

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 10	*	IROQUOIS	31	1
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-329(105)	
*07-00216-01-BR		CONTRACT	#87401	

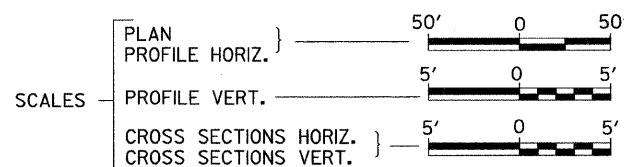
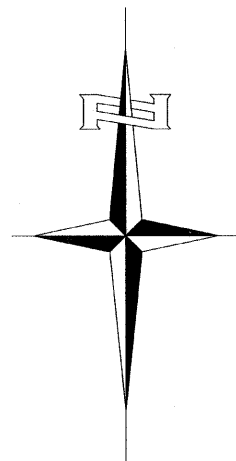
**STATE OF ILLINOIS**  
**DEPARTMENT OF TRANSPORTATION**  
**PLANS FOR PROPOSED**  
**HIGHWAY BRIDGE PROGRAM**  
**IROQUOIS COUNTY**  
**SECTION 07-00216-01-BR**  
**F.A.S. 329 (CH 10) OVER FOUNTAIN CREEK TRIBUTARY**  
**PROJECT NO. BRS-329(105)**  
**JOB NUMBER C-93-111-09**

**INDEX OF SHEETS**

SHEET NO.	DESCRIPTION
1.	COVER SHEET
2.	GENERAL NOTES, DETAILS, TYPICAL SECTIONS
3.	SUMMARY OF QUANTITIES, SCHEDULES OF QUANTITIES
4.	TRAFFIC CONTROL PLAN
5.	STORMWATER POLLUTION PREVENTION PLAN
6.	PLAN AND PROFILE
7.-21.	STRUCTURE PLANS
22.-31.	CROSS SECTIONS

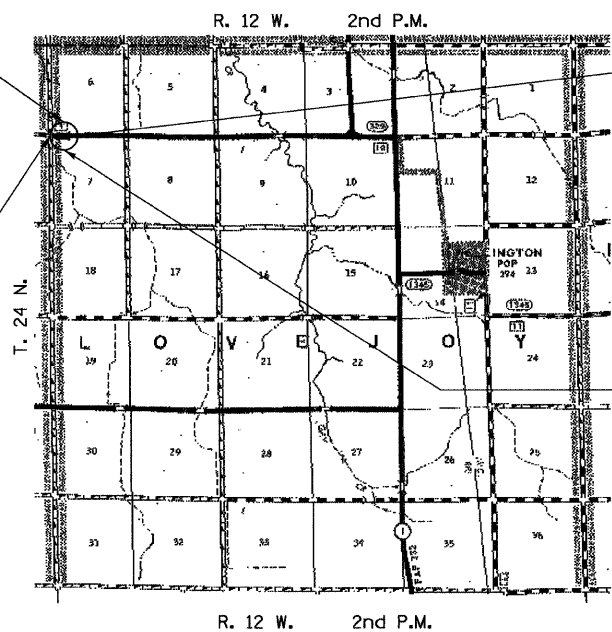
**REQUIRED HIGHWAY STANDARDS**

- 000001-05
- 280001-04
- 515001-03
- 542301-02
- 601101-01
- 602301-02
- 604036-02
- 630301-05
- 631032-04
- 635006-03
- 701901-01
- 780001-02
- BLR 21-8



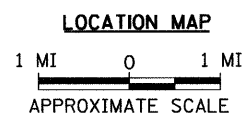
PROPOSED STRUCTURE NO. 038-4404  
 SINGLE SPAN CONCRETE DECK ON W30  
 STEEL I-BEAM SUPERSTRUCTURE ON  
 CONCRETE INTEGRAL ABUTMENTS,  
 71'-6" BK. TO BK. AND 30'-0" O. TO O.,  
 15° SKEW LT. AH.

SECTION 07-00216-01-BR  
 BEGINS  
 STATION 15+75.00

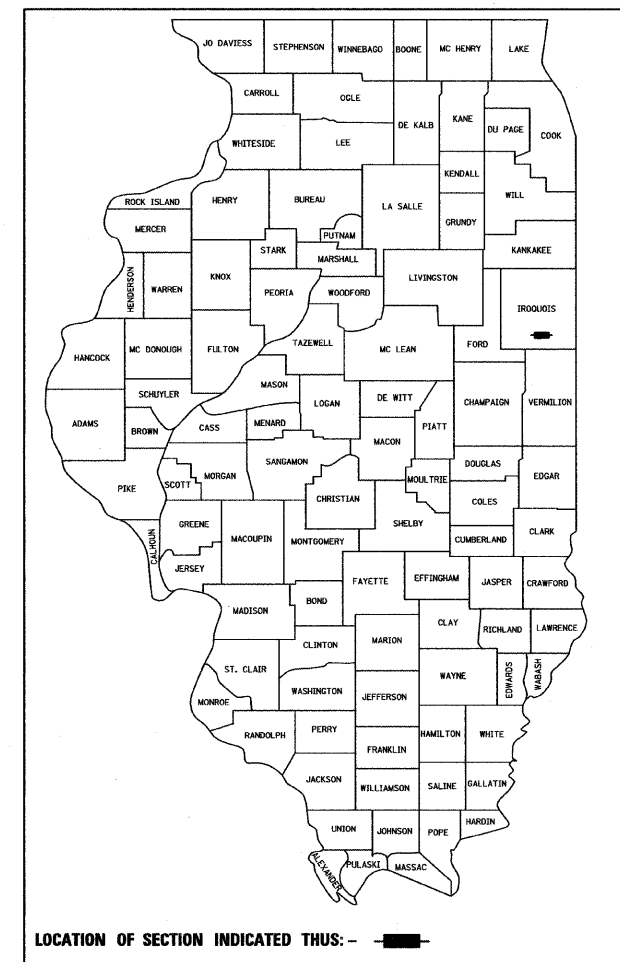


SECTION 07-00216-01-BR  
 ENDS  
 STATION 24+75.00

EXISTING STRUCTURE 038-0069  
 SINGLE SPAN REINFORCED CONCRETE  
 SLAB ON CONCRETE CLOSED ABUTMENTS  
 ON TIMBER PILE SUPPORTED CONCRETE FOOTINGS,  
 32'-6" BK. TO BK., AND 33'-6" O. TO O.,  
 NO SKEW (TO BE REMOVED)



NET LENGTH OF PROJECT = 900.00 FEET = 0.170 MILES  
 DESIGN CLASSIFICATION: MAJOR-COLLECTOR (NON-URBAN)  
 DESIGN ADT = 600 (2029)  
 DESIGN SPEED = 50 MPH



UTILITY COMPANIES  
 VERIZON  
 BLOOMINGTON, ILLINOIS

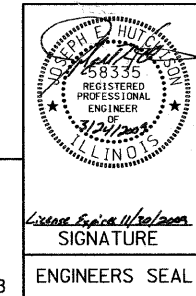
EASTERN ILLINOIS ELECTRIC COOPERATIVE  
 PAXTON, ILLINOIS

CALL J.U.L.I.E.  
 BEFORE YOU DIG  
 1-800-892-0123 OR 811

CONTRACT NO. 87401

**Hutchison Engineering, Inc.**  
 JACKSONVILLE ILLINOIS  
 SHOREWOOD ILLINOIS

2009 JOB#2598



PLANS DESIGNED IN ACCORDANCE WITH BUREAU  
 OF LOCAL ROADS AND STREETS MANUAL GUIDELINES  
 FOR TWO LANE RURAL COLLECTORS - RECONSTRUCTION

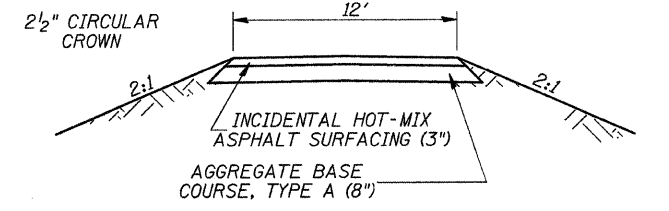
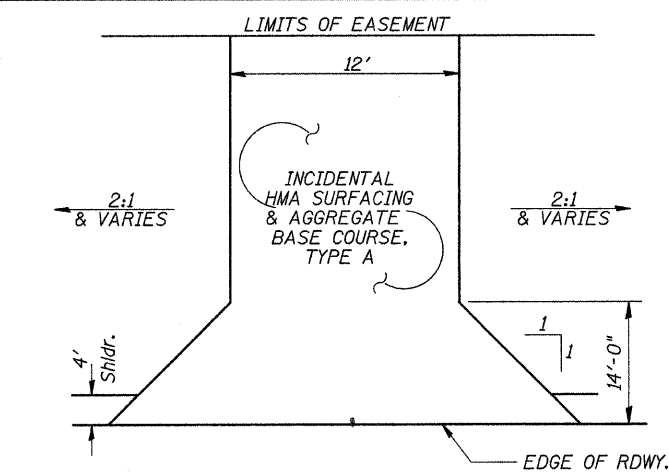
APPROVED March 25, 2009  
*[Signature]*  
 IROQUOIS COUNTY ENGINEER

PASSED March 30, 2009  
*[Signature]*  
 DISTRICT THREE ENGINEER OF  
 LOCAL ROADS & STREETS

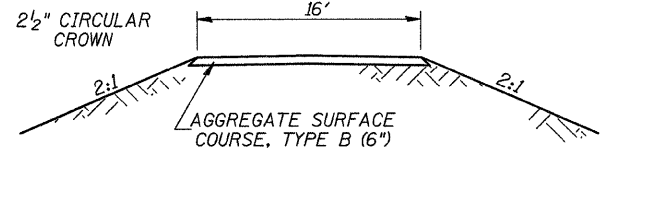
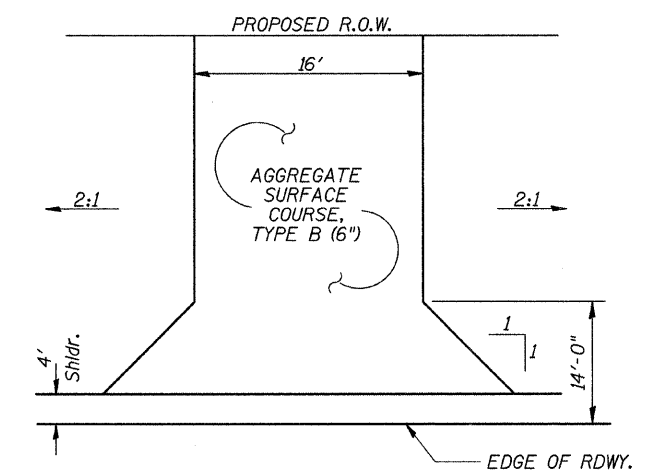
Released For  
 Bid Based on  
 Limited Review March 30, 2009

*[Signature]*  
 DEPUTY DIRECTOR OF HIGHWAYS,  
 REGION TWO ENGINEER  
 STATE OF ILLINOIS  
 DEPARTMENT OF TRANSPORTATION

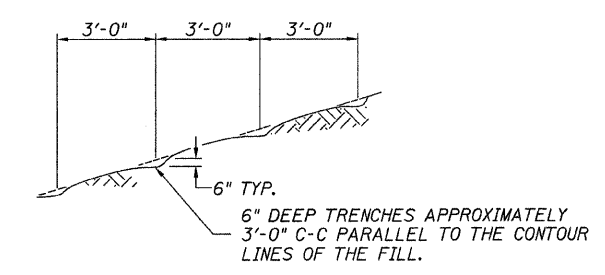
2598E001



**PROPOSED PRIVATE ENTRANCE**  
STA 18+80 LT



**PROPOSED PRIVATE ENTRANCE**  
STA 23+23 RT



NOTE: ALL SLOPES 3:1 OR STEEPER AND GREATER THAN 5' IN HEIGHT SHALL BE CONTOUR PLOWED AS SHOWN IN DETAIL. COST SHALL BE INCLUDED WITH SEEDING, CLASS 2 (SPECIAL).

**DETAIL OF CONTOUR PLOWING**

**HOT-MIX ASPHALT MIXTURE REQUIREMENTS**

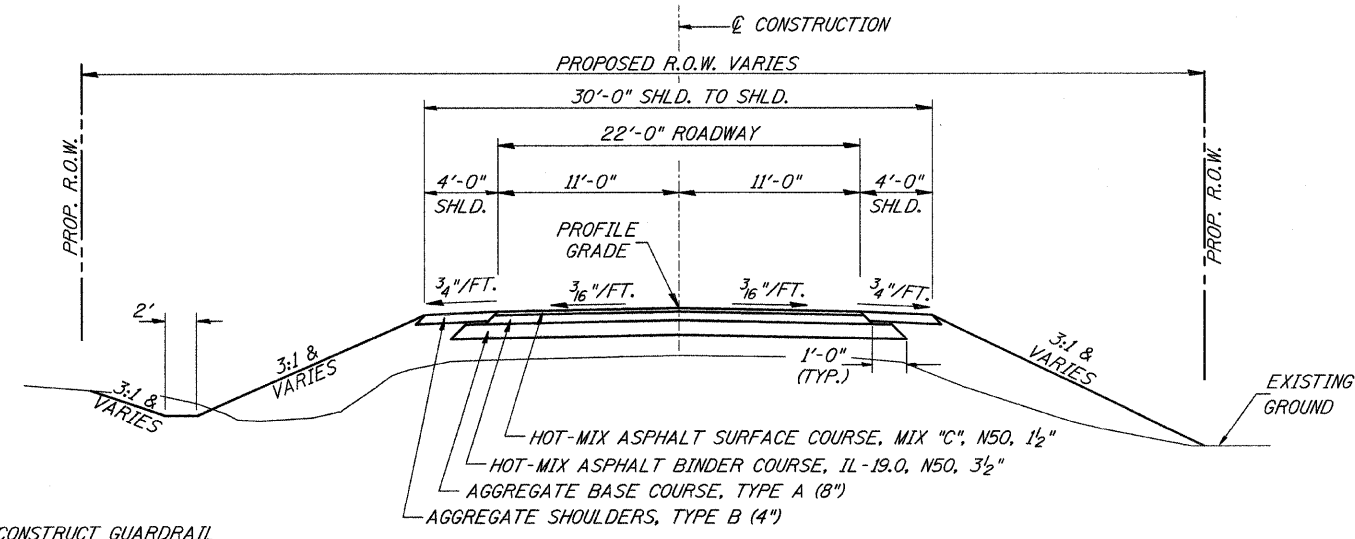
	HMA BINDER	HMA SURFACE	INCIDENTAL SURFACING
PG GRADE	PG 64-22	PG 64-22	PG 64-22
MAX % RAP ALLOWABLE**	25%	15%	25%
DESIGN AIR VOIDS	4% @ N50	4% @ N50	4% @ N50
MIXTURE COMPOSITION	IL-19.0	IL-12.5 OR IL-9.5	IL-12.5 OR IL-9.5
FRICTION AGGREGATE		MIXTURE C	MIXTURE C
DENSITY TEST METHOD	CORES	CORES	SATISFACTION OF ENGINEER

\* MATERIAL SHALL BE COMPACTED TO 93.0-97.4 PERCENT OF THE MAXIMUM THEORETICAL DENSITY, EXCEPT THAT WHEN PLACED AS FIRST LIFT ON AN UNIMPROVED SUBGRADE THE MINIMUM PERCENT COMPACTION SHALL BE 92.0 PERCENT. THE MAXIMUM THEORETICAL DENSITY SHALL BE DETERMINED FROM THE MOVING AVERAGE AS SPECIFIED IN THE QC/QA SPECIFICATION.

\*\* WHEN MORE THAN 20% RAP IS USED, A SOFTER ASPHALT BINDER (PG 58-22) MAY BE REQUIRED AS DETERMINED BY THE ENGINEER.

**STRUCTURAL DESIGN INFORMATION**  
**COUNTY HIGHWAY 10**

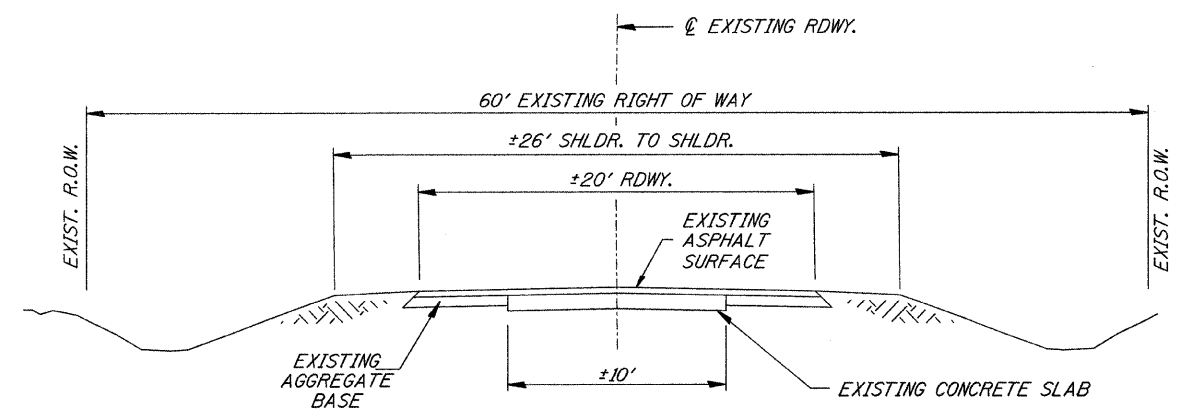
ROAD CLASSIFICATION: CLASS III 80,000 lb./20 YEAR DESIGN  
STRUCTURAL DESIGN TRAFFIC:  
PV = 528 SU = 42 MU = 30  
PERCENT OF STRUCTURAL DESIGN TRAFFIC IN DESIGN LANE:  
P = 88% S = 7% M = 5%  
MINIMUM SUBGRADE SUPPORT RATING: GRANULAR  
FLEXIBLE PAVEMENT DESIGN: MINIMUM TF = 0.162  
ASPHALT PAVEMENT THICKNESS: 5"  
AGGREGATE BASE COURSE, TYPE A: 8"



**PROPOSED TYPICAL SECTION**  
STA. 15+75.00 TO STA. 19+66.85  
STA. 20+38.35 TO STA. 24+75.00  
EXCEPT TRANSITIONS

BRIDGE OMISSION  
STA. 19+66.85 TO STA. 20+38.35

CONSTRUCT GUARDRAIL SHOULDER WIDENING IN ACCORDANCE WITH STD 630.301



**EXISTING TYPICAL SECTION**

**GENERAL NOTES**

PLAN QUANTITIES FOR TREE REMOVAL HAVE BEEN BASED ON ALL TREES WITHIN THE LIMITS OF CONSTRUCTION. THIS QUANTITY MAY BE REVISED DURING CONSTRUCTION, AT THE DIRECTION OF THE ENGINEER, BY DELETING FROM THE TREE REMOVAL QUANTITIES, SUCH TREES THAT DO NOT INTERFERE WITH THE PROPOSED CONSTRUCTION.

THE REMOVAL OF EXISTING ASPHALT SURFACE AND GRAVEL BASE COURSE OR CONCRETE SLAB WHICH MAY BE NECESSARY FOR THE CONSTRUCTION OF THE NEW BRIDGE SHALL BE REMOVED AS EARTH EXCAVATION AND NO COMPENSATION WILL BE ALLOWED FOR ADDITIONAL LABOR OR EQUIPMENT REQUIRED.

ALL WASTE OR UNDESIRABLE MATERIAL AS IDENTIFIED BY THE ENGINEER SHALL BE DISPOSED OF OUTSIDE THE LIMITS OF THE RIGHT OF WAY AT THE CONTRACTOR'S EXPENSE.

ALL EXISTING PRIVATELY OWNED UTILITIES REQUIRING ADJUSTMENT WILL BE MADE BY THE UTILITY COMPANY INVOLVED. WHERE NO PROVISIONS HAVE BEEN MADE FOR ADJUSTMENTS ON THE PLANS, NO ADDITIONAL COMPENSATION WILL BE ALLOWED DUE TO DELAYS OR INCONVENIENCES CAUSED BY THE SAID UTILITY ADJUSTMENTS.

THE PROFILE GRADE ELEVATIONS SHOWN ON THE PLAN AND PROFILE SHEETS AND IN THE STATION CROSS SECTIONS ARE TO THE TOP OF THE FINISHED SURFACE.

ALL EXISTING DRAINAGE STRUCTURES NOT BEING REMOVED BY THE CONTRACTOR THAT ARE DAMAGED DURING CONSTRUCTION SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE.

ANY REFERENCE TO STANDARDS THROUGHOUT THE PLANS SHALL BE INTERPRETED TO BE THE LATEST STANDARDS OF THE ILLINOIS DEPARTMENT OF TRANSPORTATION

THE LOCATION OF UNDERGROUND UTILITIES SHOWN ON THE PLANS REPRESENTS THE BEST KNOWLEDGE OF THE COUNTY. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LOCATIONS OF UNDERGROUND INSTALLATIONS BEFORE STARTING CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL INDEMNIFY THE COUNTY, ITS OFFICERS AND EMPLOYEES AGAINST ALL CLAIMS DUE TO DAMAGE TO CORPORATE OR PRIVATE PROPERTY RESULTING FROM HIS CONSTRUCTION OPERATIONS AS DESCRIBED IN ARTICLES 107.20 AND 107.26 OF THE STANDARD SPECIFICATIONS.

THE CONTRACTOR MAY BE REQUIRED TO CONDUCT SOME OF HIS GRADING AND TRENCHING OPERATIONS AROUND TRANSMISSION POLES AND UNDER TRANSMISSION LINES. THE ADDED COST OF SO DOING SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION.

WHERE SECTION OR SUBSECTION MONUMENTS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE REMOVED. THE CONTRACTOR SHALL PROTECT AND PRESERVE PROPERTY MARKERS UNTIL THE OWNER, AN AUTHORIZED SURVEYOR, OR AGENT, HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION.

ALL ELEVATIONS SHOWN REFER TO U.S.G.S. MEAN SEA LEVEL DATUM.

**SUMMARY OF QUANTITIES**

CODE NO.	ITEM	UNIT	QUANTITY
20100110	TREE REMOVAL (6 TO 15 UNITS DIAMETER)	UNIT	48
20100210	TREE REMOVAL (OVER 15 UNITS DIAMETER)	UNIT	24
20200100	EARTH EXCAVATION	CU YD	400
20300100	CHANNEL EXCAVATION	CU YD	520
① 20400800	FURNISHED EXCAVATION	CU YD	1,855
① 20700400	POROUS GRANULAR EMBANKMENT, SPECIAL	CU YD	85
20800150	TRENCH BACKFILL	CU YD	5
① 25001000	SEEDING, CLASS 2 (SPECIAL)	ACRE	0.9
25100630	EROSION CONTROL BLANKET	SQ YD	115
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	500
28000300	TEMPORARY DITCH CHECKS	EACH	13
28000400	PERIMETER EROSION BARRIER	FOOT	810
28000500	INLET AND PIPE PROTECTION	EACH	1
28000510	INLET FILTERS	EACH	1
28100209	STONE RIPRAP, CLASS A5	TON	480
28200200	FILTER FABRIC	SQ YD	515
28300400	AGGREGATE DITCH	TON	38
35100100	AGGREGATE BASE COURSE, TYPE A	TON	1,022
40200800	AGGREGATE SURFACE COURSE, TYPE B	TON	21
40600100	BITUMINOUS MATERIALS (PRIME COAT)	GALLON	1,181
40603080	HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	TON	406
40603310	HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N50	TON	171
40800050	INCIDENTAL HOT-MIX ASPHALT SURFACING	TON	12
48101200	AGGREGATE SHOULDERS, TYPE B	TON	167
① 50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1
① 50105220	PIPE CULVERT REMOVAL	FOOT	121
50200100	STRUCTURE EXCAVATION	CU YD	115
50300225	CONCRETE STRUCTURES	CU YD	25.9
50300255	CONCRETE SUPERSTRUCTURE	CU YD	74.1
50300260	BRIDGE DECK GROOVING	SQ YD	222
50300280	CONCRETE ENCASUREMENT	CU YD	4.6
50300300	PROTECTIVE COAT	SQ YD	238
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1
50500505	STUD SHEAR CONNECTORS	EACH	900
① 50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	18,790
51200957	FURNISHING METAL SHELL PILES 12"x0.25"	FOOT	408
51202305	DRIVING PILES	FOOT	408
51203200	TEST PILE METAL SHELLS	EACH	2
51204650	PILE SHOES	EACH	10
51500100	NAME PLATES	EACH	1
52100520	ANCHOR BOLTS, 1"	EACH	20
① 54200220	PIPE CULVERTS, CLASS D, TYPE 1 15"	FOOT	28
54213660	PRECAST REINFORCED CONCRETE FLARED END SECTION 15"	EACH	1
550B0070	STORM SEWERS, CLASS B, TYPE 1 15"	FOOT	64
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	52
① 60109580	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	120
60236200	INLETS, TYPE A, TYPE 8 GRATE	EACH	1
60500060	REMOVING INLETS	EACH	1
* 63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	2
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	2
① 63200310	GUARDRAIL REMOVAL	FOOT	723
67100100	MOBILIZATION	L SUM	1
* 78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	1,800
* 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4
* XX006199	STEEL BRIDGE RAIL, TYPE SM (SPECIAL)	FOOT	143

① SEE SPECIAL PROVISIONS CONSTRUCTION CODE TYPE: X071-2A  
\* SPECIALTY ITEMS

**PERIMETER EROSION BARRIER**

STATION TO STATION	SIDE	FOOT
15+75 - 17+00	LEFT	125
15+75 - 19+65	RIGHT	415
18+65 - 18+95	LEFT	30
23+00 - 23+35	RIGHT	40
23+50 - 24+75	LEFT	125
24+00 - 24+75	RIGHT	75
TOTAL		810

**TREE REMOVAL**

STATION	OFFSET	SIDE	6 TO 15 UNITS	OVER 15 UNITS
18+65	39'	LEFT	12	
19+80	41'	LEFT	12	
19+90	41'	LEFT		24
20+42	40'	LEFT	12	
20+42	40'	LEFT	12	
TOTAL			48	24

**PAINT PAVEMENT MARKING - LINE 4"**

STATION TO STATION	SIDE	DESCRIPTION	FOOT
15+75.00 - 24+75.00	℄	DOUBLE YELLOW	1,800
TOTAL			1,800

**PAVEMENT SCHEDULE**

STATION TO STATION	WIDTH	LENGTH	AGGREGATE BASE COURSE, TYPE A	PRIME COAT** GALLON 0.50 GAL/SQ YD	HOT-MIX ASPHALT BINDER CSE TON 112#/SQ YD/IN	HOT-MIX ASPHALT SURF CSE TON 112#/SQ YD/IN	INC HOT-MIX ASPHALT SURF TON 112#/SQ YD/IN
15+75.00 - 16+25.00	24.71' AVG.	50.00'	58				
16+25.00 - 19+66.85	25.50'	341.85'	409				
20+38.35 - 24+25.00	25.50'	386.65'	462				
24+25.00 - 24+75.00	24.77' AVG.	50.00'	58				
15+75.00 - 16+25.00	24.04' AVG.	50.00'		67			
16+25.00 - 19+66.85	24.83'	341.85'		472			
20+38.35 - 24+25.00	24.83'	386.65'		533			
24+25.00 - 24+75.00	24.10' AVG.	50.00'		67			
15+75.00 - 16+25.00	21.75' AVG.	50.00'			24		
16+25.00 - 19+66.85	22.54'	341.85'			168		
20+38.35 - 24+25.00	22.54'	386.65'			190		
24+25.00 - 24+75.00	21.81' AVG.	50.00'			24		
15+75.00 - 16+25.00	21.34' AVG.	50.00'				10	
16+25.00 - 19+66.85	22.13'	341.85'				71	
20+38.35 - 24+25.00	22.13'	386.65'				80	
24+25.00 - 24+75.00	21.40' AVG.	50.00'				10	
ENT. 18+80.00 LT	12.00' & VAR.	38.60'	35	42			*12
TOTAL			1,022	1,181	406	171	12

\*\* PRIME COAT IS TO BE MC-30 ON AGGREGATE SURFACES AND RC-70 ON HMA/CONCRETE SURFACES

\* 3" PAVEMENT THICKNESS

**STORM SEWER SCHEDULE**

STATION TO STATION	SIDE	OFFSET	TYPE	STORM SEWER, CLASS B, TYPE 1	INLETS		PRECAST REINFORCED CONCRETE FLARED END SECTION	TRENCH BACKFILL
					TYPE A, STD 602301 W/ TYPE 8 GRATE	15"		
		FOOT		FOOT	EACH		EACH	CU YD
18+50	LT	25.5	INLET		1			
			SS	64				5
19+14	LT	34.2	FES			1		
TOTALS				64	1	1		5

**AGGREGATE SURFACE COURSE, TYPE B**

STATION TO STATION	WIDTH	LENGTH	TON
ENTR. - 23+23 RT	16' & VAR.	27'	21
TOTAL			21

**AGGREGATE DITCH\*\*\***

STATION TO STATION	SIDE	WIDTH	TON	FILTER FABRIC (SQ YD)
19+20 - 19+80	LEFT	6'	21	40
23+00 - 23+50	LEFT	6'	17	34
TOTAL			38	74

\*\*\* Aggregate Ditch shall conform to material type B-3 of the Standard Specifications

**EARTHWORK SUMMARY**

STATION TO STATION	EARTH EXCAVATION	CHANNEL EXCAVATION	STRUCTURE EXCAVATION	FILL	WASTE (SHORTAGE)
	CU YD	CU YD	CU YD	CU YD	CU YD
RDWY 15+75.00 - 19+66.85	139			852	(748)
RDWY 20+38.35 - 24+75.00	259			1,299	(1,105)
CHANNEL STRUCTURE		520	115		
TOTAL	398	520	115	2,151	(1,853)
USE	400	520	115	-	(1,855)

(@ 25% SHRINKAGE)

**AGGREGATE SHOULDERS, TYPE B\*\*\*\***

STATION TO STATION	SIDE	WIDTH	LENGTH	TON
15+75.00 - 16+25.00	LEFT	3.73' AVG.	50.00'	4
15+75.00 - 16+25.00	RIGHT	3.73' AVG.	50.00'	4
16+25.00 - 18+63.00	LEFT	4.00'	238.00'	22
16+25.00 - 18+35.08	RIGHT	4.00'	210.08'	20
18+35.08 - 18+59.08	RIGHT	6.00' AVG.	24.00'	3
18+59.08 - 18+94.08	RIGHT	8.00'	35.00'	7
18+94.08 - 18+98.08	RIGHT	7.67' AVG.	4.00'	1
18+97.00 - 19+70.33	LEFT	4.00'	73.33'	7
18+98.08 - 19+50.04	RIGHT	7.33'	51.96'	9
19+50.04 - 19+63.37	RIGHT	5.67' AVG.	13.33'	2
20+34.87 - 23+02.33	RIGHT	4.00'	267.46'	25
20+41.83 - 20+55.16	LEFT	5.67' AVG.	13.33'	2
20+55.16 - 21+07.12	LEFT	7.33'	51.96'	9
21+07.12 - 21+11.12	LEFT	7.67' AVG.	4.00'	1
21+11.12 - 21+46.12	LEFT	8.00'	35.00'	7
21+46.12 - 21+70.12	LEFT	6.00' AVG.	24.00'	3
21+70.12 - 24+25.00	LEFT	4.00'	254.88'	24
23+35.50 - 24+25.00	RIGHT	4.00'	89.50'	8
24+25.00 - 27+75.00	LEFT	4.27' AVG.	50.00'	5
24+25.00 - 27+75.00	RIGHT	3.31' AVG.	50.00'	4
TOTAL				167

\*\*\*\* RAP WILL NOT BE ALLOWED FOR USE WITH AGGREGATE SHOULDERS, TYPE B

**REMOVING INLETS**

STATION	SIDE	EACH
18+47	LEFT	1
TOTAL		1

**PIPE CULVERTS, CLASS D, TYPE 1 15"**

STATION	SIDE	FOOT
23+23	RIGHT	28
TOTAL		28

**TEMPORARY DITCH CHECKS**

STATION	SIDE	EACH
17+75	LEFT	1
19+50	LEFT	1
19+80	LEFT	1
20+25	LEFT	1
20+30	RIGHT	1
21+15	RIGHT	1
21+25	LEFT	1
22+00	RIGHT	1
22+25	LEFT	1
22+50	RIGHT	1
23+00	LEFT	1
23+25	LEFT	1
23+65	RIGHT	1
TOTAL		13

**PIPE CULVERT REMOVAL**

STATION	SIZE	SIDE	FOOT
18+85	15"	LEFT	71
23+20	15"	RIGHT	23
23+75	15"	RIGHT	27
TOTAL			121

**INLET FILTERS**

STATION	SIDE	EACH
18+50	LEFT	1
TOTAL		1

**EROSION CONTROL BLANKET**

STATION TO STATION	SIDE	WIDTH	LENGTH	AREA (SQ YD)
22+00.00 - 23+04.00	RIGHT	6'	104'	70
23+32.00 - 24+00.00	RIGHT	6'	68'	45
TOTAL				115

**GUARDRAIL REMOVAL**

STATION TO STATION	SIDE	FOOT
17+69.00 - 19+84.00	RIGHT	215
18+94.00 - 19+84.00	LEFT	90
20+16.00 - 21+81.00	RIGHT	165
20+16.00 - 22+69.00	LEFT	253
TOTAL		723

**INLET & PIPE PROTECTION**

STATION	SIDE	EACH
23+32	RIGHT	1
TOTAL		1

**TRAFFIC BARRIER TERMINAL, TYPE 6A**

SIDE	STATION TO STATION	EACH
RIGHT	19+19.08 - 19+62.83	1
LEFT	20+42.37 - 20+86.12	1
TOTAL		2

**TRAFFIC BARRIER TERMINAL TYPE 1, SPECIAL (TANGENT)**

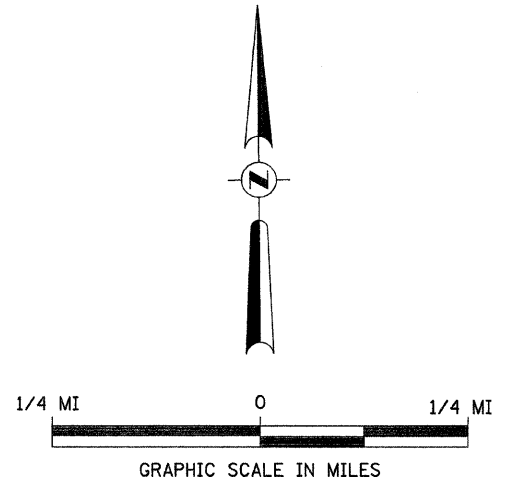
SIDE	STATION TO STATION	EACH
RIGHT	18+69.08 - 19+19.08	1
LEFT	20+86.12 - 21+36.12	1
TOTAL		2

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 10	*	IROQUOIS	31	4
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-329105	
* 07-00216-01-BR			CONTRACT #87401	



- 1 ROAD CLOSED  
1 MILE AHEAD  
LOCAL TRAFFIC ONLY  
R11-3
- 2 ROAD CLOSED  
3/4 MILE AHEAD  
LOCAL TRAFFIC ONLY  
R11-3
- 3 ROAD CLOSED  
AHEAD  
ROAD CLOSED  
AHEAD  
W20-3
- 4 ROAD CLOSED  
500 FT  
ROAD CLOSED  
500 FT  
W20-3
- 5 TYPE III BARRICADES

SEE STANDARD BLR 21  
AND SPECIAL PROVISIONS



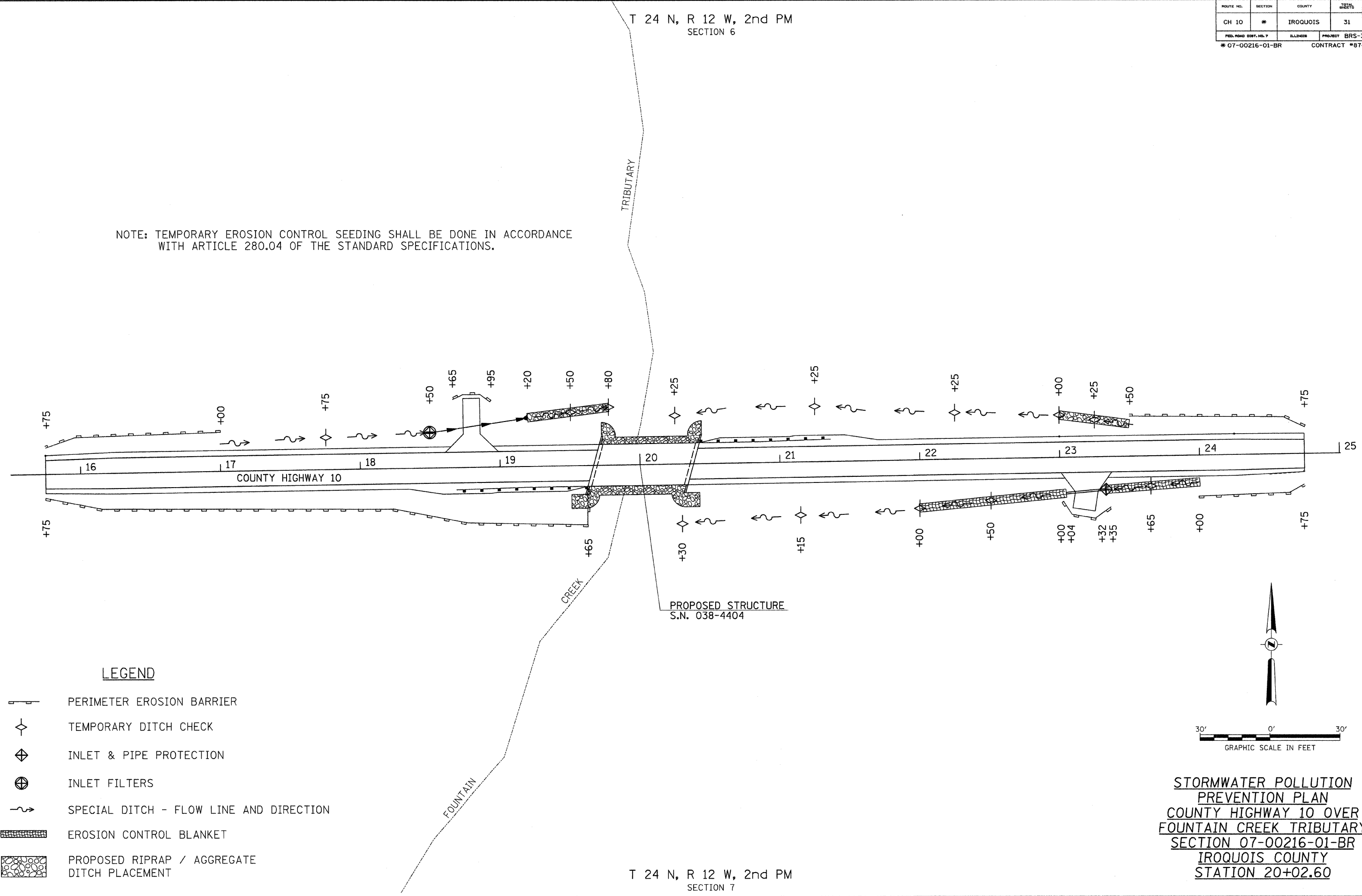
TRAFFIC CONTROL PLAN  
COUNTY HIGHWAY 10 OVER  
FOUNTAIN CREEK TRIBUTARY  
SECTION 07-00216-01-BR  
IROQUOIS COUNTY  
STATION 20+02.60










ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 10	*	IROQUOIS	31	5
FED. ROAD DIST. NO. 7		BLANKS	PROJECT BRS-329(105)	
*07-00216-01-BR		CONTRACT #87401		

T 24 N, R 12 W, 2nd PM  
SECTION 6

NOTE: TEMPORARY EROSION CONTROL SEEDING SHALL BE DONE IN ACCORDANCE WITH ARTICLE 280.04 OF THE STANDARD SPECIFICATIONS.



LEGEND

-  PERIMETER EROSION BARRIER
-  TEMPORARY DITCH CHECK
-  INLET & PIPE PROTECTION
-  INLET FILTERS
-  SPECIAL DITCH - FLOW LINE AND DIRECTION
-  EROSION CONTROL BLANKET
-  PROPOSED RIPRAP / AGGREGATE DITCH PLACEMENT

STORMWATER POLLUTION PREVENTION PLAN  
COUNTY HIGHWAY 10 OVER  
FOUNTAIN CREEK TRIBUTARY  
SECTION 07-00216-01-BR  
IROQUOIS COUNTY  
STATION 20+02.60

T 24 N, R 12 W, 2nd PM  
SECTION 7



B.M.: RR Spike in Power Pole  
Sta. 16+99, 29' Lt.  
Elev. 663.69

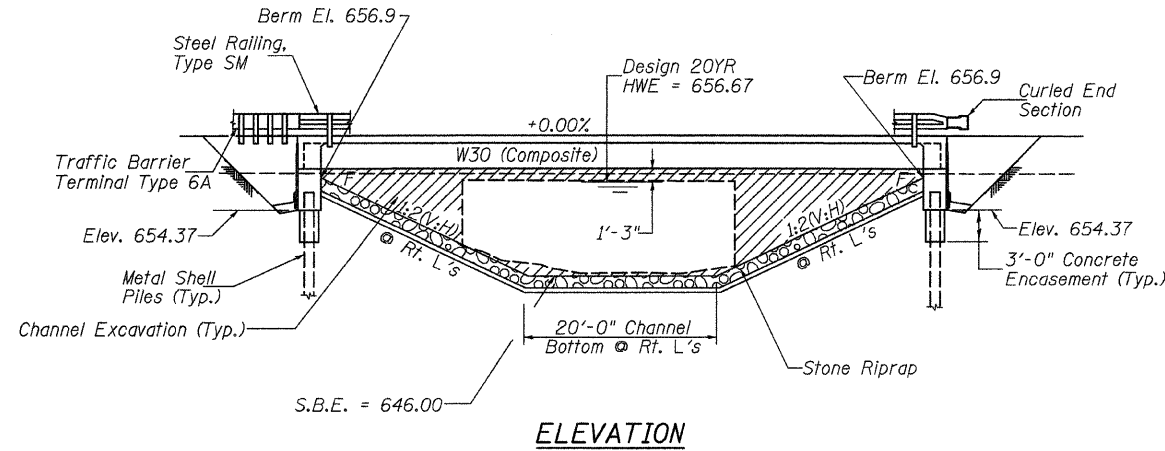
RR Spike in Power Pole  
Sta. 22+76, 28' Lt.  
Elev. 658.37

ROUTE NO.	SECTION	COUNTY	SHEET NO.	SHEET NO.
CH 10	*	IROQUOIS	31	7
FED. ROAD DIST. NO. 7		BILLINGS	PROJECT BRS-329(105)	
*07-00216-01-BR		CONTRACT #87401		

SHEET NO. 1  
OF 15 SHEETS

Existing Structure:  
Single span reinforced concrete slab with curb mounted steel rail on concrete closed abutments on timber pile supported concrete footings. The structure is 32'-6" back to back of abutments, 33'-6" out to out of deck with a 30'-0" driving surface, no skew. Existing Structure to be removed and replaced by Contractor. Str. No. 038-0069

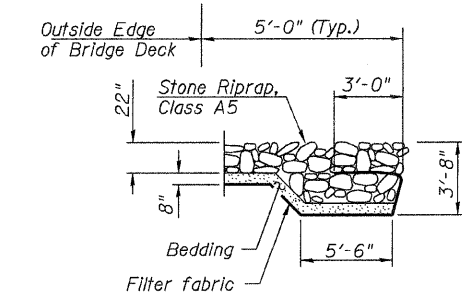
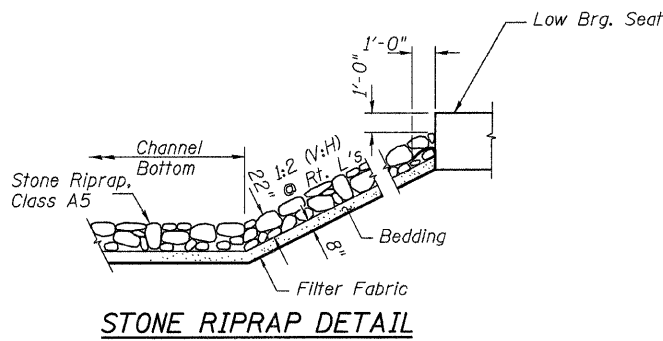
Salvage: None  
Road to be closed to traffic during construction.



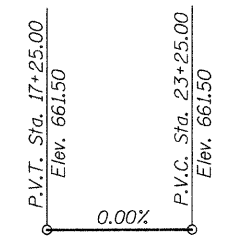
NOTE:  
For Bill of Material and General Notes,  
See Sheet 2 of 15.

**DESIGN SCOUR TABLE**

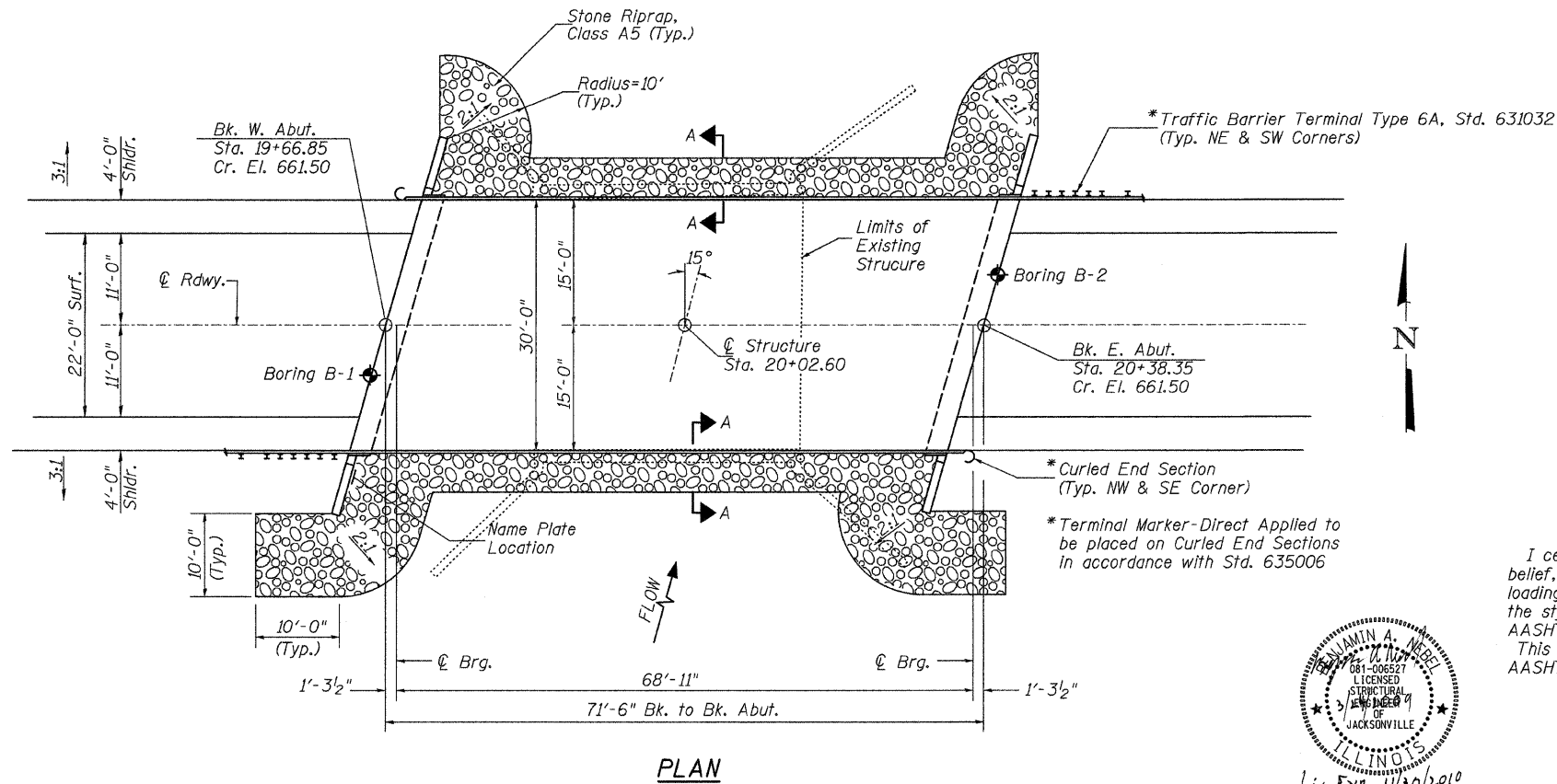
Location	W. Abut	E. Abut
Design Scour Elevation	654.37	654.37



SECTION A-A



PROFILE GRADE



PLAN

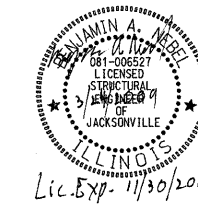
FOUNTAIN CREEK TRIBUTARY  
BUILT 20 BY  
IROQUOIS COUNTY  
SEC. 07-00216-01-BR  
C.H. 10 STATION 20+02.60  
F.A. PROJ. BRS-329(101)  
STR. NO. 038-4404 LOADING HL-93

**NAME PLATE**

Locate Name Plate at S.W. Wingwall  
Corner of Bridge (See Std. 515001)

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AASHTO LRFD Bridge Design Specifications. This design complies with all requirements of the current AASHTO Guide Specifications for Seismic Design of highway bridges.

*Benjamin A. New* 3/24/2009  
Illinois Structural No. 6527  
Expires 11/30/2010



**DESIGN SPECIFICATIONS**

2007 AASHTO LRFD Bridge Design Specifications,  
4th Edition with Interims

**DESIGN STRESSES**

**FIELD UNITS**

$f'_c = 3,500$  psi  
 $f_y = 60,000$  psi (Reinforcement)  
 $f_y = 50,000$  psi (M270 Grade 50W)

**LOADING HL-93**

Allow 50#/sq. ft. for future wearing surface.

**SEISMIC DATA**

Seismic Performance Zone (SPZ) = 1  
Design Spectral Acceleration at 1.0 sec. ( $S_{D1}$ ) = 0.05g  
Design Spectral Acceleration at 0.2 sec. ( $S_{D5}$ ) = 0.11g  
Soil Site Class = D

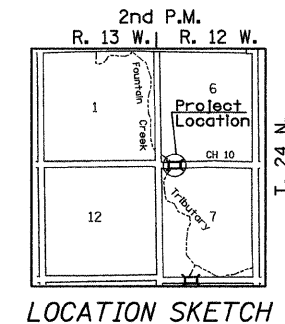
**WATERWAY INFORMATION**

Drainage Area = 16.69 Sq. Mi. Low Grade Elev. = 661.50 @ Sta. 20+02.60

Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.		Nat. Head - Ft.		Headwater El.		
			Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
Design	20	1,935	288	441	656.67	0.77	0.15	657.44	656.82
Base	100	2,850	288	493	657.48	1.71	0.29	659.19	657.77

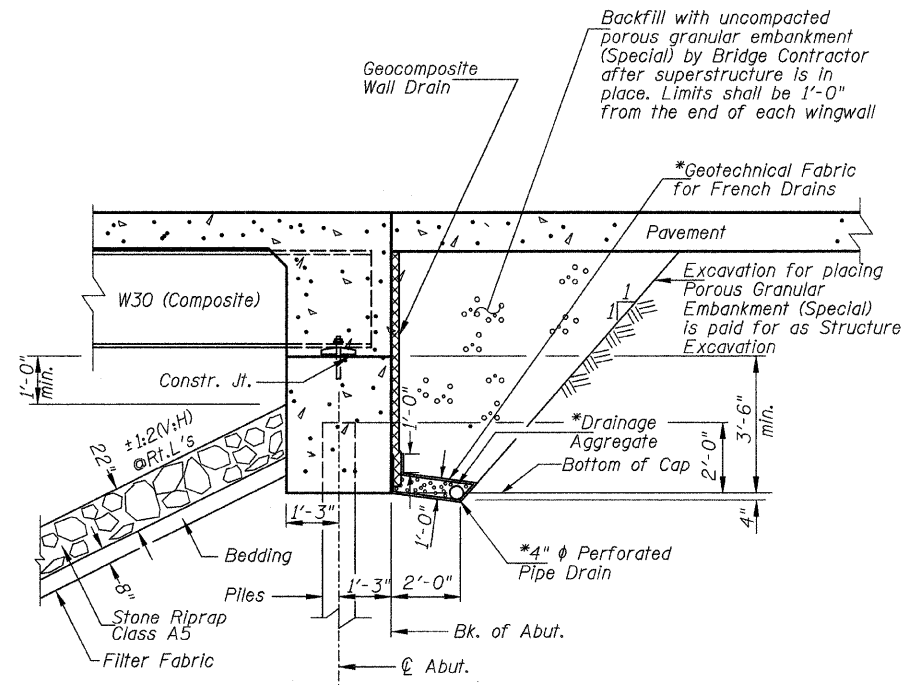
Construction of this project complies with IDNR,  
Office of Water Resources Statewide Permit No. 2

DESIGNED	B.A.N.
CHECKED	J.O.H.
DRAWN	T.A.C.
CHECKED	B.A.N.



LOCATION SKETCH

**GENERAL PLAN & ELEVATION**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SEC. 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**  
**STRUCTURE NO. 038-4404**



\*Included in the cost of Pipe Underdrains for Structures. All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101)

**SECTION THRU ABUTMENTS**  
(Horiz. dim. @ Rt. L's)

**GENERAL NOTES**

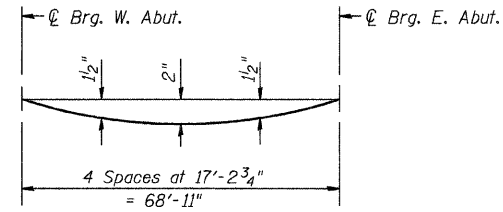
All structural steel shall be AASHTO M 270 Grade 50W.  
Calculated weight of Structural Steel = 49,350 lbs, Grade 50W.  
Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts (in painted areas and M164 Type 3 in unpainted areas). Bolts 3/4 in. φ, holes 15/16 in. φ, unless otherwise noted.  
No field welding is permitted except as specified in the contract documents.  
For Soil Boring Logs, see sheet 14 & 15 of 15.  
Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions  
Reinforcement bars designated (E) shall be epoxy coated.  
Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete cap plus 3 inches. Those areas shall be primed in the shop with a Department approved zinc rich primer. No field painting shall be required. All structural steel shall be cleaned as specified in the Special Provision for "Surface Preparation and Painting Requirements for Weathering Steel".  
Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.  
The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at substructures specified or approved by the Engineer before ordering the remainder of piles.  
All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted.  
Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure.

**TOTAL BILL OF MATERIAL**

ITEM	UNIT	SUPER	SUB	TOTAL
① Removal of Existing Structures	EACH	—	—	1
Structure Excavation	CU YD	—	115	115
Channel Excavation	CU YD	—	520	520
Concrete Superstructure	CU YD	74.1	—	74.1
Concrete Structures	CU YD	—	25.9	25.9
Furnishing Metal Shell Piles 12"x0.25"	FOOT	—	408	408
Driving Piles	FOOT	—	408	408
Test Pile Metal Shells	EACH	—	2	2
Pile Shoes	EACH	—	10	10
Concrete Encasement	CU YD	—	4.6	4.6
Furnishing and Erecting Structural Steel	L SUM	1	—	1
Anchor Bolts, 1"	EACH	—	20	20
① Reinforcement Bars, Epoxy Coated	POUND	15,280	3,510	18,790
Stone Riprap, Class A5	TON	—	480	480
Filter Fabric	SQ YD	—	440	440
Steel Bridge Rail, Type SM (Special)	EACH	143	—	143
Protective Coat	SQ YD	238	—	238
Bridge Deck Grooving	SQ YD	222	—	222
Stud Shear Connectors	EACH	900	—	900
Name Plates	EACH	1	—	1
① Porous Granular Embankment, Special	CU YD	—	85	85
① Pipe Underdrains For Structures 4"	FOOT	—	120	120
① Geocomposite Wall Drain	SQ YD	—	52	52

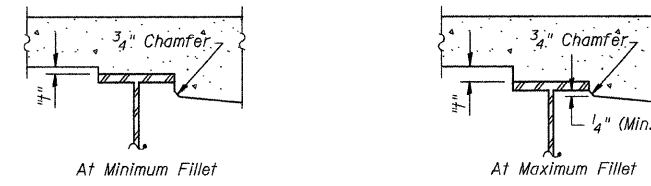
① See Special Provisions

**GENERAL NOTES & BILL OF MATERIAL**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SEC. 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**



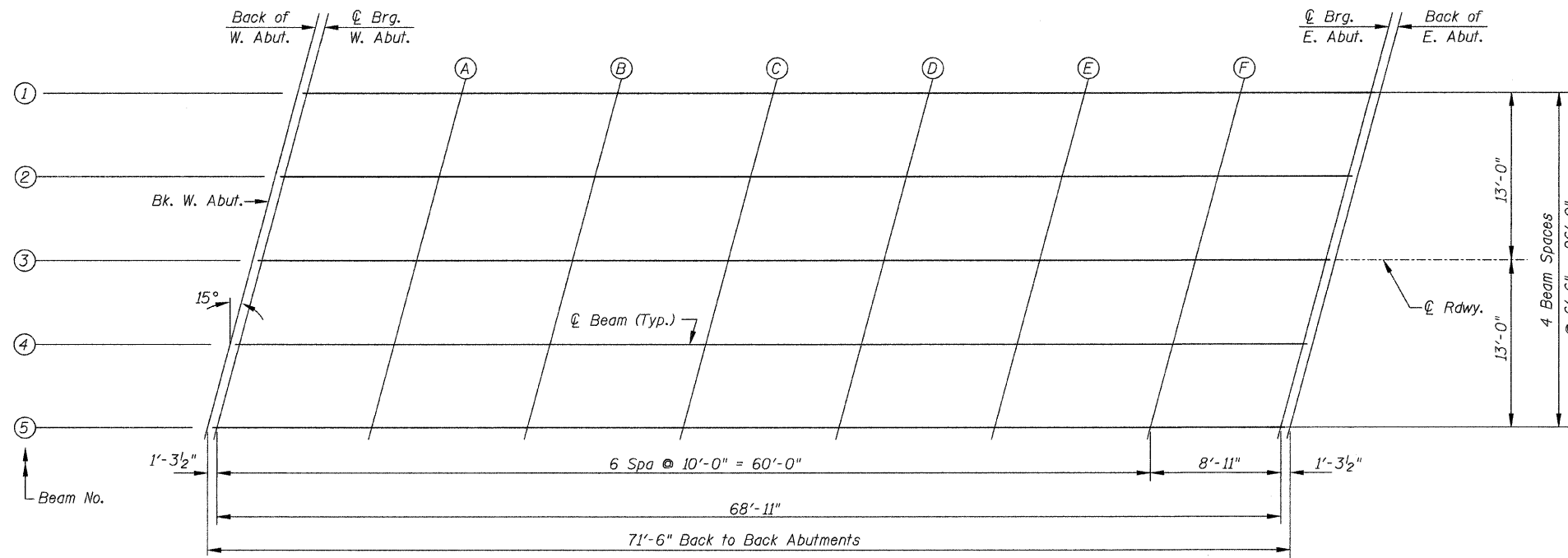
**DEAD LOAD DEFLECTION DIAGRAM**  
(Includes weight of concrete only)

Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown in the tables on sheet 4 of 15.

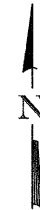


To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" minus slab thickness, equals the fillet heights "t" above top flanges of beams.

**FILLET HEIGHTS**



**PLAN (DECK ELEVATIONS)**



**TOP OF SLAB ELEVATIONS  
COUNTY HIGHWAY 10 OVER  
FOUNTAIN CREEK TRIBUTARY  
SEC. 07-00216-01-BR  
IROQUOIS COUNTY  
STATION 20+02.60**

**BEAM #1**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1970.33	-13.00	661.29	661.29
CL Brg W. Abut.	1971.63	-13.00	661.29	661.29
A	1981.63	-13.00	661.29	661.36
B	1991.63	-13.00	661.29	661.42
C	2001.63	-13.00	661.29	661.46
D	2011.63	-13.00	661.29	661.45
E	2021.63	-13.00	661.29	661.42
F	2031.63	-13.00	661.29	661.36
CL Brg E. Abut.	2040.54	-13.00	661.29	661.29
Bk E. Abutment	2041.83	-13.00	661.29	661.29

**BEAM #2**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1968.59	-6.50	661.40	661.40
CL Brg W. Abut.	1969.89	-6.50	661.40	661.40
A	1979.89	-6.50	661.40	661.48
B	1989.89	-6.50	661.40	661.54
C	1999.89	-6.50	661.40	661.57
D	2009.89	-6.50	661.40	661.57
E	2019.89	-6.50	661.40	661.53
F	2029.89	-6.50	661.40	661.47
CL Brg E. Abut.	2038.80	-6.50	661.40	661.40
Bk E. Abutment	2040.09	-6.50	661.40	661.40

**PROFILE GRADE & BEAM #3**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1966.85	0.00	661.50	661.50
CL Brg W. Abut.	1968.14	0.00	661.50	661.50
A	1978.14	0.00	661.50	661.58
B	1988.14	0.00	661.50	661.64
C	1998.14	0.00	661.50	661.67
D	2008.14	0.00	661.50	661.67
E	2018.14	0.00	661.50	661.63
F	2028.14	0.00	661.50	661.57
CL Brg E. Abut.	2037.06	0.00	661.50	661.50
Bk E. Abutment	2038.35	0.00	661.50	661.50

**BEAM #4**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1965.11	6.50	661.40	661.40
CL Brg W. Abut.	1966.40	6.50	661.40	661.40
A	1976.40	6.50	661.40	661.48
B	1986.40	6.50	661.40	661.54
C	1996.40	6.50	661.40	661.57
D	2006.40	6.50	661.40	661.57
E	2016.40	6.50	661.40	661.53
F	2026.40	6.50	661.40	661.47
CL Brg E. Abut.	2035.31	6.50	661.40	661.40
Bk E. Abutment	2036.61	6.50	661.40	661.40

**BEAM #5**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk W. Abutment	1963.37	13.00	661.29	661.29
CL Brg W. Abut.	1964.66	13.00	661.29	661.29
A	1974.66	13.00	661.29	661.36
B	1984.66	13.00	661.29	661.42
C	1994.66	13.00	661.29	661.46
D	2004.66	13.00	661.29	661.45
E	2014.66	13.00	661.29	661.42
F	2024.66	13.00	661.29	661.36
CL Brg E. Abut.	2033.57	13.00	661.29	661.29
Bk E. Abutment	2034.87	13.00	661.29	661.29

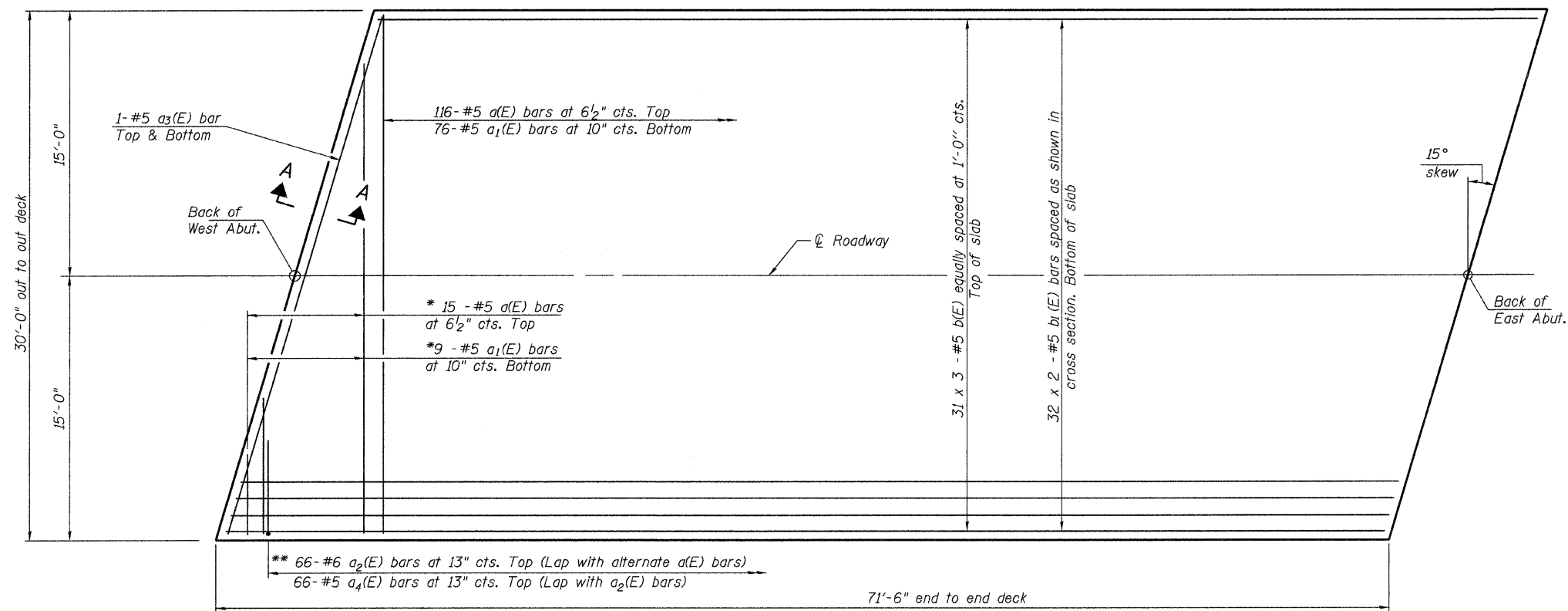
*TOP OF SLAB ELEVATIONS  
COUNTY HIGHWAY 10 OVER  
FOUNTAIN CREEK TRIBUTARY  
SEC. 07-00216-01-BR  
IROQUOIS COUNTY  
STATION 20+02.60*



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 10	*	IROQUOIS	31	11
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-329(105)	
*07-00216-01-BR		CONTRACT #87401		

SHEET NO. 5  
OF 15 SHEETS

\*Order a(E) & a<sub>1</sub>(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.

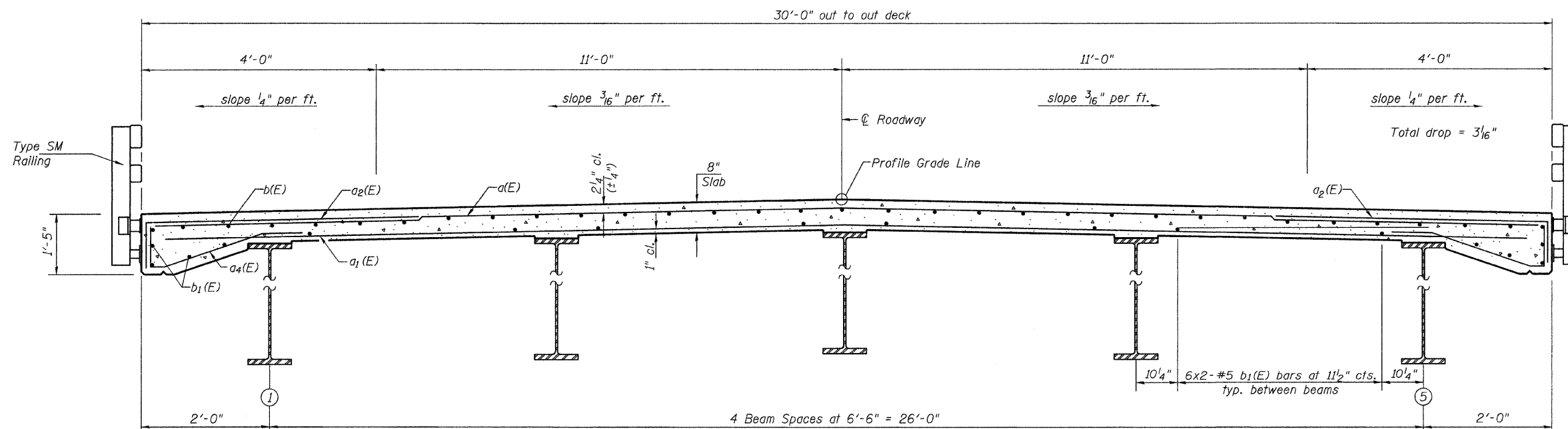


**MIN. BAR LAP**  
#5 = 1'-8"

Notes:  
See Sheet 6 of 15 for superstructure details  
and Bill of Material.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet 7 of 15 for Section A-A and  
diaphragm details.

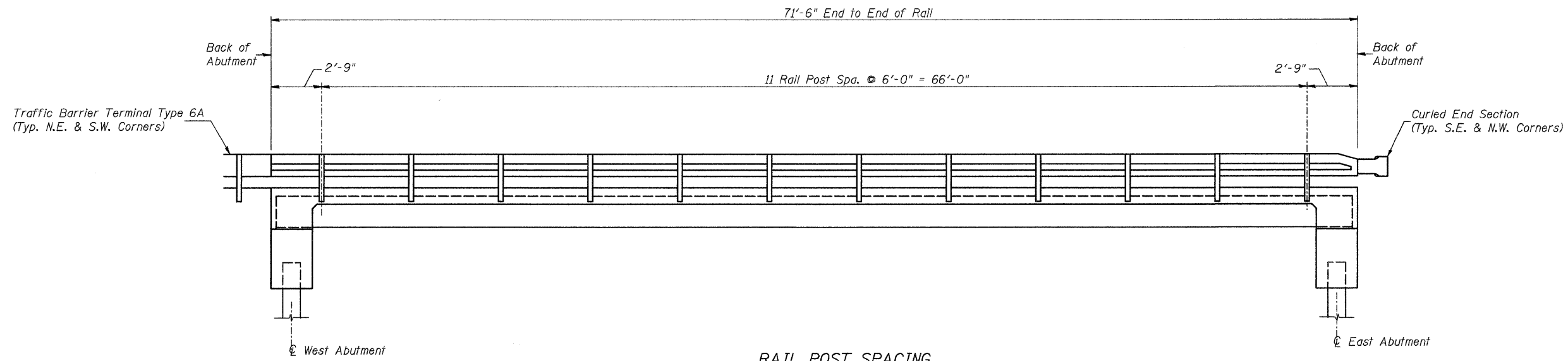
\*\* Add 2 additional #6 g (E) bars at 13" cts. at each railpost  
anchor location (Lap with alternate a(E) bars).

**PLAN**

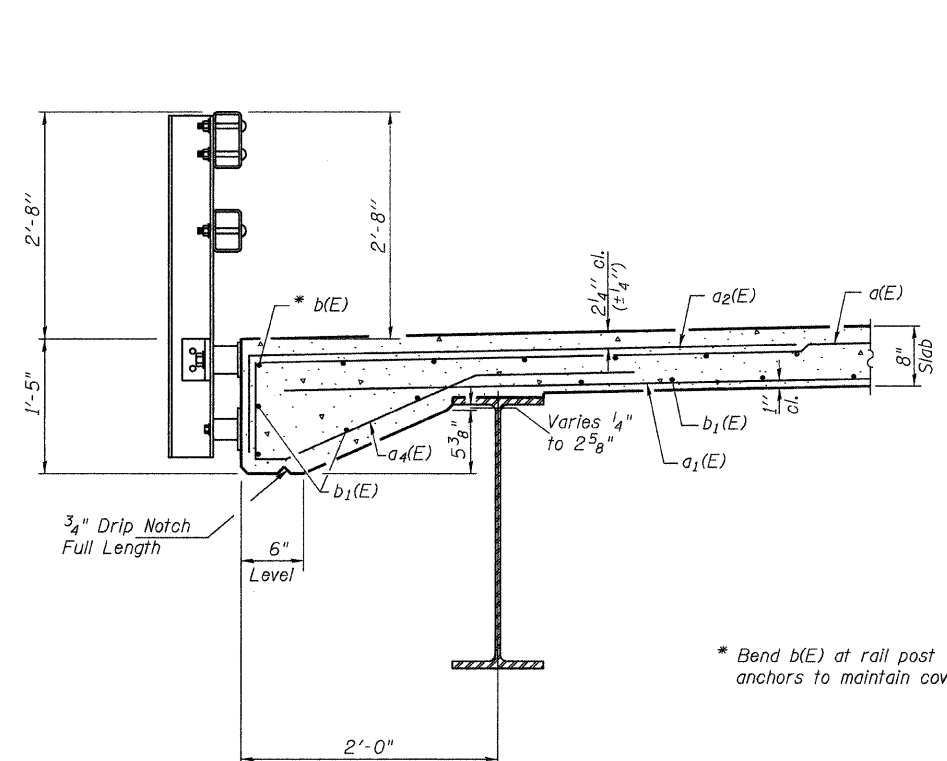


**CROSS SECTION**  
(Looking East)

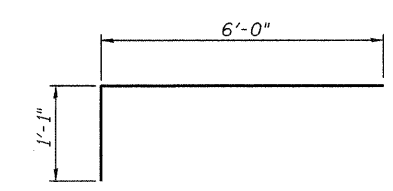
**SUPERSTRUCTURE**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SEC. 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**



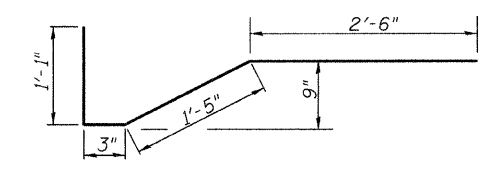
RAIL POST SPACING



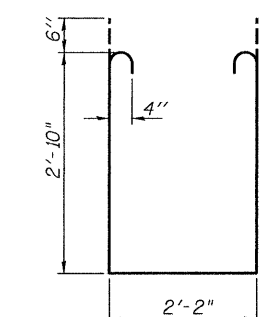
SECTION THRU PARAPET  
See Sheet 8 of 15 for Rail Post Anchor Details



BAR a2(E)



BAR a4(E)



BAR s(E)

**SUPERSTRUCTURE  
BILL OF MATERIAL**

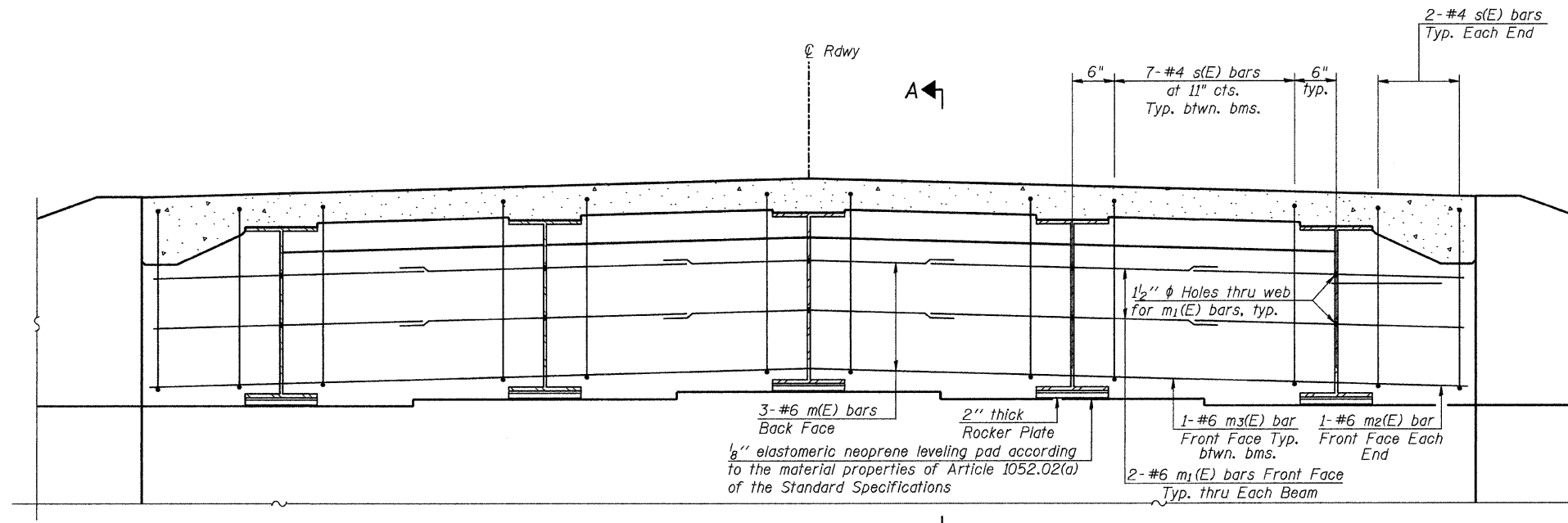
Bar	No.	Size	Length	Shape	
a(E)	131	#5	29'-9"	—	
a1(E)	85	#5	28'-9"	—	
a2(E)	180	#6	7'-1"	┌	
a3(E)	4	#5	30'-9"	—	
a4(E)	132	#5	5'-3"	└	
b(E)	93	#5	24'-11"	—	
b1(E)	64	#5	37'-4"	—	
m(E)	6	#6	30'-8"	—	
m1(E)	20	#6	8'-4"	—	
m2(E)	4	#6	1'-10"	—	
m3(E)	8	#6	6'-7"	—	
s(E)	64	#4	8'-10"	U	
Reinforcement Bars, Epoxy Coated				Pound	15,280
Concrete Superstructure				Cu. Yds.	74.1

① See Special Provisions

**SUPERSTRUCTURE DETAILS**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SEC. 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 10	*	IROQUOIS	31	13
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT BRS-329(105)	
*07-00216-01-BR		CONTRACT #87401		

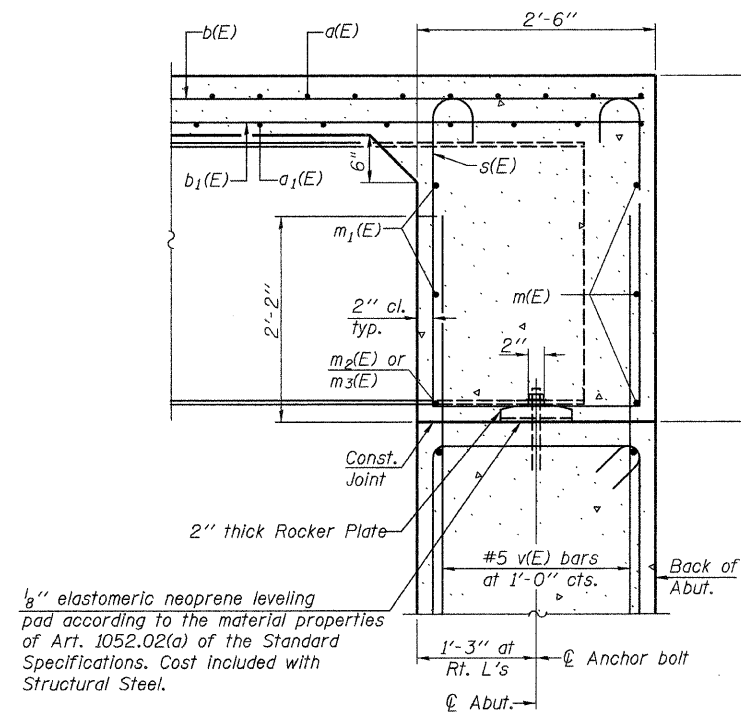
SHEET NO. 7  
OF 15 SHEETS



**DIAPHRAGM ELEVATION AT ABUTMENT**

Notes:  
Reinforcement bars in diaphragm are billed with superstructure on sheet 6 of 15.  
Concrete in diaphragm is included with Concrete Superstructure on sheet 6 of 15.  
For details of bar s(E) see sheet 6 of 15.  
The s(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.  
For placement of v(E) bars see sheet 12 of 15.

**MIN. BAR LAP**  
#6 bar = 2'-9"



**SECTION A-A**  
Dimensions at right angles to abutment, except as shown.

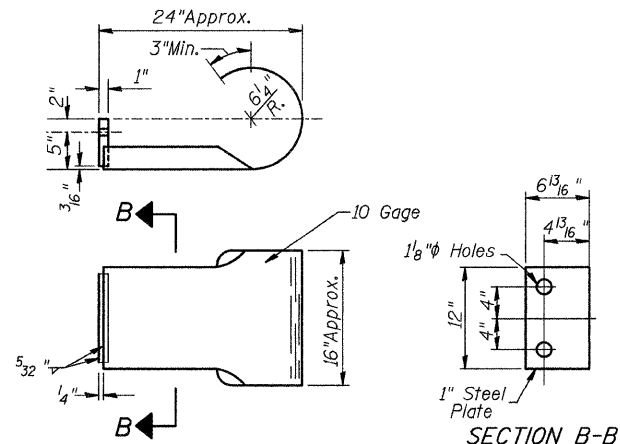
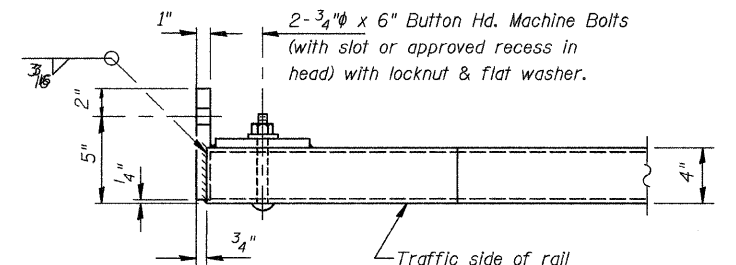
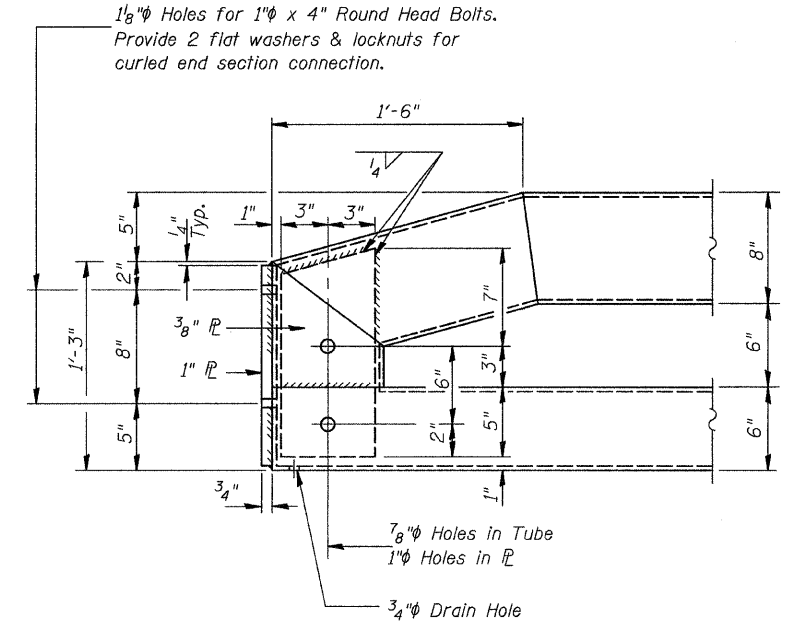
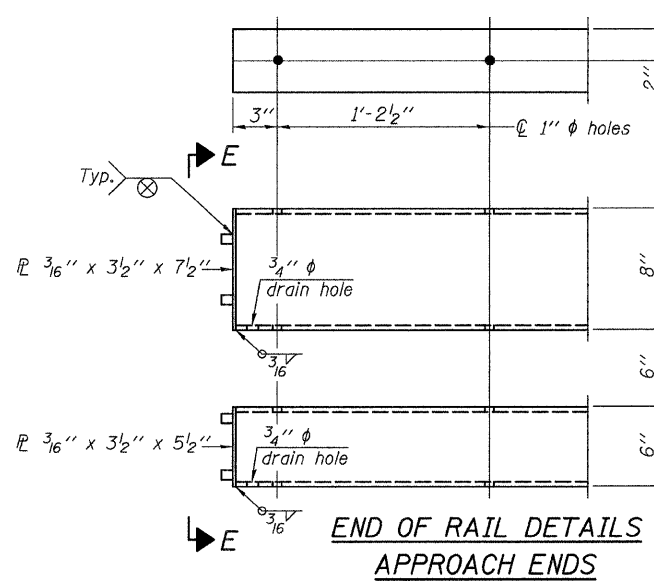
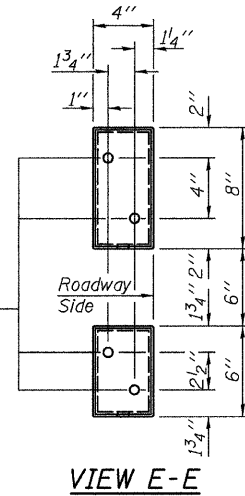
**DIAPHRAGM DETAILS**  
COUNTY HIGHWAY 10 OVER  
FOUNTAIN CREEK TRIBUTARY  
SEC. 07-00216-01-BR  
IROQUOIS COUNTY  
STATION 20+02.60



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CH 10	*	IROQUOIS	31	15
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-329(105)	
*07-00216-01-BR		CONTRACT #87401		

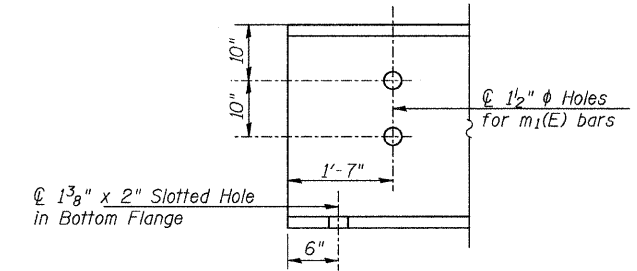
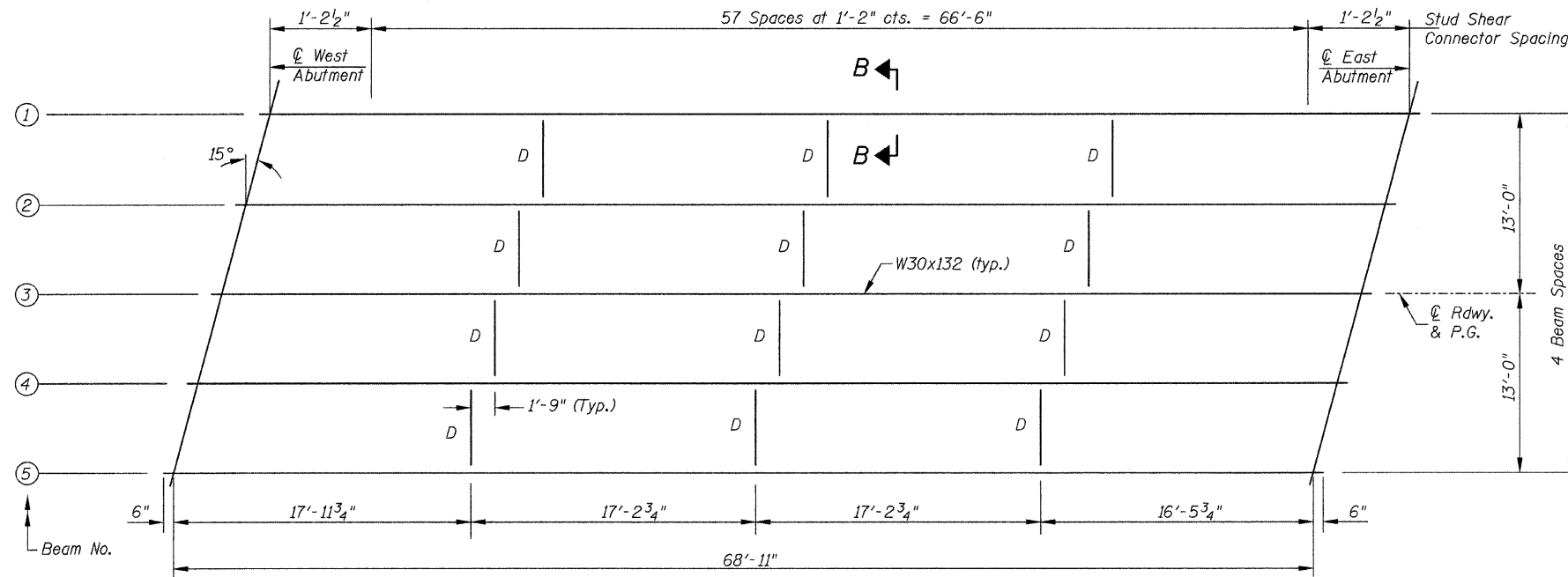
SHEET NO. 9  
OF 15 SHEETS

④ - 5/8" reduced base welded studs. Provide 4 - 5/8" washers and self-locking nuts or nuts and jam nuts for guardrail connection shown on Std. 631032



**CURLED END SECTION**  
(2 Req'd) Cost Included with Steel Bridge Rail, Type SM (Special).  
Terminal Markers - Direct Applied shall be placed on end of each Curled End Section. (N.W. & S.E. Corner)

**STEEL RAILING, TYPE SM (SPECIAL)**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SEC. 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**  
(Sheet 2 of 2)



TYPICAL END OF BEAM DETAIL

**FRAMING PLAN**

Note:  
All beams are W30x132 AASHTO M270 Gr. 50W NTR.  
Load carrying components designated "NTR" shall conform to the Supplemental Requirements of Notch Toughness, Zone 2.

INTERIOR GIRDER MOMENT TABLE		0.5 Sp. 1
$I_s$	( $in^4$ )	5770
$I_c(n)$	( $in^4$ )	15278
$I_c(3n)$	( $in^4$ )	11210
$S_s$	( $in^3$ )	380
$S_c(n)$	( $in^3$ )	557
$S_c(3n)$	( $in^3$ )	503
DC1	( $k/ft$ )	0.818
MDC1	( $k$ )	487
DC2	( $k/ft$ )	0.020
MDC2	( $k$ )	12
DW	( $k/ft$ )	0.325
MDW	( $k$ )	193
$M_L + IM$	( $k$ )	965
$M_u$ (Strength I)	( $k$ )	2602
$\phi_r M_n$	( $k$ )	2898
$f_s$ DC1	( $ksi$ )	15.38
$f_s$ DC2	( $ksi$ )	0.29
$f_s$ DW	( $ksi$ )	4.60
$f_s$ 1.3(L+IM)	( $ksi$ )	27.03
$f_s$ (Service II)	( $ksi$ )	47.30
$V_r$	( $k$ )	16.3

\* Compact sections

INTERIOR GIRDER REACTION TABLE		Abutment
RDC1	( $k$ )	28.2
RDC2	( $k$ )	0.7
RDW	( $k$ )	11.2
$R_L + IM$	( $k$ )	78.1
RTotal	( $k$ )	118.2

$I_s, S_s$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Total-Strength I, and Service II) due to non-composite dead loads ( $in^4$  and  $in^3$ ).

$I_c(n), S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_s$  (Total-Strength I, and Service II) due to short-term composite live loads ( $in^4$  and  $in^3$ ).

$I_c(3n), S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads ( $in^4$  and  $in^3$ ).

DC1: Un-factored non-composite dead load (kips/ft.).

MDC1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

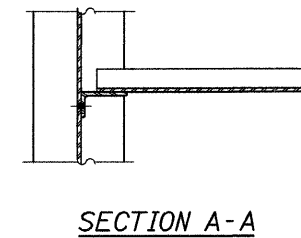
$M_L + IM$ : Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

$M_u$  (Strength I): Factored design moment (kip-ft.).  
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_L + IM$

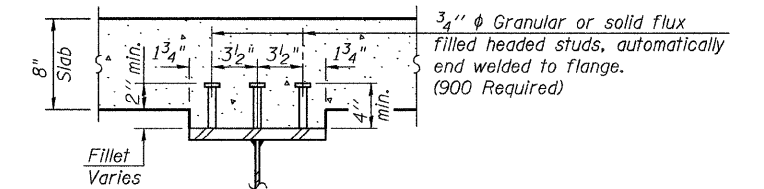
$\phi_r M_n$ : Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).

$f_s$  (Service II): Sum of stresses as computed from the moments below (ksi).  
 $M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_L + IM$

$V_r$ : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.



SECTION A-A

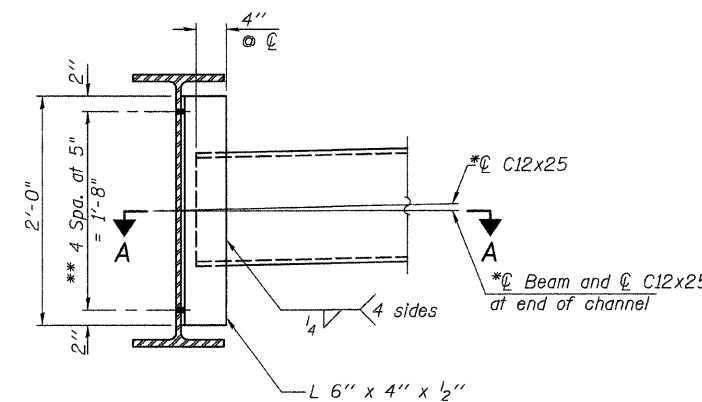


SECTION B-B

**\*\*TOP OF BEAM ELEVATIONS**

LOCATION	Beam 1	Beam 2	Beam 3	Beam 4	Beam 5
W. Abut.	660.58	660.69	660.79	660.69	660.58
E. Abut.	660.58	660.69	660.79	660.69	660.58

\*\*For fabrication only.



**INTERIOR DIAPHRAGM D**  
(12 req'd)

Note:  
Two hardened washers required for each set of oversized holes.  
\*Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.  
\*\*3/4"  $\phi$  HS bolts, 15/16"  $\phi$  holes

**STRUCTURAL STEEL**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SEC. 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**



Notes:

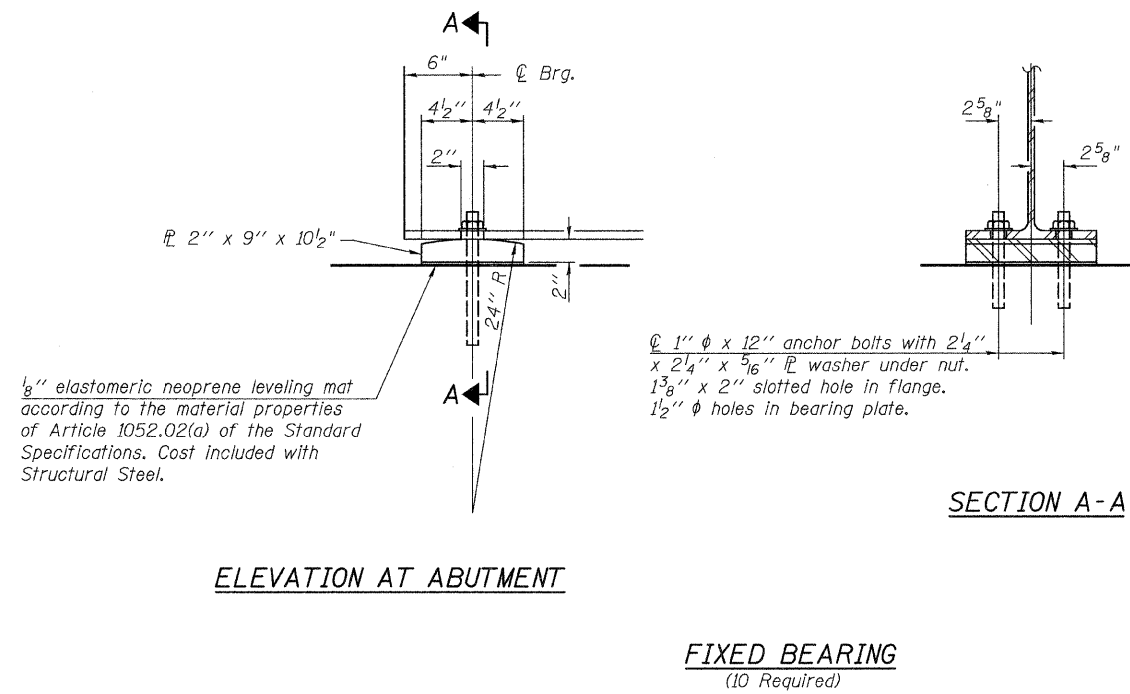
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (Fy=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

All bearing plates shall conform to the requirements of AASHTO M270, Grade 50W.

Anchors shall be set and grout cured for a minimum of 24 hours prior to forming the bridge deck.

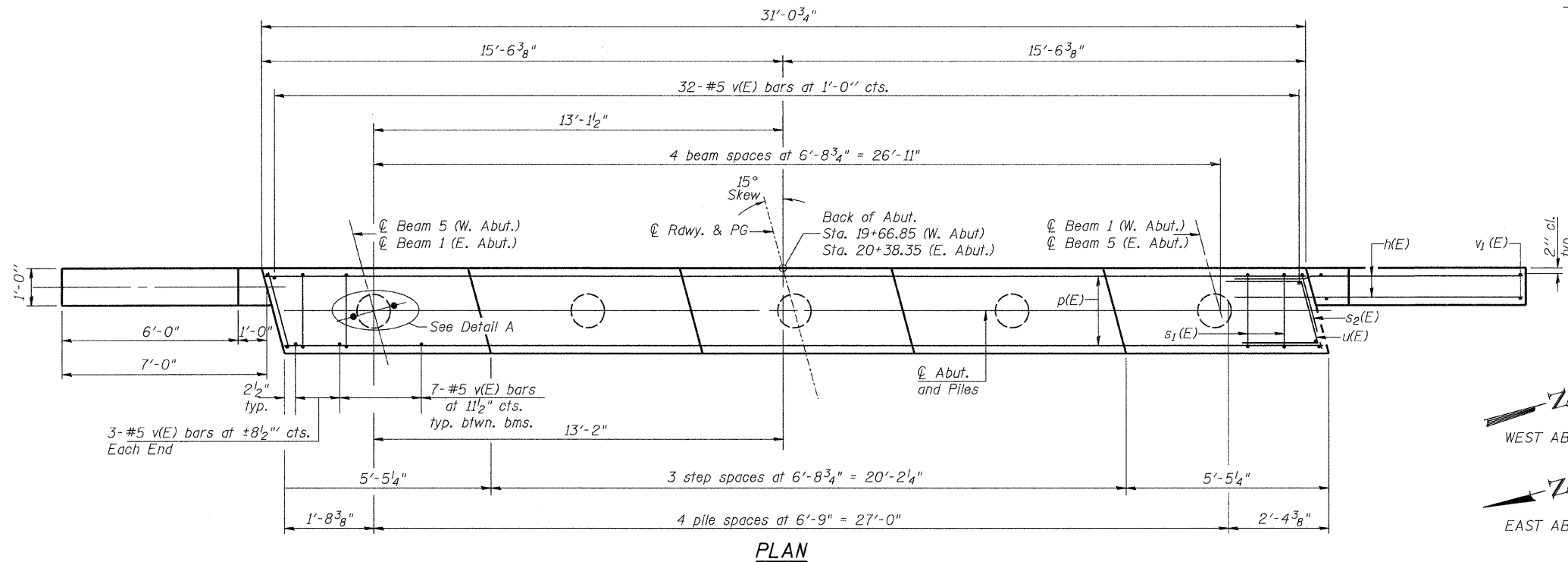
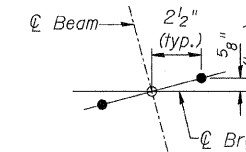
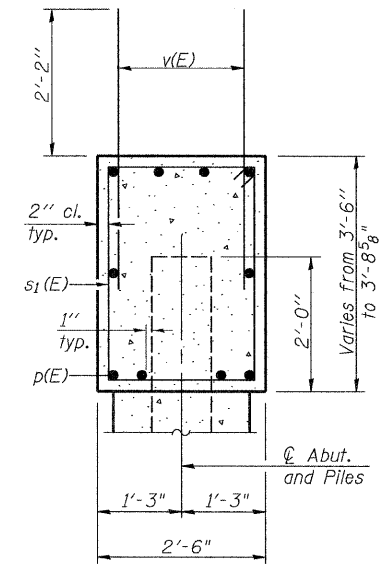
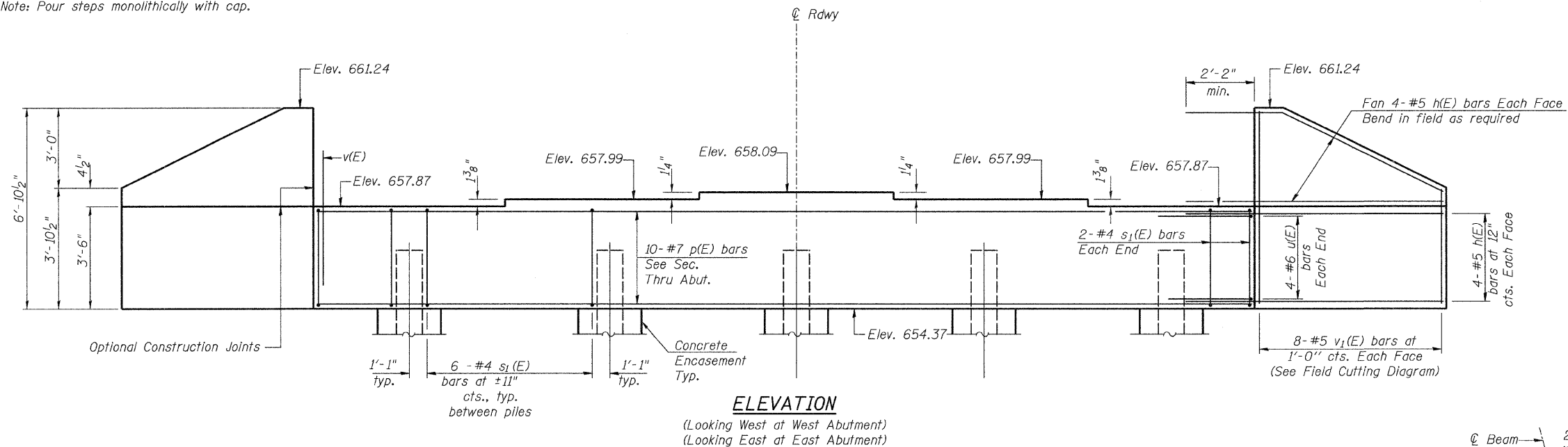


**BILL OF MATERIAL**

ITEM	UNIT	TOTAL
Anchor Bolts, 1"	Each	20

**BEARING DETAILS**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SEC. 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**

Note: Pour steps monolithically with cap.



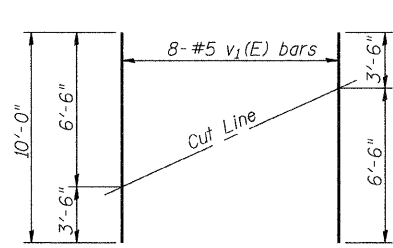
**TWO ABUTMENTS  
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h(E)	64	#5	9'-2"	—
p(E)	20	#7	30'-6"	—
s <sub>1</sub> (E)	56	#4	11'-5"	□
s <sub>2</sub> (E)	4	#4	11'-7"	□
u(E)	16	#6	10'-2"	∟
v(E)	136	#5	4'-4"	—
v <sub>1</sub> (E)	32	#5	10'-0"	—
Structure Excavation		Cu. Yd.	115	
Concrete Structures		Cu. Yd.	25.9	
Reinforcement Bars, Epoxy Coated		Pound	3,510	
Furnishing Metal Shell Piles 12"x0.25"		Foot	408	
Driving Piles		Foot	408	
Test Pile Metal Shells		Each	2	
Pile Shoes		Each	10	
Concrete Encasement		Cu. Yd.	4.6	

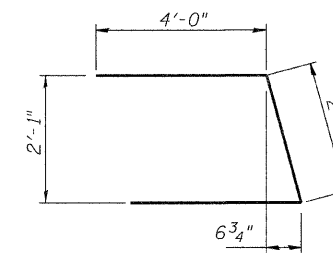
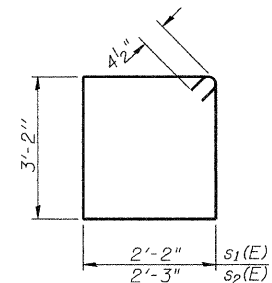
① See Special Provisions

For details of piles and Concrete Encasement, see sheet 13 of 15.  
Space reinforcement in cap to miss anchor bolts.

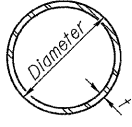
**PILE DATA**  
Type: Metal Shell-12"φx0.25" walls w/ Pile Shoes  
Nominal Required Bearing: 280 k  
Factored Resistance Available: 140 k  
Est. Length: 51'  
No. Production Piles: 8 (4 at each abutment)  
No. Test Piles: 2 (1 at each abutment)



Order v<sub>1</sub>(E) full length. Cut as shown and use remainder of bars in opposite face.

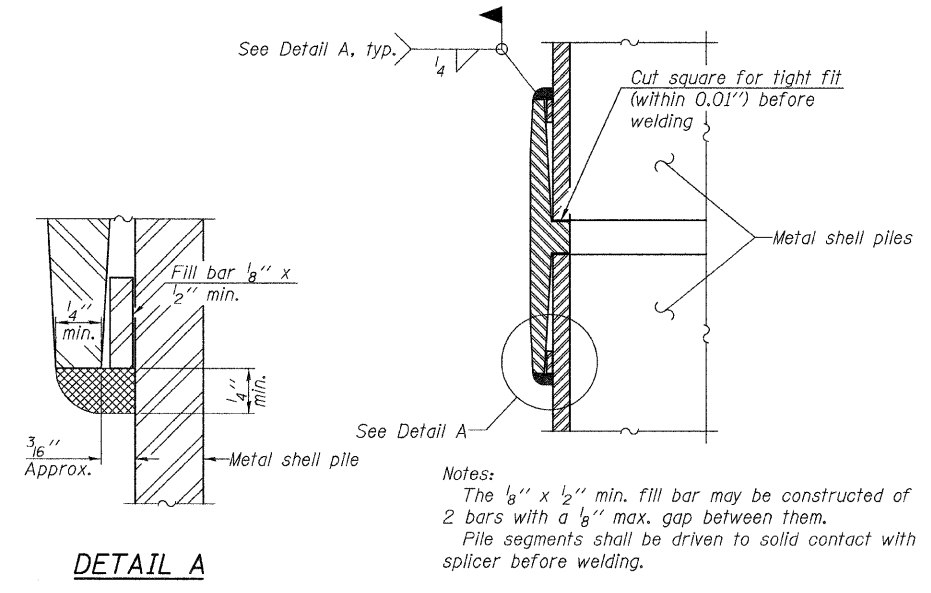


**ABUTMENTS  
COUNTY HIGHWAY 10 OVER  
FOUNTAIN CREEK TRIBUTARY  
SEC. 07-00216-01-BR  
IROQUOIS COUNTY  
STATION 20+02.60**



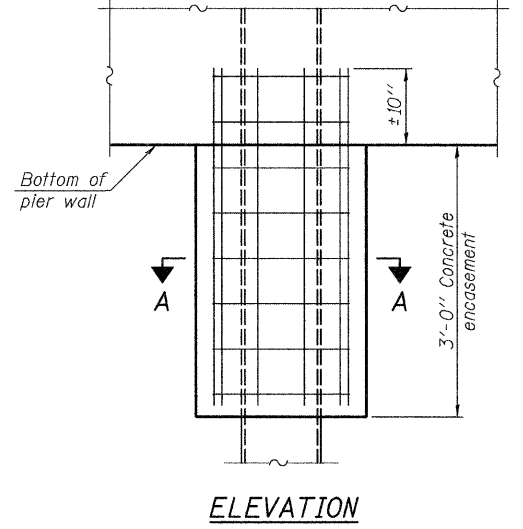
**METAL SHELL PILE TABLE**

Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. <sup>3</sup> /ft.)
PP12	0.179"	22.60	0.0274
PP12	0.250"	31.37	0.0267
PP14	0.250"	36.71	0.0368
PP14	0.312"	45.61	0.0361

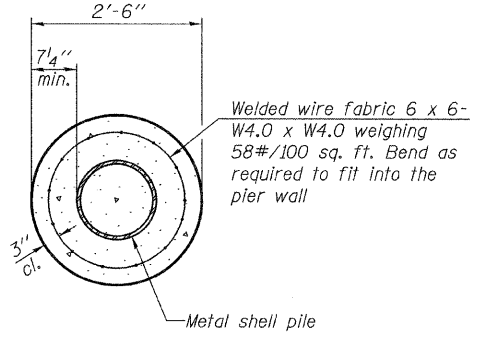


**Notes:**  
 The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them.  
 Pile segments shall be driven to solid contact with splicer before welding.

**WELDED COMMERCIAL SPLICE**



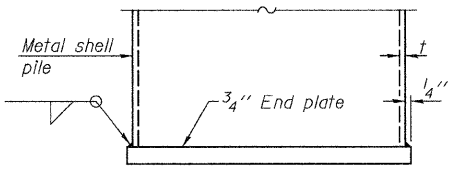
**ELEVATION**



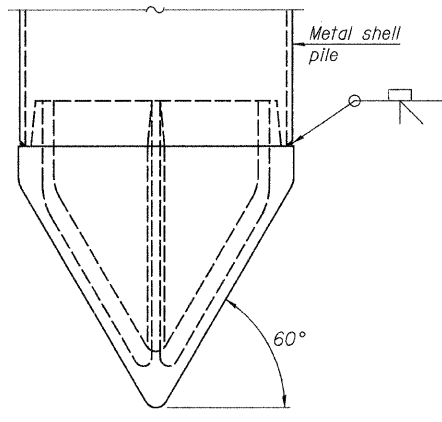
**SECTION A-A**

**Note:**  
 Forms for encasement may be omitted when soil conditions permit.

**CONCRETE ENCASEMENT AT PIERS**



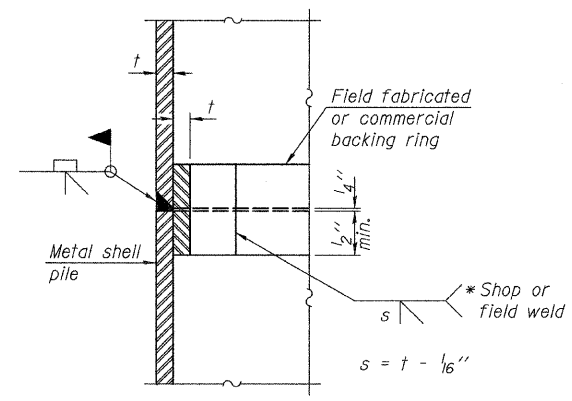
**END PLATE ATTACHMENT**



**Note A:**  
 When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A 148 Grade 90-60 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld.

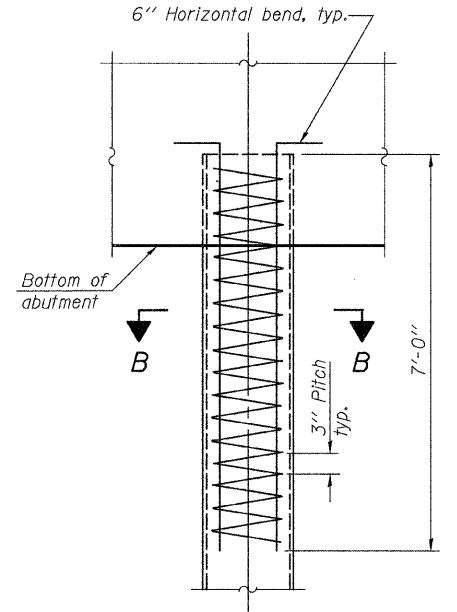
**METAL SHELL PILE SHOE ATTACHMENT**

(See Note A)

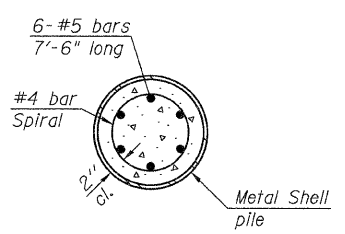


**COMPLETE PENETRATION WELD SPLICE**

\* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.



**ELEVATION**



**SECTION B-B**

**METAL SHELL REINFORCEMENT AT ABUTMENTS**

**Note:**  
 The metal shell piles shall be according to ASTM A 252 Grade 3.

**METAL SHELL PILE DETAILS  
 COUNTY HIGHWAY 10 OVER  
 FOUNTAIN CREEK TRIBUTARY  
 SEC. 07-00216-01-BR  
 IROQUOIS COUNTY  
 STATION 20+02.60**

**BORING LOG** Phone: 815-223-6696  
3705 Progress Blvd. Fax: 815-223-6659  
Peru, IL 61354 e-mail: ms376@comcast.net

Sheet 1 of 3

Client: Hutchison Engineering, Inc. Boring No. B-1  
Project Name: Bridge on C.H. 10 over Fountain Creek Surface Elev. 658.51  
Project Site: Lovejoy Twp. Iroquois County, IL Auger Depth 61.00 Rotary Depth NA  
Section No.: 07-00216-01-BR Start Date 02/06/09 Finish Date 02/06/09

Location: West Abutment  
19+65 6' RT

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES					DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear		
658.51	Bituminous over Aggregate		1						Randy Safiranski Diedrich D-120	
657.51	Stiff Brown/Gray Silty Clay Fill		2							
656.51			3	1	SS	1.2	9	B	23	
655.51			4							
654.51	Stiff Black Silty Clay		5							
653.51			6	2	SS	1.1	9	B	24	
652.51	Stiff Gray Brown Silty Clay		7							
651.51			8	3	SS	1.0	9	B	29	
650.51			9							
649.51	Very Stiff Gray Silty Clay Till		10							
648.51			11	4	SS	1.0	9	B	28	
647.51			12							
646.51	Medium Dense Gray Silt		13							
645.51			14	5	SS	3.5	13	B	21	
644.51			15							
643.51		16								
642.51		17								
641.51		18								
640.51		19								
639.51		20								
638.51		21								
		22								
		23								
		24								
		25								
		26								
		27								
		28								
		29								
		30								
		31								
		32								
		33								
		34								
		35								
		36								
		37								
		38								
		39								
		40								
		41								
		42								

Groundwater Data: Water 5' below top of ground after auger removal.  
Comments: Creek bed ± 12' below deck.

**BORING LOG** Phone: 815-223-6696  
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Sheet 2 of 3

Client: Hutchison Engineering, Inc. Boring No. B-1  
Project Name: Bridge on C.H. 10 over Fountain Creek Surface Elev. 658.51  
Project Site: Lovejoy Twp. Iroquois County, IL Auger Depth 61.00 Rotary Depth NA  
Section No.: 07-00216-01-BR Start Date 02/06/09 Finish Date 02/06/09

Location: West Abutment  
19+65 6' RT

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES					DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear		
637.51	Stiff Gray Silty Clay Loam Till		22						Randy Safiranski Diedrich D-120	
636.51			23	9	SS	1.4	15	B	16	
635.51			24							
634.51	Stiff/Very Stiff Gray Clay Till with Sand Seams		25							
633.51			26	10	SS	1.6	17	B	14	
632.51	Very Stiff Hard Silty Clay Till		27							
631.51			28	11	SS	3.1	16	B	24	
630.51			29							
629.51	Very Dense Gray Sand & Gravel		30							
628.51			31	12	SS	3.0	15	B	24	First Water
627.51	Very Stiff Gray Silty Clay Loam Till with Sand Seams		32							
626.51			33	13	SS	4.1	20	B	19	
625.51			34							
624.51	Very Dense Gray Sand & Gravel		35							
623.51			36	14	SS	5.4	26	B	12	
622.51	Very Stiff Gray Silty Clay Loam Till with Sand Seams		37							
621.51			38							
620.51			39							
619.51	Very Dense Gray Sand & Gravel		40							
618.51			41	15	SS	2.5	19	B	16	
617.51		42								

Groundwater Data: Water 5' below top of ground after auger removal.  
Comments: Creek bed + 12' below deck.

**BORING LOG** Phone: 815-223-6696  
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Peru, IL 61354 e-mail: ms376@comcast.net

Sheet 3 of 3

Client: Hutchison Engineering, Inc. Boring No. B-1  
Project Name: Bridge on C.H. 10 over Fountain Creek Surface Elev. 658.51  
Project Site: Lovejoy Twp. Iroquois County, IL Auger Depth 61.00 Rotary Depth NA  
Section No.: 07-00216-01-BR Start Date 02/06/09 Finish Date 02/06/09

Location: West Abutment  
19+65 6' RT

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES					DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear		
616.51	Very Stiff Gray Silty Clay Loam Till with Sand Seams		43						Randy Safiranski Diedrich D-120	
615.51			44							
614.51			45	20	SS	---	26	---	11	
613.51	Very Dense Sand & Gravel with Cobbles		46							
612.51			47							
611.51	Hard Gray Silty Clay Till		48							
610.51			49							
609.51			50	21	SS	---	20	---	9	
608.51	Very Dense Sand & Gravel with Cobbles		51							
607.51			52							
606.51	Hard Gray Silty Clay Till		53							
605.51			54							
604.51			55							
603.51	Very Dense Sand & Gravel with Cobbles		56							
602.51			57	22	SS	---	20	---	9	
601.51	Hard Gray Silty Clay Till		58							
600.51			59							
599.51			60							
598.51	Very Dense Sand & Gravel with Cobbles		61							
597.51			62	23	SS	7.4	68	S	10	
596.51		63								

Groundwater Data: Water 5' below top of ground after auger removal.  
Comments: Creek bed + 12' below deck.

**SOIL BORING LOGS**  
**COUNTY HIGHWAY 10 OVER**  
**FOUNTAIN CREEK TRIBUTARY**  
**SECTION 07-00216-01-BR**  
**IROQUOIS COUNTY**  
**STATION 20+02.60**

**BORING LOG**  
 Sheet 1 of 3  
 Midwest Testing Services, Inc.  
 3705 Progress Blvd.  
 Peru, IL 61354  
 Phone: 815-223-6696  
 Fax: 815-223-6659  
 e-mail: mts37@comcast.net

Client: Hutchison Engineering, Inc.  
 Project Name: Bridge on C.H. 10 over Fountain Creek  
 Project Site: Lovejoy Twp. Iroquois County, IL  
 Section No.: 07-00216-01-BR

Boring No. B-2  
 Surface Elev. 658.10  
 Auger Depth 61.00 Rotary Depth NA  
 Start Date 02/06/09 Finish Date 02/06/09

Location: East Abutment  
20+40 6' LT

DEPTH ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES					DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear		
658.10	Bituminous over Aggregate		1						Randy Safianski Dietrich D-120	
656.10	Stiff Brown/Gray Silty Clay Till		2							
655.10			3	1	1.1	9	B	23		
654.10			4							
653.10	Stiff Gray Brown Silty Clay		5							
652.10			6	2	SS	1.2	9	B		23
651.10	Brown/Gray Gravelly Loam		7							
650.10			8	3	SS	1.0	9	B		26
649.10			9							
648.10	Very Stiff Gray Silty Clay Till		10							
647.10			11	4	SS	---	11	---		25
646.10			12							
645.10	Medium Dense Gray/Brown Silt		13							
644.10			14	5	SS	---	11	---		25
643.10	Hard Gray Silty Clay Till		15							
642.10			16	6	SS	3.9	19	B		23
641.10			17							
640.10	Very Stiff Gray Silty Clay Till		18							
639.10			19	7	SS	3.0	13	B		22
638.10		20	8	SS	---	11	---	22		

Groundwater Data: Water 5' below top of ground after auger removal.  
 Comments: Creek bed ± 12' below deck.

**BORING LOG**  
 Sheet 2 of 3  
 Midwest Testing Services, Inc.  
 3705 Progress Blvd.  
 Peru, IL 61354  
 Phone: 815-223-6696  
 Fax: 815-223-6659  
 e-mail: mts37@comcast.net

Client: Hutchison Engineering, Inc.  
 Project Name: Bridge on C.H. 10 over Fountain Creek  
 Project Site: Lovejoy Twp. Iroquois County, IL  
 Section No.: 07-00216-01-BR

Boring No. B-2  
 Surface Elev. 658.10  
 Auger Depth 61.00 Rotary Depth NA  
 Start Date 02/06/09 Finish Date 02/06/09

Location: East Abutment  
20+40 6' LT

DEPTH ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES					DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear		
637.10	Stiff Gray Silty Clay Loam Till		22						Randy Safianski Dietrich D-120	
635.10			23	9	SS	2.3	11	B		18
634.10			24							
633.10	Medium Dense Gray Medium/Coarse Sand		25							
632.10			26	10	SS	---	19	---		12
631.10	Stiff/Very Stiff Gray Silty Clay Till with Silt and Sand Seams		27							
630.10			28	11	SS	3.1	19	B		21
629.10			29							
628.10	Very Stiff Gray Silty Clay Till		30							
627.10			31	12	SS	3.1	20	B		21
626.10	Very Stiff Gray Silty Clay Till		32							
625.10			33	13	SS	4.1	24	B		19
624.10			34							
623.10	Very Stiff Gray Silty Clay Till		35							
622.10			36	14	SS	---	31	---		18
621.10	Very Stiff Gray Silty Clay Till		37							
620.10			38							
619.10			39							
618.10	Very Stiff Gray Silty Clay Till		40							
617.10			41	15	SS	4.7	27	B		16

Groundwater Data: Water 5' below top of ground after auger removal.  
 Comments: Creek bed +12' below deck.

**BORING LOG**  
 Sheet 3 of 3  
 Midwest Testing Services, Inc.  
 3705 Progress Blvd.  
 Peru, IL 61354  
 Phone: 815-223-6696  
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 e-mail: mts37@comcast.net

Client: Hutchison Engineering, Inc.  
 Project Name: Bridge on C.H. 10 over Fountain Creek  
 Project Site: Lovejoy Twp. Iroquois County, IL  
 Section No.: 07-00216-01-BR

Boring No. B-2  
 Surface Elev. 658.10  
 Auger Depth 61.00 Rotary Depth NA  
 Start Date 02/06/09 Finish Date 02/06/09

Location: East Abutment  
20+40 6' LT

DEPTH ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES					DRILLED BY	REMARKS
				Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear		
616.10	Very Stiff Gray Silty Clay Till		43						Randy Safianski Dietrich D-120	
614.10			44							
613.10	Very Dense Medium Sand		45							
612.10			46	20	SS	---	70	---		10
611.10	Very Dense Gray Silty Clay Till		47							
610.10			48							
609.10			49							
608.10	Very Dense Sand & Gravel with Cobbles		50							
607.10			51	21	SS	---	60	---		9
606.10	Hard Gray Silty Clay Till		52							
605.10			53							
604.10			54							
603.10	Very Stiff Gray Silty Clay Till		55							
602.10			56	22	SS	---	60	---		9
601.10	Very Stiff Gray Silty Clay Till		57							
600.10			58							
599.10			59							
598.10	Very Stiff Gray Silty Clay Till		60							
597.10			61	23	SS	7.4	63	S		10
596.10		62								

Groundwater Data: Water 5' below top of ground after auger removal.  
 Comments: Creek bed +12' below deck.

**SOIL BORING LOGS**  
 COUNTY HIGHWAY 10 OVER  
 FOUNTAIN CREEK TRIBUTARY  
 SECTION 07-00216-01-BR  
 IROQUOIS COUNTY  
 STATION 20+02.60























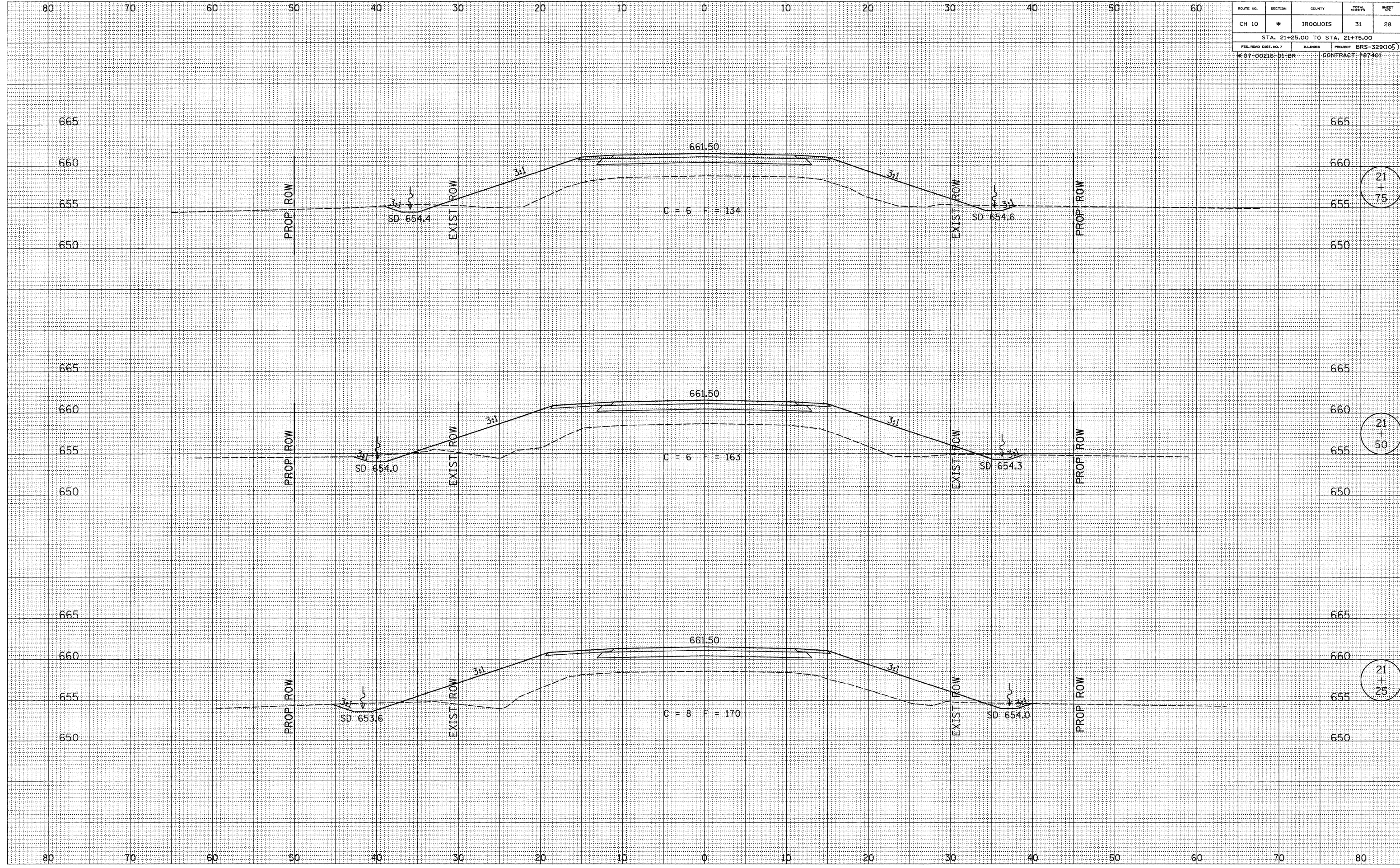




ROUTE NO.	SECTION	COUNTY	SHEET	SHEET
CH 10	*	IRROUOIS	31	28
STA. 21+25.00 TO STA. 21+75.00				
FED. ROAD DIST. NO. 7	ILLINOIS	PROJECT	BRS-329(105)	
07-00216-D1-BR		CONTRACT	P87401	

DATE \_\_\_\_\_  
 BY \_\_\_\_\_  
 SURVEYED \_\_\_\_\_  
 SURVEY \_\_\_\_\_  
 NOTE BOOK \_\_\_\_\_  
 TEMPLATE \_\_\_\_\_  
 AREAS \_\_\_\_\_  
 AREAS CHECKED \_\_\_\_\_

DATE \_\_\_\_\_  
 BY \_\_\_\_\_  
 SURVEYED \_\_\_\_\_  
 SURVEY \_\_\_\_\_  
 NOTE BOOK \_\_\_\_\_  
 TEMPLATE \_\_\_\_\_  
 AREAS \_\_\_\_\_  
 AREAS CHECKED \_\_\_\_\_



21  
+  
75

21  
+  
50

21  
+  
25











