

SHEET NO. 46
86 SHEETS

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads ($in.^4$ and $in.^3$).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads ($in.^4$ and $in.^3$).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads ($in.^4$ and $in.^3$).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

$M_L + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_U (Strength I): Factored design moment (kip-ft.).

$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_L + IM$

$\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

$\phi_f M_{nc}$: Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).

f_s (Service II): Sum of stresses as computed from the moments below (ksi).

$M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_L + IM$

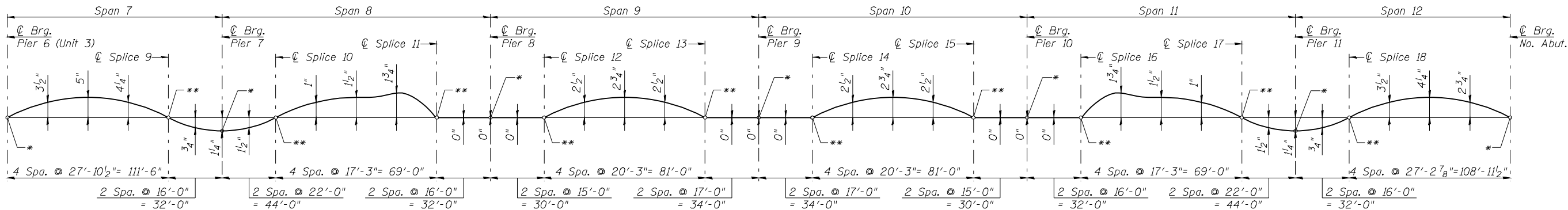
Strength I: Sum of stresses as computed from the moments below on non-compact section (ksi).

$1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_L + IM$

V_f : Factored shear range computed according to Article 6.10.10.

*** Compact Sections.
**** Non-compact and Slender Sections.

	Pier 6	Pier 7	Pier 8	Pier 9	Pier 10	Pier 11	South Abutment
<i>R_{DC1}</i> (k)	48.7	154.1	125.0	137.5	125.6	152.4	47.6
<i>R_{DC2}</i> (k)	8.3	24.7	20.9	22.2	20.9	24.4	8.2
<i>R_{DW}</i> (k)	16.7	49.4	41.8	44.4	41.9	48.9	16.3
<i>R_L + IM</i> (k)	102.1	182.0	172.5	175.9	172.4	181.1	101.5
<i>R_{Total}</i> (k)	175.8	410.2	360.2	380.0	360.8	406.8	173.6



* See Table for Final Top of Web Elevations at abutments and piers.
 ** Theoretical Top of Web Elevations before dead load deflections.

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Girder Number	℄ Brg. Pier 6	℄ Splice No. 9	℄ Brg. Pier 7	℄ Splice No. 10	℄ Splice No. 11	℄ Brg. Pier 8	℄ Splice No. 12	℄ Splice No. 13	℄ Brg. Pier 9	℄ Splice No. 14	℄ Splice No. 15	℄ Brg. Pier 10	℄ Splice No. 16	℄ Splice No. 17	℄ Brg. Pier 11	℄ Splice No. 18	So. Abut.
Girder 1	475.22	474.86	474.57	474.41	473.98	473.85	473.73	473.30	473.13	472.96	472.58	472.40	472.22	471.96	471.66	471.63	471.07
Girder 2	475.34	474.97	474.67	474.51	474.09	473.96	473.84	473.41	473.24	473.06	472.68	472.51	472.32	472.06	471.77	471.74	471.17
Girder 3	475.43	475.05	474.76	474.60	474.17	474.04	473.92	473.49	473.32	473.15	472.77	472.59	472.41	472.15	471.85	471.82	471.26
Girder 4	475.43	475.04	474.74	474.58	474.16	474.03	473.90	473.48	473.30	473.13	472.75	472.58	472.39	472.13	471.84	471.80	471.24
Girder 5	475.32	474.92	474.63	474.47	474.04	473.91	473.79	473.36	473.19	473.02	472.64	472.46	472.28	472.02	471.72	471.69	471.13
Girder 6	475.20	474.79	474.49	474.33	473.91	473.78	473.65	473.22	473.05	472.88	472.50	472.33	472.14	471.88	471.59	471.55	470.99

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