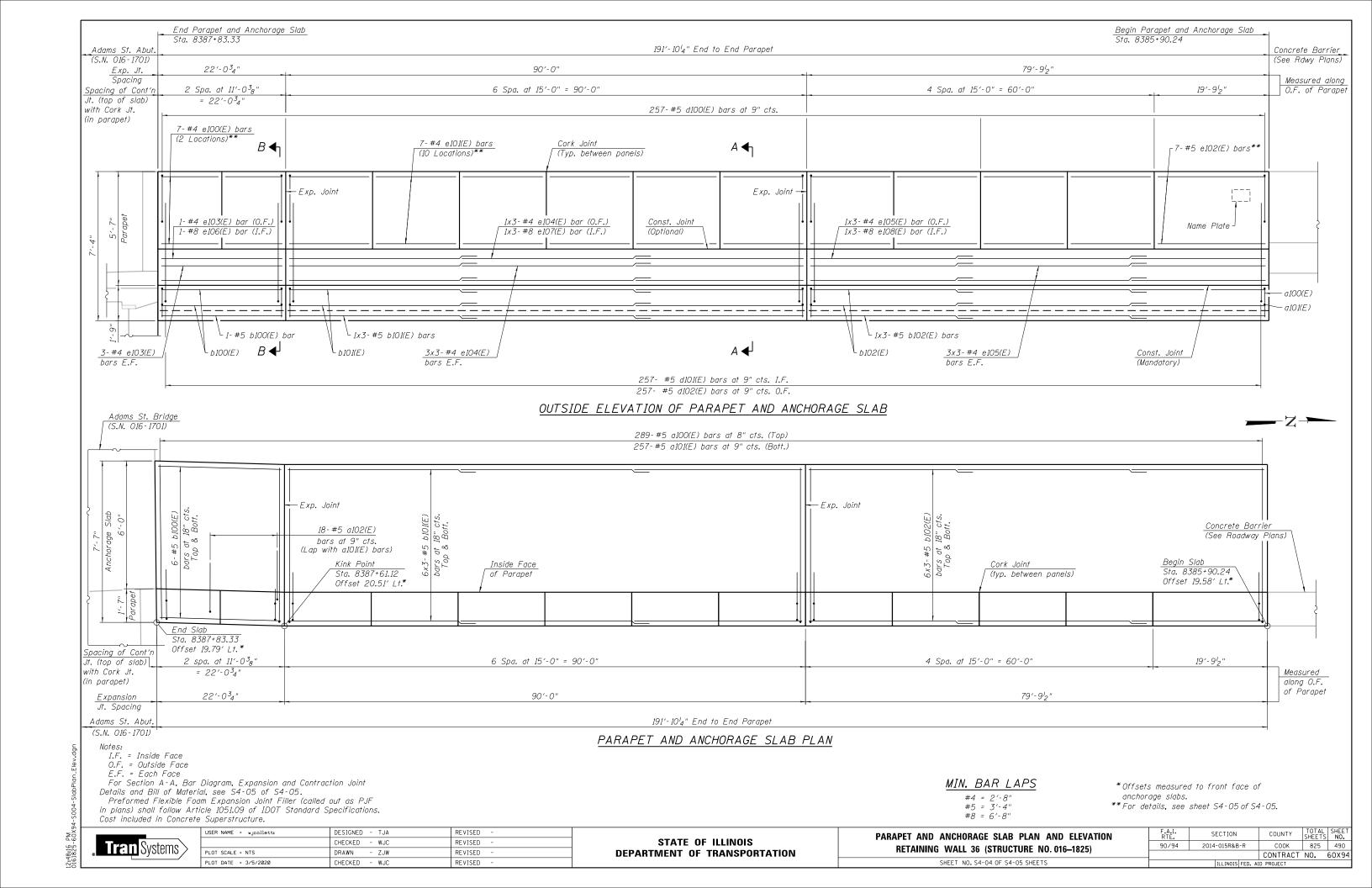
B.F. - denotes Back Face.

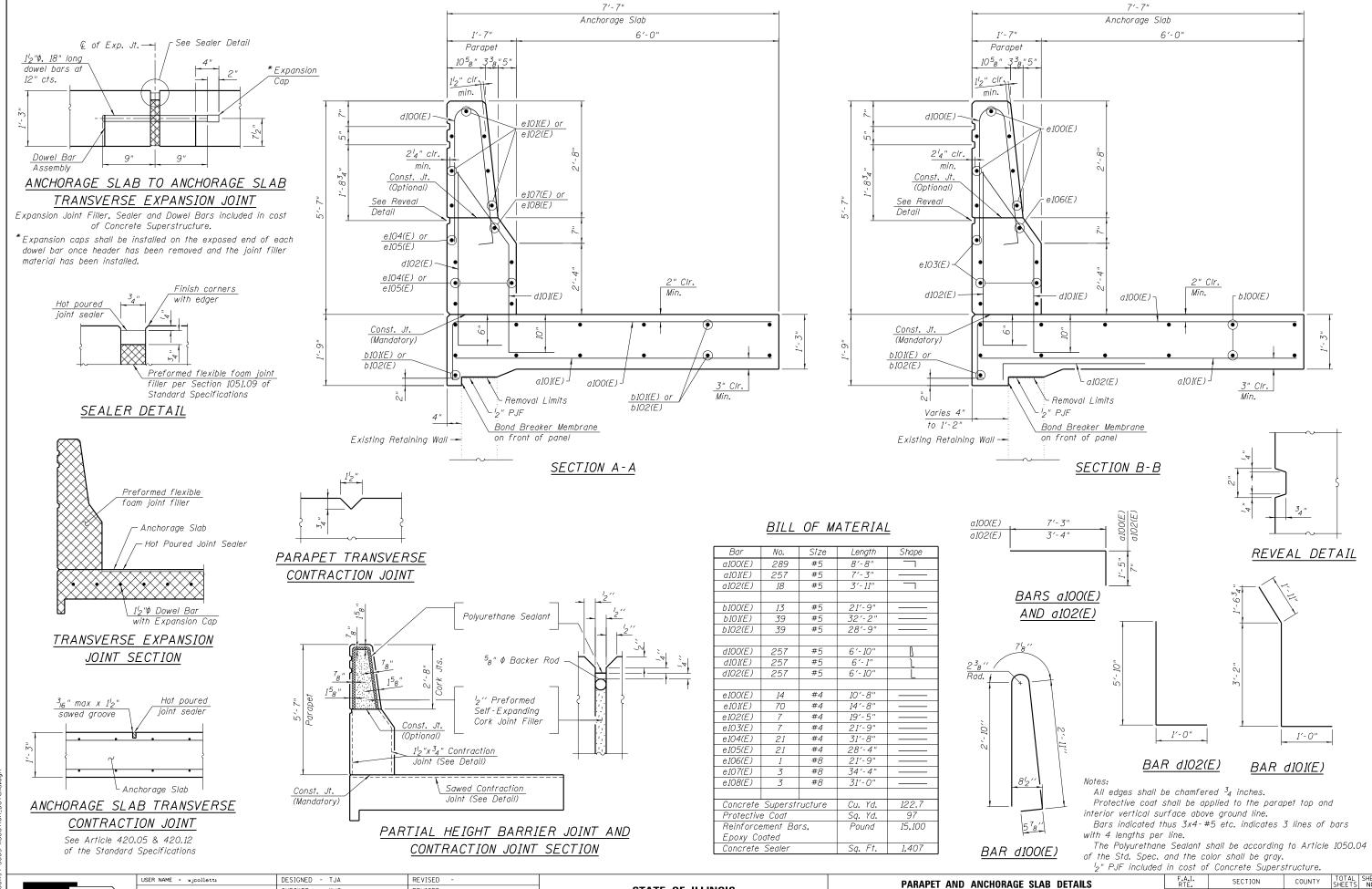
F.F. - denotes Front Face.

Concrete Removal



REVISED





STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

2014-015R&B-R

RETAINING WALL 36 (STRUCTURE NO. 016-1825)

SHEET NO. S4-05 OF S4-05 SHEETS

COOK 825 491

CONTRACT NO. 60X94

Tran Systems

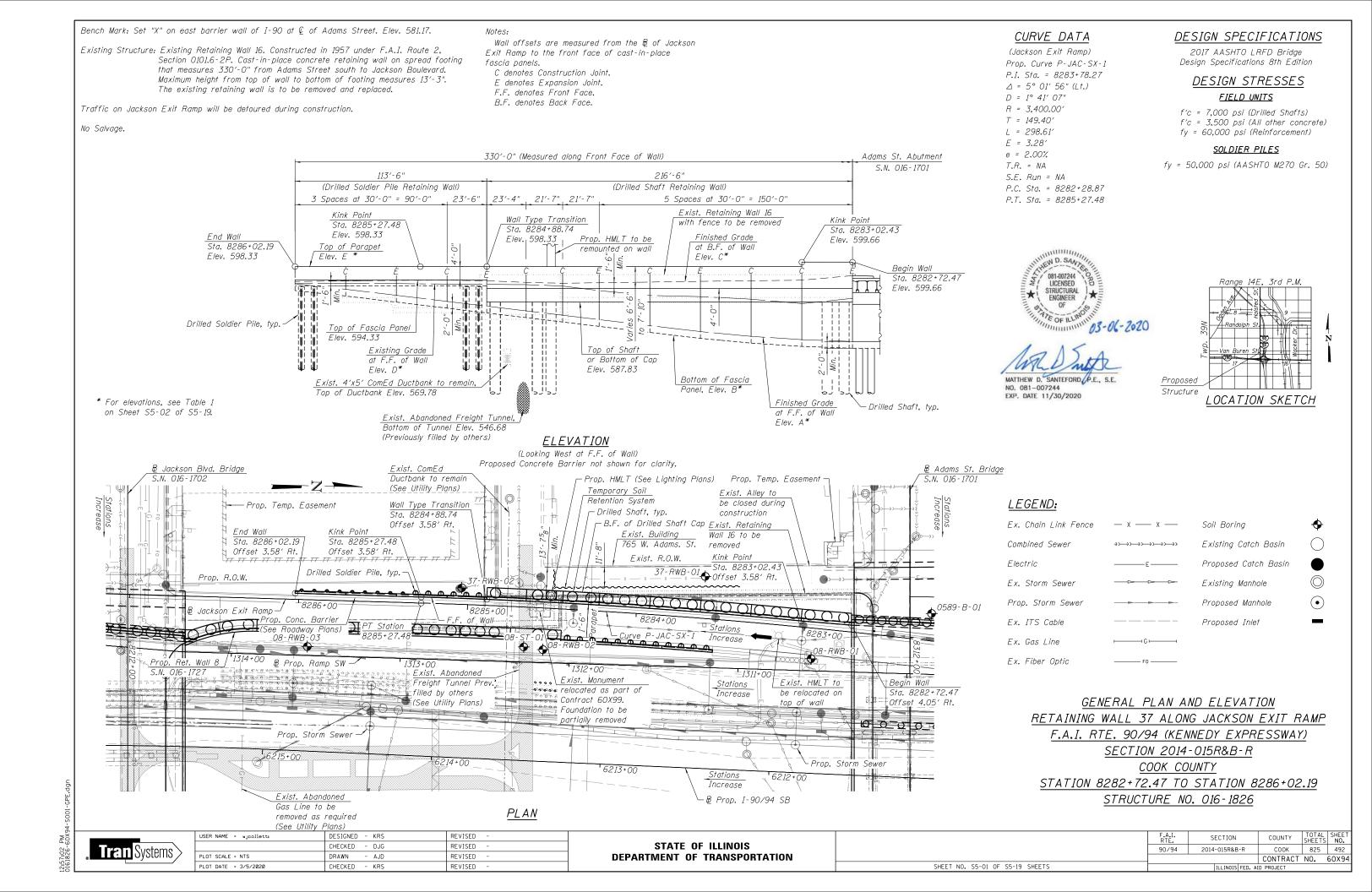
PLOT DATE = 3/5/2020

CHECKED - WJC

CHECKED - WJC

REVISED

REVISED



GENERAL NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. The Contractor shall exercise extreme caution during construction to make certain that construction activities, live load surcharge and other loads applied to the structures will not have detrimental effects on the adjacent building foundations. Any damage during construction shall be repaired by the Contractor at his expense and no charge to the department. Driving piles and temporary sheet piling is not allowed.
- 3. The Contractor shall provide vibration and displacement monitoring at the locations specified in the Special Provisions for Construction Vibration Monitoring and Monitoring Adjacent Structures, to ensure that removal/construction activities in the vicinity of the structures do not have detrimental effects on building foundations. No additional compensation shall be provided to the Contractor for alternative means and methods, or additional precautionary measures, required during removal/ construction activities to satisfy these requirements. See Contract Special Provisions for details.
- Drilled shaft construction above existing grade shall not be paid separately but shall be included with Drilled Shaft in Soil.
- 5. Slipforming of parapets is not allowed.
- The Contractor shall field verify locations of existing underground utilities. The
 Contractor shall take precautions to protect existing utilities during construction of
 the wall. Any damage to the existing utilities shall be the responsibility of the
 Contractor.
- 7. Concrete for the Drilled Shafts shall be in accordance with Section 516 of Standard Specifications and shall have the minimum compressive strength of 7,000 psi prior to excavation in front of shafts and installation of lagging system.
- 8. For drilled shaft locations where permanent casing is required as shown on the plans, the casing will be paid for under Permanent Casing. If Contractor elects to use permanent casing for ease of construction in locations where it is not required on the plans, the casing will not be paid for separately and is included in Drilled Shaft in Soil.
- 9. Wall to be built along straight chords between construction and expansion joints.
- 10. Concrete Sealer shall be applied to the exposed top, front, and back faces of the parapet, and to the exposed front faces of cap and fascia panels.
- 11. Limited groundwater elevation data is available in the boring logs. In addition, groundwater may also be present in deeper granular layers. The groundwater may rise in the shafts to an elevation above the top of granular layers. The Contractor shall consider this information when choosing construction methods. The Contractor will not be compensated for issues related to the groundwater elevation.
- 12. The Contractor shall take all necessary precautions not to contaminate groundwater during the drilled shaft construction operation. Contractor is responsible for the proper containment and disposal of the contaminated groundwater and spoils resulting from the Contractor's means and methods. No additional cost will be paid for this effort.
- 13. Due to the squeeze potential of the clay soils, the use of temporary casing will be required to properly construct the shafts. Casing may be pulled or remain in place, as determined by the Contractor at no cost to the Department.
- 14. A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
- 15. The contractor shall coordinate the construction of the proposed structure with the construction of the Proposed Adams Street Bridge and Proposed Retaining Wall 8. See MOT plan sheets and special provisions, including the Available Work Areas and Sequencing Requirements special provision, for additional construction and coordination requirements.
- 16. The Contractor shall provide a method to assure the soldier piles achieve at least the plan tip elevations. The soldier pile locations and elevations shall meet the tolerances provided in the Special Provisions. Any additional measures required to satisfy the construction tolerances will not be paid for separately but shall be included in Drilling and Setting Soldier Piles (In Soil).
- 17. Soldier piles shall be cleaned and given one shop coat of Inorganic Zinc Rich Primer. Cost included with Furnishing Soldier Piles (W Section).
- 18. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

SUGGESTED CONSTRUCTION SEQUENCE

- 1. Install temporary soil retention system along alley.
- Remove portion of Existing Retaining Wall 16 that is in conflict with the proposed drilled shafts.
- 3. Drill shafts and install soldier piles.
- 4. Remove portion of Existing Retaining Wall 16 in front of the proposed wall and excavate to install timber lagging.
- 5. Install drainage structures along Jackson Exit Ramp. (See Drainage Plans)
- 6. Construct concrete fascia panels, cap, and parapet.

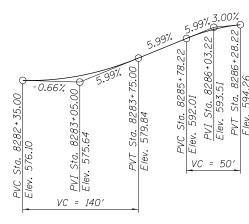
TOTAL BILL OF MATERIAL

Item	Unit	Total Quantity
Removal of Existing Structures No. 3	Each	1
Structure Excavation	Cu. Yd.	578
Concrete Structures	Cu. Yd.	224,6
Concrete Superstructure	Cu. Yd.	107.3
Stud Shear Connectors	Each	81
Reinforcement Bars	Pound	438,910
Reinforcement Bars, Epoxy Coated	Pound	25,330
Mechanical Splicers	Each	480
Name Plates	Each	1
Permanent Casing	Foot	180
Drilled Shaft in Soil	Cu. Yd.	1,885.0
Temporary Soil Retention System	Sq. Ft.	573
Furnishing Soldier Piles (W Section)	Foot	765
Drilling and Setting Soldier Piles (In Soil)	Cu. Ft.	3,731
Untreated Timber Lagging	Sq. Ft.	484
Concrete Structures (Retaining Wall)	Cu. Yd.	32.7
Concrete Sealer	Sq. Ft.	6,756
Geocomposite Wall Drain	Sq. Yd.	42
Crosshole Sonic Logging Access Ducts	Foot	1,800
Crosshole Sonic Logging Testing	Each	4
Class SI Concrete (Miscellaneous)	Cu. Yd.	123.9
Slope Inclinometer	Each	1
Pipe Underdrain for Structures 4"	Foot	330

INDEX OF SHEETS

General Plan and Flevation

33-01	General Flair and Lievarion
S5-02	General Data
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S5-04	Plan and Elevation 1
S5-05	Plan and Elevation 2
S5-06	Plan and Elevation 3
S5-07	Plan and Elevation 4
S5-08	Drilled Shaft Wall Sections and Details 1
S5-09	Drilled Shaft Wall Sections and Details 2
S5 - 10	Drilled Shaft Wall Sections and Details 3
S5-11	Drilled Soldier Pile Wall Sections and Details 1
S5-12	Drilled Soldier Pile Wall Sections and Details 2
S5-13	Architectural Details
S5 - 14	Bar Splicer Assembly and Mechanical Splicer Deta
S5 - 15	Boring Logs 1
S5-16	Boring Logs 2
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S5 - 18	Boring Logs 4
S5 - 19	Boring Logs 5



PROFILE GRADE
(# Jackson Exit Ramp)

TABLE 1 - WALL ELEVATIONS

	Station	Offset	Elevation A	Elevation B	Elevation C	Elevation D	Elevation E	Wall Type
	8282+72.47	4.05′ Rt.	<i>576.15</i>	574.15	595.66	588.91	599.66	
#	8283+02.43	3.58′ Rt.	576.70	574.70	594.29	587 . 36	599.66	
<i>‡ ‡</i>	8283+02.43	3.58′ Rt.	576.70	574.70	594.29	587.36	598.33	
	8283+32.40	3.58′ Rt.	577.68	575.68	594.14	588.67	598.33	Drilled
	8283+62.37	3.58′ Rt.	579.08	577.08	593.95	589.43	598.33	Shafts
	8283+92.34	3.58′ Rt.	580.84	578.84	593.74	589,76	598.33	Wall
	8284+22.31	3.58′ Rt.	582.63	580.63	593.54	591.23	598.33	Wan
	8284+43.87	3.58′ Rt.	583.92	581.92	593.40	592.56	598.33	
	8284+65.43	3.58′ Rt.	585.21	583 . 21	593.66	592.99	598.33	
#	8284+88.74	3.58′ Rt.	586.61	584 . 61	594.11	593.06	598.33	
<i>‡ ‡</i>	8284+88.74	3.58′ Rt.	586.61	584.61	594.11	593.06	598.33	
	8285+12.21	3.58′ Rt.	588.02	586.02	594.30	<i>593.1</i> 5	598.33	Drilled
	8285+27.48	3.58′ Rt.	588.93	586.93	594.27	593 . 22	598.33	Soldier
	8285+42.19	3.58′ Rt.	589.81	587.81	594.24	593.29	598.33	Pile
	8285+72.19	3.58′ Rt.	591.61	589.61	594.09	593 . 31	598.33	Wall
	8286+02.19	3.58′ Rt.	593.24	591.24	593.91	593,29	598.33	

Elevation A- Finished Grade at Front Face of Wall

Elevation B- Bottom of Fascia Panel

Elevation C- Finished Grade at Back Face of Wall Elevation D- Existing Grade at Front Face of Wall

Elevation E - Top of Parapet

† Elevations just to the right of joint

Elevations just to the left of joint

STATION 8282+72.47 TO 8286+02.19

BUILT 20__ BY

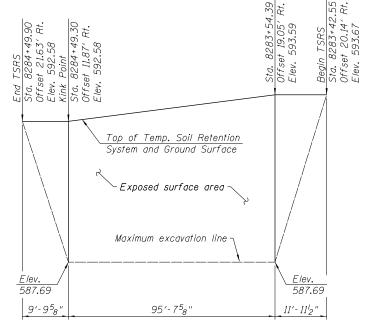
STATE OF ILLINOIS

F.A.I. RTE. 90/94 SEC. 2014-015R&B-R

LOADING HL-93

STR. NO. 016-1826

NAME PLATE See Std. 515001



TEMPORARY SOIL RETENTION SYSTEM - ELEVATION

(Unfolded View, Measured along F.F. of TSRS) For Section through TSRS, See Sheet S5-08 of S5-19



2:57:33

USER NAME = wjcolletti	DESIGNED - KRS	REVISED -
	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - AJD	REVISED -
PLOT DATE = 3/5/2020	CHECKED - KRS	REVISED -

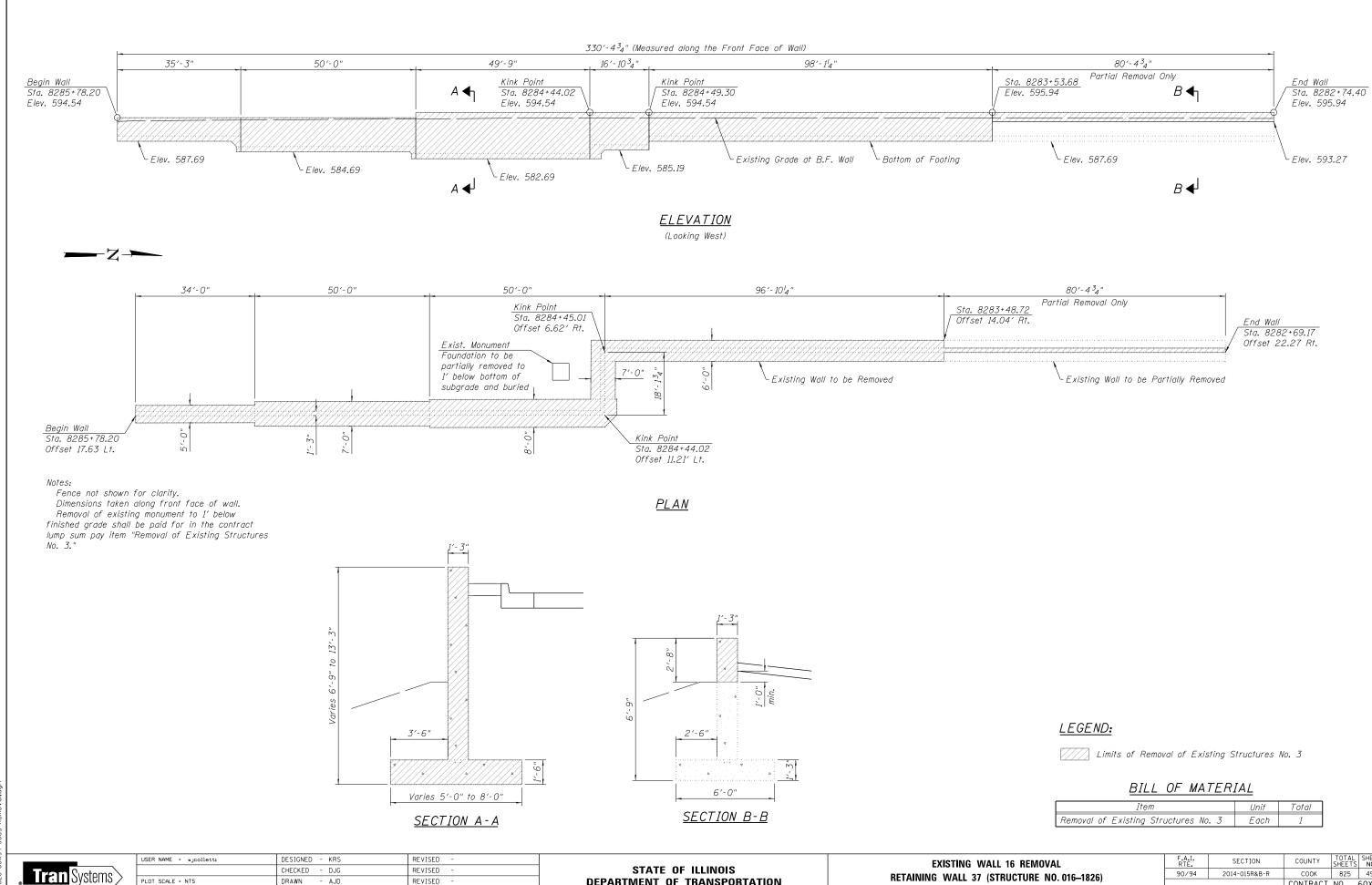
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL DATA

RETAINING WALL 37 (STRUCTURE NO. 016–1826)

SHEET NO. S5-02 OF S5-19 SHEETS

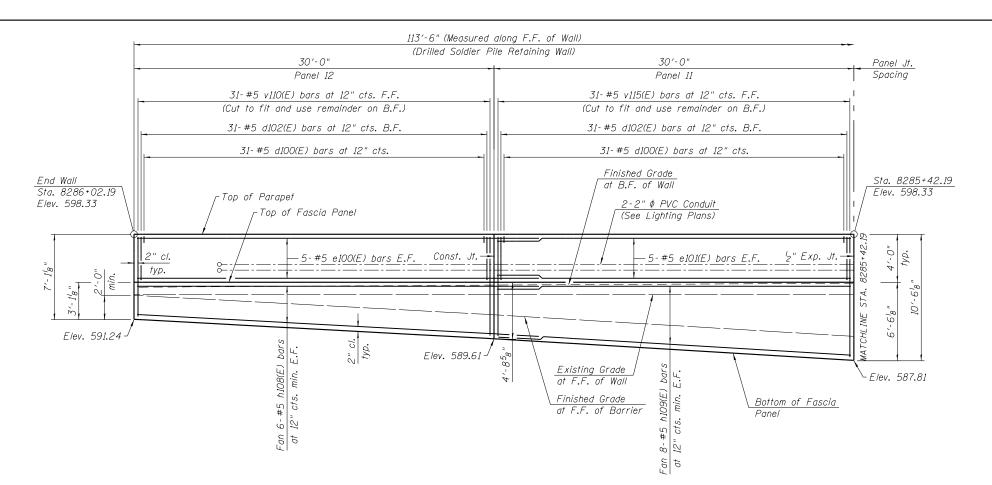
A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.				
90/94	2014-015R&B-R	COOK	825	493				
CONTRACT NO. 60X94								
TILLINOIS FED. ATD PROJECT								



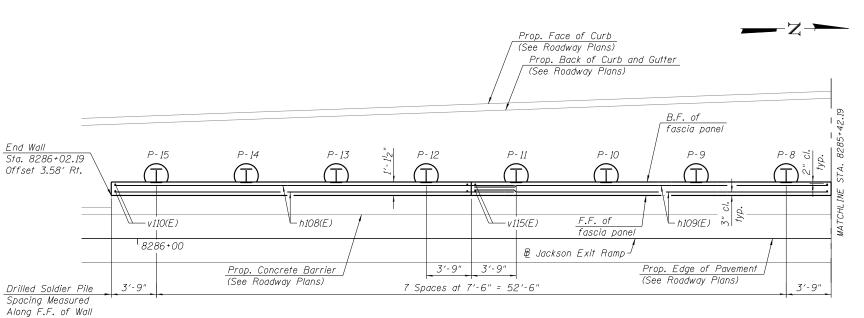
CHECKED - DJG REVISED - AJD REVISED CHECKED - KRS PLOT DATE = 3/5/2020 REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** RETAINING WALL 37 (STRUCTURE NO. 016-1826) SHEET NO. S5-03 OF S5-19 SHEETS

COOK 825 494 90/94 2014-015R&B-R CONTRACT NO. 60X94



WALL ELEVATION (Looking West)



PLAI

(Parapet reinforcement not shown for clarity)

Notes:

F.F. = Front Face.

B.F. = Back Face.

E.F. = Each Face.

For soldier pile wall cross sections and details, see Sheet S5-11 of S5-19.

For soldier pile layout, sections and details and Bill of Material, see Sheet S5-12 of S5-19.

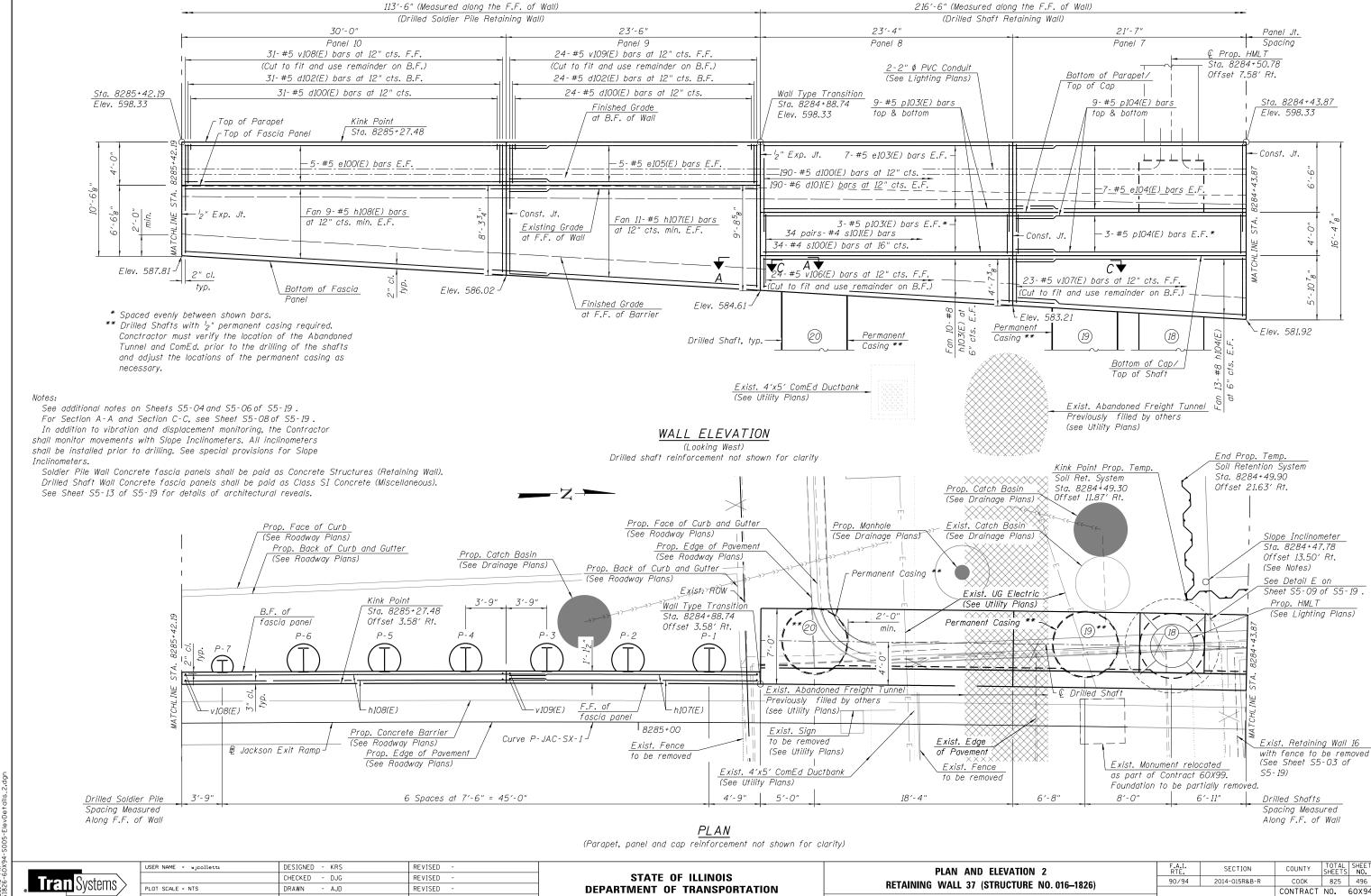
Concrete fascia panels shall be paid as Concrete Structures (Retaining Wall).

See Sheet S5-13 of S5-19 for details of architectural reveals.



USER NAME = wjcolletti	DESIGNED - KRS	REVISED -
	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - AJD	REVISED -
PLOT DATE = 3/5/2020	CHECKED - KRS	REVISED -

PLAN AND ELEVATION 1		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
RETAINING WALL 37 (STRUCTURE NO. 016-1826)	90/94	2014-015R&B-R	СООК	825	495
NETAINING WALE 37 (STRUCTORE NO. 010-1020)			CONTRACT	NO.	60X94
SHEET NO. S5-04 OF S5-19 SHEETS		ILLINOIS FED. AI	D PROJECT		

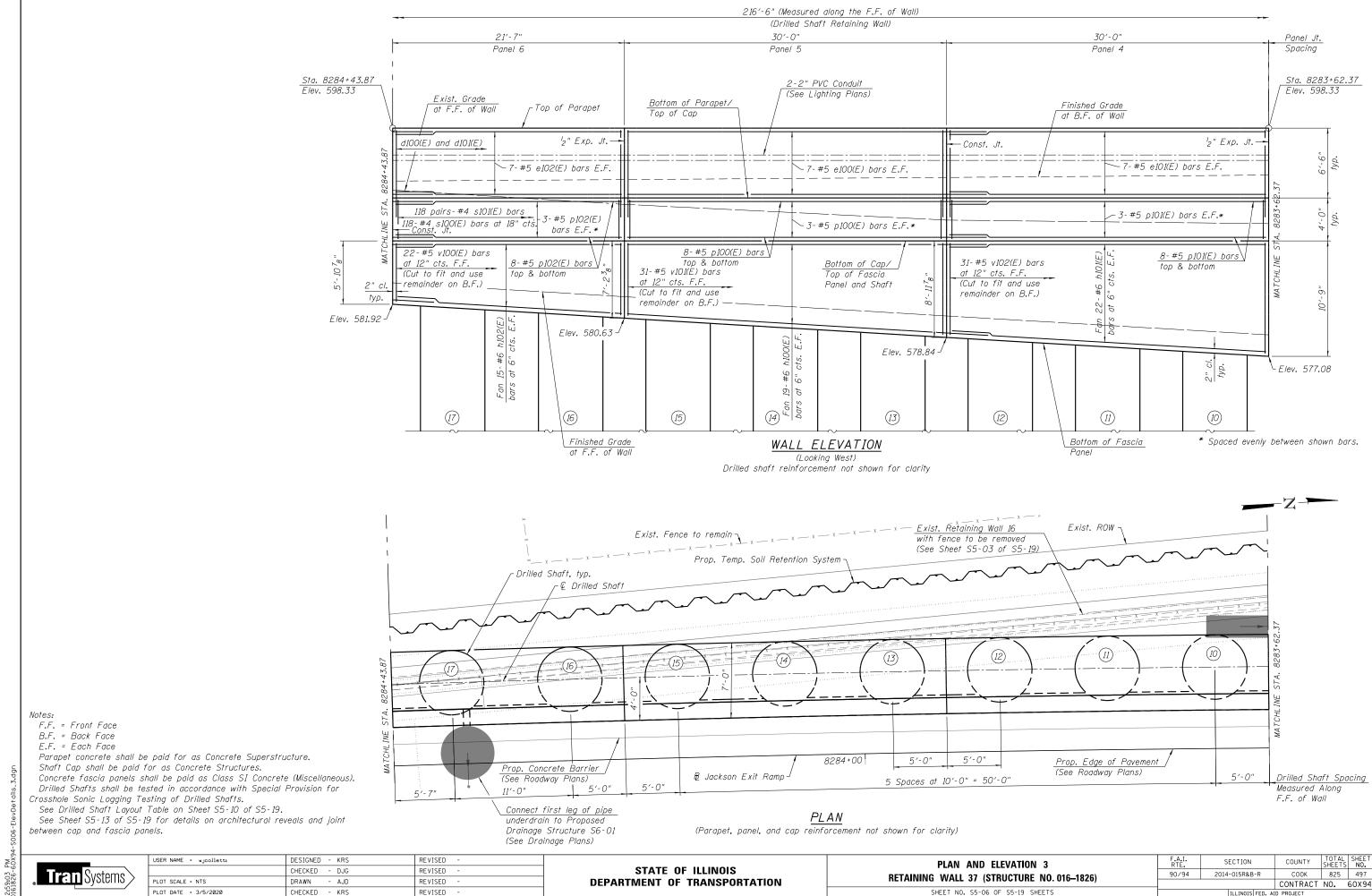


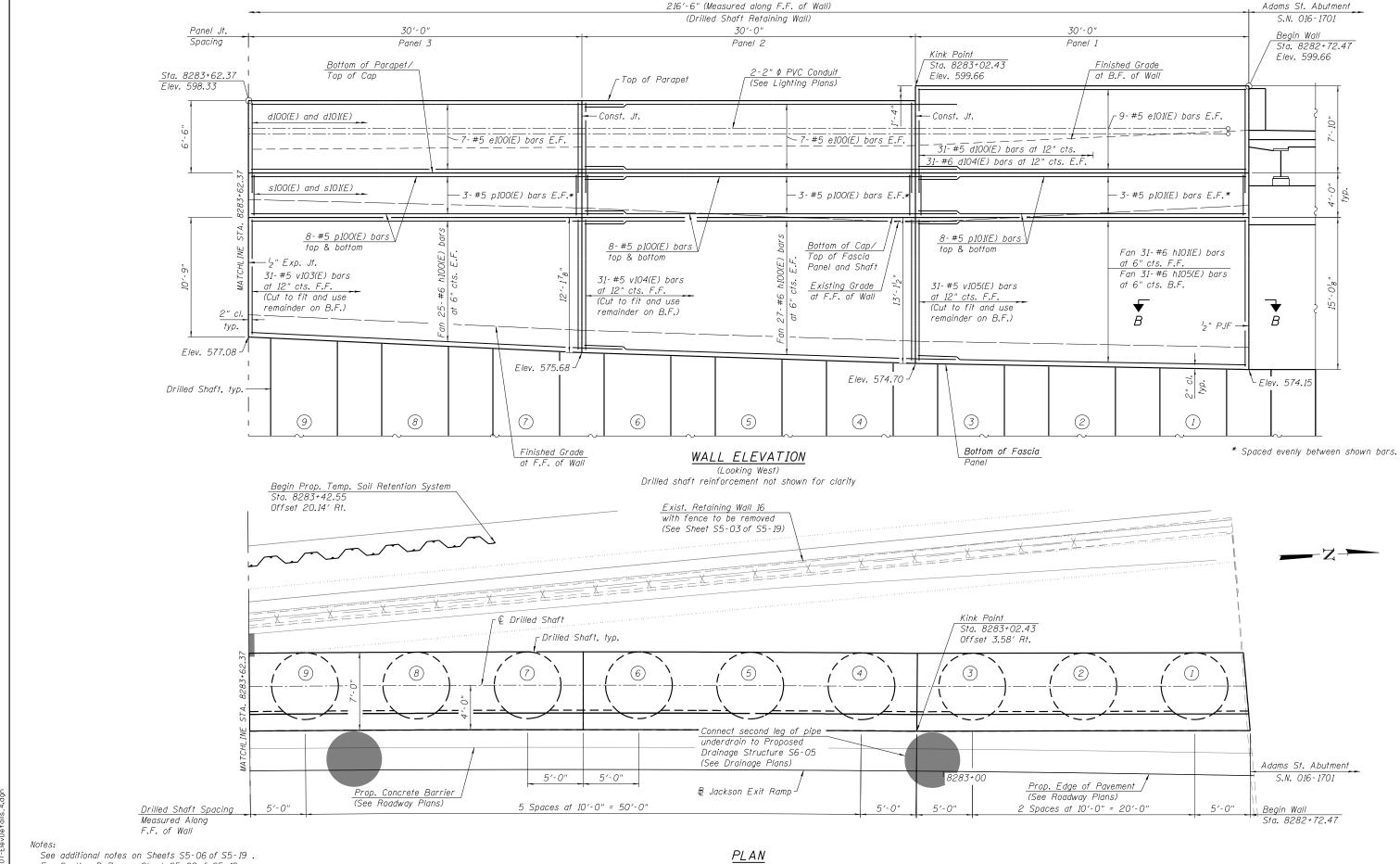
SHEET NO. S5-05 OF S5-19 SHEETS

PLOT DATE = 3/5/2020

CHECKED - KRS

REVISED





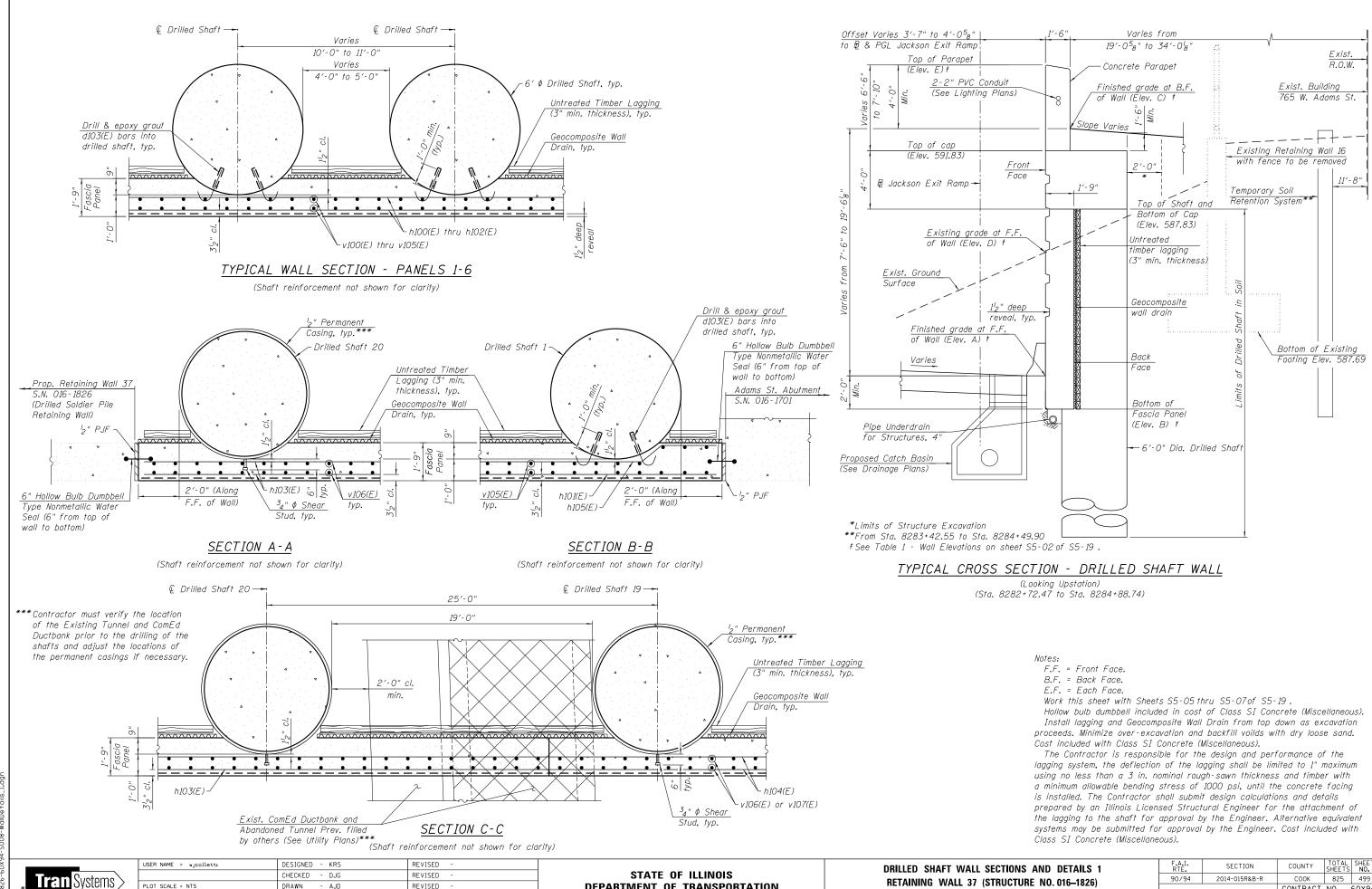
For Section B-B, see Sheet S5-08 of S5-19.

(Parapet, panel, and cap reinforcement not shown for clarity)



USER NAME = wjcollett1	DESIGNED - KRS	REVISED -
	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - AJD	REVISED -
PLOT DATE = 3/5/2020	CHECKED - KRS	REVISED -

RTE.	SECTION	COUNTY	SHEETS	NO.
90/94	2014-015R&B-R	COOK	825	498
		CONTRACT	NO.	60X94
	ILLINOIS FED. A	ID PROJECT		



DEPARTMENT OF TRANSPORTATION

REVISED

REVISED

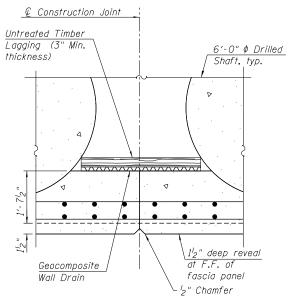
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PLOT DATE = 3/5/2020

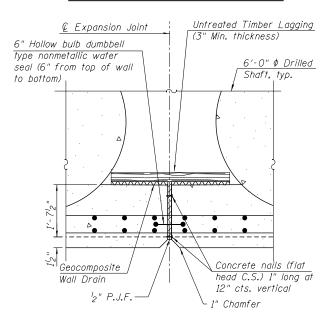
RETAINING WALL 37 (STRUCTURE NO. 016-1826)

SHEET NO. S5-08 OF S5-19 SHEETS

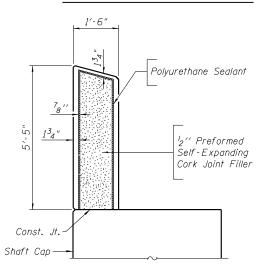
CONTRACT NO. 60X94



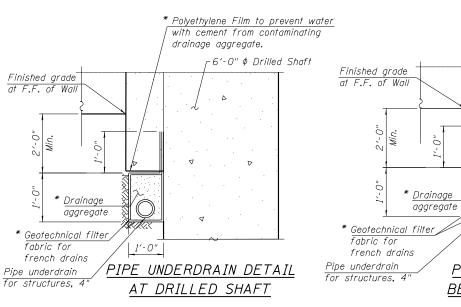
CONSTRUCTION JOINT DETAILS



EXPANSION JOINT DETAILS

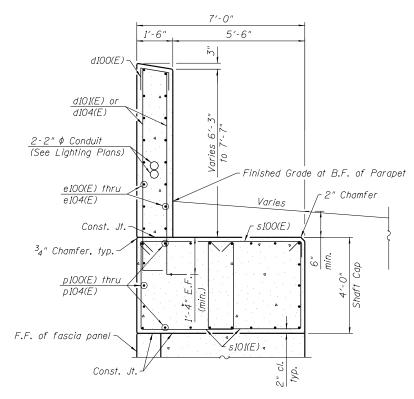


TRANSVERSE EXPANSION JOINT SECTION



* Polyethylene Film to prevent water with cement from contaminating drainage aggregate. -6'-0" ϕ Drilled Shaft Wall Drain Place ${}^3\!_4$ " gap to allow for drainage, typ. entire height of lagging Untreated timber lagging. (3" min. thickness) PIPE UNDERDRAIN DETAIL BETWEEN DRILLED SHAFTS Alternate 12 - #14 v111 bars

* Cost included with Pipe Underdrains for Structures, 4".



TYPICAL SECTION OF PARAPET AND CAP

(Shaft and fascia panel reinforcement not shown for clarity)

Notes:

F.F. = Front Face.

B.F. = Back Face.

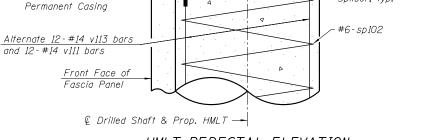
E.F. = Each Face.

Work this sheet with Sheets S5-05 thru S5-07 of S5-19 . The Polyurethane Sealant shall be according to Article 1050.04 of Std. Spec.

and the color shall be gray.

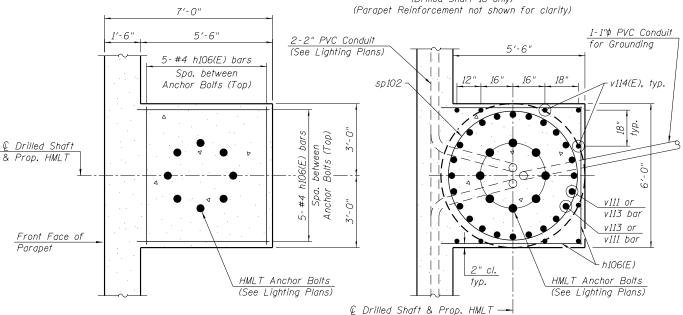
HMLT Pedestal Concrete included in the cost of Concrete Superstructure. For Anchor Rod Cage Details, see IDOT Standard BE-506.

Cost of Anchor Rod Cage and Grounding Conduit included in the cost of Concrete Superstructure.



HMLT PEDESTAL ELEVATION

(Drilled Shaft 18 only)



Elev. 598.33

Front <u>Face of</u>

Elev. 591.83

D

Parapet

2-2" PVC Conduit (See Lighting Plans)

and 12-#14 v113 bars

Top of Fascia Panel,

Drilled Shaft and

DETAIL E

SECTION D-D

(Parapet Reinforcement not shown for clarity)



USER NAME = wjcolletti	DESIGNED - KRS	REVISED -
	CHECKED - DJG	REVISED -
PLOT SCALE = NTS	DRAWN - AJD	REVISED -
PLOT DATE = 3/5/2020	CHECKED - KRS	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** DRILLED SHAFT WALL SECTIONS AND DETAILS 2 RETAINING WALL 37 (STRUCTURE NO. 016-1826) SHEET NO. S5-09 OF S5-19 SHEETS

A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.		
90/94	2014-015R&B-R	COOK	825	500		
		CONTRACT	NO.	60X9		
THE INDIS FED. AID PROJECT						

10-#4 v114(E) bars

Spa. as shown in Section D-D

1-1" PVC Conduit

(See Lighting Details)

r Elev. 595.02

D

- Elev. 587.83

Mechanical

Splicer, typ.