

TYPICAL END OF BEAM DETAIL

FRAMING PLAN

Note:
All beams are W30x132 AASHTO M270 Gr. 50W NTR.
Load carrying components designated "NTR" shall conform to the Supplemental Requirements of Notch Toughness, Zone 2.

| INTERIOR GIRDER MOMENT TABLE | | 0.5 Sp. 1 |
|------------------------------|------------|-----------|
| I_s | (in^4) | 5770 |
| $I_c(n)$ | (in^4) | 15278 |
| $I_c(3n)$ | (in^4) | 11210 |
| S_s | (in^3) | 380 |
| $S_c(n)$ | (in^3) | 557 |
| $S_c(3n)$ | (in^3) | 503 |
| DC1 | (k/ft) | 0.818 |
| MDC1 | (k) | 487 |
| DC2 | (k/ft) | 0.020 |
| MDC2 | (k) | 12 |
| DW | (k/ft) | 0.325 |
| MDW | (k) | 193 |
| $M_L + IM$ | (k) | 965 |
| M_u (Strength I) | (k) | 2602 |
| $\phi_r M_n$ | (k) | 2898 |
| f_s DC1 | (ksi) | 15.38 |
| f_s DC2 | (ksi) | 0.29 |
| f_s DW | (ksi) | 4.60 |
| f_s 1.3(L+IM) | (ksi) | 27.03 |
| f_s (Service II) | (ksi) | 47.30 |
| V_r | (k) | 16.3 |

* Compact sections

| INTERIOR GIRDER REACTION TABLE | | Abutment |
|--------------------------------|---------|----------|
| RDC1 | (k) | 28.2 |
| RDC2 | (k) | 0.7 |
| RDW | (k) | 11.2 |
| $R_L + IM$ | (k) | 78.1 |
| RTotal | (k) | 118.2 |

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in^4 and in^3).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in^4 and in^3).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in^4 and in^3).

DC1: Un-factored non-composite dead load (kips/ft.).

MDC1: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

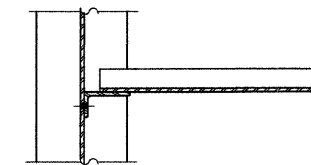
$M_L + IM$: Un-factored live load moment plus dynamic load allowance (Impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_L + IM$

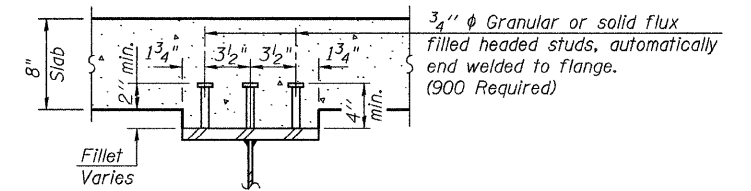
$\phi_r M_n$: Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).

f_s (Service II): Sum of stresses as computed from the moments below (ksi).
 $M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_L + IM$

V_r : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.



SECTION A-A

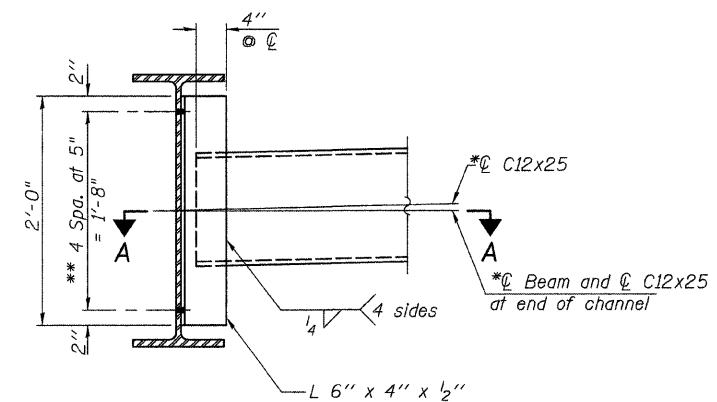


SECTION B-B

****TOP OF BEAM ELEVATIONS**

| LOCATION | Beam 1 | Beam 2 | Beam 3 | Beam 4 | Beam 5 |
|----------|--------|--------|--------|--------|--------|
| W. Abut. | 660.58 | 660.69 | 660.79 | 660.69 | 660.58 |
| E. Abut. | 660.58 | 660.69 | 660.79 | 660.69 | 660.58 |

**For fabrication only.



INTERIOR DIAPHRAGM D
(12 req'd)

Note:
Two hardened washers required for each set of oversized holes.
*Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.
**3/4" ϕ HS bolts, 15/16" ϕ holes

STRUCTURAL STEEL
COUNTY HIGHWAY 10 OVER
FOUNTAIN CREEK TRIBUTARY
SEC. 07-00216-01-BR
IROQUOIS COUNTY
STATION 20+02.60