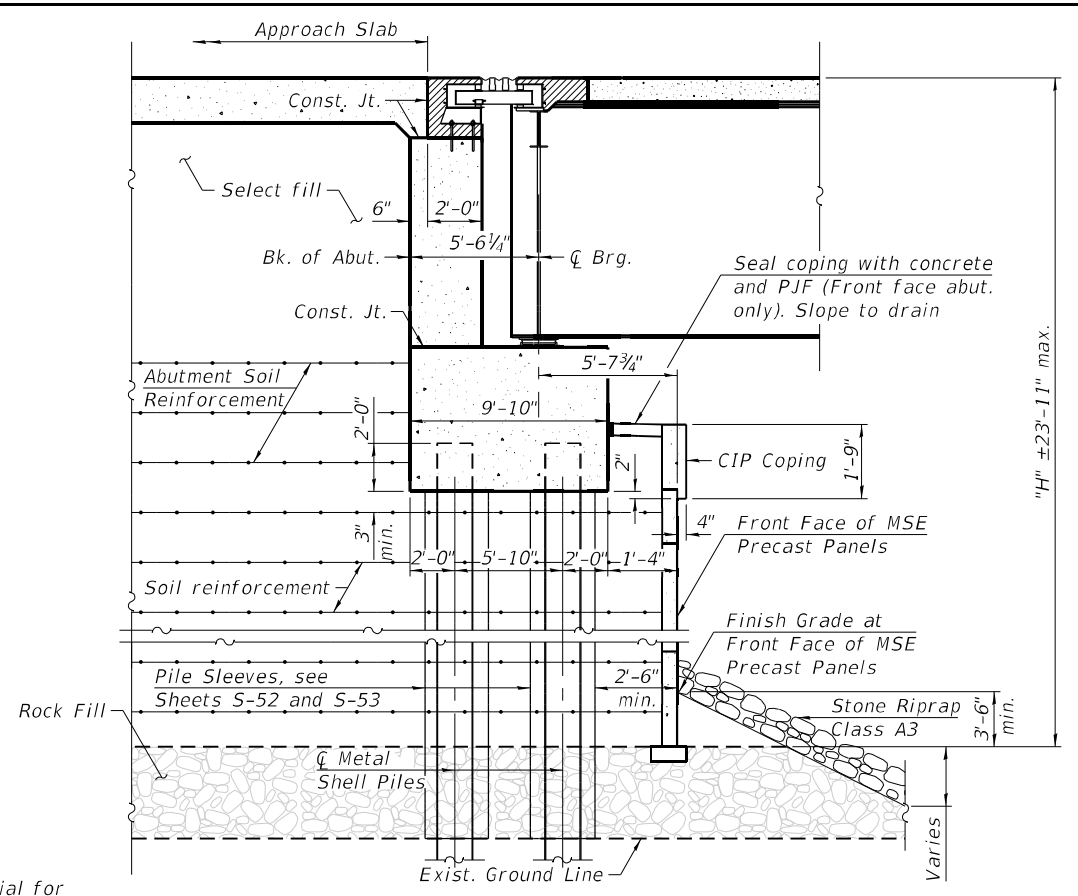


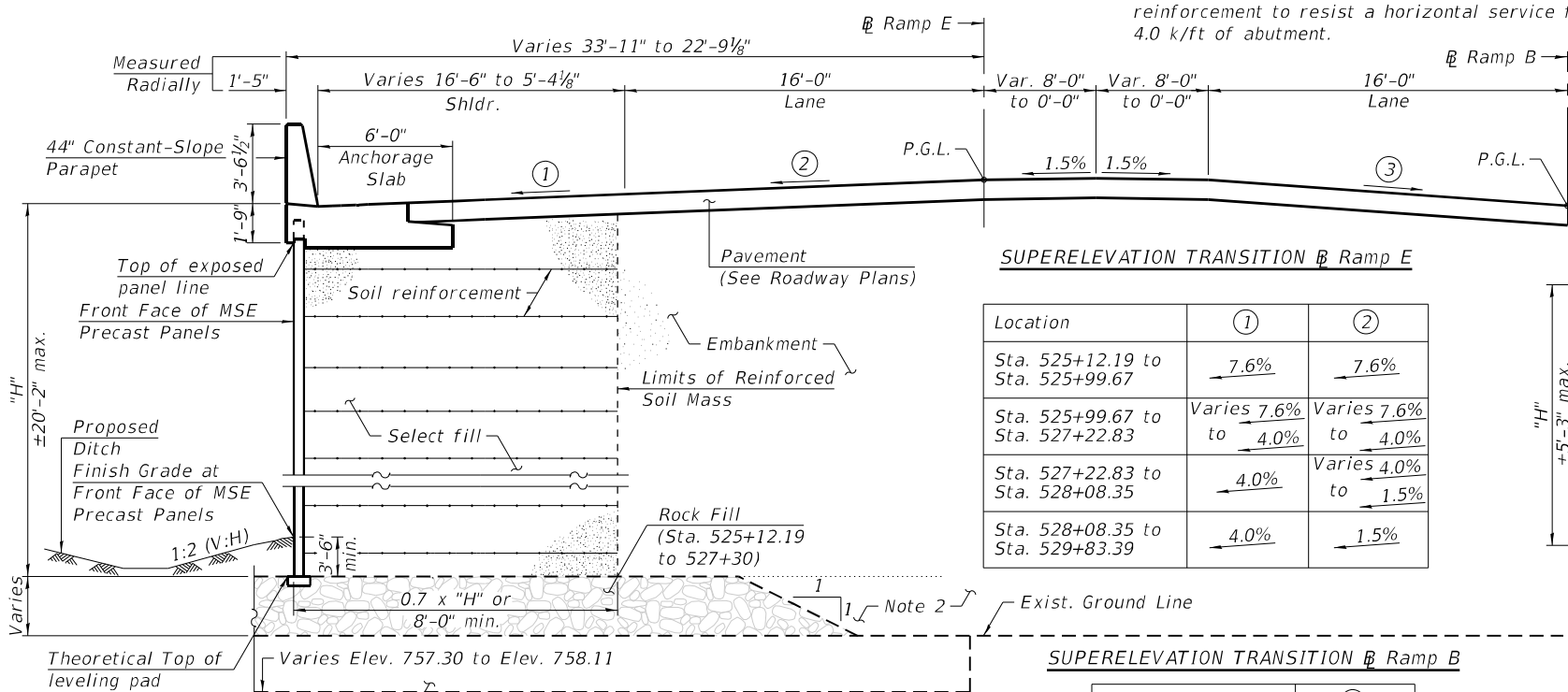
SECTION AT APPROACH SLAB
(Looking Upstation)



SECTION THRU ABUTMENT

NOTES:

- Limits of Removal and Disposal of Unsuitable Material for Structures, replace with embankment except where Rock Fill in removal area.
- Embankment shall be placed from top of leveling pad at north wall to existing ground line except where Rock Fill is located.
- Work this sheet with S-53 of S-106.
- The MSE wall supplier shall design the abutment soil reinforcement to resist a horizontal service force of 4.0 k/ft of abutment.



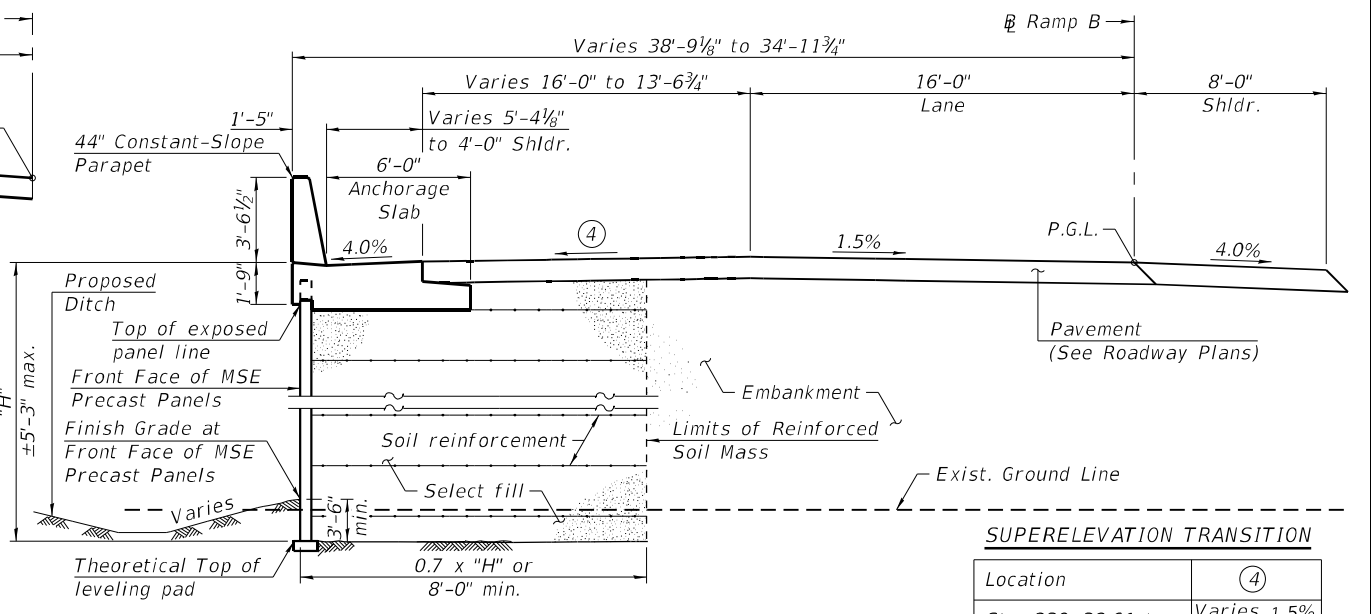
SECTION AT ROADWAY
(Looking Upstation)

SUPERELEVATION TRANSITION @ Ramp E

Location	①	②
Sta. 525+12.19 to Sta. 525+99.67	7.6%	7.6%
Sta. 525+99.67 to Sta. 527+22.83	Varies 7.6% to 4.0%	Varies 7.6% to 4.0%
Sta. 527+22.83 to Sta. 528+08.35	4.0%	Varies 4.0% to 1.5%
Sta. 528+08.35 to Sta. 529+83.39	4.0%	1.5%

SUPERELEVATION TRANSITION @ Ramp B

Location	③
Sta. 236+05.00 to Sta. 238+19.04	Varies 7.8% to 1.5%



SECTION AT ROADWAY
(Looking Upstation)
(@ Ramp B Sta 239+38.61 to Sta 240+00.00)

SUPERELEVATION TRANSITION

Location	④
Sta. 239+38.61 to Sta. 240+38.61	Varies 1.5% to 1.5%

MODEL: Sheet
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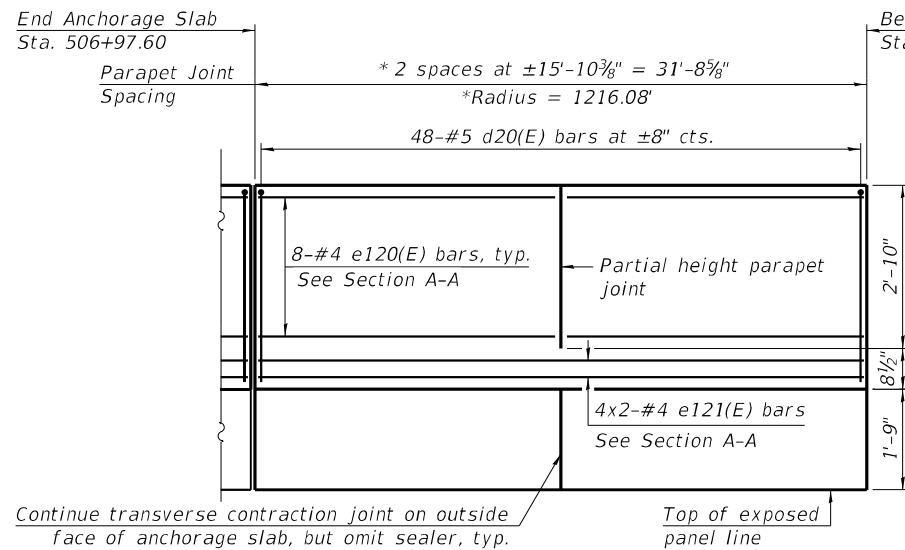


DESIGNED - KWB	REVISED -
CHECKED - RPW	REVISED -
DRAWN - LMC	REVISED -
CHECKED - MDC	REVISED -

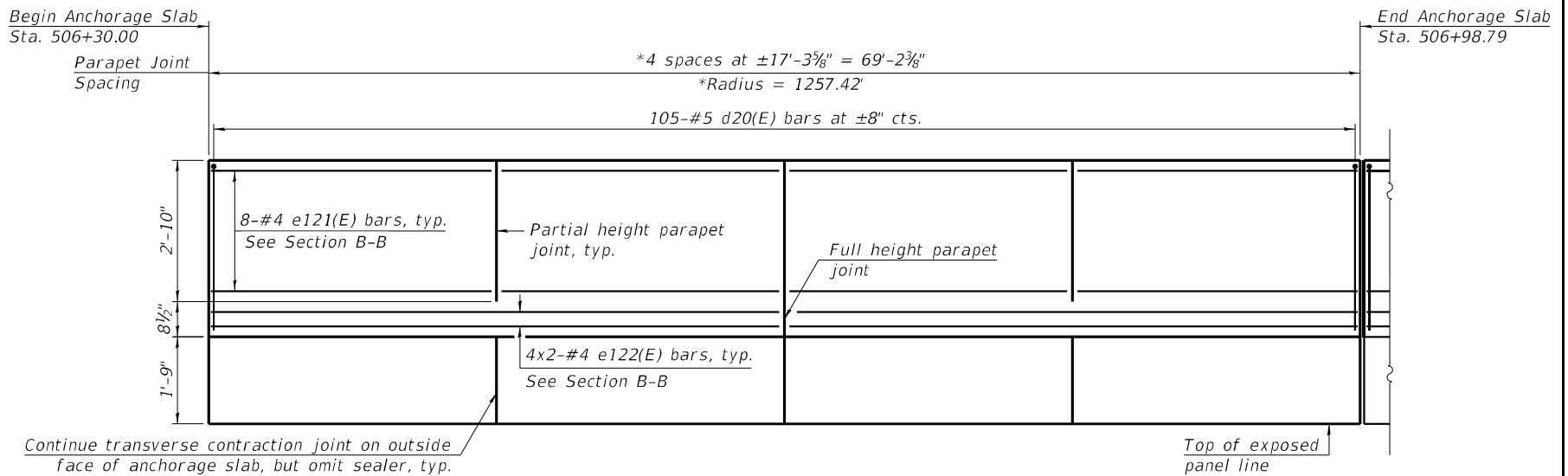
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WEST ABUTMENT - MSE WALL - SECTIONS
STRUCTURE NO. 010-1001
SHEET NO. S-59 OF S-106 SHEETS

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 701
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



OUTSIDE ELEVATION OF SOUTH PARAPET - EAST APPROACH



OUTSIDE ELEVATION OF NORTH PARAPET - EAST APPROACH

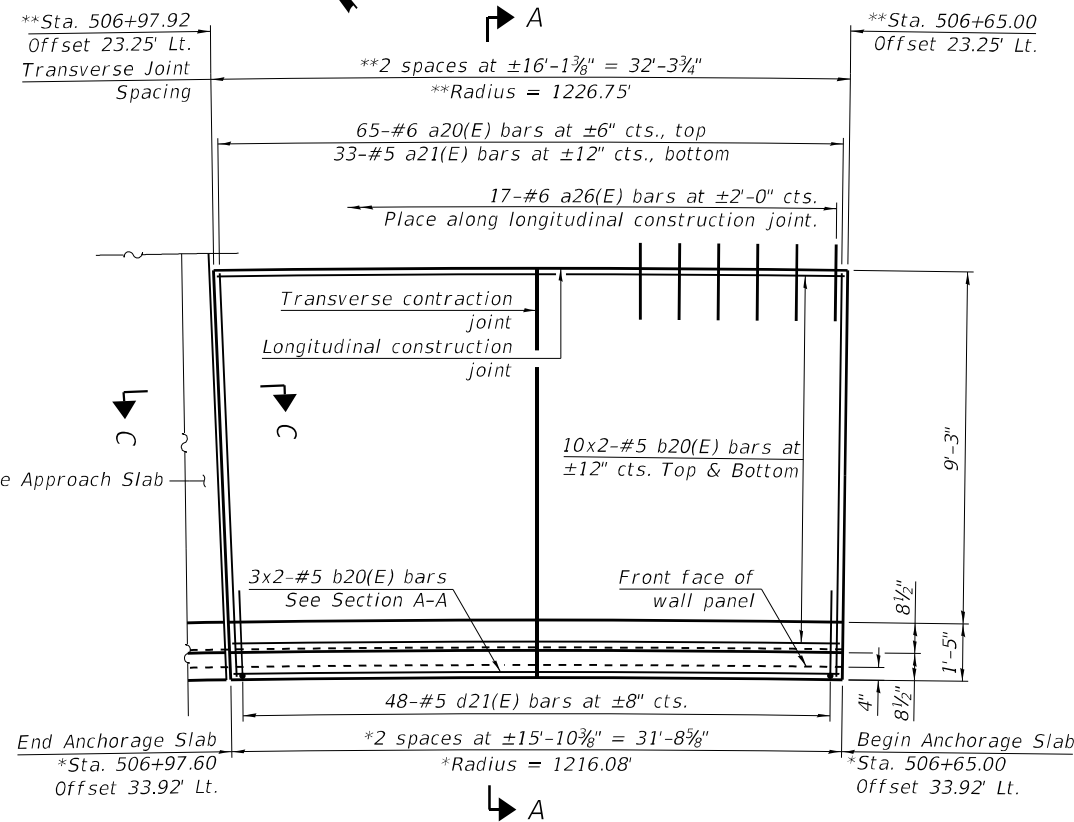
* Measured along outside face of parapet
 ** Measured along inside edge of anchorage slab

MINIMUM BAR LAP

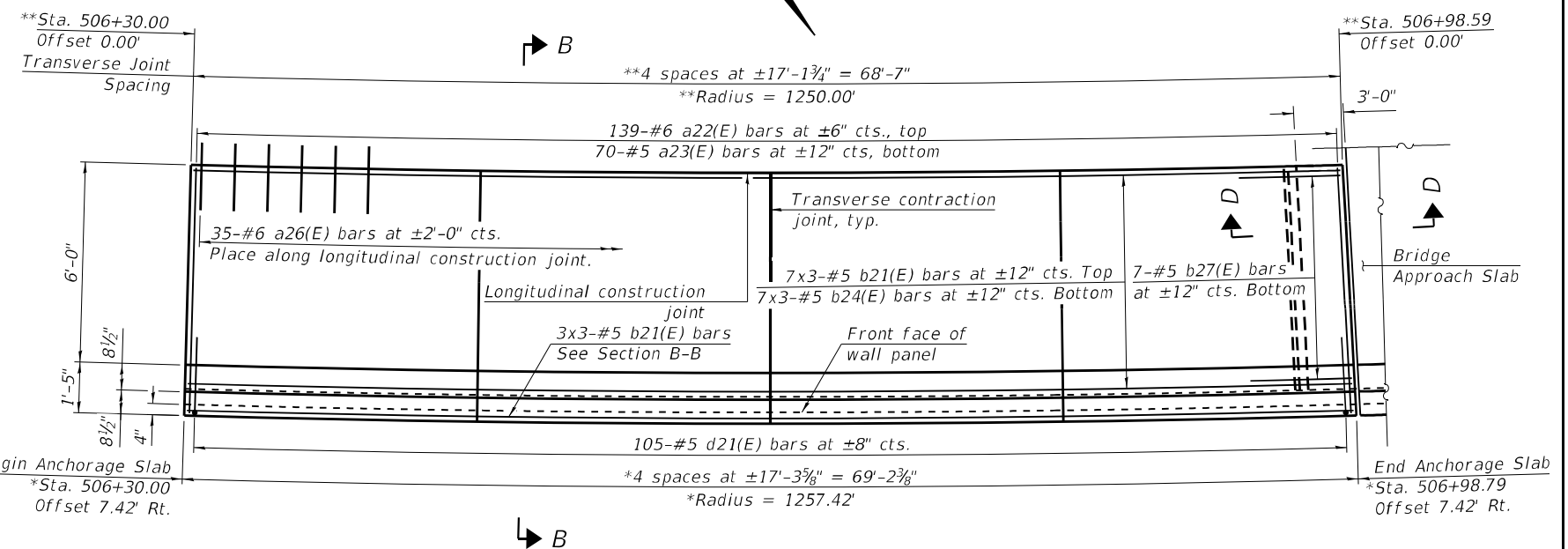
#4 Bar = 2'-5"
 #5 Bar = 3'-0"

NOTES:

1. Place Transverse Contraction Joints perpendicular to outside face of parapet.
2. For Sections A-A, B-B, C-C, and D-D, see Sheet S-61 of S-106.
3. Bars indicated thus 10x2-#5 etc. indicates 10 lines of bars with 2 lengths per line.
4. For bar details and Bill of Material, see Sheet S-61 of S-106.
5. Work this sheet with Sheets S-61 and S-69 of S-106.
6. Stations and offsets are measured from Baseline Ramp E.



PLAN - SOUTH PARAPET & ANCHORAGE SLAB - EAST APPROACH



PLAN - NORTH PARAPET & ANCHORAGE SLAB - EAST APPROACH

MODEL: Sheet
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DESIGNED - KWB	REVISED -
CHECKED - RPW	REVISED -
DRAWN - LMC	REVISED -
CHECKED - MDC	REVISED -

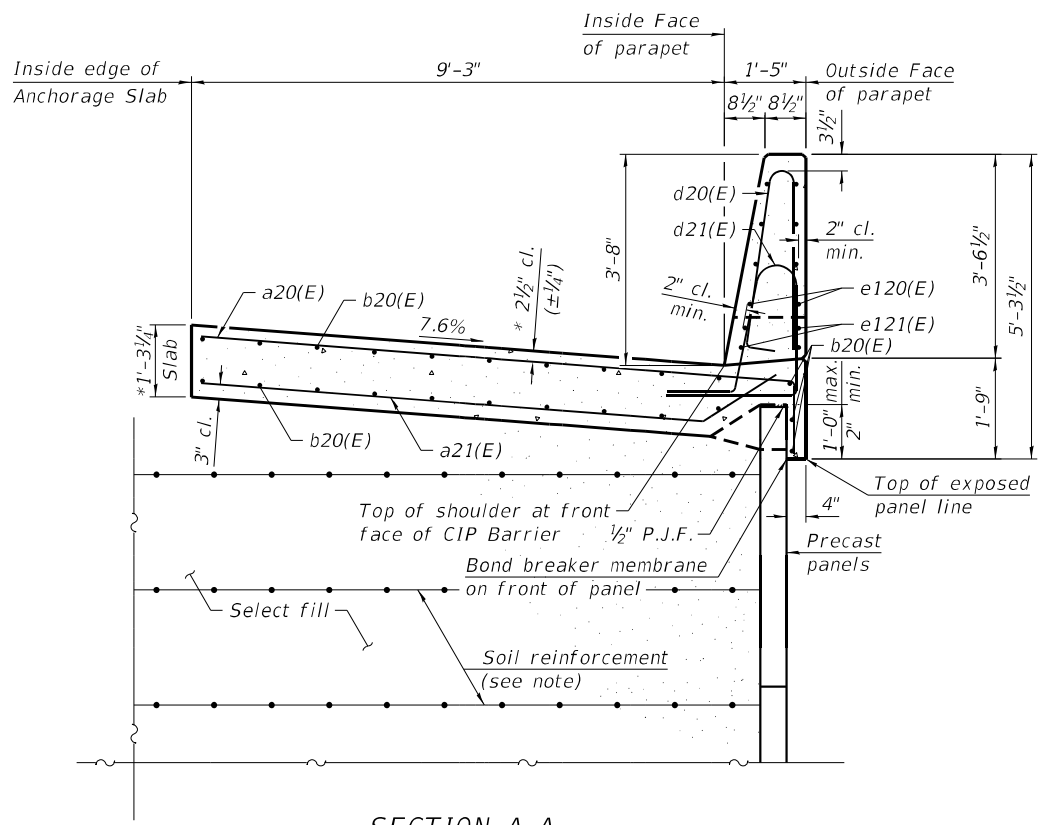
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

EAST PARAPET AND ANCHORAGE SLAB
 STRUCTURE NO. 010-1001

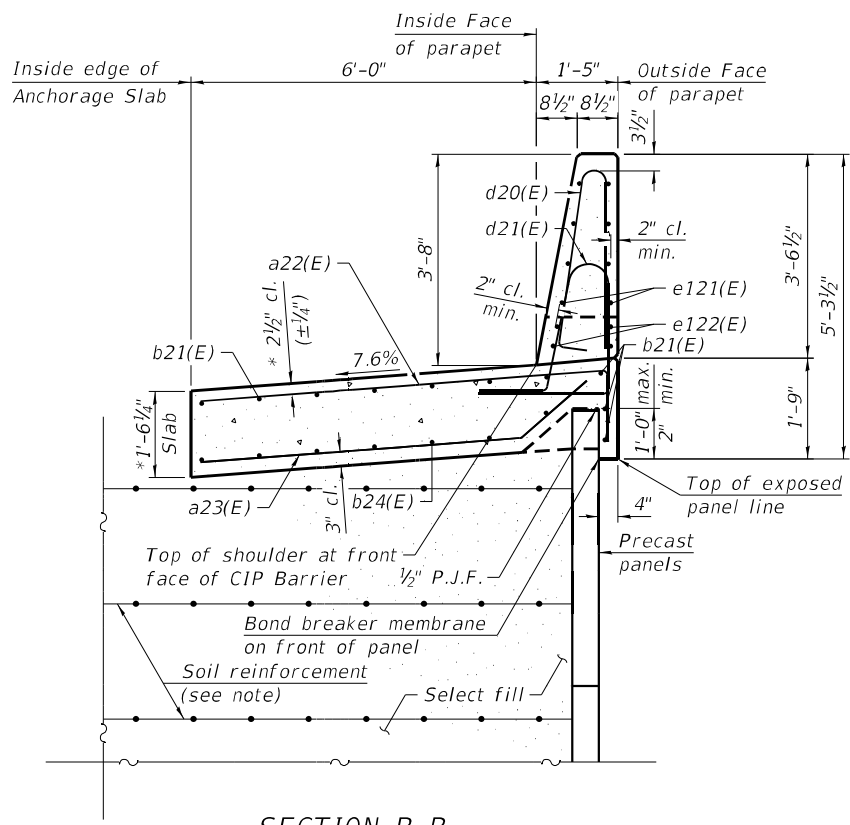
SHEET NO. S-60 OF S-106 SHEETS

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 702
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	

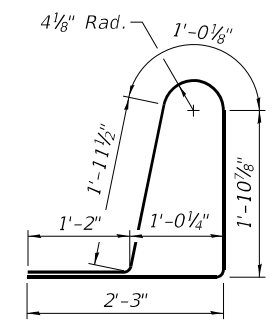
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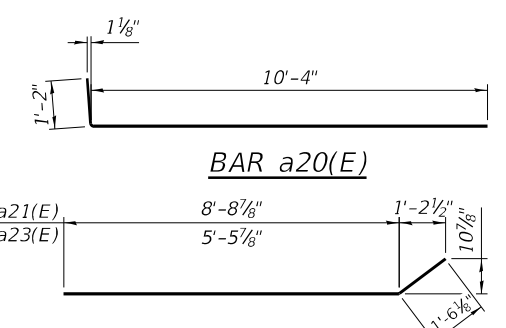
SECTION A-A



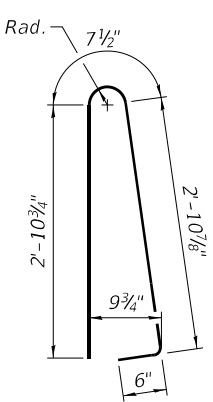
SECTION B-B



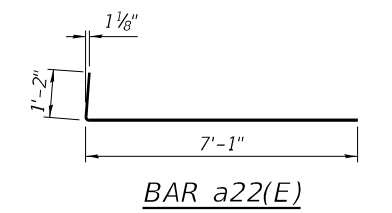
BAR d21(E)



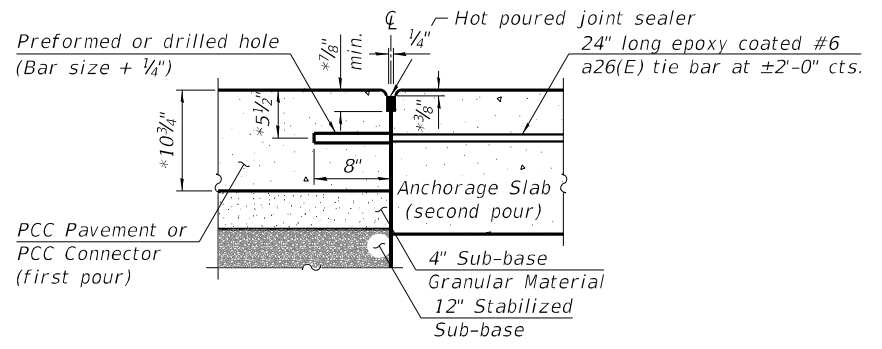
BARS a21(E) OR a23(E)



BAR d20(E)

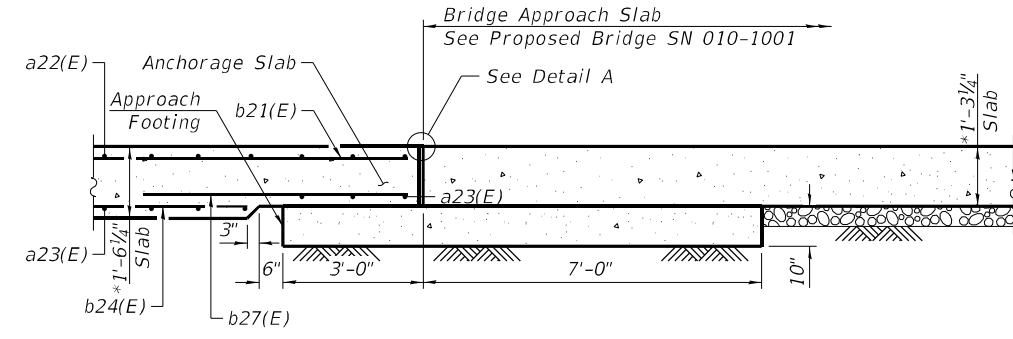


BAR a22(E)

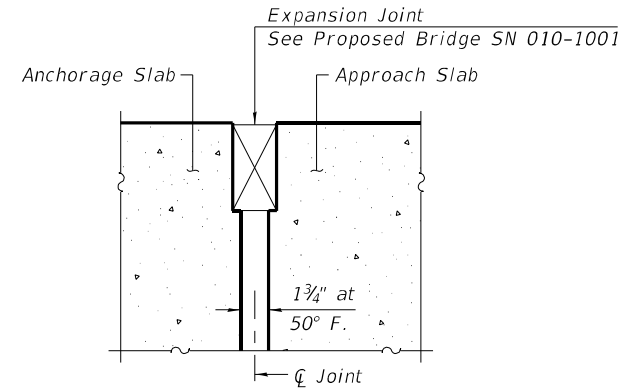


LONGITUDINAL CONSTRUCTION JOINT GROUTED-IN-PLACE TIE BAR

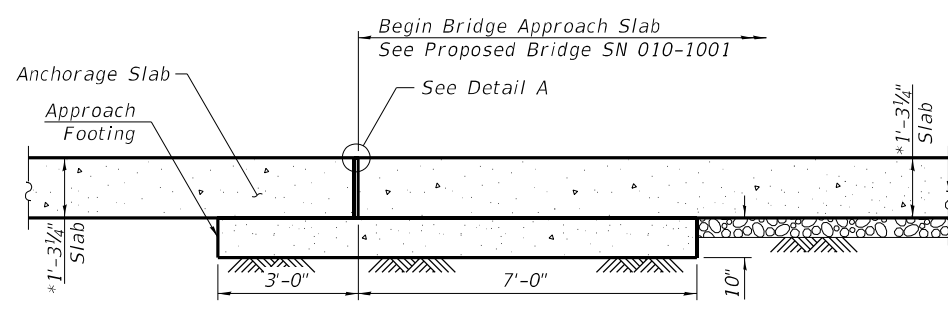
* Prior to grinding



SECTION D-D



DETAIL A



SECTION C-C

NOTES:

The M.S.E. wall supplier's internal stability design shall account for the anchorage slab's bearing pressure surcharge of 1.0 ksf and horizontal sliding force of 1.15 kips/ft. of wall. Reinforcement bar bending dimensions are out to out.

BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
a20(E)	65	#6	11'-6"	—	
a21(E)	33	#5	10'-3"	—	
a22(E)	139	#6	8'-3"	—	
a23(E)	70	#5	7'-0"	—	
a26(E)	52	#6	2'-0"	—	
b20(E)	46	#5	17'-6"	—	
b21(E)	30	#5	25'-0"	—	
b24(E)	21	#5	24'-0"	—	
b27(E)	7	#5	6'-9"	—	
d20(E)	153	#5	7'-0"	⏏	
d21(E)	153	#5	8'-4"	⏏	
e120(E)	16	#4	15'-7"	—	
e121(E)	40	#4	17'-0"	—	
e122(E)	16	#4	18'-5"	—	
Item				Unit	Total
Reinforcement Bars, Epoxy Coated				Pound	9,330
Concrete Superstructure				Cu. Yd.	59.7
Protective Coat				Sq. Yd.	129



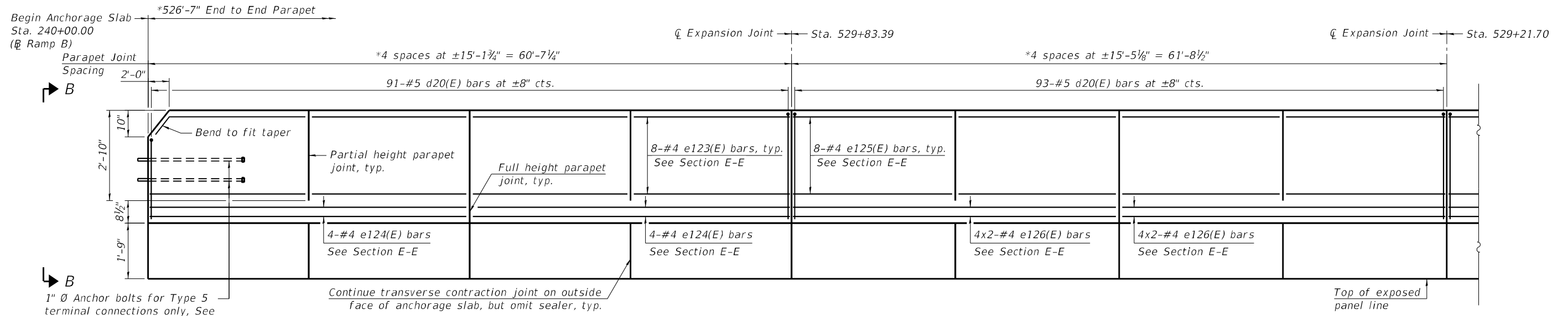
DESIGNED	—	KWB	REVISED	—
CHECKED	—	RPW	REVISED	—
DRAWN	—	LMC	REVISED	—
CHECKED	—	MDC	REVISED	—

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

EAST ANCHORAGE SLAB AND WALL DETAILS
 STRUCTURE NO. 010-1001

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 703
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	

SHEET NO. S-61 OF S-106 SHEETS



PARTIAL OUTSIDE ELEVATION OF SOUTH PARAPET - WEST APPROACH

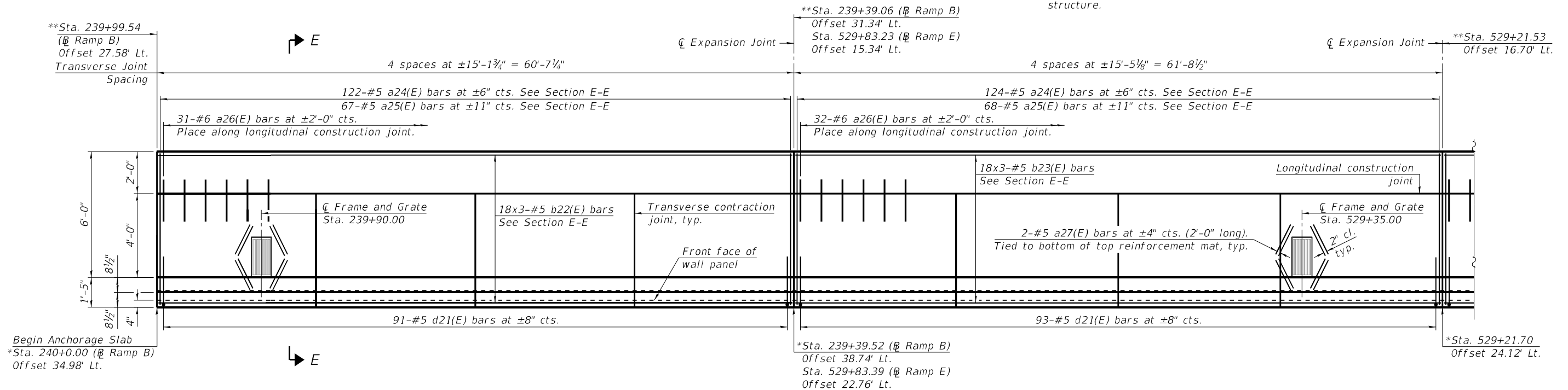
* Measured along outside face of parapet
 ** Measured along inside edge of anchorage slab

MINIMUM BAR LAP

#4 Bar = 2'-5"
 #5 Bar = 3'-0"

NOTES:

1. Place Transverse Contraction Joints and Transverse Expansion Joints perpendicular to outside face of parapet.
2. For Section E-E, see Sheet S-68 of S-106.
3. Bars indicated thus 18x3-#5 etc. indicates 18 lines of bars with 3 lengths per line.
4. For bar details and Bill of Material, see Sheet S-68 of S-106.
5. Work this sheet with Sheets S-68 and S-69 of S-106.
6. Stations and offsets are measured from Baseline Ramp E, U.N.O.
7. For Drainage Structure Details, see Drainage Plans.
8. Cut transverse and longitudinal bars as necessary to provide 2" clearance to drainage structure.



PARTIAL PLAN - SOUTH PARAPET & ANCHORAGE SLAB - WEST APPROACH

MODEL: Sheet
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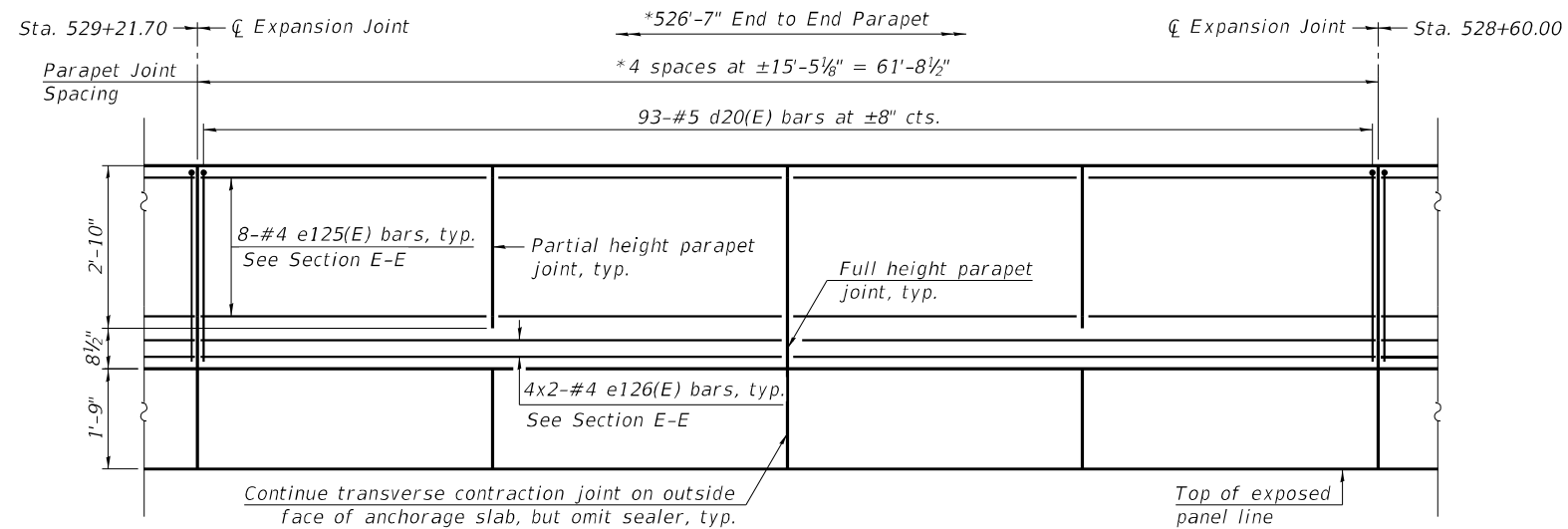
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CHECKED -	RPW	REVISED -	
DRAWN -	LMC	REVISED -	
CHECKED -	MDC	REVISED -	

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**WEST PARAPET AND ANCHORAGE SLAB (1 OF 6)
 STRUCTURE NO. 010-1001**

SHEET NO. S-62 OF S-106 SHEETS

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 704
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



PARTIAL OUTSIDE ELEVATION OF SOUTH PARAPET - WEST APPROACH

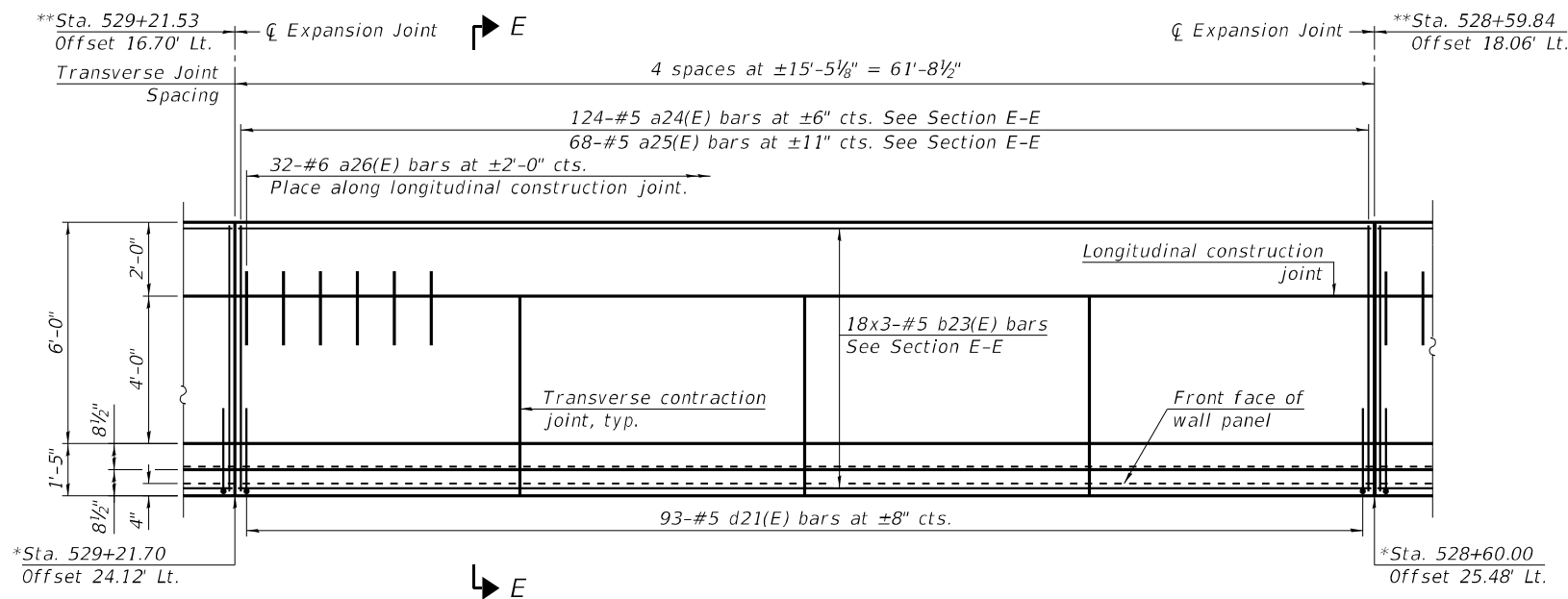
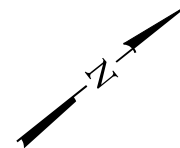
* Measured along outside face of parapet
 ** Measured along inside edge of anchorage slab

MINIMUM BAR LAP

#4 Bar = 2'-5"
 #5 Bar = 3'-0"

NOTES:

1. Place Transverse Contraction Joints and Transverse Expansion Joints perpendicular to outside face of parapet.
2. For Section E-E, see Sheet S-68 of S-106.
3. Bars indicated thus 18x3-#5 etc. indicates 18 lines of bars with 3 lengths per line.
4. For bar details and Bill of Material, see Sheet S-68 of S-106.
5. Work this sheet with Sheets S-68 and S-69 of S-106.
6. Stations and offsets are measured from Baseline Ramp E.



PARTIAL PLAN - SOUTH PARAPET & ANCHORAGE SLAB - WEST APPROACH

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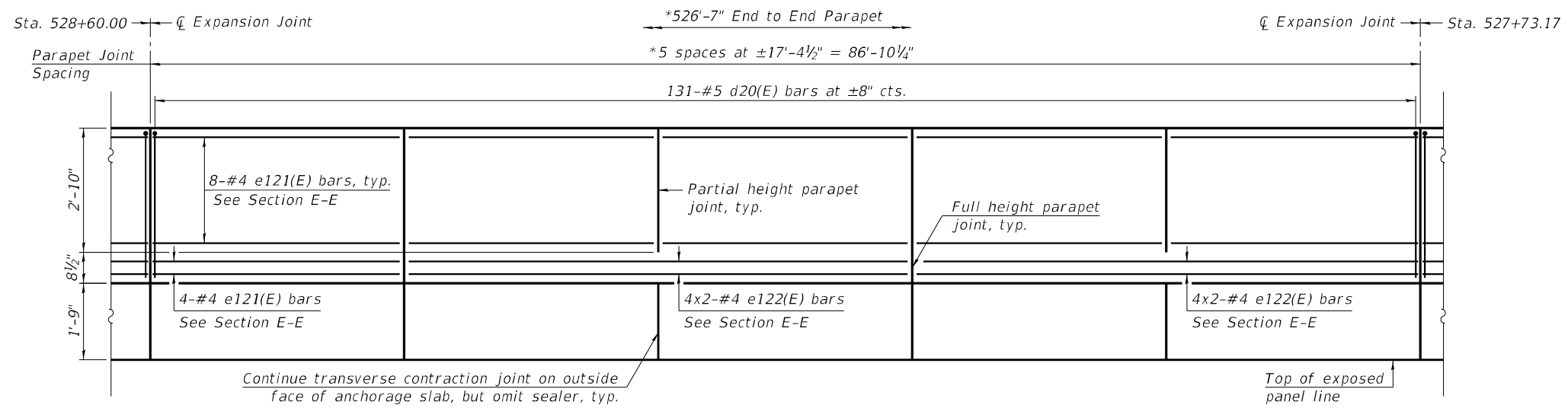
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CHECKED - RPW	REVISED -
DRAWN - LMC	REVISED -
CHECKED - MDC	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

WEST PARAPET AND ANCHORAGE SLAB (2 OF 6)
 STRUCTURE NO. 010-1001

SHEET NO. S-63 OF S-106 SHEETS

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 705
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



PARTIAL OUTSIDE ELEVATION OF SOUTH PARAPET - WEST APPROACH

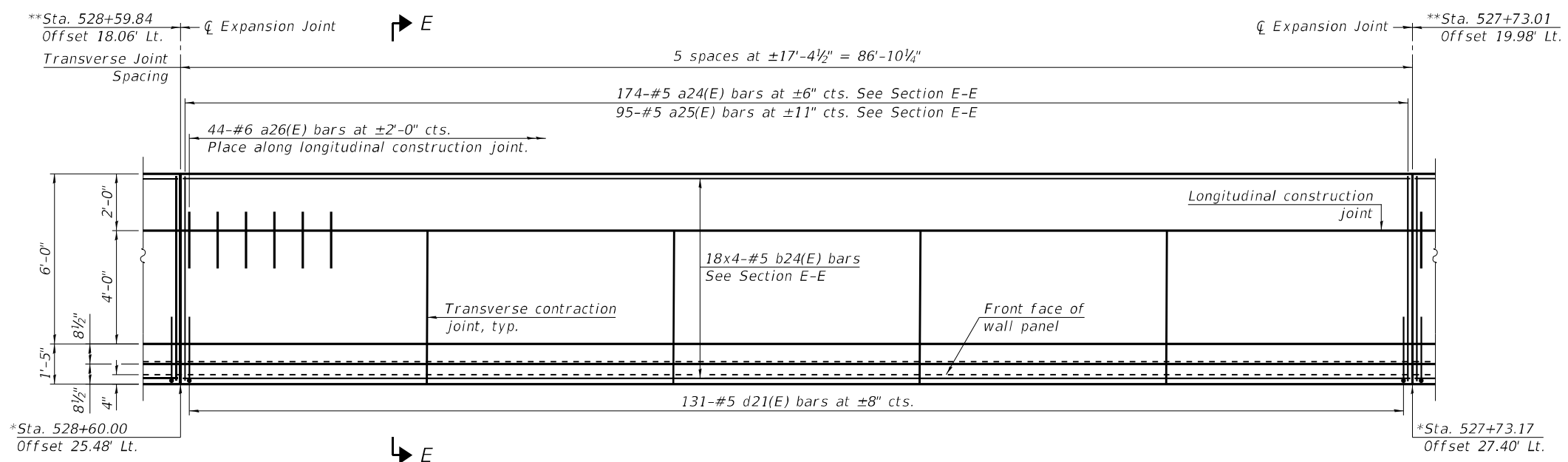
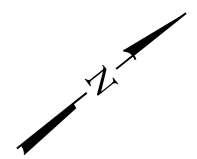
* Measured along outside face of parapet
 ** Measured along inside edge of anchorage slab

MINIMUM BAR LAP

- #4 Bar = 2'-5"
- #5 Bar = 3'-0"

NOTES:

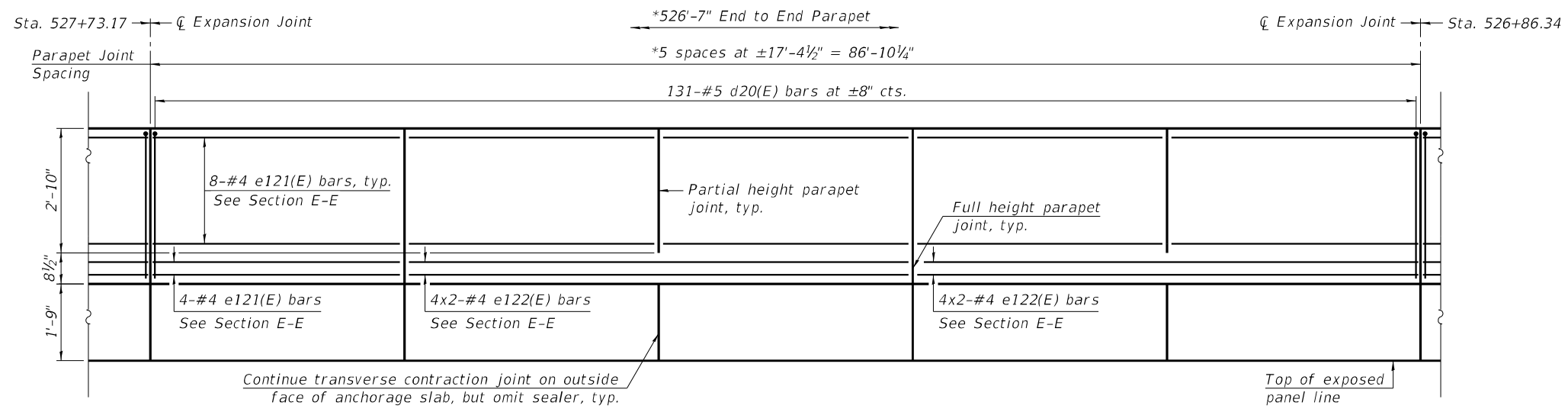
1. Place Transverse Contraction Joints and Transverse Expansion Joints perpendicular to outside face of parapet.
2. For Section E-E, see Sheet S-68 of S-106.
3. Bars indicated thus 18x4-#5 etc. indicates 18 lines of bars with 4 lengths per line.
4. For bar details and Bill of Material, see Sheet S-68 of S-106.
5. Work this sheet with Sheets S-68 and S-69 of S-106.
6. Stations and offsets are measured from Baseline Ramp E.



PARTIAL PLAN - SOUTH PARAPET & ANCHORAGE SLAB - WEST APPROACH

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 QUIGG ENGINEERING INC	DESIGNED - KWB	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	WEST PARAPET AND ANCHORAGE SLAB (3 OF 6) STRUCTURE NO. 010-1001	F.A.I. RTÉ.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	CHECKED - RPW	REVISED -			74 & 57	(10-34-1) HBK	CHAMPAIGN	1187	706
	DRAWN - LMC	REVISED -			CONTRACT NO. 70B99				
	CHECKED - MDC	REVISED -			ILLINOIS FED. AID PROJECT				
				SHEET NO. S-64 OF S-106 SHEETS					



PARTIAL OUTSIDE ELEVATION OF SOUTH PARAPET - WEST APPROACH

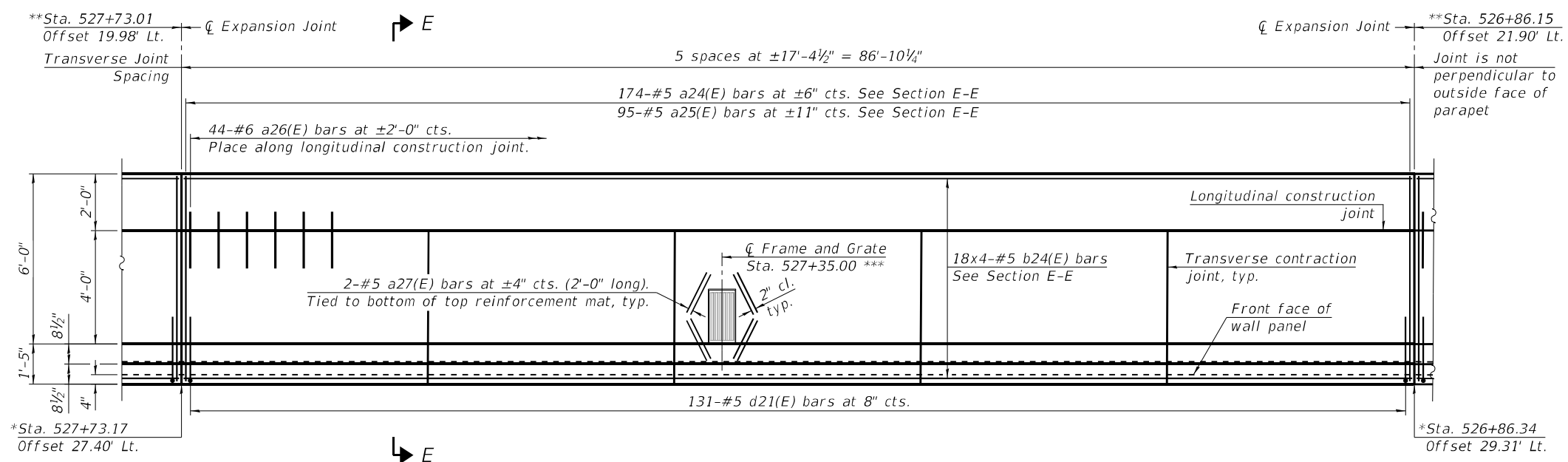
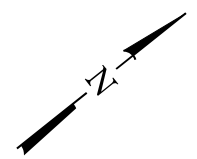
* Measured along outside face of parapet
 ** Measured along inside edge of anchorage slab

MINIMUM BAR LAP

#4 Bar = 2'-5"
 #5 Bar = 3'-0"

NOTES:

1. Place Transverse Contraction Joints and Transverse Expansion Joints perpendicular to outside face of parapet, U.N.O.
2. For Section E-E, see Sheet S-68 of S-106.
3. Bars indicated thus 18x4-#5 etc. indicates 18 lines of bars with 4 lengths per line.
4. For bar details and Bill of Material, see Sheet S-68 of S-106.
5. Work this sheet with Sheets S-68 and S-69 of S-106.
6. Stations and offsets are measured from Baseline Ramp E.
7. For Frame and Grate Details see Drainage Plans.
8. Cut transverse and longitudinal bars as necessary to provide 2" clearance to drainage structure.



PARTIAL PLAN - SOUTH PARAPET & ANCHORAGE SLAB - WEST APPROACH

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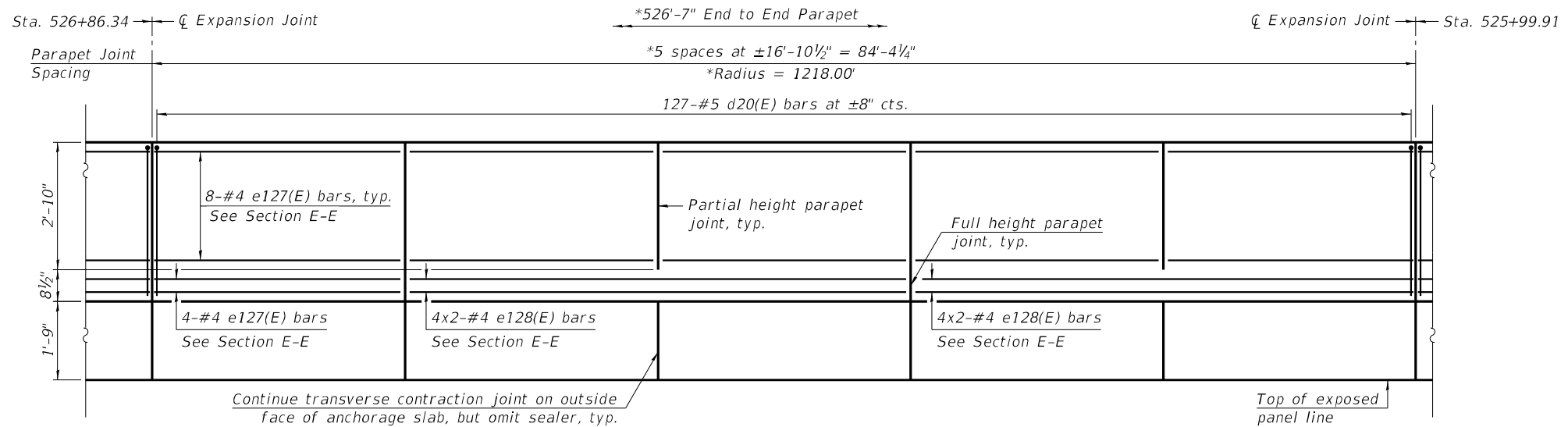
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DRAWN - LMC	REVISED -
CHECKED - MDC	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

WEST PARAPET AND ANCHORAGE SLAB (4 OF 6)
 STRUCTURE NO. 010-1001

SHEET NO. S-65 OF S-106 SHEETS

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 707
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



PARTIAL OUTSIDE ELEVATION OF SOUTH PARAPET - WEST APPROACH

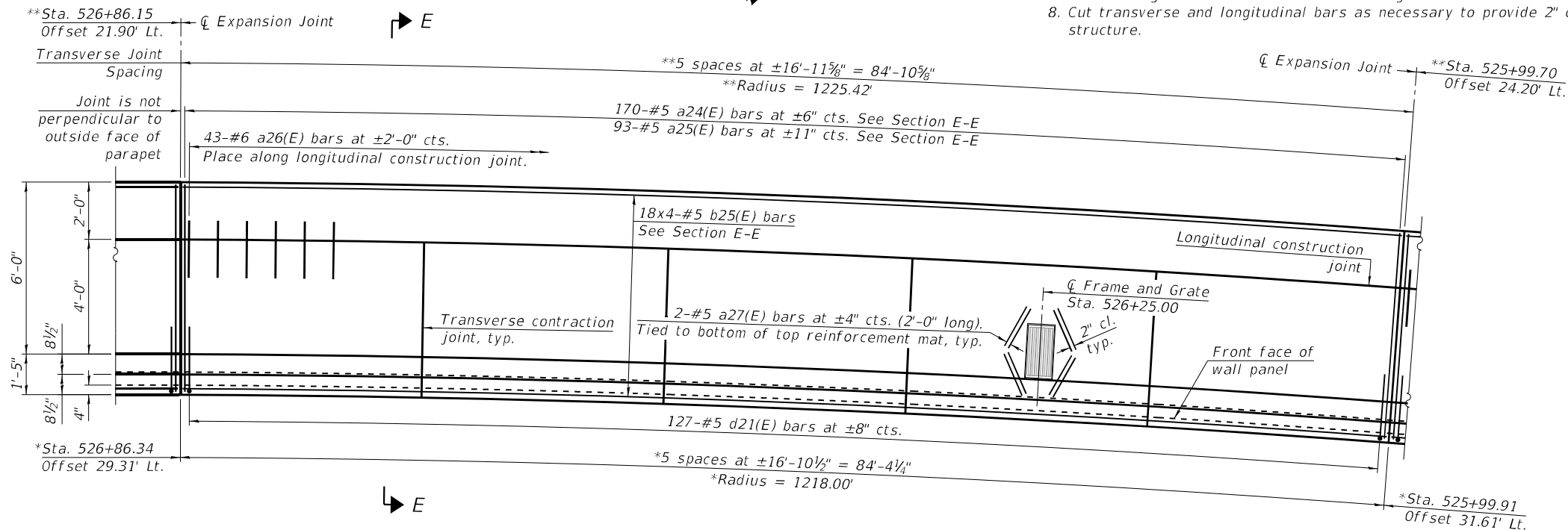
* Measured along outside face of parapet
 ** Measured along inside edge of anchorage slab

MINIMUM BAR LAP

#4 Bar = 2'-5"
 #5 Bar = 3'-0"

NOTES:

1. Place Transverse Contraction Joints and Transverse Expansion Joints perpendicular to outside face of parapet, U.N.O.
2. For Section E-E, see Sheet S-68 of S-106.
3. Bars indicated thus 18x4-#5 etc. indicates 18 lines of bars with 4 lengths per line.
4. For bar details and Bill of Material, see Sheet S-68 of S-106.
5. Work this sheet with Sheets S-68 and S-69 of S-106.
6. Stations and offsets are measured from Baseline Ramp E.
7. For Drainage Structure Details see Drainage Plans.
8. Cut transverse and longitudinal bars as necessary to provide 2" clearance to drainage structure.



PARTIAL PLAN - SOUTH PARAPET & ANCHORAGE SLAB - WEST APPROACH

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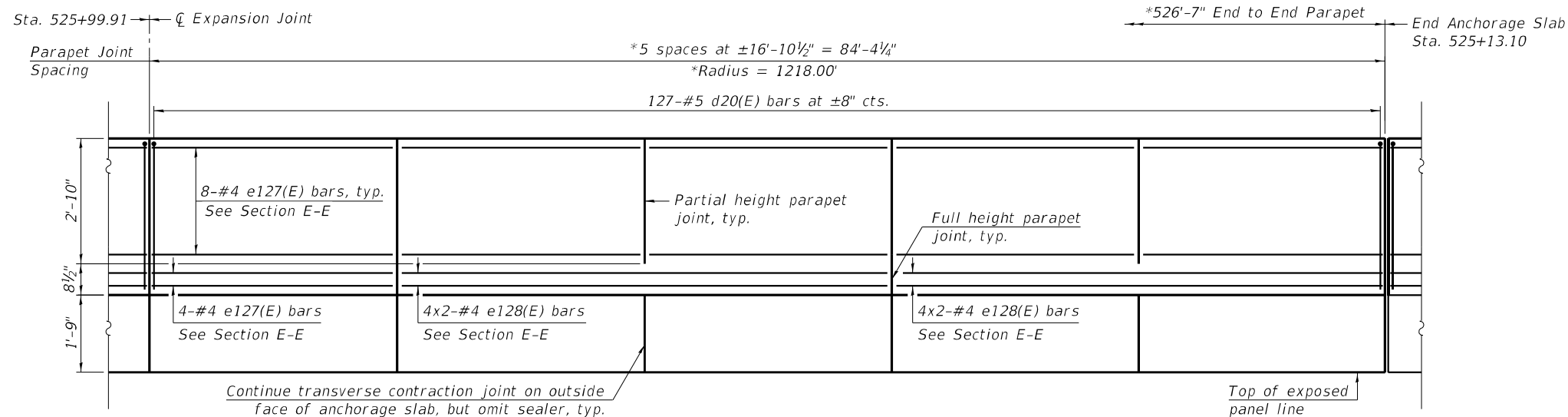
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DRAWN - LMC	REVISED -
CHECKED - MDC	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

WEST PARAPET AND ANCHORAGE SLAB (5 OF 6)
 STRUCTURE NO. 010-1001

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 708
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	

SHEET NO. S-66 OF S-106 SHEETS



PARTIAL OUTSIDE ELEVATION OF SOUTH PARAPET - WEST APPROACH

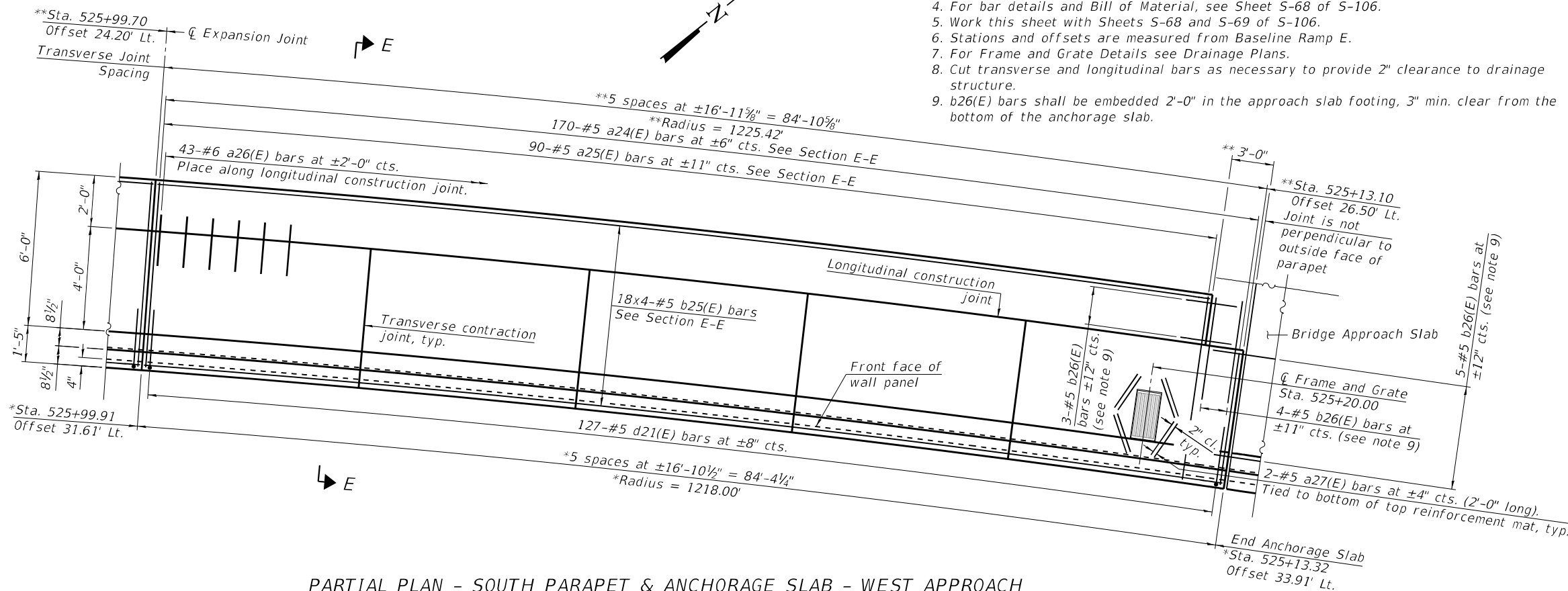
* Measured along outside face of parapet
 ** Measured along inside edge of anchorage slab

MINIMUM BAR LAP

#4 Bar = 2'-5"
 #5 Bar = 3'-0"

NOTES:

1. Place Transverse Contraction Joints and Transverse Expansion Joints perpendicular to outside face of parapet, U.N.O.
2. For Section E-E, see Sheet S-68 of S-106.
3. Bars indicated thus 18x4-#5 etc. indicates 18 lines of bars with 4 lengths per line.
4. For bar details and Bill of Material, see Sheet S-68 of S-106.
5. Work this sheet with Sheets S-68 and S-69 of S-106.
6. Stations and offsets are measured from Baseline Ramp E.
7. For Frame and Grate Details see Drainage Plans.
8. Cut transverse and longitudinal bars as necessary to provide 2" clearance to drainage structure.
9. b26(E) bars shall be embedded 2'-0" in the approach slab footing, 3" min. clear from the bottom of the anchorage slab.



PARTIAL PLAN - SOUTH PARAPET & ANCHORAGE SLAB - WEST APPROACH

MODEL: Sheet
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DESIGNED - KWB	REVISED -
CHECKED - RPW	REVISED -
DRAWN - LMC	REVISED -
CHECKED - MDC	REVISED -

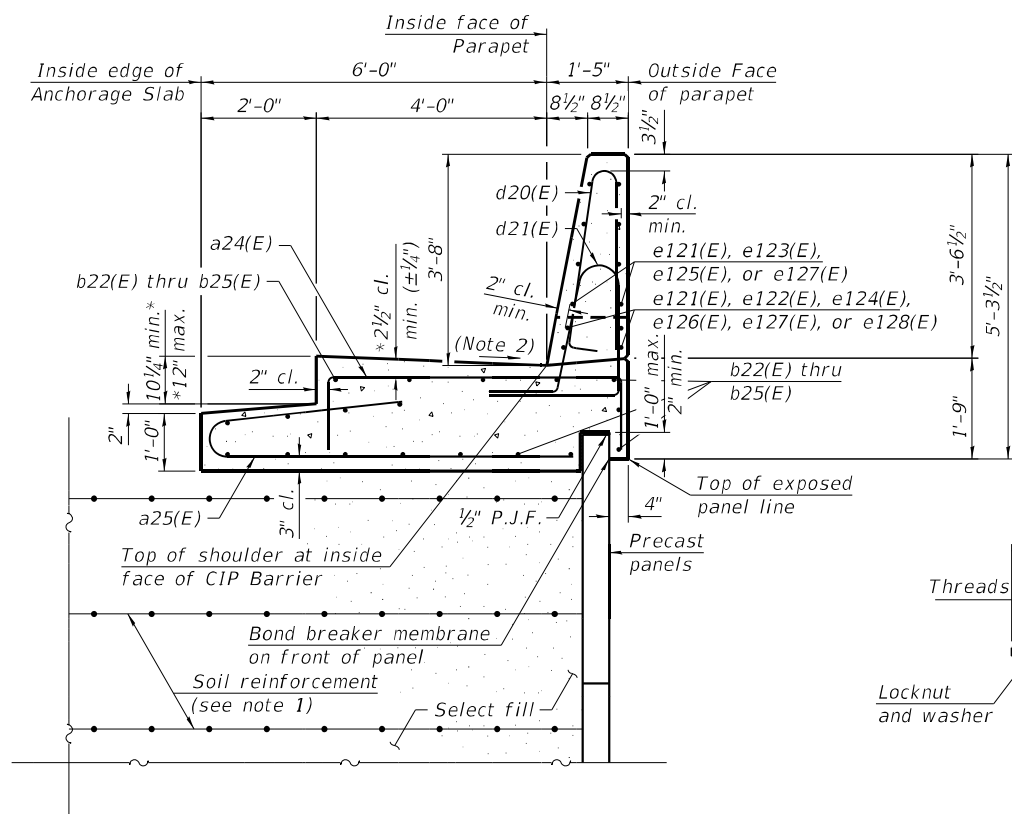
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

WEST PARAPET AND ANCHORAGE SLAB (6 OF 6)
 STRUCTURE NO. 010-1001

SHEET NO. S-67 OF S-106 SHEETS

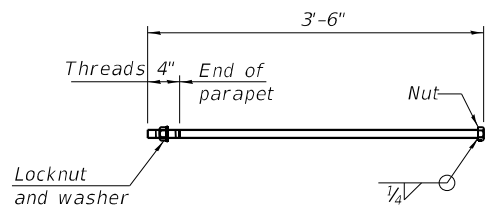
F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 709
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	

MODEL: Sheet
 FILE NAME: p:\v\cmengr-pw-bentley.com\cmt-projects\Documents\Projects\DOT115066-011\Draw\Structures\CADD_Sheets\RAMPE-70B99-068-West Moment Slab Sections and Details-QEIdgn
 5/3/2021 1:49:25 PM



SECTION E-E

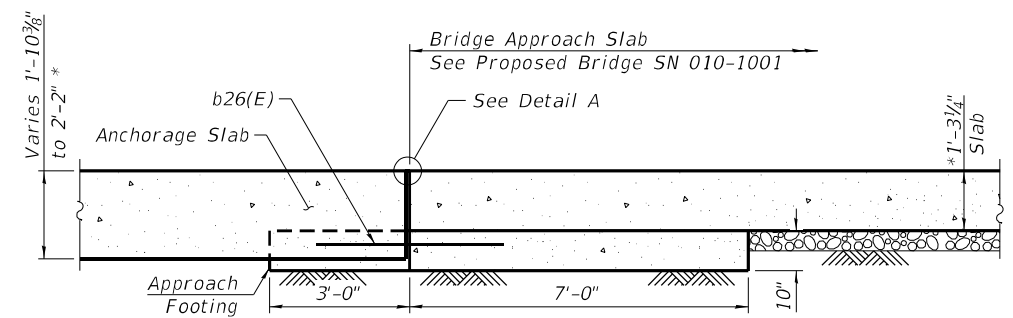
* Prior to grinding



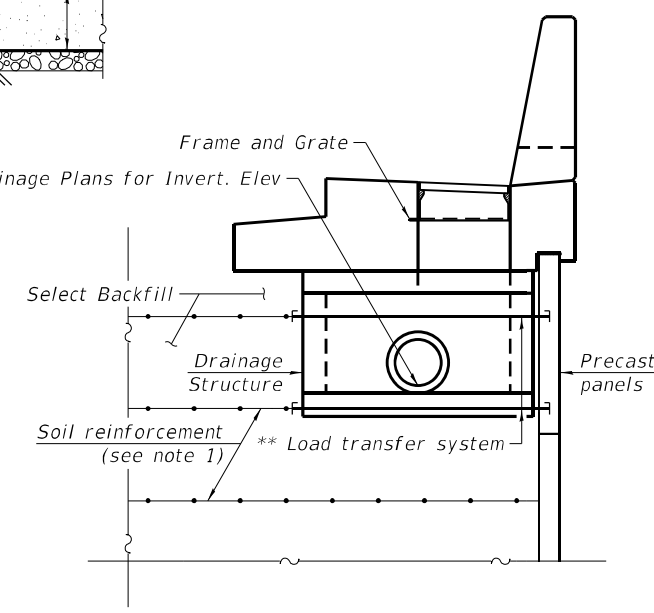
***** 1" Ø ANCHOR BOLT**
 (Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications)
 *** Cost included with Concrete Superstructure

NOTES:

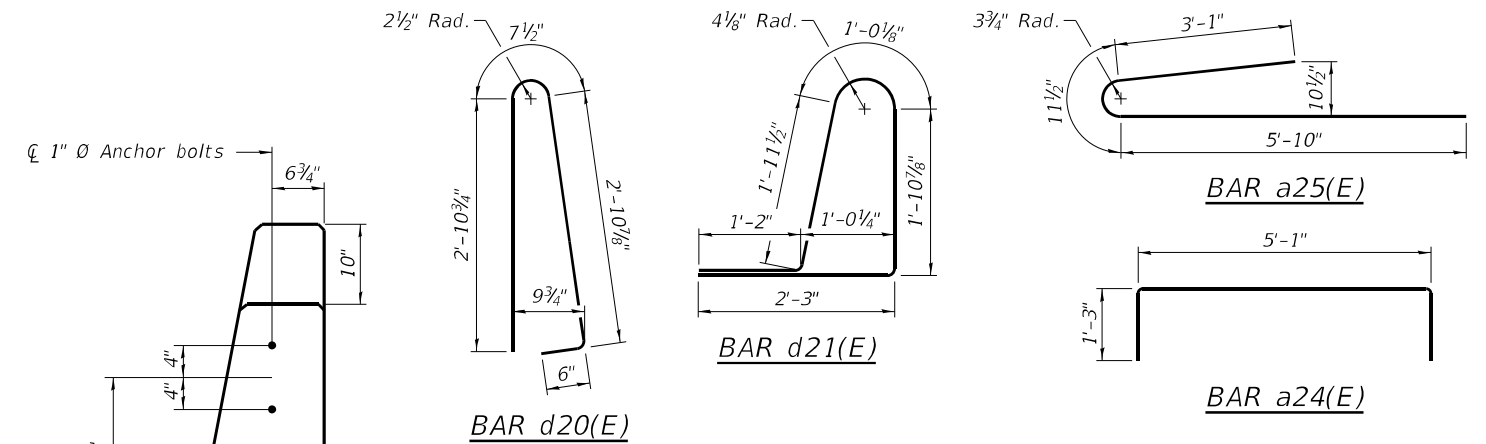
- The M.S.E. wall supplier's internal stability design shall account for the anchorage slab's bearing pressure surcharge of 1.0 ksf and horizontal sliding force of 1.15 kips/ft. of wall.
- Varies 7.6% to 4.0%
- Reinforcement bar bending dimensions are out to out.



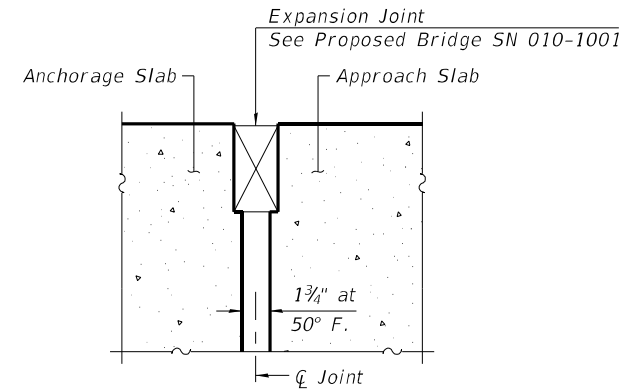
SECTION AT ANCHORAGE SLAB AND BRIDGE APPROACH SLAB



ANCHORAGE SLAB INLET SECTION

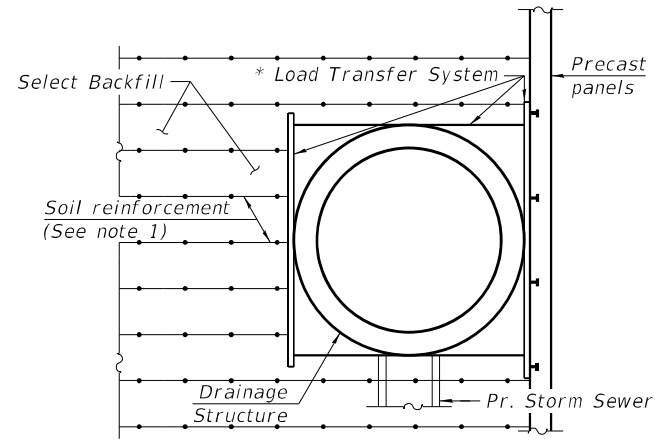


VIEW B-B

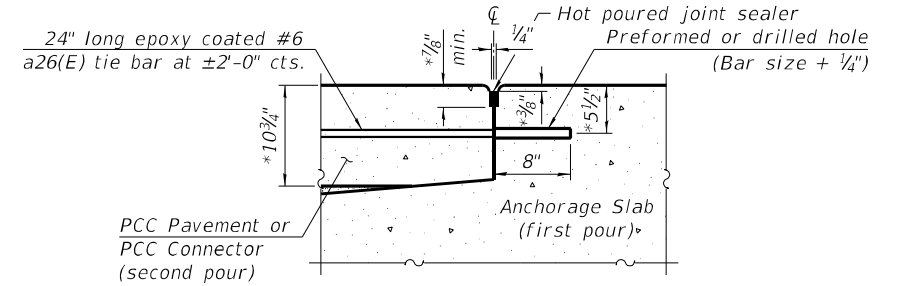


DETAIL A

** M.S.E. supplier to design load transfer system to accommodate concrete pipe and drainage structure.



ANCHORAGE SLAB INLET PLAN



LONGITUDINAL CONSTRUCTION JOINT GROUTED-IN-PLACE TIE BAR

BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
a24(E)	1,058	#5	7'-7"	┌───┐	
a25(E)	576	#5	9'-11"	┌───┐	
a26(E)	269	#6	2'-0"	───	
a27(E)	40	#5	2'-0"	───	
b22(E)	54	#5	22'-2"	───	
b23(E)	108	#5	22'-6"	───	
b24(E)	144	#5	23'-11"	───	
b25(E)	144	#5	23'-5"	───	
b26(E)	12	#5	4'-0"	───	
d20(E)	793	#5	7'-0"	┌───┐	
d21(E)	793	#5	8'-4"	┌───┐	
e121(E)	88	#4	17'-0"	───	
e122(E)	32	#4	18'-5"	───	
e123(E)	32	#4	14'-10"	───	
e124(E)	8	#4	30'-0"	───	
e125(E)	64	#4	15'-1"	───	
e126(E)	32	#4	16'-6"	───	
e127(E)	88	#4	16'-7"	───	
e128(E)	32	#4	17'-11"	───	
Item				Unit	Total
Reinforcement Bars, Epoxy Coated				Pound	43,070
Concrete Superstructure				Cu. Yd.	318.0
Protective Coat				Sq. Yd.	494



DESIGNED - KWB
 CHECKED - RPW
 DRAWN - LMC
 CHECKED - MDC

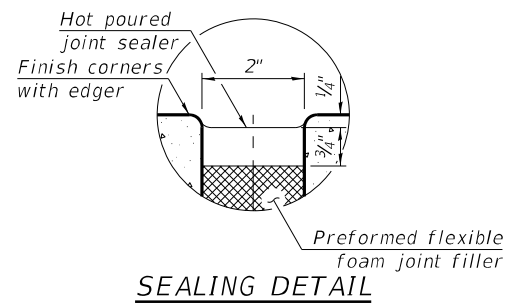
REVISED -
 REVISED -
 REVISED -
 REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

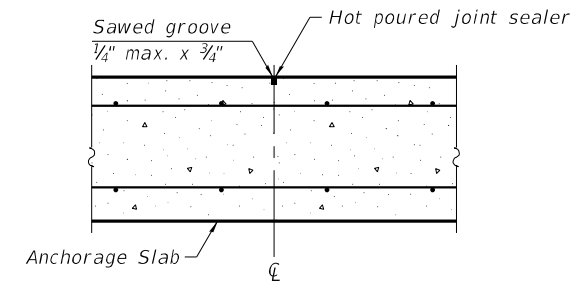
WEST ANCHORAGE SLAB AND WALL DETAILS
STRUCTURE NO. 010-1001

SHEET NO. 5-68 OF S-106 SHEETS

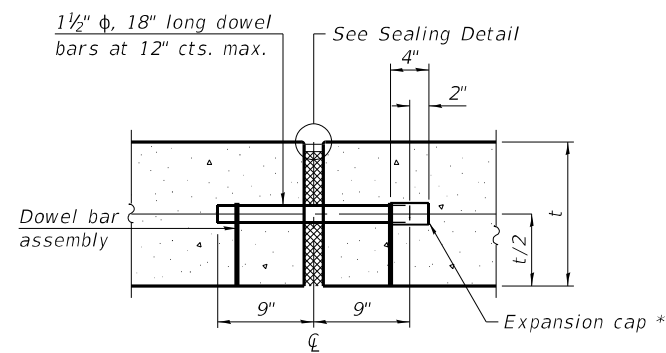
F.A.I. SECTION COUNTY TOTAL SHEETS SHEET NO.
 74 & 57 (10-34-1) HBK CHAMPAIGN 1187 710
 CONTRACT NO. 70B99
 ILLINOIS FED. AID PROJECT



SEALING DETAIL



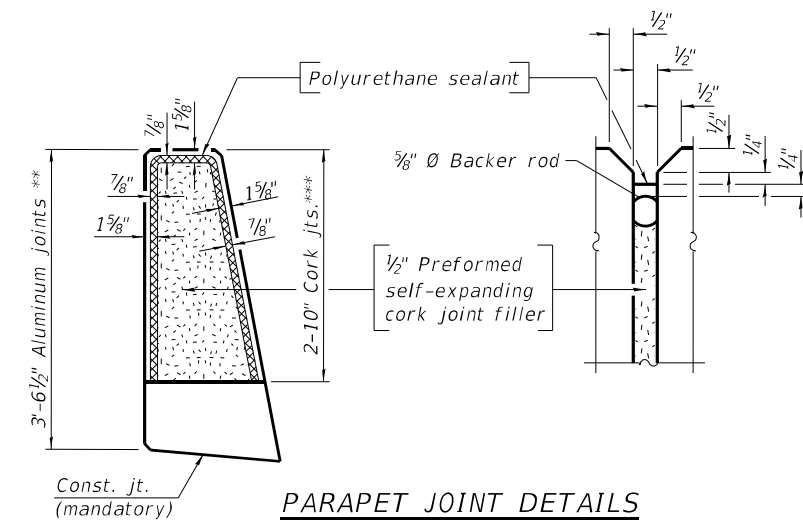
TRANSVERSE CONTRACTION JOINT



ANCHORAGE SLAB EXPANSION JOINT

Expansion joint and dowel bars included in the cost of Concrete Superstructure

* Expansion caps shall be installed on the exposed end of each dowel bar once the header has been removed and the joint filler material has been installed.



PARAPET JOINT DETAILS

** Full height joint only
*** Partial height joint only

MODEL: Sheet
FILE NAME: p:\v\cmengr-pw-bentley.com\cmt-projects\Documents\Projects\DOT15066-011\Draw\Structures\CADD_Sheets\RAWP_E\RAMPE-70899-069-East and West Moment Slabs Shared Details-QEIdgn



QUIGG ENGINEERING INC

DESIGNED	-	KWB	REVISED	-
CHECKED	-	RPW	REVISED	-
DRAWN	-	LMC	REVISED	-
CHECKED	-	MDC	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ANCHORAGE SLAB AND MSE WALL - DETAILS
STRUCTURE NO. 010-1001

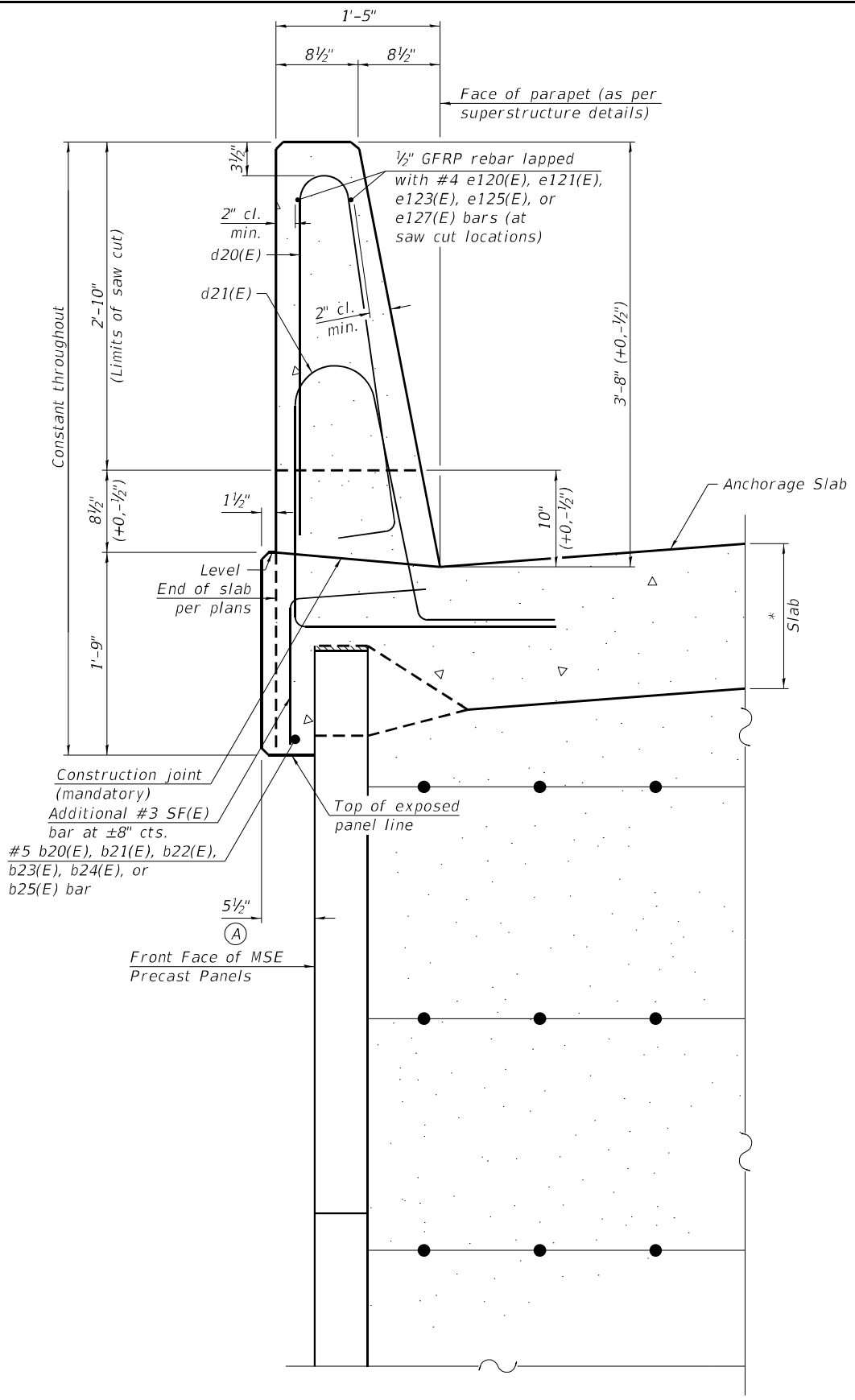
SHEET NO. S-69 OF S-106 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74 & 57	(10-34-1) HBK	CHAMPAIGN	1187	711
CONTRACT NO. 70B99				

ILLINOIS FED. AID PROJECT

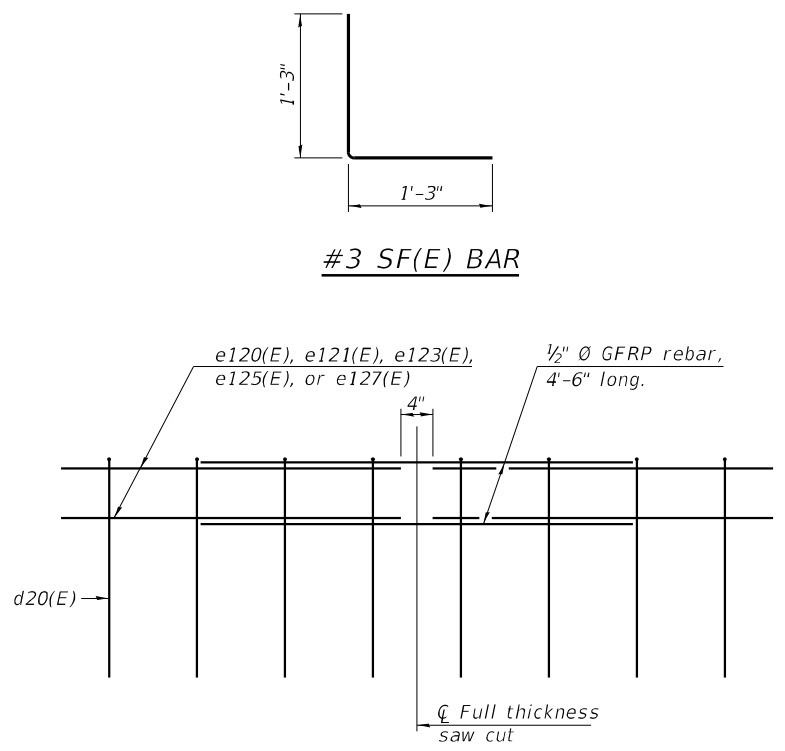
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 5/3/2021 1:49:47 PM



**44" CONSTANT-SLOPE
 PARAPET SECTION**
 (Showing dimensions, d20(E), d21(E) and 1/2" Ø GFRP rebar)
 (Showing reinforcement clearances for slip forming and additional reinforcement)

*See Anchorage Slab Details.



GFRP REBAR STIFFENING DETAIL
 (Place as shown in parapet section at each parapet joint location.)

GENERAL NOTES

All dimensions shall remain the same as shown on superstructure details, except dimension A which is to be revised as shown. Additional concrete needed to revise dimension A = 0.01 cu. yds./ft.
 Place full depth aluminum sheets as shown on superstructure details.
 Replace all cork joint filler locations with a full thickness saw cut.



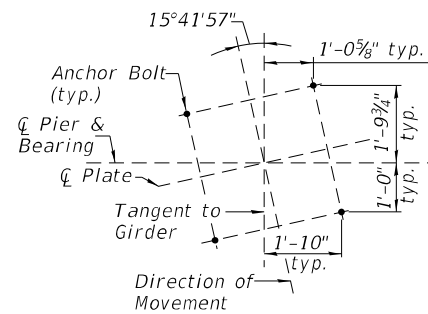
DESIGNED	- KWB	REVISED	-
CHECKED	- RPW	REVISED	-
DRAWN	- LMC	REVISED	-
CHECKED	- MDC	REVISED	-

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

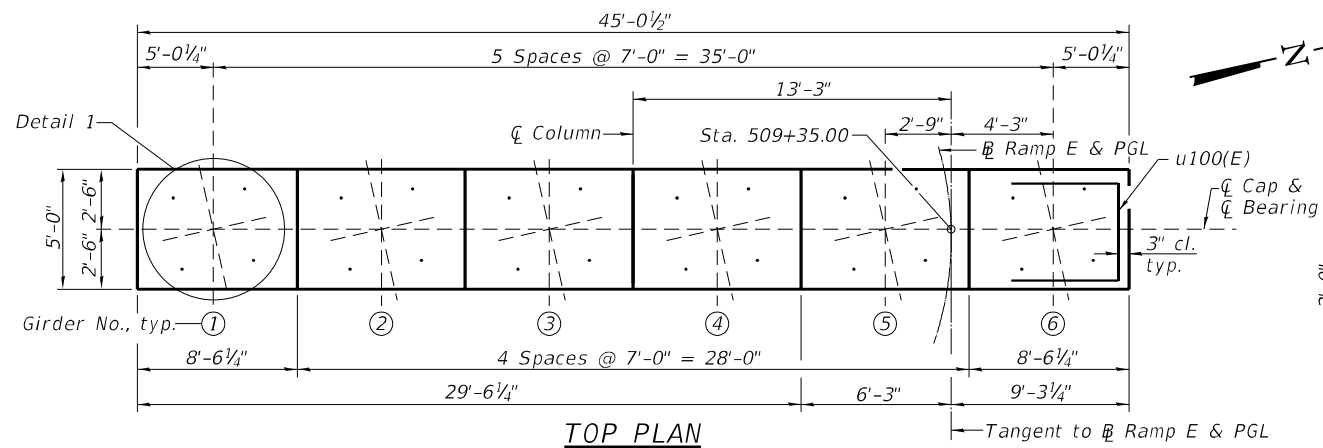
**CONCRETE PARAPET SLIPFORMING OPTION
 STRUCTURE NO. 010-1001**

SHEET NO. S-70 OF S-106 SHEETS

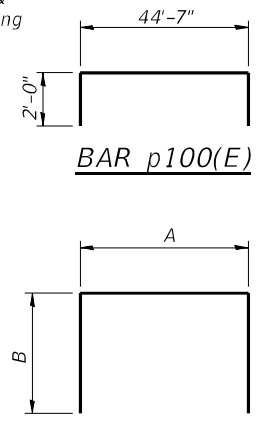
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74 & 57	(10-34-1) HBK	CHAMPAIGN	1187	712
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



DETAIL 1
Bearing Orientation
(Typ. at Each Girder)



TOP PLAN



BAR p100(E)

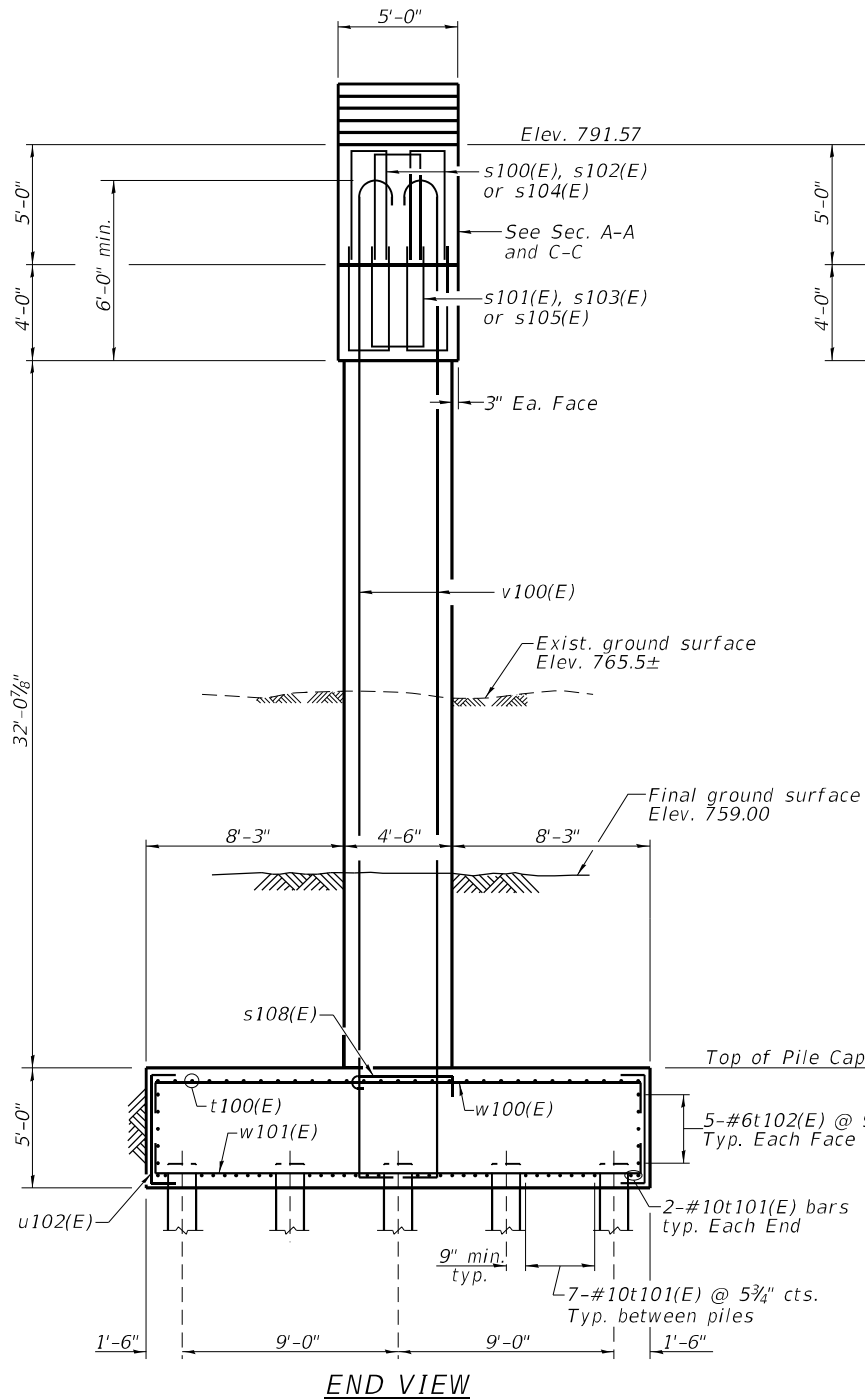
BARS

A & B DIMENSIONS

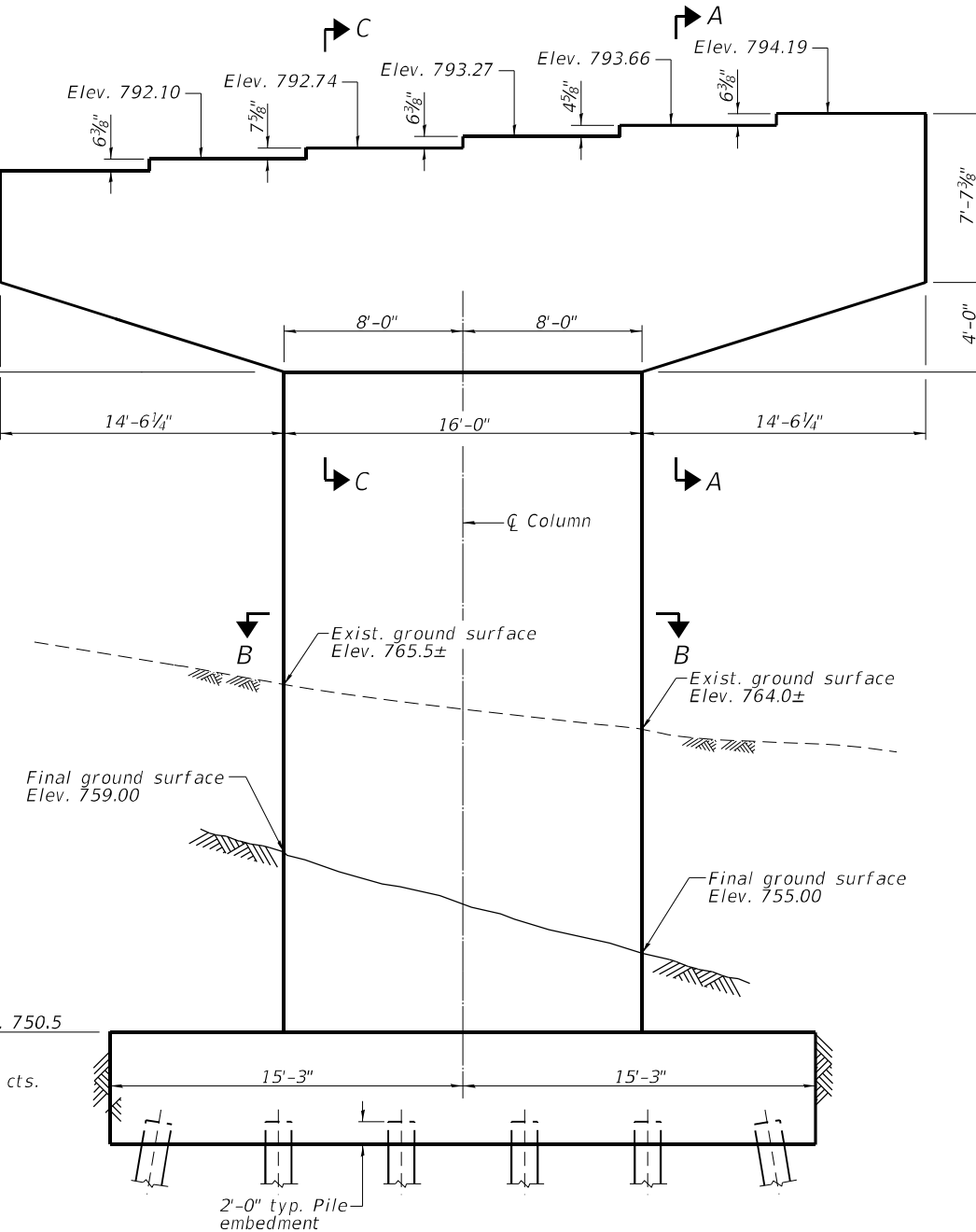
Bar	A	B
s100(E)	2'-0"	4'-4"
s101(E)	2'-6"	4'-4"
s102(E)	2'-0"	5'-6"
s103(E)	2'-6"	5'-6"
s104(E)	2'-0"	6'-7"
s105(E)	2'-6"	6'-7"
s106(E)	3'-1"	6'-7"
t100(E)	30'-2"	1'-0"
t101(E)	30'-2"	1'-10"
t102(E)	29'-11"	1'-0"
u100(E)	4'-6"	3'-10"
u101(E)	4'-8"	3'-10"
u102(E)	4'-7"	1'-0"
u103(E)	4'-2"	3'-2"
w100(E)	20'-8"	1'-0"
w101(E)	20'-8"	1'-10"
w102(E)	20'-5"	1'-0"

BILL OF MATERIAL

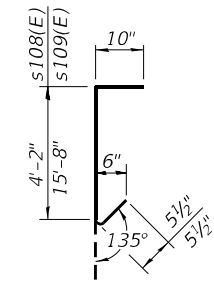
Bar	No.	Size	Length	Shape
h100(E)	12	#6	44'-8"	▬
h101(E)	2	#6	43'-2"	▬
h102(E)	2	#6	36'-0"	▬
h103(E)	2	#6	28'-9"	▬
h104(E)	2	#6	21'-6"	▬
h105(E)	24	#6	6'-8"	▬
h106(E)	12	#6	8'-2"	▬
h107(E)	196	#5	15'-8"	▬
p100(E)	9	#11	48'-7"	▬
p101(E)	9	#11	44'-8"	▬
p102(E)	12	#8	26'-7"	▬
s100(E)	20	#5	10'-8"	▬
s101(E)	10	#5	11'-2"	▬
s102(E)	64	#5	13'-0"	▬
s103(E)	32	#5	13'-6"	▬
s104(E)	132	#5	15'-2"	▬
s105(E)	66	#5	15'-8"	▬
s106(E)	56	#5	16'-3"	▬
s108(E)	1068	#5	5'-6"	▬
s109(E)	178	#5	17'-0"	▬
t100(E)	29	#6	32'-2"	▬
t101(E)	32	#10	33'-10"	▬
t102(E)	10	#6	31'-11"	▬
u100(E)	16	#6	12'-2"	▬
u101(E)	45	#6	12'-4"	▬
u102(E)	106	#6	6'-7"	▬
u103(E)	196	#5	10'-6"	▬
v100(E)	54	#11	46'-5"	▬
w100(E)	42	#6	22'-8"	▬
w101(E)	54	#10	24'-4"	▬
w102(E)	10	#6	22'-5"	▬
Structure Excavation	Cu. Yd.		631	
Concrete Structures	Cu. Yd.		279.7	
Reinforcement Bars, Epoxy Coated	Pound		56,750	
Furnishing Metal Shell Piles 16"x.312"	Foot		1,914	
Driving Piles	Foot		1,914	
Test Pile Metal Shell	Each		1	
Pile Shoes	Each		30	



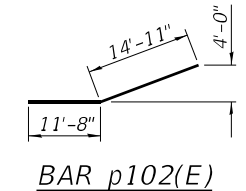
END VIEW



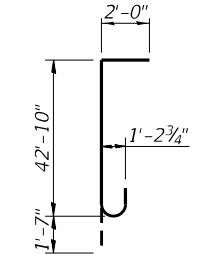
ELEVATION
(Looking Upstation)



BAR s108(E) & s109(E)



BAR p102(E)



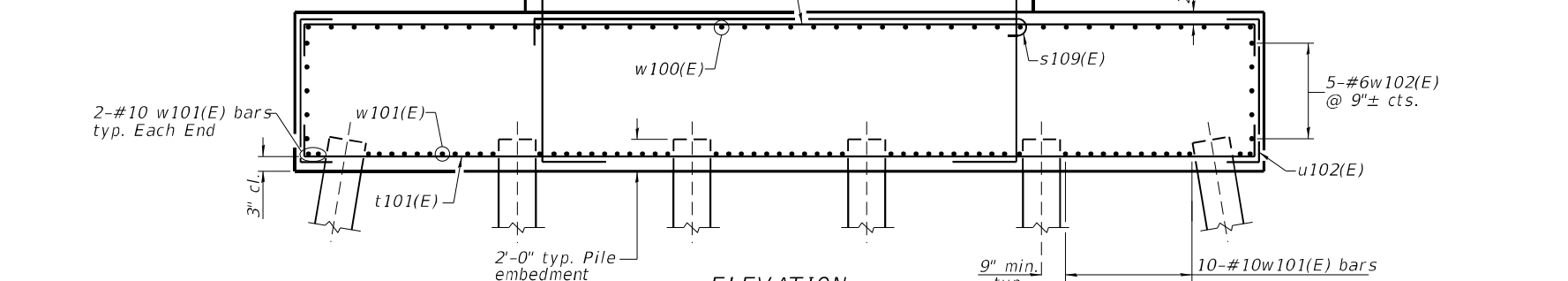
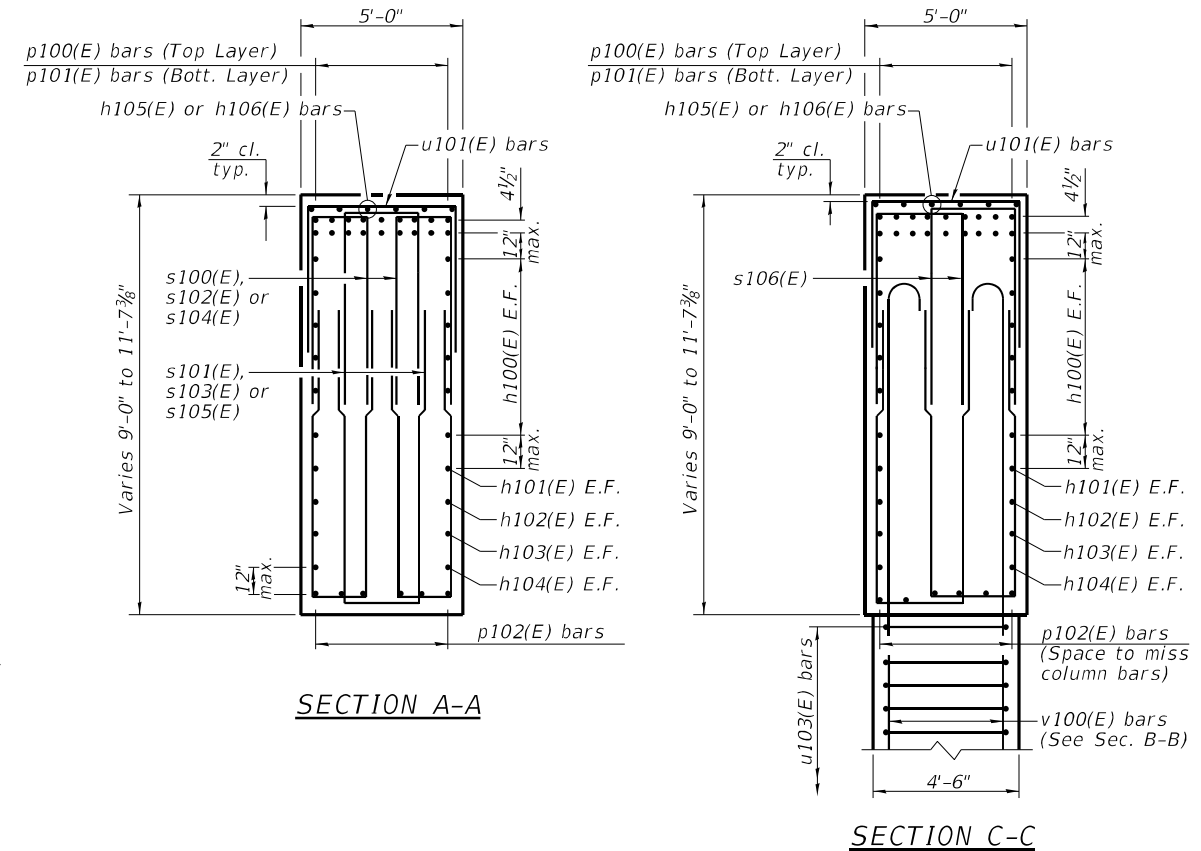
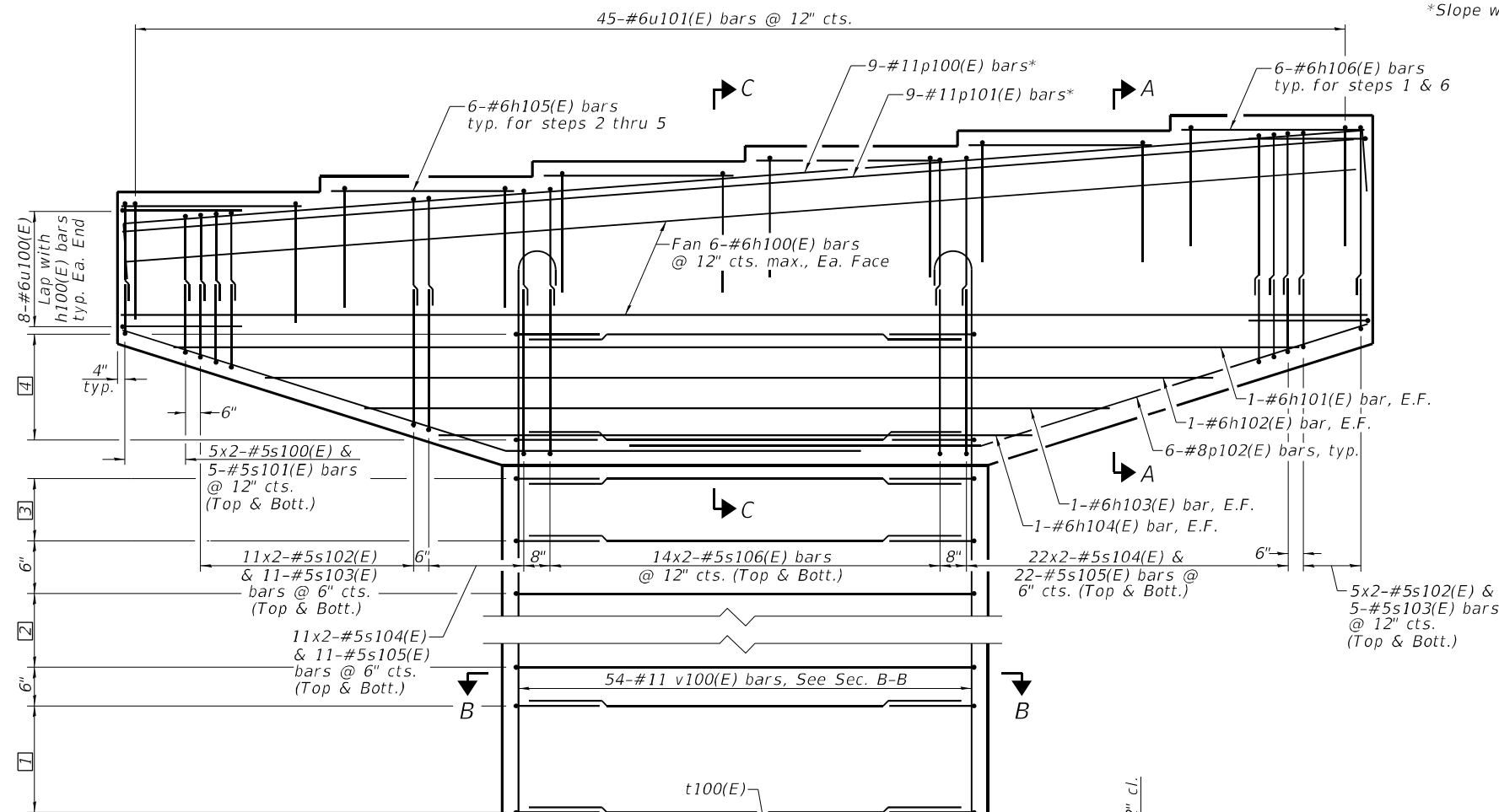
BAR v100(E)

MIN. LAP LENGTH

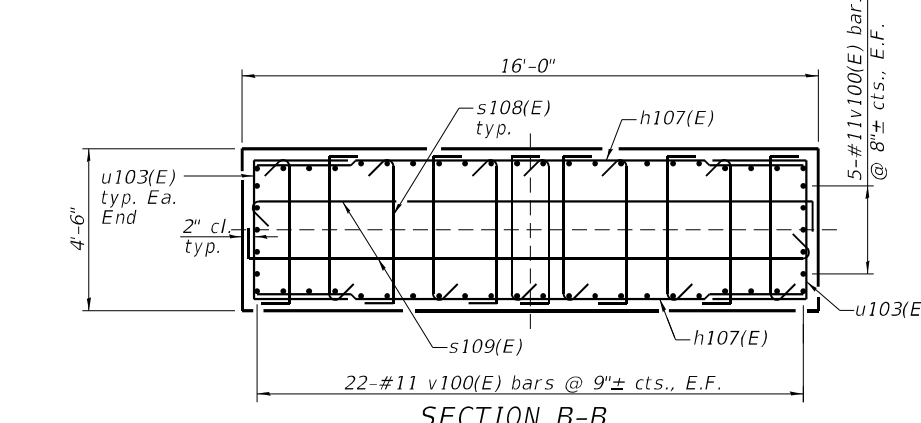
#5 bars: 3'-2"
#6 bars: 3'-10"

NOTES:

1. Pour steps monolithically with cap.
2. For Anchor Bolts Details see sheet S-46
3. See sheet S-72 of S-106 for Sections A-A, B-B & C-C
3. Reinforcement bar bending dimensions are out to out.



- 1 23 Sets - 2-#5h107(E) bars spliced with 2-#5u103(E), 12-#5s108(E) & 2-#5s109(E) at 3" cts.
- 2 42 Sets - 2-#5h107(E) bars spliced with 2-#5u103(E), 12-#5s108(E) & 2-#5s109(E) bars at 6" cts.
- 3 23 Sets - 2-#5h107(E) bars spliced with 2-#5u103(E), 12-#5s108(E), & 2-#5s109(E) at 3" cts.
- 4 10 Sets - 2-#5h107(E) bars spliced with 2-#5u103(E) at 3" cts.

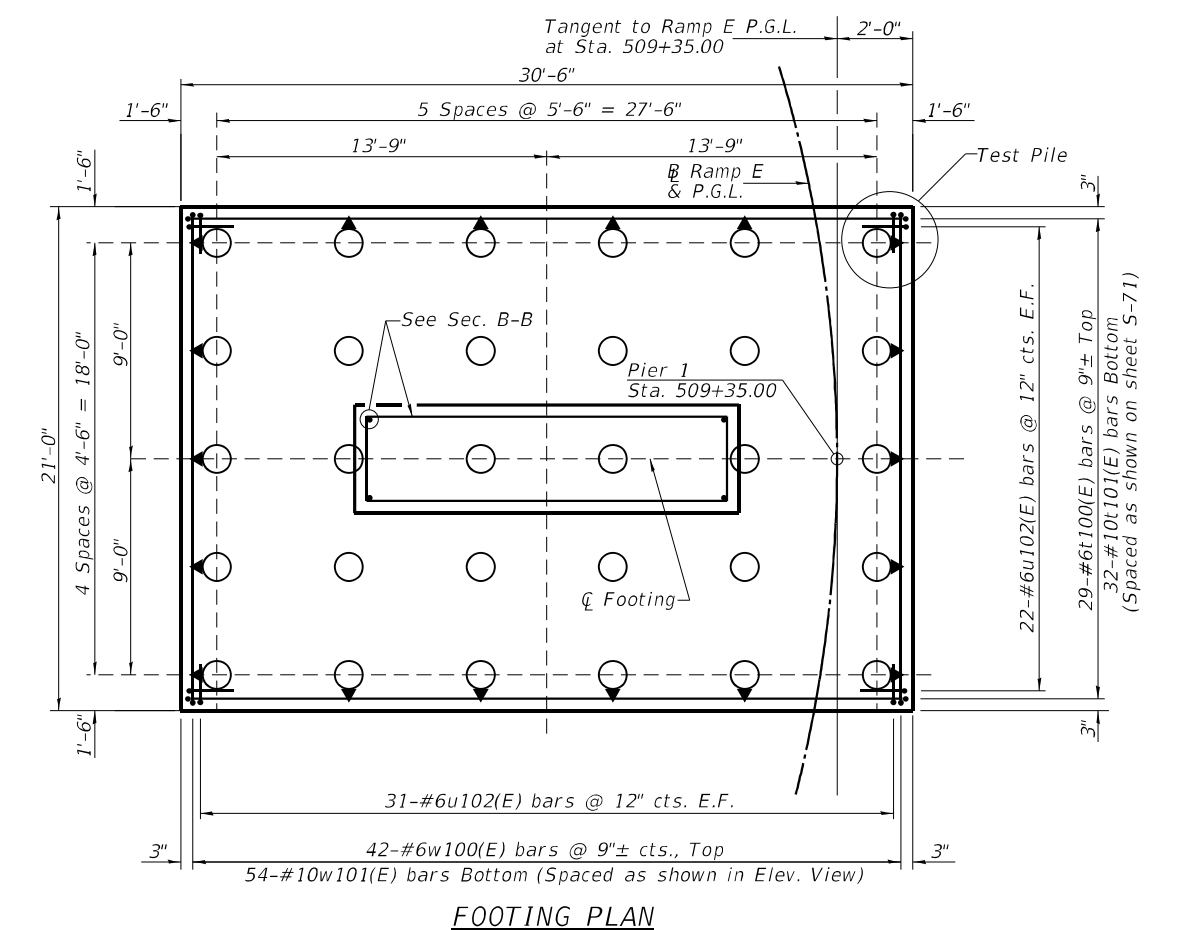


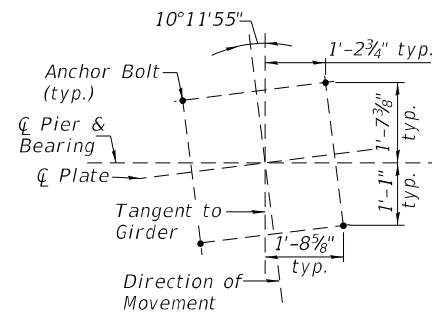
NOTES:

1. Space reinforcement in cap to miss anchor bolts.
2. 4:12 Batter on outside piles.
3. A single layer of cross ties s108(E) & s109(E) shall be provided across the top layer of footing reinforcement.
4. Bars noted thus, 3x2-#5 indicates 3 lines of bars with 2 lengths of bars per line.

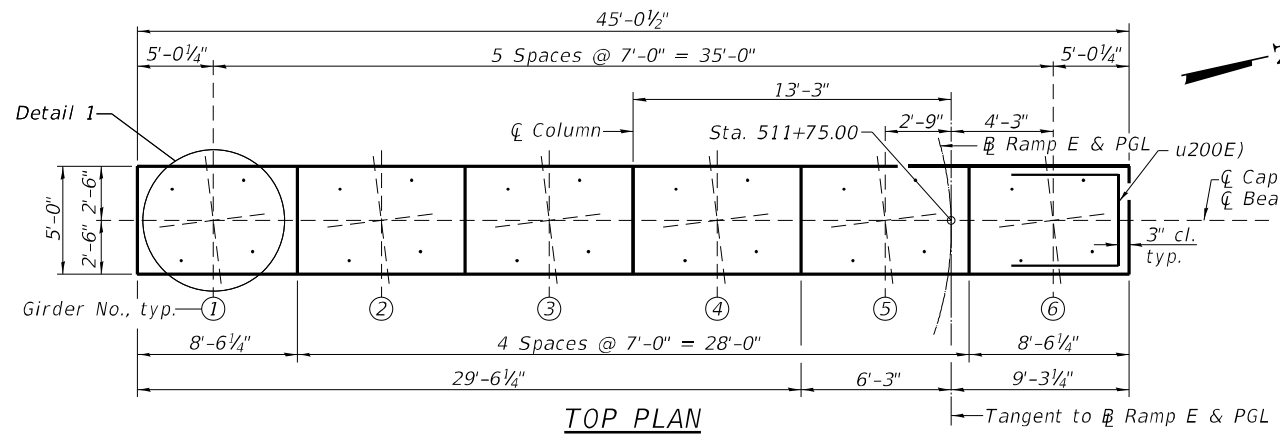
PILE DATA

Type: Steel Metal Shell, 16"Ø w/.312" walls with pile shoes
 Nominal Required Bearing: 566 kips
 Factored Resistance Available: 311 kips
 Est. Length: 66'
 No. Production Piles: 29
 No. Test Piles: 1

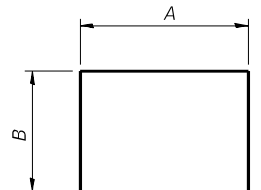
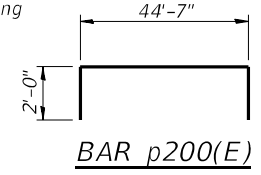




DETAIL 1
Bearing Orientation
(Typ. at Each Girder)



TOP PLAN

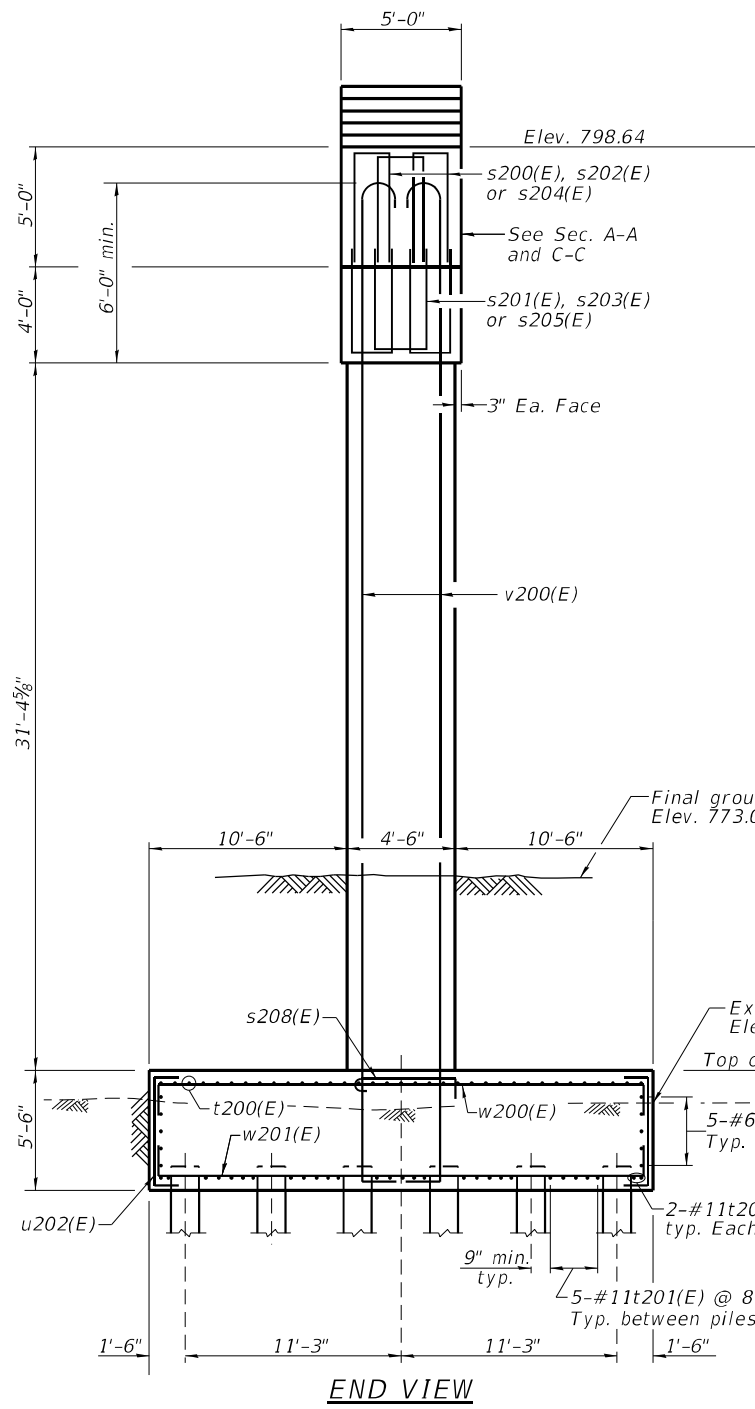


BARS
A & B DIMENSIONS

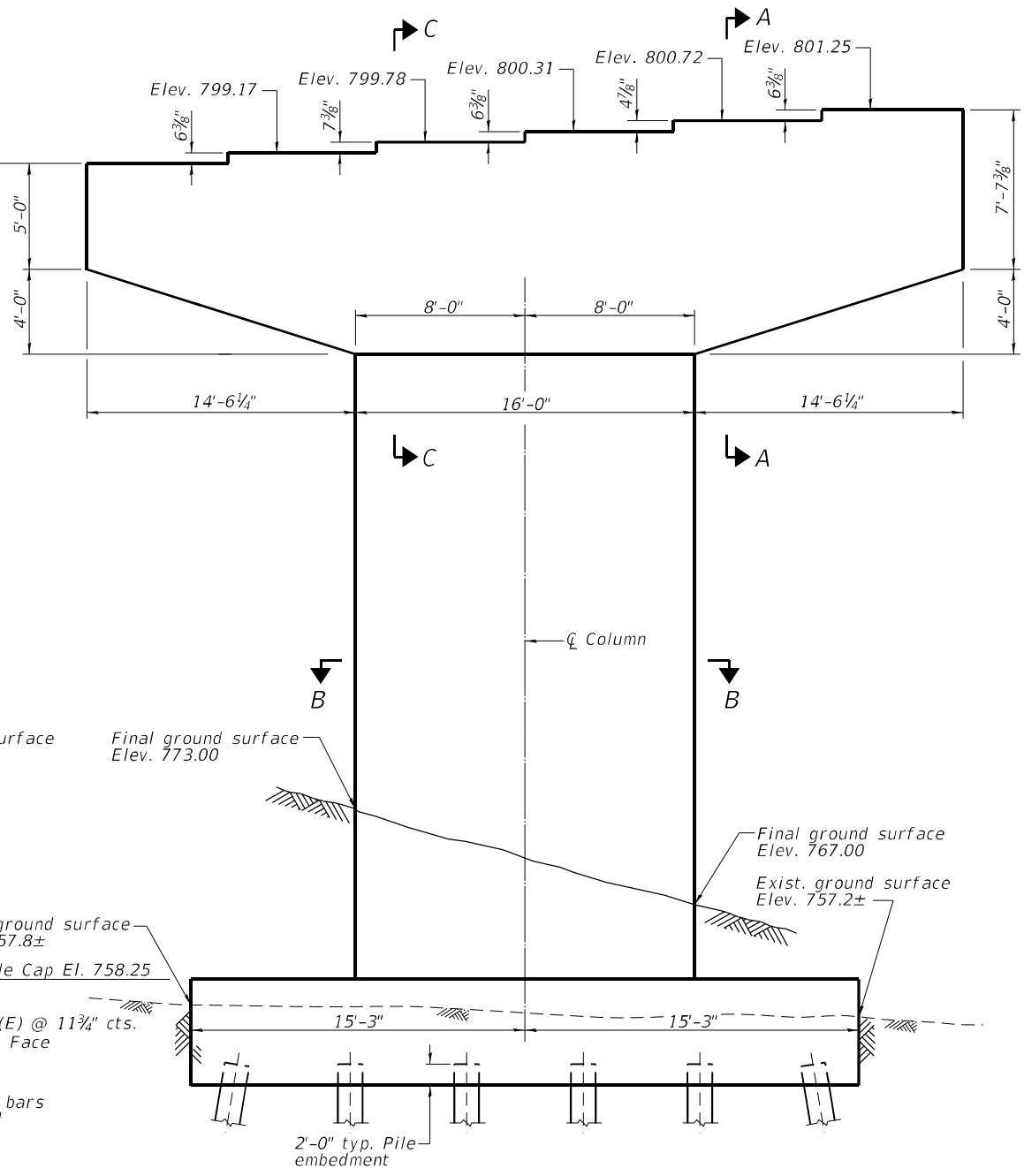
Bar	A	B
s200(E)	2'-0"	4'-4"
s201(E)	2'-6"	4'-4"
s202(E)	2'-0"	5'-6"
s203(E)	2'-6"	5'-6"
s204(E)	2'-0"	6'-7"
s205(E)	2'-6"	6'-7"
s206(E)	3'-1"	6'-7"
t200(E)	30'-2"	1'-0"
t201(E)	30'-2"	2'-0"
t202(E)	29'-11"	1'-0"
u200(E)	4'-6"	3'-10"
u201(E)	4'-8"	3'-10"
u202(E)	5'-1"	1'-0"
u203(E)	4'-2"	3'-2"
w200(E)	25'-2"	1'-0"
w201(E)	25'-2"	2'-0"
w202(E)	24'-11"	1'-0"

BILL OF MATERIAL

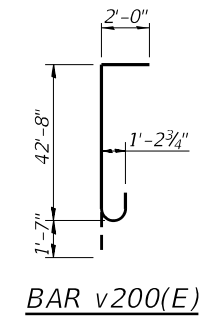
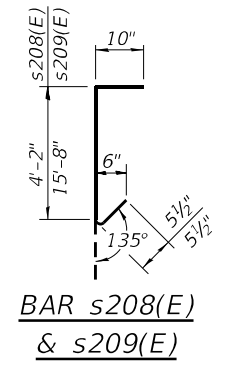
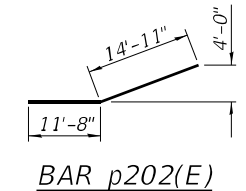
Bar	No.	Size	Length	Shape
h200(E)	12	#6	44'-8"	
h201(E)	2	#6	43'-2"	
h202(E)	2	#6	36'-0"	
h203(E)	2	#6	28'-9"	
h204(E)	2	#6	21'-6"	
h205(E)	24	#6	6'-8"	
h206(E)	12	#6	8'-2"	
h207(E)	192	#5	15'-8"	
p200(E)	9	#11	48'-7"	
p201(E)	9	#11	44'-8"	
p202(E)	12	#8	26'-7"	
s200(E)	20	#5	10'-8"	
s201(E)	10	#5	11'-2"	
s202(E)	64	#5	13'-0"	
s203(E)	32	#5	13'-6"	
s204(E)	132	#5	15'-2"	
s205(E)	66	#5	15'-8"	
s206(E)	56	#5	16'-3"	
s208(E)	1044	#5	5'-6"	
s209(E)	174	#5	17'-0"	
t200(E)	35	#6	32'-2"	
t201(E)	29	#11	34'-2"	
t202(E)	10	#6	31'-11"	
u200(E)	16	#6	12'-2"	
u201(E)	45	#6	12'-4"	
u202(E)	112	#6	7'-1"	
u203(E)	192	#5	10'-6"	
v200(E)	54	#11	46'-3"	
w200(E)	42	#6	27'-2"	
w201(E)	49	#11	29'-2"	
w202(E)	10	#6	26'-11"	
Structure Excavation		Cu. Yd.	189	
Concrete Structures		Cu. Yd.	317.6	
Reinforcement Bars, Epoxy Coated		Pound	59,720	
Furnishing Metal Shell Piles 16"x.312"		Foot	2,450	
Driving Piles		Foot	2,450	
Test Pile Metal Shell		Each	1	
Pile Shoes		Each	36	



END VIEW



ELEVATION
(Looking Upstation)



MIN. LAP LENGTH

#6 bars: 3'-10"
#5 bars: 3'-2"

- NOTES:**
1. Pour steps monolithically with cap.
 2. For Anchor Bolts Details see sheet S-46
 3. See sheet S-74 of S-106 for Sections A-A, B-B & C-C
 4. Pile cap concrete pour shall be in accordance with Article 1020.15, Heat of Hydration Control for Concrete Structures according to Standard Specifications. Cost included under "Concrete Structures."
 5. Reinforcement bar bending dimensions are out to out.

FILE NAME =
CMT
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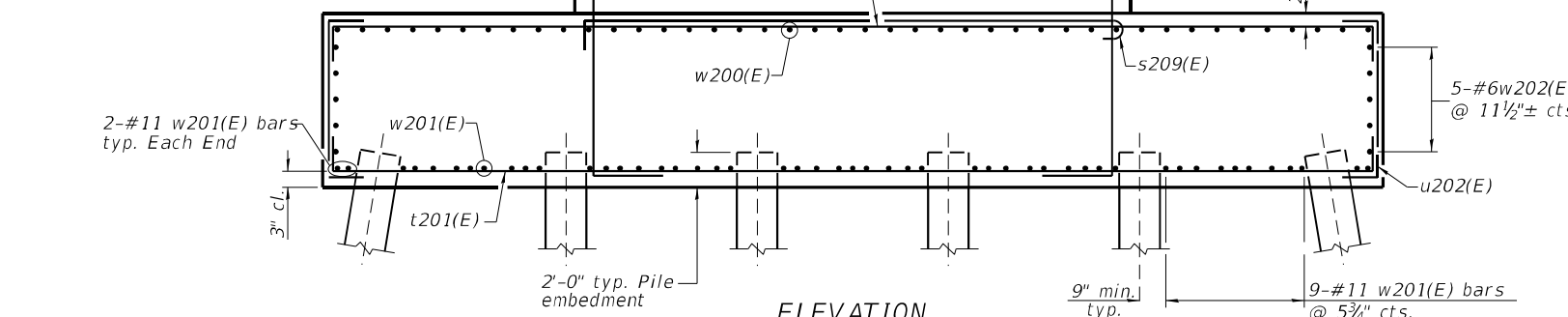
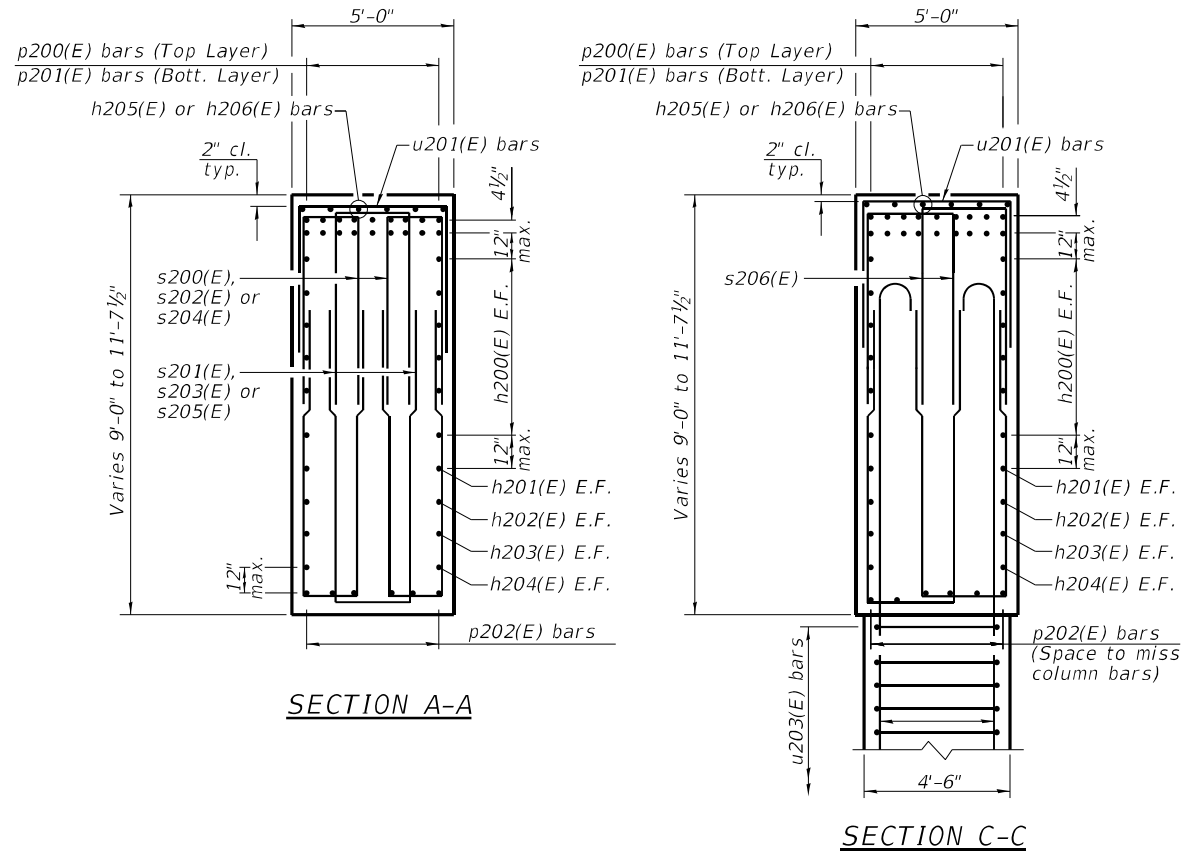
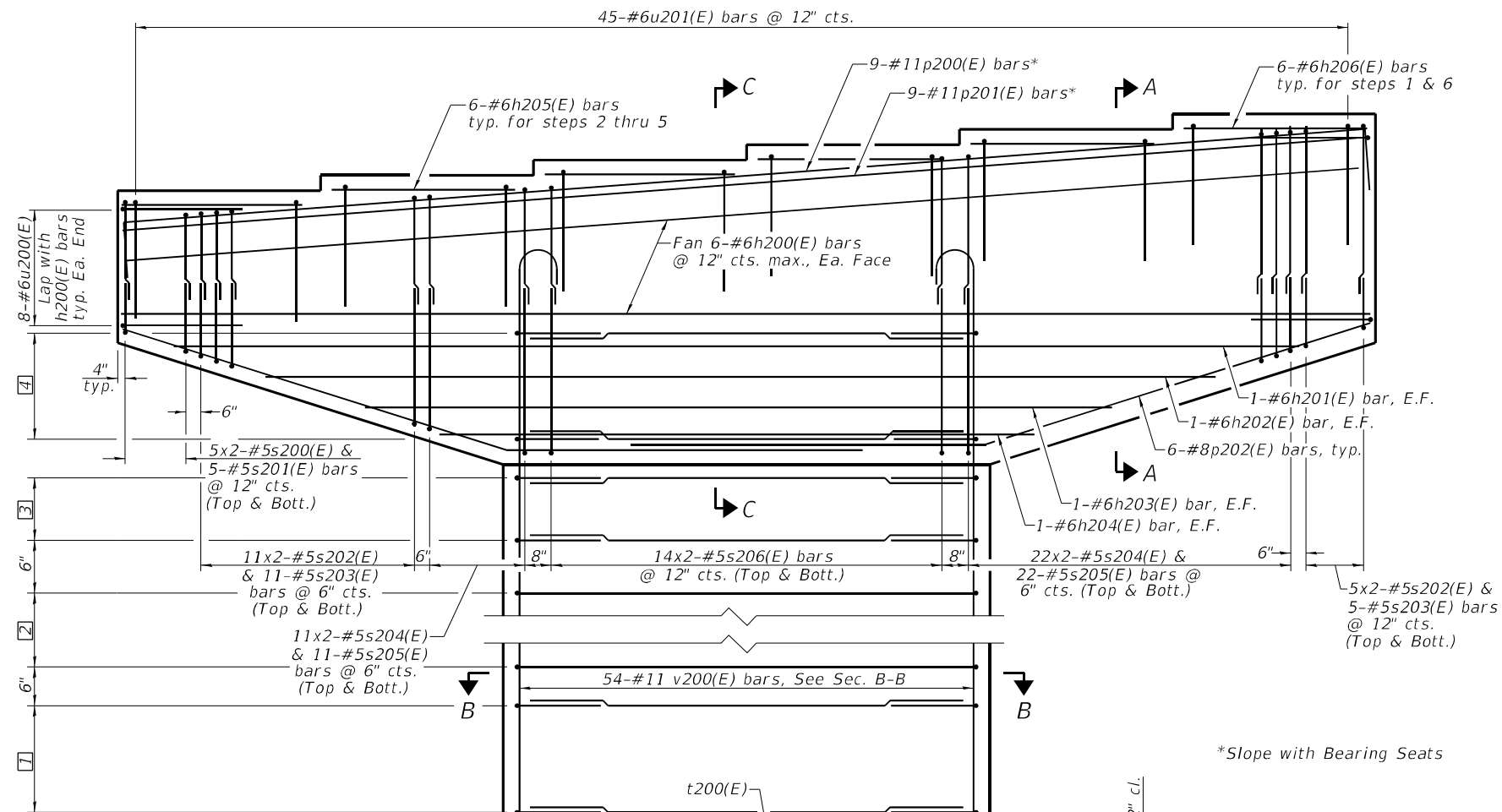
USER NAME = Denise Herrera	DESIGNED - LM	REVISED -
PLOT SCALE = NA	CHECKED - JDJ	REVISED -
PLOT DATE = 05/03/2021	DRAWN - GLD	REVISED -
	CHECKED - LM	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

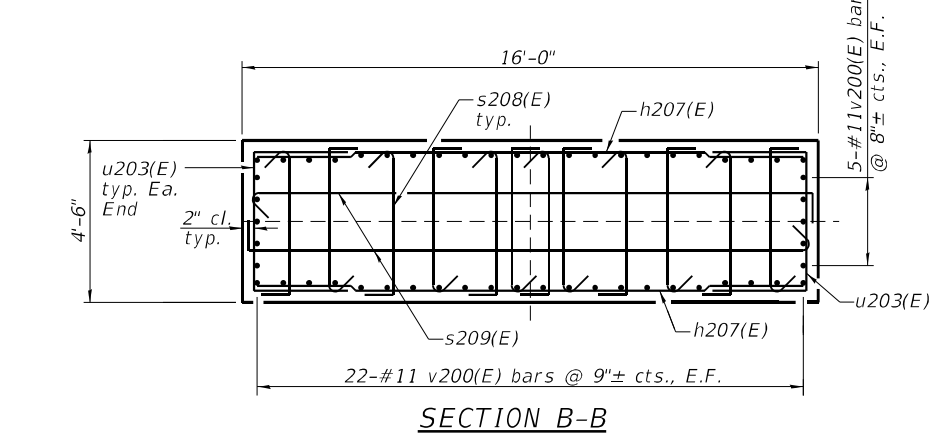
PIER 2 - PLAN AND ELEVATION
STRUCTURE NO. 010-1001

SHEET NO. S-73 OF S-106 SHEETS

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 715
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	

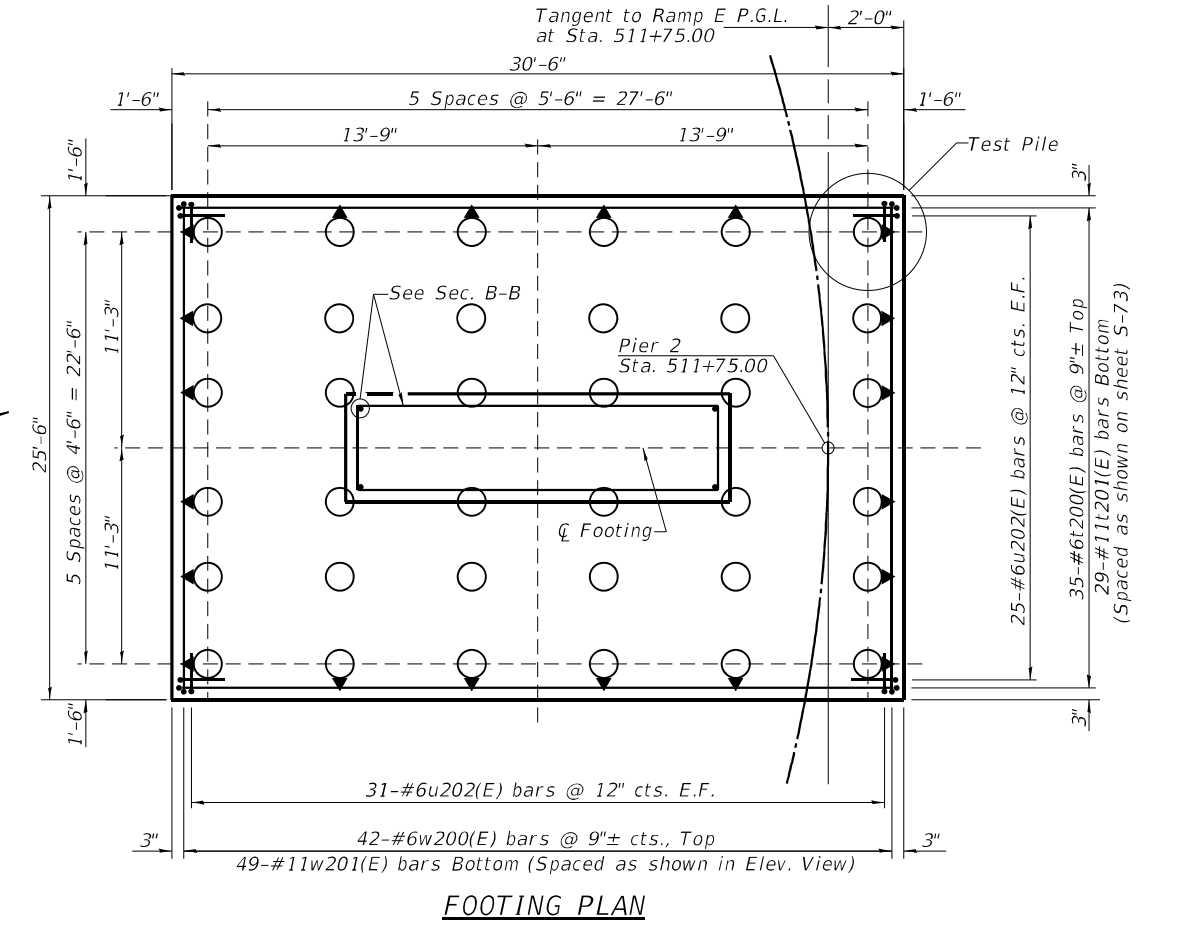


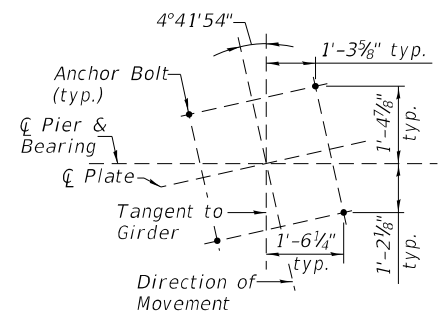
- 1 23 Sets - 2-#5h207(E) bars spliced with 2-#5u203(E), 12-#5s208(E) & 2-#5s209(E) at 3" cts.
- 2 40 Sets - 2-#5h207(E) bars spliced with 2-#5u203(E), 12-#5s208(E) & 2-#5s209(E) at 6" cts.
- 3 23 Sets - 2-#5h207(E) bars spliced with 2-#5u203(E), 12-#5s208(E), & 2-#5s209(E) at 3" cts.
- 4 10 Sets - 2-#5h207(E) bars spliced with 2-#5u203(E) at 3" cts.



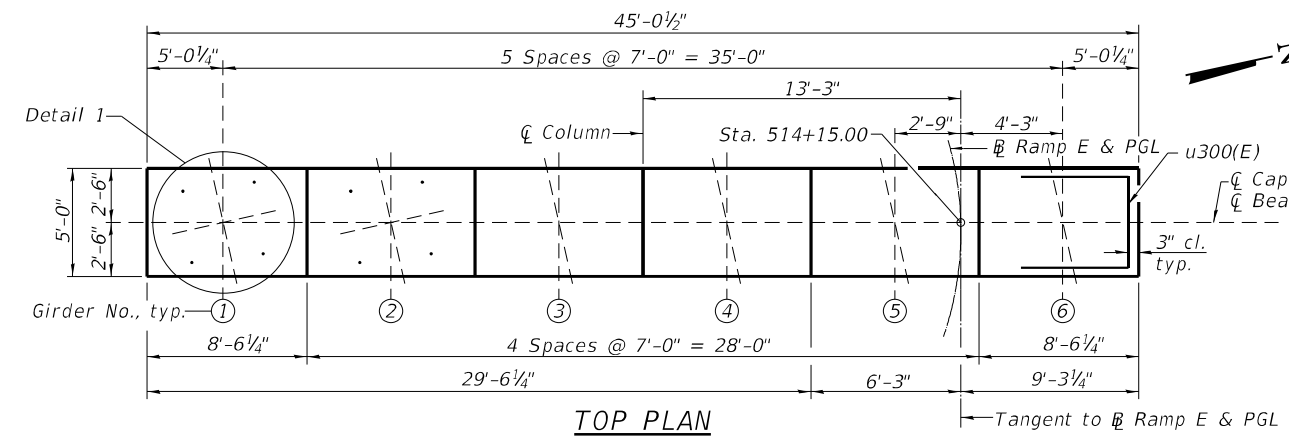
- NOTES:**
1. Space reinforcement in cap to miss anchor bolts.
 2. 4:12 Batter on outside piles.
 3. A single layer of cross ties s208(E) & s209(E) shall be provided across the top layer of footing reinforcement.
 4. Bars noted thus, 3x2-#5 indicates 3 lines of bars with 2 lengths of bars per line.

PILE DATA
 Type: Steel Metal Shell, 16"Ø w/.312" walls with pile shoes
 Nominal Required Bearing: 548 kips
 Factored Resistance Available: 301 Kips
 Est. Length: 70'
 No. Production Piles: 35
 No. Test Piles: 1

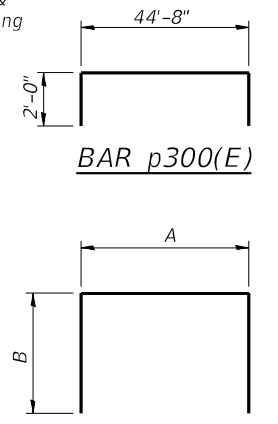




DETAIL 1
Bearing Orientation
(Typ. at Each Girder)



TOP PLAN

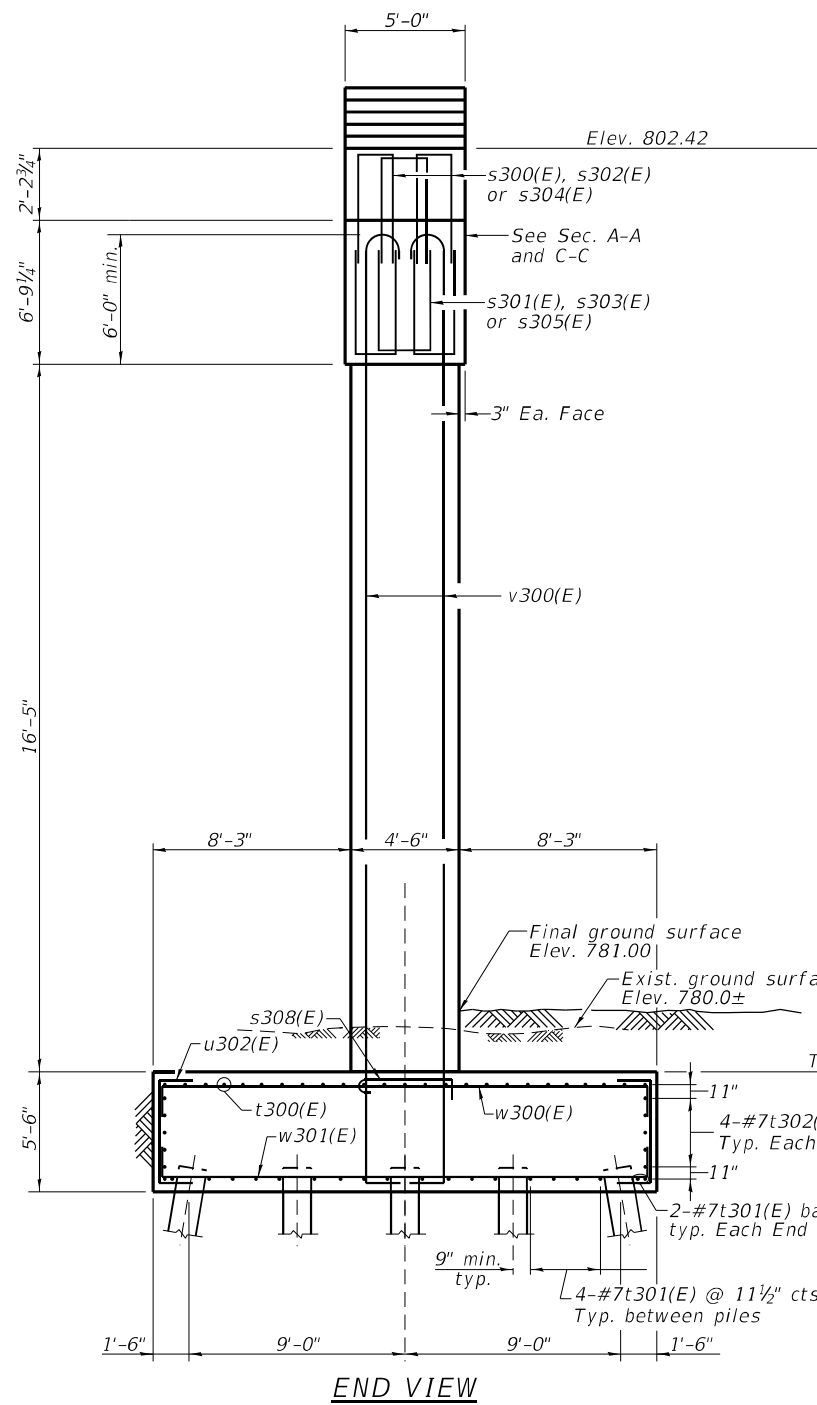


BARS
A & B DIMENSIONS

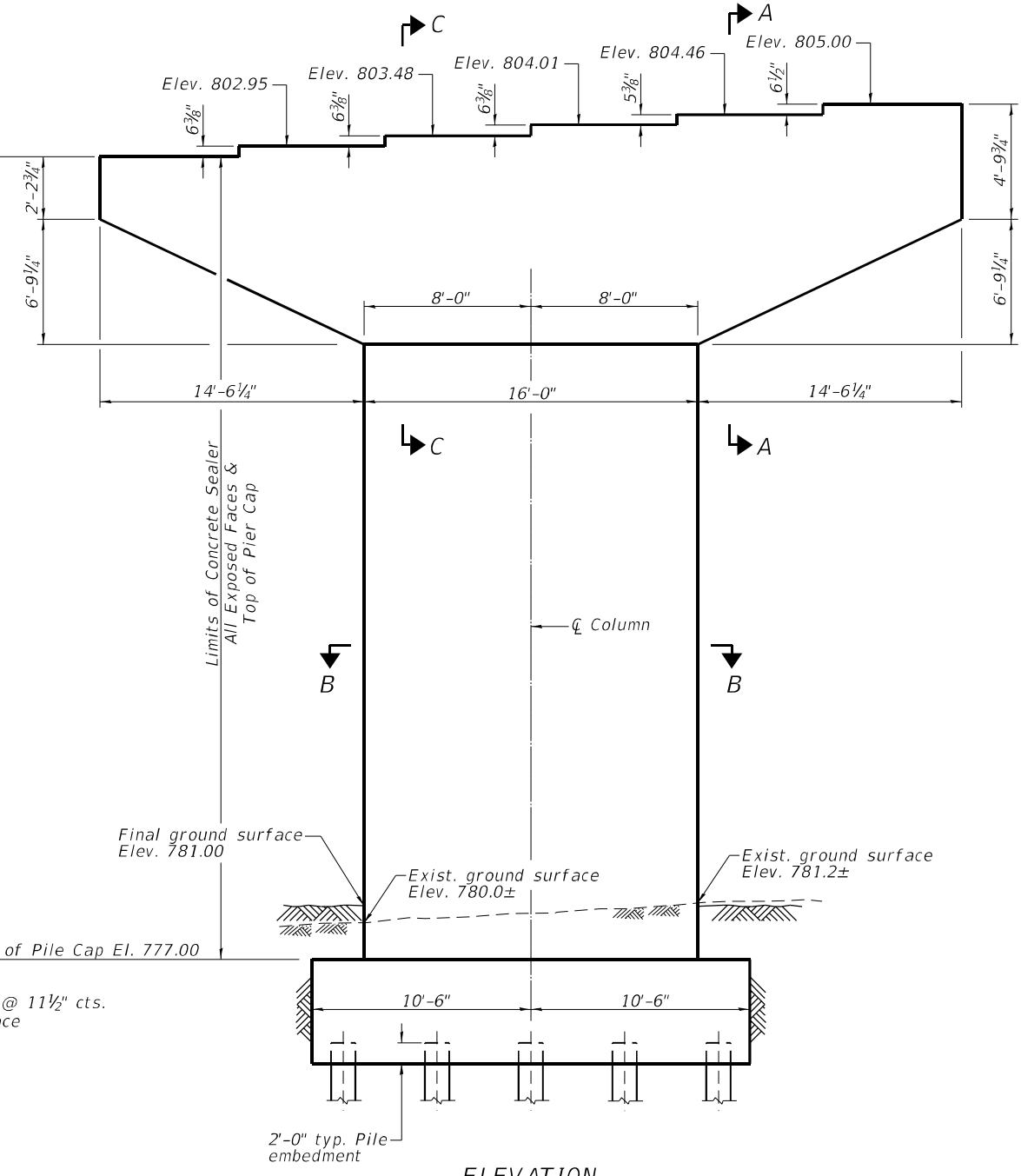
Bar	A	B
s300(E)	2'-0"	3'-5"
s301(E)	2'-6"	3'-5"
s302(E)	2'-0"	4'-8"
s303(E)	2'-6"	4'-8"
s304(E)	2'-0"	6'-8"
s305(E)	2'-6"	6'-8"
s306(E)	3'-1"	6'-8"
t300(E)	20'-8"	1'-2"
t301(E)	20'-8"	1'-2"
t302(E)	20'-5"	1'-2"
u300(E)	4'-6"	3'-10"
u301(E)	4'-8"	3'-10"
u302(E)	5'-1"	1'-0"
u303(E)	4'-2"	3'-2"
w300(E)	20'-8"	1'-2"
w301(E)	20'-8"	2'-0"
w302(E)	20'-5"	1'-2"

BILL OF MATERIAL

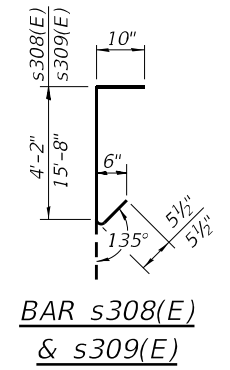
Bar	No.	Size	Length	Shape
h300(E)	8	#6	43'-4"	
h301(E)	2	#6	40'-5"	
h302(E)	2	#6	37'-9"	
h303(E)	2	#6	33'-5"	
h304(E)	2	#6	29'-2"	
h305(E)	24	#6	6'-8"	
h306(E)	12	#6	8'-2"	
h307(E)	122	#5	15'-8"	
h308(E)	2	#6	24'-10"	
h309(E)	2	#6	20'-7"	
p300(E)	9	#11	48'-8"	
p301(E)	9	#11	44'-8"	
p302(E)	12	#8	27'-5"	
s300(E)	20	#5	8'-10"	
s301(E)	10	#5	9'-4"	
s302(E)	56	#5	11'-4"	
s303(E)	28	#5	11'-10"	
s304(E)	132	#5	15'-4"	
s305(E)	66	#5	15'-10"	
s306(E)	60	#5	16'-5"	
s308(E)	624	#5	5'-6"	
s309(E)	104	#5	17'-0"	
t300(E)	21	#7	23'-0"	
t301(E)	20	#7	23'-0"	
t302(E)	8	#7	22'-9"	
u300(E)	8	#6	12'-2"	
u301(E)	45	#6	12'-4"	
u302(E)	84	#6	7'-1"	
u303(E)	122	#5	10'-6"	
v300(E)	54	#11	31'-3"	
w300(E)	21	#7	23'-0"	
w301(E)	36	#11	24'-8"	
w302(E)	8	#7	22'-9"	
Structure Excavation	Cu. Yd.		209	
Concrete Structures	Cu. Yd.		201.3	
Reinforcement Bars, Epoxy Coated	Pound		40,300	
Furnishing Metal Shell Piles 16"x.312"	Foot		1,656	
Driving Piles	Foot		1,656	
Test Pile metal Shell	Each		1	
Pile shoes	Each		25	
Concrete Sealer	Sq. Ft.		2,038	



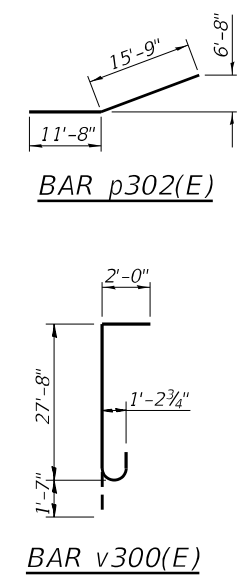
END VIEW



ELEVATION
(Looking Upstation)



BAR s308(E)
& s309(E)



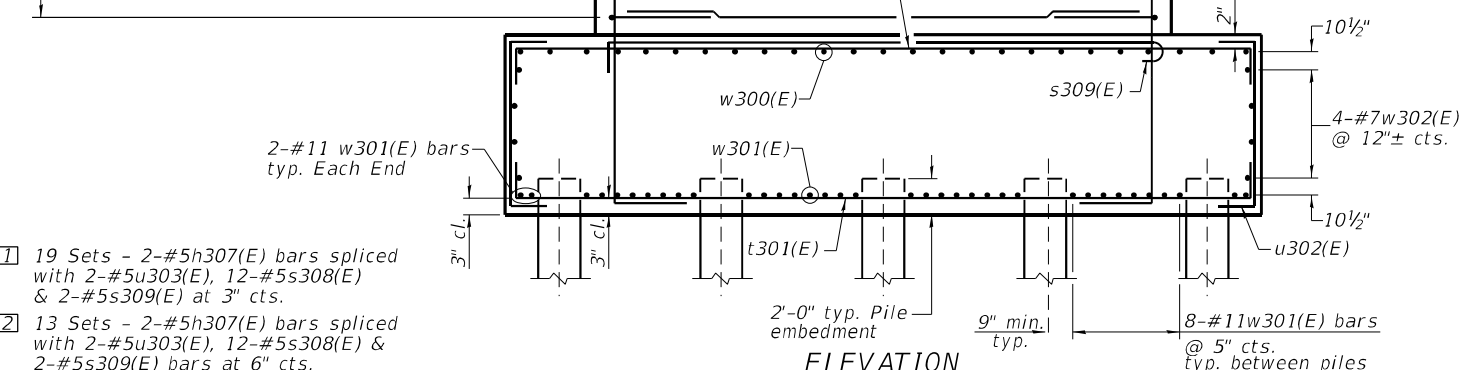
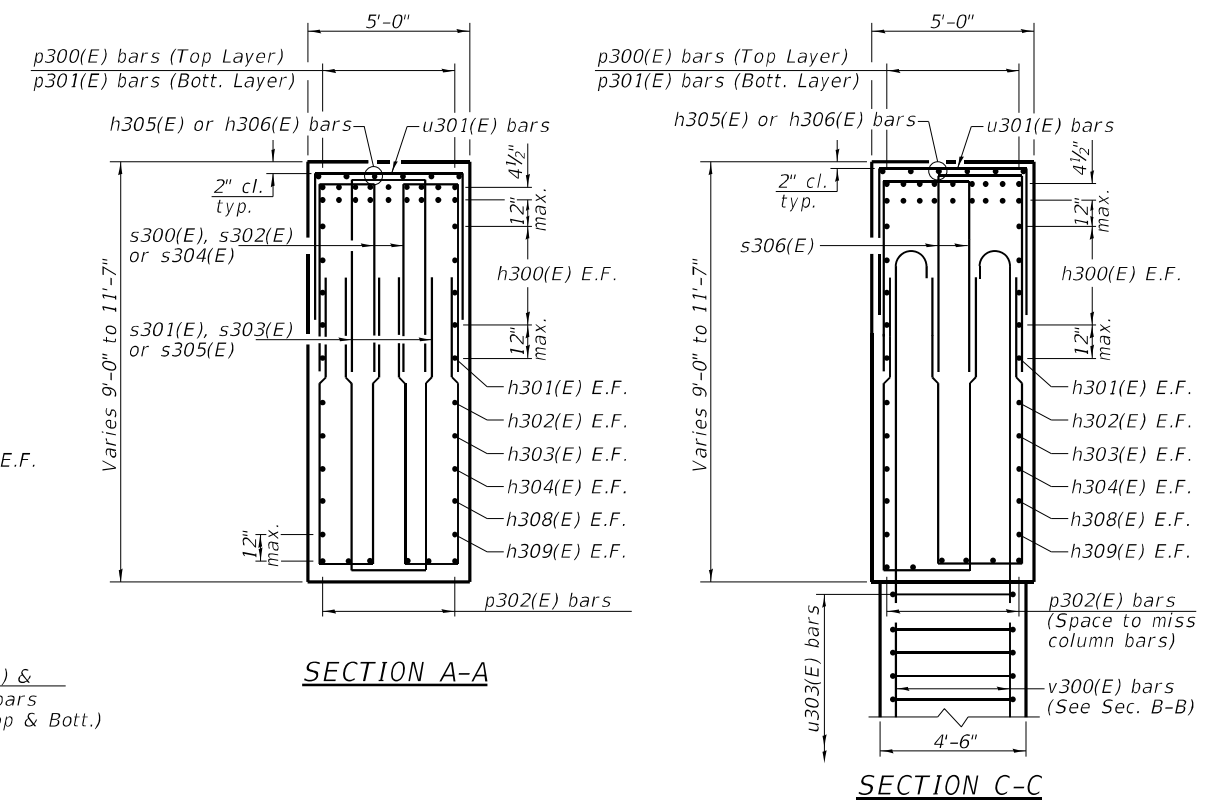
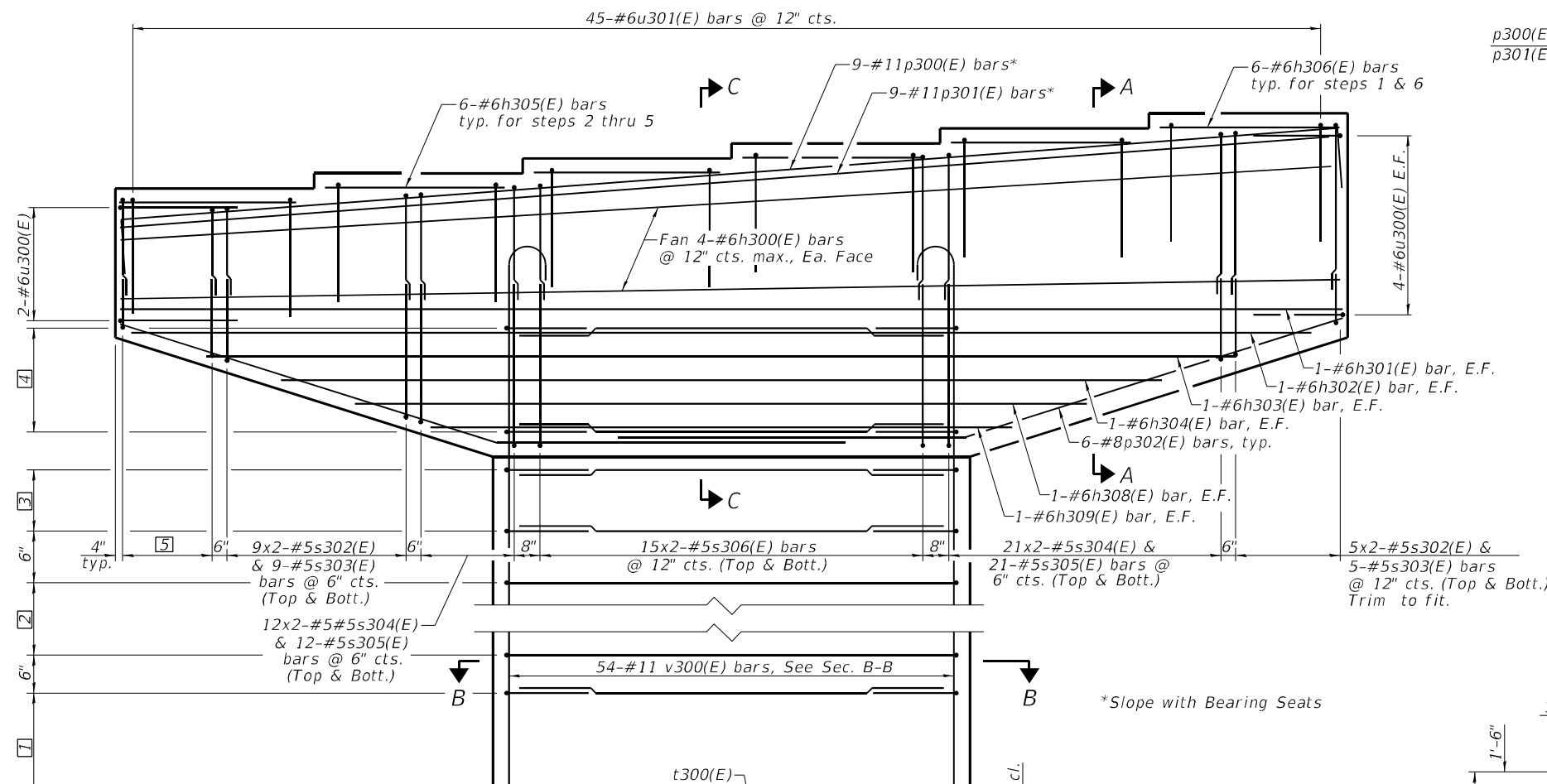
BAR v300(E)

MIN. LAP LENGTH

#5 bars: 3'-2"
#6 bars: 3'-10"

NOTES:

1. Pour steps monolithically with cap.
2. For Anchor Bolts Details see sheet S-48
3. See sheet S-76 of S-106 for Sections A-A, B-B & C-C
4. Pile cap concrete pour shall be in accordance with Article 1020.15, Heat of Hydration Control for Concrete Structures according to Standard Specifications. Cost included under "Concrete Structures."
5. Reinforcement bar bending dimensions are out to out.



- 1 19 Sets - 2-#5h307(E) bars spliced with 2-#5u303(E), 12-#5s308(E) & 2-#5s309(E) at 3" cts.
- 2 13 Sets - 2-#5h307(E) bars spliced with 2-#5u303(E), 12-#5s308(E) & 2-#5s309(E) bars at 6" cts.
- 3 19 Sets - 2-#5h307(E) bars spliced with 2-#5u303(E), 12-#5s308(E) & 2-#5s309(E) at 3" cts.
- 4 10 Sets - 2-#5h307(E) bars spliced with 2-#5u303(E) at 3" cts.

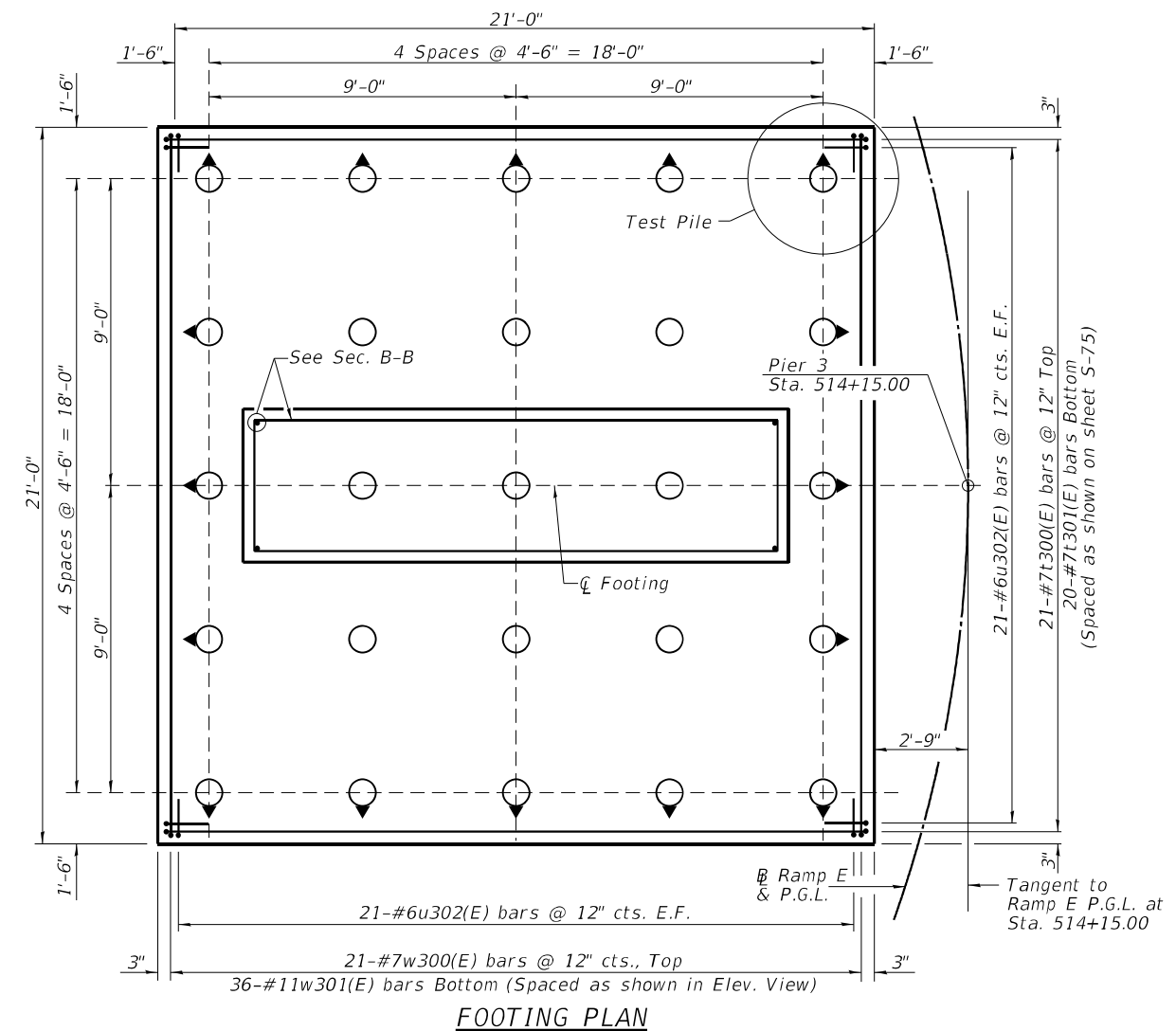
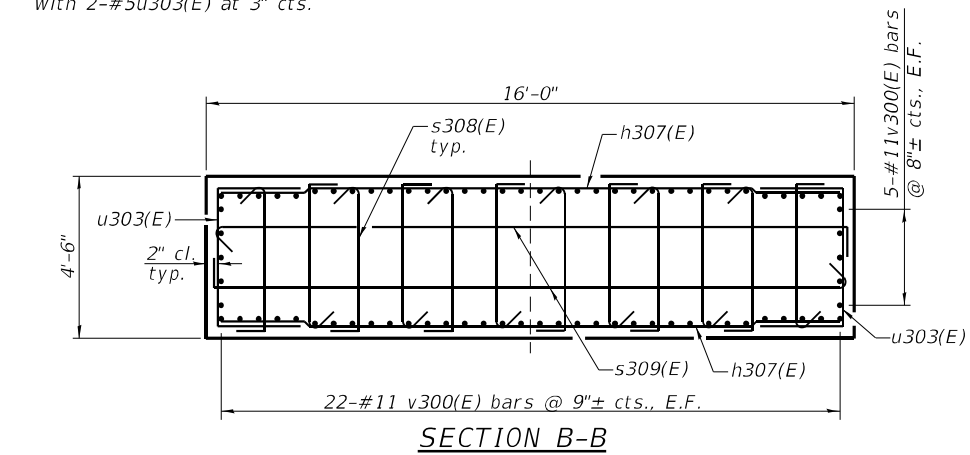
- 5 5x2-#5s300(E) & 5-#5s301(E) bars @ 12" cts. (Top & Bott.) Trim to fit.

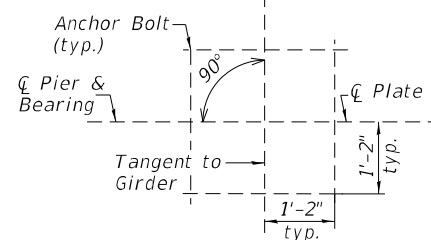
NOTES:

1. Space reinforcement in cap to miss anchor bolts.
2. 4:12 Batter on outside piles.
3. A single layer of cross ties s308(E) & s309(E) shall be provided across the top layer of footing reinforcement.
4. Bars noted thus, 3x2-#5 indicates 3 lines of bars with 2 lengths of bars per line.

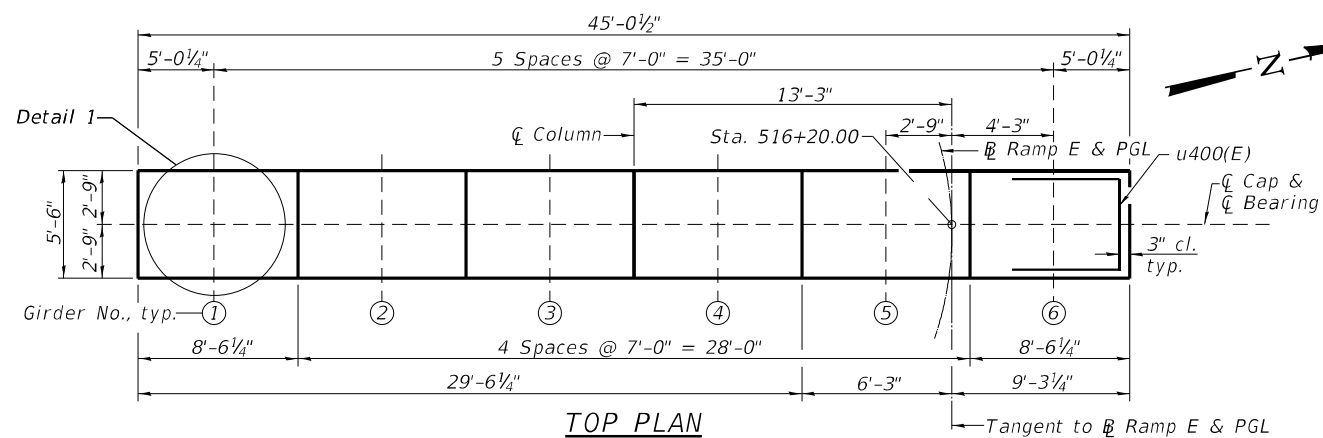
PILE DATA

Type: Steel Metal Shell, 16"Ø w/.312" walls with pile shoes
 Nominal Required Bearing: 617 kips
 Factored Resistance Available: 339 kips
 Est. Length: 69'
 No. Production Piles: 24
 No. Test Piles: 1

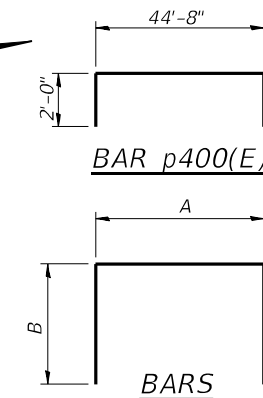




DETAIL 1
Bearing Orientation
(Typ. at Each Girder)



TOP PLAN



A & B DIMENSIONS

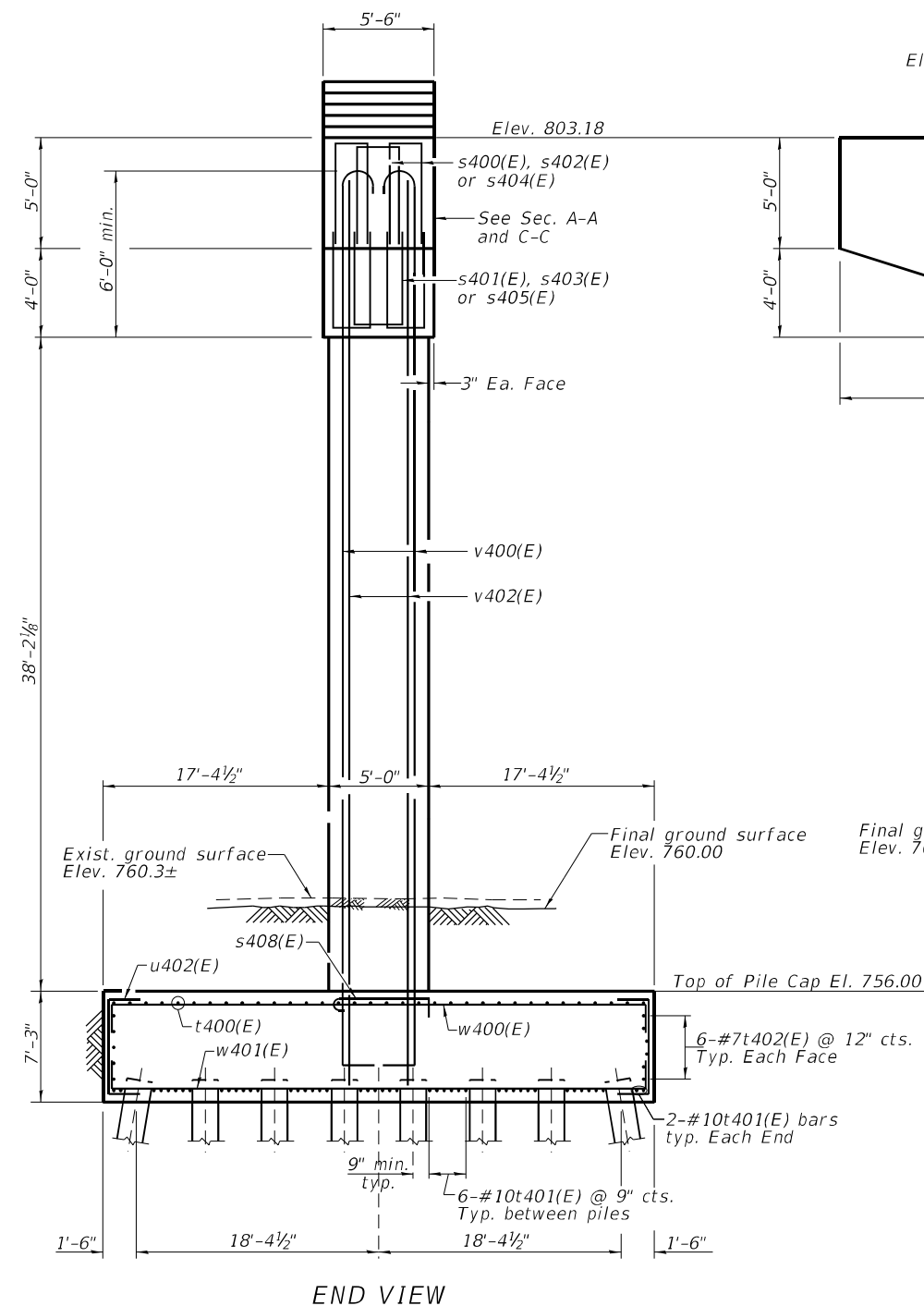
Bar	A	B
s400(E)	2'-2"	4'-5"
s401(E)	2'-8"	4'-5"
s402(E)	2'-2"	5'-9"
s403(E)	2'-8"	5'-9"
s404(E)	2'-2"	6'-9"
s405(E)	2'-8"	6'-9"
s406(E)	3'-4"	6'-9"
t400(E)	37'-8"	1'-2"
t401(E)	37'-8"	2'-0"
t402(E)	37'-5"	1'-2"
u400(E)	5'-0"	3'-10"
u401(E)	5'-2"	3'-10"
u402(E)	6'-10"	1'-0"
u403(E)	4'-8"	3'-2"
w400(E)	39'-5"	1'-2"
w401(E)	39'-5"	2'-0"
w402(E)	39'-2"	1'-2"

BILL OF MATERIAL

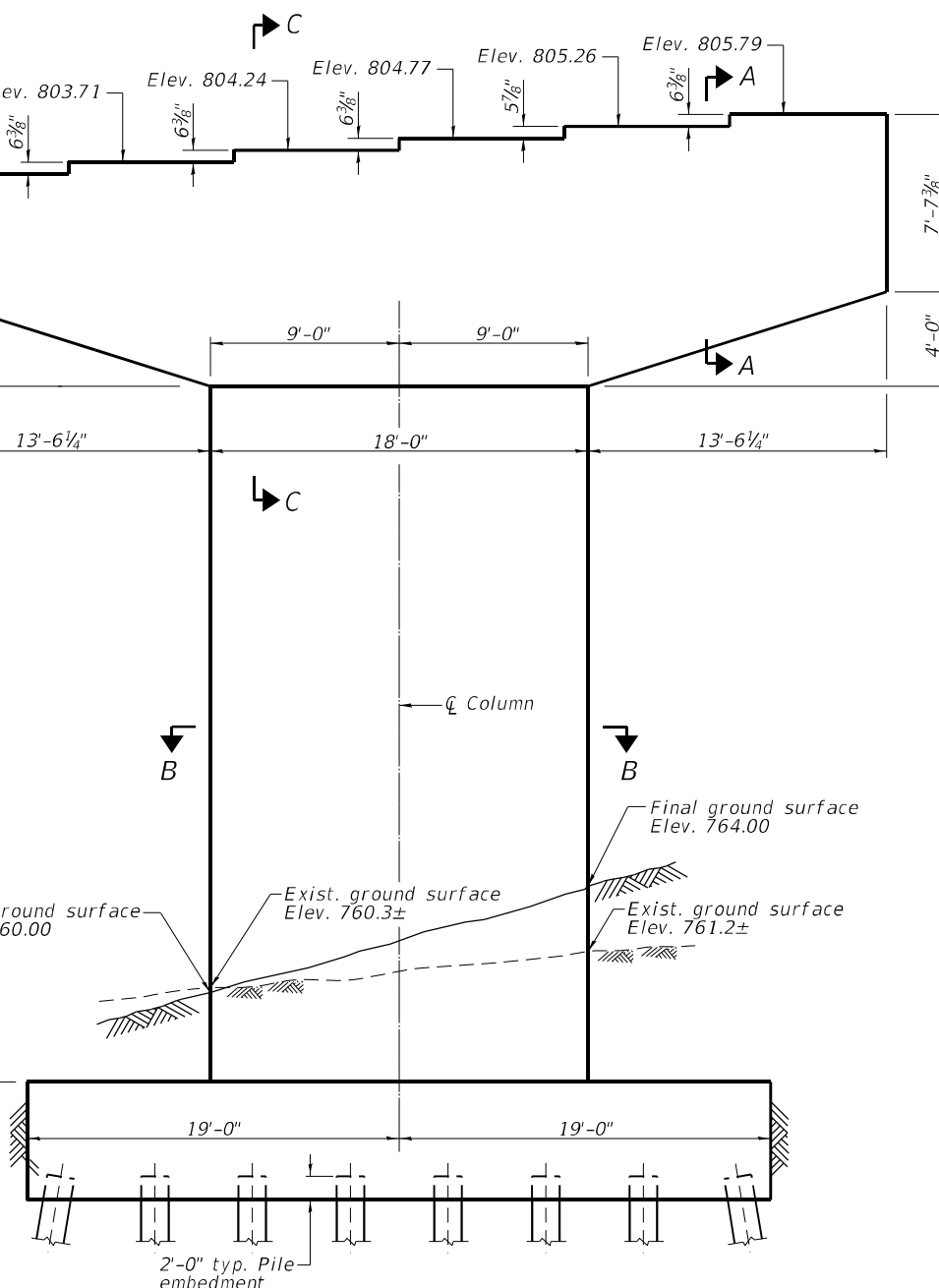
Bar	No.	Size	Length	Shape
h400(E)	12	#6	44'-8"	
h401(E)	2	#6	43'-5"	
h402(E)	2	#6	36'-8"	
h403(E)	2	#6	29'-11"	
h404(E)	2	#6	23'-1"	
h405(E)	24	#6	6'-8"	
h406(E)	12	#6	8'-2"	
h407(E)	226	#5	17'-8"	
p400(E)	9	#11	48'-8"	
p401(E)	9	#11	44'-8"	
p402(E)	12	#8	26'-7"	
s400(E)	20	#5	11'-0"	
s401(E)	10	#5	11'-6"	
s402(E)	76	#5	13'-8"	
s403(E)	38	#5	14'-2"	
s404(E)	96	#5	15'-8"	
s405(E)	48	#5	16'-2"	
s406(E)	68	#5	16'-10"	
s408(E)	1664	#5	6'-0"	
s409(E)	208	#5	19'-0"	
t400(E)	40	#7	40'-0"	
t401(E)	46	#10	41'-8"	
t402(E)	12	#7	39'-9"	
u400(E)	12	#6	12'-8"	
u401(E)	45	#6	12'-10"	
u402(E)	156	#6	8'-10"	
u403(E)	226	#5	11'-0"	
v400(E)	72	#11	54'-7"	
v402(E)	64	#11	51'-3"	
w400(E)	38	#7	41'-9"	
w401(E)	74	#11	43'-5"	
w402(E)	12	#7	41'-6"	
Structure Excavation		Cu. Yd.	834	
Concrete Structures		Cu. Yd.	616.5	
Reinforcement Bars, Epoxy Coated		Pound	109,160	
Furnishing Metal Shell Piles 16"x.312"		Foot	3,339	
Driving Piles		Foot	3,339	
Test Pile metal Shell		Each	1	
Pile shoes		Each	64	

MIN. LAP LENGTH

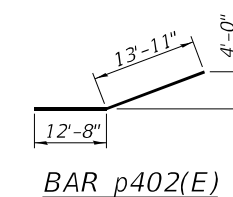
#5 bars: 3'-2"
#6 bars: 3'-10"



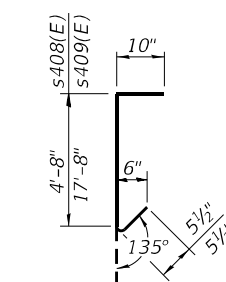
END VIEW



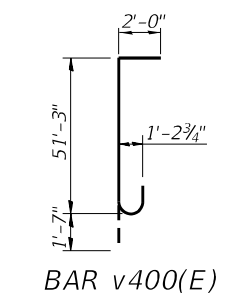
ELEVATION
(Looking Upstation)



BAR p402(E)



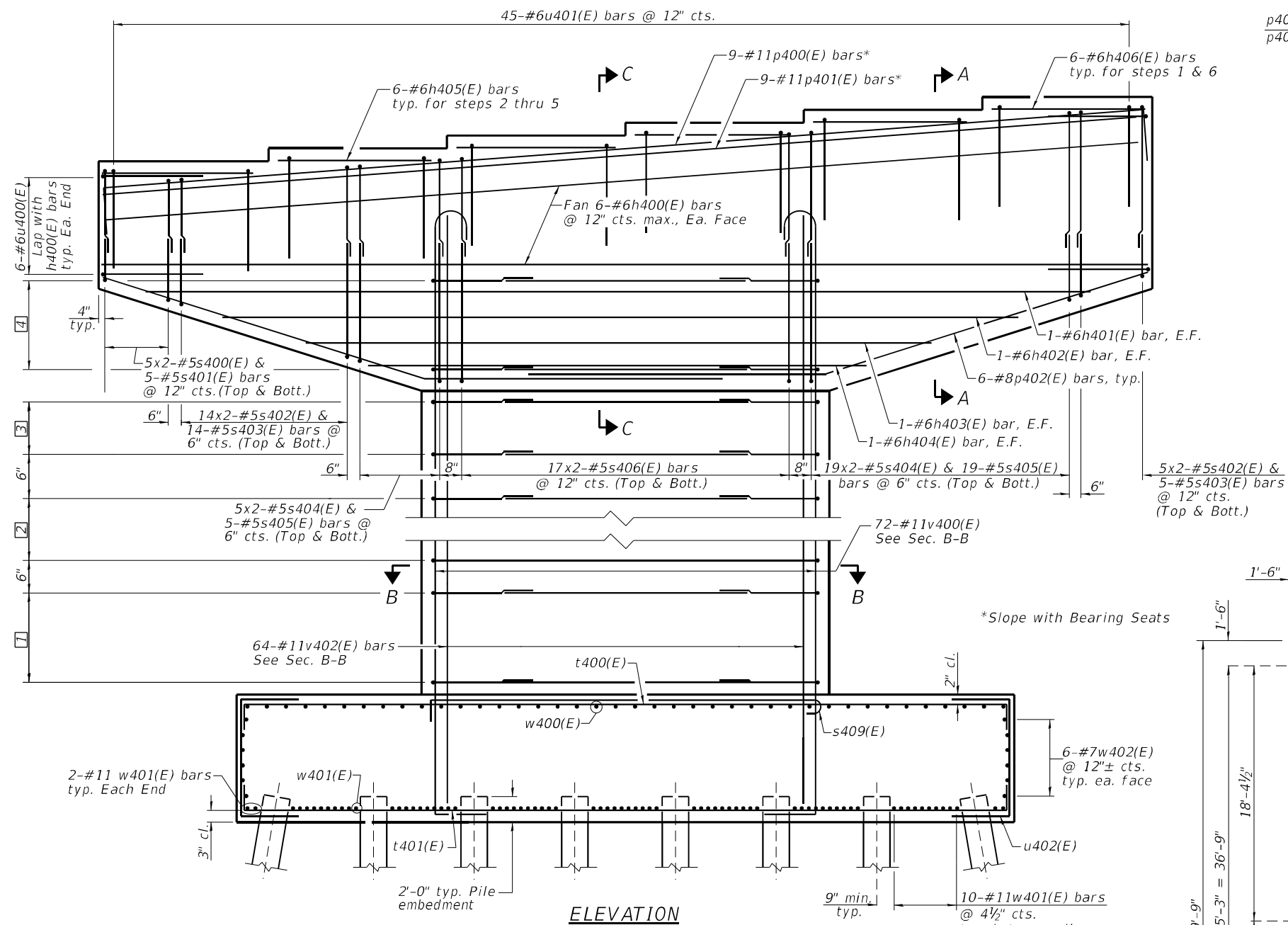
BAR s408(E) & s409(E)



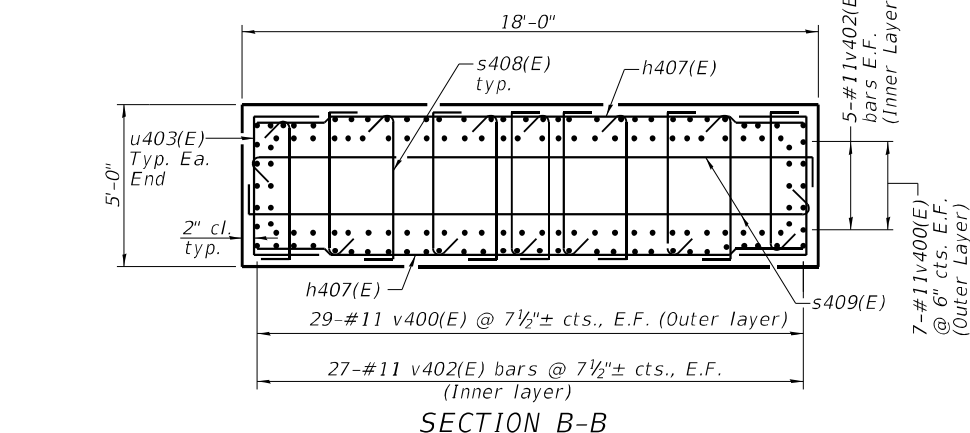
BAR v400(E)

NOTES:

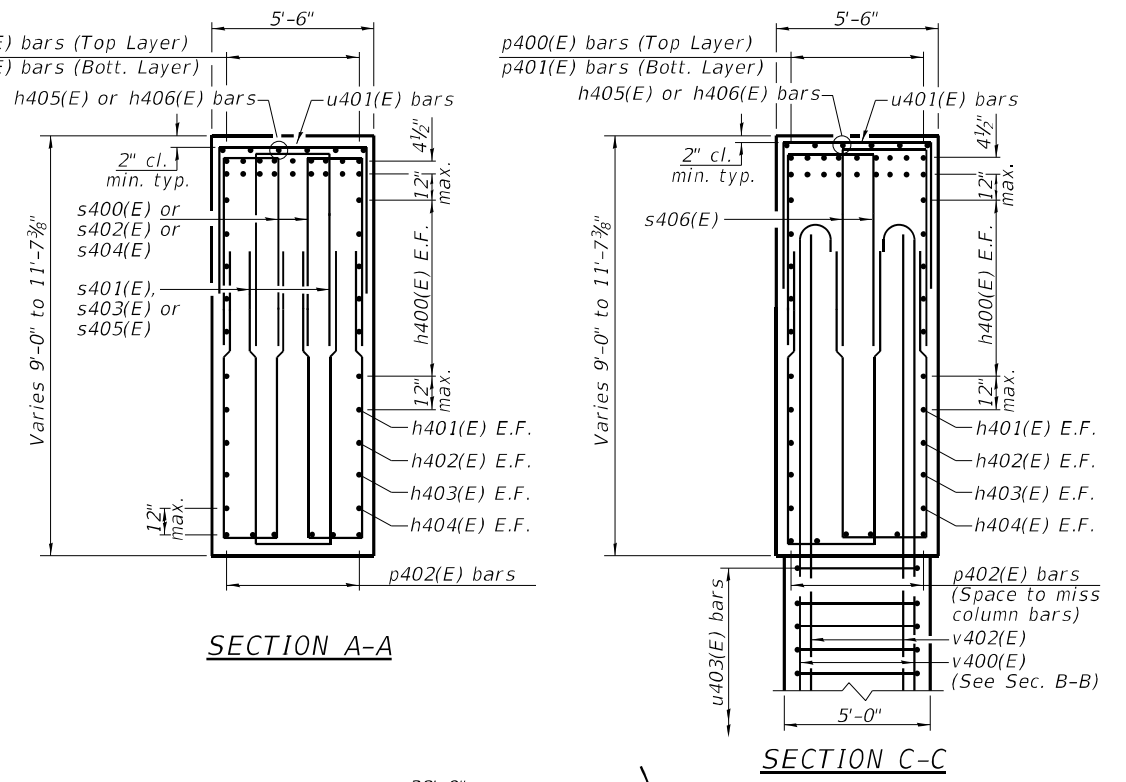
- Four steps monolithically with cap.
- For Anchor Bolts Details see sheet S-48
- See sheet S-78 of S-106 for Sections A-A, B-B & C-C
- Pile cap concrete pour shall be in accordance with Article 1020.15, Heat of Hydration Control for Concrete Structures according to Standard Specifications. Cost included under "Concrete Structures."
- Reinforcement bar bending dimensions are out to out.



- 1 27 Sets - 2-#5h407(E) bars spliced with 2-#5u403(E), 16-#5s408(E) & 2-#5s409(E) at 3" cts.
- 2 49 Sets - 2-#5h407(E) bars spliced with 2-#5u403(E) 16-#5s408(E) & 2-#5s409(E) at 6"± cts.
- 3 27 Sets - 2-#5h407(E) bars spliced with 2-#5u403(E), 16-#5s408(E) & 2-#5s409(E) at 3" cts.
- 4 10 Sets - 2-#5h407(E) bars spliced with 2-#5u403(E) at 3" cts.

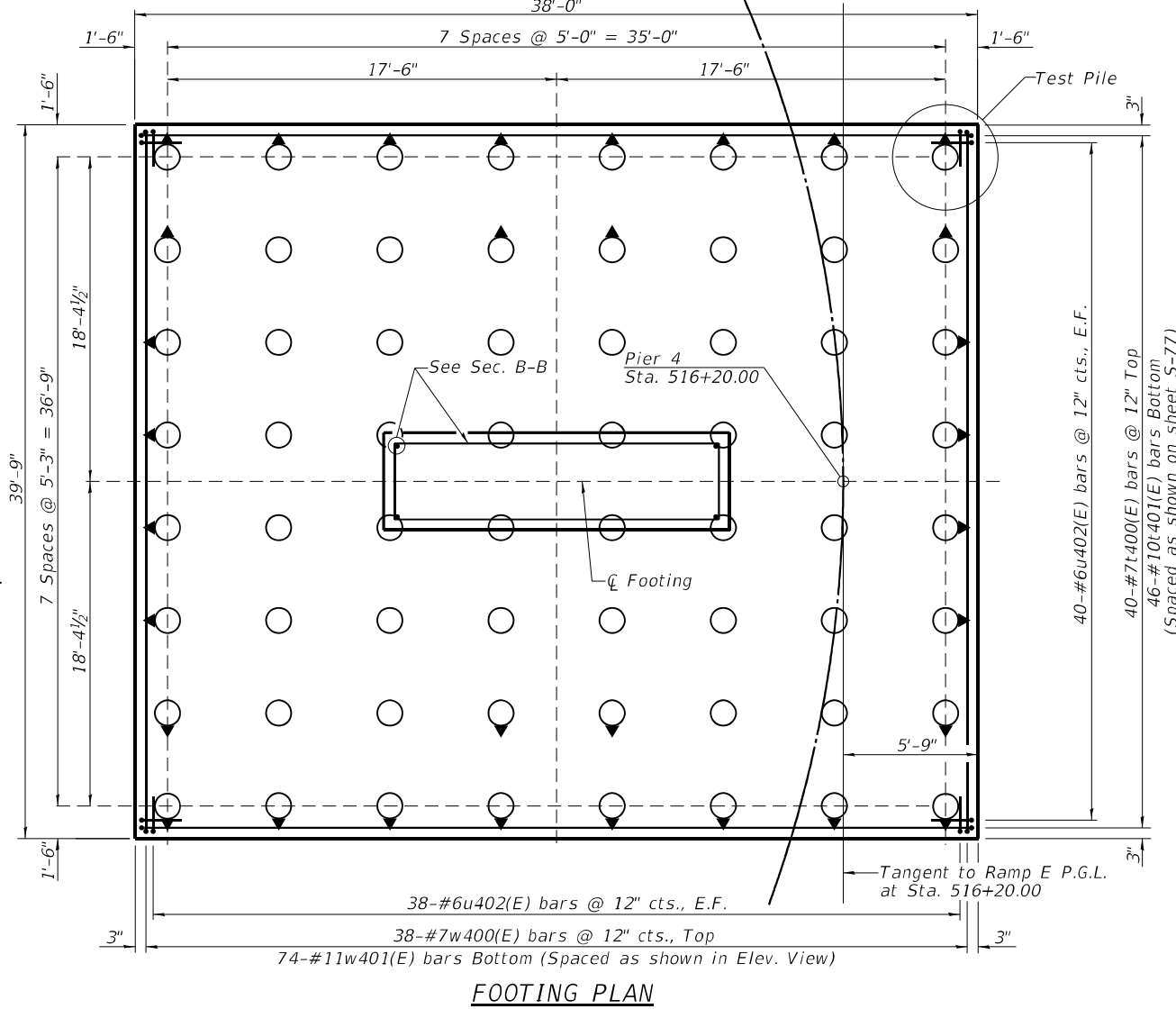


SECTION B-B



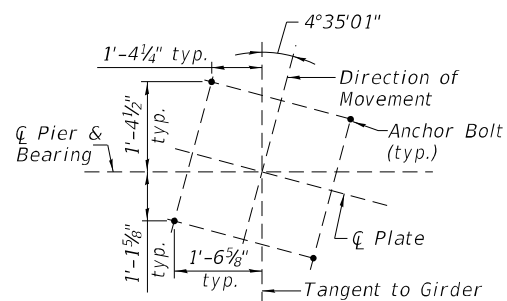
SECTION A-A

SECTION C-C

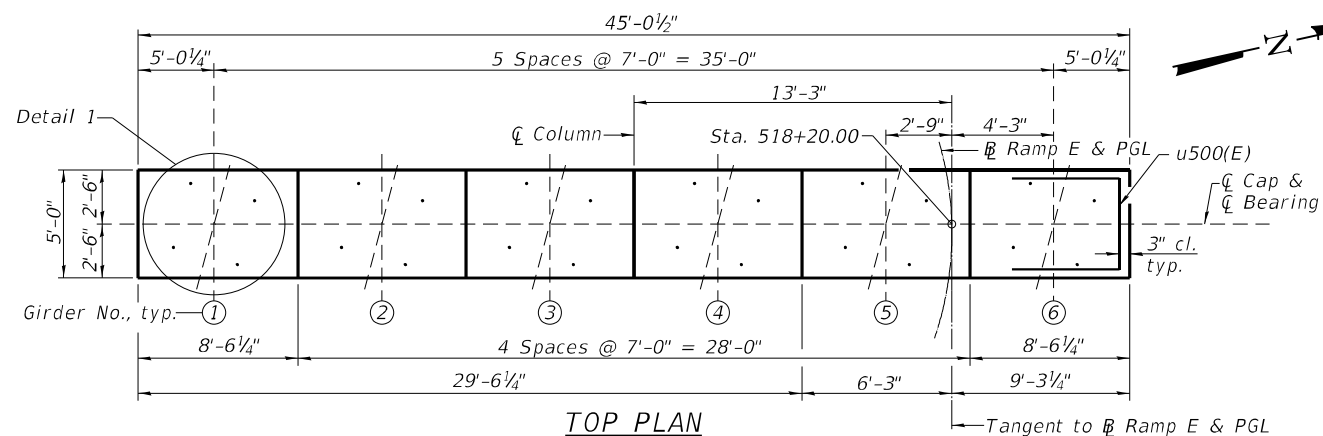


FOOTING PLAN

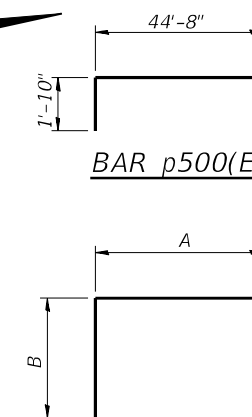
- NOTES:**
1. Space reinforcement in cap to miss anchor bolts.
 2. 4:12 Batter on indicated piles.
 3. A single layer of cross ties s408(E) & s409(E) shall be provided across the top layer of footing reinforcement.
 4. Bars noted thus, 3x2-#5 indicates 3 lines of bars with 2 lengths of bars per line.
- PILE DATA**
- Type: Steel Metal Shell, 16" \varnothing w/.312" walls with pile shoes
 Nominal Required Bearing: 375 kips
 Factored Resistance Available: 206 kips
 Est. Length: 53'
 No. Production Piles: 63
 No. Test Piles: 1



DETAIL 1
Bearing Orientation
(Typ. at Each Girder)



TOP PLAN



BARS

A & B DIMENSIONS

Bar	A	B
s500(E)	3'-1"	4'-5"
s501(E)	3'-1"	5'-8"
s502(E)	3'-1"	6'-8"
t500(E)	25'-2"	1'-2"
t501(E)	25'-2"	2'-0"
t502(E)	24'-11"	1'-2"
u500(E)	4'-6"	3'-10"
u501(E)	4'-8"	3'-10"
u502(E)	5'-7"	1'-0"
u503(E)	4'-2"	3'-2"
w500(E)	25'-2"	1'-2"
w501(E)	25'-2"	2'-0"
w502(E)	24'-11"	1'-2"

BILL OF MATERIAL

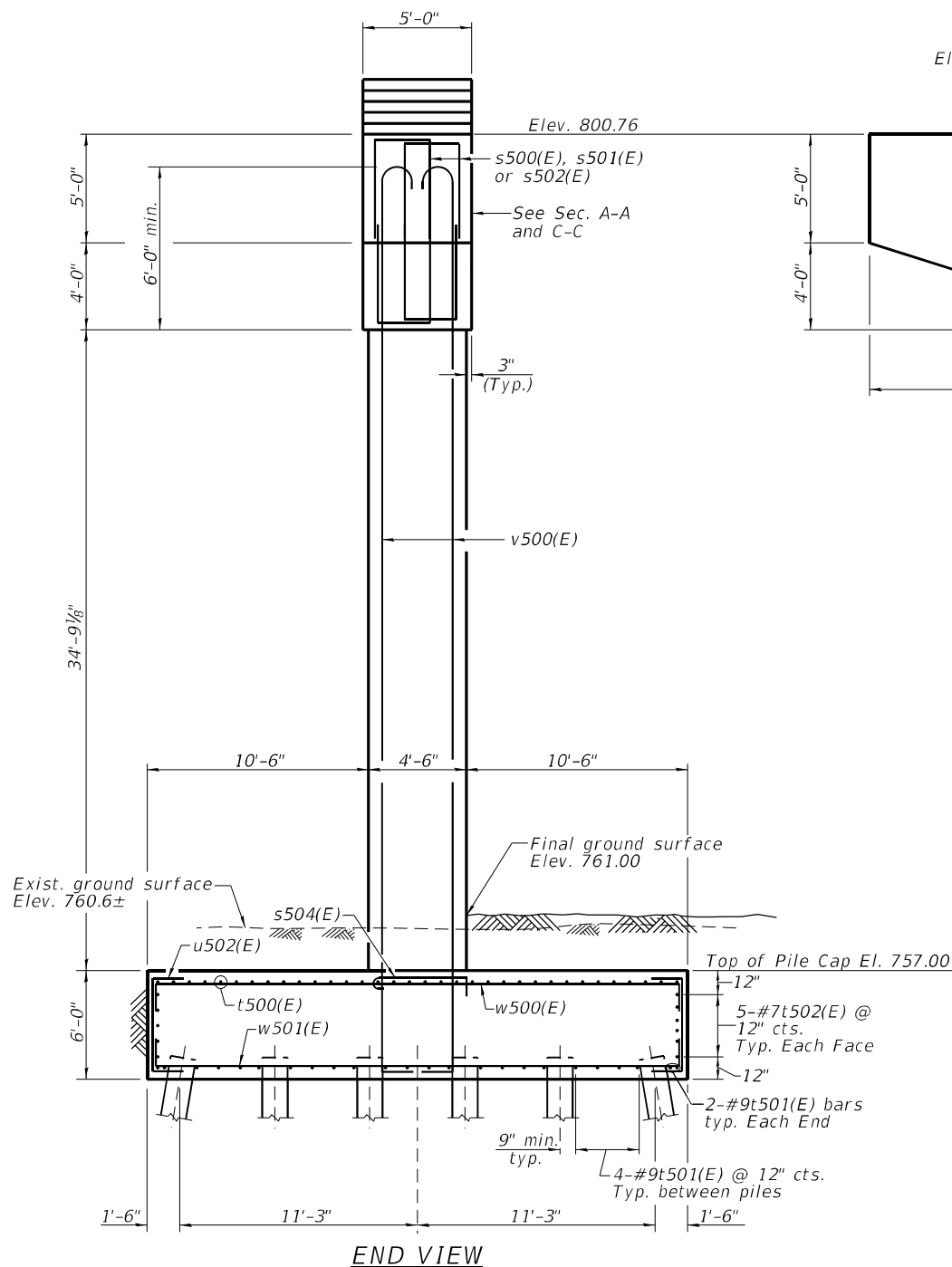
Bar	No.	Size	Length	Shape
h500(E)	12	#6	44'-8"	
h501(E)	2	#6	43'-3"	
h502(E)	2	#6	36'-0"	
h503(E)	2	#6	28'-9"	
h504(E)	2	#6	21'-6"	
h505(E)	24	#6	6'-8"	
h506(E)	12	#6	8'-2"	
h507(E)	208	#5	15'-8"	
p500(E)	9	#10	48'-4"	
p501(E)	9	#10	44'-8"	
p502(E)	12	#8	26'-7"	
s500(E)	20	#5	11'-11"	
s501(E)	80	#5	14'-5"	
s502(E)	168	#5	16'-5"	
s504(E)	1,140	#5	5'-6"	
s505(E)	190	#5	17'-0"	
t500(E)	26	#7	27'-6"	
t501(E)	24	#9	29'-2"	
t502(E)	10	#7	27'-3"	
u500(E)	16	#6	12'-2"	
u501(E)	45	#6	12'-4"	
u502(E)	104	#6	7'-7"	
u503(E)	208	#5	10'-6"	
v500(E)	54	#11	50'-1"	
w500(E)	26	#7	27'-6"	
w501(E)	39	#11	29'-2"	
w502(E)	10	#7	27'-3"	
Structure Excavation		Cu. Yd.	355	
Concrete Structures		Cu. Yd.	312.2	
Reinforcement Bars, Epoxy Coated		Pound	52,760	
Furnishing Metal Shell Piles 16"x.312"		Foot	2,345	
Driving Piles		Foot	2,345	
Test Pile metal Shell		Each	1	
Pile shoes		Each	36	

MIN. LAP LENGTH

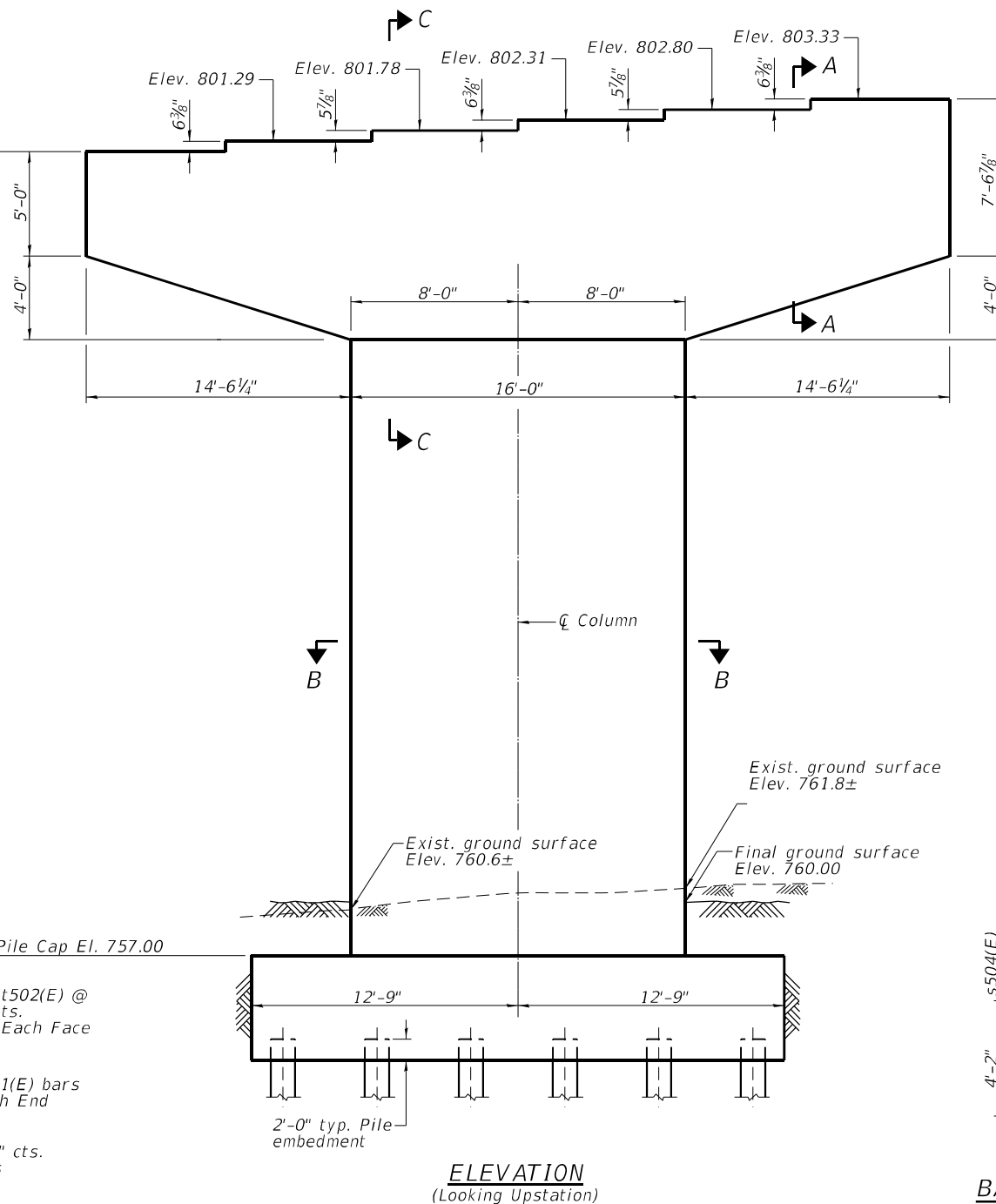
#5 bars: 3'-2"
#6 bars: 3'-10"

NOTES:

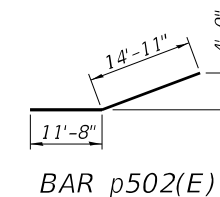
1. Pour steps monolithically with cap.
2. For Anchor Bolts Details see sheet S-48
3. See sheet S-80 of S-106 for Sections A-A, B-B & C-C
4. Pile cap concrete pour shall be in accordance with Article 1020.15, Heat of Hydration Control for Concrete Structures according to Standard Specifications. Cost included under "Concrete Structures."
5. Reinforcement bar bending dimensions are out to out.



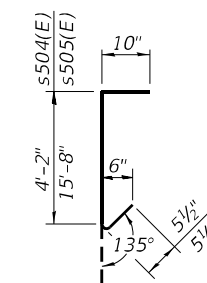
END VIEW



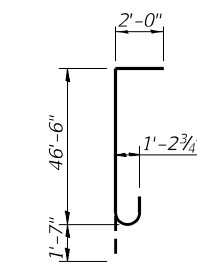
ELEVATION
(Looking Upstation)



BAR p502(E)



BAR s504(E) & s505(E)



BAR v500(E)

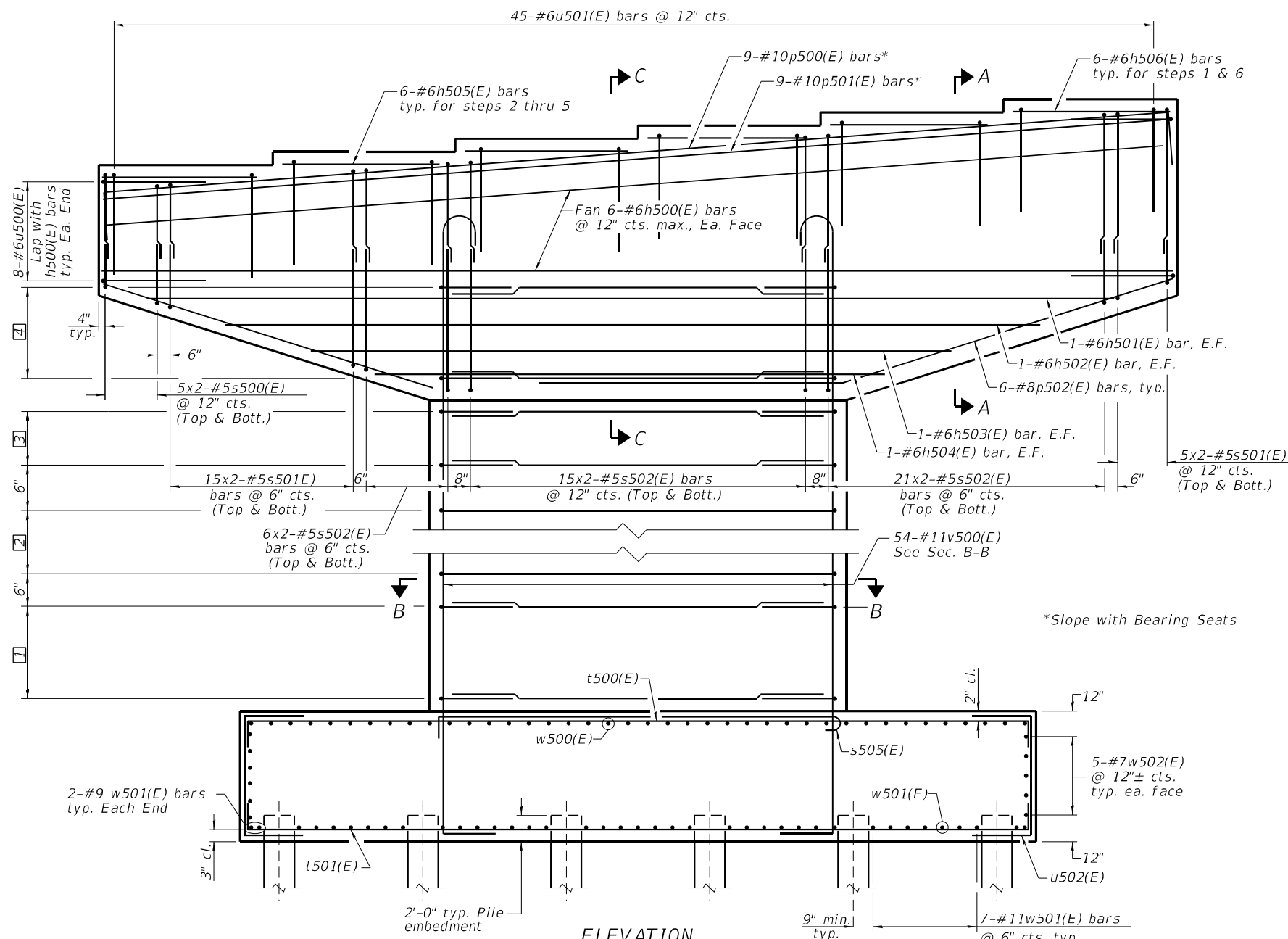
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		CHECKED - JDJ	REVISED -
		DRAWN - GLD	REVISED -
		CHECKED - LM	REVISED -
PLOT SCALE = NA	PLOT DATE = 05/03/2021		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

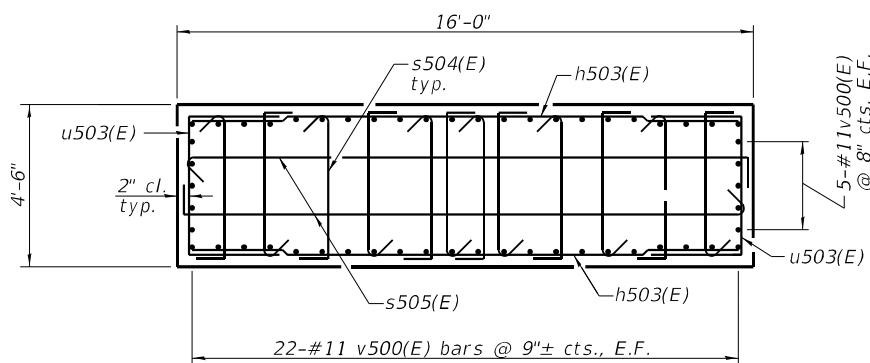
PIER 5 - PLAN AND ELEVATION
STRUCTURE NO. 010-1001

SHEET NO. S-79 OF S-106 SHEETS

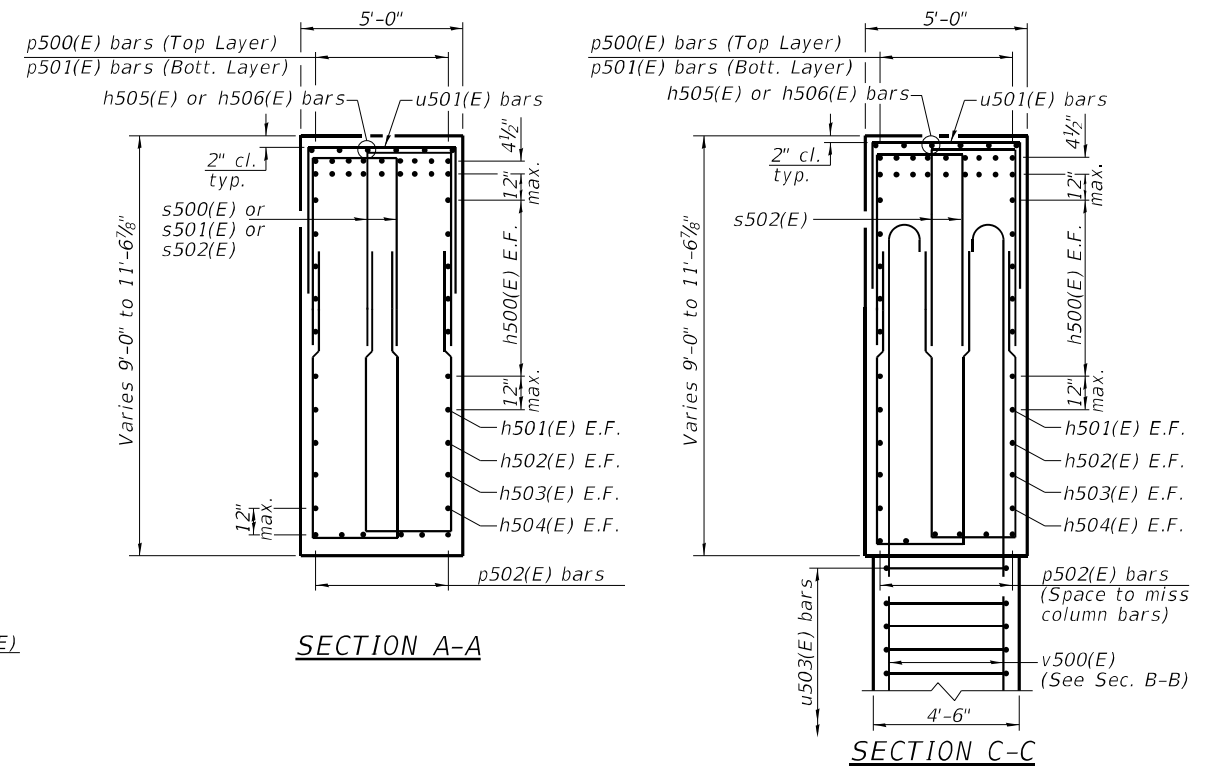
F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 721
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



- 1 25 Sets - 2-#5h507(E) bars spliced with 2-#5u503(E), 12-#5s504(E) & 2-#5s505(E) at 3" cts.
- 2 44 Sets - 2-#5h507(E) bars spliced with 2-#5u503(E), 12-#5s504(E) & 2-#5s505(E) at 6"± cts.
- 3 25 Sets - 2-#5h507(E) bars spliced with 2-#5u503(E), 12-#5s504(E) & 2-#5s505(E) at 3" cts.
- 4 10 Sets - 2-#5h507(E) bars spliced with 2-#5u503(E) at 3" cts.

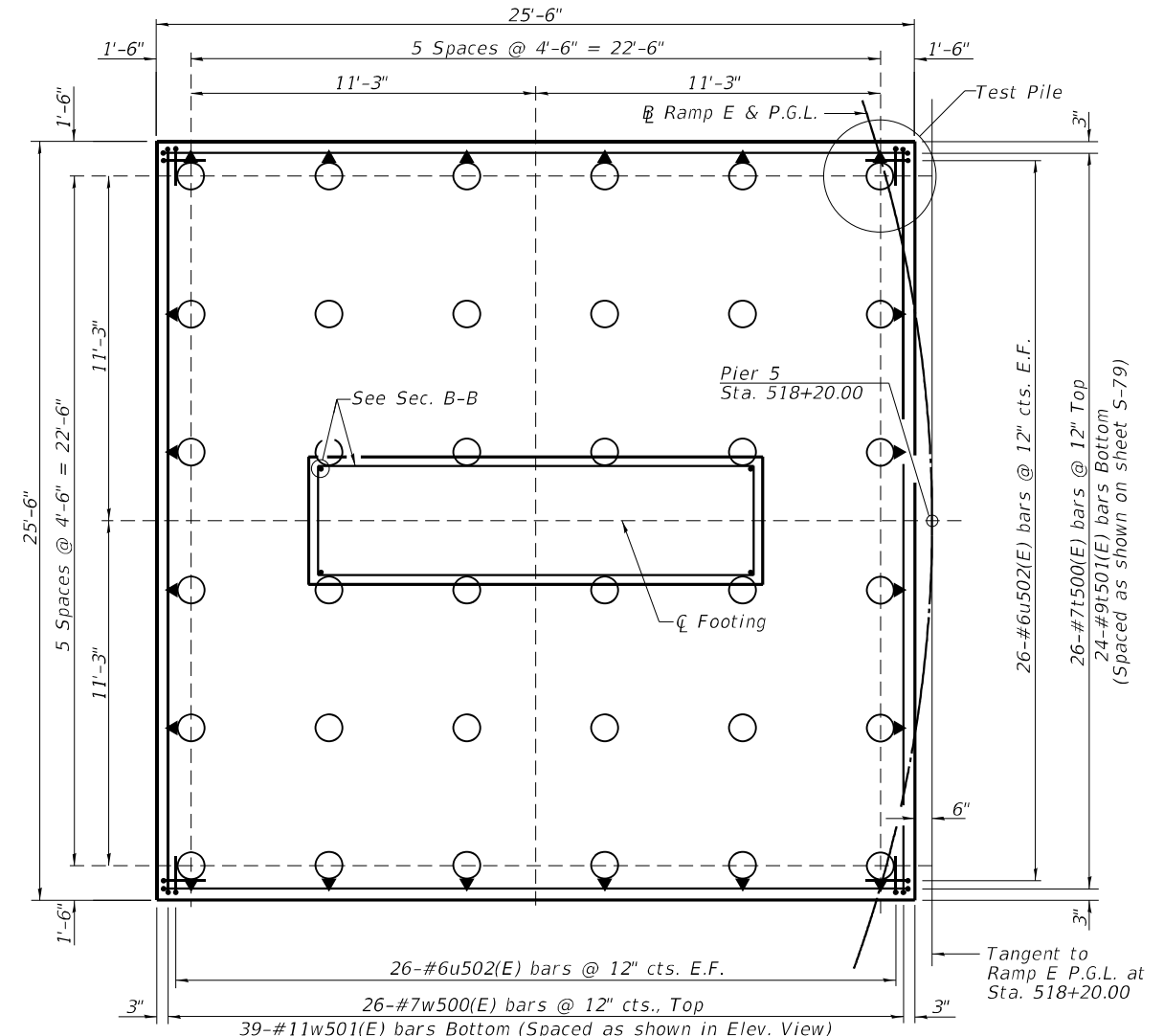


SECTION B-B



SECTION A-A

SECTION C-C



FOOTING PLAN

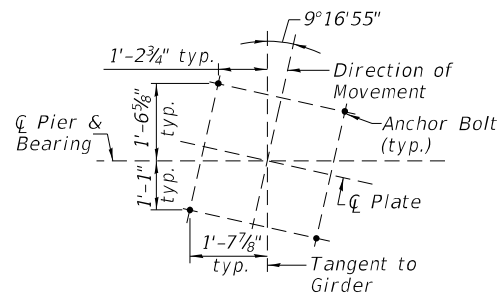
NOTES:

1. Space reinforcement in cap to miss anchor bolts.
2. 4:12 Batter on outside piles.
3. A single layer of cross ties s504(E) & s505(E) shall be provided across the top layer of footing reinforcement.
4. Bars noted thus, 3x2-#5 indicates 3 lines of bars with 2 lengths of bars per line.

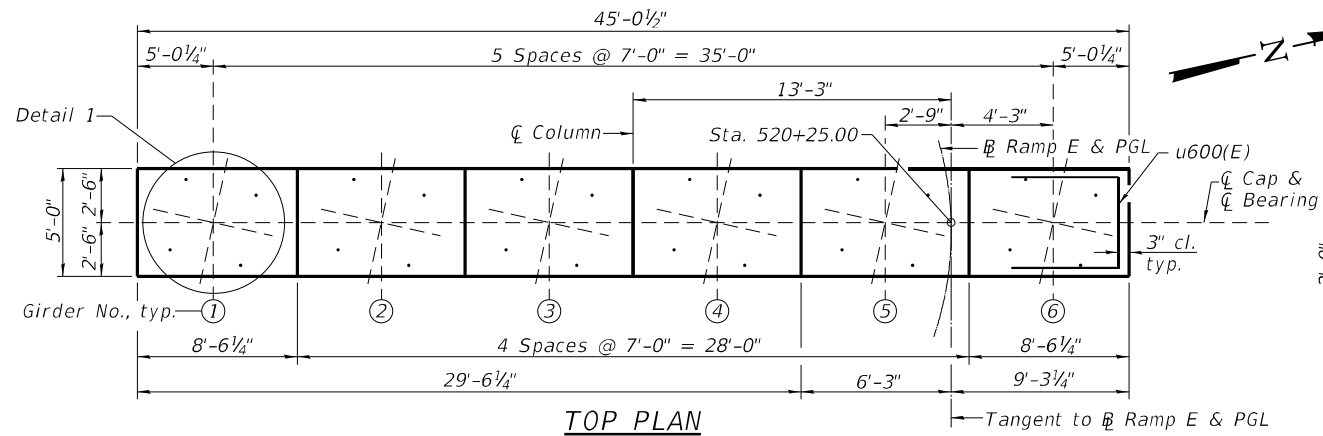
PILE DATA

Type: Steel Metal Shell, 16"Ø w/.312" walls with pile shoes
 Nominal Required Bearing: 473 kips
 Factored Resistance Available: 260 kips
 Est. Length: 67'
 No. Production Piles: 35
 No. Test Piles: 1

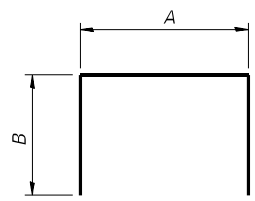
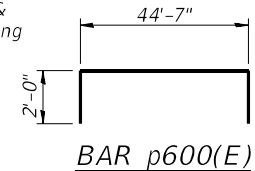
CMT <small>License No. 184-008613 Copyright CMT, Inc.</small>	USER NAME = Denise Herrera PLOT SCALE = NA PLOT DATE = 05/03/2021	DESIGNED - LM CHECKED - JDJ DRAWN - GLD CHECKED - LM	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PIER 5 DETAILS STRUCTURE NO. 010-1001	F.A.I. R.T.E. 74 & 57 SECTION (10-34-1) HBK COUNTY CHAMPAIGN TOTAL SHEETS 1187 SHEET NO. 722 CONTRACT NO. 70B99	SHEET NO. S-80 OF S-106 SHEETS ILLINOIS FED. AID PROJECT



DETAIL 1
Bearing Orientation
(Typ. at Each Girder)



TOP PLAN

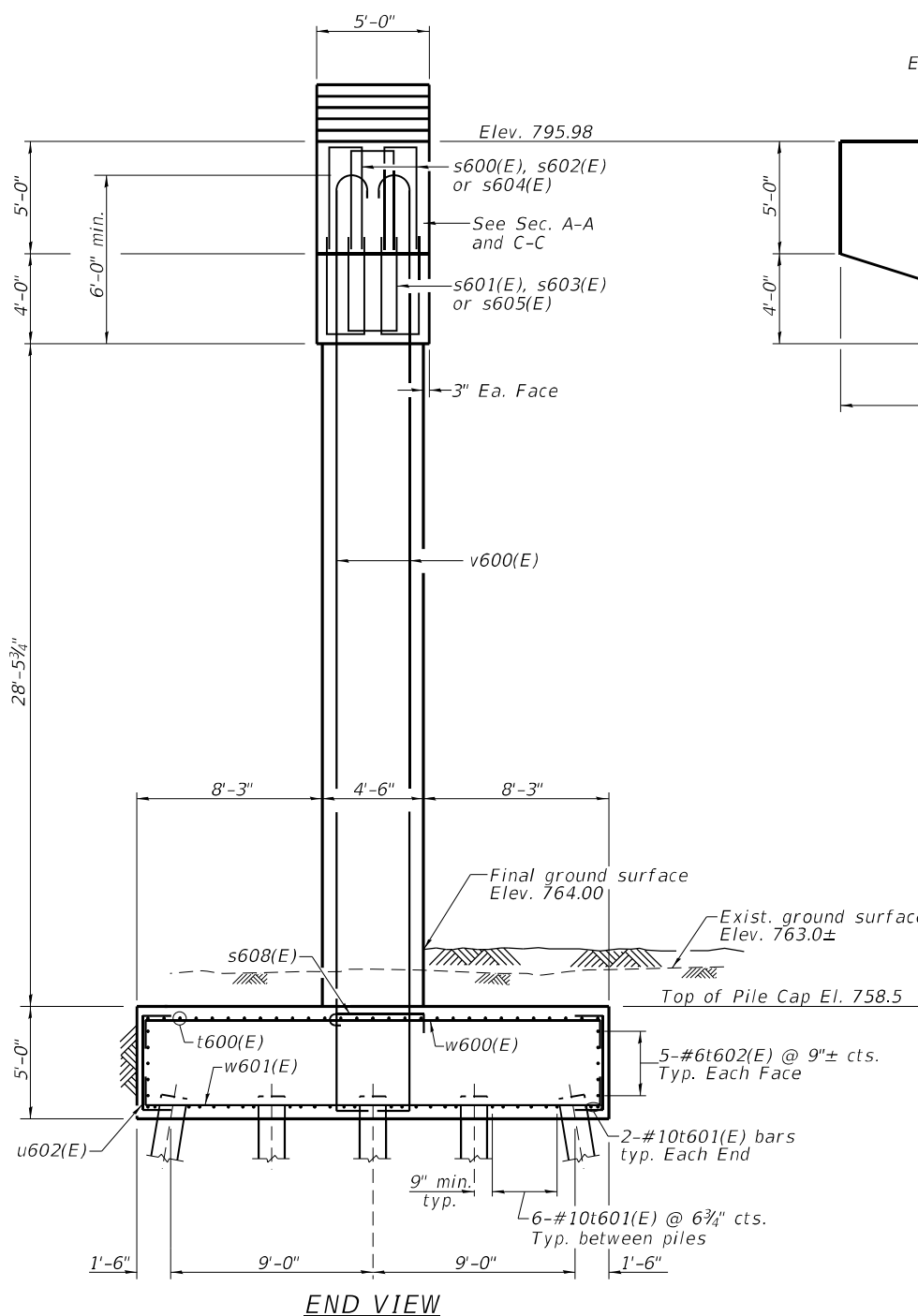


A & B DIMENSIONS

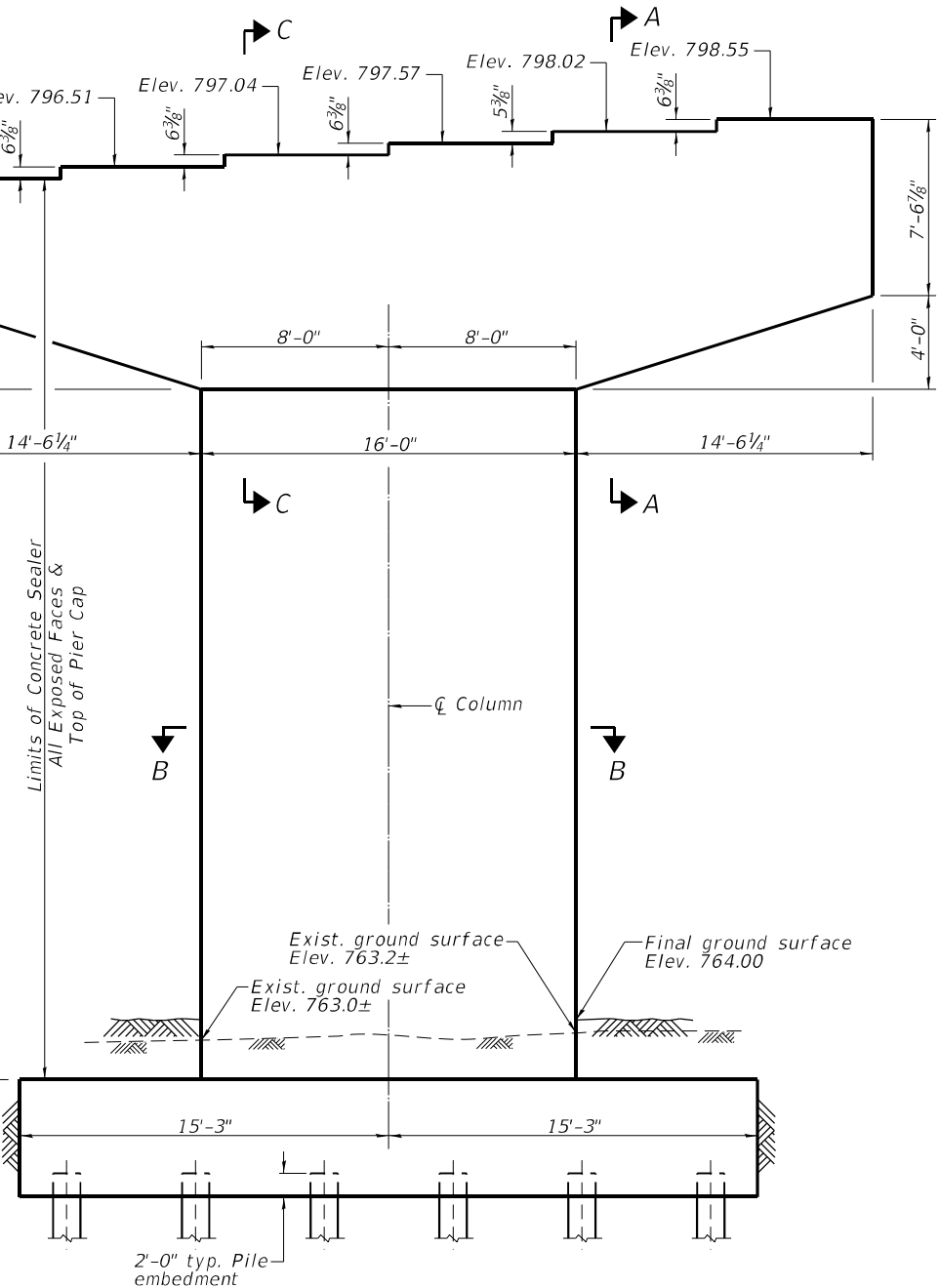
Bar	A	B
s600(E)	2'-0"	4'-4"
s601(E)	2'-6"	4'-4"
s602(E)	2'-0"	5'-6"
s603(E)	2'-6"	5'-6"
s604(E)	2'-0"	6'-7"
s605(E)	2'-6"	6'-7"
s606(E)	3'-1"	6'-7"
t600(E)	30'-2"	1'-0"
t601(E)	30'-2"	1'-10"
t602(E)	29'-11"	1'-0"
u600(E)	4'-6"	3'-10"
u601(E)	4'-8"	3'-10"
u602(E)	4'-7"	1'-0"
u603(E)	4'-2"	3'-2"
w600(E)	20'-8"	1'-0"
w601(E)	20'-8"	1'-10"
w602(E)	20'-5"	1'-0"

BILL OF MATERIAL

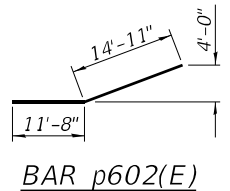
Bar	No.	Size	Length	Shape
h600(E)	12	#6	44'-8"	
h601(E)	2	#6	43'-2"	
h602(E)	2	#6	36'-0"	
h603(E)	2	#6	28'-9"	
h604(E)	2	#6	21'-6"	
h605(E)	24	#6	6'-8"	
h606(E)	12	#6	8'-2"	
h607(E)	176	#5	15'-8"	
p600(E)	9	#11	48'-7"	
p601(E)	9	#11	44'-8"	
p602(E)	12	#8	26'-7"	
s600(E)	20	#5	10'-8"	
s601(E)	10	#5	11'-2"	
s602(E)	64	#5	13'-0"	
s603(E)	32	#5	13'-6"	
s604(E)	132	#5	15'-2"	
s605(E)	66	#5	15'-8"	
s606(E)	56	#5	16'-3"	
s608(E)	948	#5	5'-6"	
s609(E)	158	#5	17'-0"	
t600(E)	29	#6	32'-2"	
t601(E)	28	#10	33'-10"	
t602(E)	10	#6	31'-11"	
u600(E)	16	#6	12'-2"	
u601(E)	45	#6	12'-4"	
u602(E)	106	#6	6'-7"	
u603(E)	176	#5	10'-6"	
v600(E)	54	#11	42'-10"	
w600(E)	42	#6	22'-8"	
w601(E)	49	#10	24'-4"	
w602(E)	10	#6	22'-5"	
Structure Excavation	Cu. Yd.		304	
Concrete Structures	Cu. Yd.		269.7	
Reinforcement Bars, Epoxy Coated	Pound		53,030	
Furnishing Metal Shell Piles 16"x.312"	Foot		1,914	
Driving Piles	Foot		1,914	
Test Pile metal Shell	Each		1	
Pile shoes	Each		30	
Concrete Sealer	Sq. Ft.		2,632	



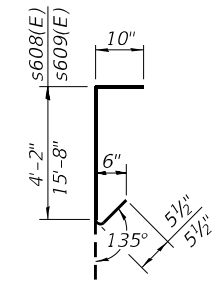
END VIEW



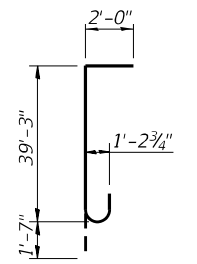
ELEVATION
(Looking Upstation)



BAR p602(E)



BAR s608(E) & s609(E)



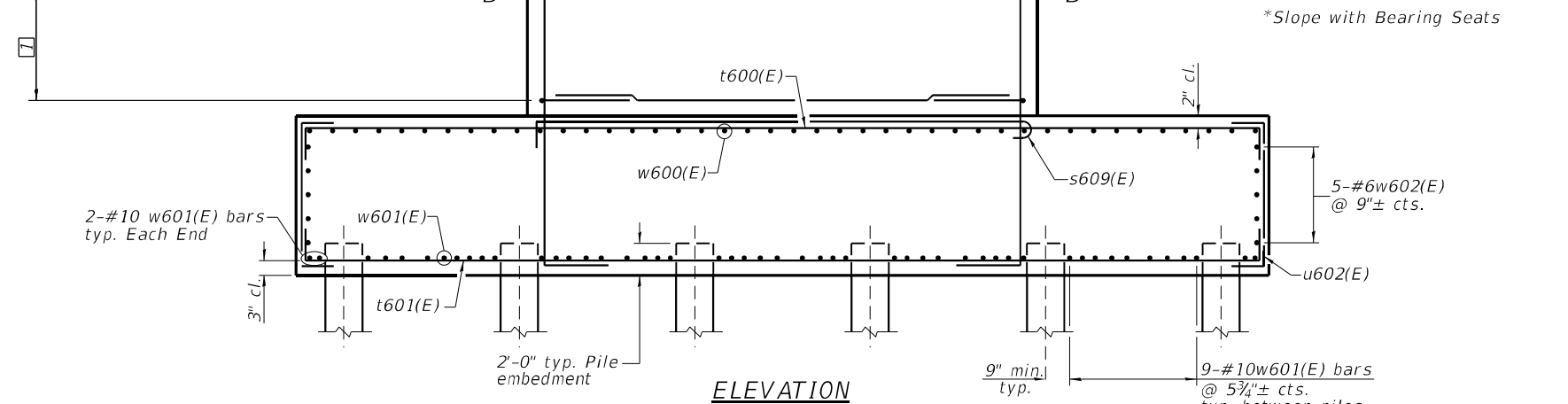
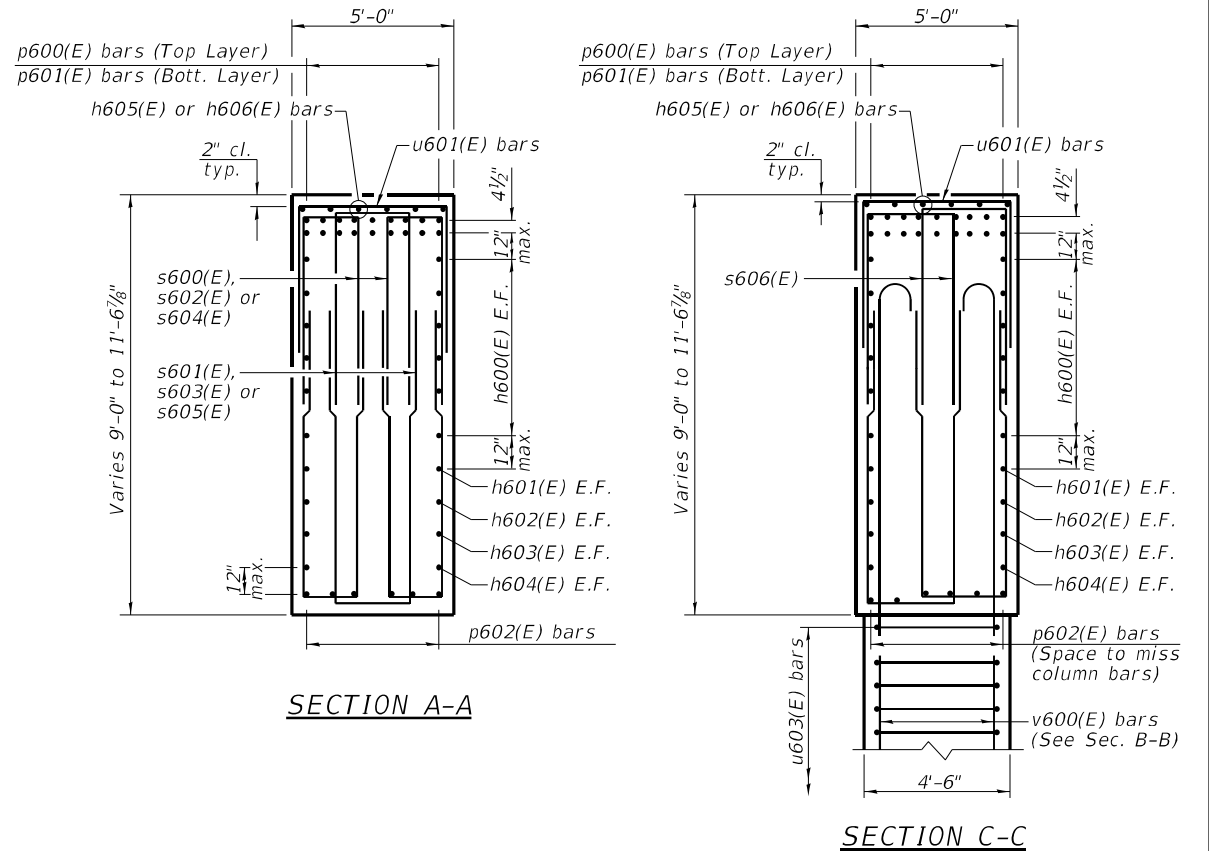
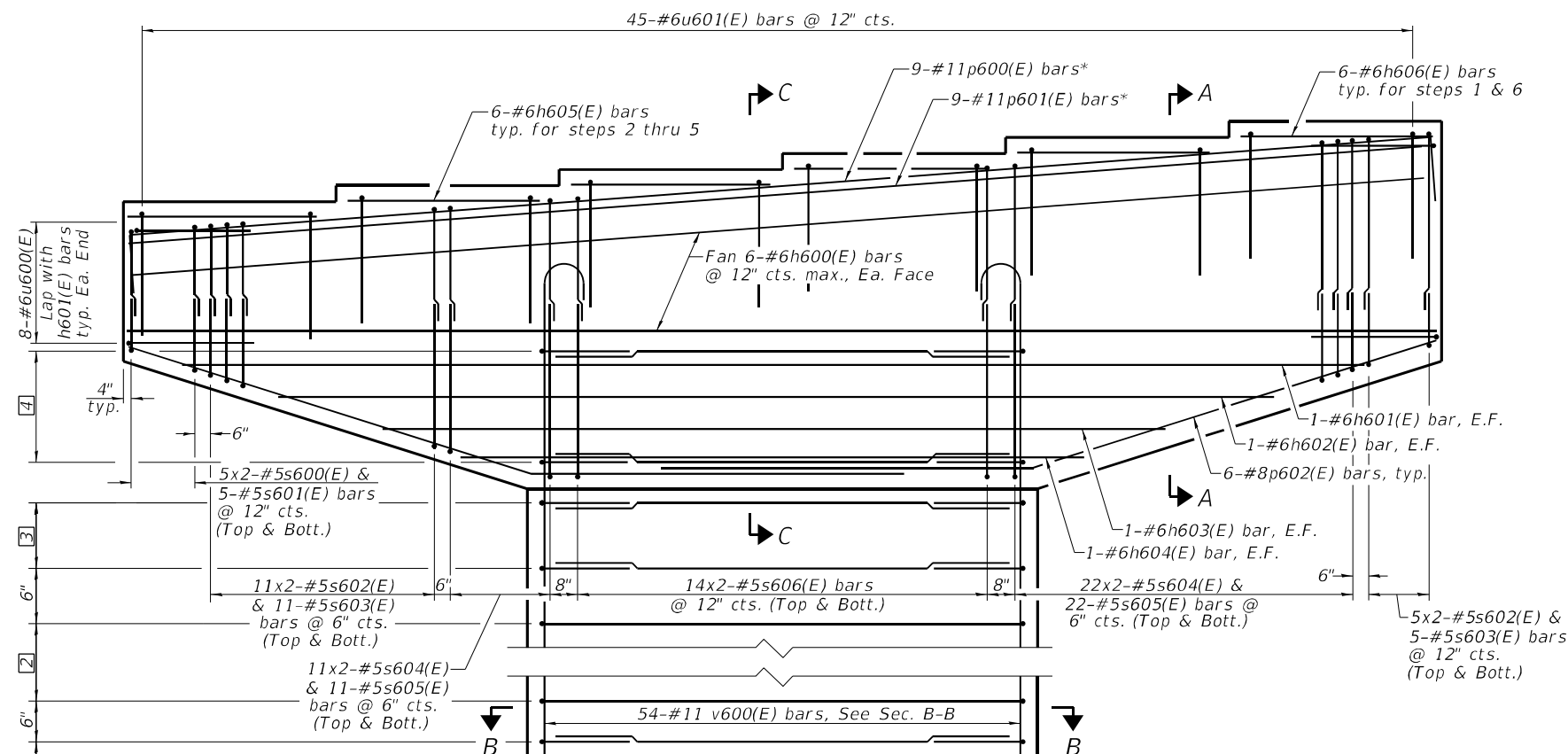
BAR v600(E)

MIN. LAP LENGTH

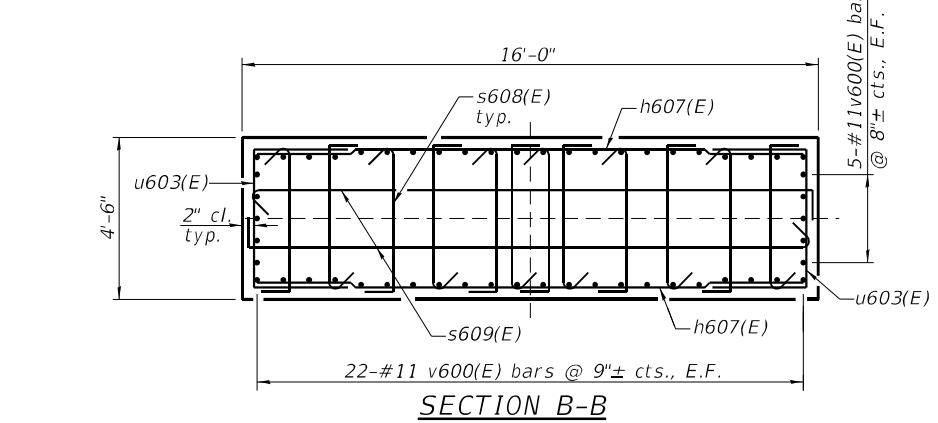
#5 bars: 3'-2"
#6 bars: 3'-10"

NOTES:

1. Pour steps monolithically with cap.
2. For Anchor Bolts Details see sheet S-46
3. See sheet S-82 of S-106 for Sections A-A, B-B & C-C
4. Reinforcement bar bending dimensions are out to out.

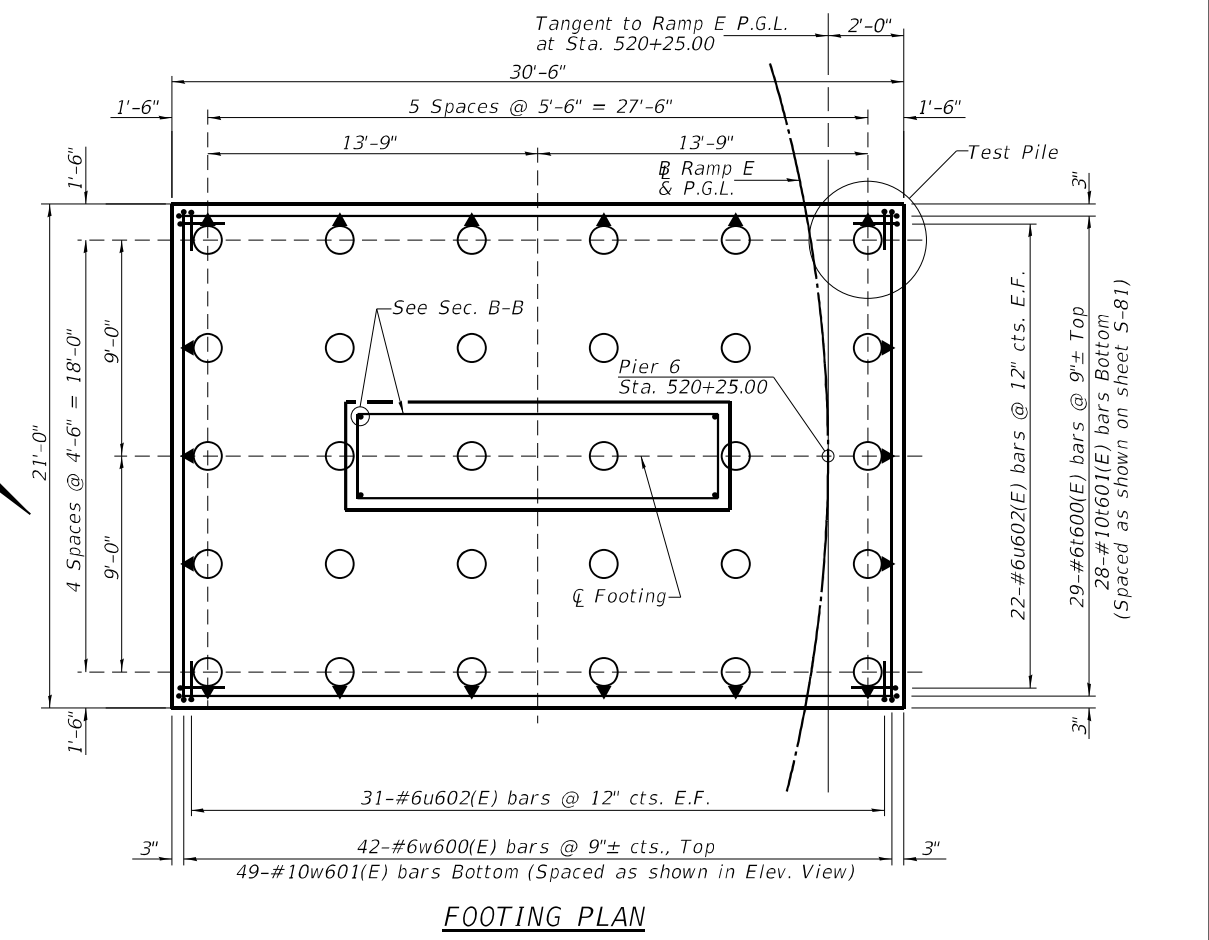


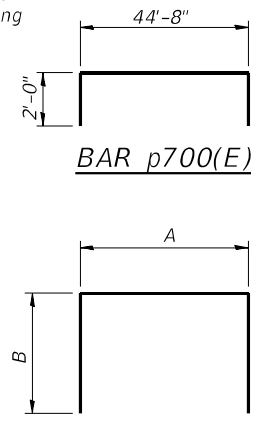
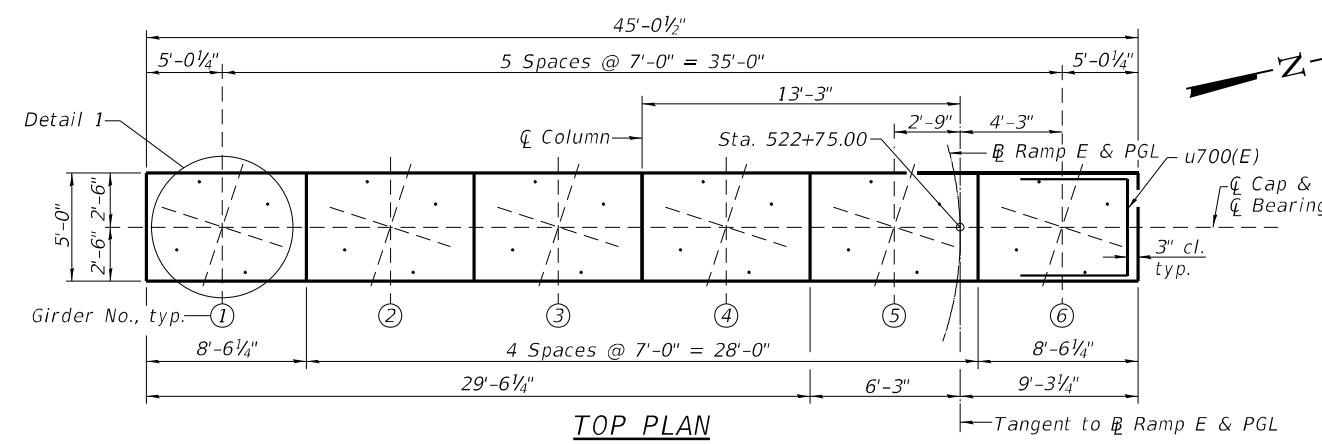
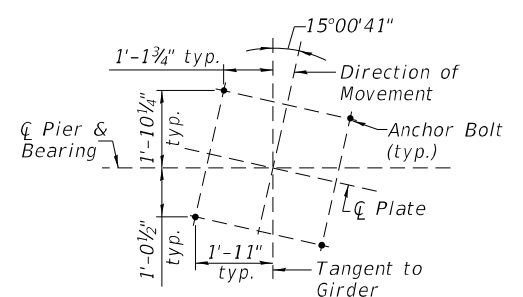
- 1 21 Sets - 2-#5h607(E) bars spliced with 2-#5u603(E), 12-#5s608(E) & 2-#5s609(E) at 3' cts.
- 2 36 Sets - 2-#5h607(E) bars spliced with 2-#5u603(E), 12-#5s608(E) & 2-#5s609(E) bars at 6' cts.
- 3 21 Sets - 2-#5h607(E) bars spliced with 2-#5u603(E), 12-#5s608(E), & 2-#5s609(E) at 3' cts.
- 4 10 Sets - 2-#5h607(E) bars spliced with 2-#5u603(E) at 3' cts.



- NOTES:**
1. Space reinforcement in cap to miss anchor bolts.
 2. 4:12 Batter on outside piles.
 3. A single layer of cross ties s608(E) & s609(E) shall be provided across the top layer of footing reinforcement.
 4. Bars noted thus, 3x2-#5 indicates 3 lines of bars with 2 lengths of bars per line.

PILE DATA
 Type: Steel Metal Shell, 16"Ø w/.312" walls with pile shoes
 Nominal Required Bearing: 453 kips
 Factored Resistance Available: 349 kips
 Est. Length: 66'
 No. Production Piles: 29
 No. Test Piles: 1



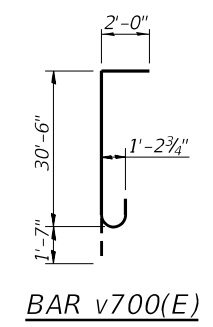
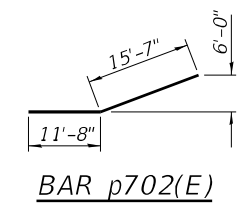
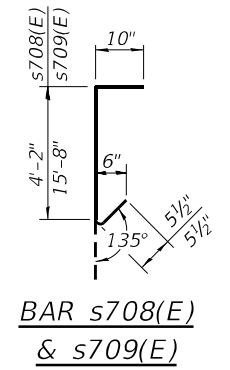
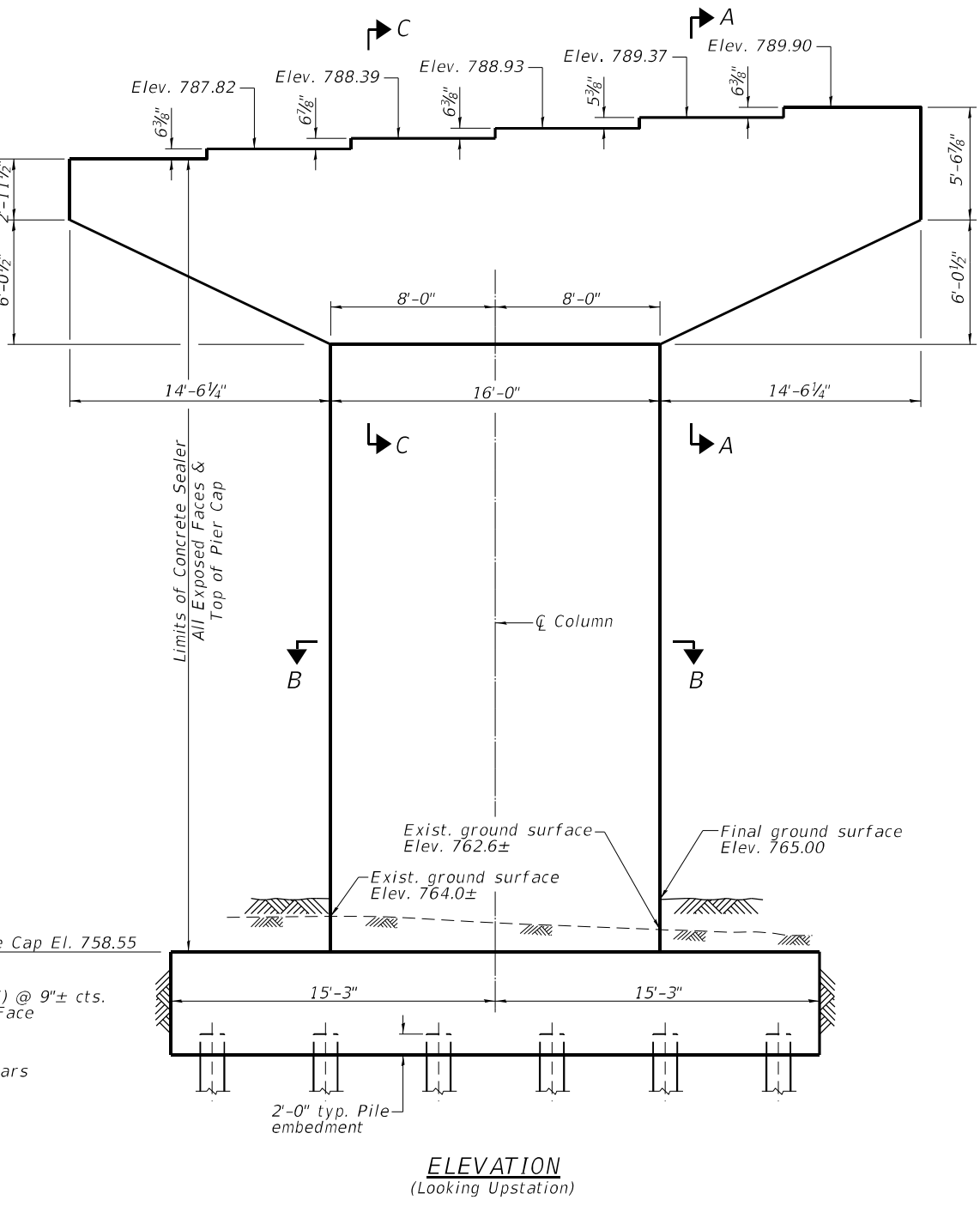
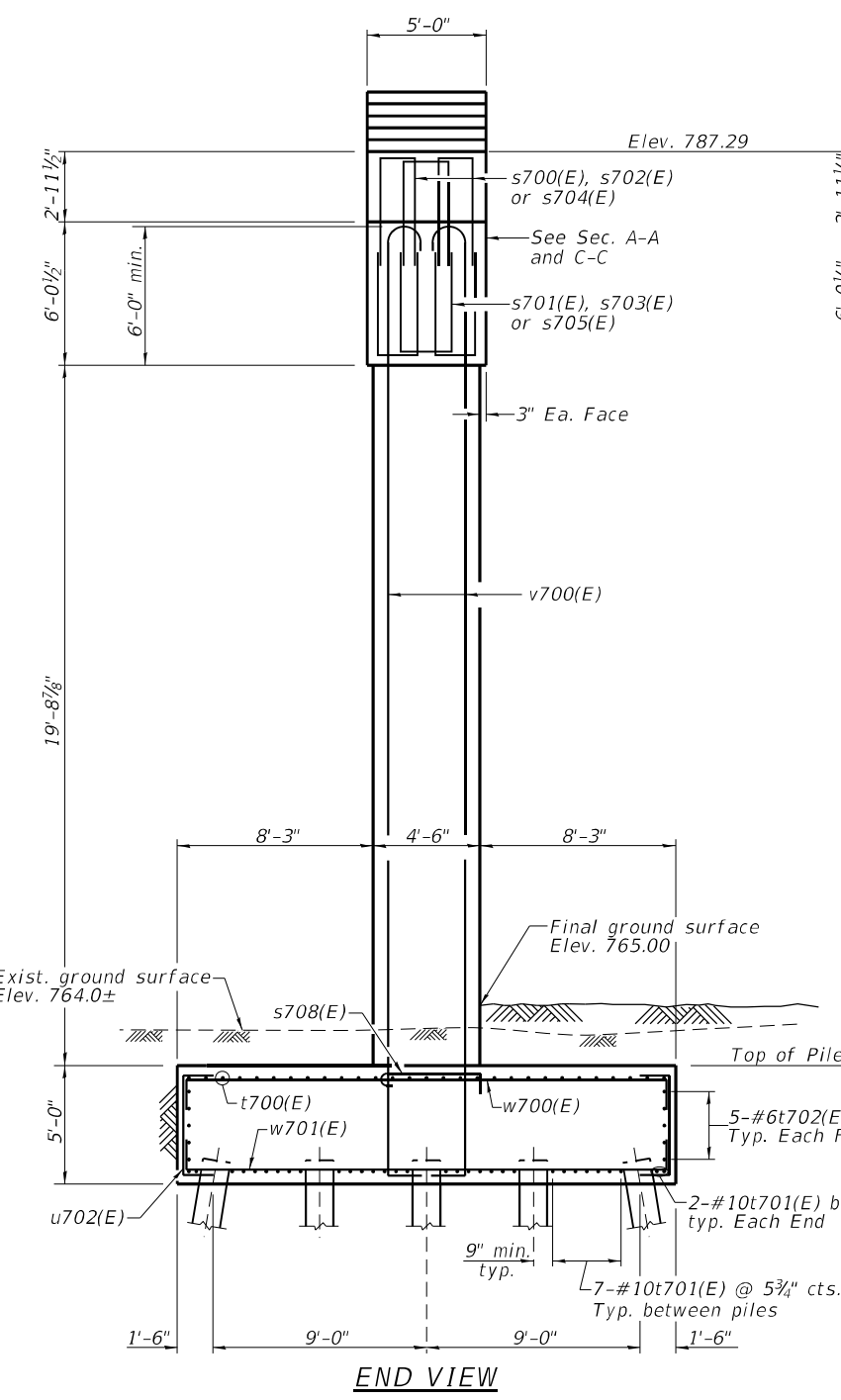


BARS
A & B DIMENSIONS

Bar	A	B
s700(E)	2'-0"	3'-8"
s701(E)	2'-6"	3'-8"
s702(E)	2'-0"	5'-0"
s703(E)	2'-6"	5'-0"
s704(E)	2'-0"	6'-9"
s705(E)	2'-6"	6'-9"
s706(E)	3'-1"	6'-9"
t700(E)	30'-2"	1'-0"
t701(E)	30'-2"	1'-10"
t702(E)	29'-11"	1'-0"
u700(E)	4'-6"	3'-10"
u701(E)	4'-8"	3'-10"
u702(E)	4'-7"	1'-0"
u703(E)	4'-2"	3'-2"
w700(E)	20'-8"	1'-0"
w701(E)	20'-8"	1'-10"
w702(E)	20'-5"	1'-0"

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h700(E)	8	#6	44'-8"	▬
h701(E)	2	#6	43'-6"	▬
h702(E)	2	#6	40'-4"	▬
h703(E)	2	#6	35'-7"	▬
h704(E)	2	#6	30'-9"	▬
h705(E)	24	#6	6'-8"	▬
h706(E)	12	#6	8'-2"	▬
h707(E)	138	#5	15'-8"	▬
h708(E)	2	#6	26'-0"	▬
h709(E)	2	#6	21'-2"	▬
p700(E)	9	#11	48'-8"	▬
p701(E)	9	#11	44'-8"	▬
p702(E)	12	#8	27'-3"	▬
s700(E)	20	#5	9'-4"	▬
s701(E)	10	#5	9'-10"	▬
s702(E)	64	#5	12'-0"	▬
s703(E)	32	#5	12'-6"	▬
s704(E)	124	#5	15'-6"	▬
s705(E)	62	#5	16'-0"	▬
s706(E)	60	#5	16'-7"	▬
s708(E)	720	#5	5'-6"	▬
s709(E)	120	#5	17'-0"	▬
t700(E)	29	#6	32'-2"	▬
t701(E)	32	#10	33'-10"	▬
t702(E)	10	#6	31'-11"	▬
u700(E)	8	#6	12'-2"	▬
u701(E)	45	#6	12'-4"	▬
u702(E)	106	#6	6'-7"	▬
u703(E)	138	#5	10'-6"	▬
v700(E)	54	#11	34'-1"	▬
w700(E)	42	#6	22'-8"	▬
w701(E)	54	#10	24'-4"	▬
w702(E)	10	#6	22'-5"	▬
Structure Excavation		Cu. Yd.	342	
Concrete Structures		Cu. Yd.	241.2	
Reinforcement Bars, Epoxy Coated		Pound	48,240	
Furnishing Metal Shell Piles 16"x.312"		Foot	1,914	
Driving Piles		Foot	1,914	
Test Pile metal Shell		Each	1	
Pile shoes		Each	30	
Concrete Sealer		Sq. Ft.	2,202	

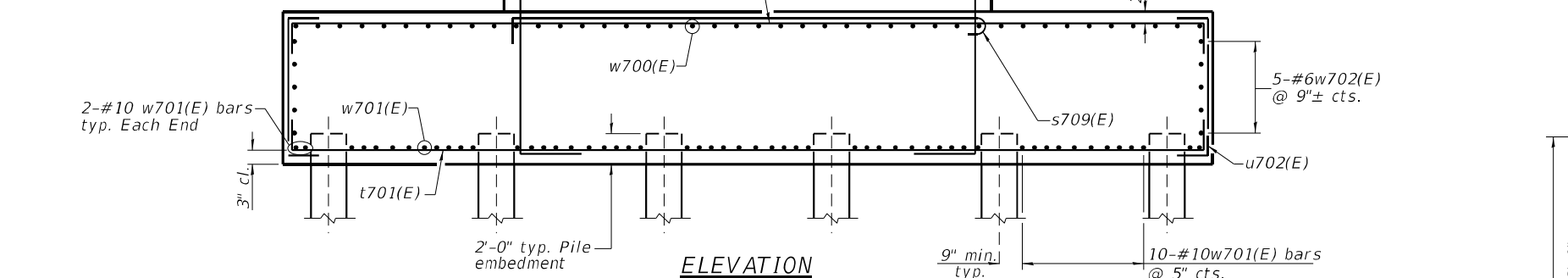
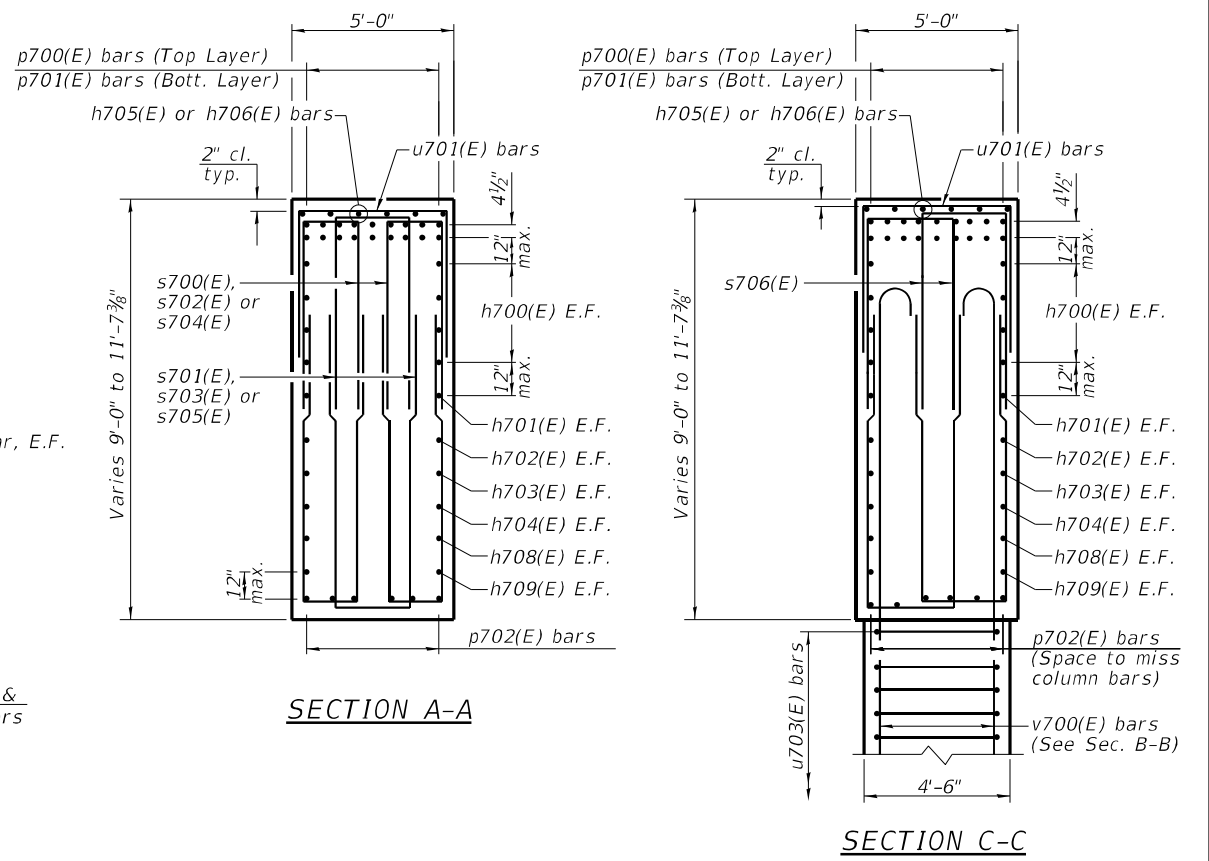
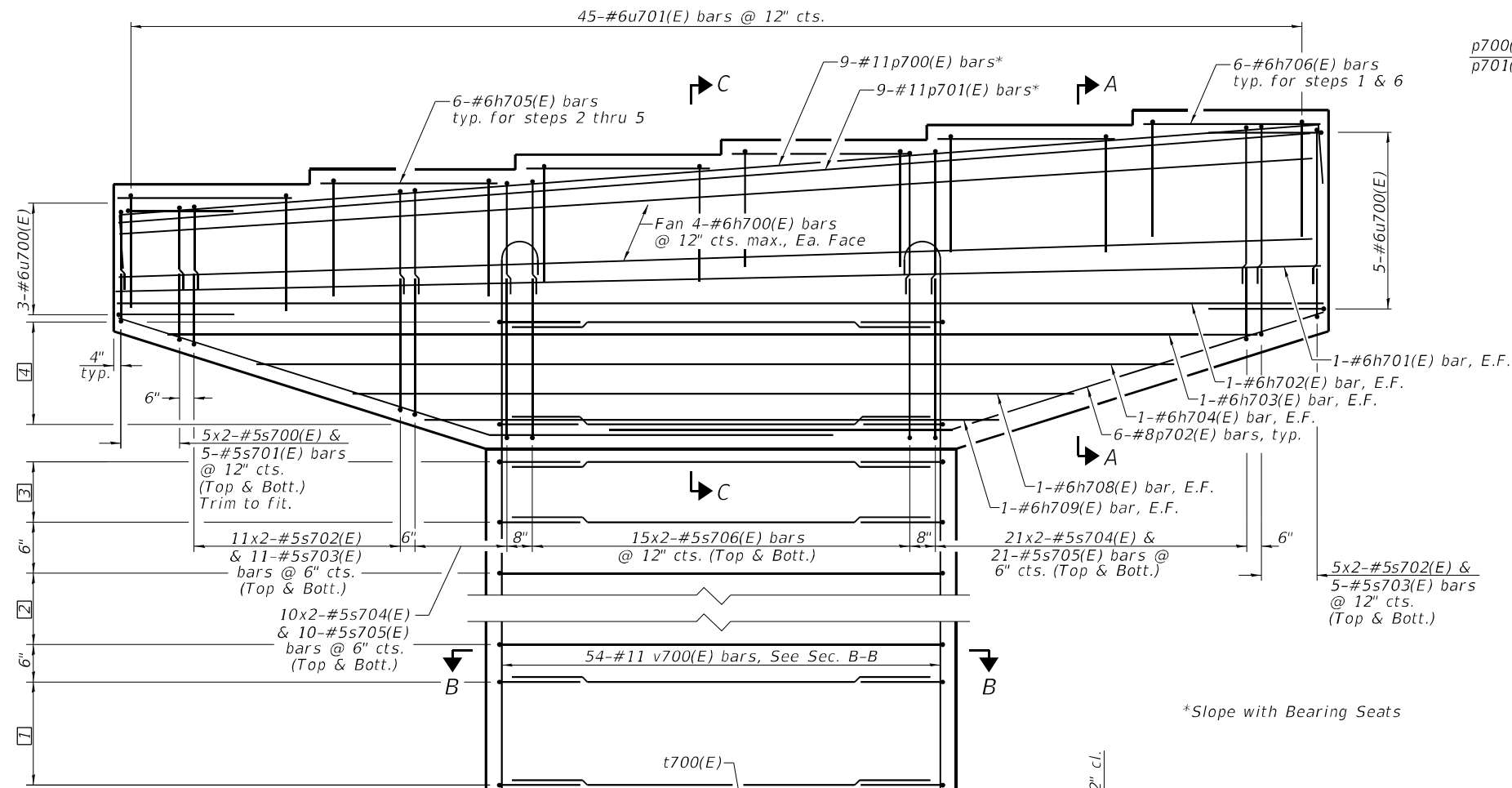


MIN. LAP LENGTH

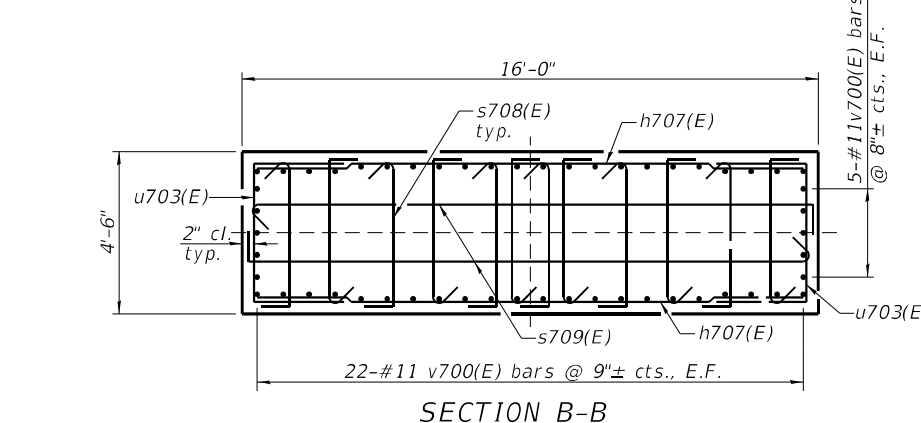
#5 bars: 3'-2"
#6 bars: 3'-10"

NOTES:

1. Pour steps monolithically with cap.
2. For Anchor Bolts Details see sheet S-46
3. See sheet S-84 of S-106 for Sections A-A, B-B & C-C
4. Reinforcement bar bending dimensions are out to out.



- 1 19 Sets - 2-#5h707(E) bars spliced with 2-#5u703(E), 12-#5s708(E) & 2-#5s709(E) at 3" cts.
- 2 21 Sets - 2-#5h707(E) bars spliced with 2-#5u703(E), 12-#5s708(E) & 2-#5s709(E) at 6"± cts.
- 3 19 Sets - 2-#5h707(E) bars spliced with 2-#5u703(E), 12-#5s708(E), & 2-#5s709(E) at 3" cts.
- 4 10 Sets - 2-#5h707(E) bars spliced with 2-#5u703(E) at 3" cts.

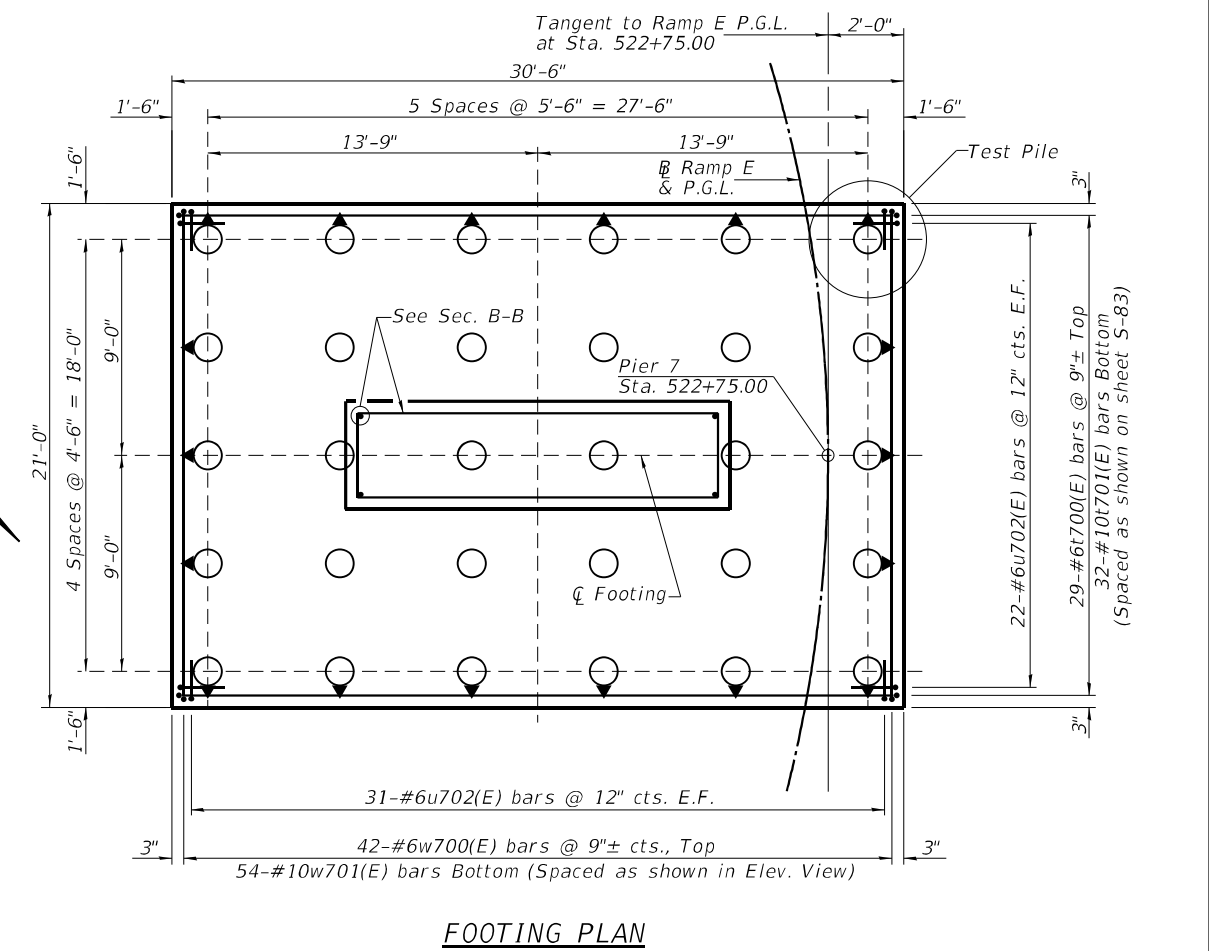


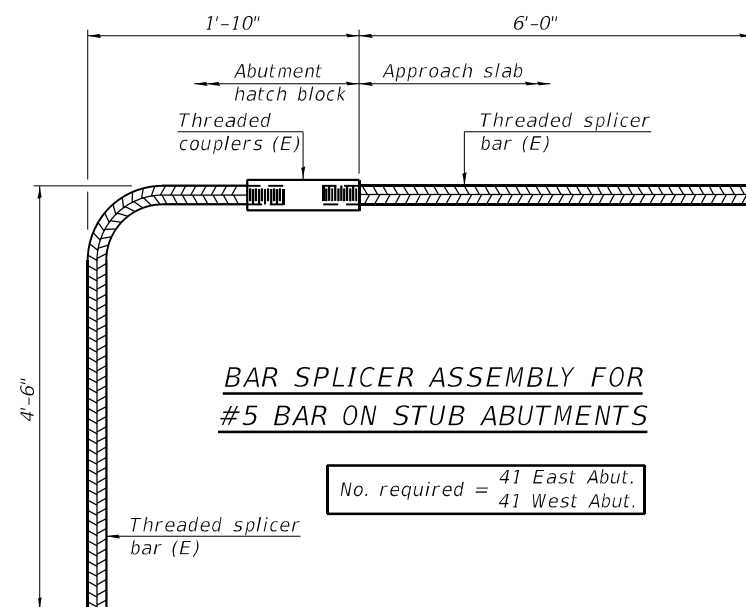
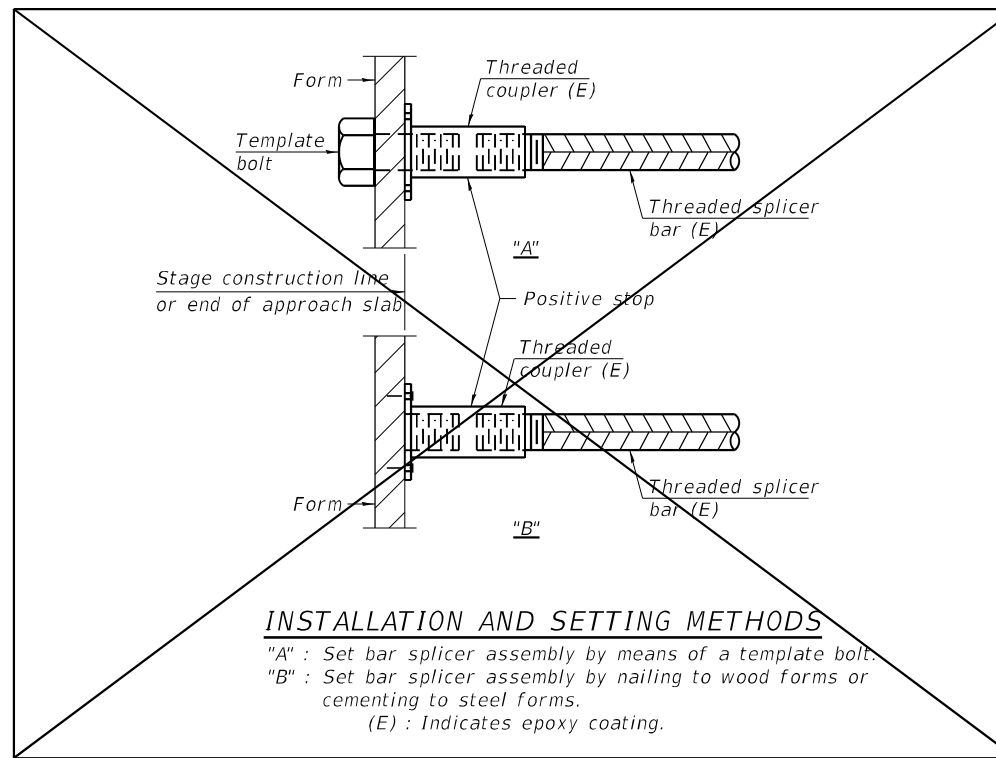
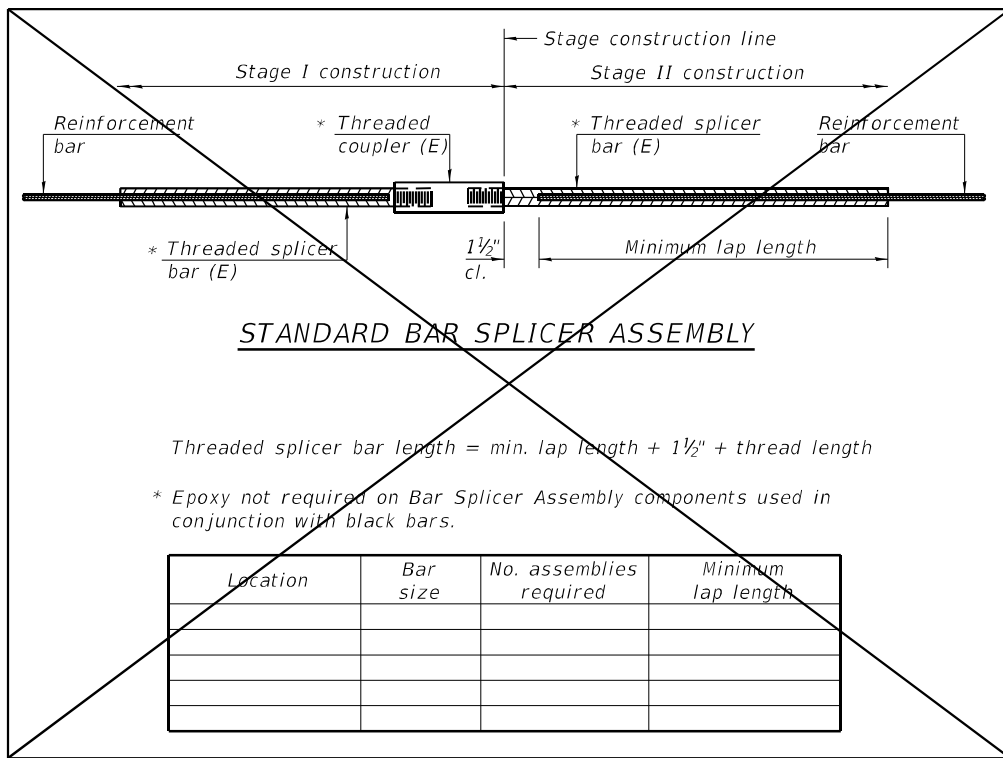
NOTES:

1. Space reinforcement in cap to miss anchor bolts.
2. 4:12 Batter on outside piles.
3. A single layer of cross ties s708(E) & s709(E) shall be provided across the top layer of footing reinforcement.
4. Bars noted thus, 3x2-#5 indicates 3 lines of bars with 2 lengths of bars per line.

PILE DATA

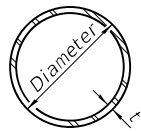
Type: Steel Metal Shell, 16"Ø w/.312" walls with pile shoes
 Nominal Required Bearing: 586 kips
 Factored Resistance Available: 322 kips
 Est. Length: 66'
 No. Production Piles: 29
 No. Test Piles: 1





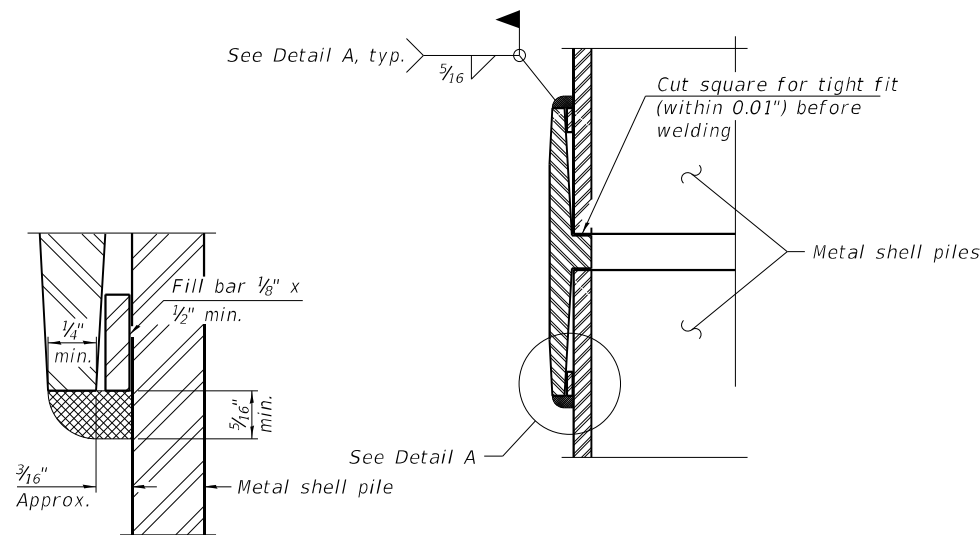
NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
 All reinforcement shall be lapped and tied to the splicer bars.
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
 See approved list of bar splicer assemblies and mechanical splicers for alternatives.

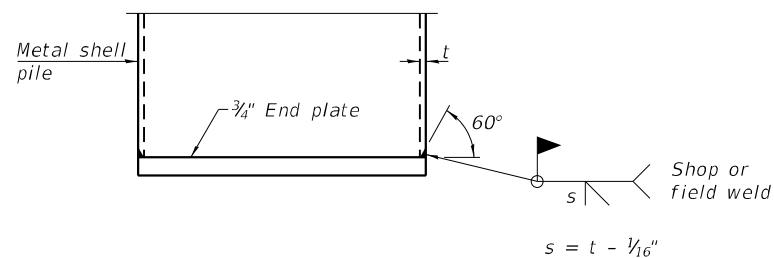


METAL SHELL PILE TABLE

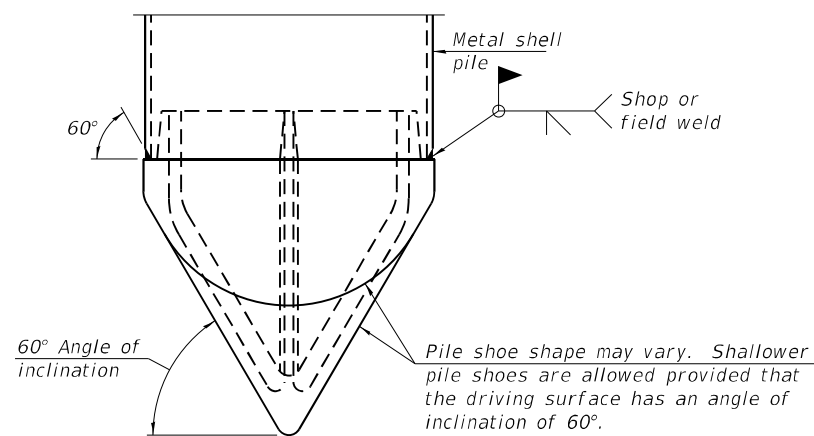
Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. ³ /ft.)
PP12	0.250"	31.37	0.0267
PP14	0.250"	36.71	0.0368
PP14	0.312"	45.61	0.0361
PP16	0.312"	52.32	0.0478
PP16	0.375"	62.64	0.0470



DETAIL A



END PLATE ATTACHMENT

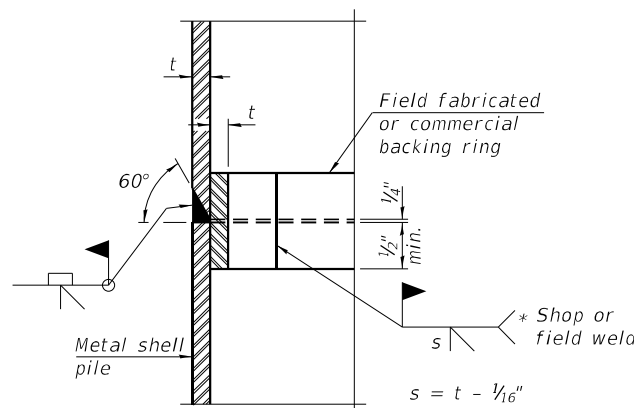


PILE SHOE ATTACHMENT

(When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A 148 Grade 80-50 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld).

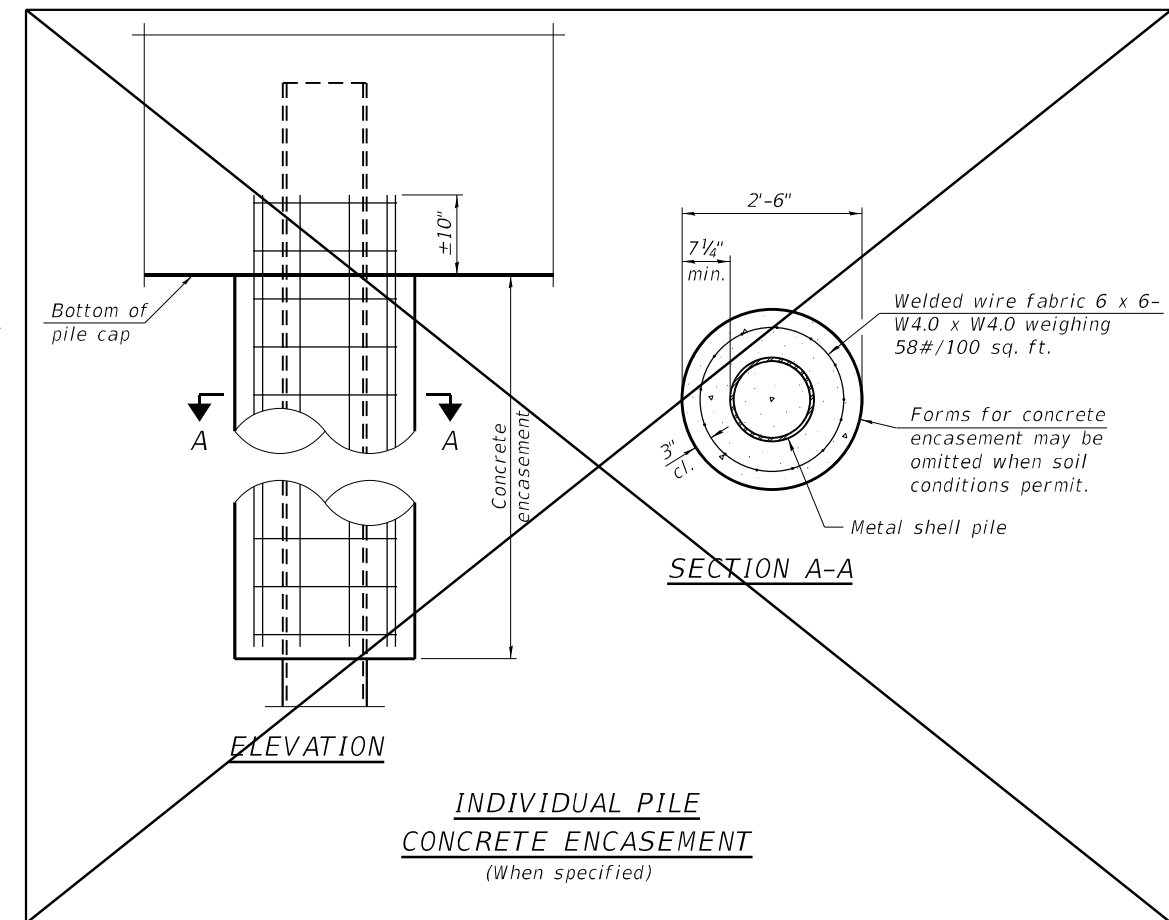
WELDED COMMERCIAL SPLICE

Notes:
The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them.
Pile segments shall be driven to solid contact with splicer before welding.

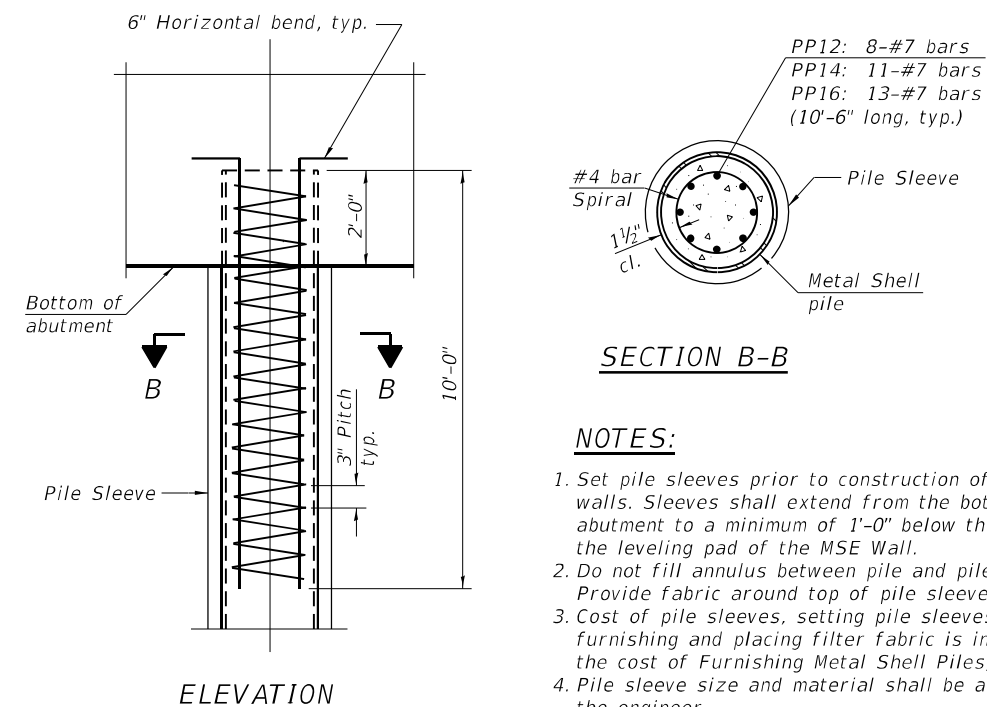


COMPLETE PENETRATION WELD SPLICE

* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.



INDIVIDUAL PILE CONCRETE ENCASEMENT
(When specified)

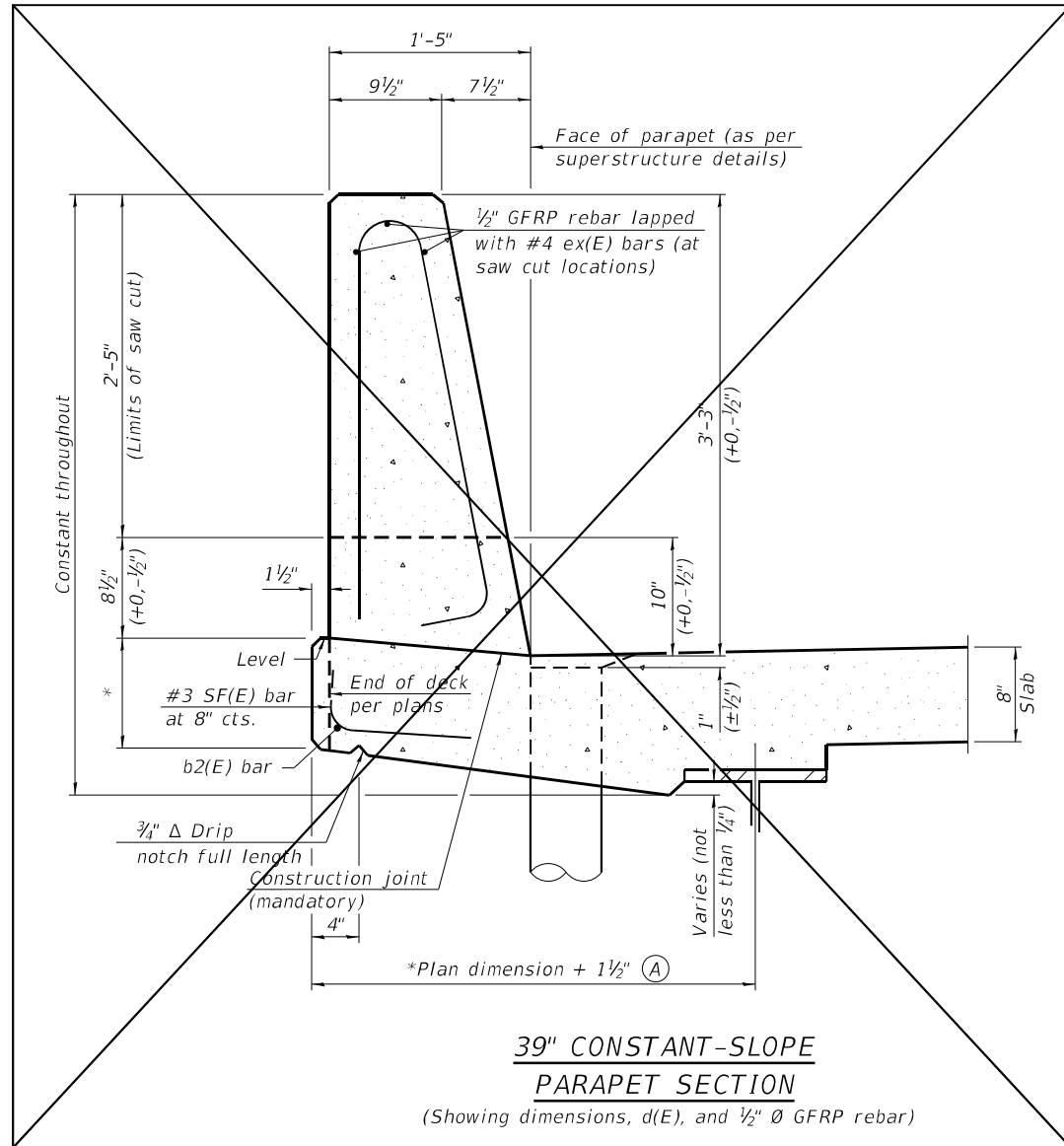


REINFORCEMENT AT ABUTMENTS

NOTES:

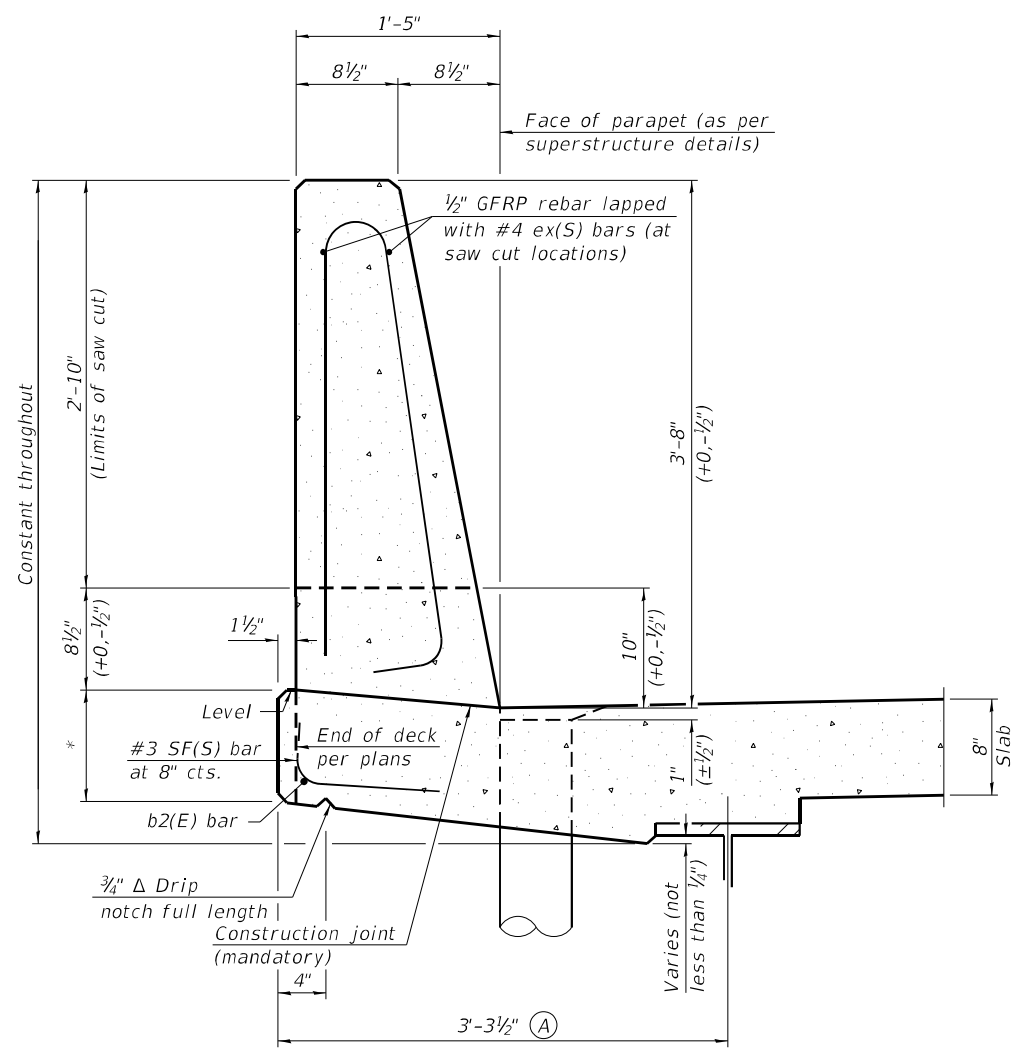
1. Set pile sleeves prior to construction of the MSE walls. Sleeves shall extend from the bottom of the abutment to a minimum of 1'-0" below the bottom of the leveling pad of the MSE Wall.
2. Do not fill annulus between pile and pile sleeve. Provide fabric around top of pile sleeve.
3. Cost of pile sleeves, setting pile sleeves, furnishing and placing filter fabric is included in the cost of Furnishing Metal Shell Piles, 12"x0.25".
4. Pile sleeve size and material shall be approved by the engineer.

Note:
The metal shell piles shall be according to Article 1006.05 of the Standard Specifications.



**39" CONSTANT-SLOPE
PARAPET SECTION**

(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)

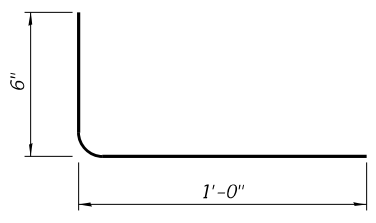


**44" CONSTANT-SLOPE
PARAPET SECTION**

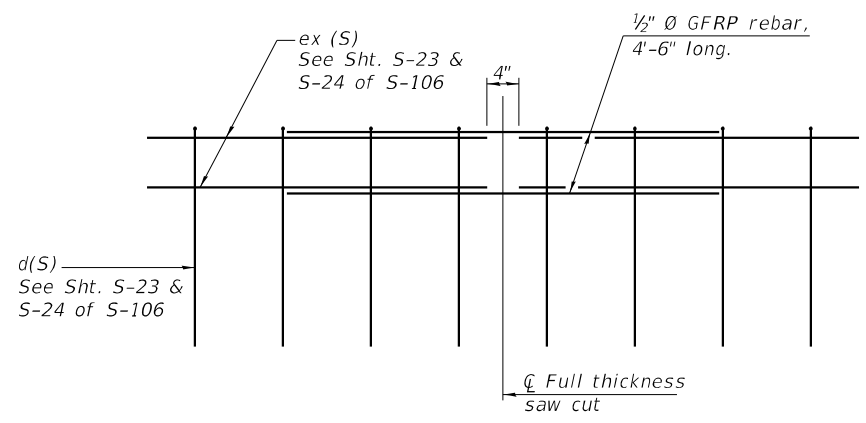
(Showing dimensions, d(S), and 1/2" Ø GFRP rebar)

Notes:
 All dimensions shall remain the same as shown on superstructure details, except dimension A which is to be revised as shown. Additional concrete needed to revise dimension A = 0.00348 cu. yds./ft. for 39" and 44" parapets.
 Place full depth aluminum sheets as shown on superstructure details.
 Replace all cork joint filler locations with a full thickness saw cut.
 Steel superstructure shown. Other superstructure types similar.

*See Superstructure Details.



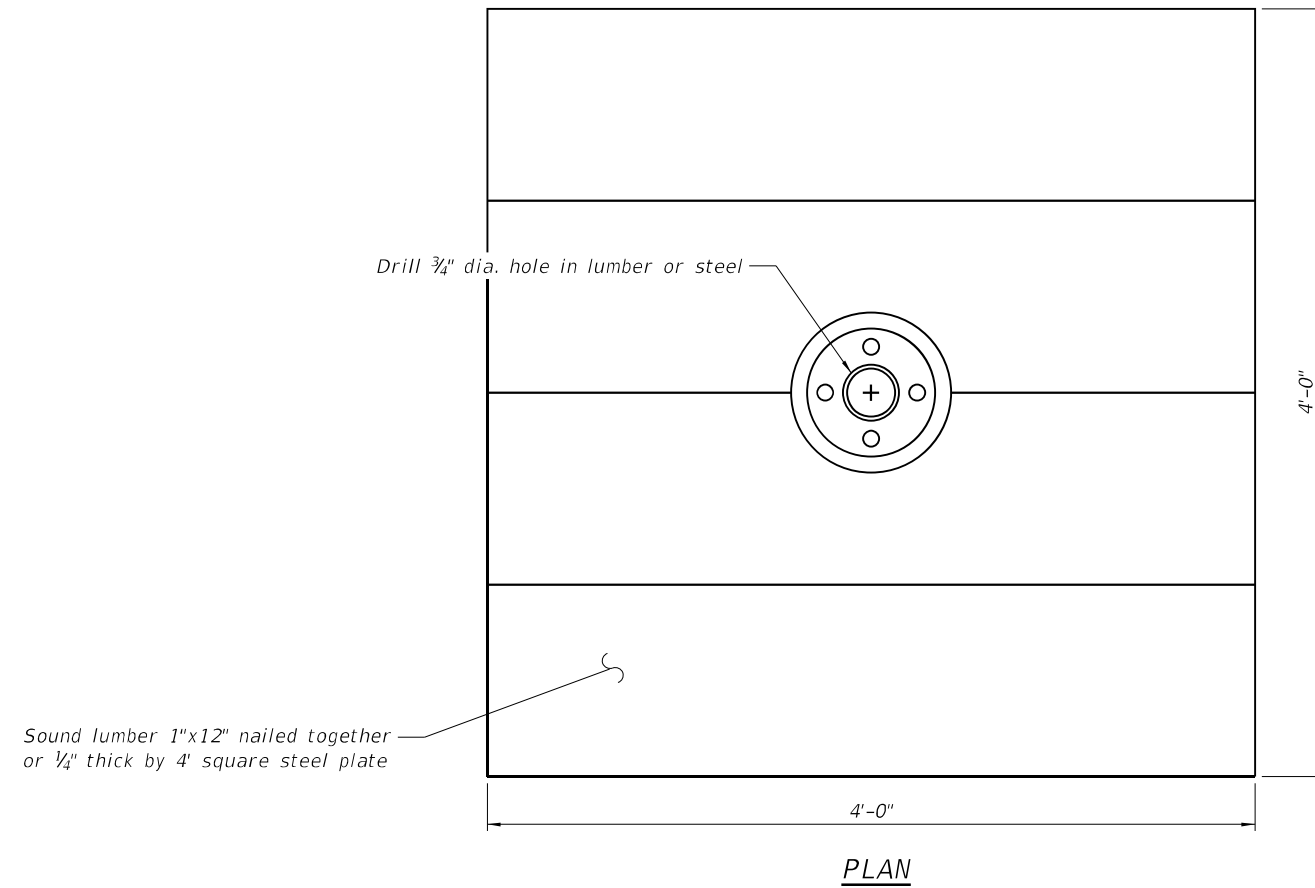
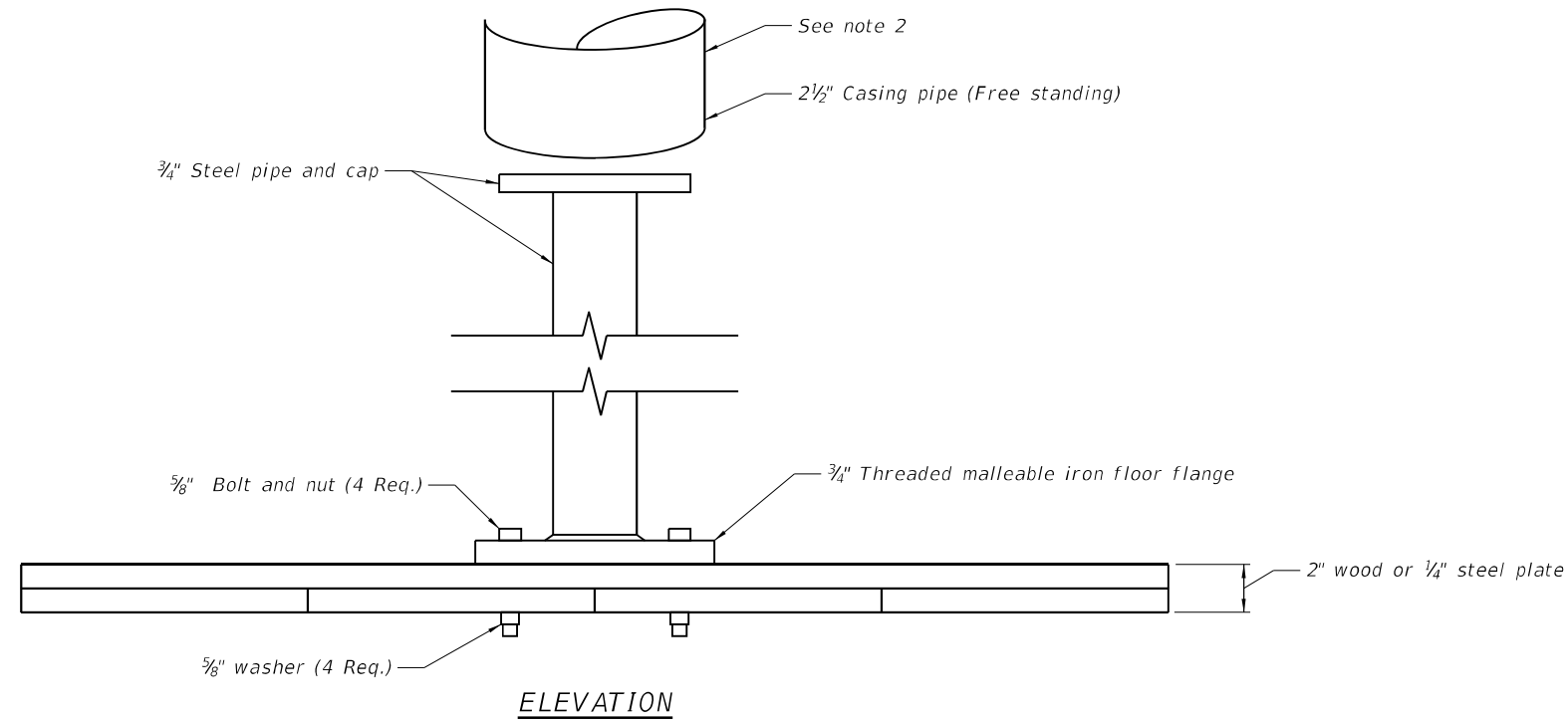
#3 SF (S) BAR



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)

FILE NAME = CMT <small>License No. 184-C00613 © Copyright CMT, Inc.</small>	USER NAME = Denise Herrera	DESIGNED - LM CHECKED - RJK	REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CONCRETE PARAPET SLIPFORMING OPTION STRUCTURE NO. 010-1001	F.A.I. RTÉ. SECTION COUNTY TOTAL SHEETS SHEET NO. 74 & 57 (10-34-1) HBK CHAMPAIGN 1187 729
	PLOT SCALE = NA PLOT DATE = 05/03/2021	DRAWN - GLD CHECKED - LM	REVISED - REVISED -			CONTRACT NO. 70B99 SHEET NO. S-87 OF S-106 SHEETS ILLINOIS FED. AID PROJECT



NOTES:

1. Settlement platform shall be in accordance with the applicable portions of Article 204.06 of the Standard Specifications.
2. Do not install casing pipe until after one section of 3/4" steel pipe has been covered with earth. The casing pipe should not rest on platform.

FILE NAME = 	USER NAME = Denise Herrera	DESIGNED - LM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SETTLEMENT PLATFORM STRUCTURE NO. 010-1001	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE = NA	DRAWN - MAC	REVISED -			74 & 57	(10-34-1) HBK	CHAMPAIGN	1187	730
PLOT DATE = 05/03/2021	CHECKED - LM	REVISED -			SHEET NO. S-88 OF S-106 SHEETS	CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT		



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION EAST ~~South~~ Abut - Ramp E MSE Wall LOGGED BY TC
(10-34-1) HBK
SECTION ~~105-1-B-14018R~~ LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.
Latitude 40.144661, Longitude -88.280481
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE Auto

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-18
Station 506+81.72
Offset 5.1 ft RT
Ground Surface Elev. 754.24 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. n/a ft	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
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TOPSOIL: Silty Clay, dark brown				753.24	SILTY CLAY TILL: Gray, stiff, moist (continued)			
	1							
	4	0.8	26					
	3	B		751.24				
SILTY CLAY LOAM: Gray to Brown, soft, moist								
	1	0.3	23			6		
	1	B				8	1.2	11
	1			748.74				
SAND: Brown, medium dense, wet, coarse								
	5		17					
	9			746.24				
SILTY CLAY TILL: Gray, soft, wet								
	3					5		
	5	0.4	13			7	1.5	11
	8	B		743.74				
SILTY CLAY LOAM: Brown, stiff, moist								
	3							
	5	1.1	14			7		
	7	B		741.24				
SILTY CLAY TILL: Gray, stiff, moist								
	6					5		
	7	1.7	11			13	0.5	10
	11	B				13	P	
	3							
	6	1.2	12					
	9	B						
SILTY CLAY TILL: Gray, stiff, moist								
	6					6		
	8	1.4	11			8	1.3	12
	10	B				11	B	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrator)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION EAST ~~South~~ Abut - Ramp E MSE Wall LOGGED BY TC
(10-34-1) HBK
SECTION ~~105-1-B-14018R~~ LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.
Latitude 40.144661, Longitude -88.280481
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE Auto

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-18
Station 506+81.72
Offset 5.1 ft RT
Ground Surface Elev. 754.24 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. n/a ft	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
-------------------------------	--------------------------------	----------------------------	------------------------------	----------------------------	-------------------------------	--------------------------------	----------------------------	------------------------------

SILTY CLAY TILL: Gray, stiff, moist (continued)								
	7							
	9	1.4	12			4		
	12	B				9	1.6	13
	12			687.24				
SILTY CLAY TILL: Gray, very stiff, moist								
	6					7		
	9	1.4	12			10	2.1	13
	15	B		682.74		15	B	
	6							
	7	1.3	14			9	1.5	13
	11	B				15	B	
	5							
	6					6		
	7	1.3	14			9	1.5	13
	11	B				15	B	
	6							
	10	1.7	14					
	15	B						
End of Boring								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrator)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Date 2/3/15

ROUTE I-57/74 DESCRIPTION Pier 1 ~~Box~~ Boring Ramp E LOGGED BY TC
(10-34-1) HBK
SECTION ~~10500RSxxxxxxx~~ LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.
Latitude 40.144966, Longitude -88.281075
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-19
Station 508+81.19
Offset 1.4 ft RT
Ground Surface Elev. 756.38 ft

DEPTH H S	BLOW W S	UCS Qu	MOIST T	Surface Water Elev. _____ ft	Stream Bed Elev. _____ ft	GROUNDWATER ELEVATION H S Qu T	DEPTH H S	BLOW W S	UCS Qu	MOIST T	
											(ft)
6" TOPSOIL 755.88											
SILTY CLAY LOAM: Brown, medium											
	2										
	4	0.7	31								
753.38											
SILTY CLAY LOAM: Brown, soft											
	1							4			
	2	0.4	28					7	2.3	12	
	3	B					-25	9	B		
	1										
	2	0.5	29								
	2	B									
748.38											
SANDY CLAY LOAM: Brown, stiff											
	18							6			
	15	1.8	14					12	3.3	9	
	4	P					-30	19	B		
	12										
	23	1.5	16								
	11	P									
743.38											
SANDY CLAY LOAM: Gray, stiff											
	4							6			
	6	1.9	13					14		9	
	8	B					-35	19	B		
	12										
	23	1.5	16								
	11	P									
740.88											
SILTY CLAY TILL: Gray, stiff											
	3										
	8	2.9	12								
	8	B									
	3										
	6	2.3	12					4			
	7	B					-40	11	2.4	13	
	7	B									
water in pipe											

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Date 2/3/15

ROUTE I-57/74 DESCRIPTION Pier 1 ~~Box~~ Boring Ramp E LOGGED BY TC
(10-34-1) HBK
SECTION ~~10500RSxxxxxxx~~ LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.
Latitude 40.144966, Longitude -88.281075
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-19
Station 508+81.19
Offset 1.4 ft RT
Ground Surface Elev. 756.38 ft

DEPTH H S	BLOW W S	UCS Qu	MOIST T	Surface Water Elev. _____ ft	Stream Bed Elev. _____ ft	GROUNDWATER ELEVATION H S Qu T	DEPTH H S	BLOW W S	UCS Qu	MOIST T
SILTY CLAY TILL: Gray, very stiff (continued)										
	5									
	8	2.1	13							
	12	B					-45	12	B	
	3									
	6	2.0	14							
	8	B					-50	8	B	
	3									
	6	2.0	14							
	8	B					-50	8	B	
704.38										
SILTY CLAY LOAM TILL: Gray, stiff										
	4									
	9	1.8	13							
	12	B					-55	12	B	
	7									
	11	2.7	13							
	16	B					-75	16	B	
681.38										
End of Boring										
SILTY CLAY TILL: Gray, stiff										
	4									
	6	1.9	13							
	11	B					-60	11	B	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 2 ~~Ramp~~ Ramp E LOGGED BY TLM
 (10-34-1) HBK
 SECTION 1050083XXXXR LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
 Latitude 40.145200, Longitude -88.281732
 COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
 Station 516+05.45
 BORING NO. B-20A
 Station 510+83.87
 Offset 1.2 ft LT
 Ground Surface Elev. 756.18 ft

DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)	Soil Description	DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)
755.68				6" TOPSOIL: Silty Clay, brown				
	7			SILTY CLAY: Brown, very stiff				
	8	3.3	15					
	7	B						
753.18				SANDY CLAY LOAM TILL: Gray, hard				
	3					8		
	4	1.0	17			20	3.3	10
	6	B				22	B	
750.68				SAND: Gray, fine, dense				
	4							
	8	4.1	13					
	10	B						
748.18				SAND AND GRAVEL: Gray, medium, medium dense				
	5			(washed sand from auger)		7		
	8	3.7	16			14		11
	11	B				8		
746.68				SILTY CLAY LOAM TILL: Gray, very stiff to hard				
	7							
	10	4.5	11					
	10	B						
743.18				SILTY CLAY TILL: Gray, very hard				
	4					22		
	6	3.3	11			22	4.5	14
	7	B				50/1"	P	
				End of Boring				
	3							
	6	2.5	13					
	7	B						
738.18				SILTY CLAY TILL: Gray, stiff				
	2							
	4	1.8	13					
	5	B						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 2 ~~RAMP~~ Ramp E LOGGED BY TLM
 (10-34-1) HBK
 SECTION ~~1050031014068~~ LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
 Latitude 40.145367, Longitude -88.282250
 COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001 Station 516+05.45	DEP (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)	Surface Water Elev. n/a ft	DEP (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)	
BORING NO. B-21 Station 512+39.59 Offset 13.5 ft RT Ground Surface Elev. 759.18 ft	H	S	Qu	T	Stream Bed Elev. ft	H	S	Qu	T	
10" TOPSOIL: Silty Clay, dark brown	758.38									SILTY SAND: Gray, fine, medium dense (continued)
SILTY CLAY: Brown, very stiff	4									
	7	2.5	24		737.18					
	7	B								SILTY CLAY LOAM TILL: Gray, stiff
	756.18									
SILTY CLAY: Brown, stiff	2									
	4	1.4	21							
	6	B								
	753.68									
SILTY CLAY: Brown, very stiff	3									
	7	2.1	15		732.18					
	7	B								SAND: Gray, fine to medium, medium dense, trace gravel
	751.18									
SILTY CLAY TILL: Gray, very stiff	3									
	6	2.1	12							
	8	B								
	748.68									
SILTY CLAY LOAM TILL: Gray, stiff	3									
	5	1.7	12							
	7	B								
	739.68									
	3									
	4	1.8	12							
	5	B								
	2									
	3	1.2	13							
	5	B								
	739.68									
	2									
	6	1.0	12							
	11	B								
	20									
	11	B								
	18									
	15	3.9	11							
	18	B								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 2 ~~RAMP~~ Ramp E LOGGED BY TLM
 (10-34-1) HBK
 SECTION ~~1050031014068~~ LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
 Latitude 40.145367, Longitude -88.282250
 COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001 Station 516+05.45	DEP (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)	Surface Water Elev. n/a ft	DEP (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)	
BORING NO. B-21 Station 512+39.59 Offset 13.5 ft RT Ground Surface Elev. 759.18 ft	H	S	Qu	T	Stream Bed Elev. ft	H	S	Qu	T	
SILTY CLAY LOAM TILL: Brown, very stiff (continued)	698.18									SILT: Gray, hard (continued)
	717.18									
SILTY CLAY TILL: Gray, medium stiff	6									
	8	1.0	14							
	11	B								
	712.18									
SILTY CLAY LOAM TILL: Gray, stiff	2									
	3	1.4	15							
	5	B								
	707.18									
SILTY CLAY LOAM TILL: Gray, very stiff	4									
LL: 21 PL:11 PI: 10	7	2.1	14							
	12	B								
	702.18									
SILT: Gray, hard	5									
	9	4.3	17							
	20	B								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 3 ~~Ramp~~ Ramp E LOGGED BY TLM
SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
Latitude 40.145461, Longitude -88.282938
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-22
Station 514+32.86
Offset 15.9 ft RT
Ground Surface Elev. 780.54 ft

DEPTH (ft)	BLOW COUNT (blows/ft)	UCS (tsf)	MOISTURE (%)	Surface Water Elev. ft		Stream Bed Elev. ft		DEPTH (ft)	BLOW COUNT (blows/ft)	UCS (tsf)	MOISTURE (%)
				ft	ft	ft	ft				
8" TOPSOIL				FILL: Silty Clay, brown/gray, stiff							
779.87											
FILL: Silty Clay, brown, medium											
	1										
	2	0.8	17								
	4	B									
777.54											
FILL: Silty Clay, brown, stiff, trace gravel											
	6										
	9	1.8	9								
	9	B									
775.04											
FILL: Silty Clay, brown, very stiff, with limestone pieces											
	5										
	5	2.9	12								
	13	B				753.54					
FILL: Silty Clay, brown, very stiff				SILTY CLAY: Brown, very stiff							
	10										
	11	2.3	9								
	13	B									
770.04											
FILL: Silty Clay, brown, stiff, with pieces of concrete											
	5										
	14	1.5	12								
	17	P									
767.54											
FILL: Silty Clay, brown, very stiff				SILTY CLAY TILL: Gray, hard							
	5										
	8	2.9	16								
	11	B									
	6										
	9	2.9	14								
	9	B									
	4										
	8	4.6	18								
	6	B									
760.54											

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 3 ~~Ramp~~ Ramp E LOGGED BY TLM
SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
Latitude 40.145461, Longitude -88.282938
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-22
Station 514+32.86
Offset 15.9 ft RT
Ground Surface Elev. 780.54 ft

DEPTH (ft)	BLOW COUNT (blows/ft)	UCS (tsf)	MOISTURE (%)	Surface Water Elev. ft		Stream Bed Elev. ft		DEPTH (ft)	BLOW COUNT (blows/ft)	UCS (tsf)	MOISTURE (%)
				ft	ft	ft	ft				
SILTY CLAY TILL: Gray, very stiff (continued)				SILTY CLAY TILL: Gray, very stiff (continued)							
	4										
	8	2.1	12								
	8	B									
734.04											
SAND: Gray, dense, medium to coarse, trace gravel				LL: 22 PL: 13 PI: 9							
	13		13								
	19										
	25	3.7	8								
		B									
731.54											
SILTY CLAY LOAM TILL: Gray, very stiff											
	3										
	6	1.1	14								
	10	B									
708.54											
SILTY CLAY LOAM TILL: Gray, stiff				LL: 21 PL: 12 PI: 9							
	4										
	7	1.9	14								
	10	B									
728.04											
SILTY CLAY TILL: Brown, stiff											
	5										
	9	1.9	13								
	11	B									
705.54											
End of Boring											
	5										
	8	2.7	12								
	11	B									
723.04											
SILTY CLAY TILL: Gray, very stiff											

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation
Division of Highways
Bacone Farmer Workmand Engineering & Testing, LLC

SOIL BORING LOG

Page 1 of 2

Date 2/9/15

ROUTE I-57/74 DESCRIPTION Pier 4 ~~Ramp~~ Boring Ramp E LOGGED BY TLM
SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM, Latitude 40.145461, Longitude -88.283600
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-23
Station 516+15.94
Offset 13.3 ft RT
Ground Surface Elev. 761.81 ft

DEPTH (ft)	BLOW COUNT (blows/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)	SOIL DESCRIPTION	DEPTH (ft)	BLOW COUNT (blows/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)
0				TOPSOIL: Silty Clay, dark brown				
2				CLAY: Brown, stiff				
3	2.3		25					
5				SILTY CLAY: Brown, soft				
2								
2	1.5		28	SILTY CLAY: Gray, soft				
2								
-5				SILTY CLAY: Brown, soft				
2								
756.31				SILTY CLAY: Gray, soft				
1								
755.31				SILTY CLAY: Brown, soft				
1	0.4		29					
753.81				SILTY CLAY TILL: Brown/Gray, very stiff				
2								
3	0.7		16	SILTY CLAY TILL: Gray, very stiff				
6								
-10				SILTY CLAY TILL: Gray, very stiff				
3								
5	2.5		13	SILTY CLAY LOAM TILL: Gray, soft				
8								
748.81				SILTY CLAY TILL: Gray, very stiff				
4								
6	2.3		12	SILTY CLAY LOAM TILL: Gray, very stiff				
9								
-15				SILTY CLAY TILL: Gray, stiff				
3								
6	2.3		11	SILTY CLAY LOAM TILL: Gray, very hard				
8								
743.81				SILTY CLAY TILL: Gray, stiff				
3								
4	1.8		12					
6								
-20								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation
Division of Highways
Bacone Farmer Workmand Engineering & Testing, LLC

SOIL BORING LOG

Page 2 of 2

Date 2/9/15

ROUTE I-57/74 DESCRIPTION Pier 4 ~~Ramp~~ Boring Ramp E LOGGED BY TLM
SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM, Latitude 40.145461, Longitude -88.283600
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-23
Station 516+15.94
Offset 13.3 ft RT
Ground Surface Elev. 761.81 ft

DEPTH (ft)	BLOW COUNT (blows/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)	SOIL DESCRIPTION	DEPTH (ft)	BLOW COUNT (blows/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)
0				SILTY CLAY LOAM TILL: Gray, very hard (continued)				
699.81				SILTY CLAY TILL: Gray, stiff				
3								
4				SILTY CLAY TILL: Gray, stiff				
4	1.6		13					
-45				SILTY CLAY TILL: Gray, stiff				
4								
714.81				SILTY CLAY TILL: Gray, stiff				
3								
4	1.1		14	SILTY CLAY TILL: Gray, stiff				
6								
-50				SILTY CLAY TILL: Gray, stiff				
4								
5	1.7		14	SILTY CLAY TILL: Gray, stiff, with silt seams				
8								
-55				SILTY CLAY TILL: Gray, stiff, with silt seams				
3								
5	1.2		15					
7								
-60								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 5 ~~Ramp~~ Ramp E LOGGED BY TLM
SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-24
Station 517+87.57
Offset 19.2 ft RT
Ground Surface Elev. 760.71 ft

DEPTH (ft)	BLOW COUNT (blows/6")	UNIFORMITY COEFFICIENT (tsf)	MOISTURE (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (blows/6")	UNIFORMITY COEFFICIENT (tsf)	MOISTURE (%)
0				5" TOPSOIL: Silty Clay, dark brown				
2				SILTY CLAY: Brown, stiff				
4	1.2	20						
5	S							
3						3		
5	1.9	19				5	1.7	13
4	B					7	B	
755.21				SILTY CLAY: Brown/Gray, stiff				
3								
5	1.5	15						
6	B							
752.71				SILTY CLAY LOAM: Gray, stiff, with silt seams				
3						4		
6	1.7	14				7	1.5	10
7	S					8	P	
750.21				SILTY CLAY: Gray, very stiff				
3								
4	2.5	16						
8	B							
747.71				SILTY CLAY TILL: Gray, very stiff				
3						3		
4	2.1	11				5	2.5	12
8	B					6	B	
742.71				SILTY CLAY TILL: Gray, stiff				
3						7		
7	1.7	13				9		12
8	B					9		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 5 ~~Ramp~~ Ramp E LOGGED BY TLM
SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
Station 516+05.45
BORING NO. B-24
Station 517+87.57
Offset 19.2 ft RT
Ground Surface Elev. 760.71 ft

DEPTH (ft)	BLOW COUNT (blows/6")	UNIFORMITY COEFFICIENT (tsf)	MOISTURE (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (blows/6")	UNIFORMITY COEFFICIENT (tsf)	MOISTURE (%)
0				SILTY CLAY TILL: Gray, very stiff, with thin sand seams				
3								
5	1.7	14				3		
6	B					5	1.1	13
718.71				SILTY CLAY TILL: Gray, stiff, with 4" silt seam @ 44'				
5								
6	1.7	14				6	B	
694.21				SILTY CLAY LOAM TILL: Gray, very stiff				
3								
6	1.7	13				3		
7	S					5	2.2	13
750.21				SILTY CLAY: Gray, very stiff				
3								
4	2.5	16						
8	B							
728.71				SILTY CLAY TILL: Gray, very stiff, with thin sand seams				
3								
4	2.1	11				5		
8	B					12	1.8	12
747.71				SILTY CLAY TILL: Gray, very stiff				
3								
4	2.3	12						
8	B							
742.71				SILTY CLAY TILL: Gray, stiff				
3								
7	1.7	13						
8	B							
685.71				End of Boring				
3								
4	1.0	14						
7	B							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 6 ~~Box~~ Boring Ramp E LOGGED BY TLM
 SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
 COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001 Station 516+05.45
 BORING NO. B-25 Station 519+75.77
 Offset 7.9 ft LT
 Ground Surface Elev. 763.51 ft

DEPTH (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)	SOIL DESCRIPTION	DEPTH (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)
762.01	2			SILTY CLAY LOAM TILL: Gray, medium (continued)				
741.51	5 9	1.9 B	21	CLAY TILL: Gray, medium				
	3					2		
	4	1.6	14			3	0.9	15
758.01	7	B		1" sand seam at 24.5 ft.	-25	4	B	
	3							
755.51	6 8	1.2 B	14	SILTY CLAY TILL: Gray, stiff	736.51			
	3							
	6	1.5	14			3	5	1.5
	8	B				7	B	12
753.01				1" sand seam @ 9.95 ft.				
	3							
	6	2.1	11					
	8	B						
750.51								
	3			SILTY CLAY LOAM TILL: Gray, stiff		3		
	4	1.8	14			4		11
	7	B		(<1" recovery, rock in shoe)	-35	6		
748.01								
	2							
	4	1.7	18					
	7	B						
745.51								
	2			SILTY CLAY TILL: Brown, very stiff	726.51			
	4	1.0	12			6		
	4	B				8	2.5	12
	4	B				10	B	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 6 ~~Box~~ Boring Ramp E LOGGED BY TLM
 SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
 COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001 Station 516+05.45
 BORING NO. B-25 Station 519+75.77
 Offset 7.9 ft LT
 Ground Surface Elev. 763.51 ft

DEPTH (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)	SOIL DESCRIPTION	DEPTH (ft)	BLOW (ft)	UCS (tsf)	M O I S T (%)
721.51				SILTY CLAY TILL: Brown, very stiff (continued)				
	3							
	4	2.1	15			6	1.6	13
	7	B			-65	8	B	
716.51				SILTY CLAY TILL: Gray, stiff				
	3							
	5	1.7	14			3		
	7	B				5	1.9	13
	8					7	B	
	11			(sample very disturbed from rock in shoe)		6		
	12					7	1.6	13
					688.51	10	B	
				End of Boring				
	3							
	5	1.3	14					
	7	B						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 7 ~~R&R~~ Boring Ramp E LOGGED BY TLM
 SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.
 COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
 Station 516+05.45
 BORING NO. B-26
 Station 522+02.29
 Offset 21.0 ft RT
 Ground Surface Elev. 762.81 ft

DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)
760.81	2	1.0	27	SILTY CLAY: Brown/Gray, soft	740.81			
757.31	3	0.5	24	SILTY CLAY: Brown/Gray, medium, trace gravel				
754.81	3	0.7	16	SILTY CLAY: Brown, medium				
752.31	3	0.7	16	SILTY CLAY TILL: Gray, very stiff				
	2	2.1	12					
	6	2.5	11					
	7	B						
	3							
	5	2.5	11					
	6	B						
	4							
	5	2.1	12					
	6	B						
	4							
	5	2.1	12					
	6	B						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION Pier 7 ~~R&R~~ Boring Ramp E LOGGED BY TLM
 SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.
 COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001
 Station 516+05.45
 BORING NO. B-26
 Station 522+02.29
 Offset 21.0 ft RT
 Ground Surface Elev. 762.81 ft

DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)
720.81				SILTY CLAY TILL: Gray, very stiff				
	4							
	6	2.3	12					
	7	B						
	3							
	5	2.3	12					
	8	B						
	3							
	7	2.3	13					
	9	B						
	4							
	8	2.7	12					
	9	B						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



**Illinois Department
of Transportation**
Division of Highways
Bacone Farmer Workmand Engineering & Testing, LLC

SOIL BORING LOG

Date 2/13/15

ROUTE I-57/74 DESCRIPTION WEST & Abutment Boring Ramp E LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM

Latitude 40.144513, Longitude -88.286391

COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001 Station 516+05.45
BORING NO. B-27 Station 524+80.74
Offset 3.1 ft RT
Ground Surface Elev. 766.63 ft

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft	D E P T H	B L O W S	U C S Qu	M O I S T	Stream Bed Elev. _____ ft
(ft)	(/6")	(tsf)	(%)		(ft)	(/6")	(tsf)	(%)	

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft	D E P T H	B L O W S	U C S Qu	M O I S T	Stream Bed Elev. _____ ft
(ft)	(/6")	(tsf)	(%)		(ft)	(/6")	(tsf)	(%)	
10" HMA	765.83								
SILTY CLAY: Brown, stiff	8								
	764.63	3	1.5	22					
SILTY CLAY: Black, stiff	3	P			744.13				
	3					2			
	4	1.7	25			3	1.7	12	
	7					6	B		
	2								
	3	1.5	20						
	3	P							
	758.63								
SILTY CLAY: Brown and Gray, soft	1			No recovery		5			
	1	0.3	29			8			
	2	P				8			
	756.13								
SILTY CLAY LOAM: Brown, very stiff, possible till	3								
	5	3.7	12		734.63				
	7	B							
	753.63								
SILTY CLAY LOAM TILL: Brown to Gray, very stiff	4					5			
	7	2.7	11			6	2.5	11	
	9	B				9	B		
	751.13								
SILTY CLAY LOAM TILL: Gray, very stiff	3								
	5	2.5	10		729.63				
	6	B							
	3								
	5	2.7	11			WR			
	20	B				WR		13	
						6			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



**Illinois Department
of Transportation**
Division of Highways
Bacone Farmer Workmand Engineering & Testing, LLC

SOIL BORING LOG

Date 2/13/15

ROUTE I-57/74 DESCRIPTION WEST & Abutment Boring Ramp E LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM

Latitude 40.144513, Longitude -88.286391

COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. 010-1001 Station 516+05.45
BORING NO. B-27 Station 524+80.74
Offset 3.1 ft RT
Ground Surface Elev. 766.63 ft

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft	D E P T H	B L O W S	U C S Qu	M O I S T	Stream Bed Elev. _____ ft
(ft)	(/6")	(tsf)	(%)		(ft)	(/6")	(tsf)	(%)	

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft	D E P T H	B L O W S	U C S Qu	M O I S T	Stream Bed Elev. _____ ft
(ft)	(/6")	(tsf)	(%)		(ft)	(/6")	(tsf)	(%)	
SAND AND GRAVEL: Gray, loose (continued)									
	724.63								
SAND AND GRAVEL: Gray, medium dense sand, fine gravel Washed sand and gravel from auger at 45 ft.	6								
	10		17						
	15								
	717.63	8							
SILTY CLAY LOAM TILL: Gray, very stiff	7	3.9	11						
	10	B							
	50								
	714.63								
SILTY CLAY LOAM TILL: Gray, hard	7								
	11	7.0	12						
	14	B							
	55								
	691.63	4							
	75	9	2.1	13					
	11	B							
	75								
End of Boring									
	709.63								
SILTY CLAY TILL: Gray, very stiff	4								
	7	3.3	12						
	12	B							
	80								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



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815 780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 7/26/17

ROUTE I-57/74 DESCRIPTION Ramp E, east abutment LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION, SEC., TWP., RNG.,

Latitude 40.144676, Longitude -88.280732

COUNTY Champaign DRILLING METHOD Hollow Stem Auger HAMMER TYPE Automatic

STRUCT. NO. 010-1001
Station 516+05.45

BORING NO. EE 2
Station 507+38.00
Offset 35.0 ft Lt.

Ground Surface Elev. 754.39 ft

D E P T H S	B L O W S	U C S Qu	M O I S T %	Surface Water Elev. _____ ft	D E P T H	B L O W S	U C S Qu	M O I S T %
(ft)	(/6")	(tsf)	(%)	Stream Bed Elev. _____ ft	(ft)	(/6")	(tsf)	(%)

Dark Brown Silty Clay Loam Topsoil						2		
752.39	3					3	1.7	12
Dark Brown Very Stiff Silty Clay, moist	4	3.3	22			5	B	
751.14	3	B						
Loose Loamy Sand and Gravel, wet	2					2		
	1	-	18			4	1.8	11
	-5	1				5	B	
747.09	5					7		
Very Stiff Brown Silty Clay Loam Till, moist	5	2.3	13			9	4.5	10
	4	B				11	B	
745.39	2							
Very Stiff to Stiff Gray Silty Clay Loam Till, moist	5	3.3	13			6	2.0	13
	5	B				12	P	
	-10					11	2.5	
	4							
	5	2.1	12					
	7	B						
	3					8		
	5	2.3	12			15	1.5	11
	-15	6				16	P	
	4							
	4	1.5	12					
	5	P						
	2							
	3	1.0	12					
	-20	4	P					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



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815-780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 7/26/17

ROUTE I-57/74 DESCRIPTION Ramp E, east abutment MSE Wall LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION , SEC. , TWP. , RNG. ,
Latitude 40.144582, Longitude -88.280339

COUNTY Champaign DRILLING METHOD Hollow Stem Auger HAMMER TYPE Automatic

STRUCT. NO. 010-1001
Station 516+05.45

BORING NO. EE 5
Station 506+30.00
Offset 8.0 ft Rt.
Ground Surface Elev. 753.62 ft

D E P T H S	B L O W S	U C S Qu	M O I S T %	Surface Water Elev. _____ ft	D E P T H S	B L O W S	U C S Qu	M O I S T %
(ft)	(/6")	(tsf)	(%)	Stream Bed Elev. _____ ft	(ft)	(/6")	(tsf)	(%)

Stiff Dark Brown Silty Clay Loam, moist						4				Very Stiff to Stiff Gray Silty Clay Loam Till (continued)
751.87	3					4	1.3	13		
Very Stiff Brown/Gray Silty Clay, moist	3	2.9	22			6	B			
750.37	4	B								
Very Loose Gray Sandy Loam, some clay, wet	2					3				
	1	-	23			3	1.7	13		
748.12	-5	1				6	B			
Medium Dense Loamy Sand and Gravel										
	3					5				
	4	-	17			5	1.2	12		
745.62	6					9	B			
Dense Brown Gravel, very angular Limestone pieces										
	4					4				
	18	-	13			5	1.4	11		
743.12	-10	14				8	B			
Very Stiff to Stiff Gray Silty Clay Loam Till										
	8									
	5	2.1	12							
	8	B								
	4					4				
	5	2.1	12			7	2.5	13		
	-15	8				10	B			
					718.62	-35				
										End of Boring
	5									
	7	3.0	12							
	8	P								
Shelby Tube collected from 18.5 ft. to 20.5 ft.										
	3	ST								
	3	1.0	11							
	-20	2								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)



USER NAME = Denise Herrera	DESIGNED - LM	REVISED -
PLOT SCALE = NA	CHECKED - RJK	REVISED -
PLOT DATE = 05/03/2021	DRAWN - GLD	REVISED -
	CHECKED - LM	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOG - 13
STRUCTURE NO. 010-1001
SHEET NO. S-101 OF S-106 SHEETS

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 743
			CONTRACT NO. 70B99	



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815-780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 7/25/17

ROUTE I-57/74 DESCRIPTION Ramp E, westabut. MSE Ret. Wall LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION SEC., TWP., RNG.,

COUNTY Champaign DRILLING METHOD Mud Rotary HAMMER TYPE Automatic

STRUCT. NO. 010-1001
Station 516+05.45

BORING NO. EW 1
Station 525+05.00
Offset 8.0 ft Rt.
Ground Surface Elev. 764.60 ft

D E P T H S T	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft		D E P T H S T	B L O W S	U C S Qu	M O I S T
				(ft)	(/6")				
Dark Brown and Black Silty Clay, organic, moist				Very Stiff Gray Clay Loam, 2" Gray Fine to Medium Sand Seam at 26.5' (continued)					
	3		ST				5		
	4	1.5	27				7	2.9	13
	4	P					9	B	
	3						5		
	2	1.1	33				6	2.1	13
	2	B					8	B	
	-5						-25		
----- 759.10				----- 759.10					
Soft to Stiff Brown/Gray Silty Clay Loam				Very Stiff Silty Clay Loam Till					
	1	0.5	18				4		
	2	B					5	3.1	13
	4	1.3	15				5	B	
		B							
----- 756.60				----- 756.60					
Hard Brown Clay Loam Till				Very Stiff Gray Clay Loam Till					
	5						4		
	7	4.5	14				6	2.1	12
	10	B					8	B	
	-10						-30		
----- 754.10				----- 754.10					
Very Stiff Gray Clay Loam Till				Hard Gray Clay Loam Till					
	5						7		
	8	2.9	12				10	2.9	11
	8	B					13	B	
----- 751.60				----- 751.60					
Hard Gray Clay Loam Till				Very Stiff Gray Clay Loam, 2" Gray Fine to Medium Sand Seam at 26.5'					
	6						7		
	9	5.2	12				10	2.9	11
	11	B					13	B	
	-15						-35		
----- 749.10				----- 749.10					
Very Stiff Gray Clay Loam, 2" Gray Fine to Medium Sand Seam at 26.5'				Dense Gray Fine to Medium Sand, trace gravel (<3/8")					
	6						7		
	9	3.3	12				10	3.3	12
	10	B							
----- 727.60				----- 727.60					
Dense Gray Fine to Medium Sand, trace gravel (<3/8")				Dense Gray Fine to Medium Sand, trace gravel (<3/8")					
	4						7		
	6	3.1	13				11	-	16
	7	B					14		
	-20						-40		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



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815-780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 7/25/17

ROUTE I-57/74 DESCRIPTION Ramp E, westabut. MSE Ret. Wall LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION SEC., TWP., RNG.,

COUNTY Champaign DRILLING METHOD Mud Rotary HAMMER TYPE Automatic

STRUCT. NO. 010-1001
Station 516+05.45

BORING NO. EW 1
Station 525+05.00
Offset 8.0 ft Rt.
Ground Surface Elev. 764.60 ft

D E P T H S T	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft		D E P T H S T	B L O W S	U C S Qu	M O I S T
				(ft)	(/6")				
Dense Gray Fine to Medium Sand, trace gravel (<3/8") (continued)				Very Stiff Silty Clay Loam Till (continued)					
	11						6		
	21	-	17				10	2.9	13
	26	B					13	B	
	-45						-65		
----- 717.60				----- 717.60					
Very Stiff Silty Clay Loam Till				End of Boring					
	6						5		
	8	2.3	15				10	2.1	13
	11	B					15	B	
	-50						-70		
----- 689.60				----- 689.60					
End of Boring				End of Boring					
	5						7		
	7	2.5	13				11	2.0	15
	13	B					15	P	
	-55						-75		
----- 689.60				----- 689.60					
	6						7		
	9	3.3	14				11	2.0	15
	15	B					15	P	
	-60						-80		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

SOIL BORING LOG

Solutions You Can Build On

Date 7/24/17

ROUTE I-57/74 DESCRIPTION Ramp E, west abut. MSE Ret. Wall LOGGED BY TLM

SECTION (10-34-1) HBK
~~(10-34-1) HBK~~ LOCATION, SEC., TWP., RNG. Latitude 40.143802, Longitude -88.287079

COUNTY Champaign DRILLING METHOD Mud Rotary HAMMER TYPE Automatic

STRUCT. NO. 010-1001 Station 516+05.45	D E P T H S	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft	D E P T H S	B L O W S	U C S Qu	M O I S T	Stream Bed Elev. _____ ft
BORING NO. EW 8 Station 528+05.55 Offset 22.0 ft Lt. Ground Surface Elev. 764.25 ft	(ft)	(/6")	(tsf)	(%)	Groundwater Elev.:	(ft)	(/6")	(tsf)	(%)	First Encounter _____ ft
										Upon Completion _____ ft
										After _____ Hrs.

Hard Brown Silty Clay	---	---	---	---	Stiff Gray Silty Clay Loam Till with large pieces of angular gravel Gravel pieces are sub-rounded to rounded 2" Gray Medium Sand seam at 29' <i>(continued)</i>	---	3	---	---	---
		7	4.5	17			5	1.3	13	
		7	P				6	B		
	761.00	11					4			
Medium Stiff Brown Clay Loam	---	5	---	---			5	1.7	14	
		3	0.6	11			7	B		
		-5					-25			
	758.75	3								
Stiff Brown Silty Clay, trace sand and gravel	---	2	---	---			6		---	
		3	1.7	18			7	1.5	16	
		2	B				10	P		
		3					4		---	
		5	2.0	16			5	1.6	13	
		-10	B				-30	B		
	753.25	6	5.0	14						
Hard Brown Silty Clay Loam Till	---	7	B							
	752.25	9	2.5	---	Very Stiff Gray Silty Clay Loam Till 2" Gray Fine to Medium Sand seam at 34'		6	---	---	
		4	P				6	3.3	11	
Very Stiff Gray Silty Clay Loam Till	---	6	2.5	16			13	B		
		-15	P				-35			
	748.75	7								
Stiff Gray Silty Clay Loam Till with large pieces of angular gravel Gravel pieces are sub-rounded to rounded 2" Gray Medium Sand seam at 29'	---	4	---	---					---	
		4	1.3	17						
		5	P		Dense Gray Gravel (3/8" top size)				19	-
		4							16	
		5	1.7	13					16	
		-20	P						-40	16
	724.75	7			Very Stiff Gray Silty Clay Loam Till					2.5P

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)

SOIL BORING LOG

Solutions You Can Build On

Date 7/24/17

ROUTE I-57/74 DESCRIPTION Ramp E, west abut. MSE Ret. Wall LOGGED BY TLM

SECTION (10-34-1) HBK
~~(10-34-1) HBK~~ LOCATION, SEC., TWP., RNG. Latitude 40.143802, Longitude -88.287079

COUNTY Champaign DRILLING METHOD Mud Rotary HAMMER TYPE Automatic

STRUCT. NO. 010-1001 Station 516+05.45	D E P T H S	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft	D E P T H S	B L O W S	U C S Qu	M O I S T	Stream Bed Elev. _____ ft
BORING NO. EW 8 Station 528+05.55 Offset 22.0 ft Lt. Ground Surface Elev. 764.25 ft	(ft)	(/6")	(tsf)	(%)	Groundwater Elev.:	(ft)	(/6")	(tsf)	(%)	First Encounter _____ ft
										Upon Completion _____ ft
										After _____ Hrs.

Very Stiff Gray Silty Clay Loam Till <i>(continued)</i>	---	---	---	---	Very Stiff Gray Clay Loam Till <i>(continued)</i>	---	---	---	---	---
		13	---	---			8	---	---	
	720.25	19	3.1	13			11	3.3	13	
		-45	B				-65	B		
		19								
Dense Gray Silty Fine Sand	---	6	---	---			6	---	---	
		12	---	---			7	2.3	14	
		13	-	13			11	B		
		-50					-70			
	717.25	19								
Dense Gray Clean Medium Sand to Fine Gravel	---	10	3.0	11			9	1.7	15	
		9	P				12	B		
		10								
		13								
		-60	B				-80			
	709.75	16	3.3	13						
Very Stiff Gray Clay Loam Till	---	8	---	---	End of Boring					
		16								
		-55								
	689.25	13	B							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



3705 Progress Blvd, Ste 2
Peru, Illinois 61354
815-780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 2/12/19

ROUTE I-57/74 DESCRIPTION Ramp E MSE Retaining Wall W. Abut. LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION SE 1/4, SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.
Latitude 40.144404, Longitude 88.28635

COUNTY Champaign DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. 010-1001
Station 516+05.45

BORING NO. E-101 Shld. of Exist. Ramp
Station 525+00
Offset 33.5 ft Lt.

Ground Surface Elev. 765.76 ft

DEPTH (ft)	BLOW COUNT (blows/ft)	UCS (tsf)	M-O-I-S-T (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (blows/ft)	UCS (tsf)	M-O-I-S-T (%)
765.26				HMA Shoulder				
764.26	3			Frozen brown/gray Silty Clay fill, moist		4		
	5	4.5	14	Frost		6	2.3	12
	8	P				7	B	
762.76				Stiff black Silty Clay Topsoil				
	2					4		
761.26	4	3.1	27	Very stiff gray/brown Clay, moist		6	2.3	12
	5	B				7	B	
760.26				Stiff brown Silty Clay, moist				
	2					4		
	2	1.7	23			6	2.1	12
	3	B				7	B	
757.76				Soft brown Silty Clay				
	1					4		
	2	0.3	25			5	2.5	12
	2	B				7	B	
	-10					-30		
753.76				Very stiff brown Silty Clay Loam Till, moist				
	1							
	3	2.1	17					
	7	B						
	4							
	6	3.5	15	Loose gray Medium Coarse Sand	731.76	4		
	8	P		Very stiff to hard gray Clay Loam Till w/sand seams, moist	731.26	4	2.5	12
	-15					6	B	
750.26				Hard to very stiff gray Clay Loam Till, moist				
	5							
	6	5.0	11					
	9	B						
	4					11		
	6	2.3	12			15	3.7	17
	7	B				13	B	
-20				End of Boring	725.76	-40		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



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Peru, Illinois 61354
815-780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 2/12/19

ROUTE I-57/74 DESCRIPTION Ramp E MSE Retaining Wall W. Abut. LOGGED BY TLM

SECTION (10-34-1) HBK LOCATION SE 1/4, SEC. 34, TWP. 20N, RNG. 8E, 3rd PM.

COUNTY Champaign DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. 010-1001
Station 516+05.45

BORING NO. E-102 Backslope of Ditch
Station 526+00
Offset 31.2 ft Lt.

Ground Surface Elev. 765.36 ft

D E P T H S	B L O W S	U C S Qu	M O I S T %	Surface Water Elev. _____ ft	D E P T H S	B L O W S	U C S Qu	M O I S T %	Stream Bed Elev. _____ ft
----------------------------	-----------------------	-------------------	----------------------------	------------------------------	----------------------------	-----------------------	-------------------	----------------------------	---------------------------

Black Clay Loam Topsoil	764.19								Stiff to hard gray Clay Loam Till (continued)
		1				3			
Brown Clay, moist		2	1.5	17		5	2.3	15	
		3	B			6	B		
		1				3			
	760.86	2	<0.25	26		5	1.6	13	
Soft brown Sandy Clay to Clay Loam		-5	1	P		6	B		
	759.86								739.86
Stiff yellow brown Silty Clay, moist		0				4			Very stiff gray Silty Clay Loam Till w/ 2" sand seams spaced 4" apart, sand is wet, SiCL is moist
		1	1.1	16		5	3.7	12	
		2	B			5	B		
	757.36								
Stiff brown Silty Clay Till, moist		2				2			736.36
		3	1.9	14		4	3.5	11	Very stiff to stiff gray Clay Loam Till, moist
		-10	4	B		6	B		
	754.86								
Stiff to hard gray Clay Loam Till		3							
		5	2.3	12					
		7	B						
		4				5			
		7	3.9	11		6	1.9	11	
		-15	10	S		9	B		
		6							
		8	4.1	11					
		11	B						
		4				4			
		7	3.9	11		3	1.5	10	
		-20	9	B		2	B		
End of Boring	725.36					-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



USER NAME = Denise Herrera	DESIGNED - LM	REVISED -
PLOT SCALE = NA	CHECKED - RJK	REVISED -
PLOT DATE = 05/03/2021	DRAWN - GLD	REVISED -
	CHECKED - LM	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BORING LOG - 17
STRUCTURE NO. 010-1001

F.A.I. RTE. 74 & 57	SECTION (10-34-1) HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 747
			CONTRACT NO. 70B99	



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Peru, Illinois 61354
815-780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 2/12/19

ROUTE I-57/74 DESCRIPTION Ramp E MSE Retaining Wall W. Abut. LOGGED BY TLM
 SECTION (10-34-1) HBK LOCATION SE 1/4, SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
~~XXXXXX~~ XXXXXX XXXXXX Latitude 40.144031, Longitude 88.286858
 COUNTY Champaign DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. 010-1001
 Station 516+05.45
 BORING NO. E-103 Backslope of Ditch
 Station 527+00
 Offset 28.6 ft Lt.
 Ground Surface Elev. 765.19 ft

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev.	D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)	- ft	(ft)	(/6")	(tsf)	(%)
Brown Silty Clay, moist				764.02	5			
	1							
	2	1.3	31					
	3	P						
Stiff brown Clay, moist				762.19	3			
	1							
	0	<0.25	27					
	1	P						
Very soft yellow/brown Silty Clay, wet				759.69	3			
	2							
	4	1.7	17					
	5	B						
Stiff brown Clay Till, moist					2			
	2							
	4	2.1	17					
	6	B						
Stiff gray Clay Loam Till, moist				753.69	2			
	4							
	6	2.1	12					
	7	B						
Gray Clay Loam Till w/ 2" sand seams spaced 6" apart				733.19	3			
	4							
	4	2.1	13					
	5	B						
Dense Silt Loam Till, moist				728.19	2			
	2							
	3	1.0	13					
	5	B						
Medium dense Medium Coarse Sand, wet End of Boring				726.19	2			
	3	1.4	13					
	4	B						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrator)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



3705 Progress Blvd, Ste 2
Peru, Illinois 61354
815-780-8486

SOIL BORING LOG

Solutions You Can Build On

Date 2/13/19

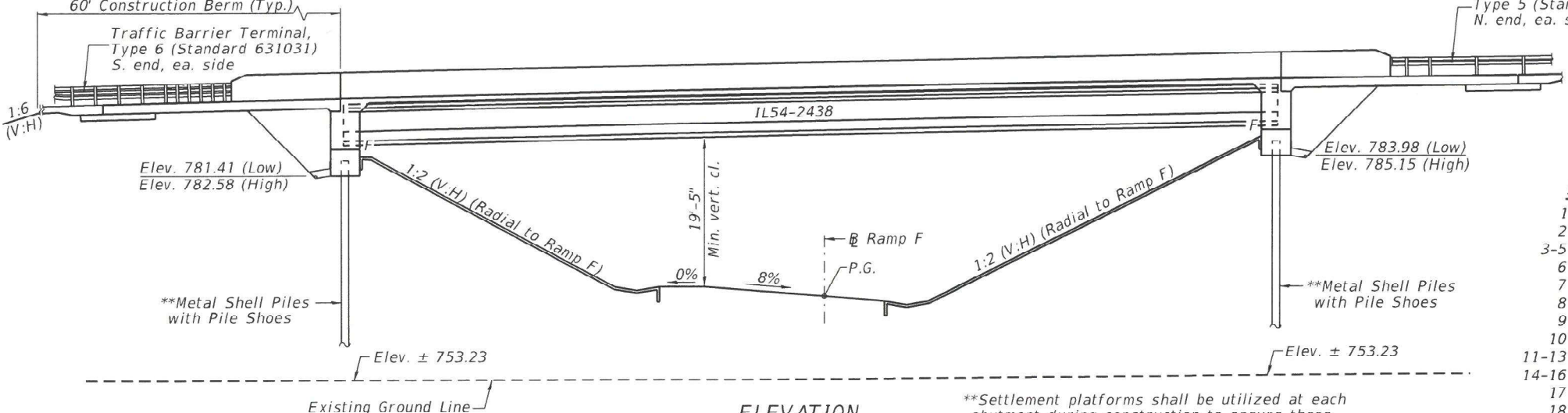
ROUTE I-57/74 DESCRIPTION Ramp E MSE Retaining Wall LOGGED BY TLM
 SECTION (10-34-1) HBK LOCATION SE 1/4, SEC. 34, TWP. 20N, RNG. 8E, 3rd PM
~~XXXXXX~~ XXXXXX XXXXXX Latitude 40.144031, Longitude 88.286858
 COUNTY Champaign DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. 010-1001
 Station 516+05.45
 BORING NO. E-103ST Backslope of Ditch
 Station 527+00
 Offset 28.6 ft Lt.
 Ground Surface Elev. 765.19 ft

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev.	D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)	- ft	(ft)	(/6")	(tsf)	(%)
Brown Silty Clay, moist				764.02	5			
	1							
	2	1.3	31					
	3	P						
Stiff brown Clay, moist				762.19	3			
	1							
	0	<0.25	27					
	1	P						
Very soft yellow/brown Silty Clay, wet				759.69	3			
	2							
	4	1.7	17					
	5	B						
Stiff brown Clay Till, moist					2			
	2							
	4	2.1	17					
	6	B						
Stiff gray Clay Loam Till, moist				753.69	2			
	4							
	6	2.1	12					
	7	B						
Gray Clay Loam Till w/ 2" sand seams spaced 6" apart				733.19	3			
	4							
	4	2.1	13					
	5	B						
Dense Silt Loam Till, moist				728.19	2			
	2							
	3	1.0	13					
	5	B						
Medium dense Medium Coarse Sand, wet End of Boring				726.19	2			
	3	1.4	13					
	4	B						

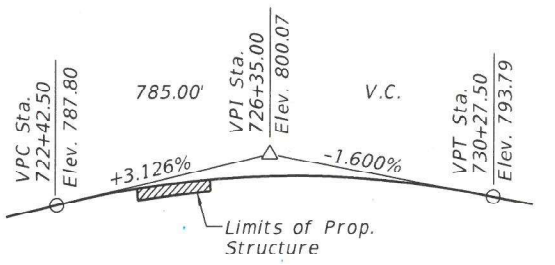
The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrator)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)

Bench Mark: Chiseled "□" on top of N.W. corner of light pole foundation #50-107 on Ramp DB, Sta. 1068+46.46 Elev. 769.173
 Existing Structure: None No Salvage



INDEX OF SHEETS

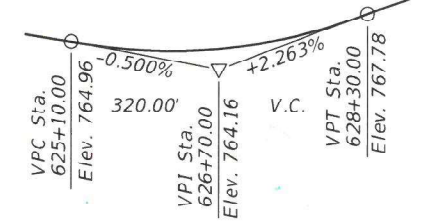
- 1 General Plan and Elevation
- 2 General Data
- 3-5 Top of Slab Elevations
- 6 Top of South Approach Slab Elevations
- 7 Top of North Approach Slab Elevations
- 8 Superstructure
- 9 Superstructure Details
- 10 Diaphragm Details
- 11-13 South Approach Slab
- 14-16 North Approach Slab
- 17 Framing Plan
- 18 IL54N Beam
- 19 IL54N Beam Details
- 20-21 South Abutment
- 22-23 North Abutment
- 24 Wingwall Extension
- 25 Metal Shell Pile Details
- 26 Concrete Parapet Slipforming Option
- 27 Settlement Platform
- 28-30 Soil Boring Logs



PROFILE GRADE RAMP G
 (Along B Ramp G)
 Note: The profile grade shows the final elevations after grinding.

PROPOSED RAMP G CURVE DATA

P.I. Sta. = 730+86.74 L = 1,736.70'
 $\Delta = 60^\circ-51'-35''$ (Rt.) E = 261.20'
 D = 3°-30'-16" S.E. = 6.7%
 R = 1,635' P.C. Sta. = 721+26.34
 T = 960.40' P.T. Sta. = 738+63.05



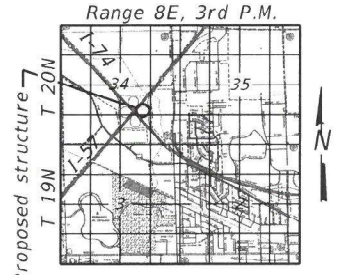
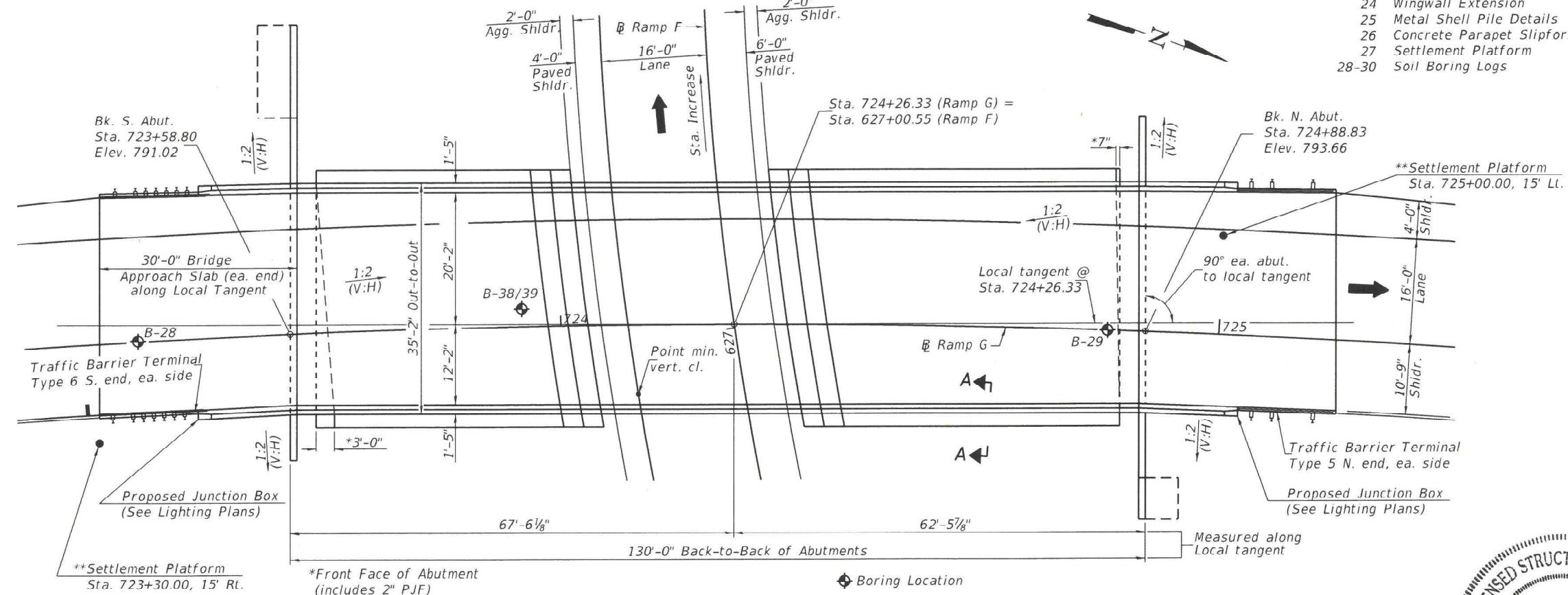
PROFILE GRADE RAMP F
 (Along B Ramp F)

PROPOSED RAMP F CURVE DATA

P.I. Sta. = 630+93.16 L = 1041.65'
 $\Delta = 124^\circ-20'-18''$ (Rt.) E = 548.14'
 D = 11°-56'-12" S.E. = 8.0%
 R = 480.00' P.C.C. Sta. = 621+83.95
 T = 909.21' P.C.C. Sta. = 632+25.60

Note:
 Up to 1/4" may be ground off the bridge deck and the bridge approach slab.
 See Sheet 2 of 30 for Offset Sketch.

**Settlement platforms shall be utilized at each abutment during construction to ensure there is less than 0.4 inches or less left of settlement prior to the installation of the piles to ensure the effects of down drag forces are negligible



LOCATION SKETCH

DESIGN STRESSES
 FIELD UNITS

$f'_c = 3,500$ psi (Cast-in-Place)
 $f'_c = 4,000$ psi (Superstructure Concrete)
 $f_y = 60,000$ psi (Reinforcement)

PRECAST PRESTRESSED UNITS

$f'_c = 8,500$ psi
 $f_{ci} = 6,500$ psi
 $f_{pu} = 270,000$ psi (0.6" \emptyset low lax strands)
 $f_{pbt} = 202,300$ psi (0.6" \emptyset low lax strands)

LOADING HL-93
 Allow 50 psf for future wearing surface

DESIGN SPECIFICATIONS
 2014 AASHTO LRFD Bridge Specifications, 7th Edition w/2015 & 2016 Interims

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
 Design Spectral Acceleration at 1.0 sec (SD1) = 0.135g
 Design Spectral Acceleration at 0.2 sec (SD5) = 0.233g
 Soil Site Class = D

APPROVED
 For Structural Adequacy Only
Gerald B. Rotherham
 Engineer of Bridges & Structures



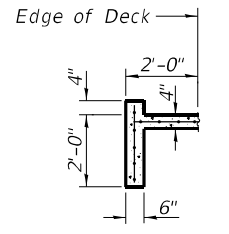
Gerald B. Rotherham
 03/11/2021
 Exp. 11/30/2022

GENERAL PLAN & ELEVATION
RAMP G OVER RAMP F
 F.A.I. RTE. 57/74
 SECTION (10-34-1)HBK
 CHAMPAIGN COUNTY
 STATION 724+26.33
 STRUCTURE NO. 010-1002

FILE NAME = 0101002-70899-001-GPE.dgn	USER NAME =	DESIGNED - GBR	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL PLAN AND ELEVATION STRUCTURE NO. 010-1002	F.A.I. RTE. 57/74	SECTION (10-34-1)HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 749	
BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - FAM	REVISED -			SHEET NO. 1 OF 30 SHEETS		CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT	
402 NORTH COURT STREET MORNING HILL, ILLINOIS 62450 PHONE: 618-947-1100	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -								
		CHECKED - GBR	REVISED -								

GENERAL NOTES

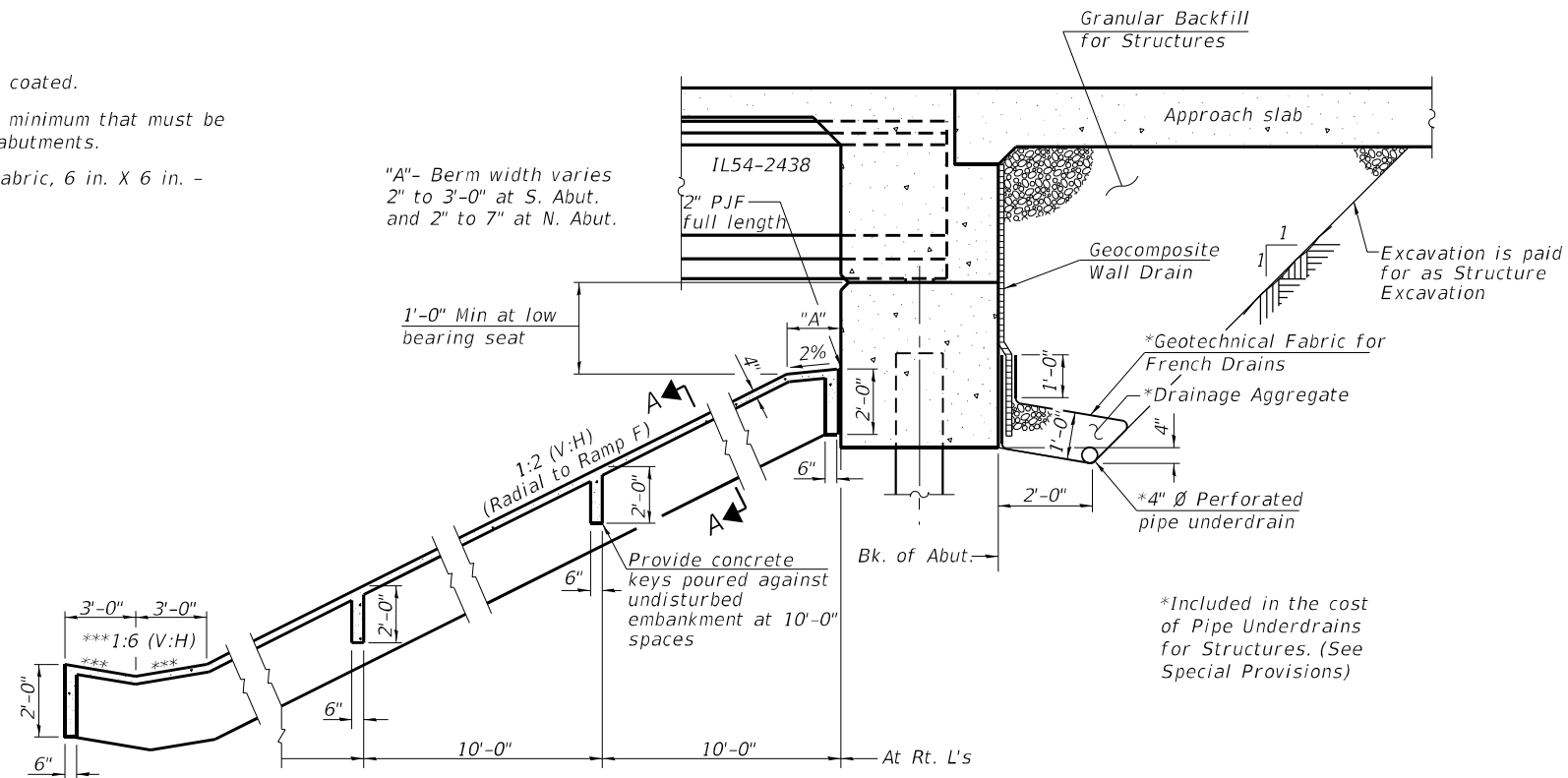
Reinforcement bars designated (E) shall be epoxy coated.
 The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.
 Slope wall shall be reinforced with welded wire fabric, 6 in. X 6 in. - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.



SECTION A-A

STATION 724+26.33
 BUILT 20__ BY
 STATE OF ILLINOIS
 F.A.I. RT. 57/74 SEC. (10-34-1)HBK
 LOADING HL-93
 STRUCTURE NO. 010-1002

NAME PLATE
 See Std. 515001

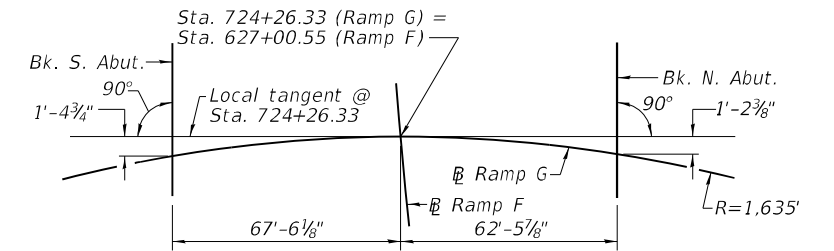


SECTION THRU INTEGRAL ABUTMENT

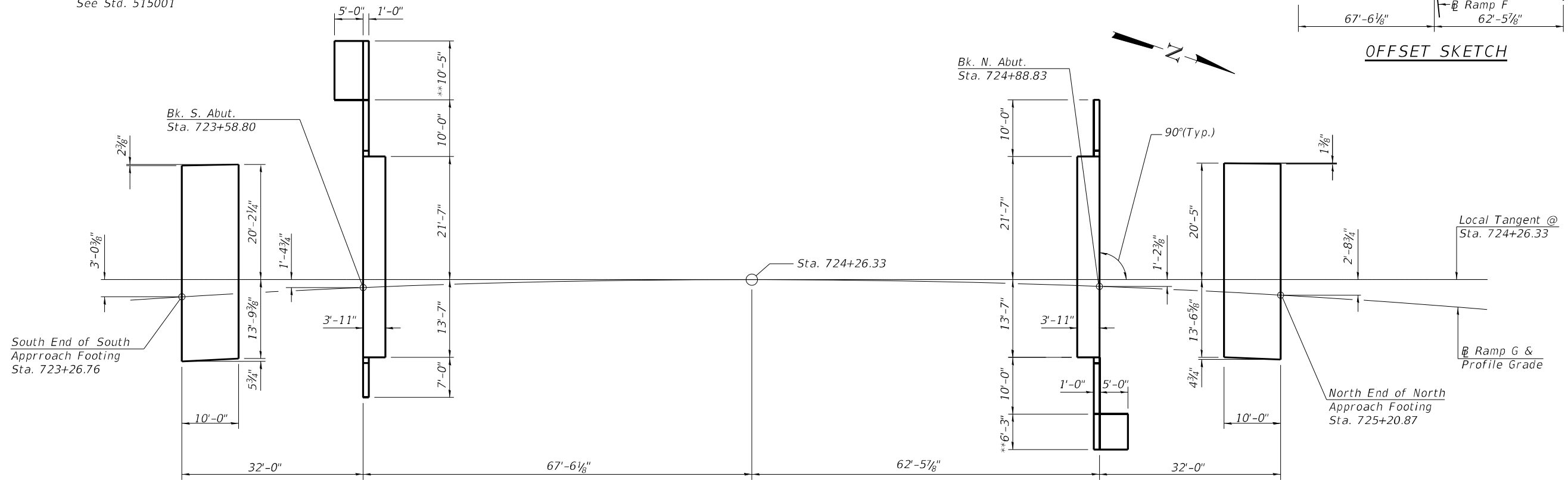
Note:
 All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Structure Excavation	Cu. Yd.		208	208
Concrete Structures	Cu. Yd.		80.4	80.4
Concrete Superstructure	Cu. Yd.	212.4		212.4
Protective Coat	Sq. Yd.	841		841
Concrete Superstructure (Approach Slab)	Cu. Yd.	98.4		98.4
Furnishing and Erecting Precast Prestressed Concrete Beams, IL54N	Foot	764		764
Reinforcement Bars, Epoxy Coated	Pound	83990	13820	97810
Slope Wall 4 Inch	Sq. Yd.		427	427
Furnishing Metal Shell Piles HP 14"x0.312"	Foot		826	826
Driving Piles	Foot		826	826
Test Pile Metal Shells	Each		2	2
Pile Shoes	Each		16	16
Name Plates	Each	1		1
Granular Backfill for Structures	Cu. Yd.		194	194
Geocomposite Wall Drain	Sq. Yd.		91	91
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	335		335
Diamond Grinding (Bridge Section)	Sq. Yd.	698		698
Pipe Underdrains for Structures 4"	Foot		155	155
Settlement Platforms	Each			2

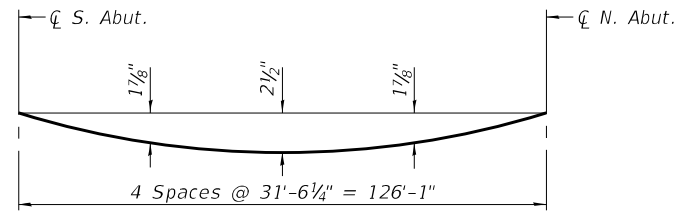


OFFSET SKETCH



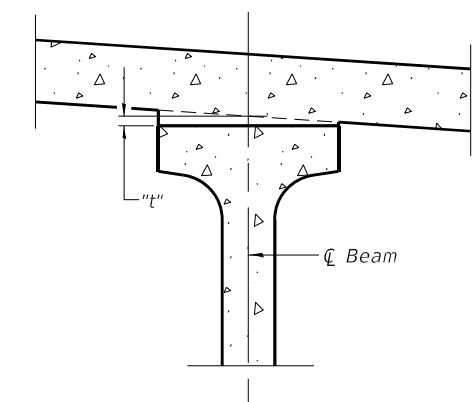
FOOTING PLAN

** Include 1/2" Preformed Joint Filler (See Sheet 24 of 30 for details)



DEAD LOAD DEFLECTION DIAGRAM
(Includes weight of concrete only, excluding beams.)

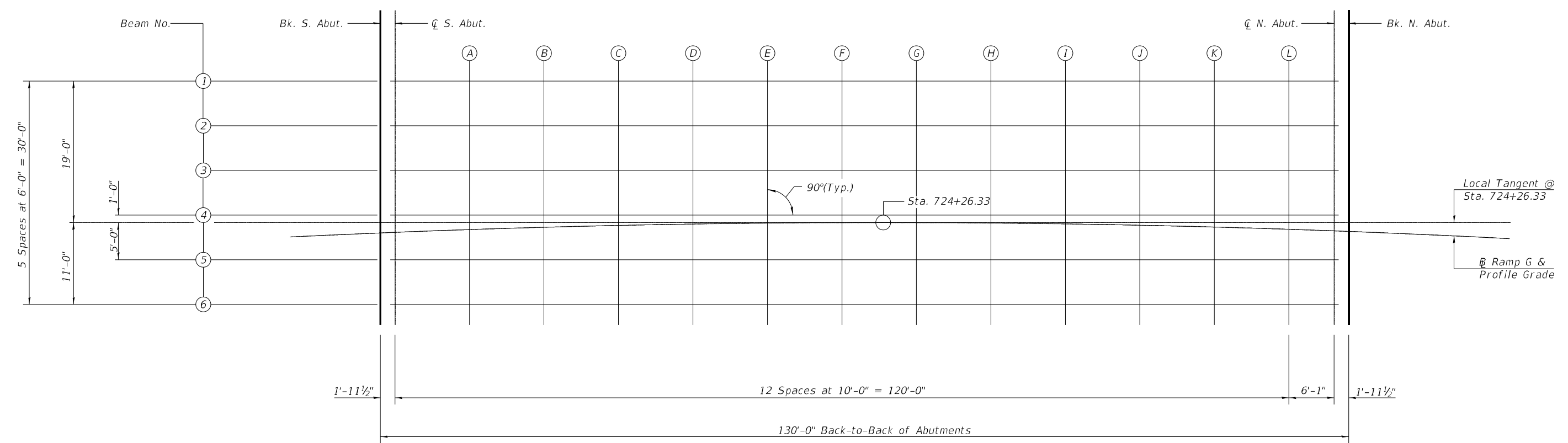
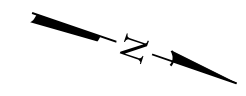
Note:
The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown on Sheet 4 and 5 of 30.



To determine "t": After all precast prestressed beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection and Grinding" shown on sheet 4 and 5 of 30, minus 8 1/4" deck thickness equals the fillet heights "t" above top flange of beams.

The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown on sheet 4 and 5 of 30. For grinding the deck, see Special Provisions.

FILLET HEIGHTS



PLAN

FILE NAME = 0101002-70B99-003-TOS Elevations.dgn 	USER NAME =	DESIGNED - GBR	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TOP OF SLAB ELEVATIONS STRUCTURE NO. 010-1002	F.A.I. RTE. =	SECTION =	COUNTY =	TOTAL SHEETS =	SHEET NO. =
	BACON FARMER WORKMAN ENGINEERING & TESTING, INC. 433 NORTH COLT ST. SUITE 100 MORGAN, ILLINOIS 61850 PHONE: 815.897.8100	PLOT SCALE =	DRAWN - BJV			REVISED -	57/74	(10-34-1)HBK	CHAMPAIGN	1187
	PLOT DATE = 3/11/2021	CHECKED - GBR	REVISED -		SHEET NO. 3 OF 30 SHEETS	CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT		

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	723+59.63	-20.38	792.41	792.44
☉ S. Abut.	723+61.57	-20.30	792.46	792.48
A	723+71.44	-19.93	792.67	792.74
B	723+81.31	-19.63	792.87	793.00
C	723+91.19	-19.38	793.08	793.25
D	724+01.07	-19.20	793.29	793.49
E	724+10.96	-19.07	793.49	793.71
F	724+20.84	-19.01	793.69	793.93
G	724+30.73	-19.01	793.89	794.12
H	724+40.61	-19.06	794.09	794.31
I	724+50.50	-19.18	794.29	794.48
J	724+60.38	-19.36	794.48	794.63
K	724+70.26	-19.60	794.67	794.78
L	724+80.13	-19.90	794.86	794.92
☉ N. Abut.	724+86.14	-20.11	794.98	795.00
Bk. N. Abut.	724+88.07	-20.18	795.01	795.03

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	723+59.39	-14.38	792.01	792.03
☉ S. Abut.	723+61.33	-14.30	792.05	792.07
A	723+71.24	-13.94	792.26	792.33
B	723+81.15	-13.63	792.47	792.59
C	723+91.06	-13.38	792.68	792.84
D	724+00.98	-13.20	792.88	793.08
E	724+10.90	-13.07	793.09	793.31
F	724+20.82	-13.01	793.29	793.52
G	724+30.74	-13.01	793.49	793.72
H	724+40.66	-13.06	793.69	793.91
I	724+50.58	-13.18	793.88	794.07
J	724+60.50	-13.36	794.08	794.23
K	724+70.42	-13.60	794.27	794.38
L	724+80.33	-13.90	794.46	794.52
☉ N. Abut.	724+86.36	-14.11	794.58	794.60
Bk. N. Abut.	724+88.30	-14.18	794.62	794.64

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	724+59.15	-8.39	791.60	791.62
☉ S. Abut.	723+61.09	-8.31	791.64	791.66
A	723+71.04	-7.94	791.85	791.93
B	723+80.98	-7.63	792.06	792.19
C	723+90.94	-7.38	792.27	792.44
D	724+00.89	-7.20	792.48	792.68
E	724+10.85	-7.07	792.68	792.91
F	724+20.80	-7.01	792.89	793.12
G	724+30.76	-7.01	793.09	793.32
H	724+40.72	-7.06	793.29	793.50
I	724+50.67	-7.18	793.48	793.67
J	724+60.63	-7.36	793.68	793.83
K	724+70.58	-7.60	793.87	793.98
L	724+80.53	-7.90	794.07	794.12
☉ N. Abut.	724+86.58	-8.12	794.18	794.20
Bk. N. Abut.	724+88.52	-8.19	794.22	794.24

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	723+58.90	-2.39	791.19	791.21
☉ S. Abut.	723+60.85	-2.31	791.23	791.25
A	723+70.83	-1.94	791.45	791.52
B	723+80.82	-1.63	791.66	791.78
C	723+90.81	-1.39	791.87	792.04
D	724+00.80	-1.20	792.07	792.28
E	724+10.79	-1.07	792.28	792.51
F	724+20.78	-1.01	792.48	792.72
G	724+30.78	-1.01	792.69	792.92
H	724+40.77	-1.06	792.89	793.10
I	724+50.76	-1.18	793.08	793.27
J	724+60.75	-1.36	793.28	793.43
K	724+70.74	-1.60	793.47	793.58
L	724+80.73	-1.91	793.67	793.72
☉ N. Abut.	724+86.80	-2.12	793.78	793.80
Bk. N. Abut.	724+88.75	-2.19	793.82	793.84

B RAMP G & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	723+58.80	0.00	791.03	791.05
☉ S. Abut.	723+60.76	0.00	791.08	791.10
A	723+70.77	0.00	791.31	791.39
B	723+80.77	0.00	791.55	791.67
C	723+90.77	0.00	791.77	791.94
D	724+00.78	0.00	791.99	792.20
E	724+10.78	0.00	792.21	792.43
F	724+20.78	0.00	792.42	792.65
G	724+30.78	0.00	792.62	792.85
H	724+40.78	0.00	792.82	793.03
I	724+50.78	0.00	793.01	793.20
J	724+60.78	0.00	793.19	793.34
K	724+70.78	0.00	793.37	793.47
L	724+80.79	0.00	793.54	793.59
☉ N. Abut.	724+86.87	0.00	793.64	793.66
Bk. N. Abut.	724+88.83	0.00	793.67	793.70

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	723+58.65	3.60	790.78	790.80
☉ S. Abut.	723+60.61	3.68	790.83	790.85
A	723+70.63	4.05	791.04	791.11
B	723+80.65	4.36	791.25	791.38
C	723+90.68	4.61	791.46	791.63
D	724+00.70	4.80	791.67	791.87
E	724+10.73	4.93	791.88	792.10
F	724+20.76	4.99	792.08	792.32
G	724+30.79	4.99	792.28	792.52
H	724+40.82	4.94	792.49	792.70
I	724+50.85	4.82	792.68	792.87
J	724+60.88	4.64	792.88	793.03
K	724+70.90	4.39	793.08	793.18
L	724+80.93	4.09	793.27	793.32
☉ N. Abut.	724+87.02	3.88	793.38	793.41
Bk. N. Abut.	724+88.98	3.80	793.42	793.44

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	723+58.40	9.60	790.38	790.40
☉ S. Abut.	723+60.37	9.68	790.42	790.44
A	723+70.42	10.05	790.63	790.71
B	723+80.48	10.36	790.85	790.97
C	723+90.54	10.61	791.06	791.23
D	724+00.61	10.80	791.27	791.47
E	724+10.67	10.93	791.47	791.70
F	724+20.74	10.99	791.68	791.91
G	724+30.81	10.99	791.88	792.12
H	724+40.88	10.94	792.08	792.30
I	724+50.94	10.82	792.28	792.47
J	724+61.01	10.63	792.48	792.63
K	724+71.07	10.39	792.68	792.78
L	724+81.13	10.09	792.87	792.92
☉ N. Abut.	724+87.24	9.87	792.99	793.01
Bk. N. Abut.	724+89.21	9.80	793.02	793.04

WEST EDGE SHOULDER/INSIDE FACE OF CURB OR PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	723+31.07	-22.36	791.83	791.85
A1	723+40.93	-22.01	792.06	792.08
A2	723+50.80	-21.73	792.29	792.31
N. End of S. Appr. Slab	723+60.67	-21.50	792.51	792.54

WEST EDGE OF ROADWAY

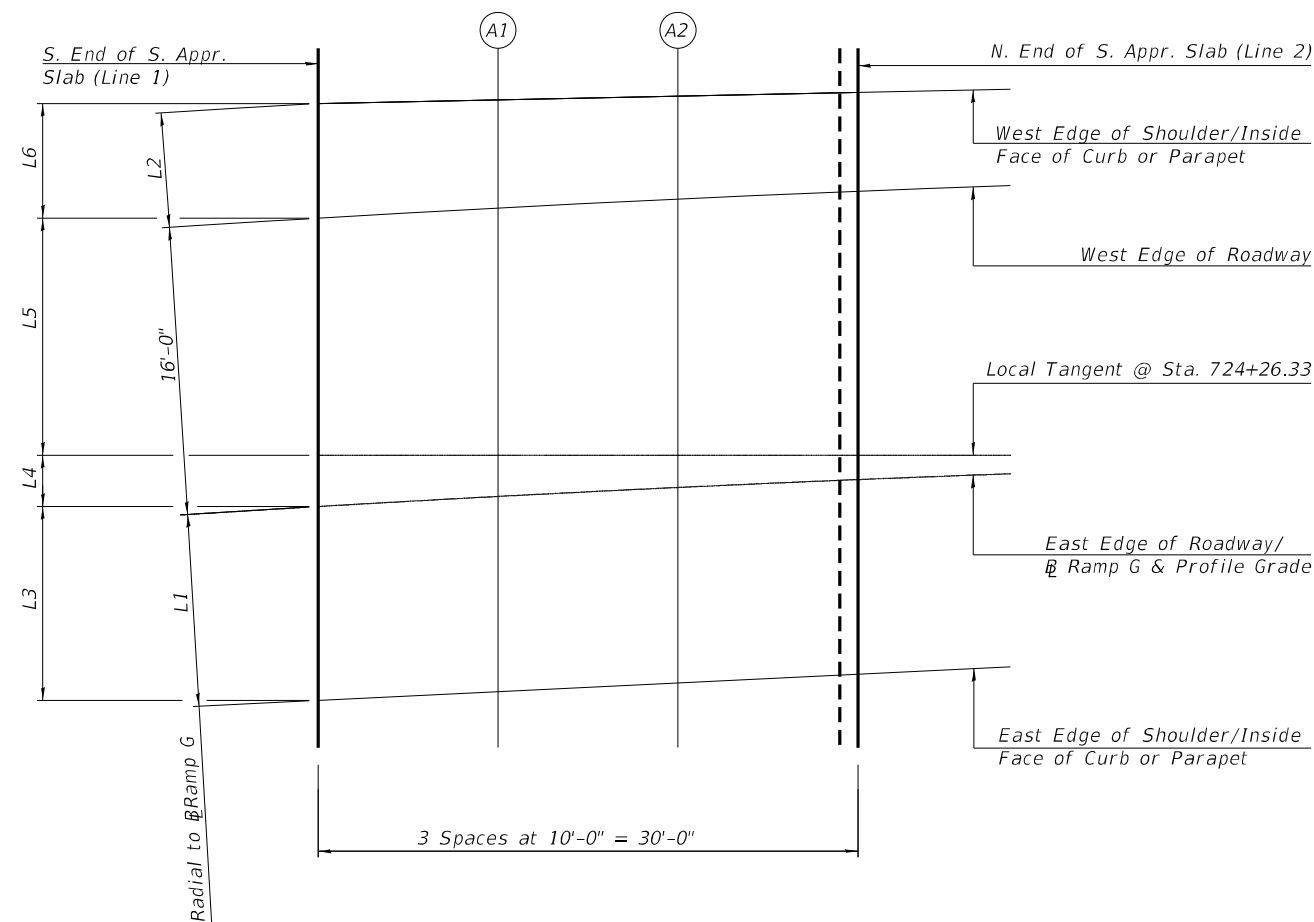
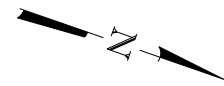
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	723+30.70	-16.00	791.40	791.42
A1	723+40.62	-16.00	791.65	791.67
A2	723+50.53	-16.00	791.90	791.92
N. End of S. Appr. Slab	723+60.45	-16.00	792.14	792.16

EAST EDGE OF ROADWAY/RAMP G & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	723+29.76	0.00	790.30	790.32
A1	723+39.78	0.00	790.56	790.58
A2	723+49.79	0.00	790.81	790.83
N. End of S. Appr. Slab	723+59.80	0.00	791.05	791.07

EAST EDGE OF SHOULDER/INSIDE FACE OF CURB OR PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	723+29.12	10.75	789.56	789.58
A1	723+39.20	10.83	789.82	789.84
A2	723+49.28	10.85	790.07	790.09
N. End of S. Appr. Slab	723+59.36	10.80	790.32	790.34



PLAN

SPACING FOR TOP OF APPROACH ELEVATIONS

LINE	L1	L2	L3	L4	L5	L6
1	10'-9"	6'-4 ¹ / ₈ "	10'-9 ¹ / ₈ "	2'-10 ¹ / ₄ "	13'-2 ¹ / ₄ "	6'-4 ³ / ₈ "
A1	10'-10"	6'-0 ¹ / ₈ "	10'-10 ¹ / ₈ "	2'-3 ¹ / ₂ "	13'-8 ³ / ₄ "	6'-0 ¹ / ₄ "
A2	10'-10 ¹ / ₈ "	5'-8 ³ / ₄ "	10'-10 ³ / ₈ "	1'-9 ¹ / ₂ "	14'-2 ³ / ₄ "	5'-8 ³ / ₄ "
2	10'-9 ⁵ / ₈ "	5'-6"	10'-9 ³ / ₄ "	1'-4 ¹ / ₄ "	14'-7 ⁷ / ₈ "	5'-6 ¹ / ₈ "

WEST EDGE SHOULDER/INSIDE FACE OF CURB OR PARAPET

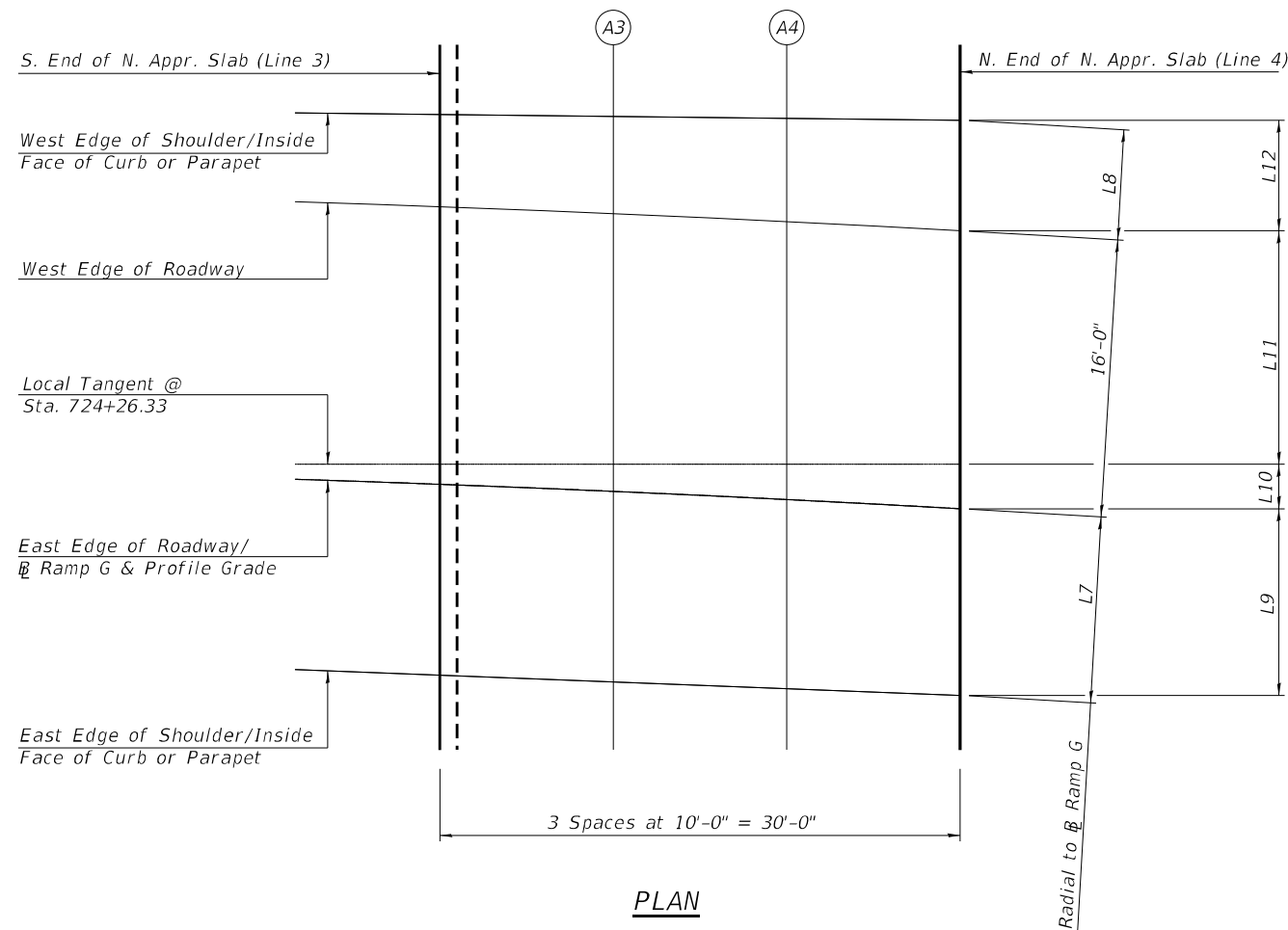
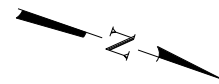
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	724+87.04	-21.31	795.07	795.09
A3	724+96.91	-21.60	795.25	795.27
A4	725+06.77	-21.95	795.43	795.45
N. End of N. Appr. Slab	725+16.63	-22.37	795.61	795.63

WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	724+87.24	-16.00	794.72	794.74
A3	724+97.15	-16.00	794.88	794.90
A4	725+07.06	-16.00	795.04	795.06
N. End of N. Appr. Slab	725+16.98	-16.00	795.18	795.21

EAST EDGE OF ROADWAY/RAMP G & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	724+87.83	0.00	793.66	793.68
A3	724+97.84	0.00	793.82	793.84
A4	725+07.85	0.00	793.98	794.00
N. End of N. Appr. Slab	725+17.87	0.00	794.13	794.15

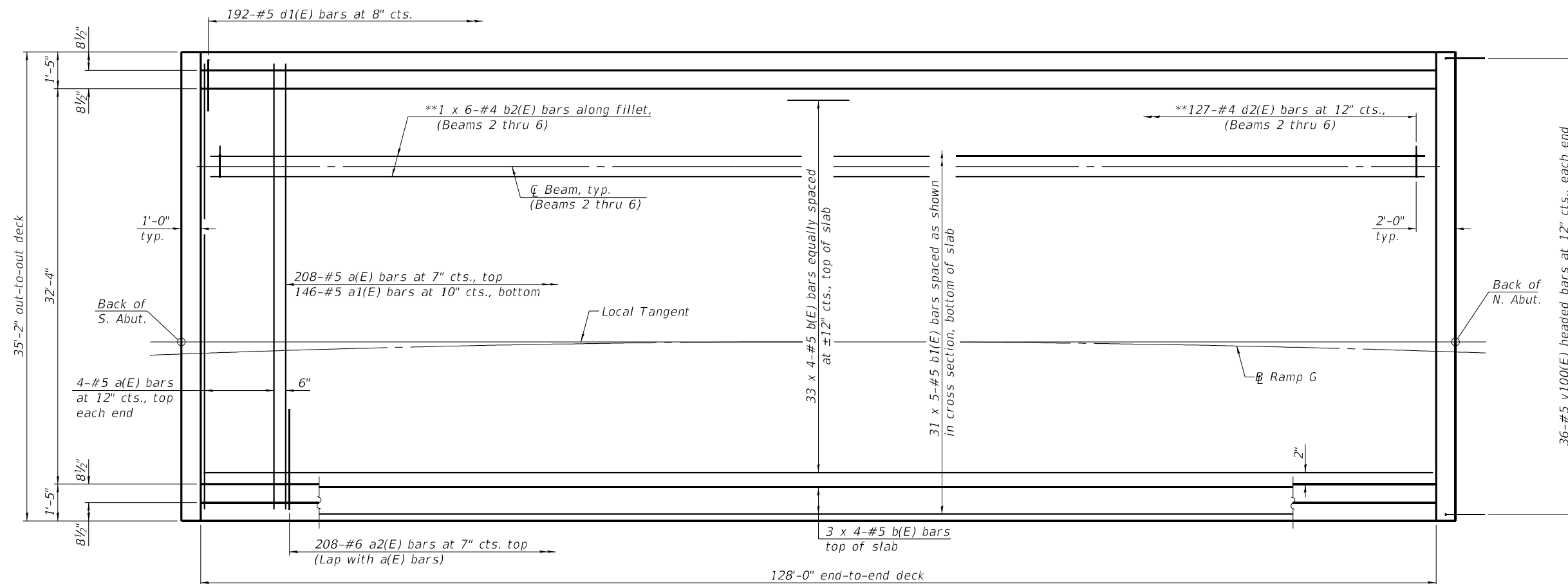


EAST EDGE OF SHOULDER/INSIDE FACE OF CURB OR PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	724+88.25	11.00	792.93	792.95
A3	724+98.33	10.98	793.09	793.11
A4	725+08.40	10.90	793.25	793.28
N. End of N. Appr. Slab	725+18.47	10.75	793.41	793.44

SPACING FOR TOP OF APPROACH ELEVATIONS

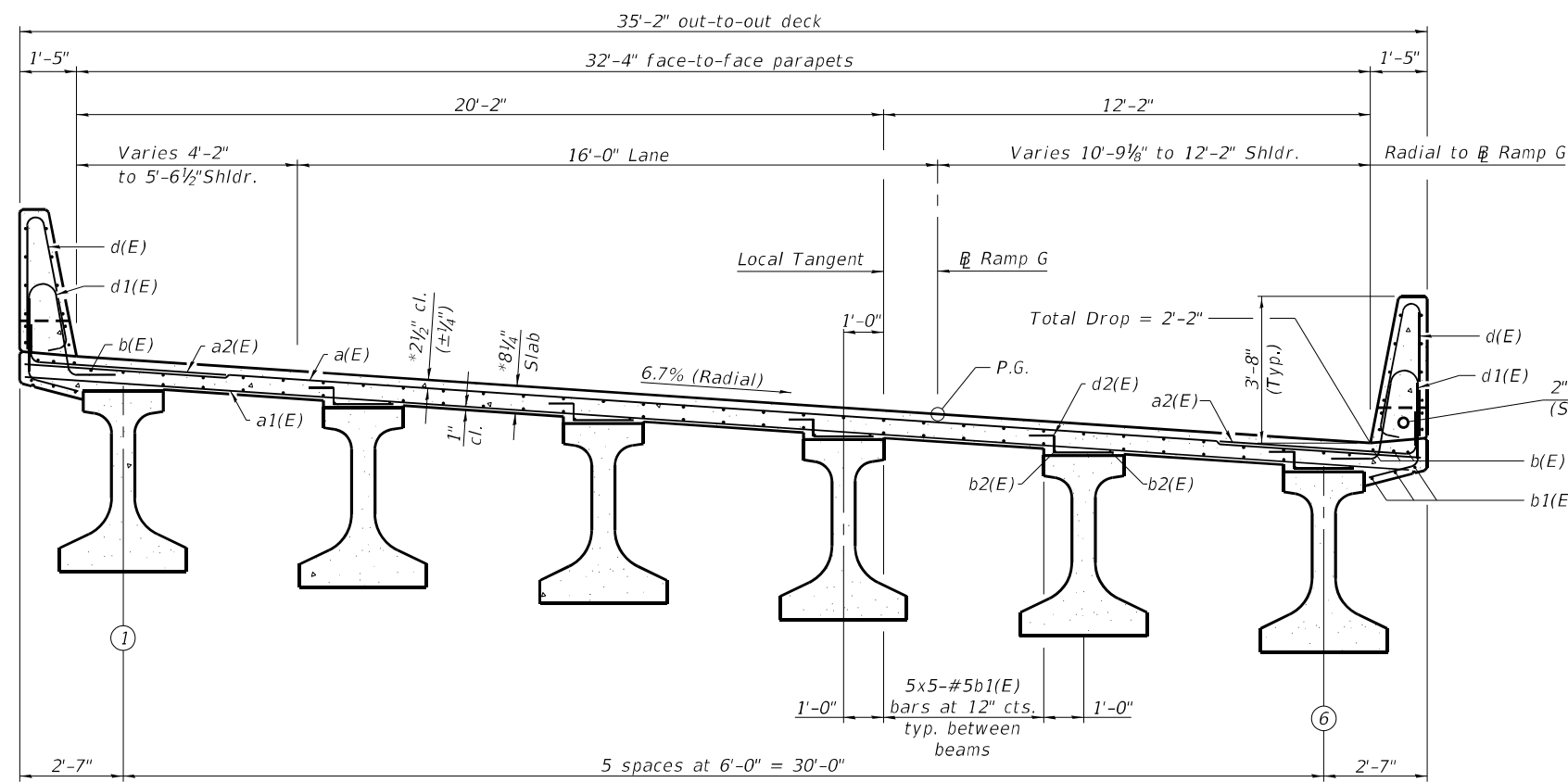
LINE	L7	L8	L9	L10	L11	L12
3	11'-0"	5'-3 ³ / ₄ "	11'-0 ¹ / ₈ "	1'-1 ⁷ / ₈ "	14'-10 ¹ / ₄ "	5'-3 ³ / ₄ "
A3	10'-11 ³ / ₄ "	5'-7 ¹ / ₂ "	10'-11 ⁷ / ₈ "	1'-6 ³ / ₄ "	14'-5 ³ / ₈ "	5'-7 ³ / ₈ "
A4	10'-10 ³ / ₄ "	5'-11 ³ / ₈ "	10'-10 ⁷ / ₈ "	2'-0 ³ / ₈ "	13'-11 ⁷ / ₈ "	5'-11 ¹ / ₂ "
4	10'-9"	6'-4 ³ / ₈ "	10'-9 ¹ / ₄ "	2'-6 ³ / ₄ "	13'-5 ¹ / ₂ "	6'-4 ¹ / ₂ "



PLAN

MINIMUM BAR LAP

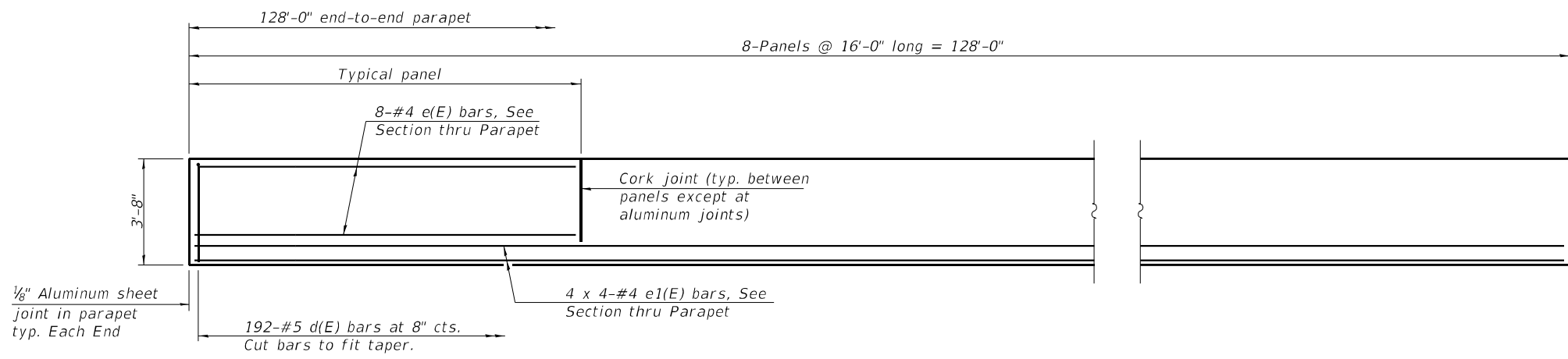
#4 bar = 2'-5"
#5 bar = 3'-6"



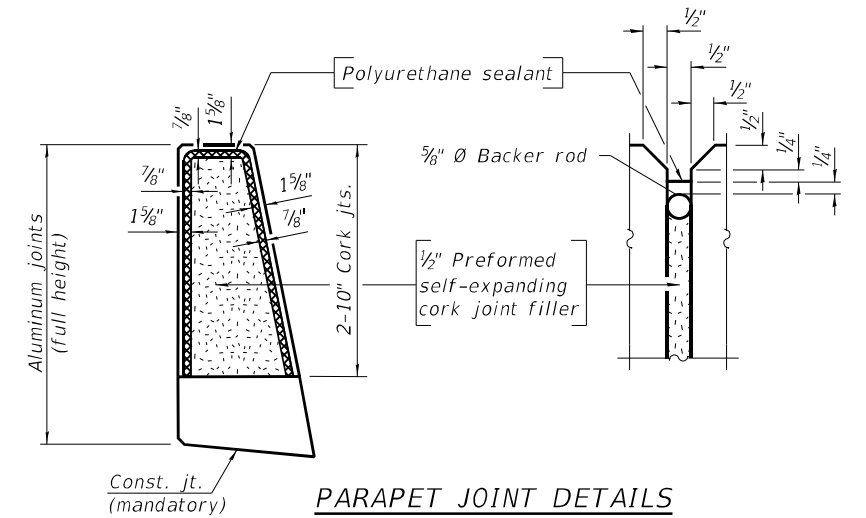
CROSS SECTION
(Looking North)

Notes:
See sheet 9 of 30 for superstructure details and Bill of Material.
Bars indicated thus 33 x 4-#5 etc. indicates 33 lines of bars with 4 lengths per line.

* Prior to Grinding
** See sheet 9 of 30 for Fillet Reinforcement Details

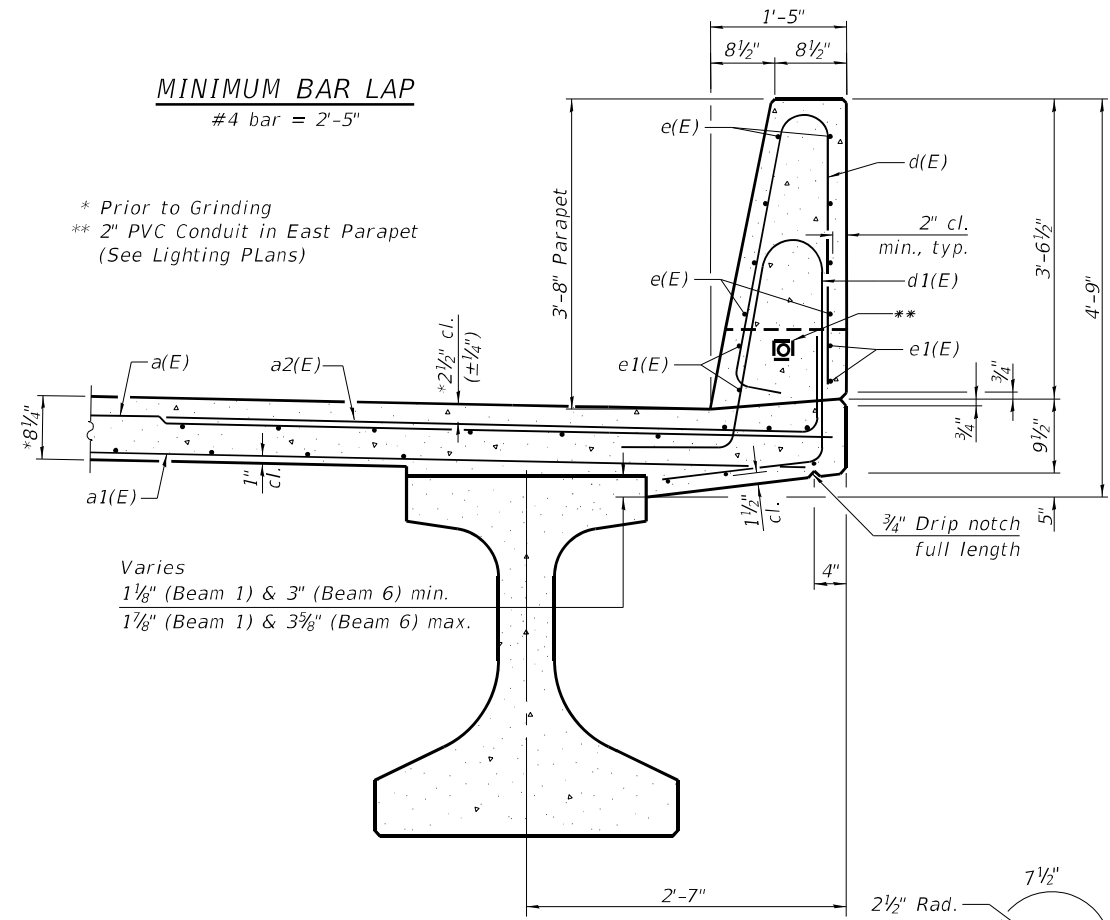


INSIDE ELEVATION OF PARAPET

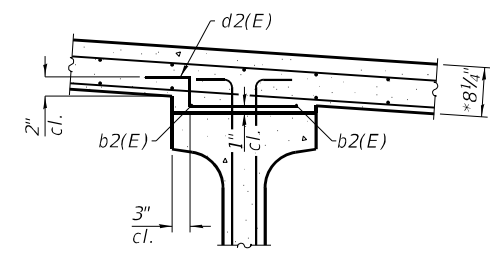


PARAPET JOINT DETAILS

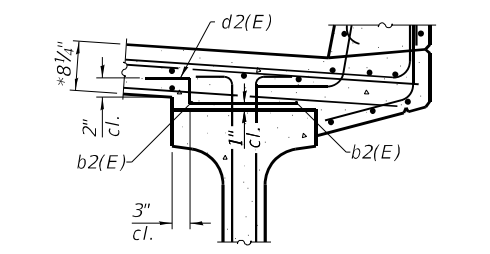
Notes:
 The Polyurethane Sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.
 The 1/8" Aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.
 Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated.



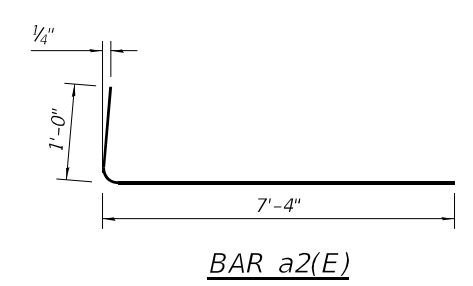
SECTION THRU PARAPET



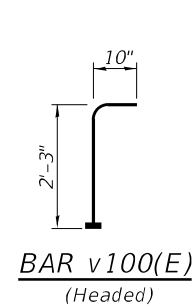
FILLET REINFORCEMENT DETAIL (Beams 2 thru 5)



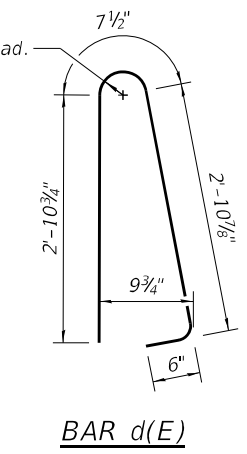
FILLET REINFORCEMENT DETAIL (Beam 6)



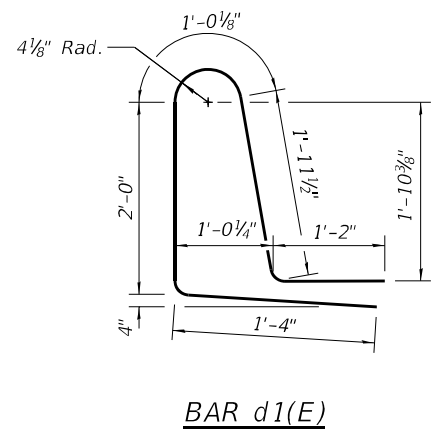
BAR a2(E)



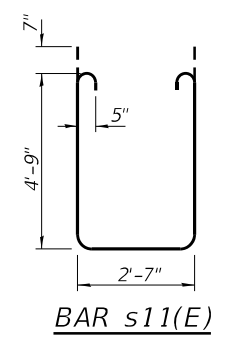
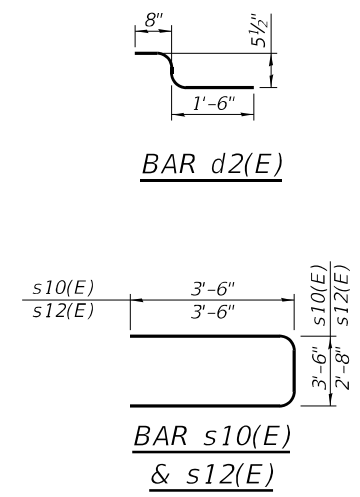
BAR v100(E) (Headed)



BAR d(E)



BAR d1(E)

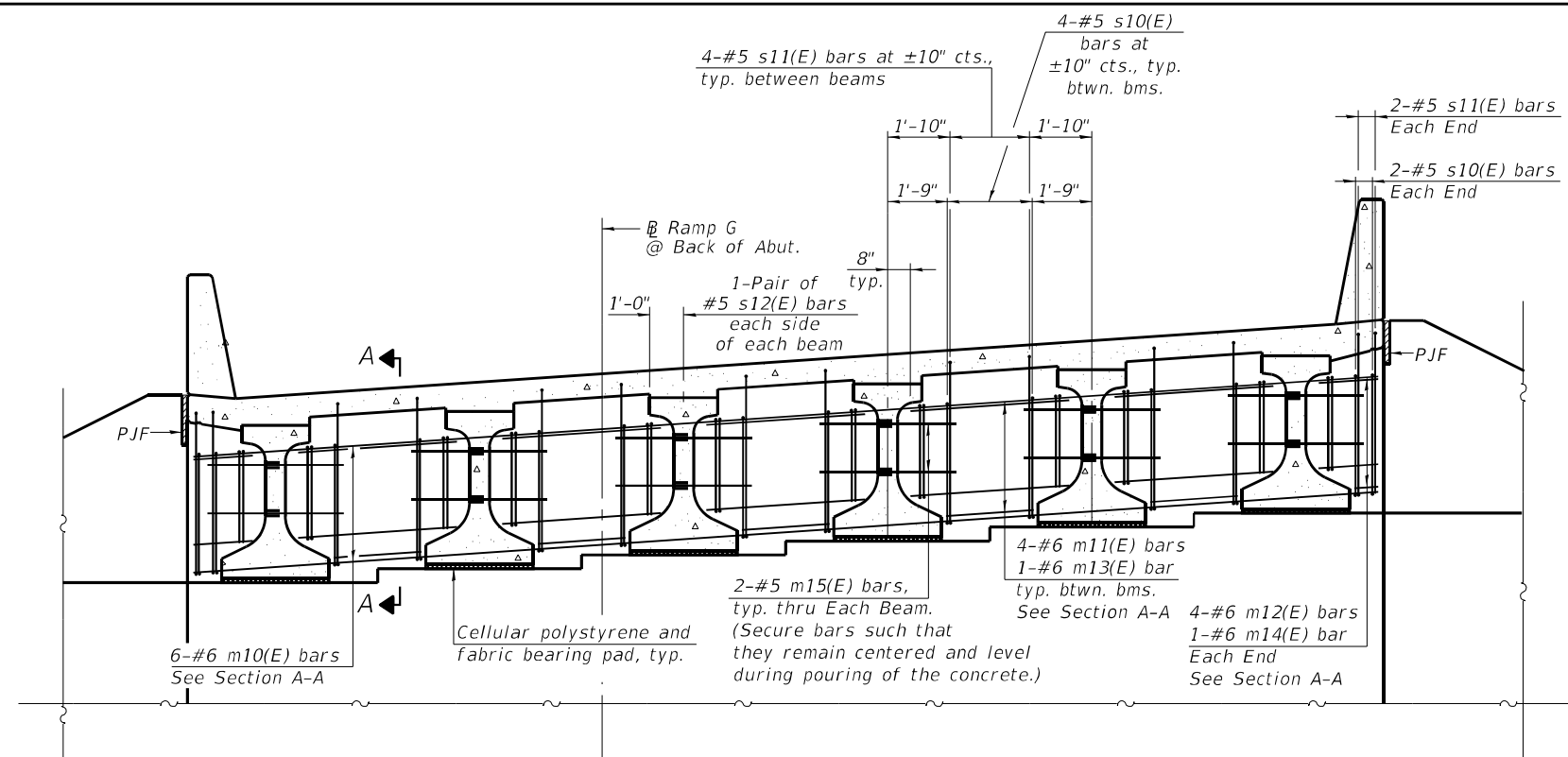


BAR s11(E)

SUPERSTRUCTURE BILL OF MATERIAL

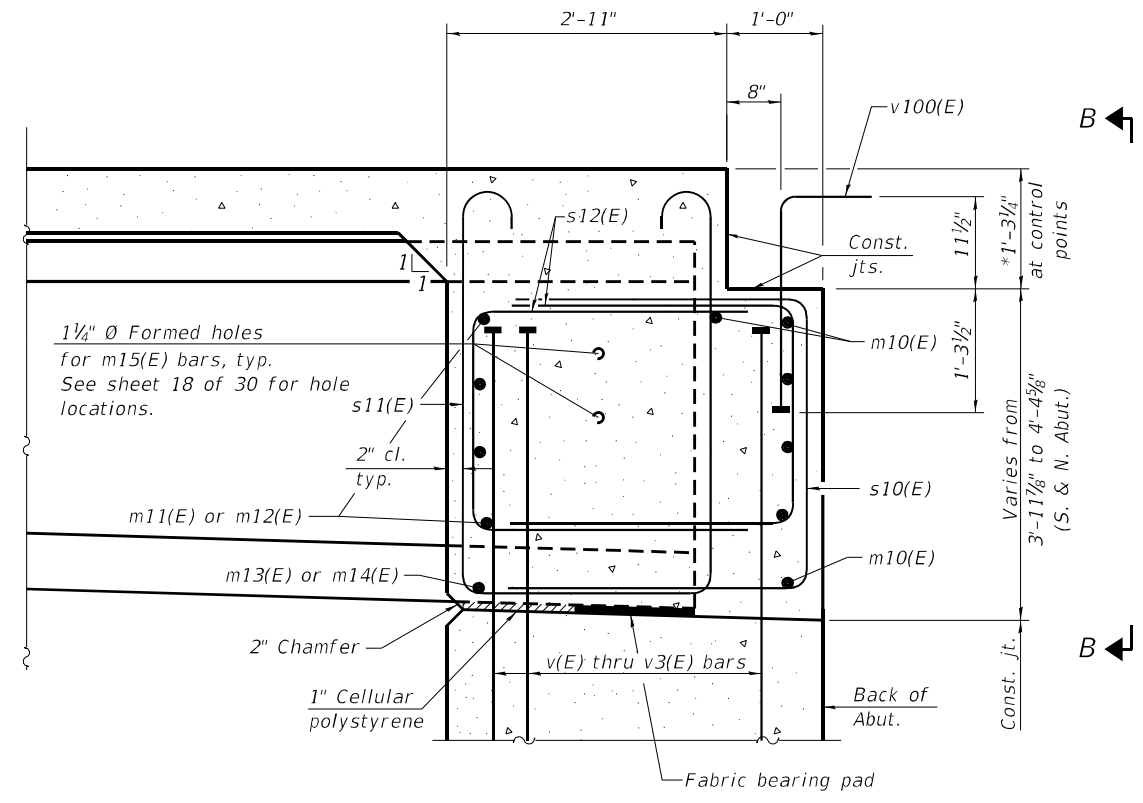
Bar	No.	Size	Length	Shape
a(E)	216	#5	35'-0"	—
a1(E)	146	#5	34'-8"	—
a2(E)	416	#6	8'-4"	—
b(E)	156	#5	34'-7"	—
b1(E)	155	#5	28'-4"	—
b2(E)	60	#4	23'-3"	—
d(E)	384	#5	7'-0"	⏏
d1(E)	384	#5	7'-6"	⏏
d2(E)	635	#4	2'-8"	⏏
e(E)	128	#4	15'-8"	—
e1(E)	32	#4	33'-9"	—
m10(E)	12	#6	34'-11"	—
m11(E)	40	#6	4'-8"	—
m12(E)	16	#6	1'-9"	—
m13(E)	10	#6	2'-7"	—
m14(E)	4	#6	9"	—
m15(E)	24	#5	4'-0"	—
s10(E)	48	#5	10'-6"	⏏
s11(E)	48	#5	13'-3"	⏏
s12(E)	48	#5	9'-8"	⏏
v100(E)	72	#5	3'-1"	⏏
Reinforcement Bars, Epoxy Coated		Lbs.		41510
Concrete Superstructure		Cu. Yds.		203.8

Bars indicated thus 4 x 4-#4 etc. indicates 4 line of bars with 4 lengths per line.



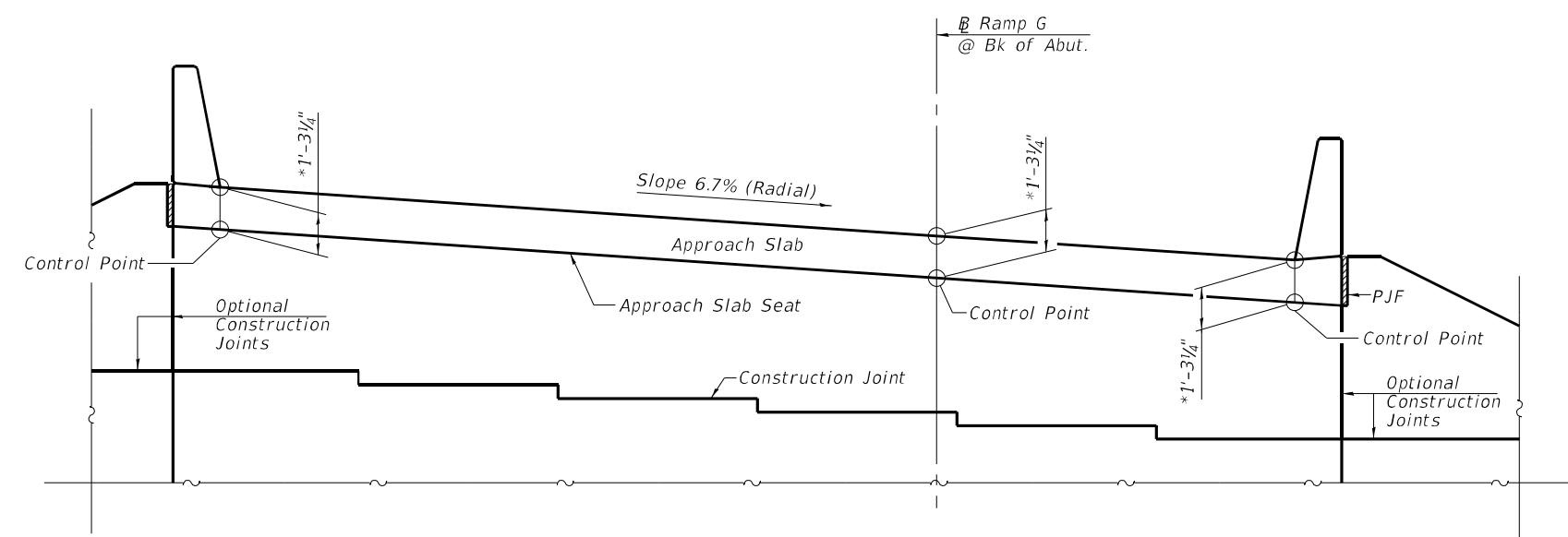
DIAPHRAGM AT ABUTMENT
 (South Abutment Shown - Looking South)
 (North Abutment Similar)

* Prior to Grinding

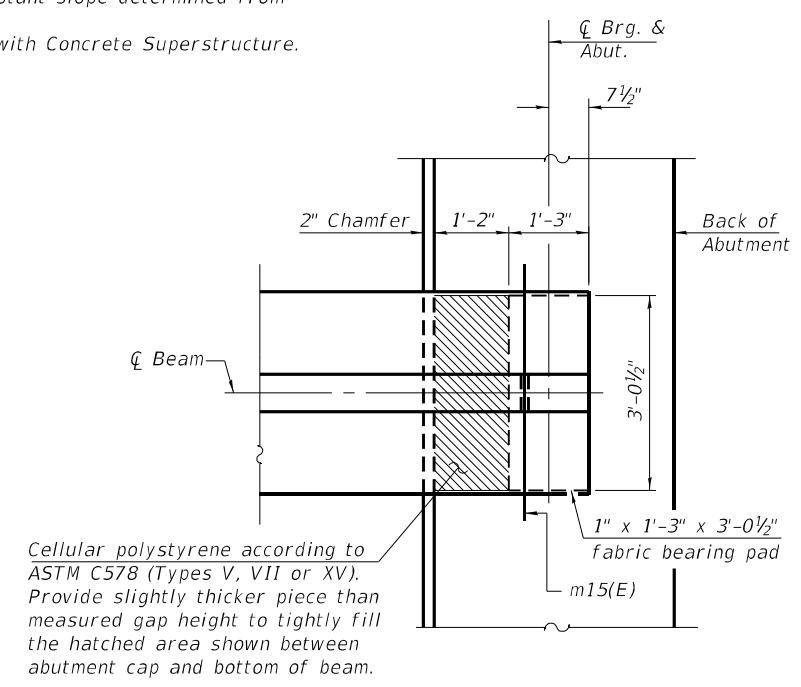


SECTION A-A

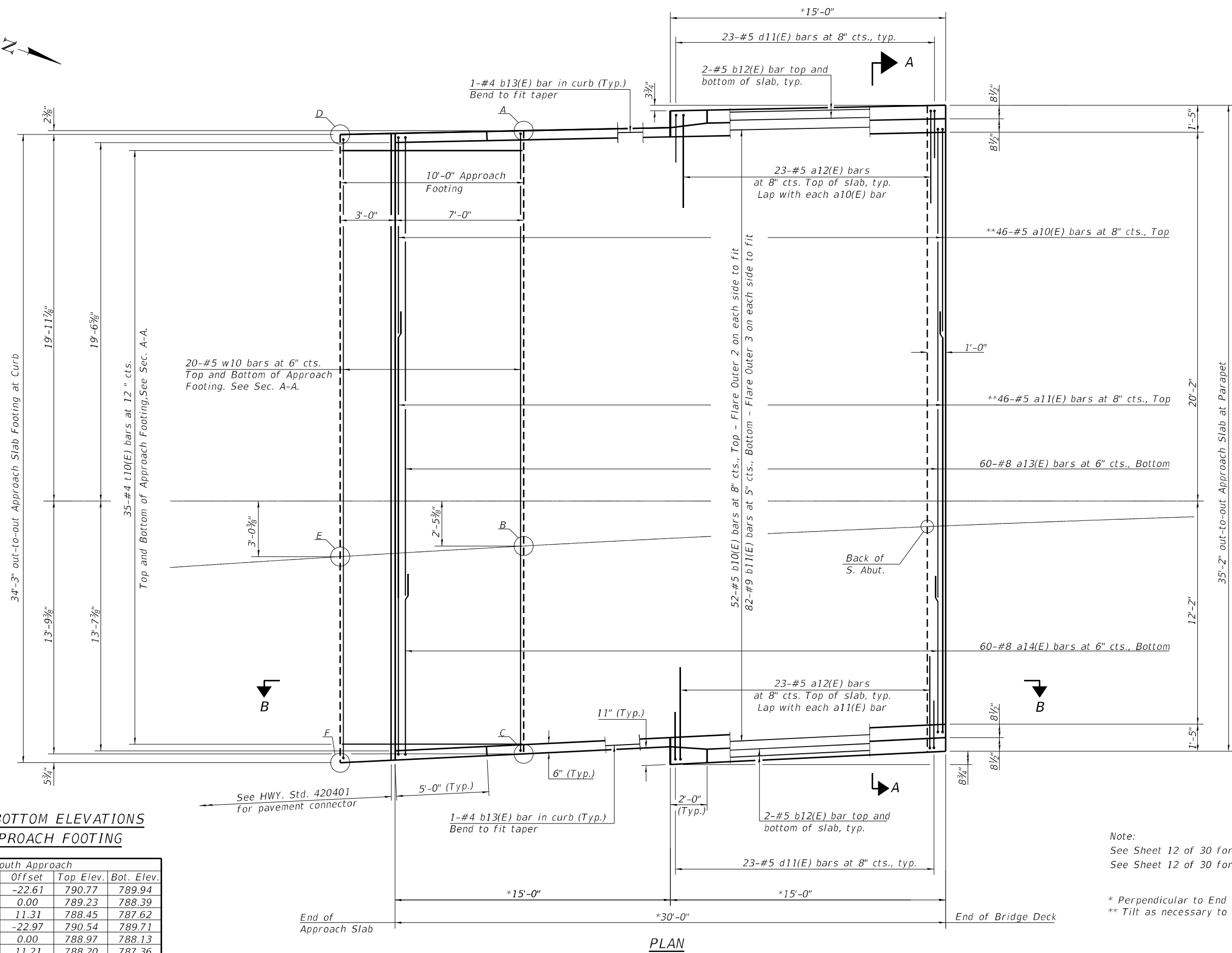
Notes:
 See sheet 9 of 30 for superstructure details and Bill of Material
 See sheet 12 and 15 of 30 for P.J.F. details
 The approach slab seat shall have a constant slope determined from the control points shown.
 Cost of cellular polystyrene is included with Concrete Superstructure.



VIEW B-B



PLAN AT ABUTMENT
 (Showing bottom flange of beam)



MINIMUM BAR LAP
 #5 bar = 3'-6"
 #8 bar = 4'-9"

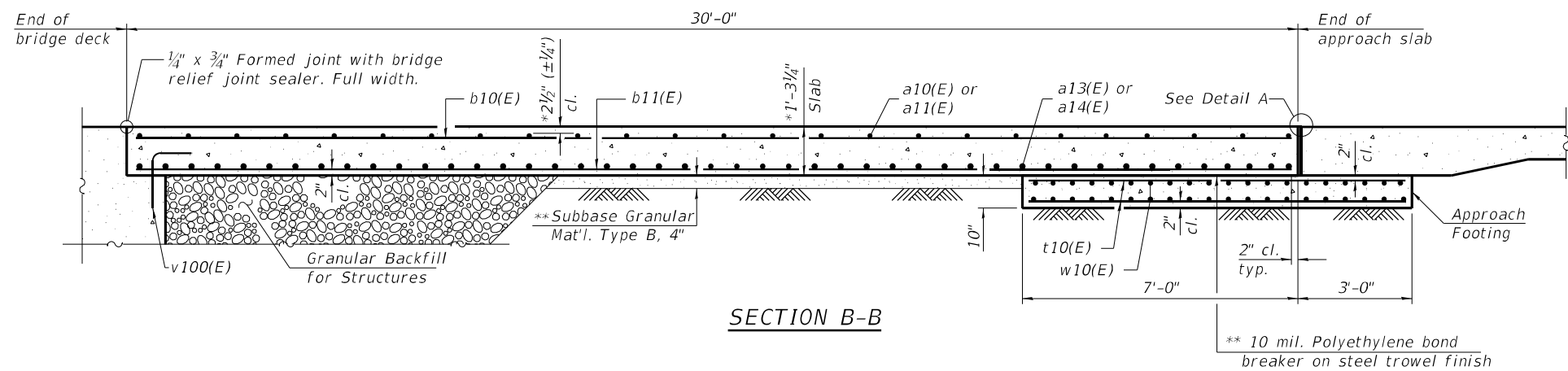
**TOP AND BOTTOM ELEVATIONS
 FOR APPROACH FOOTING**

South Approach				
Point	Station	Offset	Top Elev.	Bot. Elev.
A	723+38.00	-22.61	790.77	789.94
B	723+36.78	0.00	789.23	788.39
C	723+36.15	11.31	788.45	787.62
D	723+28.14	-22.97	790.54	789.71
E	723+26.76	0.00	788.97	788.13
F	723+26.07	11.21	788.20	787.36

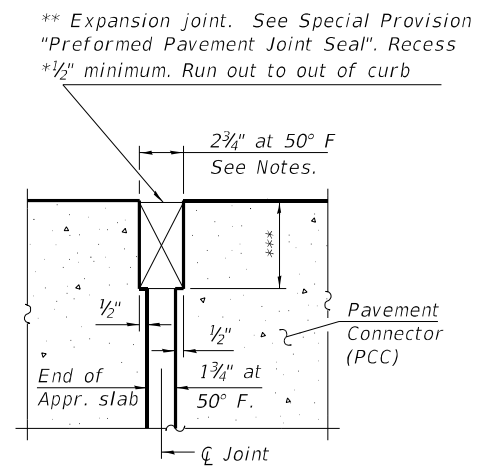
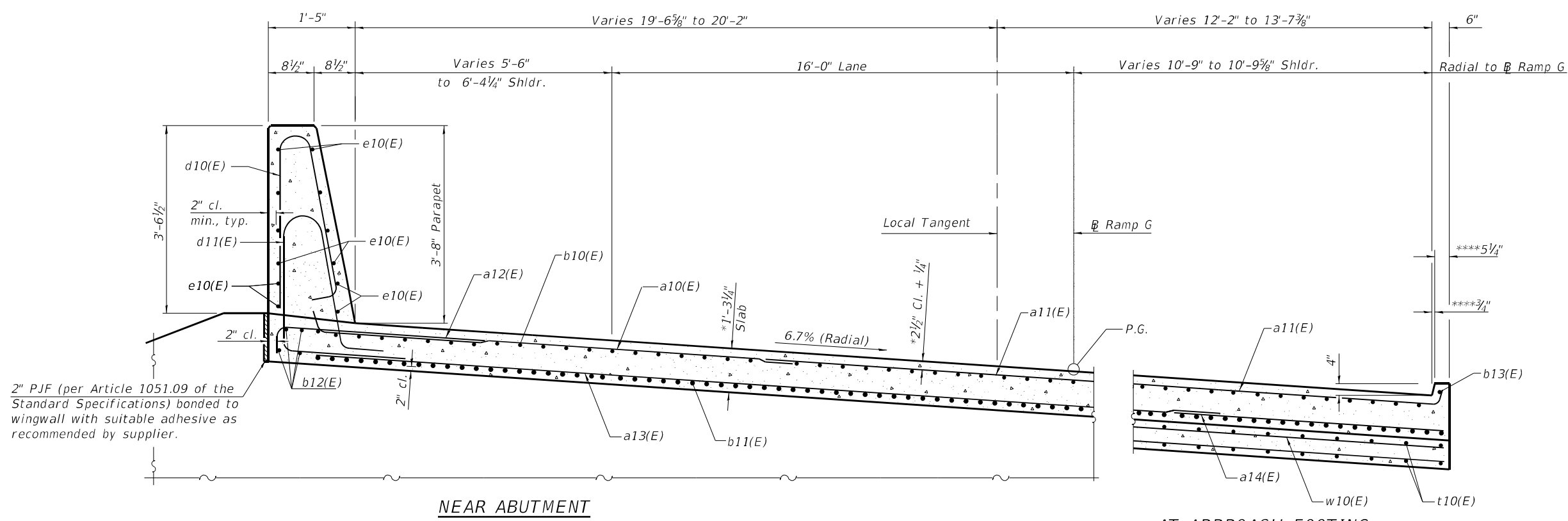
Note:
 See Sheet 12 of 30 for Section A-A.
 See Sheet 12 of 30 for Section B-B.

* Perpendicular to End of Deck and Approach slab
 ** Tilt as necessary to fit curb.

(Sheet 1 of 3)



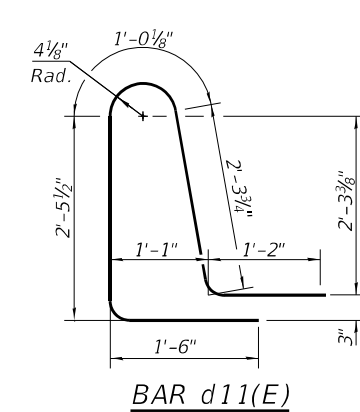
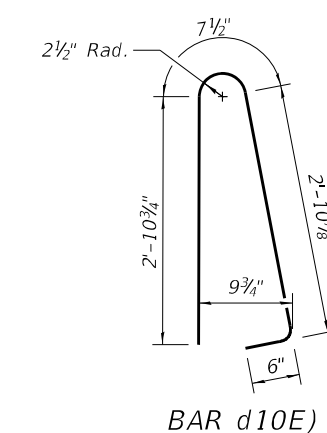
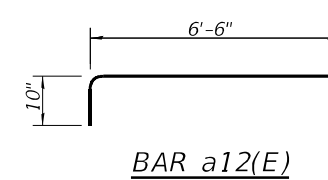
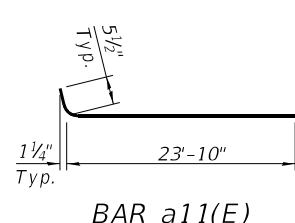
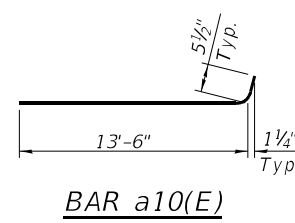
Notes:
 The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total Bridge length plus the length of the bridge approach slab.
 Parapet concrete shall be paid for as Concrete Superstructure.
 Approach slab shall be paid for as Concrete Superstructure (Approach Slab).
 Approach footing concrete shall be paid for as Concrete Structures.
 The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
 Cost of excavation for approach footing included with Concrete Structures.
 For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 30.



SOUTH APPROACH BILL OF MATERIAL

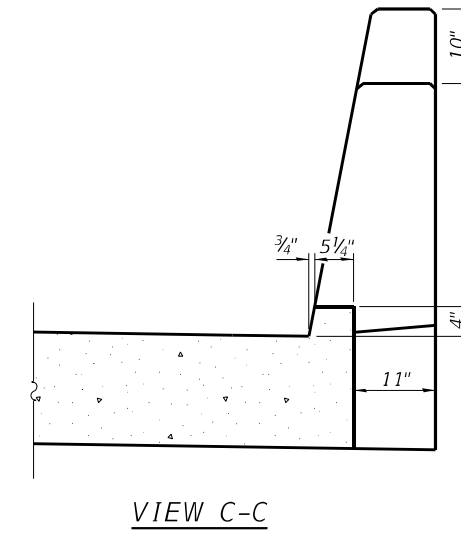
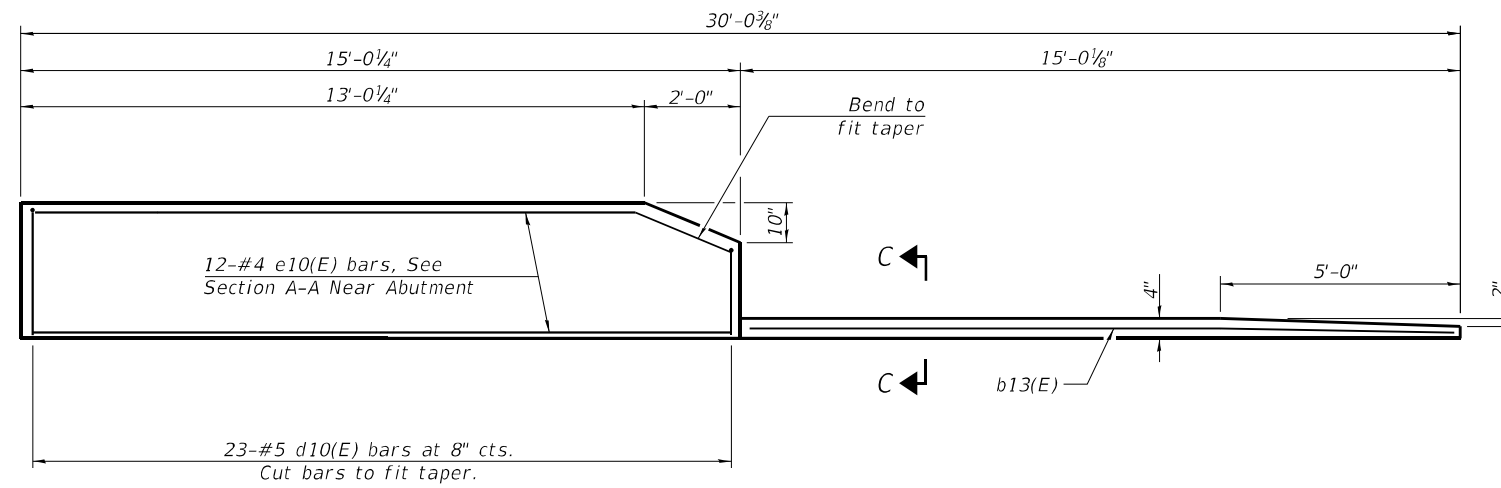
Bar	No.	Size	Length	Shape
a10(E)	46	#5	14'-0"	—
a11(E)	46	#5	24'-4"	—
a12(E)	46	#5	7'-4"	—
a13(E)	60	#8	25'-1"	—
a14(E)	60	#8	13'-6"	—
b10(E)	52	#5	29'-8"	—
b11(E)	82	#9	29'-8"	—
b12(E)	8	#5	14'-8"	—
b13(E)	2	#4	14'-8"	—
d10(E)	46	#5	7'-0"	⌋
d11(E)	46	#5	8'-6"	⌋
e10(E)	24	#4	14'-8"	—
t10(E)	70	#4	9'-8"	—
w10(E)	40	#5	33'-11"	—
Concrete Superstructure			Cu. Yd.	4.3
Concrete Superstructure (Approach Slab)			Cu. Yd.	49.2
Concrete Structures			Cu. Yd.	10.6
Reinforcement Bars, Epoxy Coated			Pound	21240

* Prior to Grinding
 ** Cost included with Concrete Superstructure (Approach Slab).
 *** Per manufacturer recommendations
 **** Perpendicular to inside face of curb.

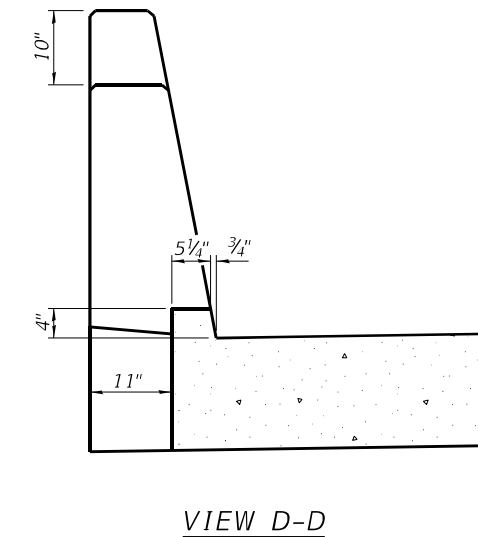
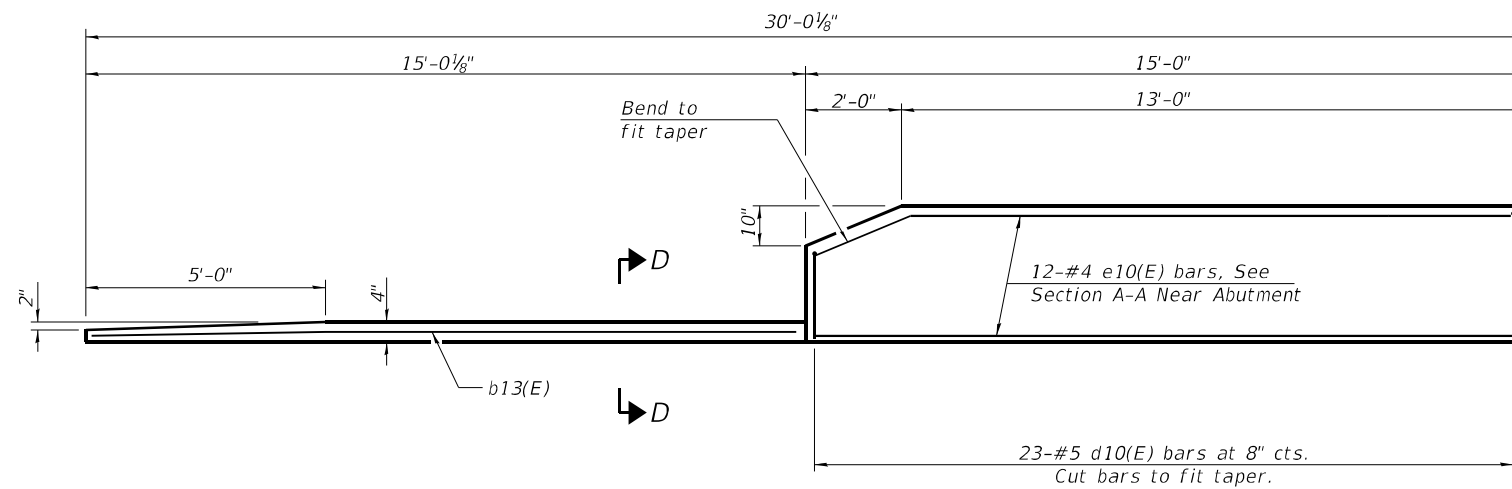


(Sheet 2 of 3)

Note:
For Type 6 terminal connections see Highway Standard 631031.



INSIDE ELEVATION OF EAST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)



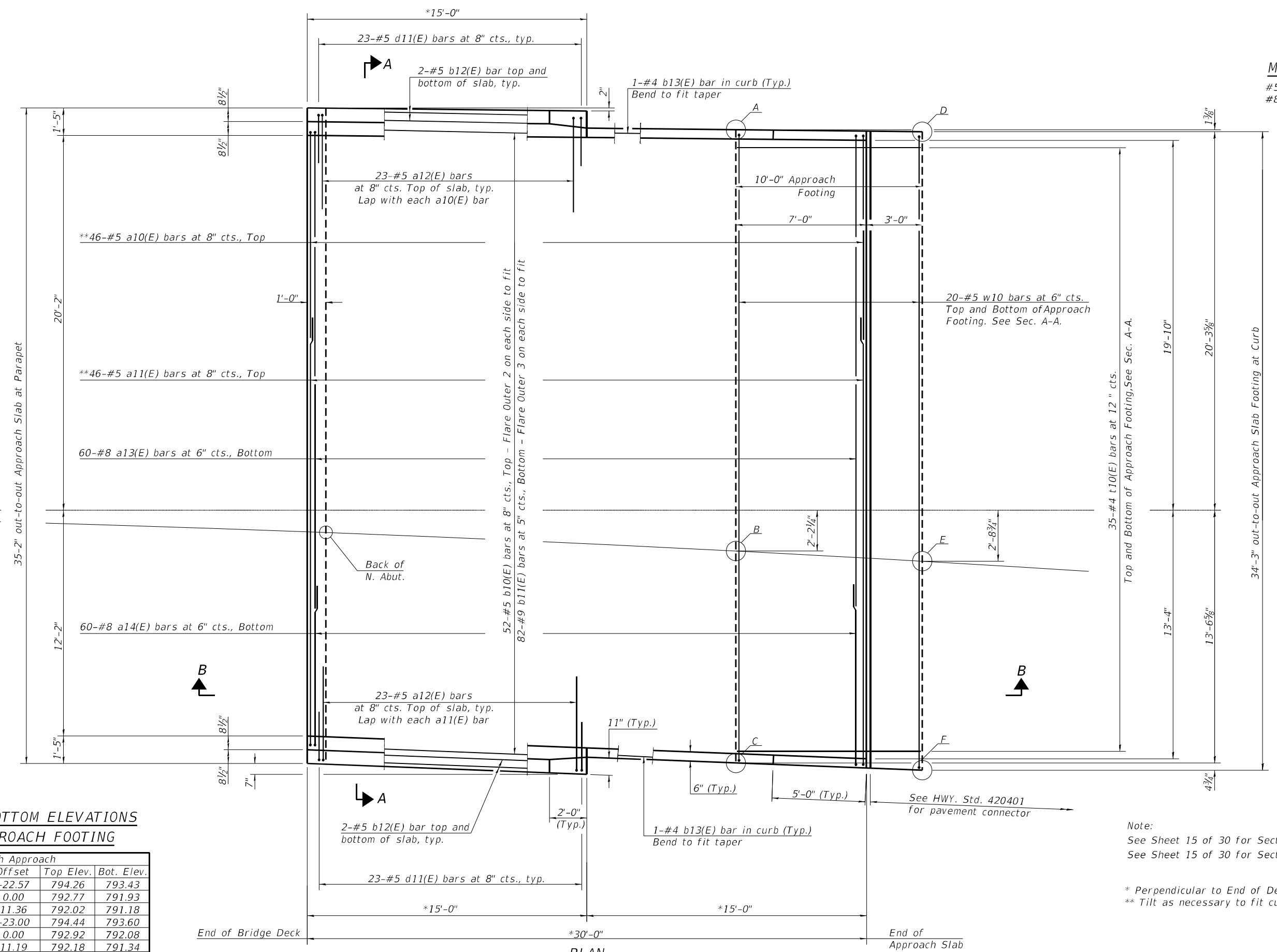
INSIDE ELEVATION OF WEST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)

(Sheet 3 of 3)

FILE NAME = 0101002-70B99-013-S_Appr_Slab_Details.dgn	USER NAME =	DESIGNED - GBR	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SOUTH BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 010-1002	F.A.I. RTE. = 57/74	SECTION = (10-34-1)HBK	COUNTY = CHAMPAIGN	TOTAL SHEETS = 1187	SHEET NO. = 761	
BFW BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - FAM	REVISED -			SHEET NO. 13 OF 30 SHEETS		CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT	
433 NORTH COLT ST. SUITE 100, ELKHART, IN 46517-1000 PHONE: 317.837.8100	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -								
		CHECKED - GBR	REVISED -								



MINIMUM BAR LAP
 #5 bar = 3'-6"
 #8 bar = 4'-9"



**TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING**

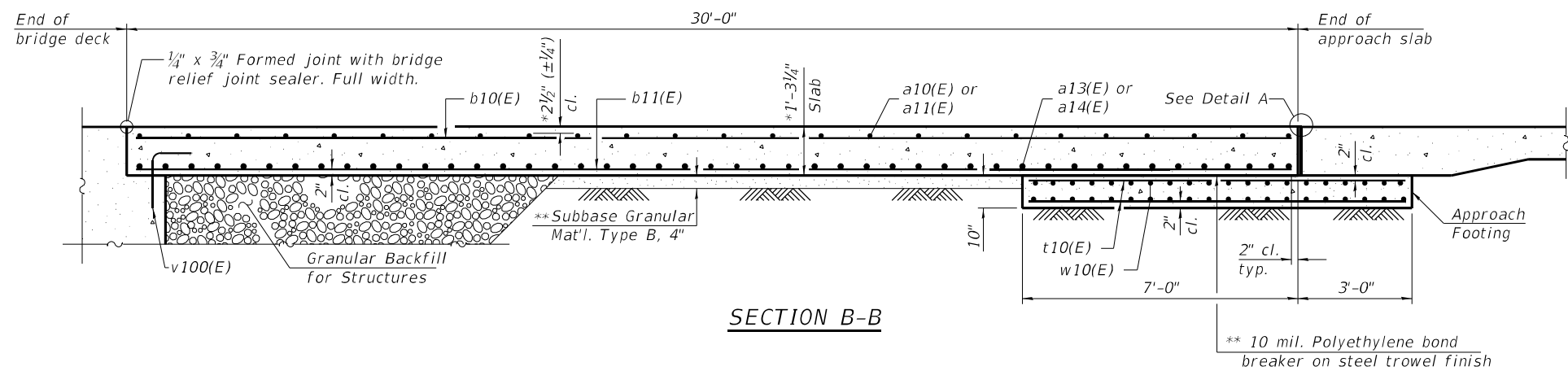
North Approach				
Point	Station	Offset	Top Elev.	Bot. Elev.
A	725+09.71	-22.57	794.26	793.43
B	725+10.86	0.00	792.77	791.93
C	725+11.45	11.36	792.02	791.18
D	725+19.56	-23.00	794.44	793.60
E	725+20.87	0.00	792.92	792.08
F	725+21.53	11.19	792.18	791.34

Note:
 See Sheet 15 of 30 for Section A-A.
 See Sheet 15 of 30 for Section B-B.

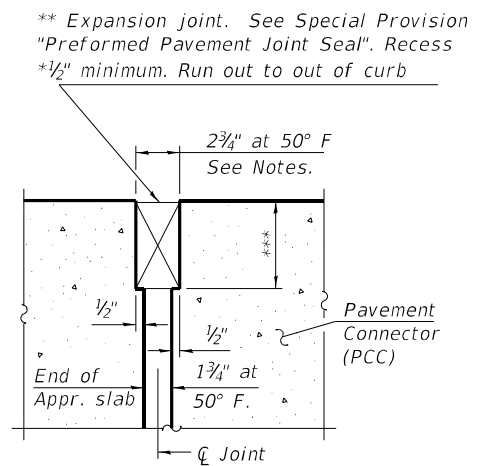
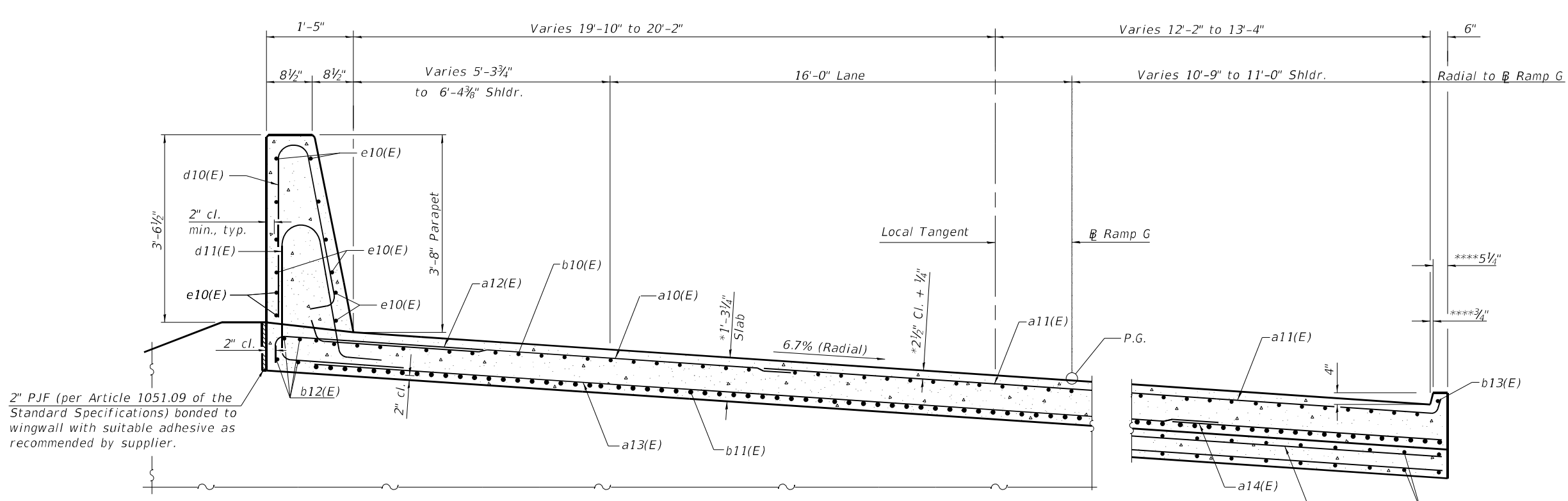
* Perpendicular to End of Deck and Approach slab
 ** Tilt as necessary to fit curb.

PLAN

(Sheet 1 of 3)



Notes:
 The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total Bridge length plus the length of the bridge approach slab.
 Parapet concrete shall be paid for as Concrete Superstructure.
 Approach slab shall be paid for as Concrete Superstructure (Approach Slab).
 Approach footing concrete shall be paid for as Concrete Structures.
 The approach footing maximum applied service bearing pressure (Q_{max}) = 2.0 ksf.
 Cost of excavation for approach footing included with Concrete Structures.
 For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 30.

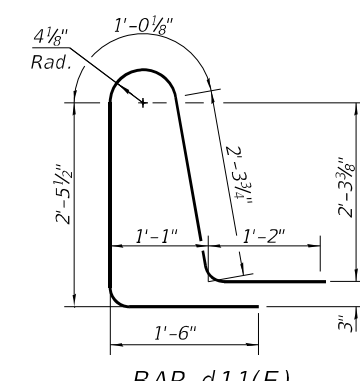
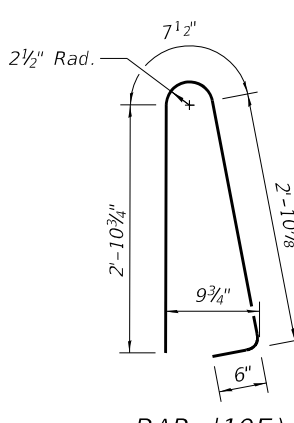
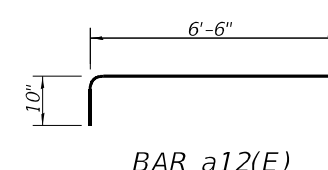
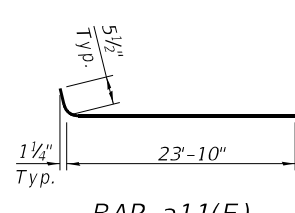
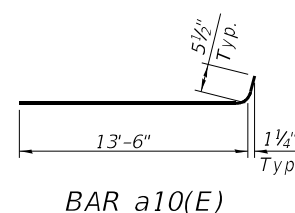


DETAIL A

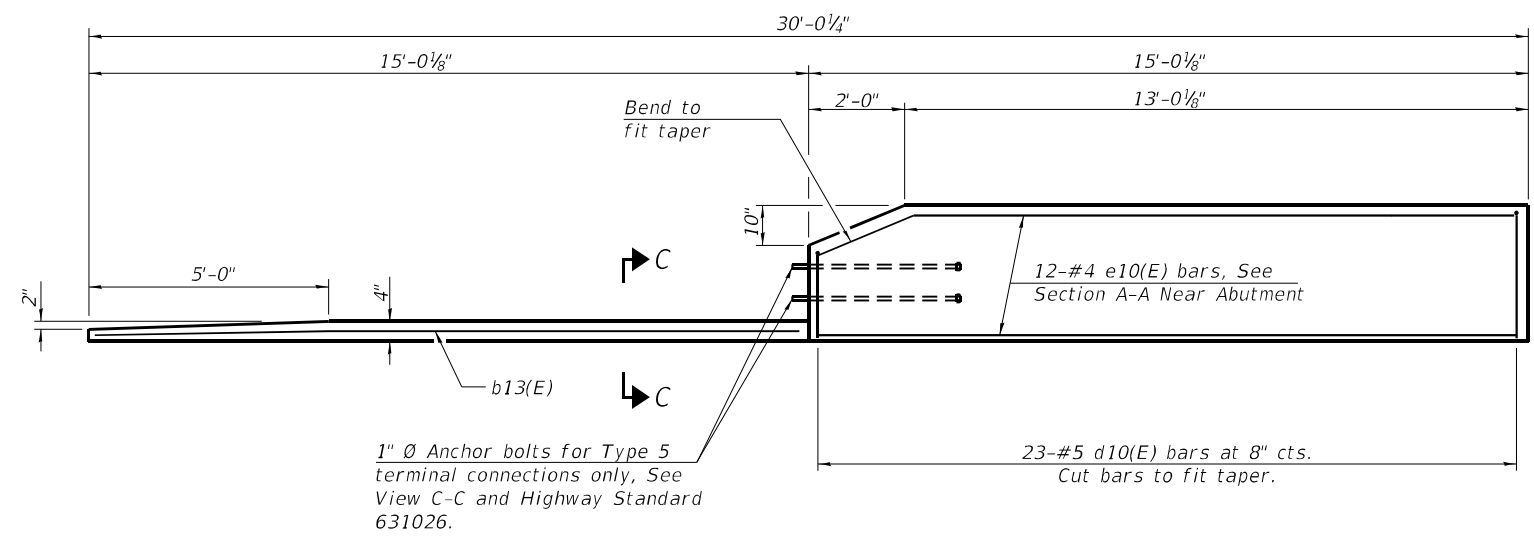
**NORTH APPROACH
 BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a10(E)	46	#5	14'-0"	—
a11(E)	46	#5	24'-4"	—
a12(E)	46	#5	7'-4"	—
a13(E)	60	#8	25'-1"	—
a14(E)	60	#8	13'-6"	—
b10(E)	52	#5	29'-8"	—
b11(E)	82	#9	29'-8"	—
b12(E)	8	#5	14'-8"	—
b13(E)	2	#4	14'-8"	—
d10(E)	46	#5	7'-0"	⌋
d11(E)	46	#5	8'-6"	⌋
e10(E)	24	#4	14'-8"	—
t10(E)	70	#4	9'-8"	—
w10(E)	40	#5	33'-11"	—
Concrete Superstructure				Cu. Yd. 4.3
Concrete Superstructure (Approach Slab)				Cu. Yd. 49.2
Concrete Structures				Cu. Yd. 10.6
Reinforcement Bars, Epoxy Coated				Pound 21240

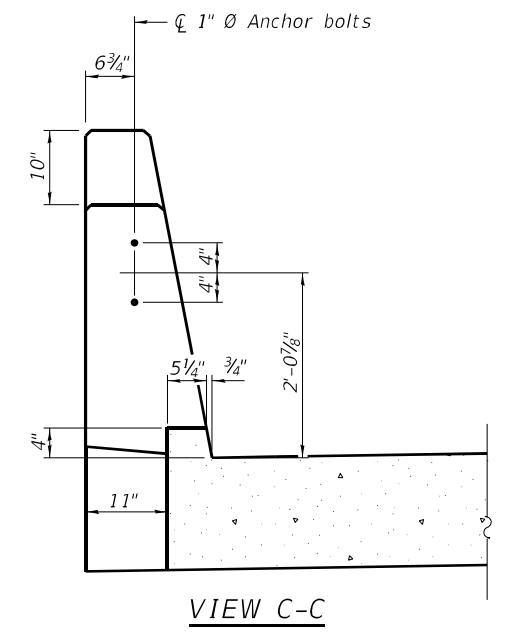
* Prior to Grinding
 ** Cost included with Concrete Superstructure (Approach Slab).
 *** Per manufacturer recommendations
 **** Perpendicular to inside face of curb.



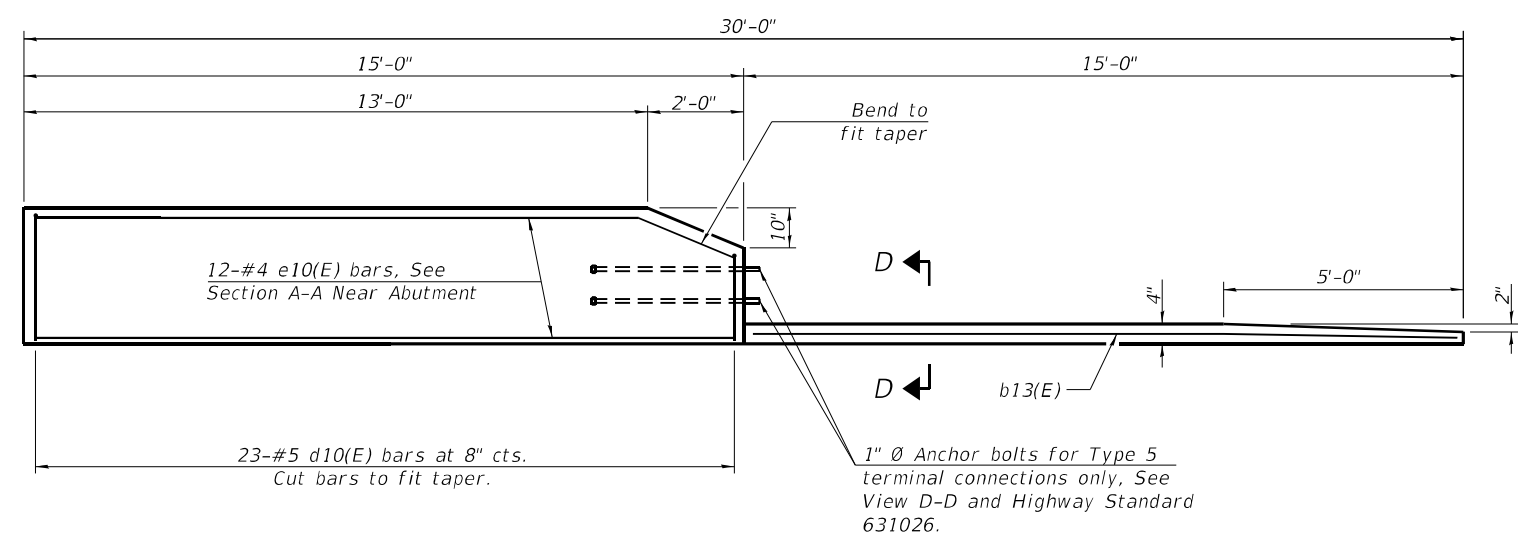
(Sheet 2 of 3)



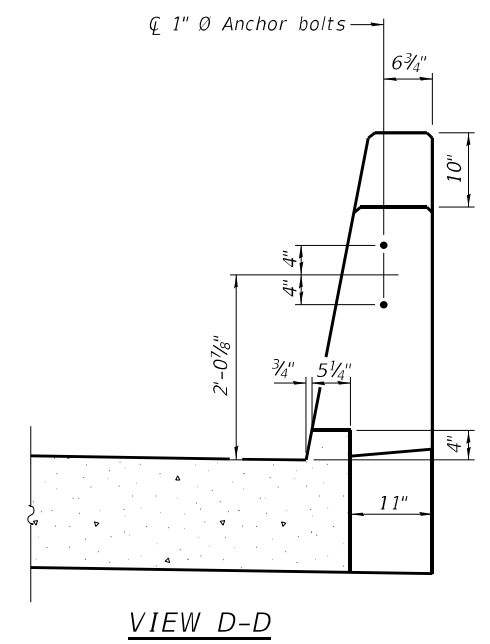
INSIDE ELEVATION OF EAST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)



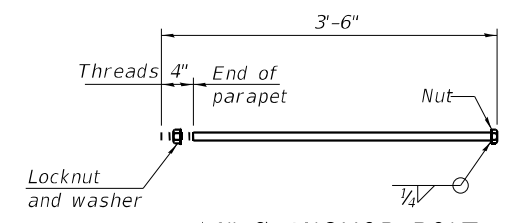
VIEW C-C



INSIDE ELEVATION OF WEST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)



VIEW D-D

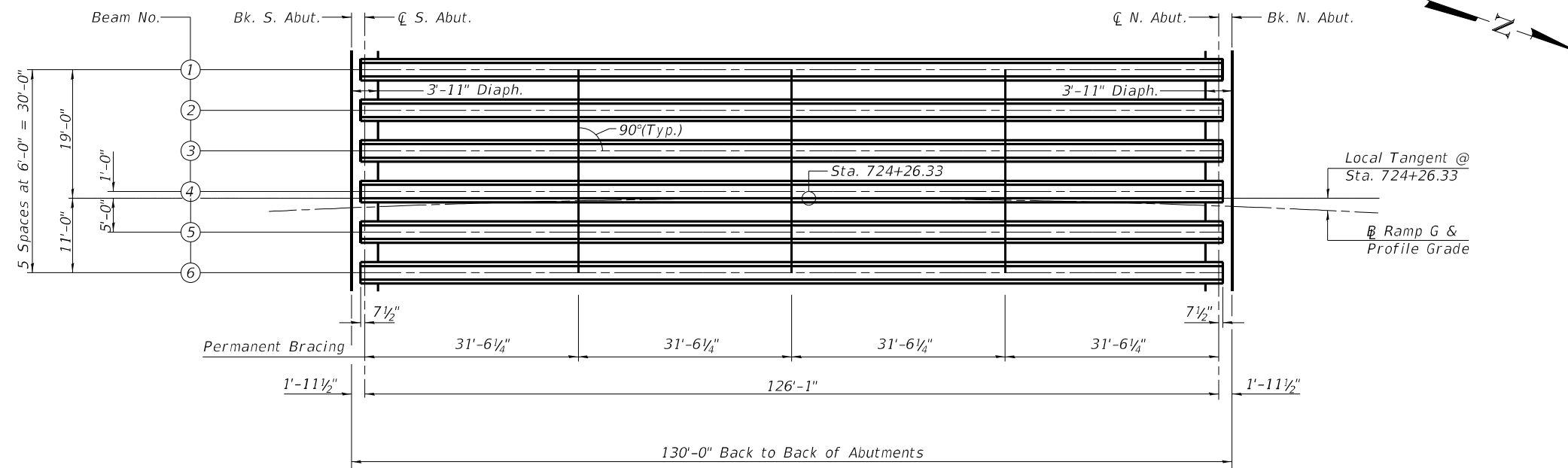


*** 1" Ø ANCHOR BOLT**
(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications)

* Cost included with Concrete Superstructure (Approach Slab).

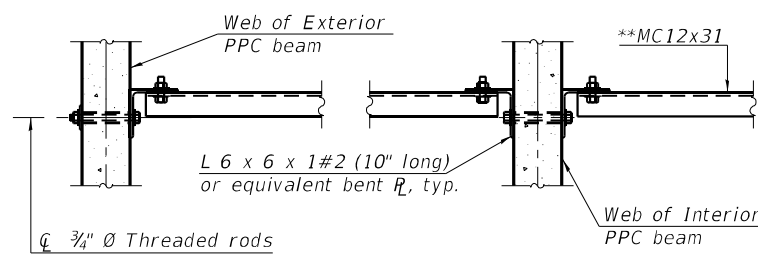
(Sheet 3 of 3)

FILE NAME = 0101002-70899-016-N_Appr Slab Details.dgn	USER NAME =	DESIGNED - GBR	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	NORTH BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 010-1002	F.A.I. R.T.E. = 57/74	SECTION = (10-34-1)HBK	COUNTY = CHAMPAIGN	TOTAL SHEETS = 1187	SHEET NO. = 764
BFW BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - FAM	REVISED -			SHEET NO. 16 OF 30 SHEETS	CONTRACT NO. 70B99	ILLINOIS FED. AID PROJECT		
433 NORTH COLT ST. SUITE 100, ELKHART, IN 46517-2100 PHONE: 812.837.8100	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -							
		CHECKED - GBR	REVISED -							

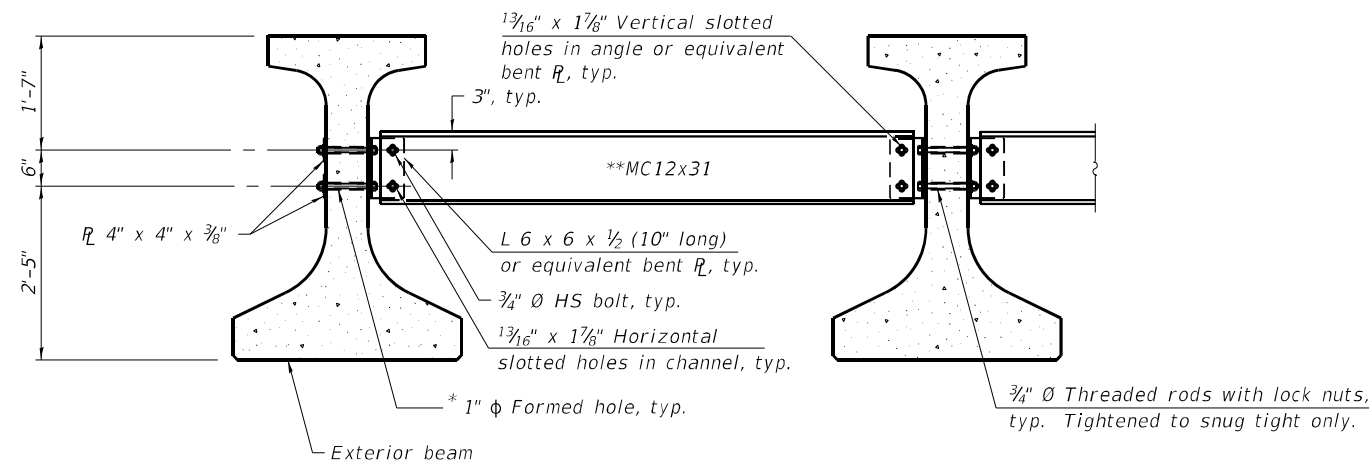


- I: Non-composite moment of inertia of beam section (in.⁴).
- I': Composite moment of inertia of beam section (in.⁴).
- Sb: Non-composite section modulus for the bottom fiber of the prestressed beam (in.³).
- Sb': Composite section modulus for the bottom fiber of the prestressed beam (in.³).
- St: Non-composite section modulus for the top fiber of the prestressed beam (in.³).
- St': Composite section modulus for the top fiber of the prestressed beam (in.³).
- DC1: Un-factored non-composite dead load (kips/ft.).
- MDC1: Un-factored moment due to non-composite dead load (kip-ft.).
- DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
- MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
- DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
- MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
- M_L + I_M: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- LLDF: Live Load Distribution Factor for Moment or Shear.
- OCF: Obtuse Correction Factor

FRAMING PLAN



PLAN



ELEVATION

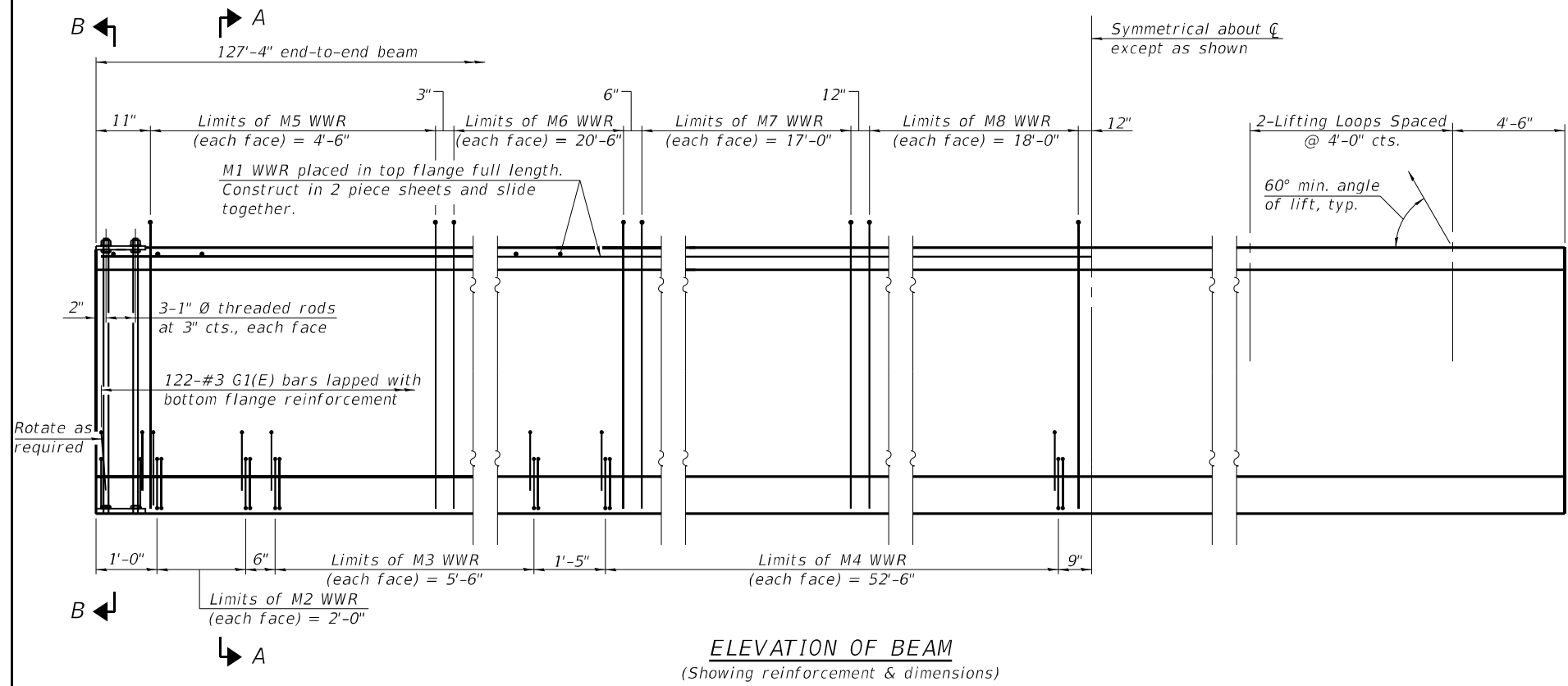
Notes:
 All material for bracing shall be hot dip galvanized according to AASHTO M 111 unless otherwise noted.
 Two hardened washers are required for each set of oversized holes.
 All holes shall be 1/16" Ø unless otherwise noted.
 5/16" x 3" x 3" plate washers are required over all slotted holes.
 All bolts, threaded rods, and hardware shall be galvanized according to AASHTO M 232.
 Threaded rods shall be ASTM F1554 Grade 55.
 Bracing shall be installed as beams are erected and tightened as soon as possible during erection.
 Permanent bracing shall not be paid for separately, but shall be included in the cost of Furnishing and Erecting Precast Prestressed Concrete Beams.

* Fabricator shall locate to miss strands within permissible tolerances.
 ** Alternate MC12x35 channels are permitted to facilitate material acquisition.

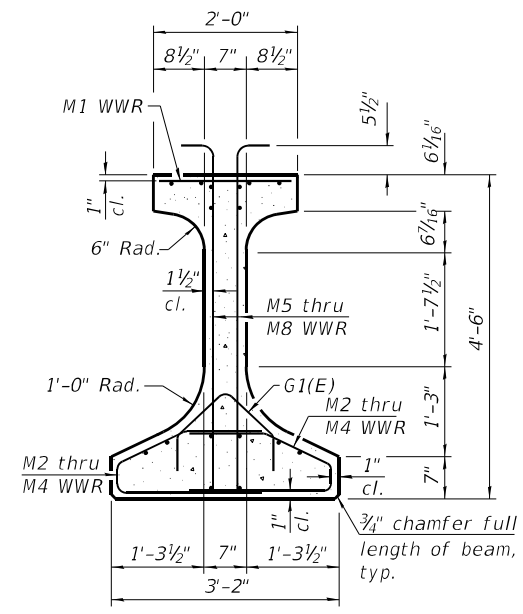
PERMANENT BRACING DETAILS
 (15 Required)

INTERIOR BEAM MOMENT TABLE		
0.5 Sp.		
I	(in ⁴)	295,427
I'	(in ⁴)	683,551
Sb	(in ³)	13,551.6
Sb'	(in ³)	19,940.2
St	(in ³)	9,174.7
St'	(in ³)	34,662.8
DC1	(k/ft)	1.559
MDC1	(k)	3.097
DC2	(k/ft)	0.190
MDC2	(k)	377.6
DW	(k/ft)	0.300
MDW	(k)	596.1
LLDF		0.524
M _L + I _M	(k)	2,055

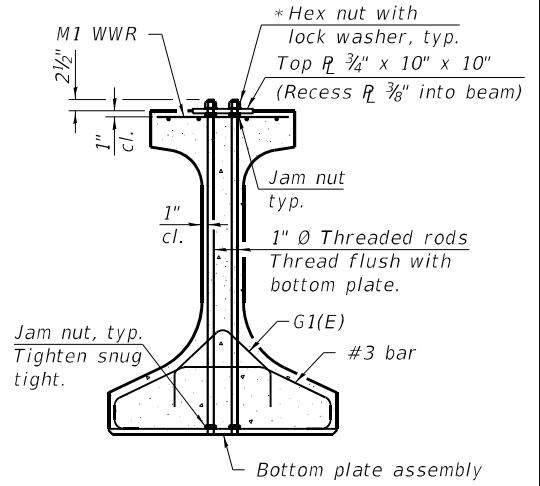
INTERIOR BEAM REACTION TABLE	
Abut.	
LLDF	0.671
OCF	1.000
RDC1	(k) 98.3
RDC2	(k) 12.0
RDW	(k) 18.9
R _L	(k) 71.8
R _{IM}	(k) 14.8
R _{Total}	(k) 215.8



ELEVATION OF BEAM
(Showing reinforcement & dimensions)

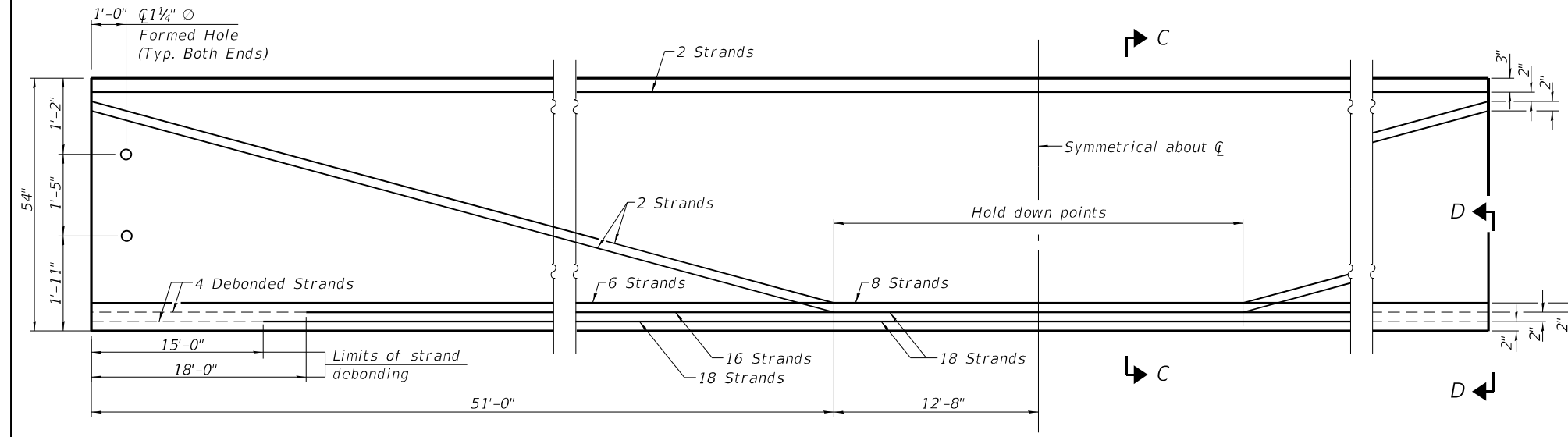


SECTION A-A

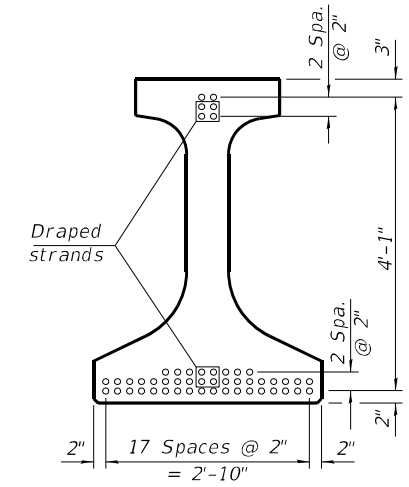


SECTION B-B

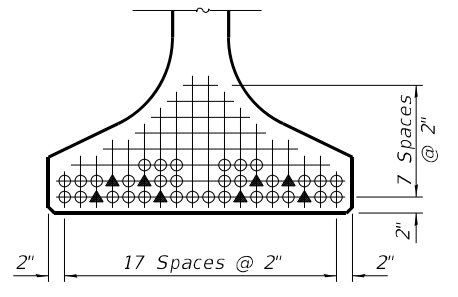
* Only tighten sufficiently to compress lock washers



ELEVATION OF BEAM
(Showing prestressing steel)



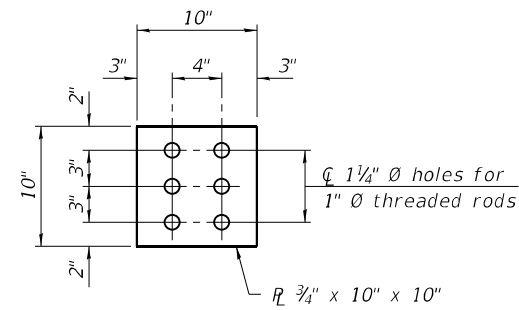
SECTION C-C
(46-0.6" Ø 270 ksi strands)



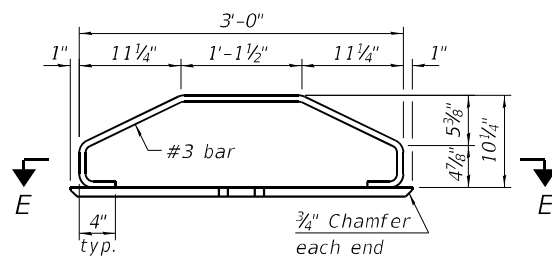
VIEW D-D

- Fully bonded strand
- ▲ Partially debonded strand

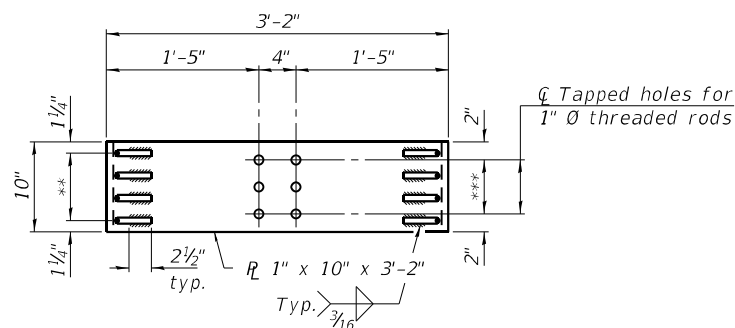
Notes:
See sheet 17 of 30 for number and location of 1" formed holes for Permanent Bracing.
See sheet 19 of 30 for additional details and Bill of Material.



PLAN - TOP PLATE



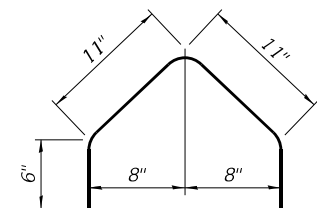
ELEVATION - BOTTOM PLATE ASSEMBLY



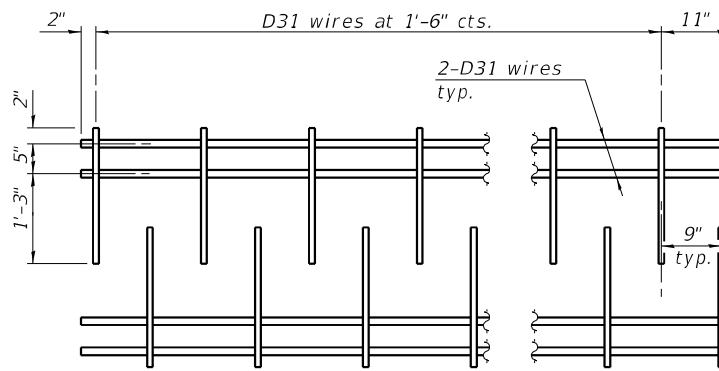
SECTION E-E

** 3 Spaces at 2 1/2" = 7 1/2"

*** 2 Spaces at 3" = 6"

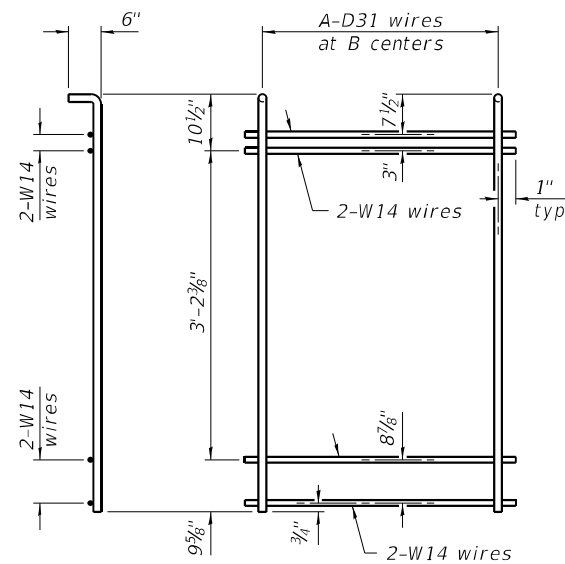


BAR G1(E)



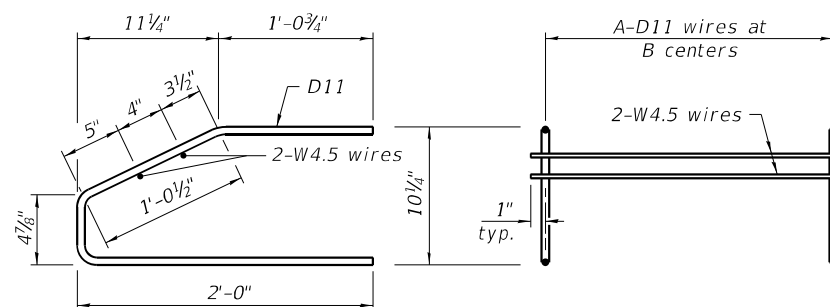
M1 WWR DETAIL

When multiple sheets of M1 WWR are required along the beam length, #5(E) bars (5'-0" long) shall be used to splice the longitudinal D31 wires together (Min. Lap 2'-2").



M5 THRU M8 WWR DETAIL

(See Table of Dimensions)



M2 THRU M4 WWR DETAIL

(See Table of Dimensions)

NOTES

Inserts for 3/4" Ø threaded dowel rods, when specified, are to be two strut, ferrule type for interior beams and single ferrule, flared loop type for exterior beams. Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter for beam strands shall be 0.6" and the nominal cross-sectional area shall be 0.217 sq. in. The nominal diameter for lifting loops shall be 1/2" and the nominal cross sectional area shall be 0.153 sq. in.

The beams shall have a final concrete compressive strength, f'c, of 8500 psi and a release concrete compressive strength, f'ci, of 6500 psi.

A minimum 2 1/2" Ø lifting pin shall be used to engage the lifting loops during handling.

The top and bottom plates shall be AASHTO M 270 Grade 50.

The top plates and bottom plate assemblies shall be galvanized according to AASHTO M 111.

The threaded rods, nuts and washers shall be galvanized according to AASHTO M 232.

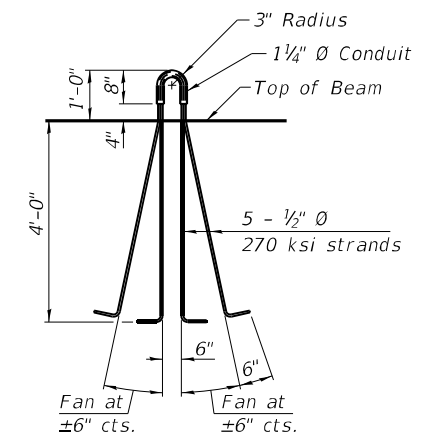
Threaded rods shall be ASTM F1554 Grade 55.

Welded Wire Reinforcement (WWR) shall conform to ASTM A884 with a Class A, Type 1 epoxy coating or ASTM A1060, Table 3 galvanized coating.

TABLE OF DIMENSIONS

(WWR tables are based on Grade 60.)

WWR	A	B
M2	9	3"
M3	12	6"
M4	36	1'-6"
M5	19	3"
M6	42	6"
M7	18	1'-0"
M8	10	2'-0"



LIFTING LOOP DETAIL

BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete Beams, IL54N	Ft.	764

FILE NAME = 0101002-70899-019-IL54 Beam Details2.dgn

USER NAME =

DESIGNED - GBR

REVISED -



BACON | FARMER | WORKMAN
ENGINEERING & TESTING, INC.
433 NORTH COLT ST. SUITE 100
MORRIS, ILLINOIS 62450
PHONE: 618.987.8100

PLOT SCALE =

DRAWN - BJV

REVISED -

PLOT DATE = 3/11/2021

CHECKED - GBR

REVISED -

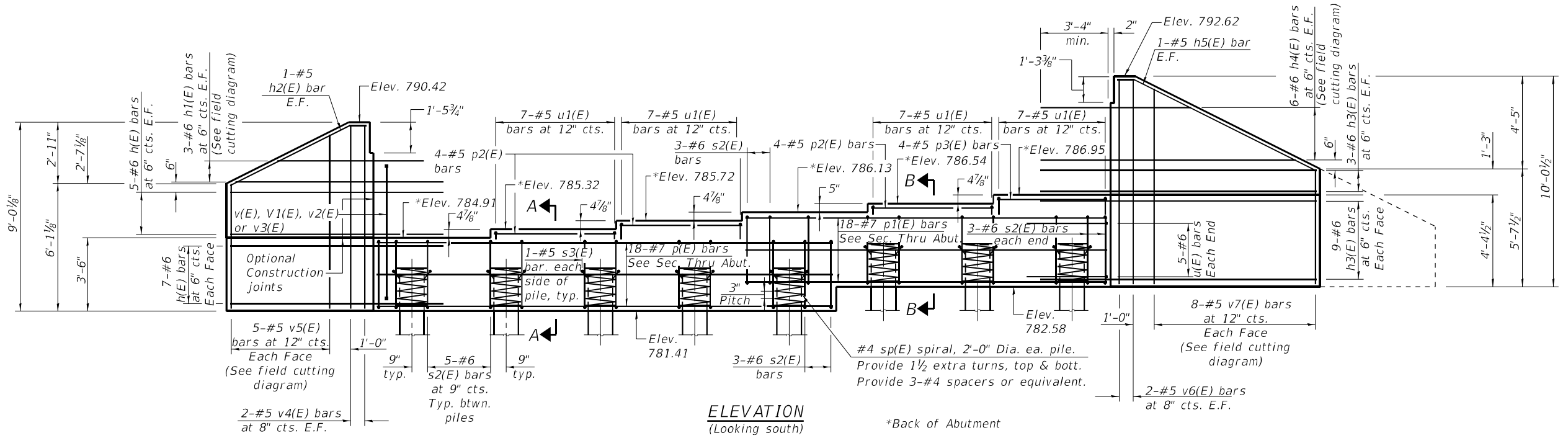
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

IL54N BEAM DETAILS
STRUCTURE NO. 010-1002

SHEET NO. 19 OF 30 SHEETS

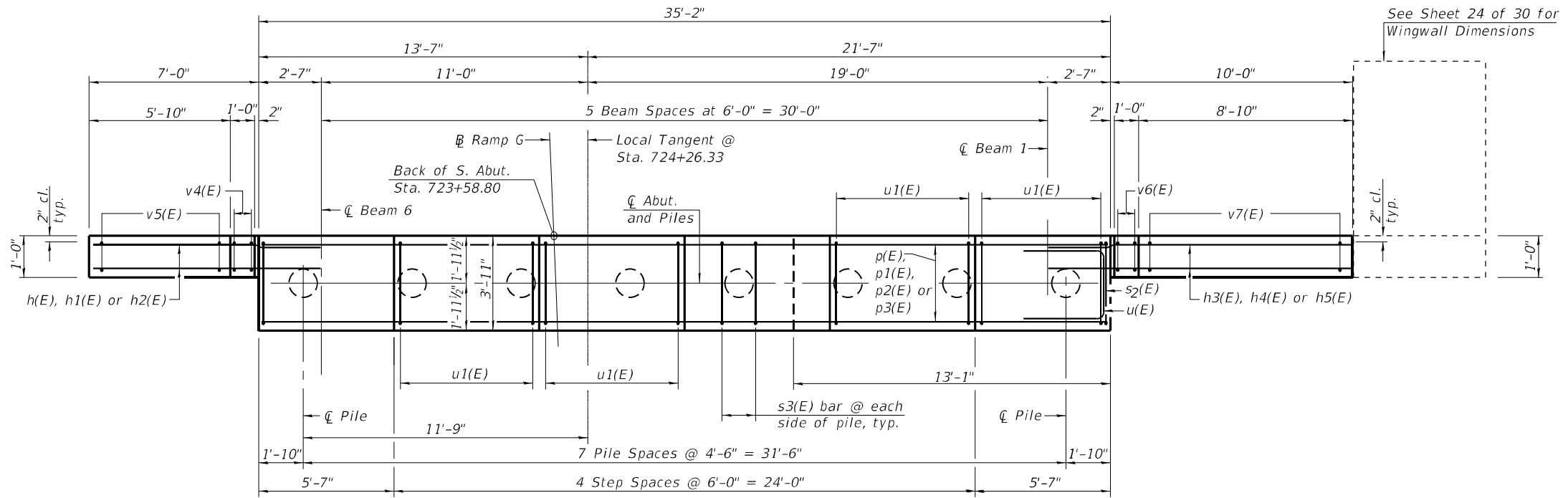
F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
57/74	(10-34-1)HKB	CHAMPAIGN	1187	767
CONTRACT NO. 70B99				

ILLINOIS FED. AID PROJECT



ELEVATION
(Looking south)

*Back of Abutment

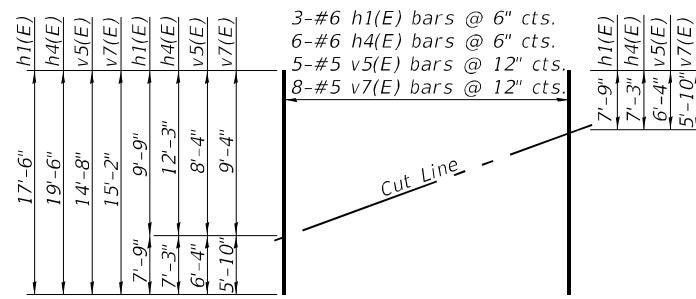


PLAN

PILE DATA

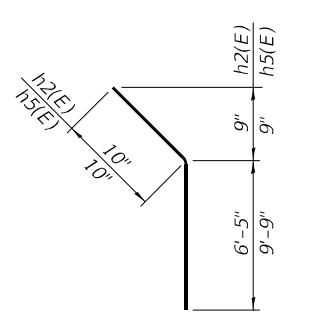
Type: MS 14" x 0.312" With Pile Shoes
 Nominal Required Bearing: 513 kips
 Factored Resistance Available: 282 kips
 Est. Length: 63'
 No. Production Piles: 7
 No. Test Piles: 1
 No. Pile Shoes: 8

Notes:
 Pour steps monolithically with cap.
 E.F. denotes Each Face.
 For Section A-A and B-B, see Sheet 21 of 30.
 For Bill of Materials, see Sheet 21 of 30.
 Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706 Cost included with Reinforcement Bars, Epoxy Coated.
 For Layout of v(E), v1(E), v2(E), & v3(E) bars see Sheet 21 of 30.

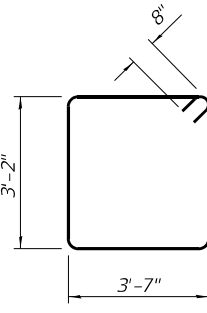


FIELD CUTTING DIAGRAM

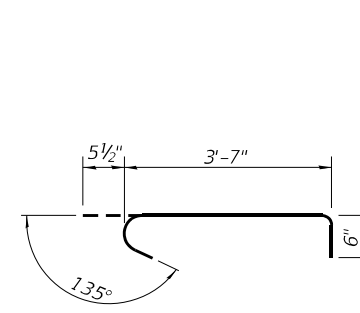
Order h1(E), h4(E), v5(E) and v7(E) Full length.
 Cut as shown and use remainder of bars in opposite face.



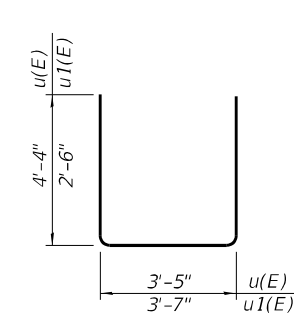
BAR h2(E) & h5(E)



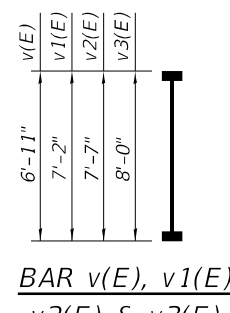
BAR s2(E)



BAR s3(E)



BAR u(E) & u1(E)



**BAR v(E), v1(E)
v2(E) & v3(E)**
(Headed)

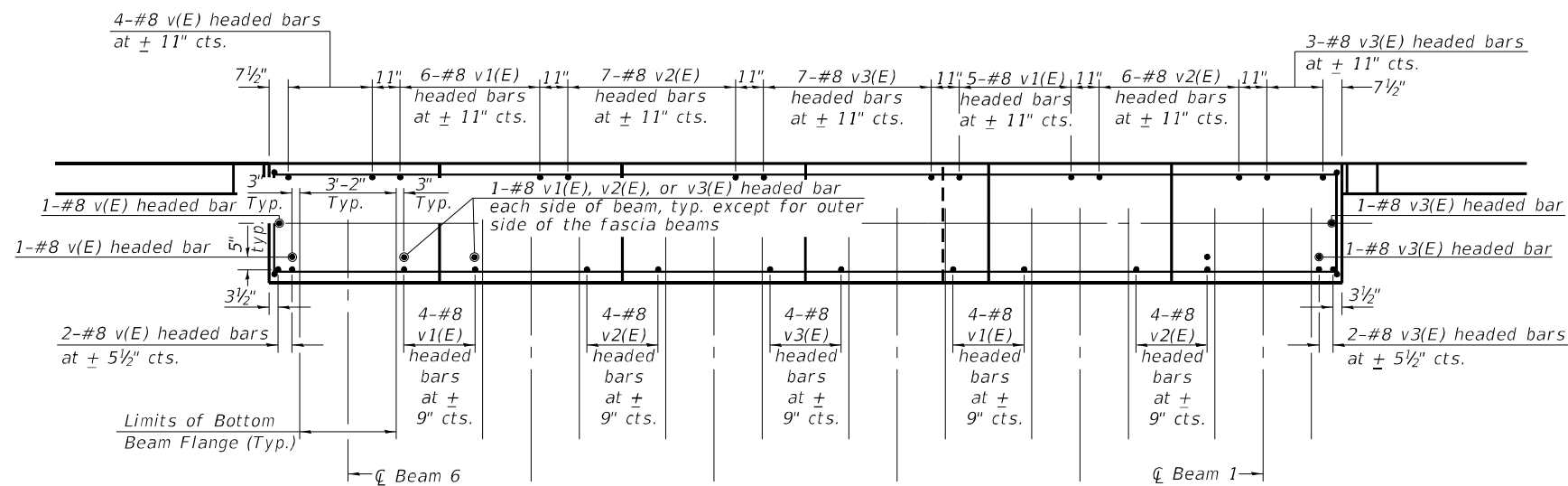
FILE NAME = 0101002-70899-020- S. Abutment.dgn	USER NAME =	DESIGNED - GBR	REVISED -
BACON FARMER WORKMAN ENGINEERING & TESTING, INC.		CHECKED - FAM	REVISED -
433 NORTH COLLET STREET MORRIS, ILLINOIS 62450 PHONE: 618.987.8100	PLOT SCALE =	DRAWN - BJV	REVISED -
	PLOT DATE = 3/11/2021	CHECKED - GBR	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

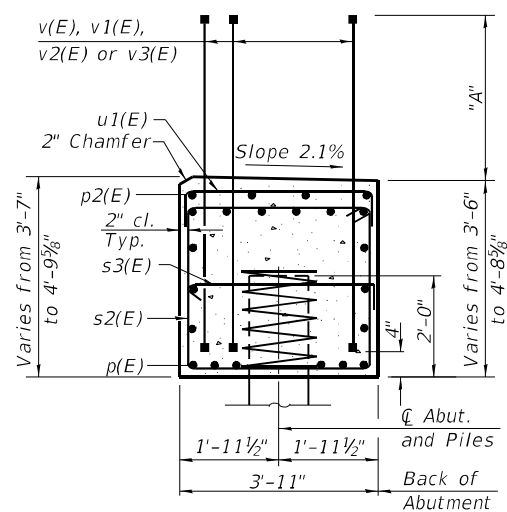
**SOUTH ABUTMENT
STRUCTURE NO. 010-1002**

SHEET NO. 20 OF 30 SHEETS

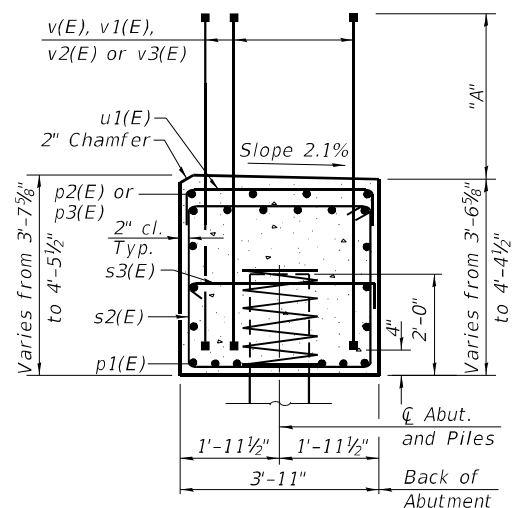
F.A.I. R.T.E. 57/74	SECTION (10-34-1)HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 768
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



Layout of v(E), v1(E), v2(E) & v3(E) Bars



SEC. A-A



SEC. B-B

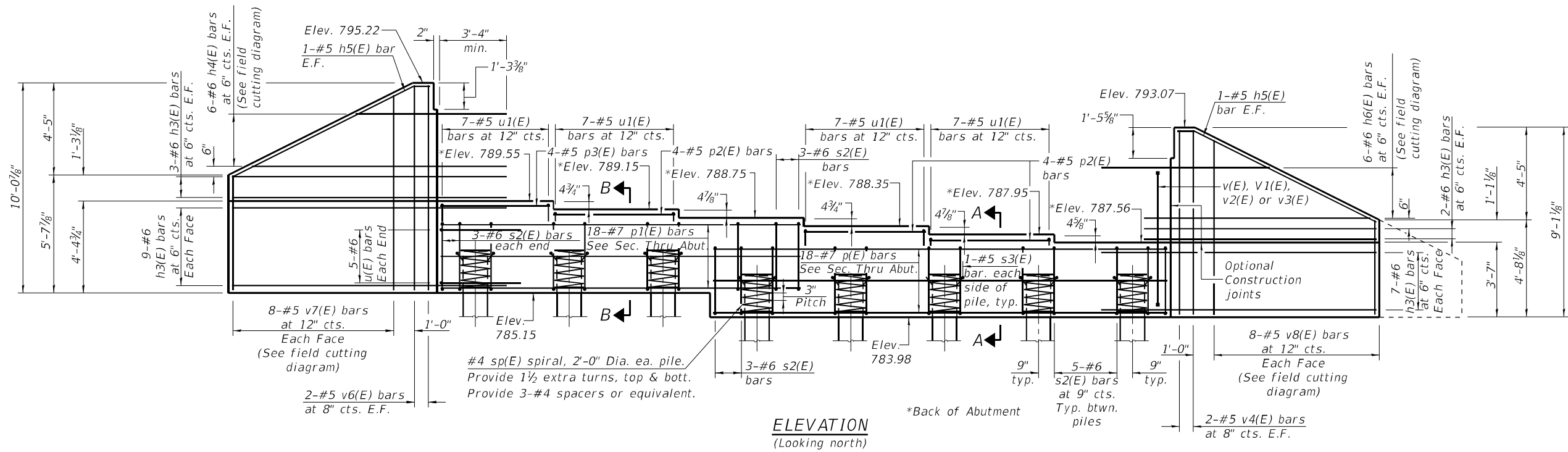
**SOUTH ABUTMENT
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h(E)	24	#6	10'-2"	—
h1(E)	3	#6	17'-6"	—
h2(E)	2	#5	7'-3"	—
h3(E)	24	#6	13'-2"	—
h4(E)	6	#6	19'-6"	—
h5(E)	2	#5	10'-7"	—
p(E)	18	#7	21'-10"	—
p1(E)	18	#7	17'-4"	—
p2(E)	12	#5	5'-10"	—
p3(E)	4	#5	5'-4"	—
s2(E)	47	#6	14'-10"	□
s3(E)	16	#5	4'-7"	◁
* sp(E)	8	#4	2'-0"	≡≡≡
u(E)	10	#6	12'-1"	—
u1(E)	28	#5	8'-7"	—
v(E)	8	#8	6'-11"	—
v1(E)	23	#8	7'-2"	—
v2(E)	25	#8	7'-7"	—
v3(E)	20	#8	8'-0"	—
v4(E)	4	#5	8'-10"	—
v5(E)	5	#5	14'-8"	—
v6(E)	4	#5	9'-10"	—
v7(E)	8	#5	15'-2"	—
Structure Excavation		Cu. Yd.	97	
Concrete Structures		Cu. Yd.	26.1	
Reinforcement Bars, Epoxy Coated		Pound	6400	
Furnishing Metal Shell Piles, 14" x 0.312"		Foot	441	
Driving Piles		Foot	441	
Test Pile, Metal Shells		Each	1	
Pile Shoes		Each	8	

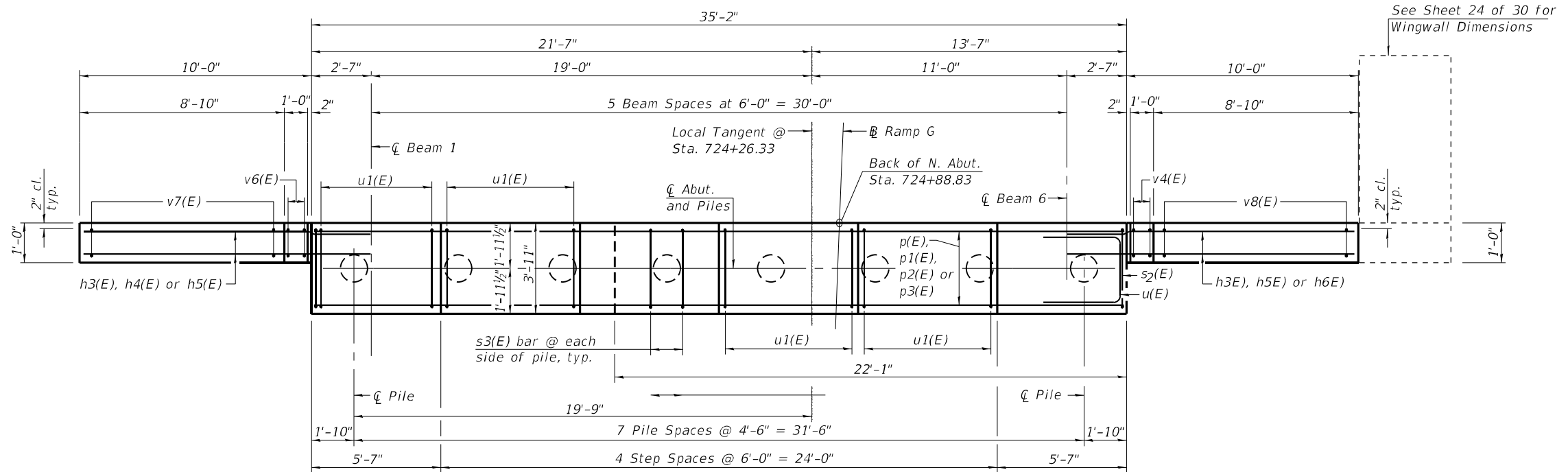
* Length is height of spiral.

**DIMENSIONS OF V-BARS
ABOVE TOP OF ABUTMENT CAP ("A")**

Bar	Minimum Length of "A"	Maximum Length of "A"
v(E)	3'-9"	3'-9"
v1(E)	3'-6 3/8"	4'-0"
v2(E)	3'-6 1/2"	4'-0 1/8"
v3(E)	3'-7 3/8"	4'-0 1/4"



ELEVATION
(Looking north)

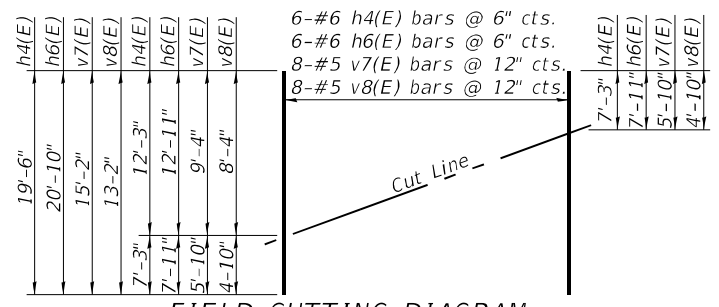


PLAN

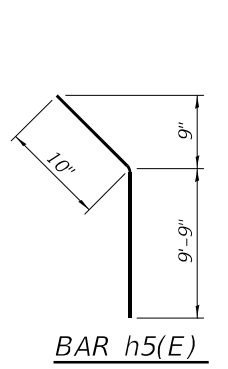
PILE DATA

Type: MS 14" x 0.312" With Pile Shoes
 Nominal Required Bearing: 513 kips
 Factored Resistance Available: 282 kips
 Est. Length: 55'
 No. Production Piles: 7
 No. Test Piles: 1
 No. Pile Shoes: 8

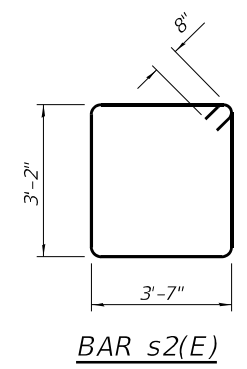
Notes:
 Pour steps monolithically with cap.
 E.F. denotes Each Face.
 For Section A-A and B-B, see Sheet 23 of 30.
 For Bill of Materials, see Sheet 23 of 30.
 For details of piles, see Sheet 25 of 30.
 Headed bars shall conform to ASTM A970 with threaded attachment: Class HA; and reinforcement bars conforming to ASTM A706 Cost included with Reinforcement Bars, Epoxy Coated.
 For Layout of v(E), v1(E), v2(E), & v3(E) bars see sheet 23 of 30.



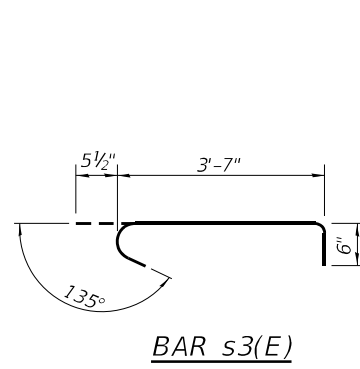
FIELD CUTTING DIAGRAM
 Order h4(E), h6(E), v7(E) and v8(E) Full length. Cut as shown and use remainder of bars in opposite face.



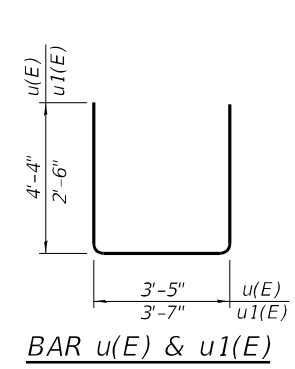
BAR h5(E)



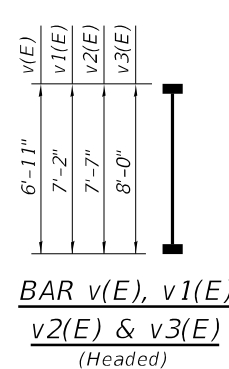
BAR s2(E)



BAR s3(E)



BAR u(E) & u1(E)



BAR v(E), v1(E) v2(E) & v3(E)
(Headed)

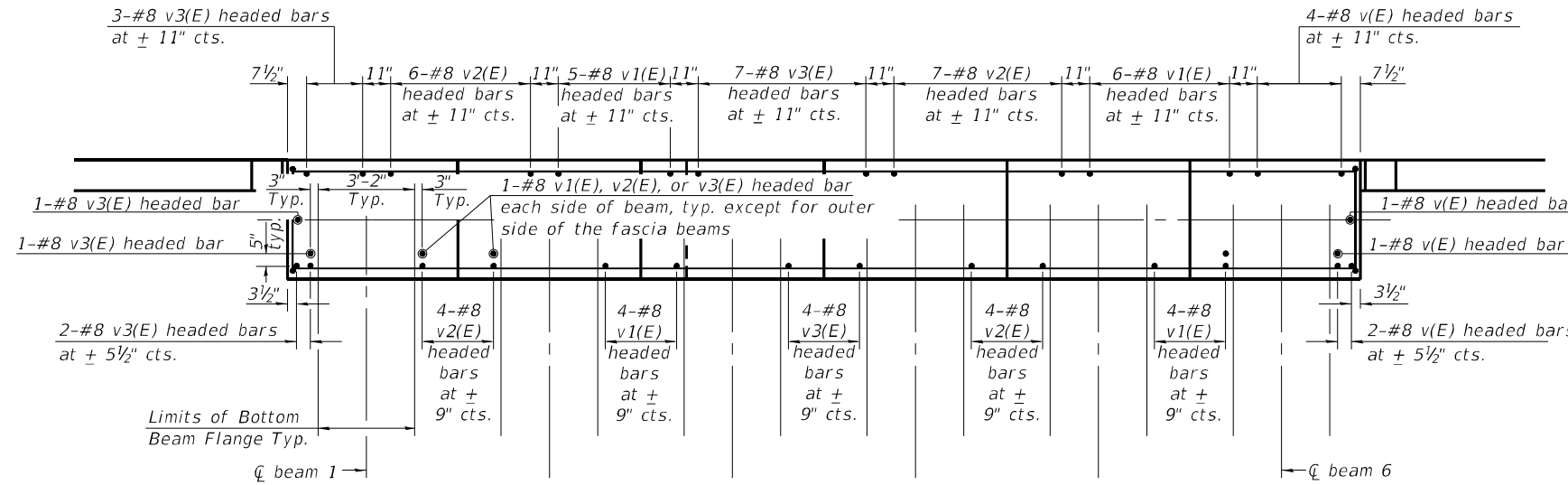
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BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - FAM	REVISED -
433 NORTH COLT ST. SUITE 100 MORRIS, IL 62458-0100 PHONE: 618.987.8100	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -
		CHECKED - GBR	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

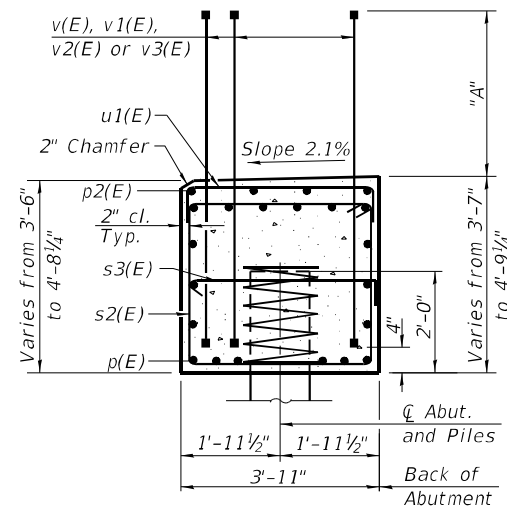
NORTH ABUTMENT
STRUCTURE NO. 010-1002

SHEET NO. 22 OF 30 SHEETS

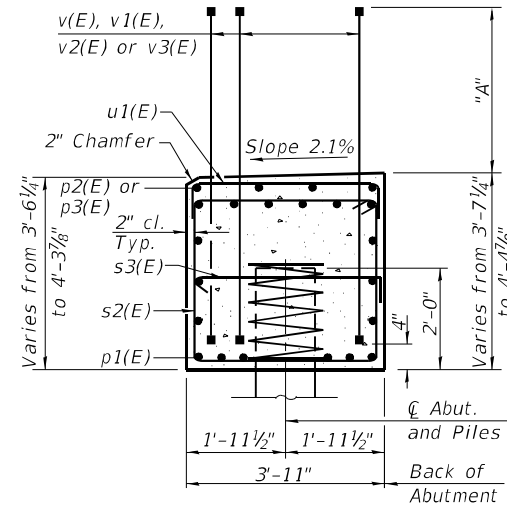
F.A.I. RE. 57/74	SECTION (10-34-1)HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 770
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



Layout of v(E), v1(E), v2(E) & v3(E) Bars



SEC. A-A



SEC. B-B

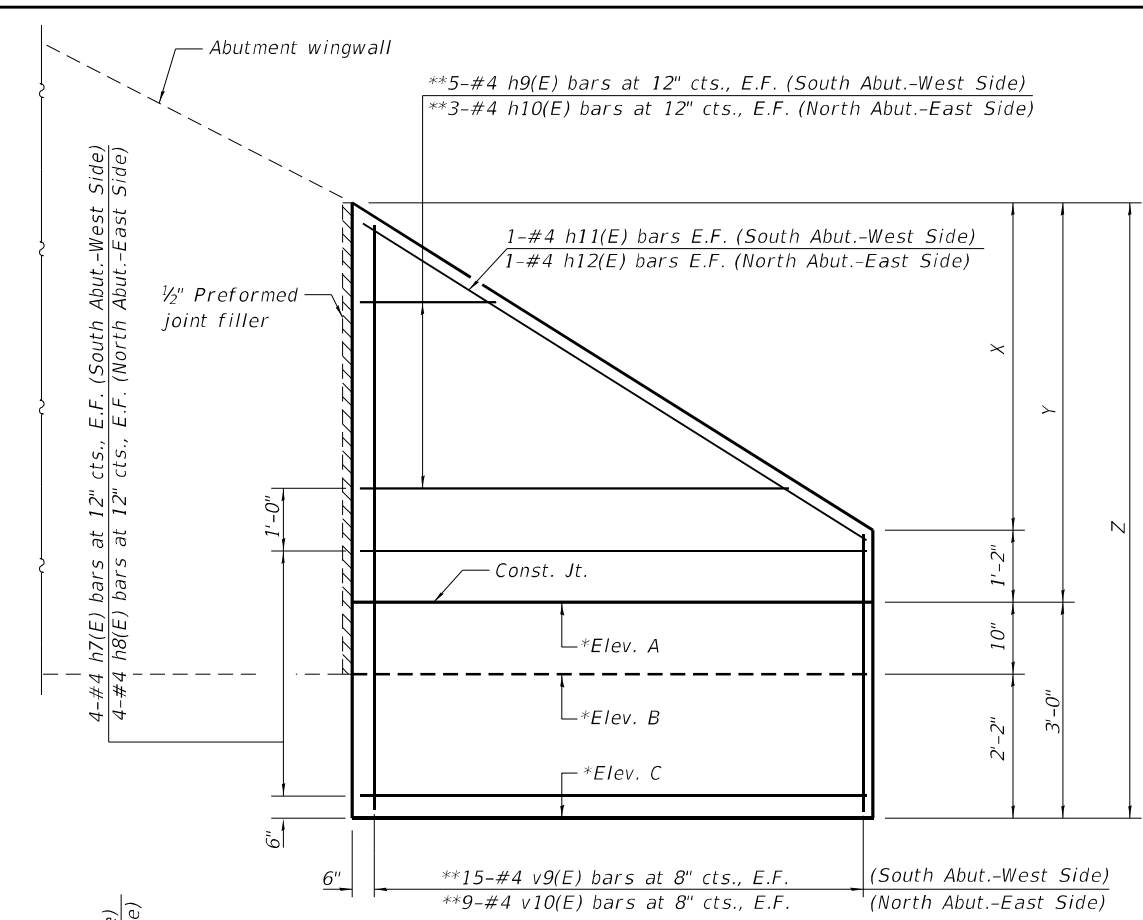
**NORTH ABUTMENT
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h3(E)	42	#6	13'-2"	—
h4(E)	6	#6	19'-6"	—
h5(E)	4	#5	10'-7"	—
h6(E)	6	#6	20'-10"	—
p(E)	18	#7	21'-10"	—
p1(E)	18	#7	17'-4"	—
p2(E)	12	#5	5'-10"	—
p3(E)	4	#5	5'-4"	—
s2(E)	47	#6	14'-10"	—
s3(E)	16	#5	4'-7"	—
* sp(E)	8	#4	2'-0"	WWM
u(E)	10	#6	12'-1"	—
u1(E)	28	#5	8'-7"	—
v(E)	8	#8	6'-11"	—
v1(E)	23	#8	7'-2"	—
v2(E)	25	#8	7'-7"	—
v3(E)	20	#8	8'-0"	—
v4(E)	4	#5	8'-10"	—
v6(E)	4	#5	9'-10"	—
v7(E)	8	#5	15'-2"	—
v8(E)	8	#5	13'-2"	—
Structure Excavation				Cu. Yd. 111
Concrete Structures				Cu. Yd. 26.6
Reinforcement Bars, Epoxy Coated				Pound 6540
Furnishing Metal Shell Piles, 14" x 0.312"				Foot 385
Driving Piles				Foot 385
Test Pile, Metal Shells				Each 1
Pile Shoes				Each 8

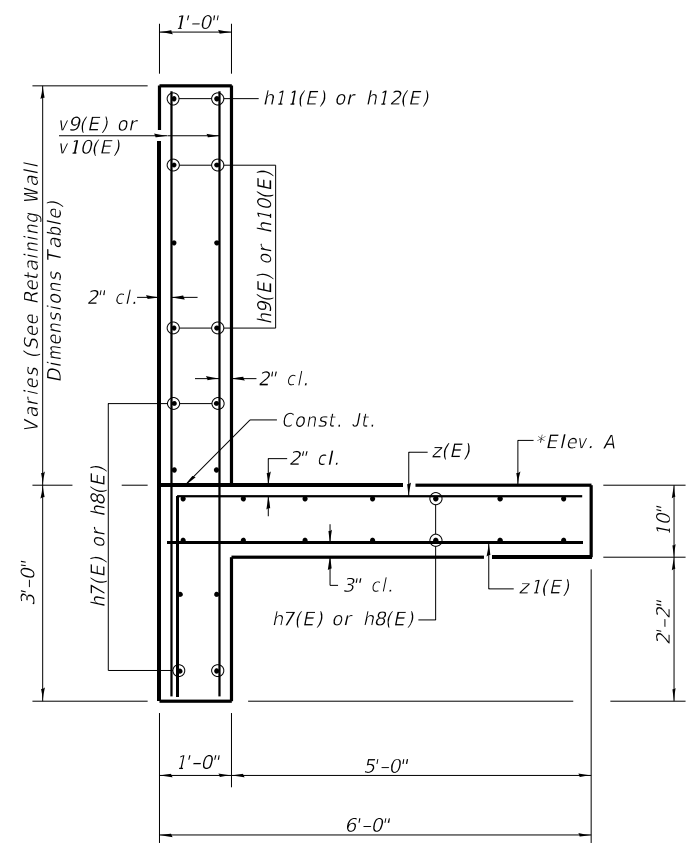
* Length is height of spiral.

**DIMENSIONS OF V-BARS
ABOVE TOP OF ABUTMENT CAP ("A")**

Bar	Minimum Length of "A"	Maximum Length of "A"
v(E)	3'-8"	3'-8"
v1(E)	3'-5 7/8"	3'-11"
v2(E)	3'-6 1/8"	3'-11 3/8"
v3(E)	3'-6 3/4"	3'-11 1/2"

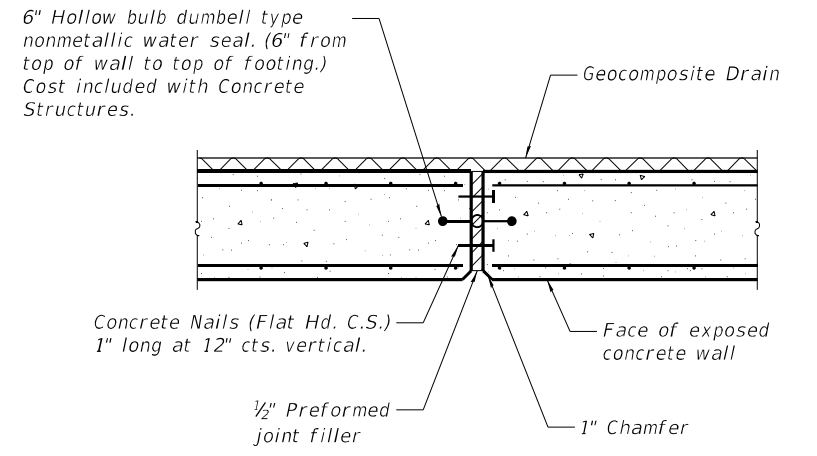


ELEVATION



SECTION A-A

Maximum Applied Service Bearing Pressure, $Q_{max} = 1453$ psf



EXPANSION JOINT DETAIL

RETAINING WALL DIMENSIONS

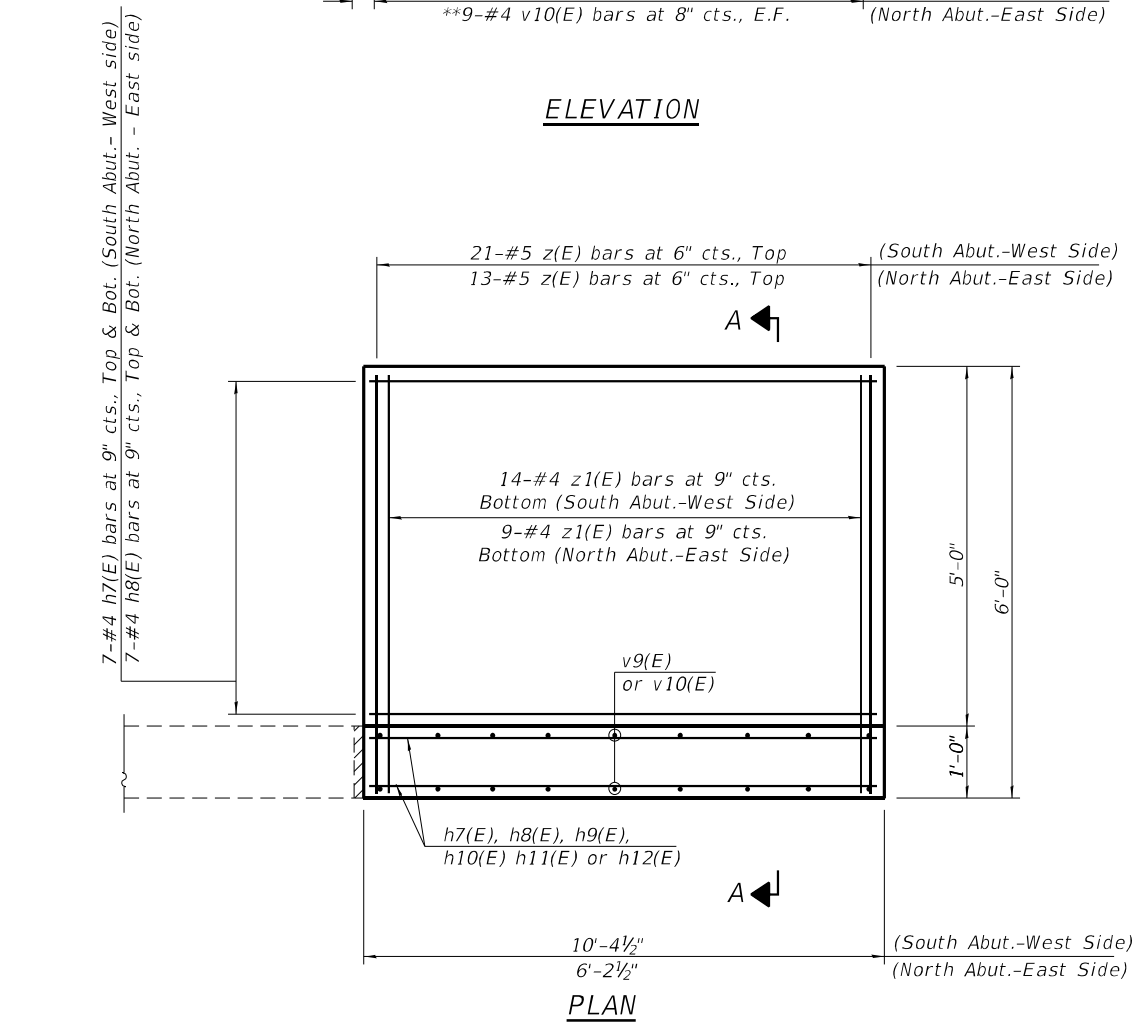
Dimension	Retaining Wall Dimensions	
	South Abutment	North Abutment
X	5'-2 1/4"	3'-1 1/4"
Y	6'-4 1/4"	4'-3 1/4"
Z	9'-4 1/4"	7'-3 1/4"

FOOTING ELEVATIONS

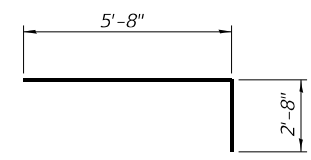
Elevation	Footing Elevations	
	South Abutment	North Abutment
A	781.83	784.36
B	781.00	783.53
C	778.83	781.36

**BILL OF MATERIAL
2 WINGWALL EXTENSIONS**

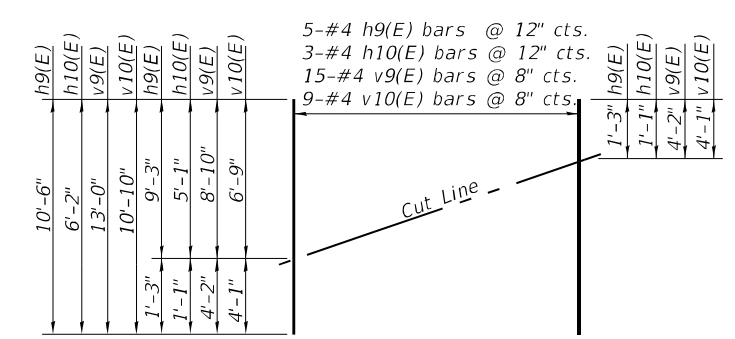
Bar	No.	Size	Length	Shape
h7(E)	22	#4	10'-1"	—
h8(E)	22	#4	5'-11"	—
h9(E)	5	#4	10'-6"	—
h10(E)	3	#4	6'-2"	—
h11(E)	2	#4	11'-4"	—
h12(E)	2	#4	6'-8"	—
v9(E)	15	#4	13'-0"	—
v10(E)	9	#4	10'-10"	—
z(E)	34	#5	8'-4"	┌
z1(E)	23	#4	5'-8"	—
Concrete Structures				Cu. Yd. 6.5
Reinforcement Bars, Epoxy Coated				Pound 880



PLAN

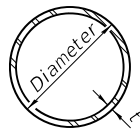


BAR z(E)



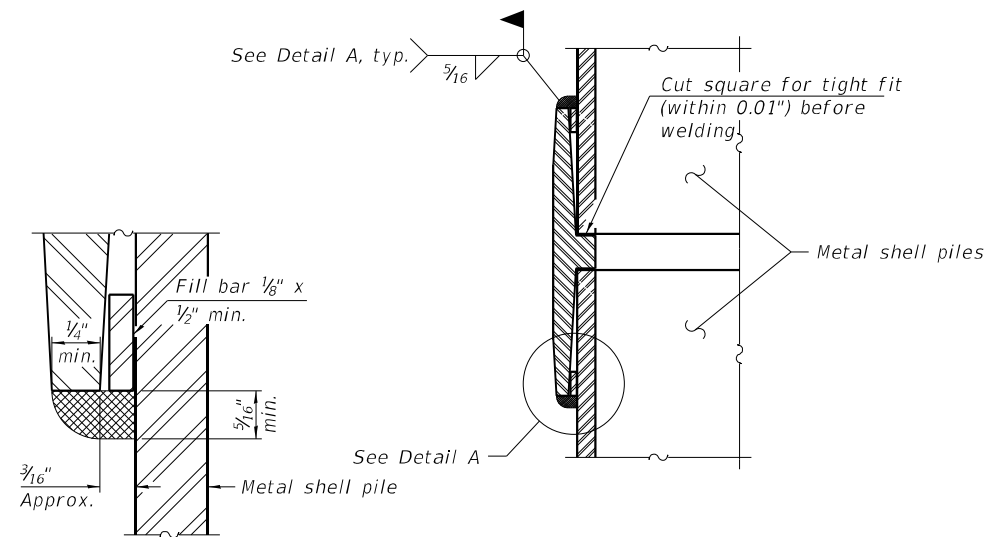
FIELD CUTTING DIAGRAM

Order h9(E), h10(E), v9(E) and v10(E) full length. Cut as shown and use remainder of bars in opposite face.

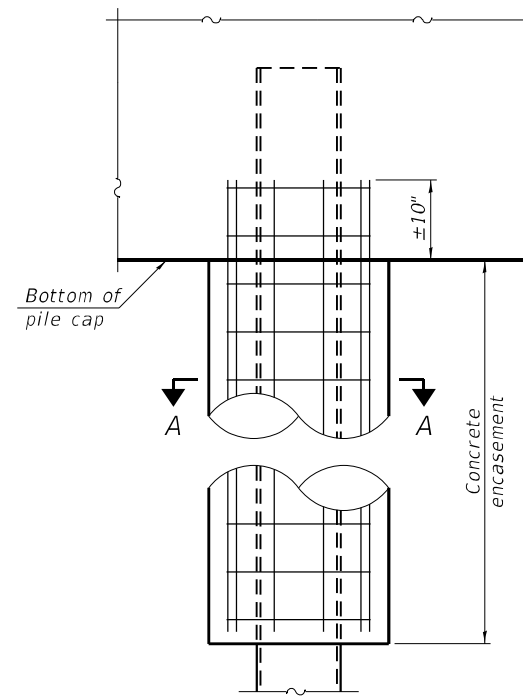


METAL SHELL PILE TABLE

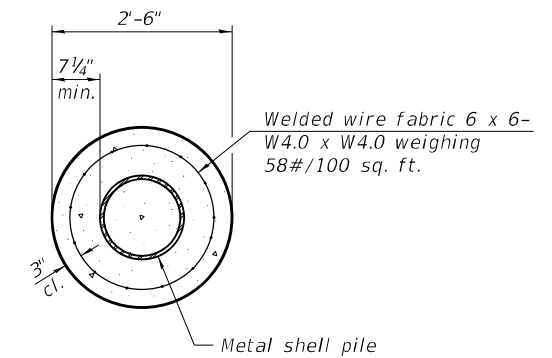
Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. ³ /ft.)
PP12	0.250"	31.37	0.0267
PP14	0.250"	36.71	0.0368
PP14	0.312"	45.61	0.0361
PP16	0.312"	52.32	0.0478
PP16	0.375"	62.64	0.0470



DETAIL A

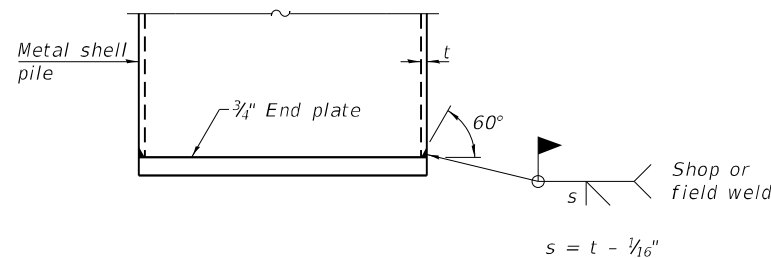


ELEVATION



SECTION A-A

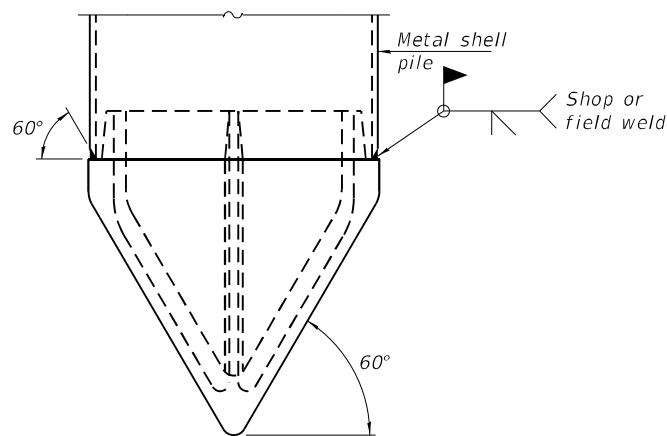
INDIVIDUAL PILE CONCRETE ENCASEMENT AT PIERS



END PLATE ATTACHMENT

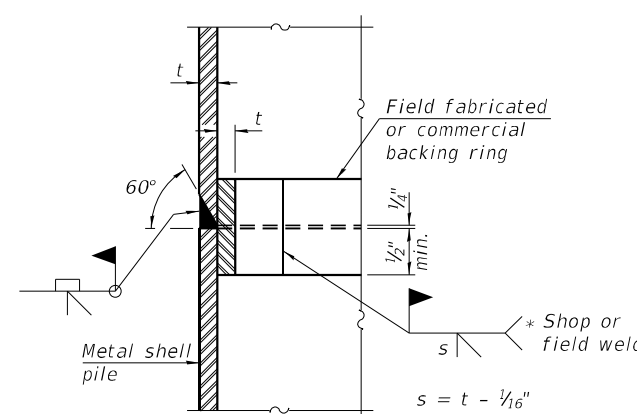
WELDED COMMERCIAL SPLICE

Notes:
 The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them.
 Pile segments shall be driven to solid contact with splicer before welding.



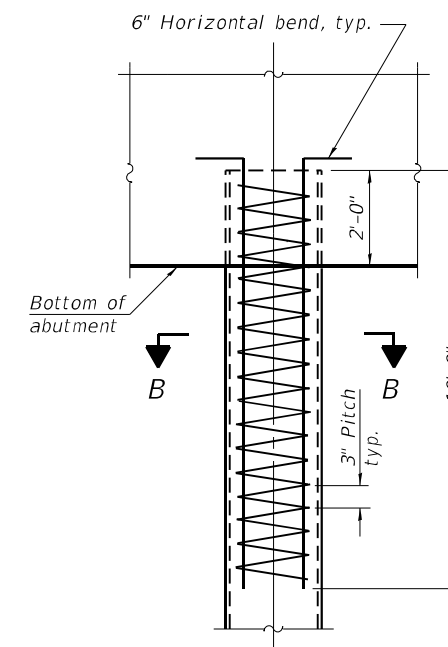
PILE SHOE ATTACHMENT

(When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A148 Grade 90-60 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld).

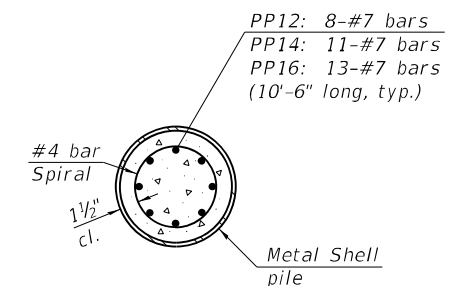


COMPLETE PENETRATION WELD SPLICE

* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.



ELEVATION



SECTION B-B

REINFORCEMENT AT ABUTMENTS

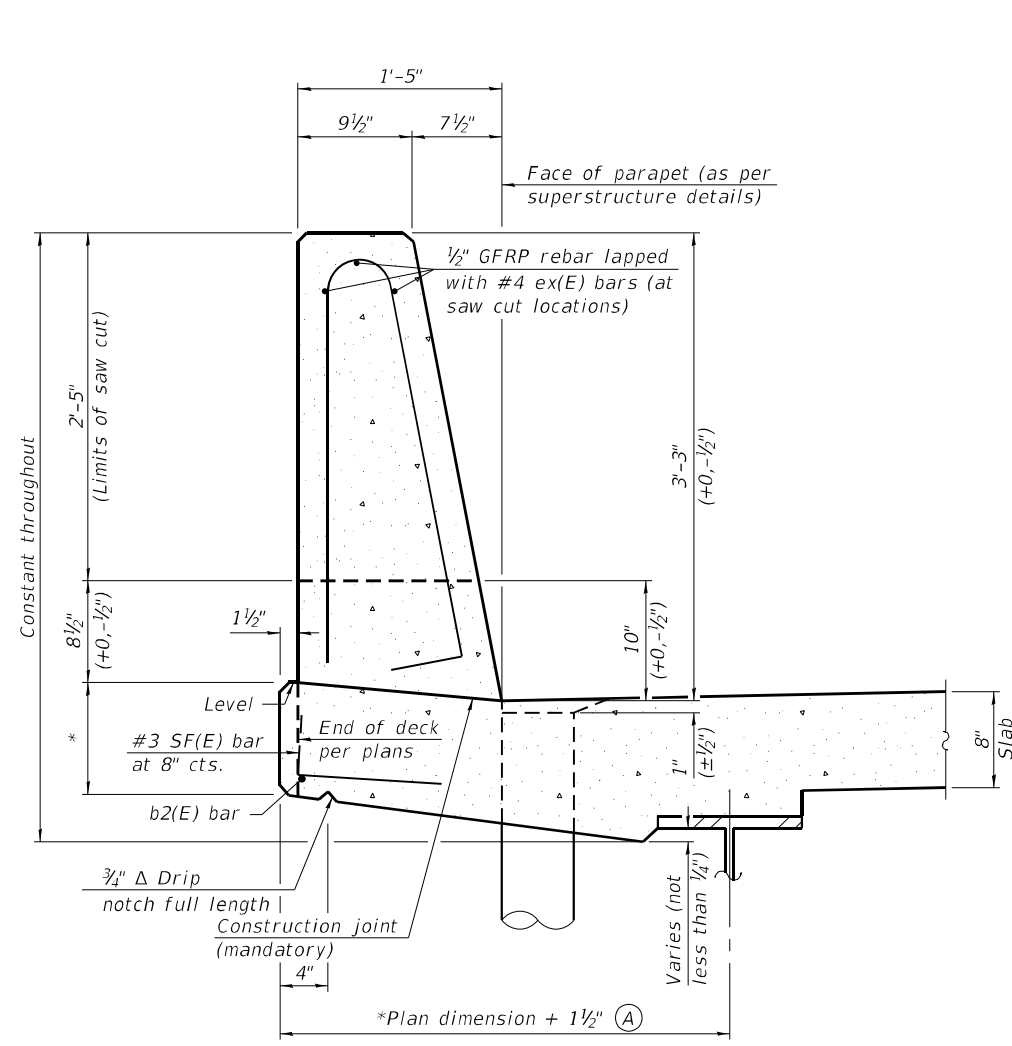
Note:
 The metal shell piles shall be according to Article 1006.05 of the Standard Specifications.

F-MS 8-11-2017

FILE NAME = 0101002-70B99-025- Pile Details.dgn	USER NAME =	DESIGNED - GBR	REVISIONS -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	METAL SHELL PILE DETAILS STRUCTURE NO. 010-1002	F.A.I. RE. 57/74	SECTION (10-34-1)HKB	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 773	
BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - FAM	REVISIONS -			SHEET NO. 25 OF 30 SHEETS		CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT	
BFW	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISIONS -								
400 NORTH COURSE STREET MORRIS, ILLINOIS 62559 PHONE - 618/361-5199	CHECKED - GBR	REVISIONS -	REVISIONS -								

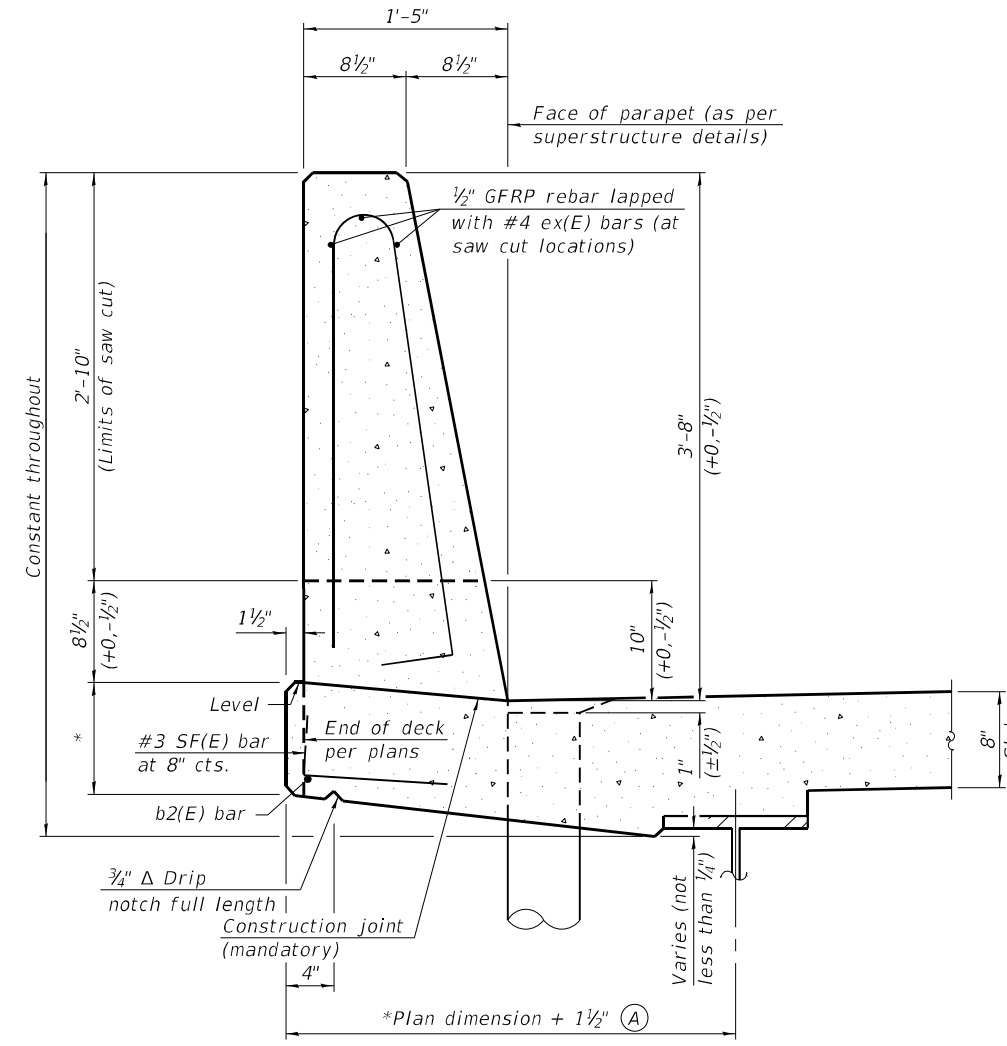
GENERAL NOTES

All dimensions shall remain the same as shown on superstructure details, except dimension A which is to be revised as shown. Additional concrete needed to revise dimension A = 0.00348 cu. yds./ft. for 39" and 44" parapets.
 Place full depth aluminum sheets as shown on superstructure details.
 Replace all cork joint filler locations with a full thickness saw cut.
 Steel superstructure shown. Other superstructure types similar.



**39" CONSTANT-SLOPE
PARAPET SECTION**

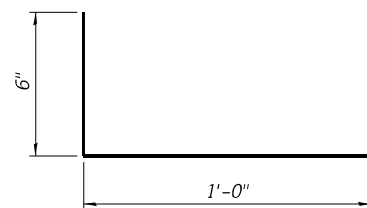
(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)



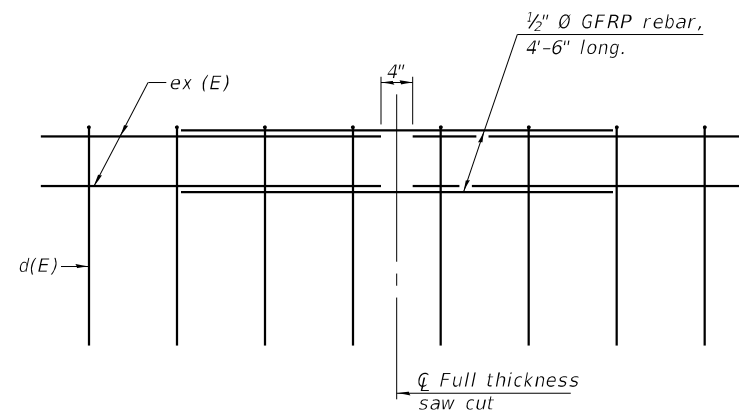
**44" CONSTANT-SLOPE
PARAPET SECTION**

(Showing dimensions, d(E), and 1/2" Ø GFRP rebar)

*See Superstructure Details.



#3 (E) BAR



GFRP REBAR STIFFENING DETAIL

(Place as shown in parapet section at each parapet joint location.)

SFP 39-44 1-14-2019

FILE NAME = 0101002-70899-026- Parapet Slipforming.dwg	USER NAME =	DESIGNED - GBR	REVISED -
BACON FARMER WORKMAN ENGINEERING & TESTING, INC.		CHECKED - FAM	REVISED -
433 NORTH COLT ST MORRIS, ILL 62458-0001 PHONE - 618-987-8100	PLOT SCALE =	DRAWN - BJV	REVISED -
	PLOT DATE = 3/11/2021	CHECKED - GBR	REVISED -

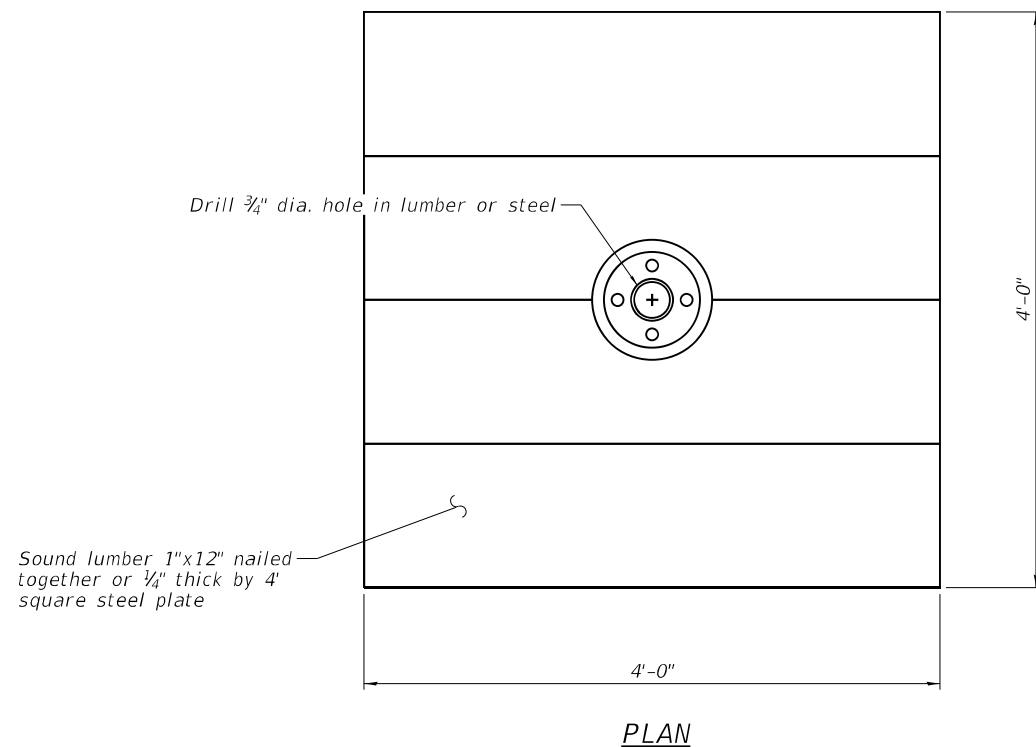
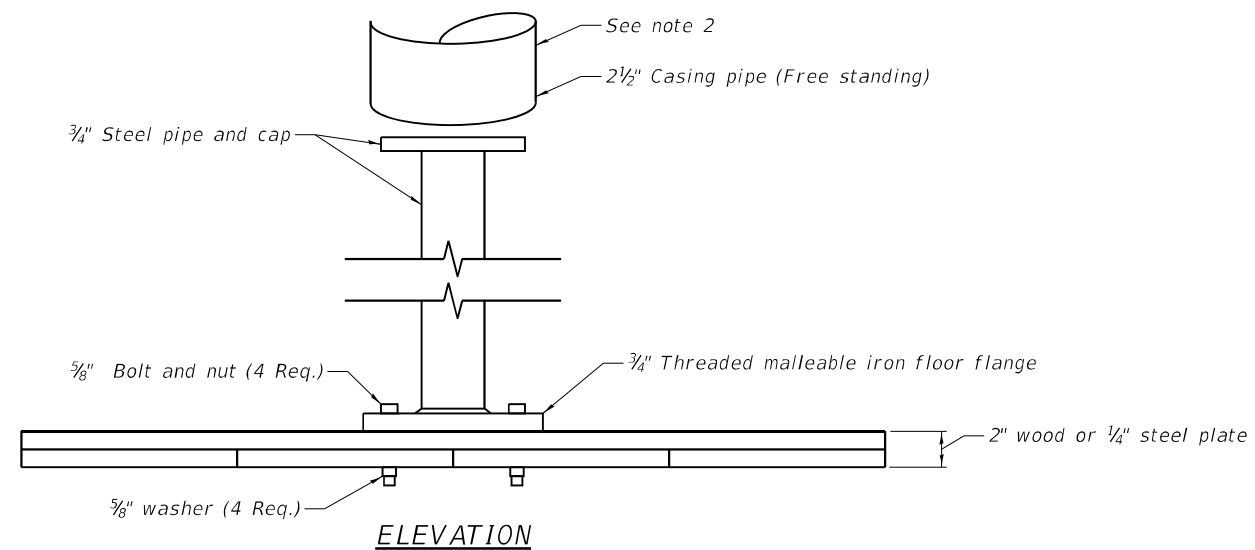
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**CONCRETE PARAPET SLIPFORMING OPTION
STRUCTURE NO. 010-1002**

SHEET NO. 26 OF 30 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
57/74	(10-34-1)HBK	CHAMPAIGN	1187	774
CONTRACT NO. 70B99				

ILLINOIS FED. AID PROJECT



NOTES:

1. Settlement platform shall be in accordance with the applicable portions of Article 204.06 of the Standard Specifications.
2. Do not install casing pipe until after one section of 3/4" steel pipe has been covered with earth. The casing pipe should not rest on platform.

FILE NAME = 0101002-70B99-027-Settlement Platform.dgn	USER NAME =	DESIGNED - GBR	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SETTLEMENT PLATFORM STRUCTURE NO. 010-1002	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
 BACON FARMER WORKMAN ENGINEERING & TESTING, INC. 433 NORTH COLT ST. SUITE 100 MORRIS, ILLINOIS 61201 PHONE: 815.987.8100	PLOT SCALE =	CHECKED - FAM	REVISED -			57/74	(10-34-1)HBK	CHAMPAIGN	1187	775	
	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -			SHEET NO. 27 OF 30 SHEETS					
		CHECKED - GBR	REVISED -			CONTRACT NO. 70B99					
						ILLINOIS FED. AID PROJECT					



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION South Abut Ramp G South Structure LOGGED BY MLL, TC
SECTION 10(5-1-RS-1, 14-1.6)R LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3 PM
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE Auto

STRUCT. NO. Station	D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ n/a ft				D E P T H	B L O W S	U C S Qu	M O I S T
					Stream Bed Elev. _____ ft							
BORING NO. B-28 Station 723+35.10 Offset 0.0ft Ground Surface Elev. 753.34 ft	(ft)	(tsf)	(%)		Groundwater Elev.:				(ft)			
					∇ First Encounter 746.3 ft ∇ Upon Completion washed ft ∇ After Hrs. _____ ft (tsf) (%)							
TOPSOIL: Silty Clay, black					SILTY CLAY TILL: Gray, very stiff (continued)							
752.34												
SILTY CLAY: Black, stiff, organics	2											
	3	1.71	18.3									
	4	B										
750.34												
SILTY CLAY LOAM: Brown/Gray, soft	1											
	2	0.49	21.5									
	3	B										
748.84												
SILTY CLAY LOAM: Gray, soft	-5											
	3	B										
747.34												
SAND: Brown, medium, coarse (washed auger)	1											
	3	0.33	12.8									
	7	B										
744.84												
SAND: Brown, medium, fine	4											
	6		13.6									
	9											
	-10											
	8											
	16		14.9									
	13											
	14											
	8		11.6									
	9											
	-15											
736.84												
SILTY CLAY TILL: Gray, stiff	3											
	7	1.81	9.9									
	8	B										
734.84												
SILTY CLAY TILL: Gray, very stiff	4											
	5	2.37	9.7									
	10	B										
	-20											
	5											
	4	3.5	7.4									
	6	P										
	-40											

File Name P:\GINT\PROJECTS\15774 CHAMPAIGN COUNTY\GPI Data Template D0TEMP\LT.GDT Date Printed 3/2/15
Latitude 40.144779 Longitude -88.26089 Datum Job Number MCE-1604

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE I-57/74 DESCRIPTION South Abut Ramp G South Structure LOGGED BY MLL, TC
SECTION 10(5-1-RS-1, 14-1.6)R LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3 PM
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE Auto

STRUCT. NO. Station	D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ n/a ft				D E P T H	B L O W S	U C S Qu	M O I S T
					Stream Bed Elev. _____ ft							
BORING NO. B-28 Station 723+35.10 Offset 0.0ft Ground Surface Elev. 753.34 ft	(ft)	(tsf)	(%)		Groundwater Elev.:				(ft)			
					∇ First Encounter 746.3 ft ∇ Upon Completion washed ft ∇ After Hrs. _____ ft (tsf) (%)							
SILTY CLAY TILL: Gray, stiff					SILTY CLAY TILL: Gray, stiff (continued)							
	4											
	5	1.24	11.4									
	8	B										
	-45											
	5											
	6	0.99	12.1									
	9	B										
	-50											
	5											
	9	2.55	10.6									
	9	B										
	-70											
	4											
	6	1.48	11.1									
	8	B										
	-55											
	5											
	9	1.81	10.6									
	12	B										
	-75											
End of Boring												
	4											
	6	1.24	10.8									
	10	B										
	-60											

File Name P:\GINT\PROJECTS\15774 CHAMPAIGN COUNTY\GPI Data Template D0TEMP\LT.GDT Date Printed 3/2/15
Latitude 40.144779 Longitude -88.26089 Datum Job Number MCE-1604

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

Page 1 of 2

Date 1/29/15

ROUTE I-57/74 DESCRIPTION Pier Ramp G South Structure LOGGED BY MLL

SECTION 10(5-1-RS-1, 14-1.6)R LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM, Latitude 40.144894, Longitude -88.280157

COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE Auto

STRUCT. NO.	DEP	BL	UCS	MOIST	Surface Water Elev.	DEP	BL	UCS	MOIST
Station	T	OW	S	T	ft	H	OW	S	T
BORING NO.	H	S	Qu		ft	H	S	Qu	
Station	(ft)	(/6")	(tsf)	(%)	ft	(ft)	(/6")	(tsf)	(%)
Offset									
Ground Surface Elev.									
753.44					n/a				
752.44									
	2								
	3	1.0	20						
	4	B							
749.94									
	1					4			
	1	0.5	17			6	3.1	9	
	3	P				9	B		
747.94									
	5								
	10		12						
	12								
743.94						7			
	8	0.6	15			8	1.5	7	
	9	B				8	P		
742.44									
	4								
	10	1.1	12						
	11	B							
739.94									
	4					4			
	7	1.5	9			5	2.0	11	
	5	B				9	B		
738.94									
	6								
	7	2.7	10						
	7	B							
	4					4			
	6	2.7	10			9	2.1	10	
	8	B				10	B		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Page 2 of 2

Date 1/29/15

ROUTE I-57/74 DESCRIPTION Pier Ramp G South Structure LOGGED BY MLL

SECTION 10(5-1-RS-1, 14-1.6)R LOCATION SEC. 34, TWP. 20N, RNG. 8E, 3rd PM, Latitude 40.144894, Longitude -88.280157

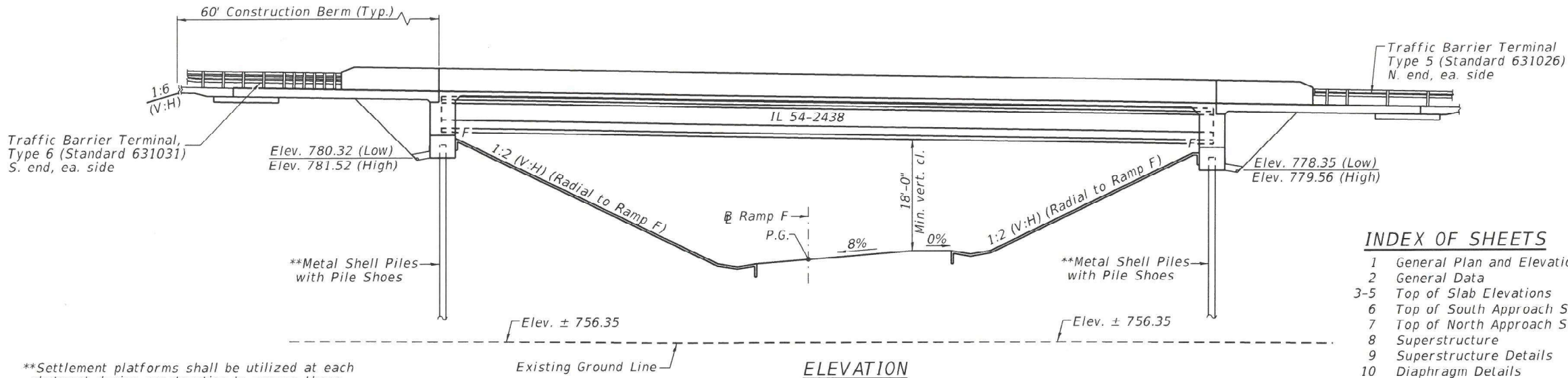
COUNTY Champaign DRILLING METHOD HSA HAMMER TYPE Auto

STRUCT. NO.	DEP	BL	UCS	MOIST	Surface Water Elev.	DEP	BL	UCS	MOIST
Station	T	OW	S	T	ft	H	OW	S	T
BORING NO.	H	S	Qu		ft	H	S	Qu	
Station	(ft)	(/6")	(tsf)	(%)	ft	(ft)	(/6")	(tsf)	(%)
Offset									
Ground Surface Elev.									
753.44					n/a				
752.44									
	4								
	6	2.0	10						
	10	B							
	10								
	11								
	11								
	15								
	15	2.0	10						
	15	B							
	6								
	11	2.0	10						
	11	B							
	6								
	8	2.1	11						
	11	B							
	6								
	8								
	11								
	14								
	20	1.5	11						
	35	P							
	6								
	10	2.9	10						
	15	B							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, form 137 (Rev. 8-99)

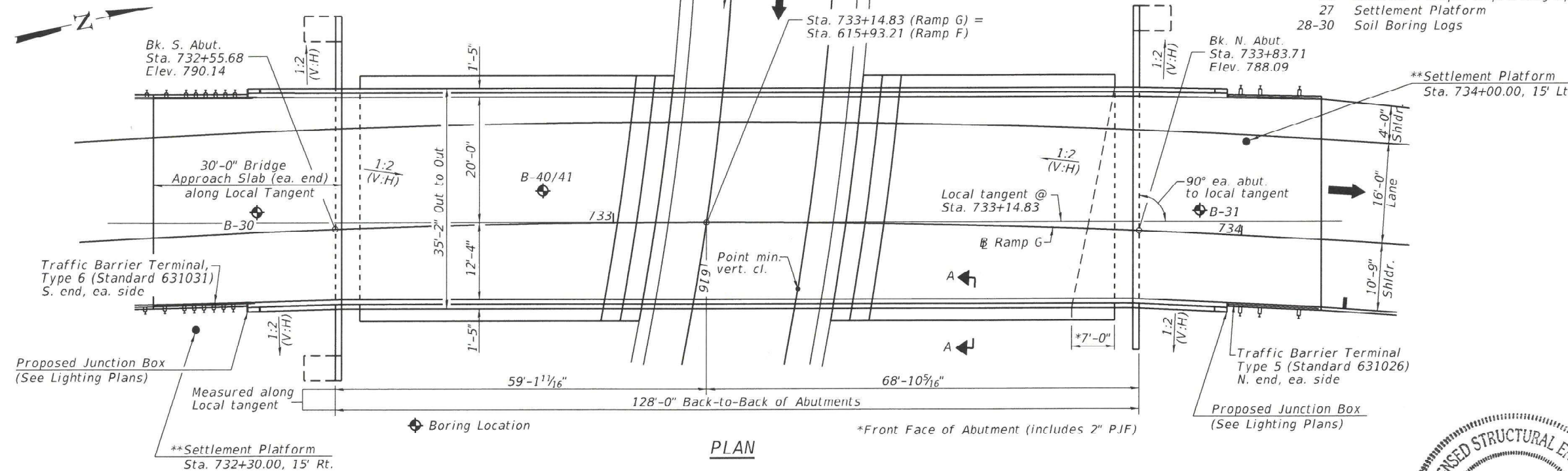
Bench Mark: Chiseled "□" on top of N.W. corner of light pole foundation #50-107 on Ramp DB, Sta. 1068+46.46 Elev. 769.173

Existing Structure: None No Salvage



**Settlement platforms shall be utilized at each abutment during construction to ensure there is less than 0.4 inches or less left of settlement prior to the installation of the piles to ensure the effects of down drag forces are negligible

Note:
Up to 1/4" may be ground off the bridge deck and the bridge approach slab.
See Sheet 2 of 30 for Offset Sketch



LOADING HL-93
Allow 50 psf for future wearing surface

DESIGN SPECIFICATIONS
2014 AASHTO LRFD Bridge Specifications, 7th Edition w/2015 & 2016 Interims

DESIGN STRESSES

FIELD UNITS
f'c = 3,500 psi (Cast-in-Place)
f'c = 4,000 psi (Superstructure Concrete)
fy = 60,000 psi (Reinforcement)

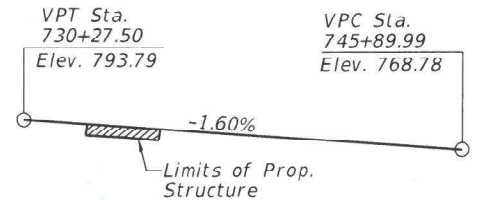
PRECAST PRESTRESSED UNITS
f'c = 8,500 psi
fci = 6,500 psi
fpu = 270,000 psi (0.6" Ø low lax strands)
fpbt = 202,300 psi (0.6" Ø low lax strands)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration at 1.0 sec (SD1) = 0.135g
Design Spectral Acceleration at 0.2 sec (SDS) = 0.233g
Soil Site Class = D

APPROVED
For Structural Adequacy Only
Carl Kump
Engineer of Bridges & Structures

LICENSED STRUCTURAL ENGINEER
GERALD B. ROTHERHAM
081-005673
STATE OF ILLINOIS
Gerald B. Rotherham
03/11/2021
EXP: 11/30/2022

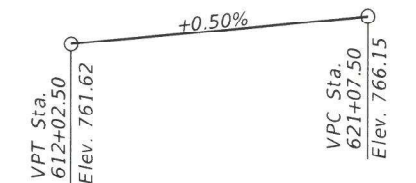


PROFILE GRADE RAMP G

(Along Ramp G)
Note: The profile grade shows the final elevations after grinding.

PROPOSED RAMP G CURVE DATA

P.I. Sta. = 730+86.74 L = 1,736.70'
Δ = 60°51'35" (Rt.) E = 261.20'
D = 3°30'16" S.E. = 6.7%
R = 1,635' P.C. Sta. = 721+26.34
T = 960.40' P.T. Sta. = 738+63.05



PROFILE GRADE RAMP F

(Along Ramp F)

PROPOSED RAMP F CURVE DATA

P.I. Sta. = 620+51.51 L = 1,041.65'
Δ = 124°20'18" (Rt.) E = 548.14'
D = 11°56'12" S.E. = 8.0%
R = 480.00' P.C. Sta. = 611+42.29
T = 909.21' P.C.C. Sta. = 621+83.95



GENERAL PLAN & ELEVATION

RAMP G OVER RAMP F

F.A.I. RTE. 57/74

SECTION (10-34-1)HBK

CHAMPAIGN COUNTY

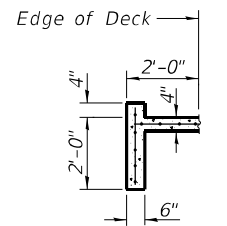
STATION 733+14.83

STRUCTURE NO. 010-1003

FILE NAME = 0101003-70899-001-GPF.dgn	USER NAME =	DESIGNED - FAM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL PLAN AND ELEVATION STRUCTURE NO. 010-1003	F.A.I. RTE. = 57/74	SECTION = (10-34-1)HBK	COUNTY = CHAMPAIGN	TOTAL SHEETS = 1187	SHEET NO. = 779	
BFW BACON FARMER WORKMAN ENGINEERING & TESTING, INC. 403 NORTH COURT STREET MARION, ILLINOIS 62959 PHONE: 618-901-7100	PLOT SCALE =	CHECKED - PS	REVISED -			SHEET NO. 1 OF 30 SHEETS					
	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -			CONTRACT NO. T0B99					
		CHECKED - GBR	REVISED -			ILLINOIS FED. AID PROJECT					

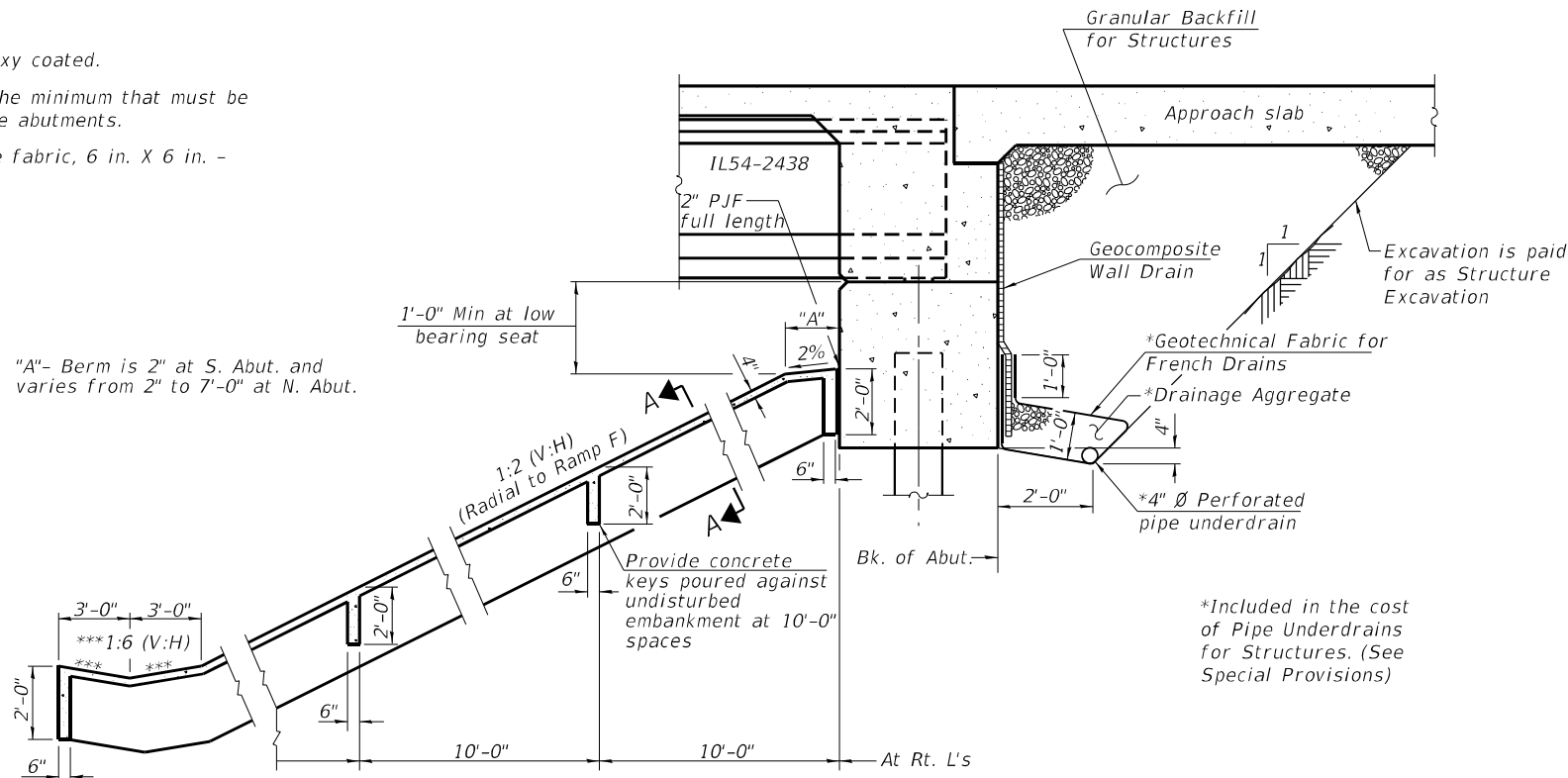
GENERAL NOTES

Reinforcement bars designated (E) shall be epoxy coated.
 The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.
 Slope wall shall be reinforced with welded wire fabric, 6 in. X 6 in. - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.



SECTION A-A

"A"- Berm is 2' at S. Abut. and varies from 2" to 7'-0" at N. Abut.



SECTION THRU INTEGRAL ABUTMENT

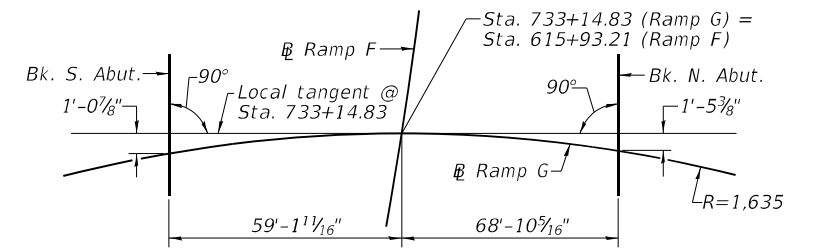
Note:
 All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Structure Excavation	Cu. Yd.		222	222
Concrete Structures	Cu. Yd.		77.7	77.7
Concrete Superstructure	Cu. Yd.	211.5		211.5
Protective Coat	Sq. Yd.	832		832
Concrete Superstructure (Approach Slab)	Cu. Yd.	98.4		98.4
Furnishing and Erecting Precast Prestressed Concrete Beams, IL54N	Foot	752		752
Reinforcement Bars, Epoxy Coated	Pound	82930	13320	96250
Slope Wall 4 Inch	Sq. Yd.		429	429
Furnishing Metal Shell Piles HP 14"x0.312"	Foot		1246	1246
Driving Piles	Foot		1246	1246
Test Pile Metal Shells	Each		2	2
Pile Shoes	Each		16	16
Name Plates	Each	1		1
Granular Backfill for Structures	Cu. Yd.		188	188
Geocomposite Wall Drain	Sq. Yd.		88	88
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	331		331
Diamond Grinding (Bridge Section)	Sq. Yd.	691		691
Pipe Underdrains for Structures 4"	Foot		155	155
Settlement Platforms	Each			2

STATION 733+14.83
 BUILT 20__ BY
 STATE OF ILLINOIS
 F.A.I. RTE. 57/74 SEC. (10-34-1)HBK
 LOADING HL-93
 STRUCTURE NO. 010-1003

NAME PLATE
 See Std. 515001



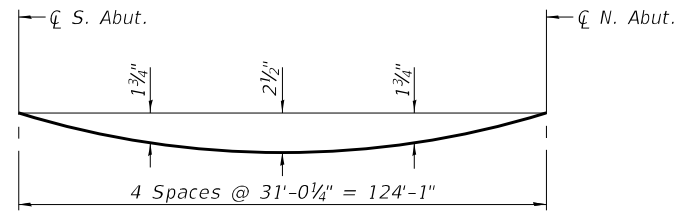
OFFSET SKETCH



FOOTING PLAN

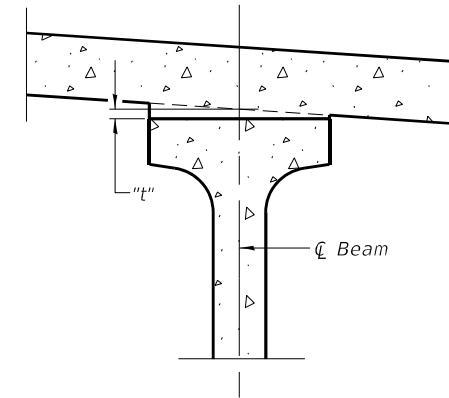
** Include 1/2" Preformed Joint Filler (See Sheet 24 of 30 for details)

FILE NAME = 0101003-70B99-002-General Data.dgn	USER NAME =	DESIGNED - FAM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL DATA STRUCTURE NO. 010-1003	F.A.I. RTE. = 57/74	SECTION = (10-34-1)HBK	COUNTY = CHAMPAIGN	TOTAL SHEETS = 1187	SHEET NO. = 780	
BFW BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - PS	REVISED -			SHEET NO. 2 OF 30 SHEETS		CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT	
403 NORTH COURT STREET MARENA, ILLINOIS 62428 PHONE - 618.972.8100	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -								
		CHECKED - GBR	REVISED -								



DEAD LOAD DEFLECTION DIAGRAM
(Includes weight of concrete only, excluding beams.)

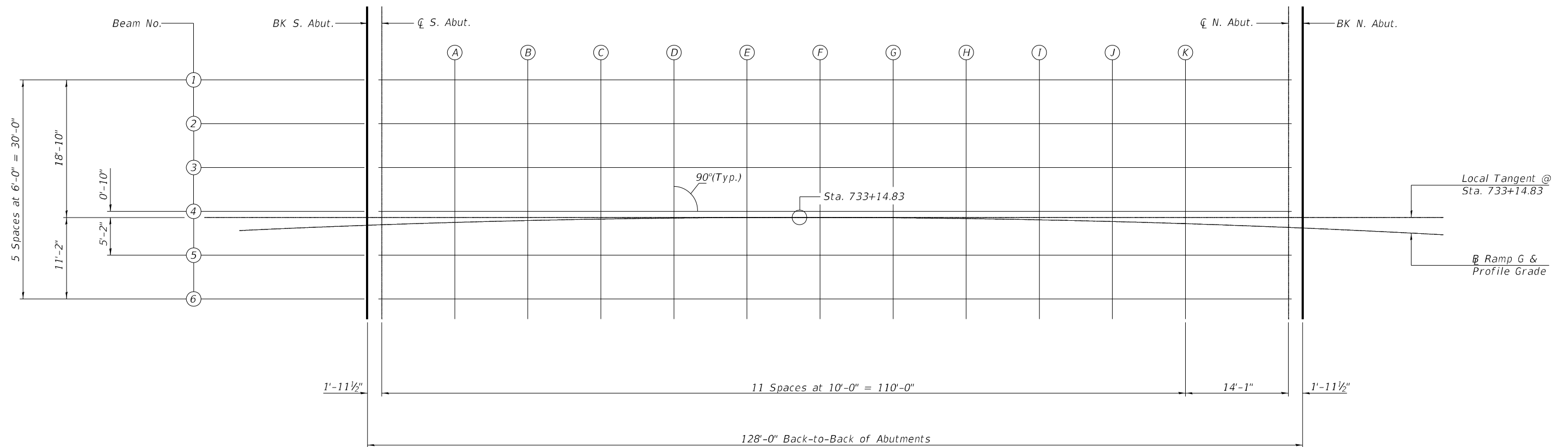
Note:
The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown on Sheet 4 and 5 of 30.



To determine "t": After all precast prestressed beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection and Grinding" shown on sheet 4 and 5 of 30, minus 8 1/4" deck thickness, equals the fillet heights "t" above top flange of beams.

The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown on sheet 4 and 5 of 30. For grinding the deck, see Special Provisions.

FILLET HEIGHTS



PLAN

FILE NAME = 0101003-70B99-003-TOS Elevations.dgn BACON FARMER WORKMAN ENGINEERING & TESTING, INC. 433 NORTH COLT ST. SUITE 100 MAHOMET, IL 61858-0100 PHONE: 618/967-8100	USER NAME =	DESIGNED - FAM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TOP OF SLAB ELEVATIONS STRUCTURE NO. 010-1003	F.A.I. RTE. =	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE =	CHECKED - PS	REVISED -			57/74	(10-34-1)HBK	CHAMPAIGN	1187	781
PLOT DATE = 3/11/2021	DRAWN - BJV	CHECKED - GBR	REVISED -	SHEET NO. 3 OF 30 SHEETS		CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT		

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	732+55.31	10.09	789.47	789.49
CL. S. Abut.	732+57.28	10.16	789.44	789.46
A	732+67.34	10.48	789.25	789.33
B	732+77.40	10.74	789.07	789.19
C	732+87.46	10.94	788.90	789.06
D	732+97.53	11.08	788.73	788.92
E	733+07.60	11.15	788.56	788.78
F	733+17.67	11.16	788.40	788.62
G	733+27.74	11.12	788.24	788.46
H	733+37.80	11.01	788.09	788.29
I	733+47.87	10.84	787.94	788.12
J	733+57.93	10.60	787.79	787.93
K	733+67.99	10.31	787.65	787.75
CL. N. Abut.	733+82.15	9.79	787.46	787.48
Bk. N. Abut.	733+84.12	9.71	787.44	787.46

WEST EDGE SHOULDER/INSIDE FACE OF CURB OR PARAPET

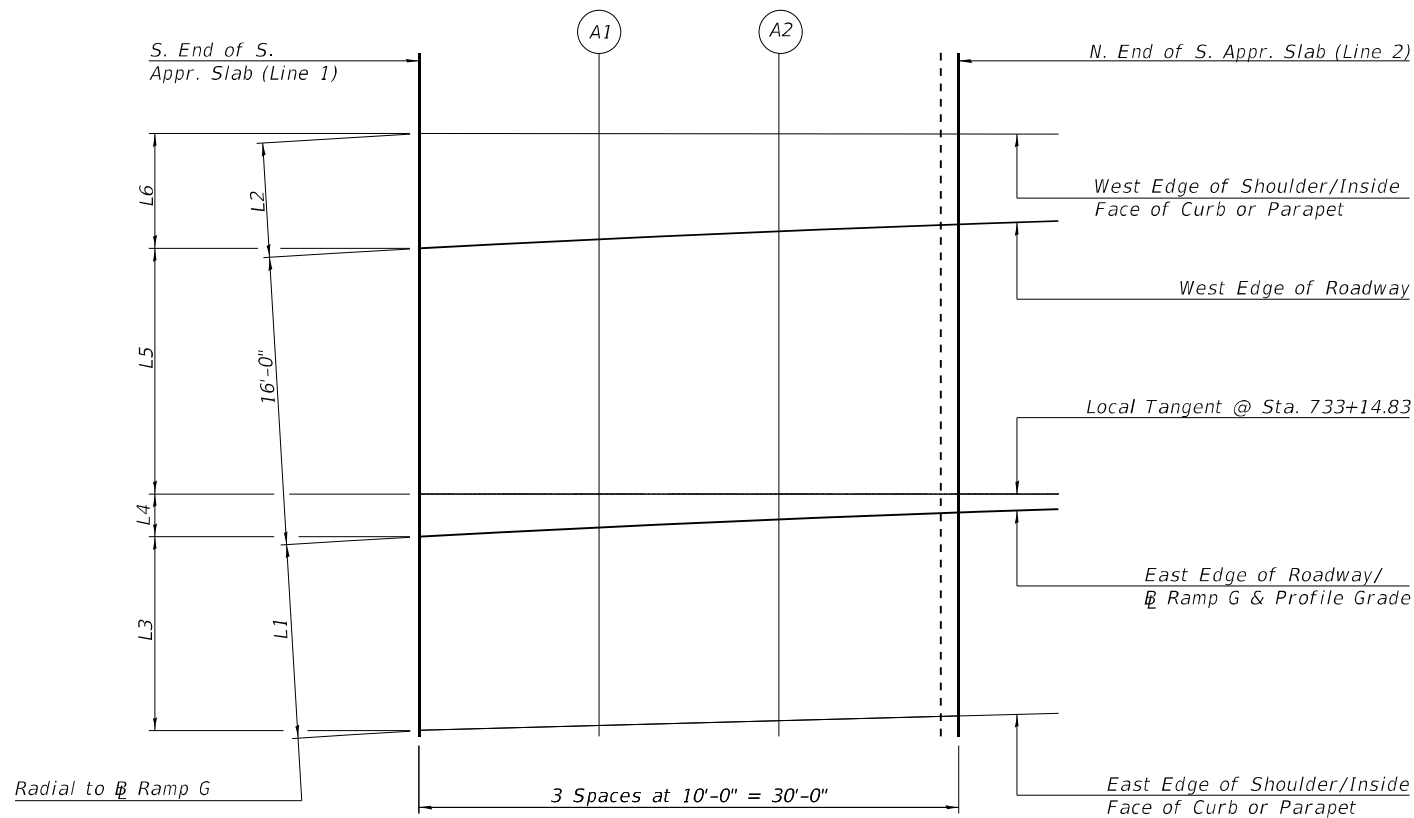
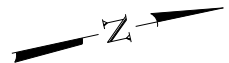
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	732+27.84	-22.37	792.09	792.11
A1	732+37.69	-21.86	791.89	791.92
A2	732+47.55	-21.41	791.71	791.73
N. End of S. Appr. Slab	732+57.42	-21.02	791.52	791.54

WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	732+27.50	-16.00	791.67	791.69
A1	732+37.42	-16.00	791.51	791.53
A2	732+47.33	-16.00	791.35	791.37
N. End of S. Appr. Slab	732+57.24	-16.00	791.19	791.21

EAST EDGE OF ROADWAY/ RAMP G & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	732+26.65	0.00	790.61	790.63
A1	732+36.66	0.00	790.45	790.47
A2	732+46.67	0.00	790.29	790.31
N. End of S. Appr. Slab	732+56.68	0.00	790.13	790.15



PLAN

EAST EDGE OF SHOULDER/INSIDE FACE OF CURB OR PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of S. Appr. Slab	732+26.06	10.75	789.90	789.92
A1	732+36.13	10.99	789.72	789.74
A2	732+46.20	11.17	789.55	789.57
N. End of S. Appr. Slab	732+56.27	11.29	789.38	789.40

SPACING FOR TOP OF APPROACH ELEVATIONS

LINE	L1	L2	L3	L4	L5	L6
1	10'-9"	6'-4 ³ / ₈ "	10'-9 ¹ / ₄ "	2'-4 ¹ / ₂ "	13'-7 ³ / ₄ "	6'-4 ¹ / ₂ "
A1	10'-11 ⁷ / ₈ "	5'-10 ¹ / ₄ "	11'-0 ¹ / ₈ "	1'-10 ³ / ₈ "	14'-1 ³ / ₄ "	5'-10 ³ / ₈ "
A2	11'-2"	5'-4 ⁷ / ₈ "	11'-2 ¹ / ₄ "	1'-5"	14'-7 ¹ / ₈ "	5'-5"
2	11'-3 ¹ / ₂ "	5'-0 ¹ / ₄ "	11'-3 ⁵ / ₈ "	1'-0 ³ / ₈ "	14'-11 ³ / ₄ "	5'-0 ¹ / ₄ "

WEST EDGE SHOULDER/INSIDE FACE OF CURB OR PARAPET

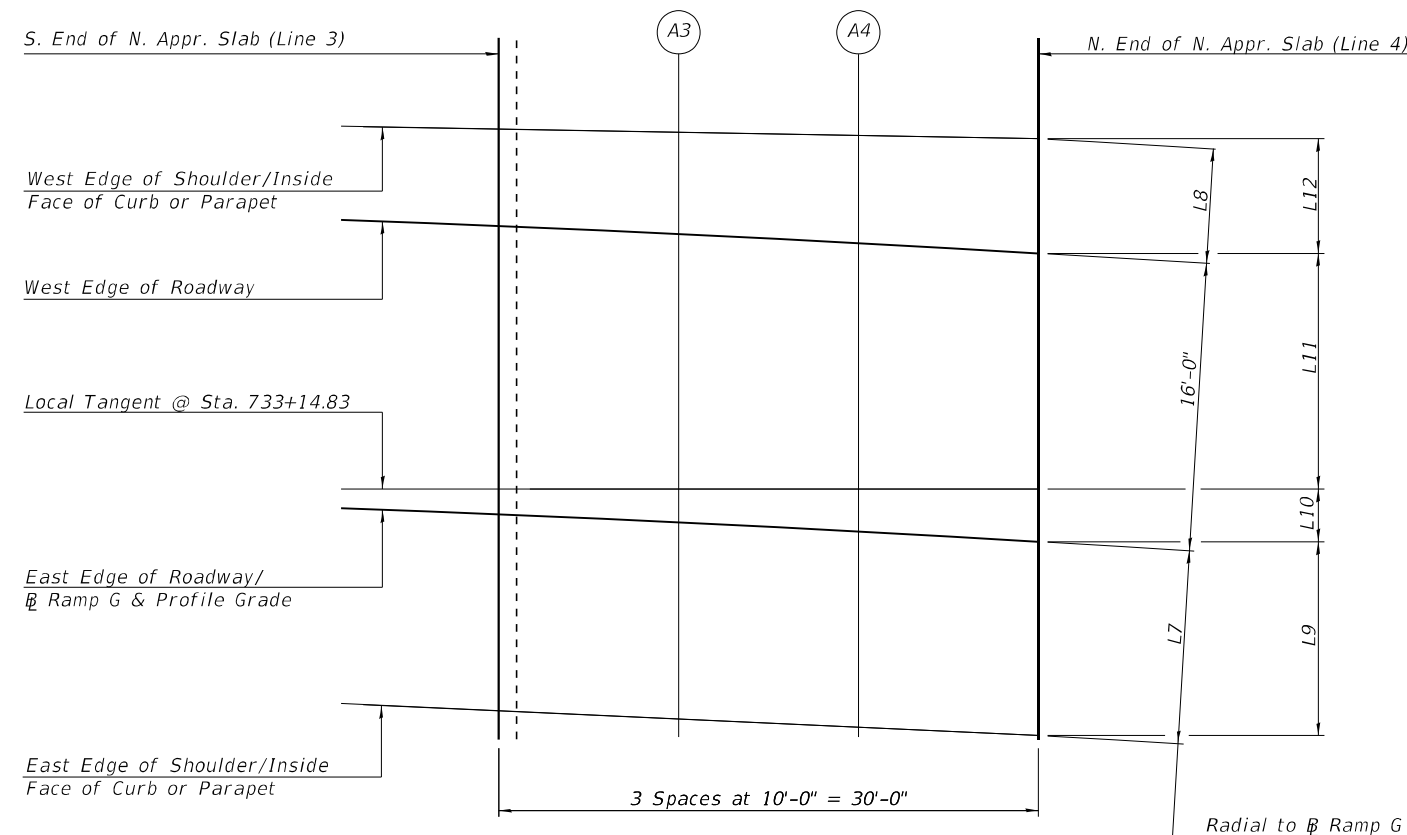
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	733+81.83	-21.39	789.56	789.58
A3	733+91.70	-21.65	789.42	789.44
A4	734+01.57	-21.98	789.28	789.30
N. End of N. Appr. Slab	734+11.43	-22.36	789.15	789.17

WEST EDGE OF ROADWAY

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	733+82.05	-16.00	789.19	789.21
A3	733+91.96	-16.00	789.03	789.05
A4	734+01.88	-16.00	788.87	788.89
N. End of N. Appr. Slab	734+11.80	-16.00	788.71	788.74

EAST EDGE OF ROADWAY/ RAMP G & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	733+82.71	0.00	788.11	788.13
A3	733+92.72	0.00	787.95	787.97
A4	734+02.73	0.00	787.79	787.81
N. End of N. Appr. Slab	734+12.75	0.00	787.63	787.65



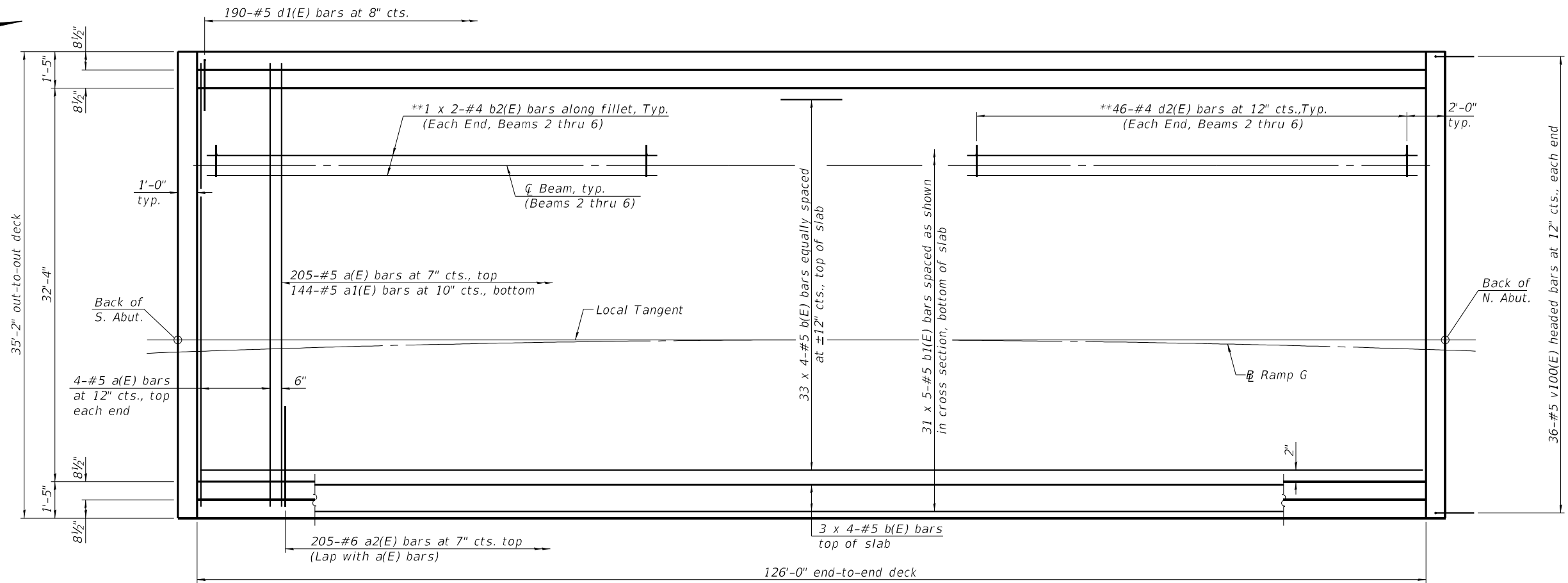
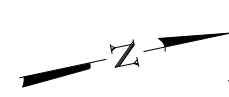
PLAN

EAST EDGE OF SHOULDER/INSIDE FACE OF CURB OR PARAPET

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted for Grinding
S. End of N. Appr. Slab	733+83.17	10.91	787.37	787.39
A3	733+93.24	10.92	787.21	787.23
A4	734+03.32	10.87	787.05	787.07
N. End of N. Appr. Slab	734+13.40	10.75	786.90	786.92

SPACING FOR TOP OF APPROACH ELEVATIONS

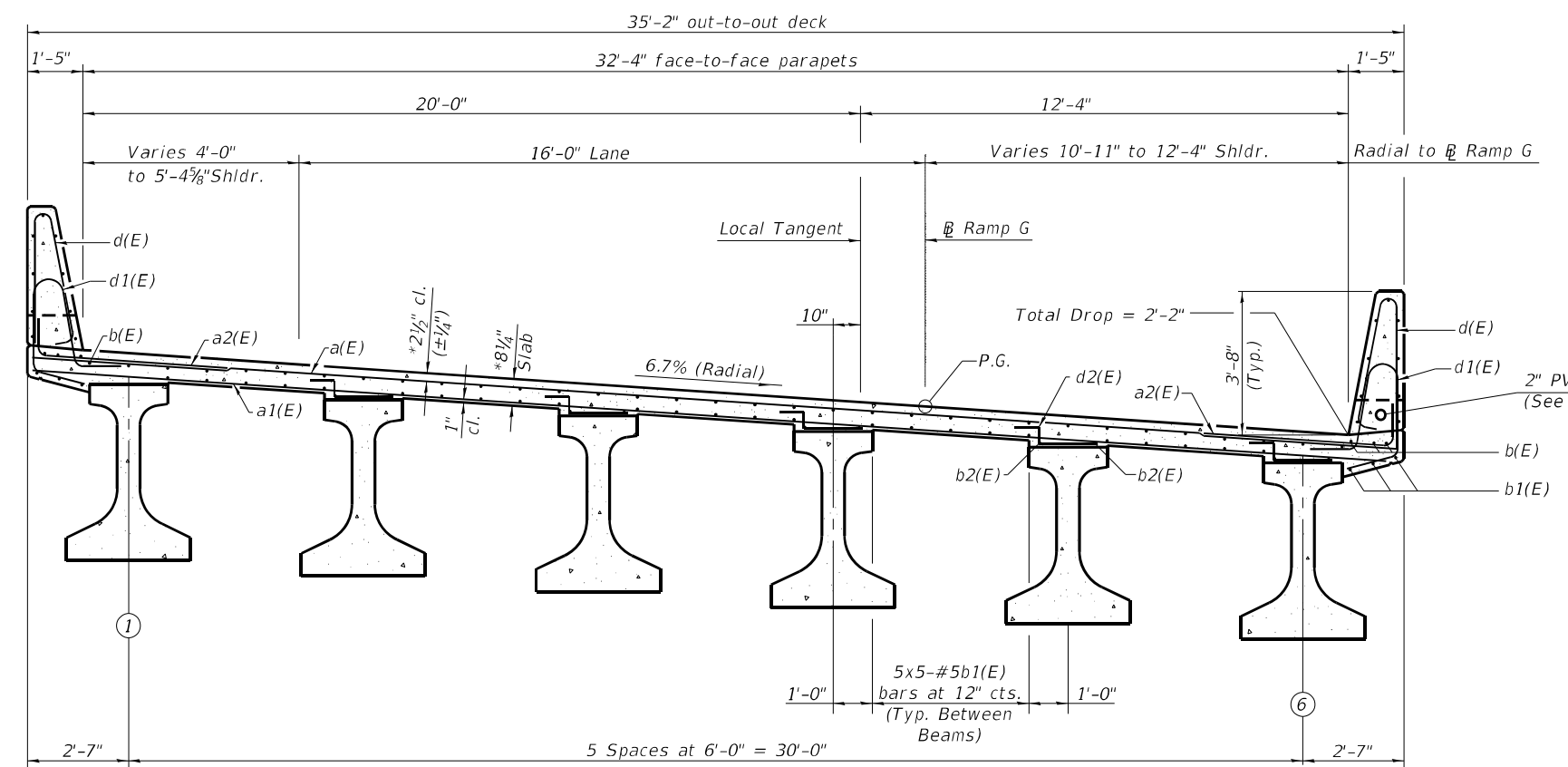
LINE	L7	L8	L9	L10	L11	L12
3	10'-11"	5'-4 ⁵ / ₈ "	10'-11 ¹ / ₈ "	1'-4 ⁷ / ₈ "	14'-7 ¹ / ₄ "	5'-4 ³ / ₄ "
A3	10'-11"	5'-7 ⁷ / ₈ "	10'-11 ¹ / ₄ "	1'-10 ¹ / ₄ "	14'-2"	5'-7 ⁷ / ₈ "
A4	10'-10 ³ / ₈ "	5'-11 ³ / ₄ "	10'-10 ⁵ / ₈ "	2'-4 ³ / ₈ "	13'-7 ⁷ / ₈ "	5'-11 ³ / ₄ "
4	10'-9"	6'-4 ¹ / ₄ "	10'-9 ¹ / ₄ "	2'-11 ¹ / ₈ "	13'-1 ¹ / ₄ "	6'-4 ³ / ₈ "



PLAN

MINIMUM BAR LAP

- #4 bar = 2'-5"
- #5 bar = 3'-6"

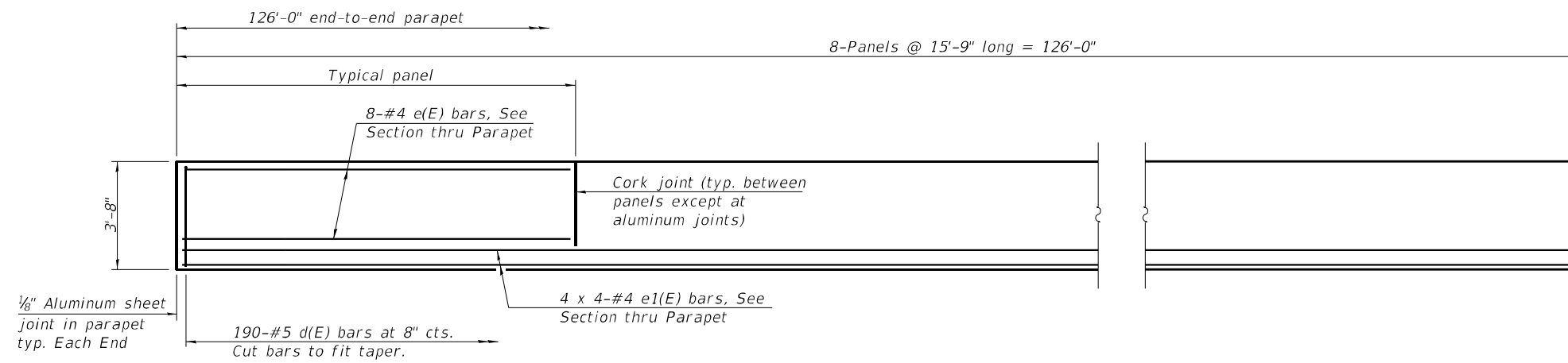


CROSS SECTION
(Looking North)

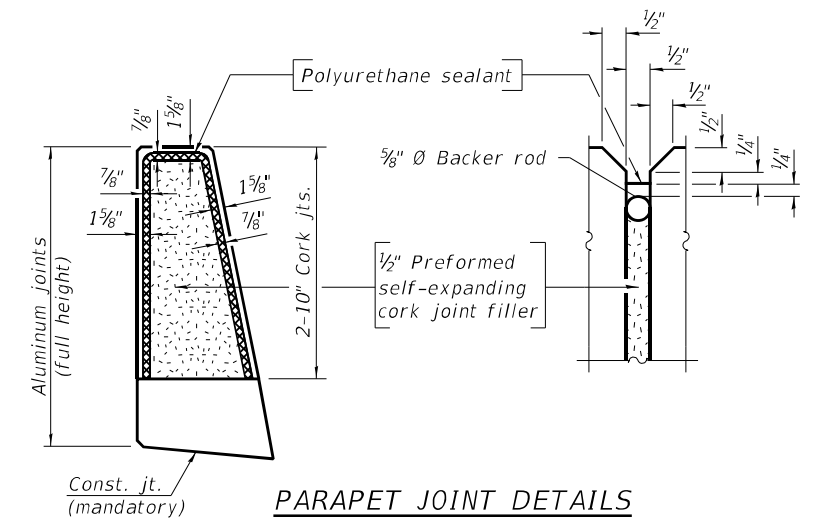
Notes:
 See sheet 9 of 30 for superstructure details and Bill of Material.
 Bars indicated thus 33 x 4-#5 etc. indicates 33 lines of bars with 4 lengths per line.

* Prior to Grinding
 ** See sheet 9 of 30 for Fillet Reinforcement Details

FILE NAME = 0101003-70899-008-Superstructure.dgn 	USER NAME =	DESIGNED - FAM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE STRUCTURE NO. 010-1003	F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	BACON FARMER WORKMAN ENGINEERING & TESTING, INC. 433 NORTH COLT ST. STREET MOUNTAIN VIEW, IL 61881-0001 PHONE: 815.987.8100	PLOT SCALE =	CHECKED - PS			REVISED -	57/74	(10-34-1)HBK	CHAMPAIGN	1187
	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -			CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT		



INSIDE ELEVATION OF PARAPET



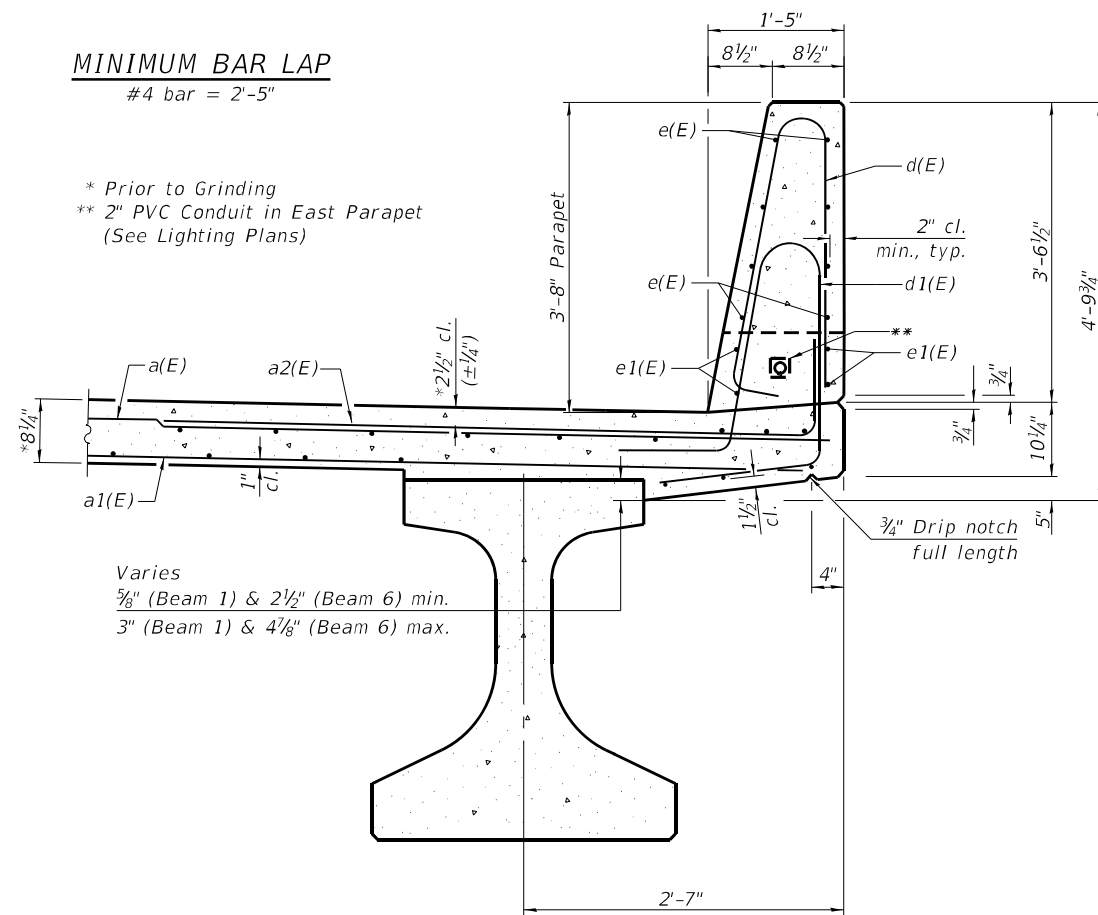
PARAPET JOINT DETAILS

Notes:
 The Polyurethane Sealant shall be according to Article 1050.04 of the Std. Spec. and the color shall be gray.
 The 1/8" Aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.
 Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated.

MINIMUM BAR LAP

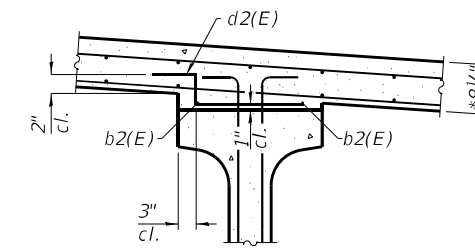
#4 bar = 2'-5"

* Prior to Grinding
 ** 2" PVC Conduit in East Parapet (See Lighting Plans)

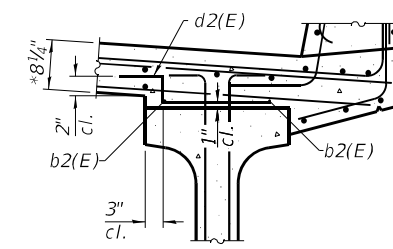


Varies
 5/8" (Beam 1) & 2 1/2" (Beam 6) min.
 3" (Beam 1) & 4 1/8" (Beam 6) max.

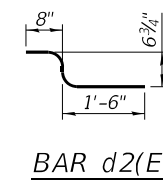
SECTION THRU PARAPET



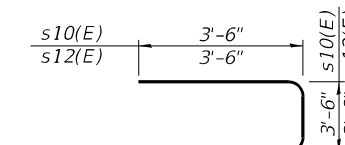
FILLET REINFORCEMENT DETAIL
(Beams 2 thru 5)



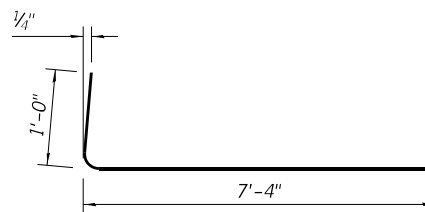
FILLET REINFORCEMENT DETAIL
(Beam 6)



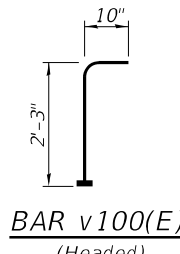
BAR d2(E)



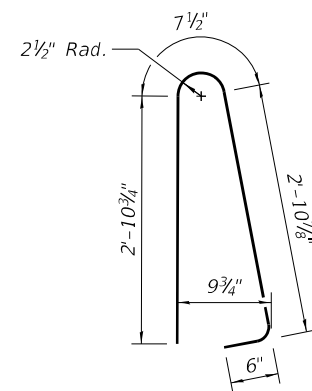
BAR s10(E) & s12(E)



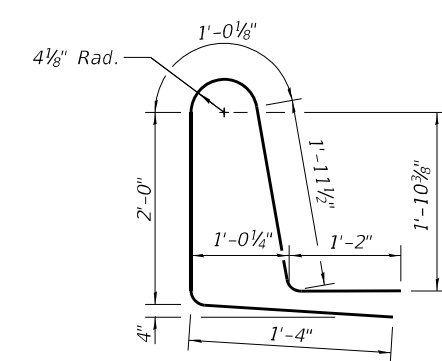
BAR a2(E)



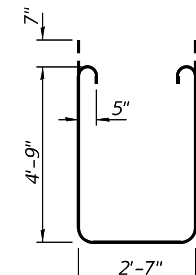
BAR v100(E)
(Headed)



BAR d(E)



BAR d1(E)

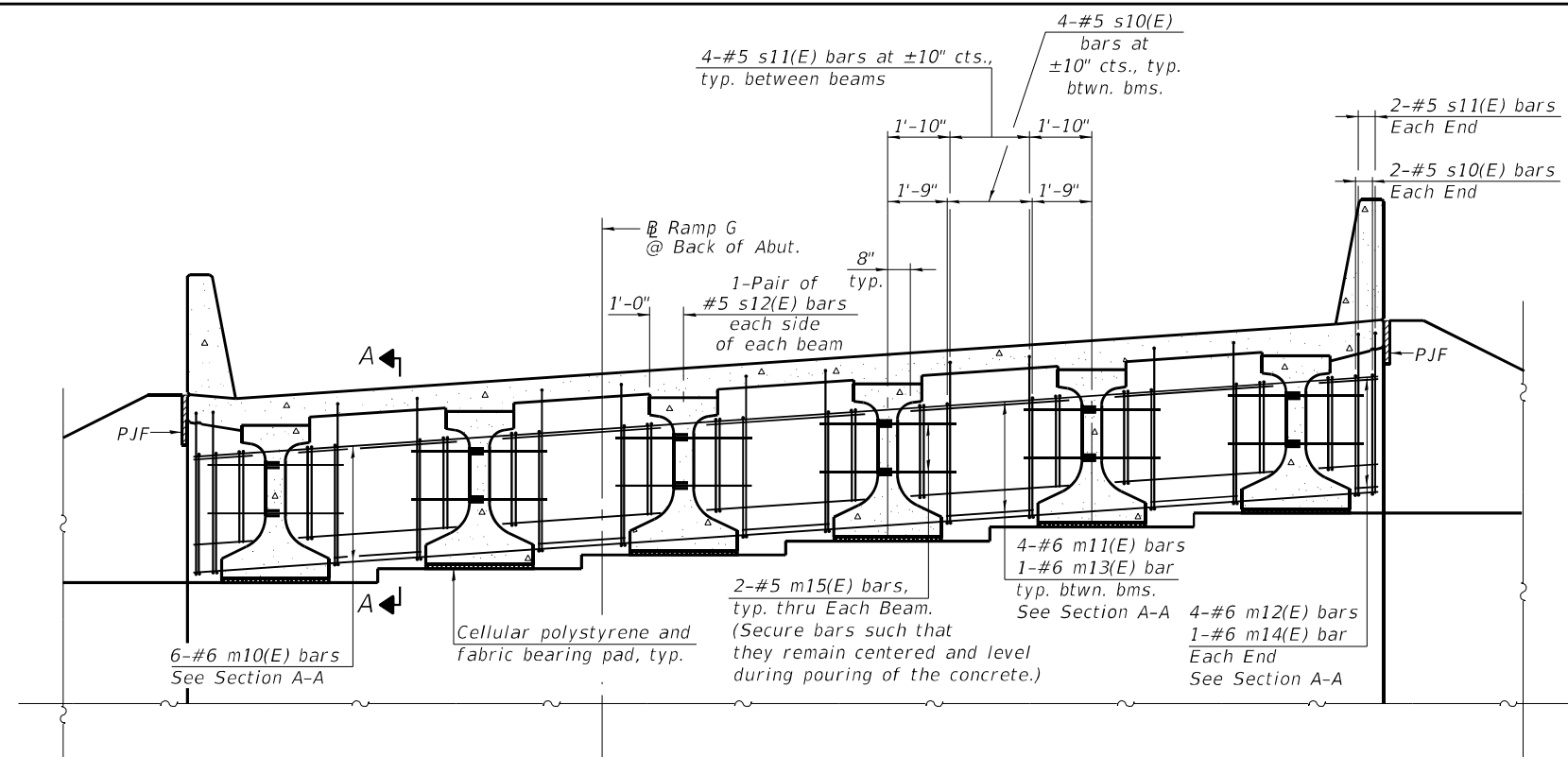


BAR s11(E)

SUPERSTRUCTURE BILL OF MATERIAL

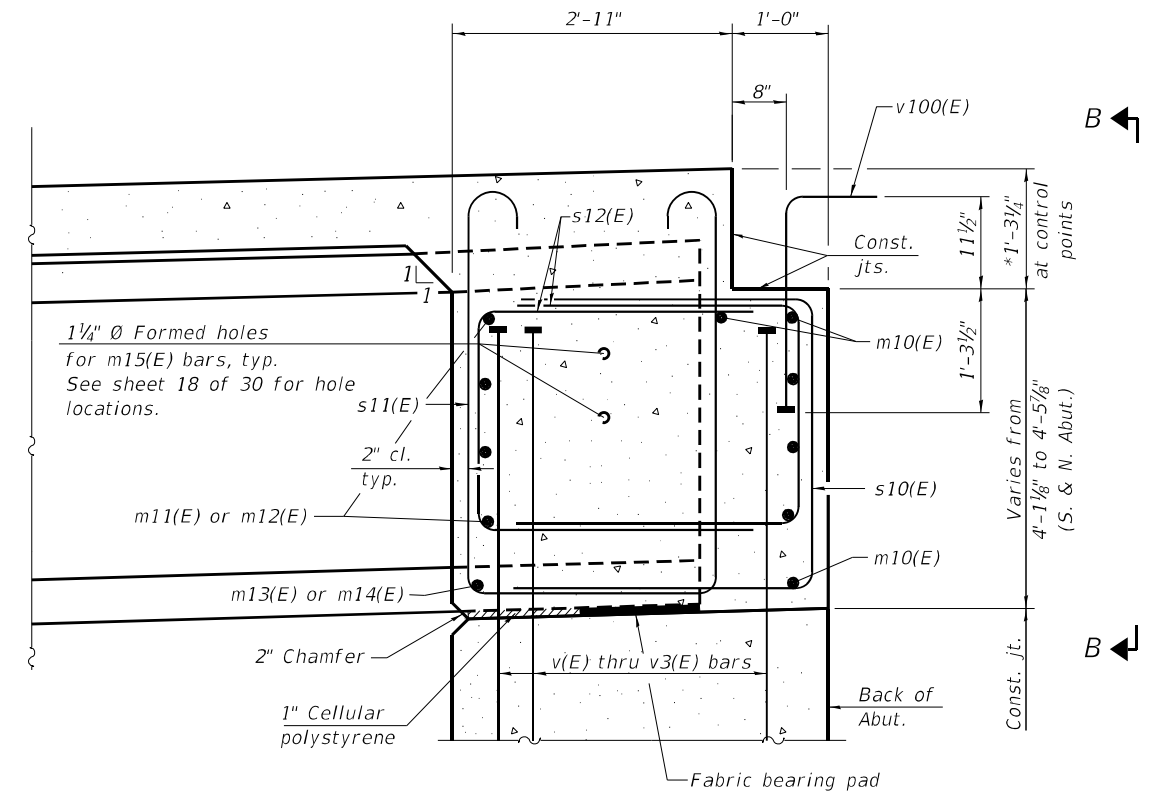
Bar	No.	Size	Length	Shape	
a(E)	213	#5	35'-0"	—	
a1(E)	144	#5	34'-8"	—	
a2(E)	410	#6	8'-4"	—	
b(E)	156	#5	34'-1"	—	
b1(E)	155	#5	28'-0"	—	
b2(E)	40	#4	24'-3"	—	
d(E)	380	#5	7'-0"	⌋	
d1(E)	380	#5	7'-6"	⌋	
d2(E)	460	#4	2'-9"	⌋	
e(E)	128	#4	15'-5"	—	
e1(E)	32	#4	33'-3"	—	
m10(E)	12	#6	34'-11"	—	
m11(E)	40	#6	4'-8"	—	
m12(E)	16	#6	1'-9"	—	
m13(E)	10	#6	2'-7"	—	
m14(E)	4	#6	9"	—	
m15(E)	24	#5	4'-0"	—	
s10(E)	48	#5	10'-6"	⌋	
s11(E)	48	#5	13'-3"	⌋	
s12(E)	48	#5	9'-8"	⌋	
v100(E)	72	#5	3'-1"	⌋	
Reinforcement Bars, Epoxy Coated				Lbs.	40450
Concrete Superstructure				Cu. Yds.	202.9

Bars indicated thus 4 x 4-#4 etc. indicates 4 line of bars with 4 lengths per line.



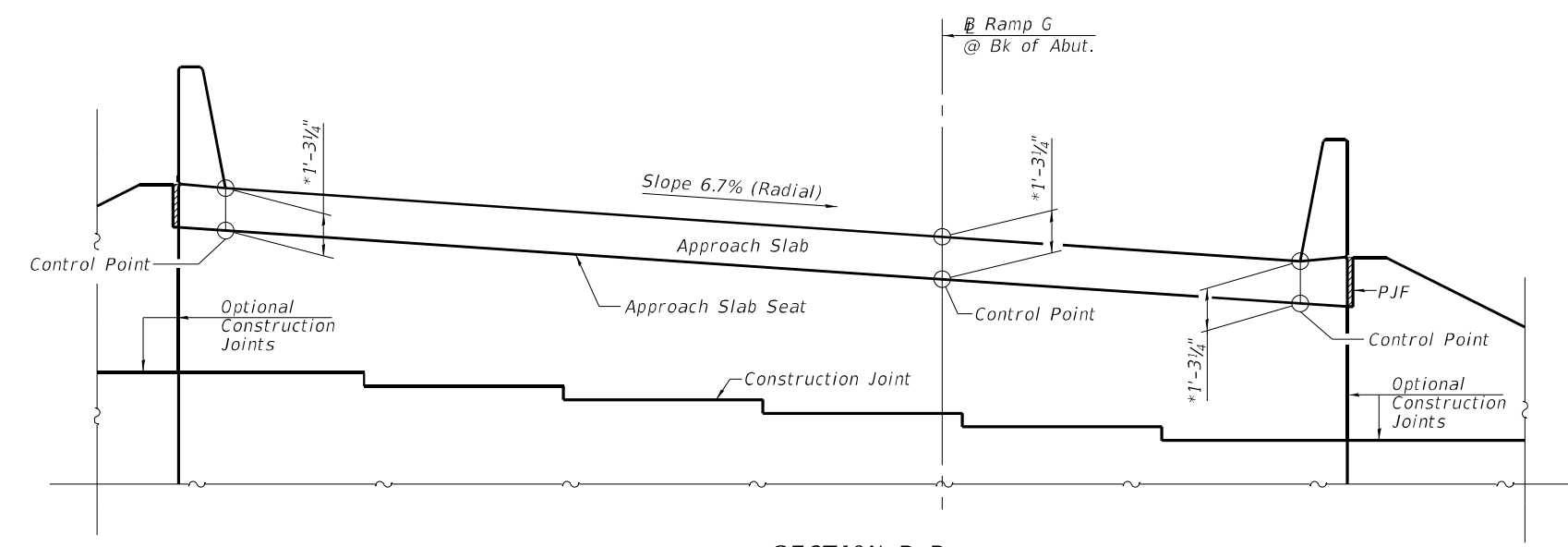
DIAPHRAGM AT ABUTMENT
 (South Abutment Shown - Looking South)
 (North Abutment Similar)

* Prior to Grinding

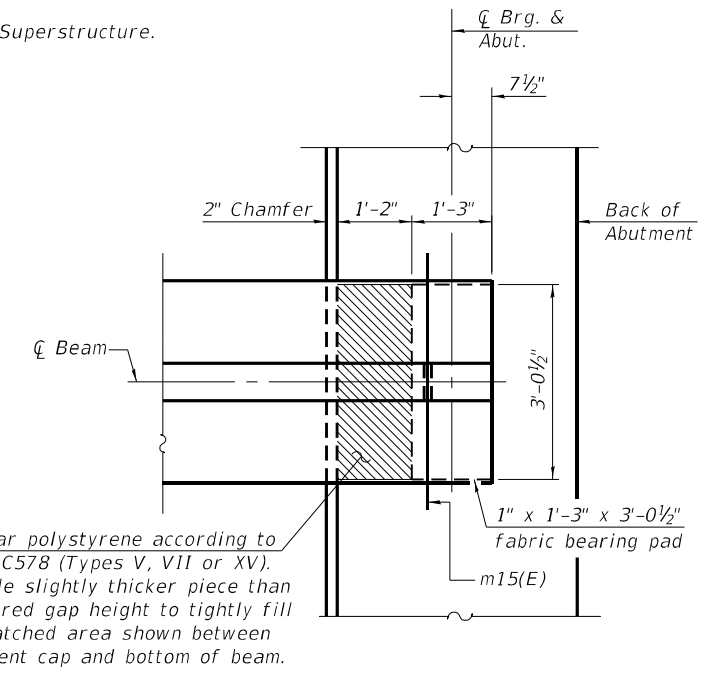


SECTION A-A

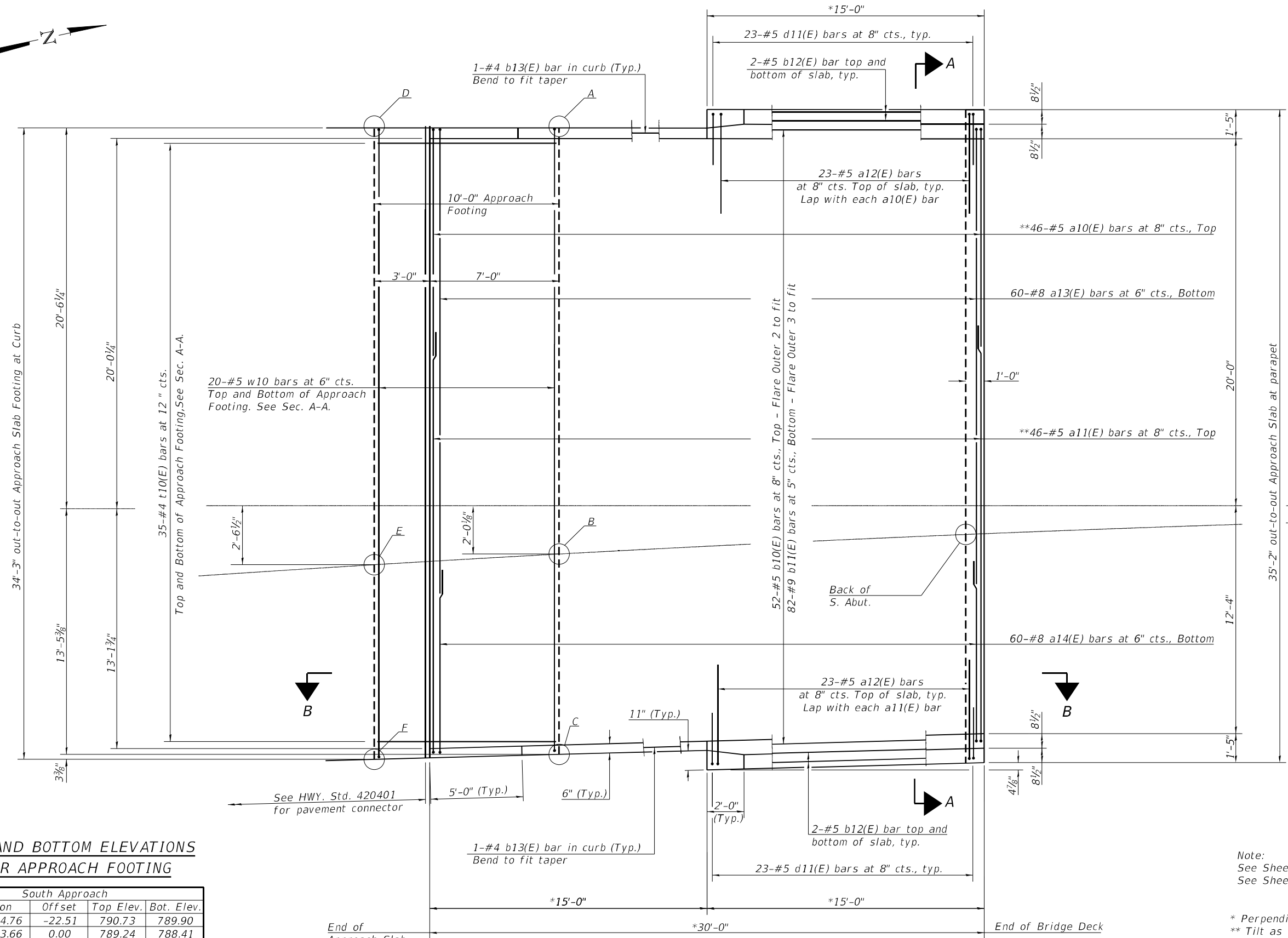
Notes:
 See sheet 9 of 30 for superstructure details and Bill of Material
 See sheet 12 and 15 of 30 for P.J.F. details
 The approach slab seat shall have a constant slope determined from the control points shown.
 Cost of cellular polystyrene is included with Concrete Superstructure.



SECTION B-B



PLAN AT ABUTMENT
 (Showing bottom flange of beam)



MINIMUM BAR LAP
 #5 bar = 3'-6"
 #8 bar = 4'-9"

**TOP AND BOTTOM ELEVATIONS
 FOR APPROACH FOOTING**

South Approach				
Point	Station	Offset	Top Elev.	Bot. Elev.
A	732+34.76	-22.51	790.73	789.90
B	732+33.66	0.00	789.24	788.41
C	732+33.08	11.43	788.49	787.65
D	732+24.91	-23.03	790.93	790.09
E	732+23.64	0.00	789.40	788.57
F	732+23.02	11.16	788.66	787.83

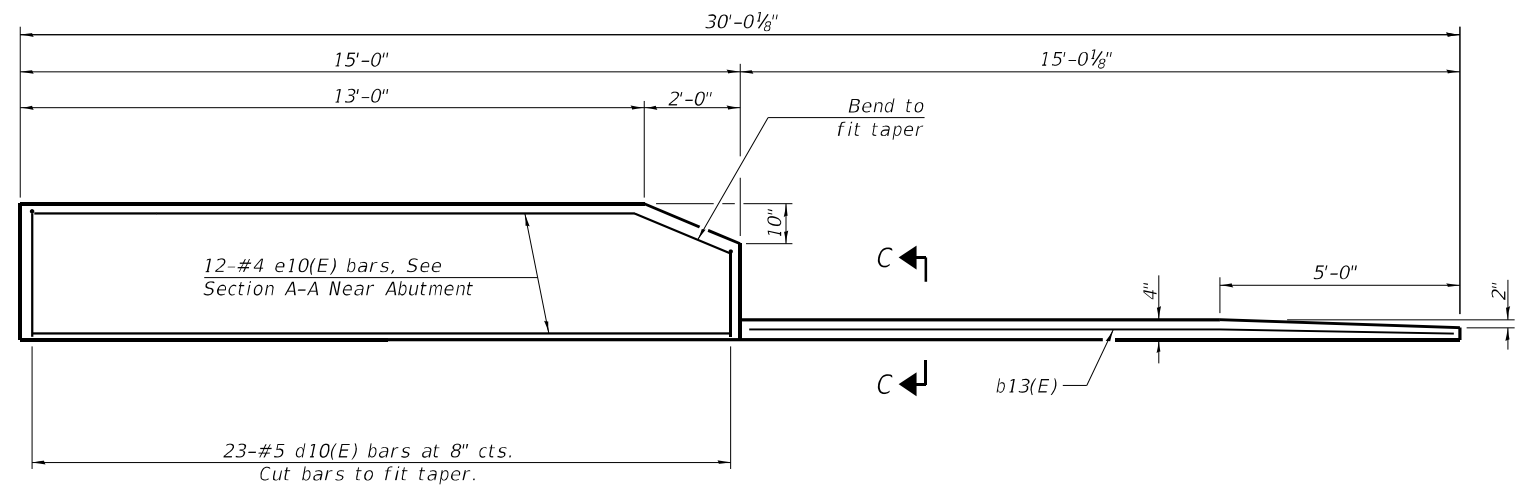
Note:
 See Sheet 12 of 30 for Section A-A.
 See Sheet 12 of 30 for Section B-B.

* Perpendicular to End of Deck and Approach slab
 ** Tilt as necessary to fit curb.

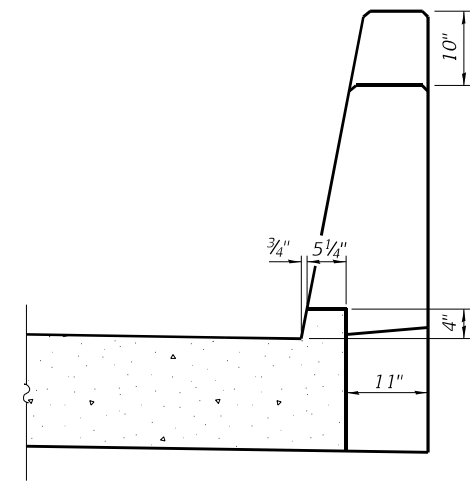
PLAN

(Sheet 1 of 3)

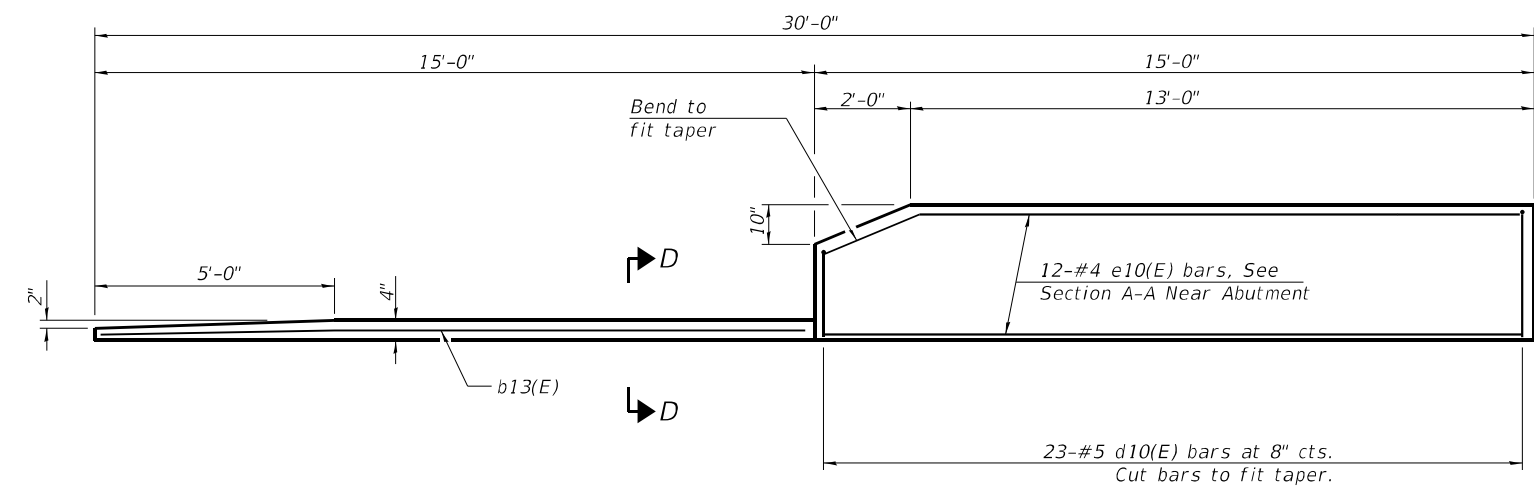
Note:
For Type 6 terminal connections see Highway Standard 631031.



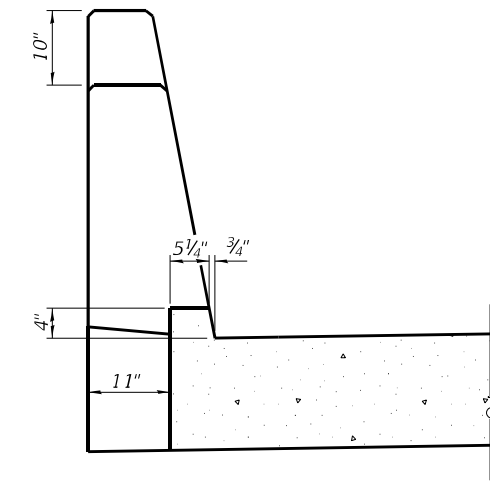
INSIDE ELEVATION OF EAST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)



VIEW C-C



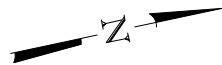
INSIDE ELEVATION OF WEST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)



VIEW D-D

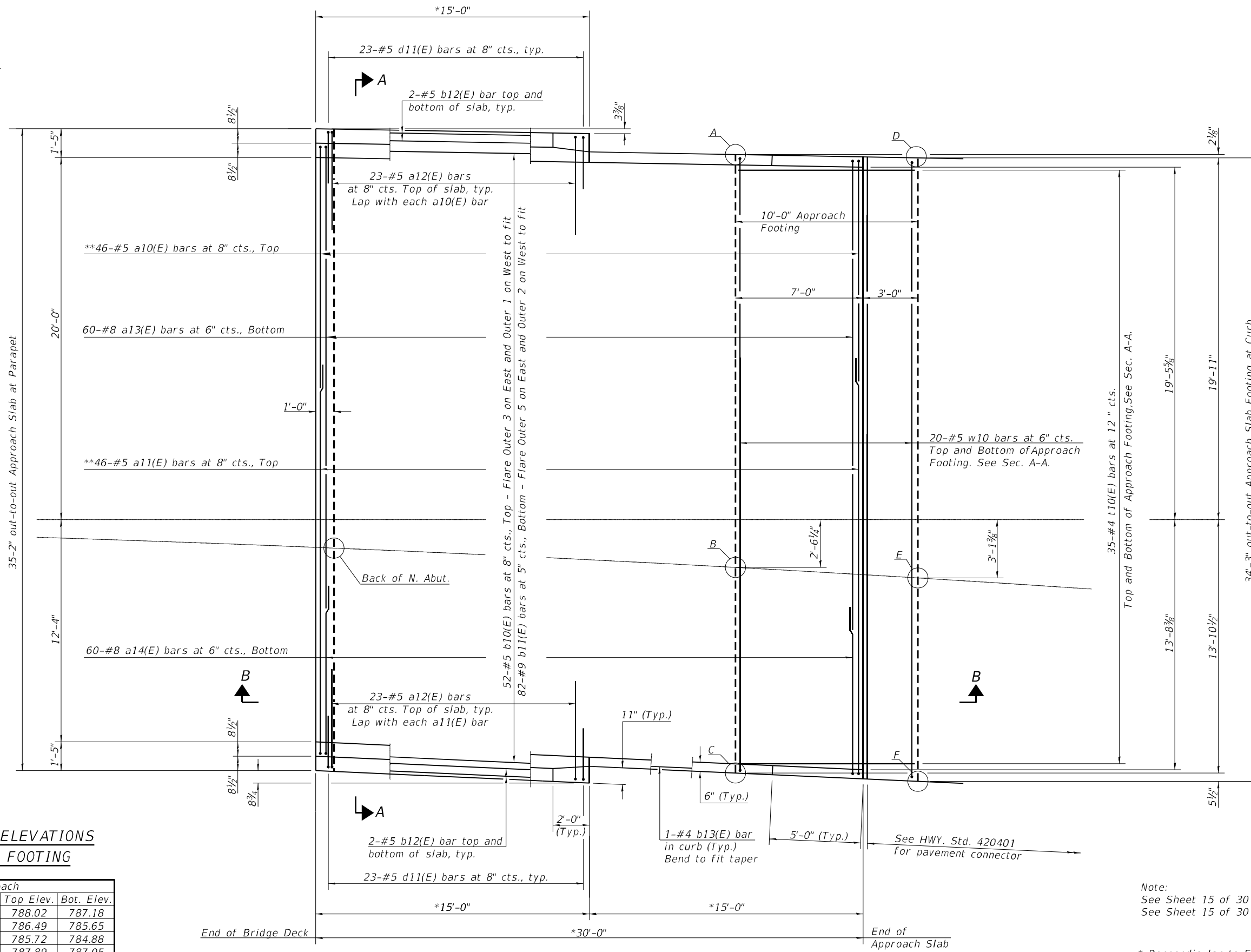
(Sheet 3 of 3)

FILE NAME = 0101003-70899-013-S_Appr_Slab_Details.dgn 	USER NAME =	DESIGNED - FAM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SOUTH BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 010-1003	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	BACON FARMER WORKMAN ENGINEERING & TESTING, INC. 433 NORTH COLT ST. SUITE 100 MAHOMET, IL 61858-0100 PHONE: 618/967-8100	PLOT SCALE =	CHECKED - PS			REVISED -	57/74	(10-34-1)HBK	CHAMPAIGN	1187
	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -		SHEET NO. 13 OF 30 SHEETS	CONTRACT NO. 70B99		ILLINOIS FED. AID PROJECT		



MINIMUM BAR LAP

#5 bar = 3'-6"
#8 bar = 4'-9"



TOP AND BOTTOM ELEVATIONS FOR APPROACH FOOTING

North Approach				
Point	Station	Offset	Top Elev.	Bot. Elev.
A	734+04.50	-22.58	788.02	787.18
B	734+05.74	0.00	786.49	785.65
C	734+06.37	11.34	785.72	784.88
D	734+14.35	-22.98	787.89	787.05
E	734+15.75	0.00	786.33	785.49
F	734+16.45	11.20	785.57	784.73

Note:
See Sheet 15 of 30 for Section A-A.
See Sheet 15 of 30 for Section B-B.

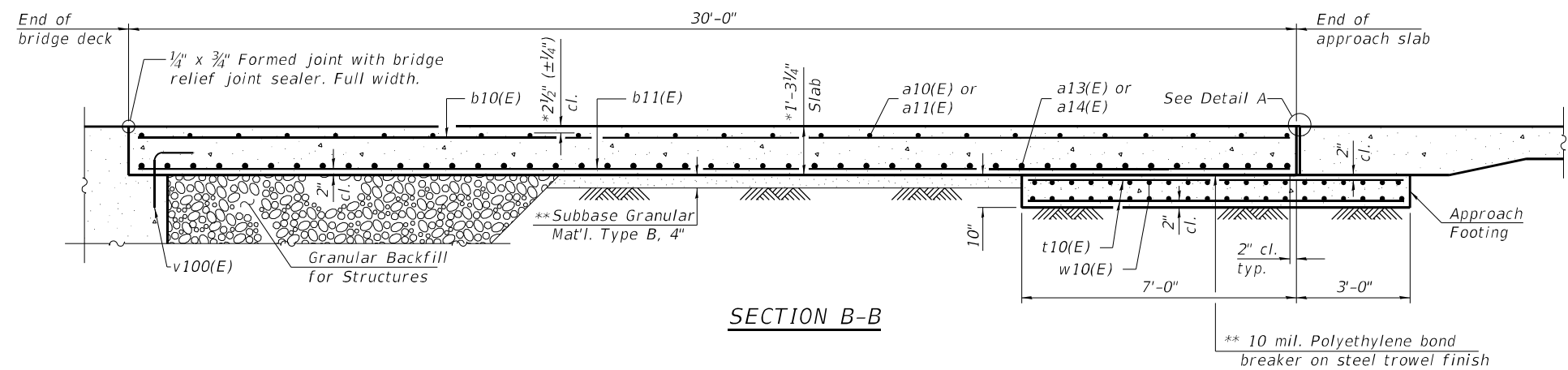
* Perpendicular to End of Deck and Approach slab
** Tilt as necessary to fit curb.

PLAN

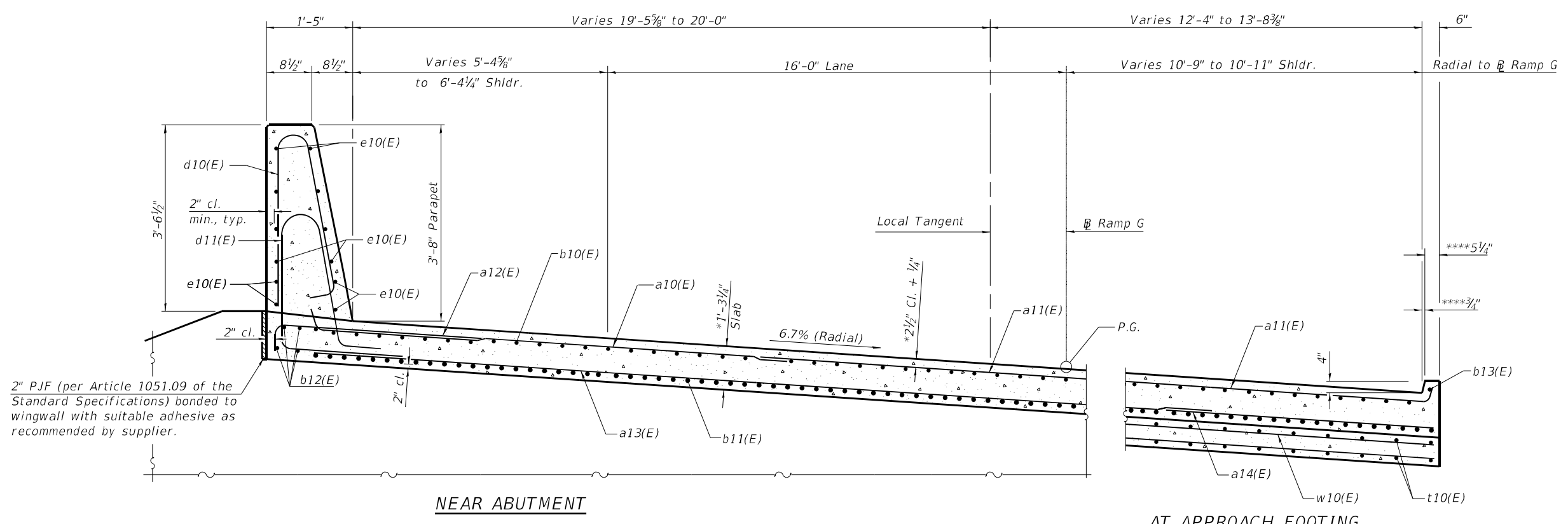
(Sheet 1 of 3)

Notes:

The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total Bridge length plus the length of the bridge approach slab.
 Parapet concrete shall be paid for as Concrete Superstructure.
 Approach slab shall be paid for as Concrete Superstructure (Approach Slab).
 Approach footing concrete shall be paid for as Concrete Structures.
 The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
 Cost of excavation for approach footing included with Concrete Structures.
 For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 30.



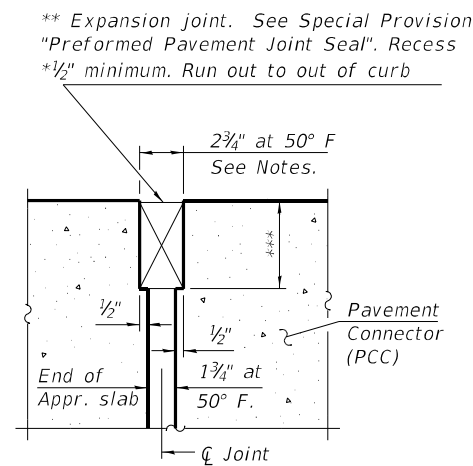
SECTION B-B



NEAR ABUTMENT

SECTION A-A

AT APPROACH FOOTING

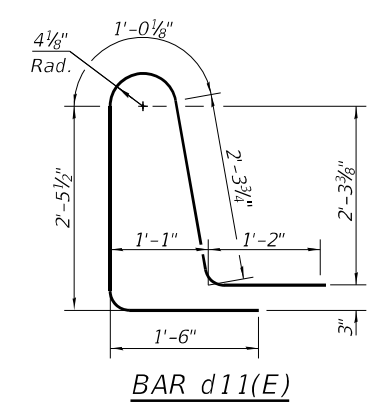
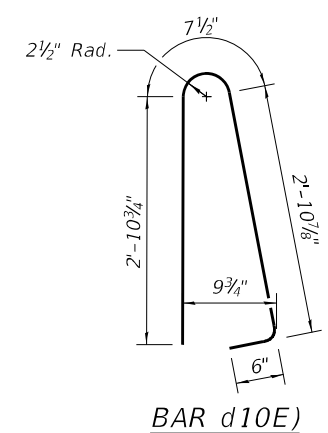
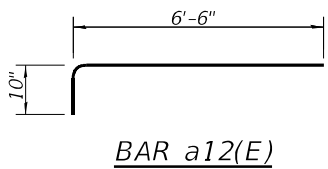
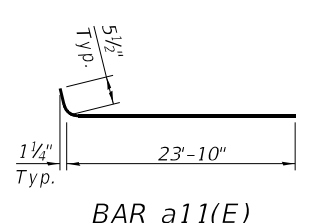
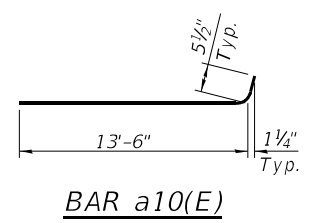


DETAIL A

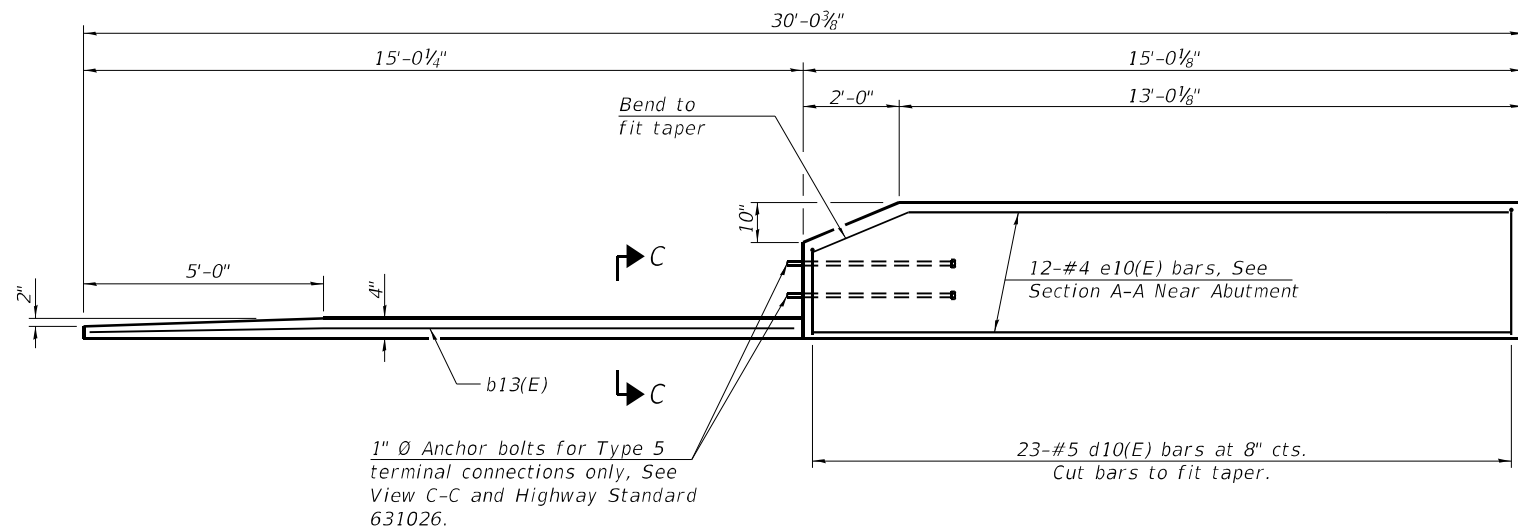
NORTH APPROACH
BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
a10(E)	46	#5	14'-0"	—	
a11(E)	46	#5	24'-4"	—	
a12(E)	46	#5	7'-4"	—	
a13(E)	60	#8	25'-1"	—	
a14(E)	60	#8	13'-6"	—	
b10(E)	52	#5	29'-8"	—	
b11(E)	82	#9	29'-8"	—	
b12(E)	8	#5	14'-8"	—	
b13(E)	2	#4	14'-8"	—	
d10(E)	46	#5	7'-0"	⌋	
d11(E)	46	#5	8'-6"	⌋	
e10(E)	24	#4	14'-8"	—	
t10(E)	70	#4	9'-8"	—	
w10(E)	40	#5	33'-11"	—	
Concrete Superstructure				Cu. Yd.	4.3
Concrete Superstructure (Approach Slab)				Cu. Yd.	49.2
Concrete Structures				Cu. Yd.	10.6
Reinforcement Bars, Epoxy Coated				Pound	21240

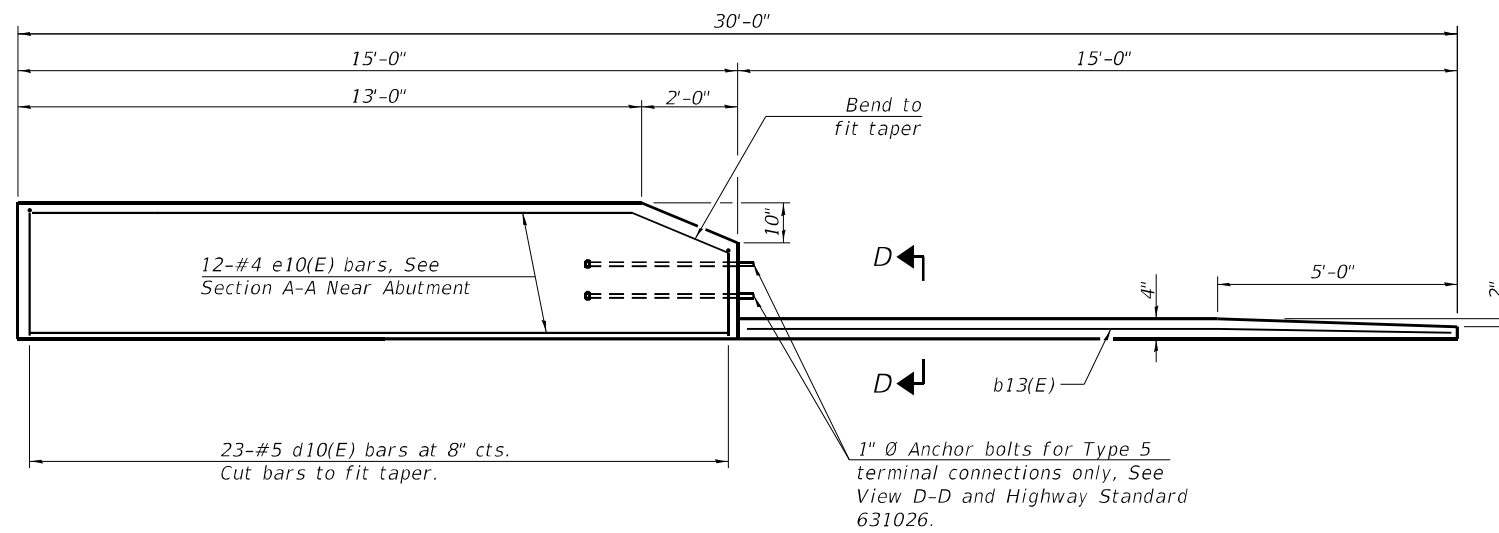
* Prior to Grinding
 ** Cost included with Concrete Superstructure (Approach Slab).
 *** Per manufacturer recommendations
 **** Perpendicular to inside face of curb.



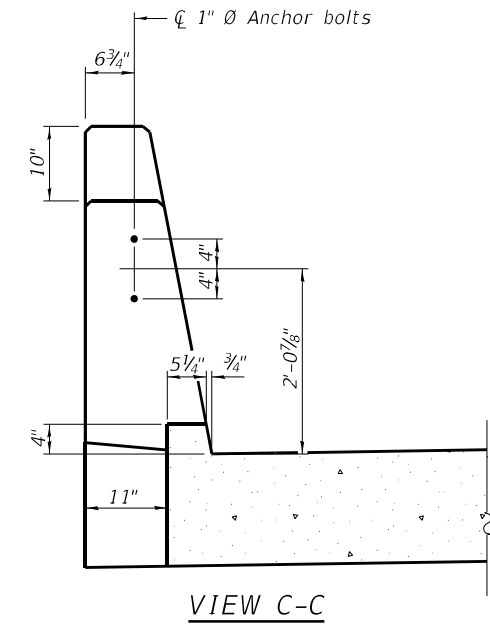
(Sheet 2 of 3)



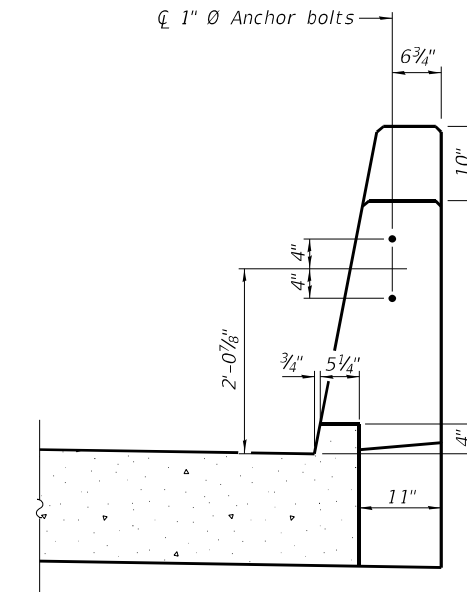
INSIDE ELEVATION OF EAST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)



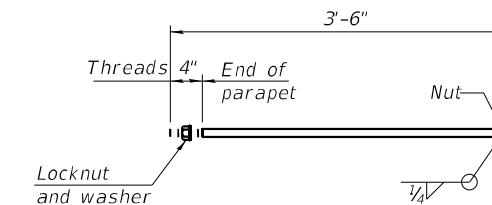
INSIDE ELEVATION OF WEST PARAPET AND CURB
(Dimensions Along Inside Face of Parapet and Curb)



VIEW C-C



VIEW D-D

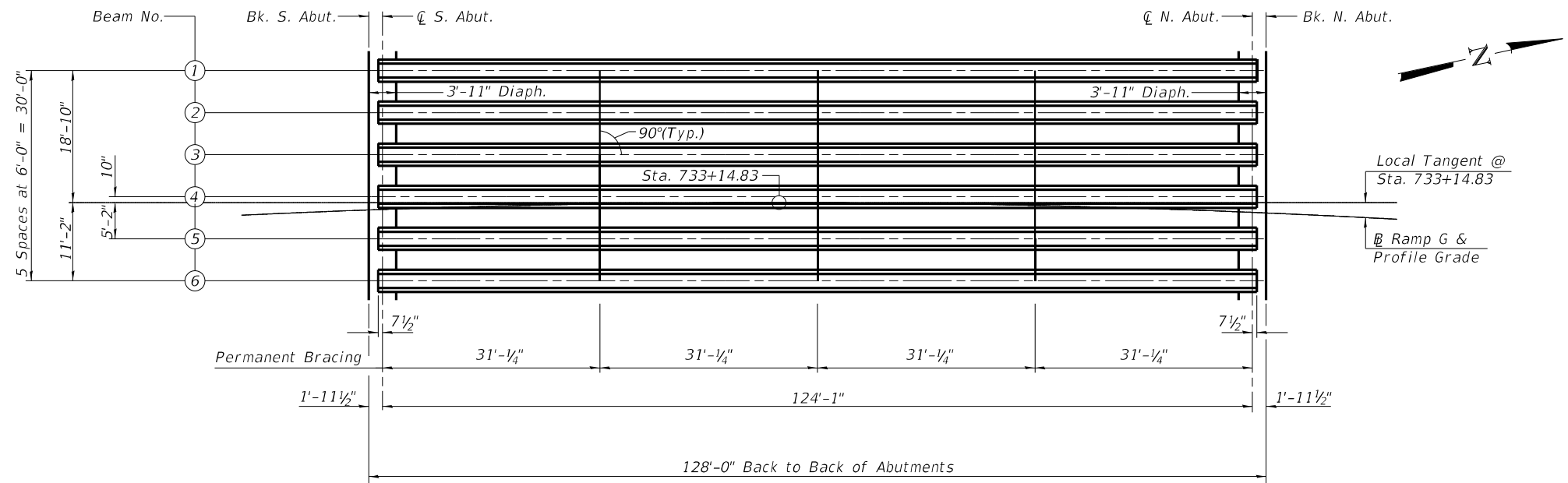


*** 1" Ø ANCHOR BOLT**
(Anchor bolt assemblies shall be galvanized according to Article 1006.09 of the Standard Specifications)

* Cost included with Concrete Superstructure (Approach Slab).

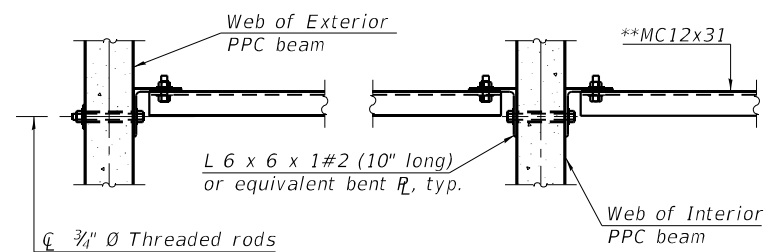
(Sheet 3 of 3)

FILE NAME = 0101003-70899-016-N_Appr Slab Details.dgn	USER NAME =	DESIGNED - FAM	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	NORTH BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 010-1003	F.A.I. RTE. = 57/74	SECTION = (10-34-1)HBK	COUNTY = CHAMPAIGN	TOTAL SHEETS = 1187	SHEET NO. = 794
BFW BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - PS	REVISED -			SHEET NO. 16 OF 30 SHEETS	CONTRACT NO. 70B99			
433 NORTH COLT ST. SUITE 615 CHAMPAIGN, IL 62602-1099 PHONE: 815.987.8100	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -			ILLINOIS FED. AID PROJECT				
		CHECKED - GBR	REVISED -							

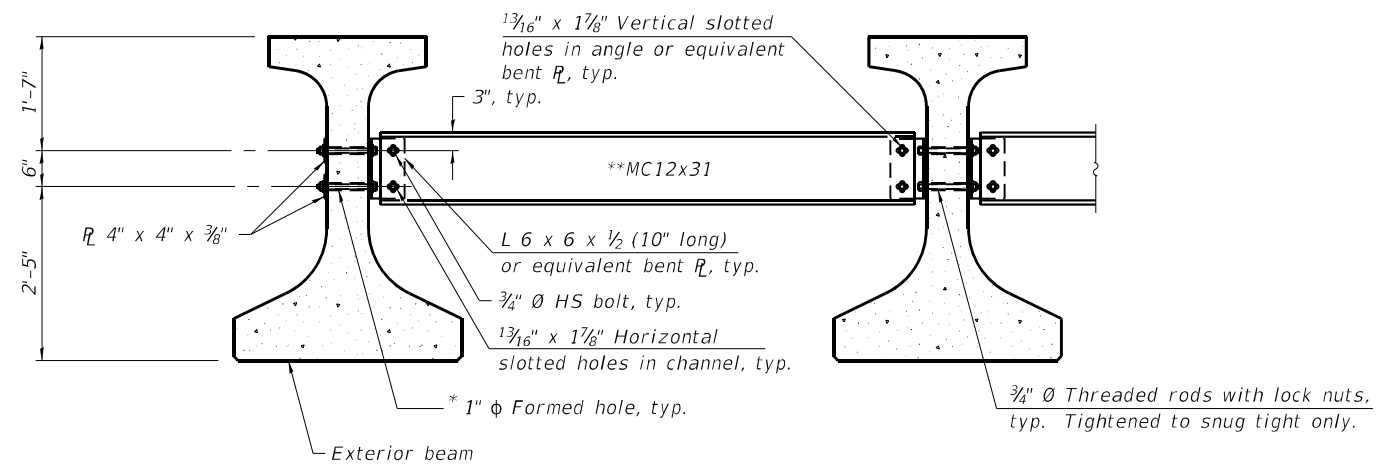


FRAMING PLAN

- I: Non-composite moment of inertia of beam section (in.⁴).
- I': Composite moment of inertia of beam section (in.⁴).
- Sb: Non-composite section modulus for the bottom fiber of the prestressed beam (in.³).
- Sb': Composite section modulus for the bottom fiber of the prestressed beam (in.³).
- St: Non-composite section modulus for the top fiber of the prestressed beam (in.³).
- St': Composite section modulus for the top fiber of the prestressed beam (in.³).
- DC1: Un-factored non-composite dead load (kips/ft.).
- MDC1: Un-factored moment due to non-composite dead load (kip-ft.).
- DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
- MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
- DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
- MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
- M_L + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- LLDF: Live Load Distribution Factor for Moment or Shear.
- OCF: Obtuse Correction Factor



PLAN



ELEVATION

Notes:

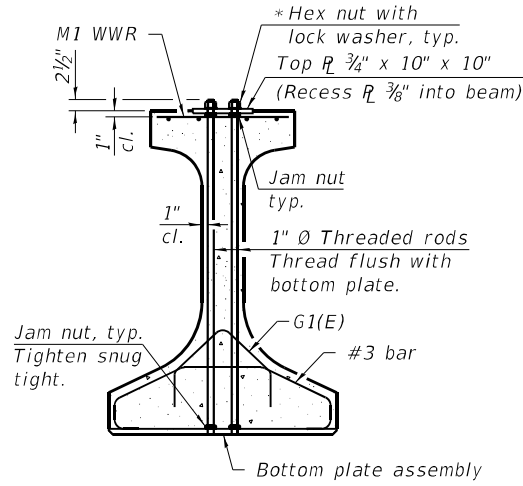
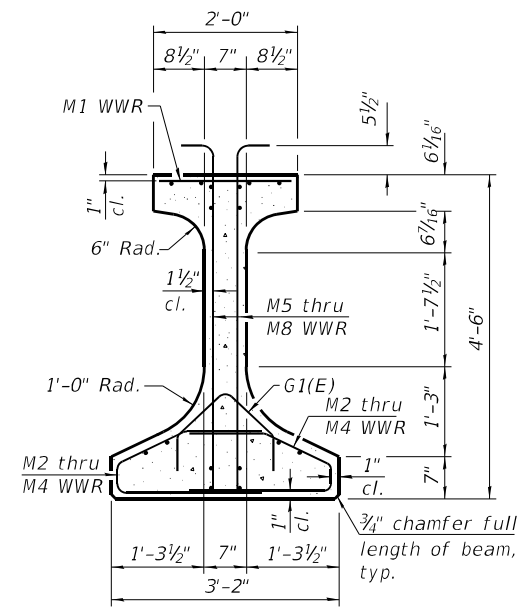
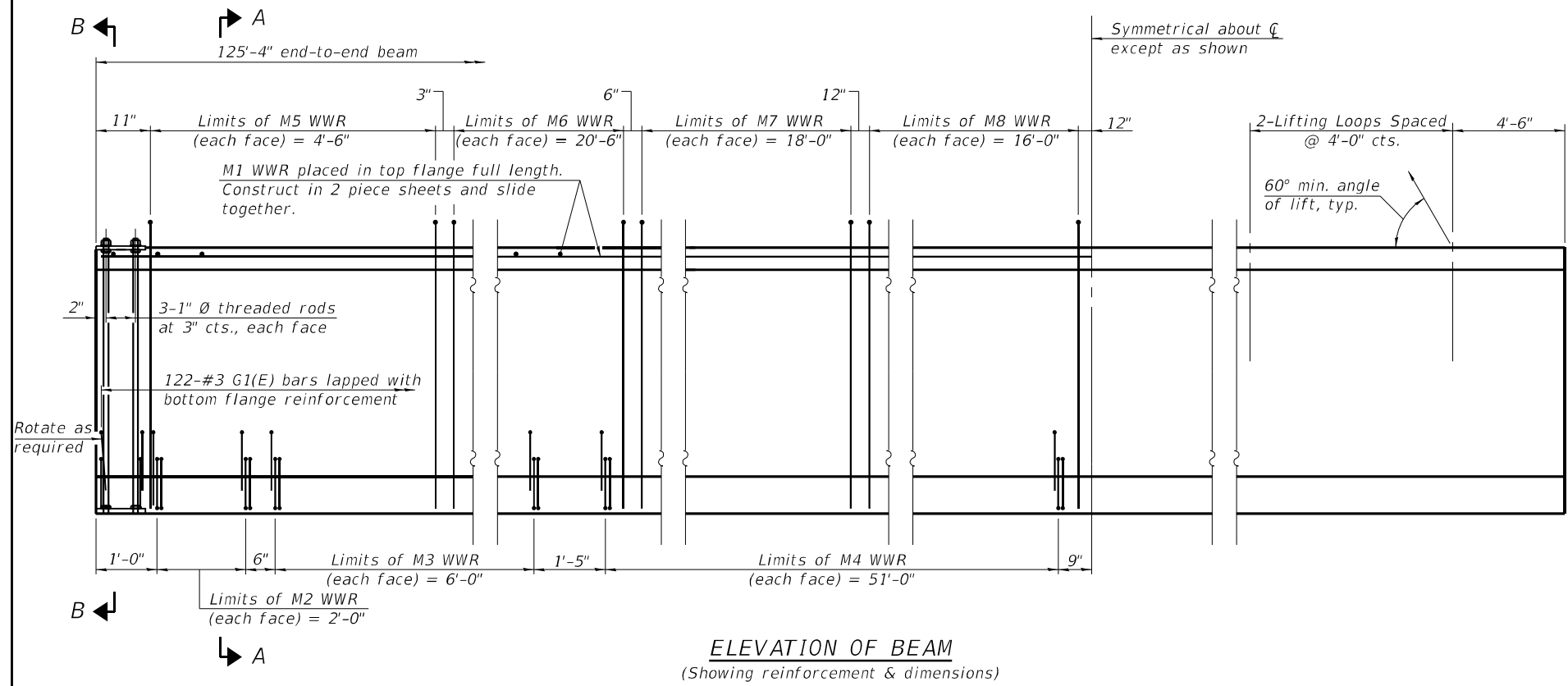
All material for bracing shall be hot dip galvanized according to AASHTO M111 unless otherwise noted.
 Two hardened washers are required for each set of oversized holes.
 All holes shall be 1 5/16" Ø unless otherwise noted.
 5/16" x 3" x 3" plate washers are required over all slotted holes.
 All bolts, threaded rods, and hardware shall be galvanized according to AASHTO M232.
 Threaded rods shall be ASTM F 1554 Grade 55.
 Bracing shall be installed as beams are erected and tightened as soon as possible during erection.
 Permanent bracing shall not be paid for separately, but shall be included in the cost of Furnishing and Erecting Precast Prestressed Concrete Beams.

- * Fabricator shall locate to miss strands within permissible tolerances.
- ** Alternate MC12x35 channels are permitted to facilitate material acquisition.

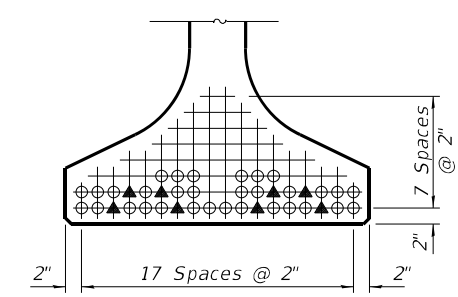
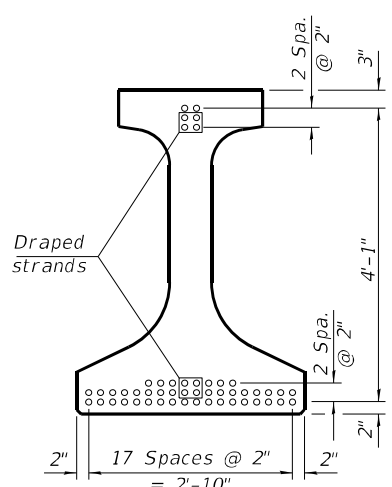
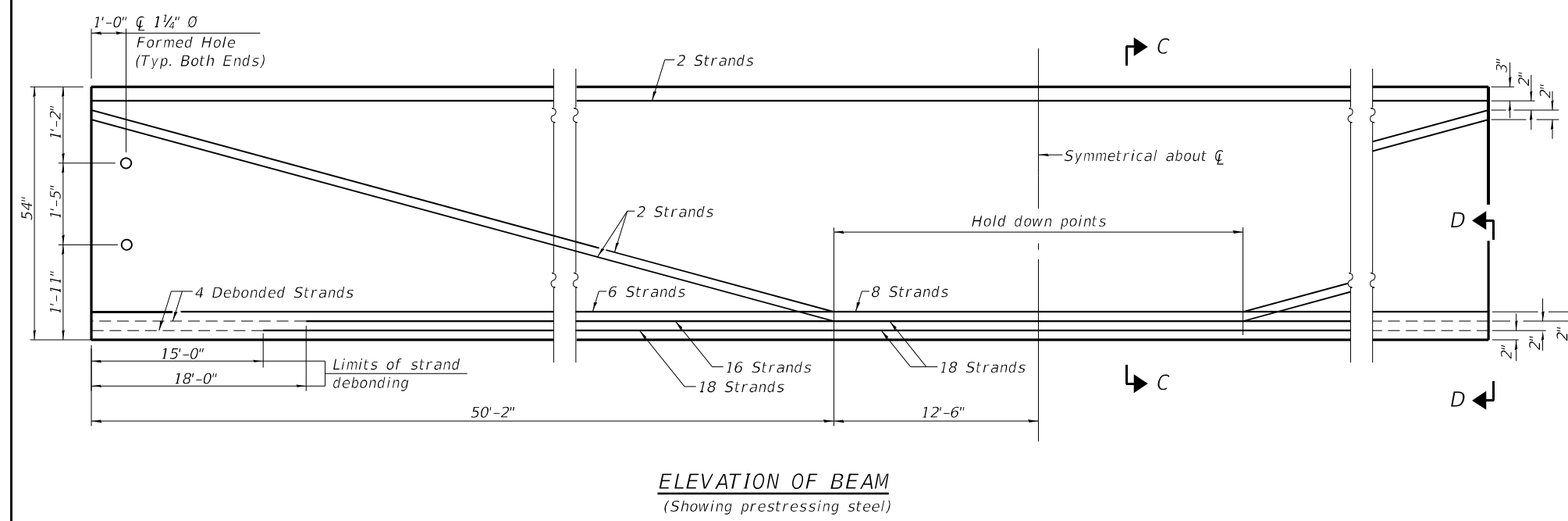
PERMANENT BRACING DETAILS
(15 Required)

INTERIOR BEAM MOMENT TABLE		
0.5 Sp.		
I	(in ⁴)	295,427
I'	(in ⁴)	683,551
Sb	(in ³)	13,551.6
Sb'	(in ³)	19,940.2
St	(in ³)	9,174.7
St'	(in ³)	34,662.8
DC1	(k/ft)	1.559
MDC1	(k)	3,000
DC2	(k/ft)	0.190
MDC2	(k)	365.7
DW	(k/ft)	0.300
MDW	(k)	577.4
LLDF		0.5266
M _L + IM	(k)	2,016.8

INTERIOR BEAM REACTION TABLE		
Abut.		
LLDF		0.671
OCF		1.000
RDC1	(k)	96.7
RDC2	(k)	11.8
RDW	(k)	18.6
R _L	(k)	71.3
R _{IM}	(k)	14.7
RTotal	(k)	213.1

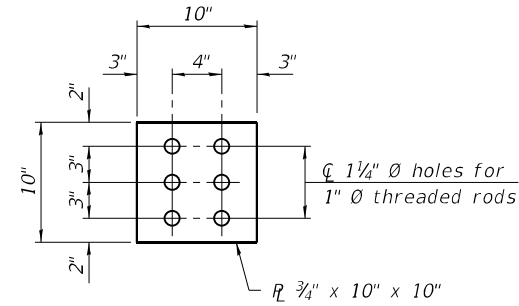


SECTION B-B
* Only tighten sufficiently to compress lock washers

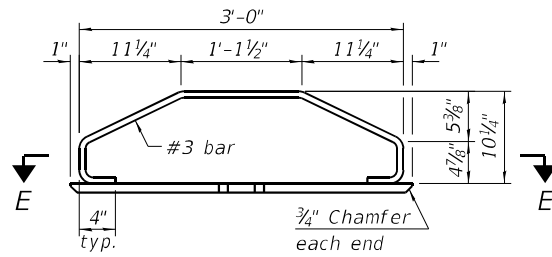


VIEW D-D
○ Fully bonded strand
▲ Partially debonded strand

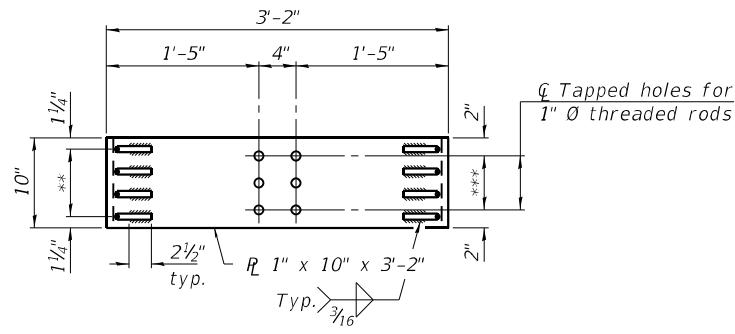
Notes:
See sheet 17 of 30 for number and location of 1" formed holes for Permanent Bracing.
See sheet 19 of 30 for additional details and Bill of Material.



PLAN - TOP PLATE



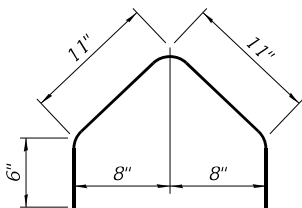
ELEVATION - BOTTOM PLATE ASSEMBLY



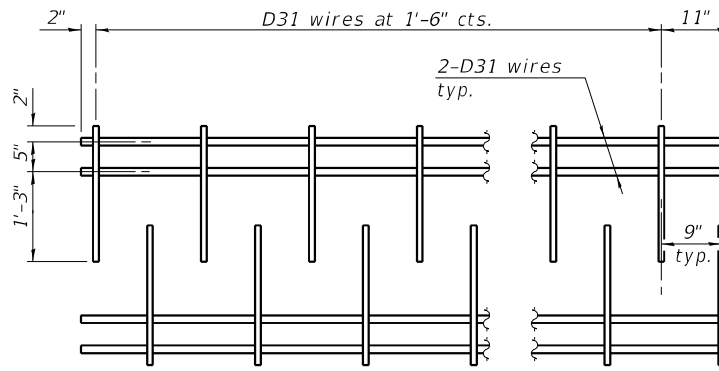
SECTION E-E

** 3 Spaces at 2 1/2" = 7 1/2"

*** 2 Spaces at 3" = 6"

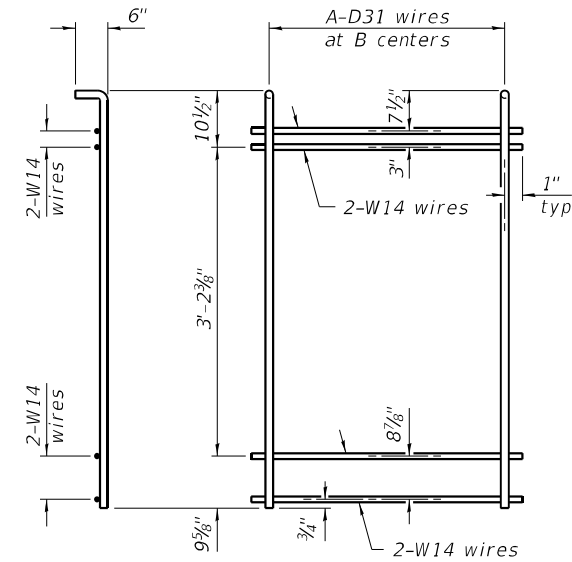


BAR G1(E)



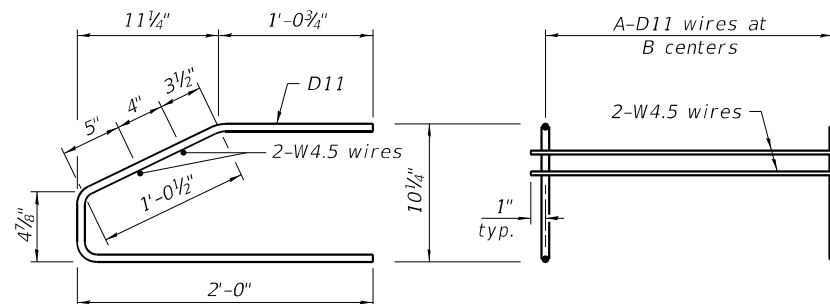
M1 WWR DETAIL

When multiple sheets of M1 WWR are required along the beam length, #5(E) bars (5'-0" long) shall be used to splice the longitudinal D31 wires together (Min. Lap 2'-2").



M5 THRU M8 WWR DETAIL

(See Table of Dimensions)



M2 THRU M4 WWR DETAIL

(See Table of Dimensions)

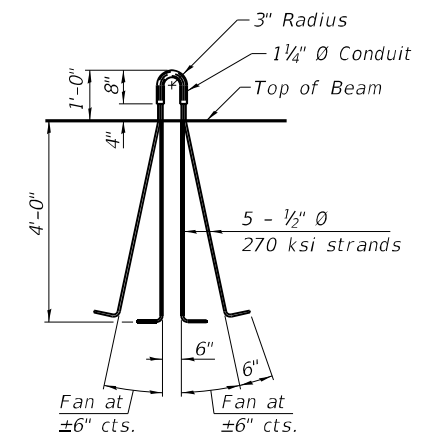
TABLE OF DIMENSIONS

(WWR tables are based on Grade 60.)

WWR	A	B
M2	9	3"
M3	13	6"
M4	35	1'-6"
M5	19	3"
M6	42	6"
M7	19	1'-0"
M8	9	2'-0"

NOTES

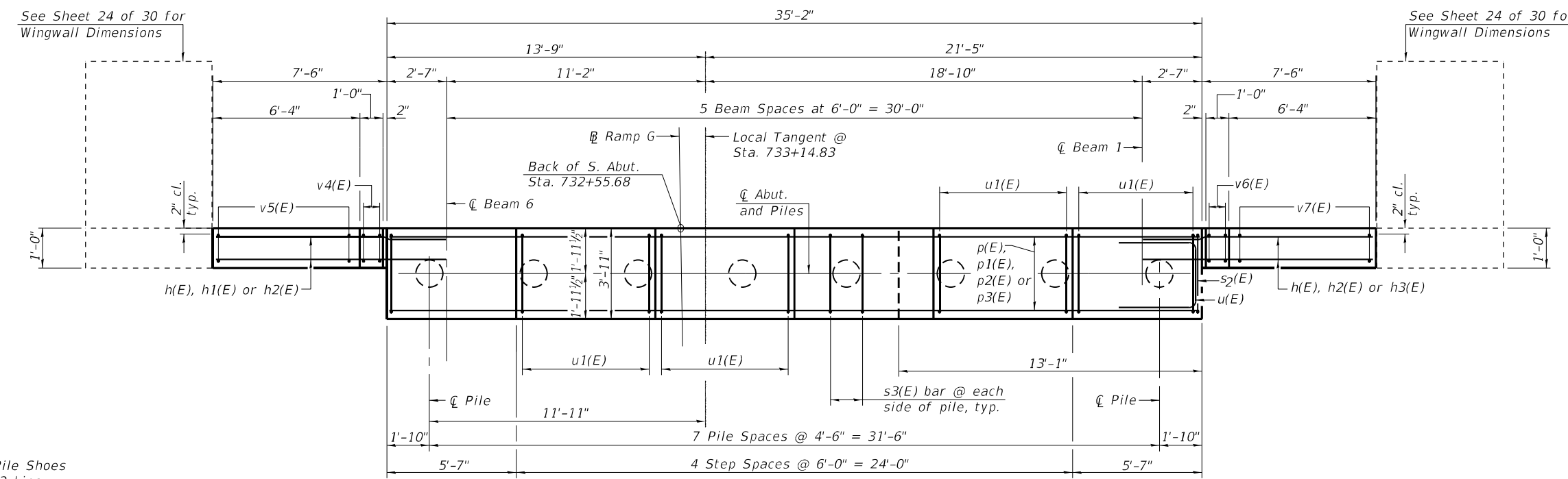
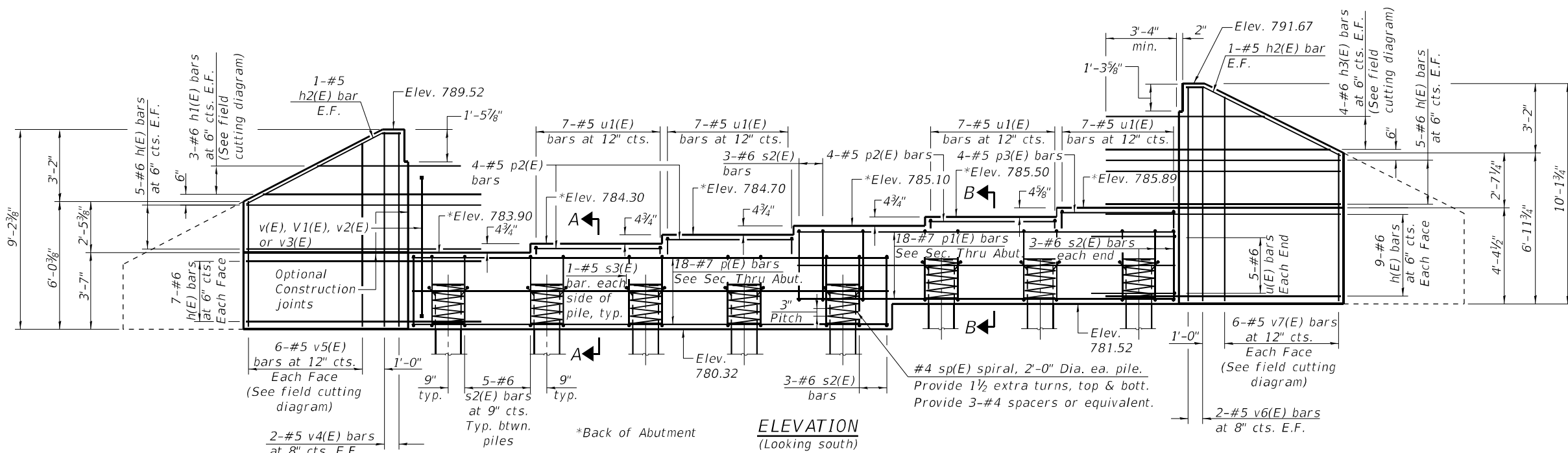
Inserts for 3/4" diameter threaded dowel rods, when specified, are to be two strut, ferrule type for interior beams and single ferrule, flared loop type for exterior beams. Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter for beam strands shall be 0.6" and the nominal cross-sectional area shall be 0.217 sq. in. The nominal diameter for lifting loops shall be 1/2" and the nominal cross sectional area shall be 0.153 sq. in. The beams shall have a final concrete compressive strength, f'c, of 8500 psi and a release concrete compressive strength, f'ci, of 6500 psi. A minimum 2 1/2" diameter lifting pin shall be used to engage the lifting loops during handling. The top and bottom plates shall be AASHTO M270 Grade 50. The top plates and bottom plate assemblies shall be galvanized according to AASHTO M111. The threaded rods, nuts and washers shall be galvanized according to AASHTO M232. Threaded rods shall be ASTM F 1554 Grade 55. Welded Wire Reinforcement (WWR) shall conform to ASTM A884 with a Class A, Type 1 epoxy coating or ASTM A1060, Table 3 galvanized coating.



LIFTING LOOP DETAIL

BILL OF MATERIAL

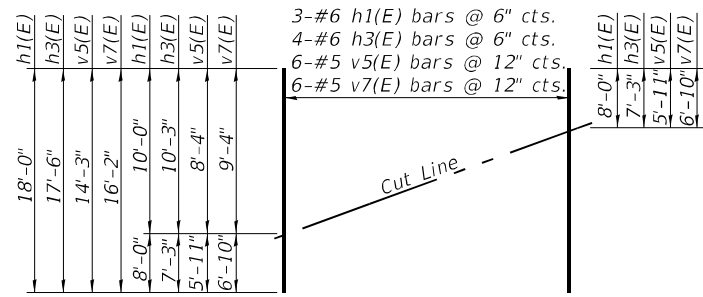
Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete Beams, IL54N	Ft.	752



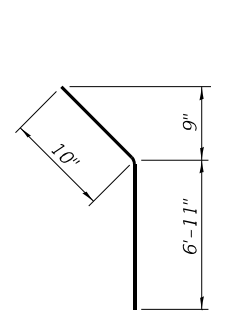
PILE DATA

Type: MS 14" x 0.312" With Pile Shoes
 Nominal Required Bearing: 513 kips
 Factored Resistance Available: 282 kips
 Est. Length: 87'
 No. Production Piles: 7
 No. Test Piles: 1
 No. Pile Shoes: 8

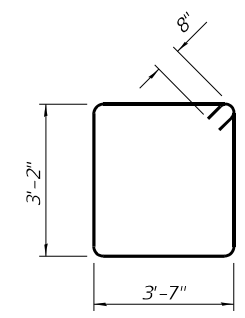
Notes:
 Pour steps monolithically with cap.
 E.F. denotes Each Face.
 For Section A-A and B-B, see Sheet 21 of 30.
 For Bill of Materials, see Sheet 21 of 30.
 For details of piles, see Sheet 25 of 30.
 Headed bars shall conform to ASTM A 970 with threaded attachment: Class HA; and reinforcement bars conforming to ASTM A 706 Coated.
 Cost included with Reinforcement Bars, Epoxy Coated.
 For Layout of v(E), v1(E), v2(E), & v3(E) bars see Sheet 21 of 30.



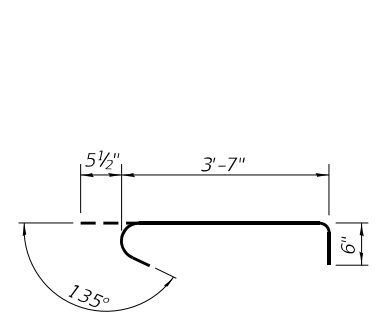
FIELD CUTTING DIAGRAM
 Order h1(E), h3(E), v5(E) and v7(E) full length. Cut as shown and use remainder of bars in opposite face.



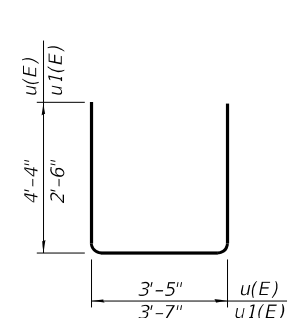
BAR h2(E)



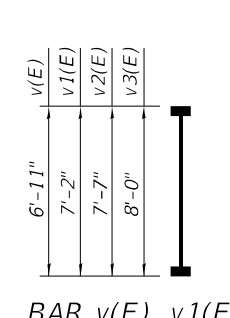
BAR s2(E)



BAR s3(E)



BAR u(E) & u1(E)



BAR v2(E) & v3(E) (Headed)

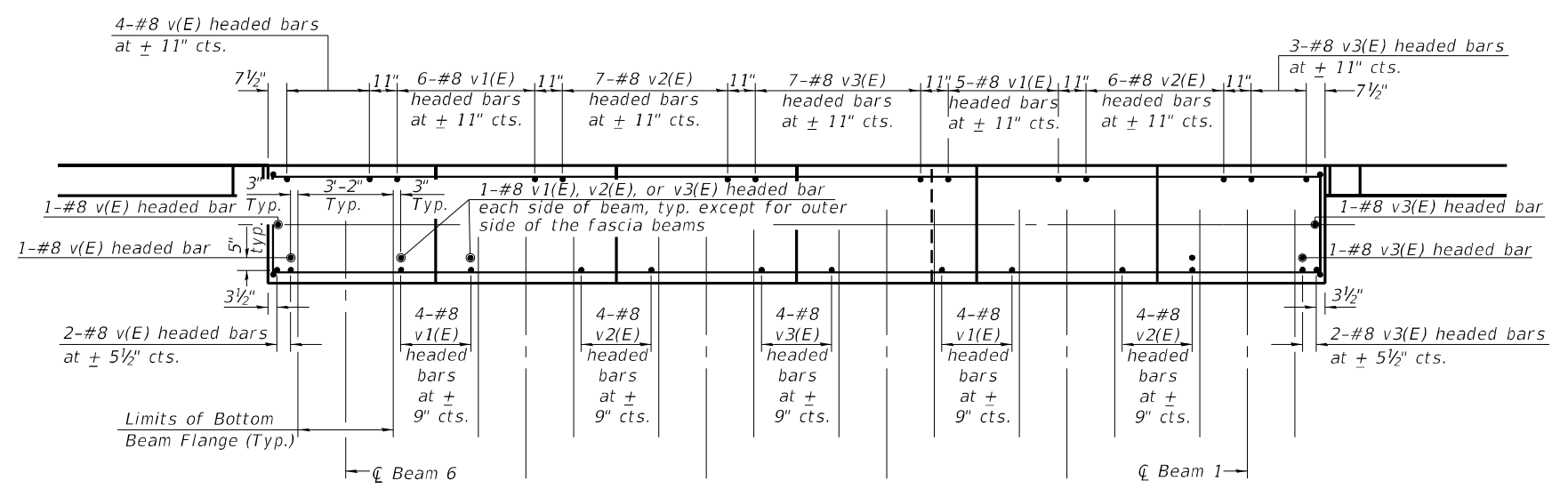
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BACON FARMER WORKMAN ENGINEERING & TESTING, INC.		CHECKED - PS	REVISED -
433 NORTH COLT ST. SUITE 100 MORRIS, IL 61208-0100 PHONE: 815.987.8100	PLOT SCALE =	DRAWN - BJV	REVISED -
	PLOT DATE = 3/11/2021	CHECKED - GBR	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

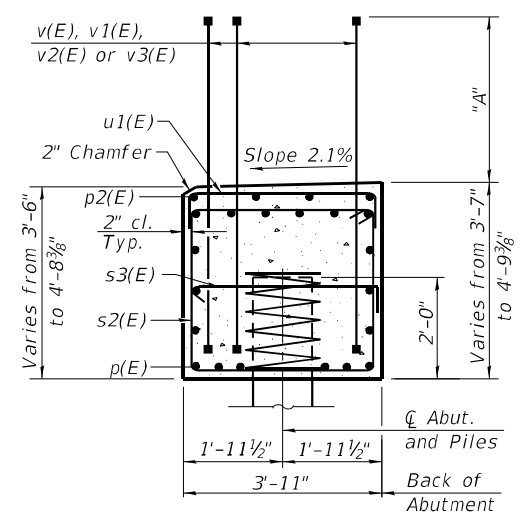
**SOUTH ABUTMENT
 STRUCTURE NO. 010-1003**

SHEET NO. 20 OF 30 SHEETS

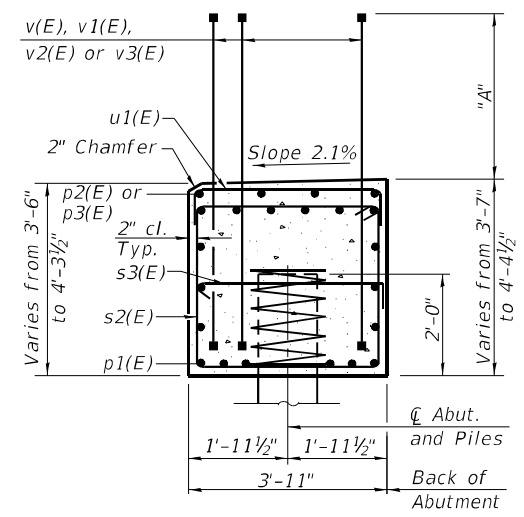
F.A.I. RTE. 57/74	SECTION (10-34-1)HKB	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 798
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	



Layout of v(E), v1(E), v2(E) & v3(E) Bars



SEC. A-A



SEC. B-B

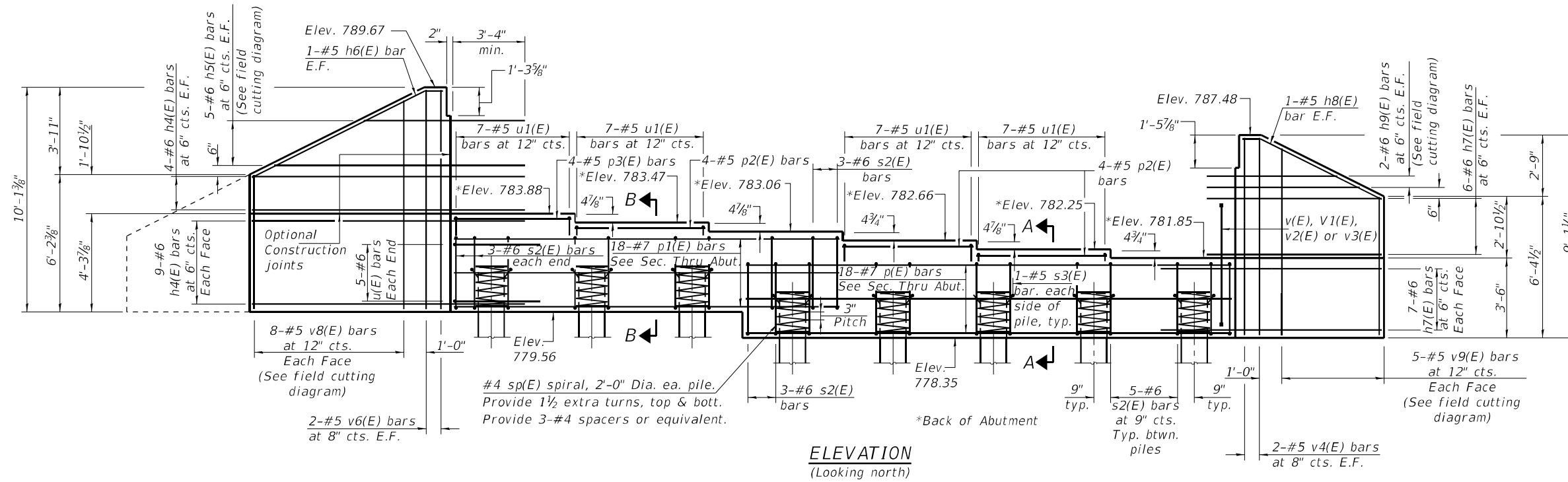
**SOUTH ABUTMENT
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h(E)	52	#6	10'-8"	—
h1(E)	3	#6	18'-0"	—
h2(E)	4	#5	7'-9"	—
h3(E)	4	#6	17'-6"	—
p(E)	18	#7	21'-10"	—
p1(E)	18	#7	17'-4"	—
p2(E)	12	#5	5'-10"	—
p3(E)	4	#5	5'-4"	—
s2(E)	47	#6	14'-10"	□
s3(E)	16	#5	4'-7"	□
sp(E)	8	#4	2'-0"	MM
u(E)	10	#6	12'-1"	—
u1(E)	28	#5	8'-7"	—
v(E)	8	#8	6'-11"	—
v1(E)	23	#8	7'-2"	—
v2(E)	25	#8	7'-7"	—
v3(E)	20	#8	8'-0"	—
v4(E)	4	#5	8'-10"	—
v5(E)	6	#5	14'-3"	—
v6(E)	4	#5	9'-10"	—
v7(E)	6	#5	16'-2"	—
Structure Excavation	Cu. Yd.		109	
Concrete Structures	Cu. Yd.		25.7	
Reinforcement Bars, Epoxy Coated	Pound		6300	
Furnishing Metal Shell Piles, 14" x 0.312"	Foot		609	
Driving Piles	Foot		609	
Test Pile, Metal Shells	Each		1	
Pile Shoes	Each		8	

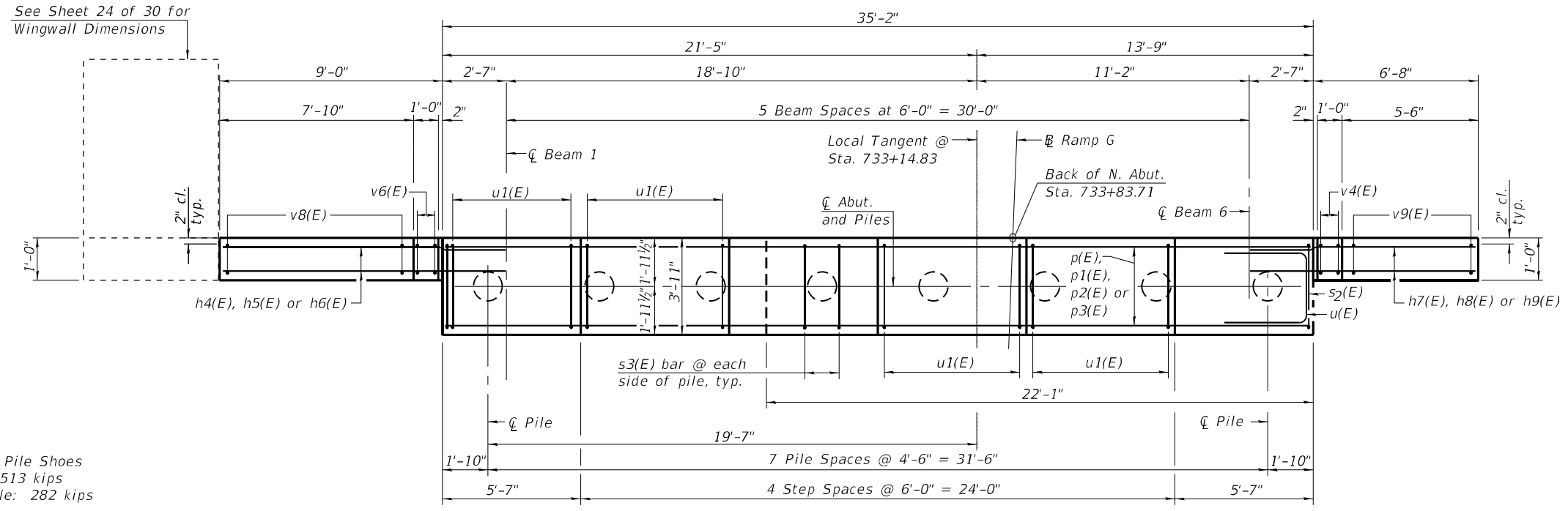
* Length is height of spiral.

**DIMENSIONS OF V-BARS
ABOVE TOP OF ABUTMENT CAP ("A")**

Bar	Minimum Length of "A"	Maximum Length of "A"
v(E)	3'-8"	3'-8"
v1(E)	3'-6 1/8"	3'-11 1/8"
v2(E)	3'-6 1/4"	3'-11 1/4"
v3(E)	3'-6 1/2"	3'-11 1/2"



ELEVATION
(Looking north)

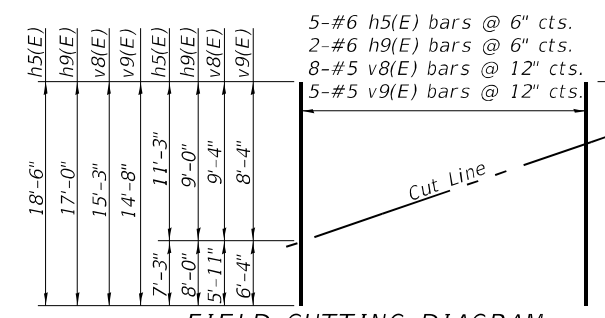


PLAN

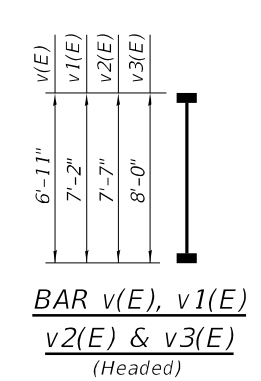
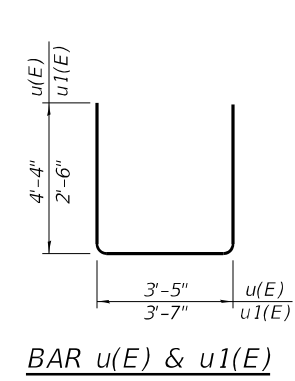
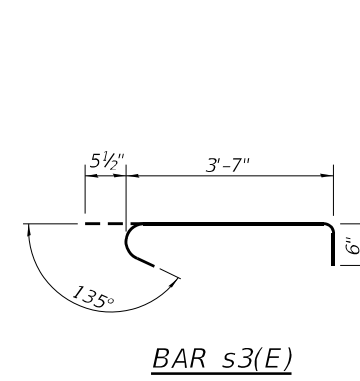
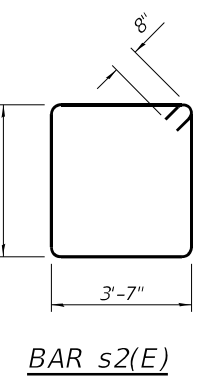
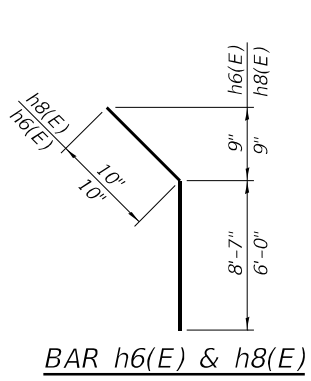
PILE DATA

Type: MS 14" x 0.312" With Pile Shoes
 Nominal Required Bearing: 513 kips
 Factored Resistance Available: 282 kips
 Est. Length: 91'
 No. Production Piles: 7
 No. Test Piles: 1
 No. Pile Shoes: 8

Notes:
 Pour steps monolithically with cap.
 E.F. denotes Each Face.
 For Section A-A and B-B, see Sheet 23 of 30.
 For Bill of Materials, see Sheet 23 of 30.
 For details of piles see Sheet 25 of 30.
 Headed bars shall conform to ASTM A970 with threaded attachment: Class HA; and reinforcement bars conforming to ASTM A706 Cost included with Reinforcement Bars, Epoxy Coated.
 For Layout of v(E), v1(E), v2(E), & v3(E) bars see Sheet 23 of 30.



FIELD CUTTING DIAGRAM
 Order h5(E), h9(E), v8(E) and v9(E) full length. Cut as shown and use remainder of bars in opposite face.



FILE NAME = 0101003-70899-022-North Abutment.dgn	USER NAME =	DESIGNED - FAM	REVISED -
BACON FARMER WORKMAN ENGINEERING & TESTING, INC.	PLOT SCALE =	CHECKED - PS	REVISED -
433 NORTH COLLET STREET MORRIS, IL 62458-0280 PHONE - 618.937.8100	PLOT DATE = 3/11/2021	DRAWN - BJV	REVISED -
		CHECKED - GBR	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT
STRUCTURE NO. 010-1003
 SHEET NO. 22 OF 30 SHEETS

F.A.I. RE. 57/74	SECTION (10-34-1)HBK	COUNTY CHAMPAIGN	TOTAL SHEETS 1187	SHEET NO. 800
CONTRACT NO. 70B99			ILLINOIS FED. AID PROJECT	