DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES: Field Units f'c = 3,500 p.s.i. fy = 60,000 p.s.i. (reinforcement)

Total

Sign Area

-€ Upper Chord

Bottom of

Rase Plate

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer. The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to AASHTO M314 Gr. 105 with a minimum Charpy V-Notch (CVN) energy of 15 lb.-ft. at 10° F.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Bridge Seat Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

FOUNDATIONS: The contract unit price for Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.

OVERHEA	DS	IGN	ST	R
DRILLED	SH,	AFT	CO	N

\\FS-@@44\am\VAULT,D-TRANS_(the ste bas * If pla	Marke, an rop and levening hurs shan be righten base plate with a minimum torque of 200 lbfr el mesh shall then be placed around the perimet se plate. Secure to base plate with stainless sta M270 Gr. 50W (M222) steel is proposed, chemis te to be used shall first be approved by the Eng table for galvanizing and welding.	t. Stainless er of the eel banding. try for	OVERHEAD SIGN STRU
ERØLDGN 27 BAJZEKKJ	OSC-A-1	7-1-10						DRILLED SHAFT CONC
3:28:	FILE NAME =	ÚSER NAME = \$USER\$	DESIGNED - AMB	REVISED -		STATE OF ILLINOIS		CANTILEVER SIGN STRUC
145-E	\$FILEL\$		DRAWN - AMB	REVISED -	DEI	PARTMENT OF TRANSPORTATION		GENERAL PLAN & ELEVA
60H 2-20	TENG & ASSOCIATES, INC.	- PLOT SCALE = \$SCALE\$	CHECKED - MDB	REVISED -		FAP ROUTE 345 / US ROUTE 20		ALUMINUM TRUSS & STEI
67	TENG & ASSOCIATES, INC. ENGINEERS/ARCHITECTS/PLANNER CHICAGO, ILLINOIS	S PLOT DATE = \$DATE\$	DATE - 12/16/11	REVISED -			SCALE: N.T.S.	SHEET NO. SS-1 OF SS-14 STA.

Design

Truss

Туре

Station

83+50.00

113+23.91

- C - A

- C - A

II-C-A

Structure

Number

1C045U020R015.1

1C045U020L015.7

IC045U020L015.8

1C045U020L016.0

Cantilever

(L)

russ Type Maximum Sign Area Maximum Length

170 Sq. Ft.

340 Sq. Ft.

400 Sa. Ft.

Maximum Length (See Table)

DESIGN WIND LOADING DIAGRAM

Installations not within dimensional limits shown

require special analysis for all components.

Note:

(1)

of the trusses.

Parameters shown are basis for I.D.O.T. Standards

30 p.s.f. on

Maximum Sign Area

(See Table)

Length Elev. A

122+24.46 III-C-A 36'-0" 822.47 20.75' 7'-0" 129.5 Sq. Ft.

135+24.28 III-C-A 33'-O" 797.32 19.75' 8'-6" 123.25 Sq. Ft

25 Ft.

30 Ft.

40 Ft.

10 p.s.f.

TIANA TANA

Trusses shall be shipped individually with adequate provision

to prevent detrimental motion during transport. This may

require ropes between horizontals and diagonals or energy

dissipating (elastic) ties to the vehicle. The contractor is

responsible for maintaining the configuration and protection

After adjustments to level truss and insure adequate vertical plearance all top and leveling puts shall be tightened against

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Dim. D

 III-C-A
 32'-0"
 824.38
 14.72'
 4'-0"
 74.0 Sq. Ft.

 III-C-A
 40'-0"
 825.72
 20.68'
 7'-0"
 119.0 Sq. Ft.

Ds

Alternate Direction of Horizontal Diagonal Bracing for Each Bay in Planes of Upper and Lower Chords Bracing, typ.	Γ
Lower Chord Bracing, typ.	
<u>TYPICAL PLAN</u> (Walkway not shown)	
Sign Panel <u>Alternate Vertical Diagonal Bracing for Each</u> Bay in Planes of Front and Back Chords	
Depth of Sign (D _s	
Walkway, railing and lights (if required) omitted for clarity	
<pre>// Minimu</pre>	-
Elev. A (Location varies)	D
Elev. A = Elevation at point of minimum clearance to sign, walkway support or truss. <u>TYPICAL ELEVATION</u>	

Looking in Direction of Traffic

Sign support structures may be subject to damaging vibrations and oscillations when sign panels are not in place during erection or maintenance of the structure. To avoid these vibrations and oscillations, consideration should be given to attaching temporary blank sign panels to the structure.

GENERAL NOTES

TOTAL BILL OF MATERIAL

CTUBES F.A.P		TOTAL SH
VCRETE FOUNDATIONS	Cu. Yd.	44.8
COUTONE CANTILEVEN THE III C A	1001	141.0
RUCTURE CANTILEVER TYPE III-C-A	Foot	141.0
ITEM	UNIT	TOTAL

CTURES		F.A.P RTE.	SEC	TION	COUNT	Y TOTAL SHEETS	SHEET NO.
ATION		345	8R	-R	KANE	794	358
EEL POST					CONTR	ACT NO.	60H45
Ά.	TO STA.	FED. RO	AD DIST. NO.	ILLINOIS FED.	AID PROJECT		