FOR INDEX OF SHEETS, SEE SHEET NO. 2

FOR LIST OF HIGHWAY STANDARDS, SEE SHEET NO. 2

STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

PROPOSED HIGHWAY PLANS

FAP ROUTE 10 (IL RTE 267)
SECTION 3BR-1
PROJECT STP-BC9H(360)
BRIDGE DECK REPLACEMENT
GREENE COUNTY

C-98-277-18

DESIGN DESIGNATION
FAP ROUTE 10 (IL RT 267)
FEDERAL AIDE
ADT 2,050 (2019)
77.9% PV
8.3% SU
13.8% MU

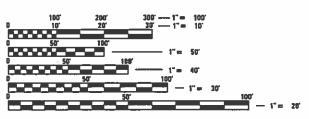
DESIGN SPEED: 55 MPH

0

0

0

STA. 385 + 40.00
SN 031-0016
IL RT 267 OVER LICK CREEK
194'-5" BK TO BK ABUTMENTS
32"-6" 0-0
IMPROVEMENT INCLUDES
DECK REPLACEMENT

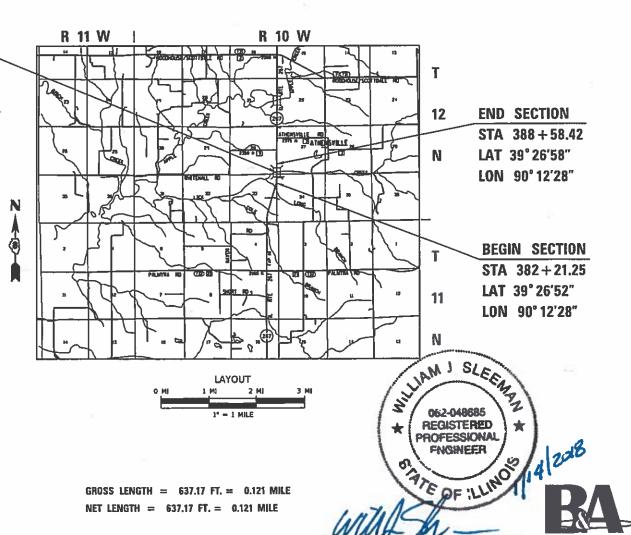


FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123
OR 811

PROJECT ENGINEER: TIM PADGETT (618) 346-3325 PROJECT MANAGER: PHIL FREIMUTH (618) 346-3194

CONTRACT NO. 76K57



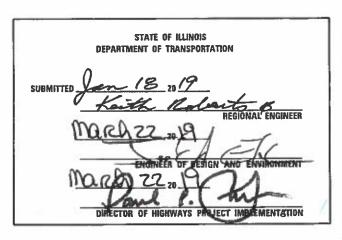
RTE. SECTION COUNTY TOTAL SHEET NO.

30 38R-1 GREENE 35 1

RILMOIS CONTRACT NO. 76K57

D-98-086-17





PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

INDEX OF SHEETS

COVER SHEET

2 INDEX OF SHEETS, GENERAL NOTES & HIGHWAY STANDARDS

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11 SCHEDULE OF QUANTITIES

12 ALIGNMENT, TIES & BENCHMARKS

13-14 PLAN AND PROFILE IL ROUTE 267
15 WIDE LOAD SIGNING PLAN

16-35 WIDE LOAD SIGNING PL 16-35 STRUCTURAL SHEETS

GENERAL NOTES

- 1. ALL ELEVATIONS SHOWN ON THE PLANS ARE BASED ON NAVD 88 DATUM.
- 2. ILLINOIS STATE LAW REQUIRES A 48-HOUR NOTICE BE GIVEN TO ALL UTILITIES WITHIN THE PROJECT AREA BEFORE DIGGING. FIELD MARKING OF FACILITIES MAY BE OBTAINED BY CONTACTING J.U.L.I.E. OR FOR NON-MEMBERS, THE UTILITY COMPANY DIRECTLY. AGENCIES KNOWN TO HAVE FACILITIES WITHIN THE PROJECT AREA ARE AS FOLLOWS:

*FRONTIER COMMUNICATIONS - COMMUNICATIONS (AERIAL & BURIED)

*GREENE COUNTY RURAL WATER DISTRICT - POTABLE WATER (BURIED)

MEMBERS OF J.U.L.I.E. CALL TOLL FREE (800) 892-0123 OR 811 AND ARE INDICATED BY AN *. NON-J.U.L.I.E. MEMBERS MUST BE NOTIFIED INDIVIDUALLY.

- 3. THE THICKNESS OF THE HOT-MIX ASPHALT MIXTURE SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE BITUMINOUS MIXTURE IS PLACED.
- 4. PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING PLANS ARE SUBJECT TO VARIATIONS FOUND IN THE FIELD. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING OF MATERIALS. ANY ADJUSTMENTS PROPOSED BY THE CONTRACTOR MUST BE APPROVED BY THE ENGINEER. SUCH VARIATIONS SHALL NOT BE CAUSE FOR ADDITIONAL COMPENSATION FOR A CHANGE IN THE SCOPE OF WORK, HOWEVER, THE CONTRACTOR WILL BE PAID FOR THE QUANTITY ACTUALLY FURNISHED BASED UPON THE UNIT PRICES.
- 5. EXCEPT WHERE DESIGNATED OTHERWISE, THE LOCATIONS AND/OR DEPTHS OF UNDERGROUND UTILITIES SHOWN HAVE BEEN TAKEN FROM OFFICE RECORD INFORMATION FURNISHED BY THE UTILITY OWNERS AND MUST BE CONSIDERED APPROXIMATE.
- 6. A QUANTITY OF 925 FEET OF TEMPORARY PAVEMENT MARKING LINE 6" YELLOW HAS BEEN INCLUDED IN THE PLANS FOR PAINTING THE BOTTOM VERTICAL 6" OF THE TEMPORARY CONCRETE BARRIER ON BOTH SIDES.
- 7. TWO CHANGEABLE MESSAGE SIGNS SHALL BE PROVIDED ALONG IL ROUTE 267 TO ALERT DRIVERS OF THE ROAD CONSTRUCTION. CHANGEABLE MESSAGE SIGNS SHALL BE PLACED 14 DAYS PRIOR TO BEGINNING ANY WORK. EXACT LOCATIONS OF THE CHANGEABLE MESSAGE SIGN SHALL BE DETERMINED BY THE ENGINEER.
- 8. THE INSTALLATION OF TEMPORARY BRIDGE SIGNALS, STOP BARS, TEMPORARY RUMBLE STRIPS, ADVANCE CONSTRUCTION AHEAD SIGNAGE, ETC. AS SHOWN ON HIGHWAY STANDARD 701321 SHALL BE INSTALLED ONCE TO ACCOMMODATE BOTH STAGE I AND II CONSTRUCTION.

HIGHWAY STANDARDS

000001-07	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
001001-02	AREAS OF REINFORCEMENT BARS
001006	DECIMAL OF AN INCH AND OF A FOOT
280001-07	TEMPORARY EROSION CONTROL SYSTEMS
442101-09	CLASS B PATCHES
515001-03	NAME PLATE FOR BRIDGES
630001-12	STEEL PLATE BEAM GUARDRAIL
630301-09	SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
631032-09	TRAFFIC BARRIER TERMINAL, TYPE 6A
642006	SHOULDER RUMBLE STRIPS, 8 IN.
701001-02	OFF-RD OPERATIONS, 2L, 2W, MORE THAN 15' AWAY
701006-05	OFF-RD OPERATIONS, 2L, 2W, 15' TO 24" FROM PAVEMENT EDGE
701201-05	LANE CLOSURE, 2L, 2W, DAY ONLY FOR SPEEDS ≥ 45 MPH
701301-04	LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
701321-17	LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
701326-04	LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING FOR SPEEDS ≥ 45 MPH
701901-08	TRAFFIC CONTROL DEVICES
704001-08	TEMPORARY CONCRETE BARRIER
725001-01	OBJECT AND TERMINAL MARKERS
780001-05	TYPICAL PAVEMENT MARKINGS
782006	GUARDRAIL AND BARRIER WALL REFLECTOR MOUNTING DETAILS

COMMITMENTS:

COMMITMENTS FOR THIS PROJECT ARE AS FOLLOWS:

NONE

MIXTURE REQUIREMENTS

ROUTE	FAP 10 (IL 267)
SECTION	3BR - 1
COUNTY	GREENE
CONTRACT	76K57

DESCRIPTION:	DECK	REPLACEMENT	-	ΙL	267	OVER	LICK	CREEK	S	OF	СН	9 -	SN	031-	0016	

ADT (CONST YR):	2000
MU%:	13
SU%	8
20 YR ESAL S	1.59

MIXTURE USE	SHOULDERS ≥ 2.25"	SHOULDERS ≤ 2.25"
AC/PG	PG 64-22	PG 64-22
RAP % (MAX)	SEE SPECIAL PROVISION	SEE SPECIAL PROVISION
DESIGN AIR VOIDS	4.0% @ Ndes=30	4.0% @ Ndes=30
MIX COMPOSITION	IL 19.0L	IL 9.5L
(GRADATION)		
FRICTION AGG		
QUALITY MGMT	QC/QA	QC/QA
PROGRAM		

FILE NAME =	USER NAME = mmcevers	DESIGNED	-	W. SLEEMAN	REVISED	-
P.\10E2166-29\FAP 10 IL 267 3BR-1 Lick Creek 76K57\C	NDD Sheets\D876K57-sht-hwystd gennotes.dgn	DRAWN	-	W. SLEEMAN	REVISED	-
	PLOT SCALE = 100.0000 '/in.	CHECKED	-	M. McEVERS	REVISED	-
Default	PLOT DATE = 1/11/2019	DATE	-	1/2019	REVISED	-

	INDEX OF SHEETS, GENERAL NOTES & HIGHWAY STANDARDS				F.A.P. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.	
					10	3B	R-1		GREENE	35	2	
	& HIGHWAT STANDARDS								CONTRAC	T NO. 76	K57	
	SHEET 1	OF 1	SHEETS	STA.	TO STA.			ILLINOIS	FED. A	D PROJECT		

			80% FED	CONSTRUC	TION CODE
			20% STATE		
CODE	ITEM	UNIT	TOTAL	BRIDGE	
NO.	ITEM	UNII	QUANTITY	0013 SN 031-0016	
20200100	EARTH EXCAVATION	CU YD	50	50	
28100109	STONE RIPRAP, CLASS A5	SQ YD	895	895	
28200200	FILTER FABRIC	SQ YD	915	915	
40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	157	157	
44000100	PAVEMENT REMOVAL	SQ YD	31	31	
44004250	DAVED CHOULDED DEMOVAL	60. VD	212	212	
44004250	PAVED SHOULDER REMOVAL	SQ YD	212	212	
44213200	SAW CUTS	FOOT	141	141	
44213200	JAW CU13	1001	141	141	
44213208	TIE BARS $1\frac{1}{4}$ "	EACH	40	40	
	/4				
48101200	AGGREGATE SHOULDERS, TYPE B	TON	61	61	
48102100	AGGREGATE WEDGE SHOULDERS, TYPE B	TON	5	5	
48203037	HOT-MIX SHOULDERS, 10"	SQ YD	316	316	
50102400	CONCRETE REMOVAL	CU YD	6.7	6.7	
50104720	REMOVAL OF EXISTING CONCRETE DECK	EACH	1	1	
50200100	STRUCTURE EXCAVATION	CU YD	7.0	7.0	

EL: Derault NAME: P:\10E2166-29\FAP 10 IL 267 3BR-1 LIck Creek 76K57\CADD Sheets\D8

	DESIGNED -	W. SLEEMAN	REVISED - 1/29/2019
	DRAWN -	W. SLEEMAN	REVISED -
PLOT SCALE = 100.0000 ' / in.	CHECKED -	M. McEVERS	REVISED -
PLOT DATE = 1/29/2019	DATE -	1/2019	REVISED -

STATE OF	ILLINOIS
DEPARTMENT OF 7	TRANSPORTATION

SUMMARY OF QUANTITIES	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS		
	10	3BR-1	GREENE	35	3	
			CONTRACT	NO. 76	5K57	
SHEET 1 OF 6 SHEETS STA. TO STA.		ILLINOIS FED. A	ID PROJECT			

			80% FED	CONSTRUC	CTION CODE
			80% FED 20% STATE		
CODE			TOTAL	BR IDGE	
NO.	ITEM	UNIT	QUANTITY	0013	
140.			QUANTITI	SN 031-0016	
50300225	CONCRETE STRUCTURES	CU YD	3.7	3.7	
50300255	CONCRETE SUPERSTRUCTURE	CU YD	175	175	
50300260	BRIDGE DECK GROOVING	SQ YD	470	470	
50300300	PROTECTIVE COAT	SQ YD	694	694	
50500405	FURNISHING AND ERECTING STRUCTURAL STEEL	POUND	4280	4280	
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	44,370	44,370	
50800515	BAR SPLICERS	EACH	565	565	
50901050	STEEL RAILING, TYPE SM	FOOT	389	389	
			_	_	
51500100	NAME PLATES	EACH	1	1	
52000110	DDEFORMED LOUNT CTD ID CEAL	FOOT	C.F.	C.F.	
52000110	PREFORMED JOINT STRIP SEAL	FOOT	65	65	
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	6	6	
32100010	ELASTOMENTO BEANTING ASSEMBLT, TITE I	Literi			
52100020	ELASTOMERIC BEARING ASSEMBLY, TYPE II	EACH	6	6	
	·				
52100505	ANCHOR BOLTS 5/8"	EACH	24	24	
52200010	TEMPORARY SHEET PILING	SQ FT	103	103	
		1	I		

DRAWN - W. SLEEMAN REVISED - PLOT SCALE = 100.0000 ' / in. CHECKED - M. McEVERS REVISED - PLOT DATE = 1/29/2019 DATE - 1/2019 REVISED -	USER NAME = mmcevers	DESIGNED	-	W. SLEEMAN	REVISED -	1/29/2019	
		DRAWN	-	W. SLEEMAN	REVISED -		
PLOT DATE = 1/29/2019 DATE - 1/2019 REVISED -	PLOT SCALE = 100.0000 ' / in.	CHECKED	-	M. McEVERS	REVISED -		
· · · · · · · · · · · · · · · · · · ·	PLOT DATE = 1/29/2019	DATE	-	1/2019	REVISED -		

	S	UMI	MAR	Y OF Q	UANTIT	ES	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
							10	3BR-1	GREENE	35	4
									CONTRACT	NO. 7	6K57
SHEET	2	OF	6	SHEETS	STA.	TO STA.		ILLINOIS FE	ED. AID PROJECT		

				000/ EED	CONSTRUC	CTION CODE
				80% FED 20% STATE		
ſ	CODE				BRIDGE	
	CODE	ITEM	UNIT	TOTAL	0013	
	NO.			QUANTITY	SN 031-0016	
	58600101	GRANULAR BACKFILL FOR STRUCTURES	CU YD	7.0	7.0	
-						
	58700300	CONCRETE SEALER	SQ FT	72	72	
ماه						
*	63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	FOOT	250.0	250.0	
*	63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	4	4	
				,	·	
*	63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	4	4	
-	63200310	GUARDRAIL REMOVAL	FOOT	570	570	
-	64200108	SHOULDER RUMBLE STRIPS, 8 INCH	FOOT	544	544	
-	67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	6	6	
	67100100	MOBILIZATION	L SUM	1	1	
_						
_	70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321	EACH	1	1	
_	70100450	TRAFFIC CONTROL AND PROTECTION, STANDARD 701201	L SUM	1	1	
-	.0100430	CONTINUE THAT ESTIMA, STANDARD TOTZOT	2 300	<u>'</u>	'	
	70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD 701326	L SUM	1	1	
-						1
	70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	2	2	
	70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1	1	
	CDECIALT					

* SPECIALTY ITEM

USER NAME = mmcevers DESIGNED -	W. SLEEMAN	REVISED - 1/29/2019
DRAWN -	W. SLEEMAN	REVISED -
PLOT SCALE = 100.0000 ' / in. CHECKED -	M. McEVERS	REVISED -
PLOT DATE = 1/29/2019 DATE -	1/2019	REVISED -

STATE OF	ILLINOIS
DEPARTMENT OF	TRANSPORTATION

		S	UMN	ΛAR	Y OF Q	UANTITI	ES	F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
								10	3BR-1		GREENE	35	5
											CONTRACT	NO. 76	6K57
SCALE:	SHEET	3	OF	6	SHEETS	STA.	TO STA.		ILLINOIS	FED. AI	D PROJECT		

			000/ 555	CONSTRUC	CTION CODE
			80% FED 20% STATE		
CODE			TOTAL	BRIDGE	
NO.	ITEM	UNIT	QUANTITY	0013	
110.			QUANTITI	SN 031-0016	
70106700	TEMPORARY RUMBLE STRIPS	EACH	6	6	
70107006	PAVEMENT MARKING BLACKOUT TAPE, 6"	FOOT	1089	1089	
70107025	CHANGEABLE MESSAGE SIGN	CAL DA	28	28	
70107023	CHANGEABLE MESSAGE STON	CAL DA	28	28	
70300100	SHORT TERM PAVEMENT MARKING	FOOT	24	24	
70300150	SHORT TERM PAVEMENT MARKING REMOVAL	SQ FT	553	553	
70300220	TEMPORARY PAVEMENT MARKING - LINE 4"	FOOT	473	473	
70300240	TEMPORARY PAVEMENT MARKING - LINE 6"	FOOT	925	925	
70000			323	323	
70400100	TEMPORARY CONCRETE BARRIER	FOOT	462.5	462.5	
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	462.5	462.5	
70600260	IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, NARROW), TEST LEVEL 3	EACH	2	2	
70600332	IMPACT ATTENUATORS, RELOCATE (FULLY REDIRECTIVE, NARROW), TEST LEVEL 3	EACH	2	2	
72501000	TERMINAL MARKER - DIRECT APPLIED	EACH	4	4	
78100300	REPLACEMENT REFLECTORS	EACH	6	6	
7920000	CHARDRAIL DEFLECTORS TYRE A	EACH	6	6	
78200005	GUARDRAIL REFLECTORS, TYPE A	EACH	6	6	

*	SPECIALTY ITEM
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USER NAME = mmcevers	DESIGNED -	W. SLEEMAN	REVISED -	
	DRAWN -	W. SLEEMAN	REVISED -	
PLOT SCALE = 100.0000 / in.	CHECKED -	M. McEVERS	REVISED -	
PLOT DATE = 1/15/2019	DATE -	1/2019	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SCALE:

SUMMARY OF QUANTITIES	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	10	3BR-1	GREENE	35	6
			CONTRACT	NO. 76	5K57
SHEET 4 OF 6 SHEETS STA TO STA		TILINOIS SED A	D DDOJECT		-

E. P. 10E2166-29\FAP 10 IL 267 3BR-1 LICK Creek 76K9

				80% FED	CONSTRUC	TION CODE
				20% STATE		
	CODE			TOTAL	BR I DGE	
	1	ITEM	UNIT	QUANTITY	0013	
	NO.			QUANTITY	SN 031-0016	
*	78200011	BARRIER WALL REFLECTORS, TYPE C	EACH	8	8	
	78300200	RAISED REFLECTOR PAVEMENT MARKER REMOVAL	EACH	6	6	
	X0327979	PAVEMENT MARKING REMOVAL - GRINDING	SQ FT	982	982	
*	X2700006	PREFORMED PLASTIC PAVEMENT MARKING, TYPE D - LINE 4"	FOOT	473	473	
	X4420682	CLASS B PATCHES, TYPE II, 10 INCH (SPECIAL)	SQ YD	44	44	
	X7040125	PINNING TEMPORARY CONCRETE BARRIER	EACH	18	18	
	X7200200	WIDE LOAD SIGNING	L SUM	1	1	
*	X7830070	GROOVING FOR RECESSED PAVEMENT MARKING 5"	FOOT	473	473	
	Z0001899	JACK AND REMOVE EXISTING BEARINGS	EACH	12	12	
	Z0001903	STRUCTURAL STEEL REMOVAL	POUND	2960	2960	
	Z0001905	STRUCTURAL STEEL REPAIR	POUND	1340	1340	
	Z0005900	BRIDGE CURB OR HUBGUARD REPAIR	FOOT	16	16	
*	Z0007112	CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES	L SUM	1	1	
*	Z0010501	CLEANING AND PAINTING STEEL BRIDGE NO. 1	L SUM	1	1	
ماد	SPECIALT	VITENA				

* SPECIALTY ITEM

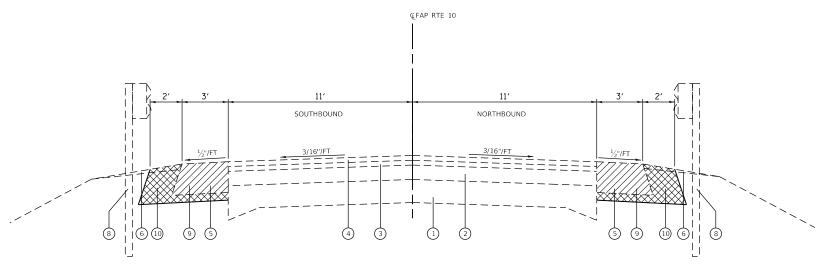
USER NAME = mmcevers	DESIGNED -	W. SLEEMAN	REVISED - 1/29/2019
	DRAWN -	W. SLEEMAN	REVISED -
PLOT SCALE = 100.0000 / in.	CHECKED -	M. McEVERS	REVISED -
PLOT DATE = 1/29/2019	DATE -	1/2019	REVISED -

		SUMMARY (F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.		
					10	3BR-1		GREENE	35	7
								CONTRACT	NO. 7	6K57
SCALE:	SHEET 5	OF 6 SH	ETS STA.	TO STA.		ILLINOIS	FED. AI	ID PROJECT		

Z000 Z000 Z000 Z000	CODE NO. 012754 013798 016001	ITEM STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES) CONSTRUCTION LAYOUT DECK SLAB REPAIR (FULL DEPTH, TYPE I)	UNIT SQ FT L SUM SQ YD	80% FED 20% STATE TOTAL QUANTITY 9	BRIDGE 0013 SN 031-0016	
Z000 Z000 Z000 Z000	NO. 012754 013798 016001	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES) CONSTRUCTION LAYOUT	SQ FT	TOTAL QUANTITY	0013 SN 031-0016	
Z000 Z000 Z000 Z000	NO. 012754 013798 016001	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES) CONSTRUCTION LAYOUT	SQ FT	QUANTITY 9	0013 SN 031-0016	
Z000 Z000 Z000 Z000	012754 013798 016001	CONSTRUCTION LAYOUT	L SUM	9	9	
z000 z000 ž 000	013798	CONSTRUCTION LAYOUT	L SUM			
z000 z000 ž 000	013798	CONSTRUCTION LAYOUT	L SUM			
z00	016001			1	1	
z00	016001			1	1	
y zoo		DECK SLAB REPAIR (FULL DEPTH, TYPE I)	SO YD			
	0076600		130 10	1	1	
	0076600					
z oc		TRAINEES	HOUR	1500	1500	
2 ZOC						
	0076604	TRAINEES TRAINING PROGRAM GRADUATE	HOUR	1500	1500	
 Ø 00₄				ı		·

USER NAME = mmcevers	DESIGNED	-	W. SLEEMAN	REVISED	-	1/29/2019	
	DRAWN	-	W. SLEEMAN	REVISED	-		
PLOT SCALE = 100.0000 ' / in.	CHECKED	-	M. McEVERS	REVISED	-		
PLOT DATE = 1/29/2019	DATE	-	1/2019	REVISED	-		
							-

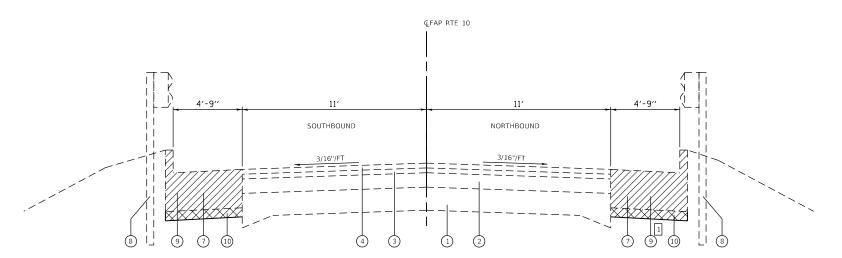
	S	UMI	ΛAR	Y OF O	UANTITI	ES		F.A.P. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
								10	31	3R-1		GREENE	35	8
												CONTRACT	NO. 76	6K57
SHEET	6	OF	6	SHEETS	STA.		TO STA.			ILLINOIS	FED. A	ID PROJECT		



EXISTING TYPICAL SECTION FAP ROUTE 10 (IL ROUTE 267)

LEFT STA. 382+98.00 TO STA. 384+31.80 STA. 386+47.90 TO STA. 387+81.00

STA. 383+02.00 TO STA. 384+31.80 STA. 386+47.90 TO STA. 387+76.00



EXISTING TYPICAL SECTION FAP ROUTE 10 (IL ROUTE 267)

STA. 384+31.80 TO STA. 384+43.04 STA. 386+36.96 TO STA. 386+47.90



PAVED SHOULDER REMOVAL



EARTH EXCAVATION

SN 031-0016 - STA. 384+42.79 TO 386+37.21

LEGEND

EXISTING CONCRETE PAVEMENT 9"-6"-9"

EXISTING HMA SURFACE COURSE

EXISTING LEVEL BINDER (MACHINE METHOD), IL-9.5 FG, N70, 3/4"

3 4 5 EXISTING HMA SURFACE COURSE, MIX "C", N70, 1 $\frac{1}{2}$ "

EXISTING HMA SHOULDER, 8"

EXISTING AGGREGATE SHOULDER WEDGE

EXISTING BRIDGE APPROACH SHOULDER PAVEMENT

EXISTING STEEL PLATE BEAM GUARDRAIL REMOVAL

PROPOSED PAVED SHOULDER REMOVAL

PROPOSED EARTH EXCAVATION

PROPOSED HOT-MIX ASPHALT SHOULDER, 10"

PROPOSED AGGREGATE WEDGE SHOULDER, TYPE B

PROPOSED SHOULDER RUMBLE STRIPS, 8"

PROPOSED CLASS B PATCHES, TYPE II, 10"

PROPOSED STEEL PLATE BEAM GUARDRAIL

16 PROPOSED SAWED LONGITUDINAL CONTRACTION JOINT 17

PROPOSED PREFORMED PLASTIC PAVEMENT MARKING, TYPE D - LINE 4" (WHITE EDGE LINE) PROPOSED PREFORMED PLASTIC PAVEMENT MARKING, TYPE D - LINE 4" (YELLOW SKIP-DASH)

NOTE:

PAVED SHOULDER REMOVAL MATERIAL = CONCRETE 8"

2 PAVED SHOULDER REMOVAL (FOR CONSTRUCTION OF CLASS B PATCH) STA. 384+36.79 TO STA. 384+43.04 RT. & LT. STA. 386+36.96 TO STA. 386+43.21 RT. & LT.

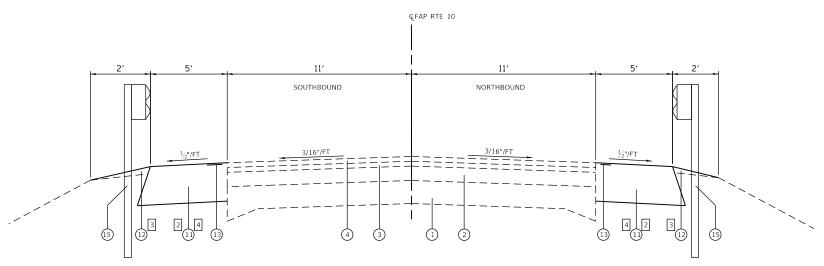
3 AGGREGATE WEDGE SHOULDER, TYPE B STA. 383+18.30 TO STA. 384+43.04 RT STA. 383+68.30 TO STA. 384+43.04 LT STA. 386+36.96 TO STA. 387+11.20 RT STA. 386+36.96 TO STA. 387+61.20 LT

BITUMINOUS MATERIALS (TACK COAT) SHALL BE USED BETWEEN LIFTS

USER NAME = mmcevers	DESIGNED -	W. SLEEMAN	REVISED -
	DRAWN -	W. SLEEMAN	REVISED -
PLOT SCALE = 100.0000 ' / in.	CHECKED -	M. McEVERS	REVISED -
PLOT DATE = 1/11/2019	DATE -	1/2019	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

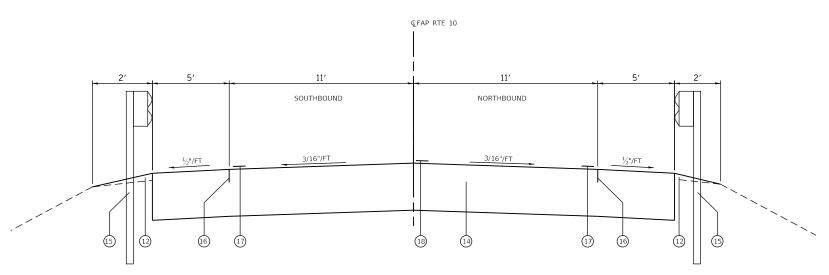
	Т	YPI	CAL SEC	TIONS		F.A.P. RTE	SECT	ΓΙΟΝ	COUNTY	TOTAL SHEETS	SHEET NO.
						10	3BI	R-1	GREENE	35	9
									CONTRAC	T NO. 76	5K57
CHEET 1	OΕ	2	CHEETC	CTA	TO CTA				 		



PROPOSED TYPICAL SECTION **FAP ROUTE 10 (IL ROUTE 267)**

STA. 382+98.00 TO STA. 384+37.79 STA. 386+42.21 TO STA. 387+81.00

STA. 382+98.00 TO STA. 384+37.79 STA. 386+42.21 TO STA. 387+76.00



PROPOSED TYPICAL SECTION FAP ROUTE 10 (IL ROUTE 267)

STA. 384+37.79 TO STA. 384+42.79 STA. 386+37.21 TO STA. 386+42.21



PAVED SHOULDER REMOVAL

SCALE:



EARTH EXCAVATION

SN 031-0016 - STA. 384+42.79 TO 386+37.21

LEGEND

EXISTING CONCRETE PAVEMENT 9"-6"-9"

EXISTING HMA SURFACE COURSE

EXISTING LEVEL BINDER (MACHINE METHOD), IL-9.5 FG, N70, 3/4"

3 4 5 EXISTING HMA SURFACE COURSE, MIX "C", N70, 1 $\frac{1}{2}$ "

EXISTING HMA SHOULDER, 8"

EXISTING AGGREGATE SHOULDER WEDGE

EXISTING BRIDGE APPROACH SHOULDER PAVEMENT

EXISTING STEEL PLATE BEAM GUARDRAIL REMOVAL

PROPOSED PAVED SHOULDER REMOVAL

PROPOSED EARTH EXCAVATION

PROPOSED HOT-MIX ASPHALT SHOULDER, 10"

PROPOSED AGGREGATE WEDGE SHOULDER, TYPE B

PROPOSED SHOULDER RUMBLE STRIPS, 8"

PROPOSED CLASS B PATCHES, TYPE II, 10"

PROPOSED STEEL PLATE BEAM GUARDRAIL

16 PROPOSED SAWED LONGITUDINAL CONTRACTION JOINT 17

PROPOSED PREFORMED PLASTIC PAVEMENT MARKING, TYPE D - LINE 4" (WHITE EDGE LINE)

PROPOSED PREFORMED PLASTIC PAVEMENT MARKING, TYPE D - LINE 4" (YELLOW SKIP-DASH)

NOTE:

PAVED SHOULDER REMOVAL MATERIAL = CONCRETE 8"

2 PAVED SHOULDER REMOVAL (FOR CONSTRUCTION OF CLASS B PATCH) STA. 384+36.79 TO STA. 384+43.04 RT. & LT. STA. 386+36.96 TO STA. 386+43.21 RT. & LT.

3 AGGREGATE WEDGE SHOULDER, TYPE B STA. 383+18.30 TO STA. 384+43.04 RT STA. 383+68.30 TO STA. 384+43.04 LT STA. 386+36.96 TO STA. 387+11.20 RT STA. 386+36.96 TO STA. 387+61.20 LT

BITUMINOUS MATERIALS (TACK COAT) SHALL BE USED BETWEEN LIFTS

USER NAME = mmcevers	DESIGNED	-	W. SLEEMAN	REVISED -
	DRAWN	-	W. SLEEMAN	REVISED -
PLOT SCALE = 100.0000 ' / in.	CHECKED	-	M. McEVERS	REVISED -
PLOT DATE = 1/11/2019	DATE	-	1/2019	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

	TYPI	CAL SEC	TIONS		F.A.P. RTE	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
					10	3BF	R-1		GREENE	35	10
									CONTRACT	NO. 76	5K57
SHEET 2	OF 2	SHEETS	STA.	TO STA.			ILLINOIS	FED. A	ID PROJECT		

EARTHWOR	K	
LOCATION STATION TO STATION	EARTH EXCAVATION	WASTE (FOR INFO ONLY)
	CU YD	CU YD
STA. 382+98.00 TO 384+43.00 LEFT	11.6	11.6
STA. 38302+00 TO 384+43.00 RIGHT	11.3	11.3
STA. 386+37.00 TO 387+76.00 RIGHT	11.1	11.1
STA. 386+37.00 TO 387+81.00 LEFT	11.6	11.6
TOTAL	45.6	45.6

1-EXCAVATION REQUIRED FOR HMA SHOULDERS

PAVEMENT						
	PAVEMENT REMOVAL	CL B PATCHES TYPE II	SAW CUTS	T I E BARS		
LOCATION		10 INCH		1 1/4"		
STATION TO STATION		(SPECIAL)				
	SQ YD	SQ YD	FOOT	EACH		
STA. 384+36.79 TO 384+43.04 LEFT	7.7	11.1	32.0	10		
STA. 384+36.79 TO 384+43.04 RIGHT	7.7	11.1	32.0	10		
STA. 384+36.79 TO 384+43.04 CL			6.3			
STA. 386+36.96 TO 386+43.21 CL			6.3			
STA. 386+36.96 TO 386+43.21 LEFT	7.7	11.1	32.0	10		
STA. 386+36.96 TO 386+43.21 RIGHT	7.7	11.1	32.0	10		
TOTAL	30.8	44.4	140.6	40		

SHOULDERS							
	PAVED	AGGREGATE	BITUMINOUS	HOT-MIX	SHOULDER	AGGREGATE	
	SHOULDER	SHOULDERS,	MATERIALS	ASPHALT	RUMBLE	WEDGE	
LOCATION	REMOVAL	TYPE B	(TACK COAT)	SHOULDERS, 10"	STRIPS,	SHOULDERS,	
STATION TO STATION					8 INCH	TYPE B	
	SQ YD	TON	POUND	SQ YD	FOOT	TON	
STA. 382+98.00 TO 384+43.04 LEFT	50.5	15.1	39.9	80.6	138.8	1.0	
STA. 383+02.00 TO 384+43.04 RIGHT	49.2	15.6	38.8	78.4	134.8	1.6	
STA. 386+36.96 TO 387+81.00 LEFT	50.1	14.9	39.6	80.0	137.8	1.6	
STA. 386+36.96 TO 387+76.00 RIGHT	48.5	14.9	38.2	77.2	132.8	0.9	
STA. 384+36.79 TO 384+43.04 LEFT (FOR PATCH)	3.5						
STA. 384+36.79 TO 384+43.04 RIGHT (FOR PATCH)	3.5						
STA. 386+36.96 TO 386+43.21 LEFT (FOR PATCH)	3.5						
STA. 386+36.96 TO 386+43.21 RIGHT (FOR PATCH)	3.5						
TOTAL	212.3	60.5	156.5	316.2	544.2	5.1	

NOTE: BITUMINOUS MATERIALS (TACK COAT) INCLUDES 2 APPLICATIONS

REFLECTIVE MAR	KERS	
LOCATION	REPLACEMENT REFLECTORS (AMBER)	*RAISED REFLECTIVE PAVEMENT
STATION TO STATION		MARKER REMOVAL
	EACH	EACH
STA. 381+43.00 TO 383+03.00 CENTERLINE	3	3
STA. 387+76.00 TO 389+40.00 CENTERLINE	3	3
TOTAL	6	6

* ONLY REFLECTORS

CONCRETE BARRIERS								
	TEMPORARY	RELOCATE	IMPACT ATTENUATORS,	IMPACT ATTENUATORS,	TEMPORARY	PINNING		
	CONCRETE	TEMPORARY	TEMPORARY (FULLY	RELOCATE (FULLY	PAVEMENT	TEMPORARY		
LOCATION	BARRIER	CONCRETE	REDIRECTIVE, NARROW)	REDIRECTIVE, NARROW)	MARKING -	CONCRETE		
STATION TO STATION		BARRIER	TEST LEVEL 3	TEST LEVEL 3	LINE 6"	BARRIER		
	FOOT	FOOT	EACH	EACH	FOOT	EACH		
STAGE I								
STA. 383+09.00 TO 384+08.50 TAPER	100.0		1		100.0	6		
STA. 384+08.5 TO 386+71.00	262.5				262.5			
STA. 386+71.00 TO 387+70.00 TAPER	100.0		1		100.0	6		
STAGE II								
STA. 383+09.00 TO 384+08.50 TAPER		100.0		1	100.0	3		
STA. 384+08.5 TO 386+71.00		262.5			262.5			
STA. 386+71.00 TO 387+70.00 TAPER		100.0		1	100.0	3		
TOTAL	462.5	462.5	2	2	925.0	18		

		GU	ARDRA I L				
LOCATION STATION TO STATION	GUARDRA I L REMOVAL	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS FOOT		TRAFFIC BARRIER TERMINAL, TY 1 (SPECIAL) TANGENT	GUARDRAIL REFLECTORS, TYPE A	BARRIER WALL REFLECTORS, TYPE C	TERMINAL MARKER - DIRECT APPLIED EACH
STA. 382+66.70 TO 384+42.79 RIGHT	176.1	F001	EACH	EACH	EACH	EACH	EACH
STA. 383+40.10 TO 384+42.79 LEFT	102.7						
STA. 386+37.21 TO 387+50.80 RIGHT	113.6						
STA. 386+37.21 TO 388+15.00 LEFT	177.8						
STA. 383+17.79 TO 384+05.29 RIGHT		87.5	1	1			1
STA. 383+67.79 TO 384+05.29 LEFT		37.5	1	1			1
STA. 386+74.71 TO 387+12.21 RIGHT		37.5	1	1			1
STA. 386+74.71 TO 387+62.21 LEFT		87.5	1	1			1
STA. 382+68.00 TO 384+43.04 RIGHT					2		
STA. 383+18.00 TO 384+43.04 LEFT					1		
STA. 384+43.04 TO 386+36.96 RIGHT						4	
STA. 384+43.04 TO 386+36.96 LEFT						4	
STA. 386+36.96 TO 387+62.00 RIGHT					1		
STA. 386+36.96 TO 388+12.00 LEFT					2		
TOTAL	570.2	250.0	4	4	6	8	4

TRAFFIC CONTROL							
LOCATION STATION TO STATION	TRAFFIC CONTROL & PROTECTION STD 701201	TRAFFIC CONTROL & PROTECTION STD 701321		TEMPORARY BRIDGE TRAFFIC SIGNALS	TEMPORARY RUMBLE STRIPS (STAGE I & II)	CHANGEABLE MESSAGE SIGNS	WIDE LOAD SIGNING
	L SUM	EACH	L SUM	EACH	EACH	CAL DA	L SUM
SN 031-0016	1	1	1	1	6	28	1
TOTAL	1	1	1	1	6	28	1

	PAVEMENT MARKING							
LOCATION STATION TO STATION	PAVEMENT MARKING BLACKOUT TAPE, 6"	PAVEMENT MARKING	SHORT TERM PAVEMENT MARKING REMOVAL	TEMPORARY PAVEMENT MARKING LINE 4"	PAVEMENT MARKING REMOVAL - GRINDING	PREFORMED PLASTIC PVMNT MARKING, TYPE D-4"	MARKING 5"	REMARKS
574 201.42 00 70 202.55 00	FOOT	FOOT	SQ FT	FOOT	SQ FT	FOOT	FOOT	CENTERLAND COLUB CTACE I
STA. 381+43.00 TO 382+66.00 STA. 381+43.00 TO 383+03.00	123.0		61.5					CENTERLINE SOLID, STAGE I CENTERLINE SKIP-DASH, STAGE I
STA. 382+98.00 TO 384+37.00	139.0		69.5					LEFT EDGE LINE, STAGE II
STA. 383+02.00 TO 387+76.00	474.0		237.0					RIGHT EDGE LINE, STAGE I
STA. 386+43.00 TO 387+81.00	138.0		69.0					LEFT EDGE LINE, STAGE II
STA. 387+76.00 TO 389+40.00	50.0		25.0					CENTERLINE SKIP-DASH, STAGE I & II
STA. 388+15.00 TO 389+40.00	125.0		62.5					CENRELINE SOLID, STAGE I & II
FINAL STAGE								
STA. 384+36.79 TO 386+43.21				206.4	69.0	206.4	206.4	LEFT EDGE LINE
STA. 384+36.79 TO 386+43.21		24.0	8.0	60.0	20.0	60.0	60.0	CENTERLINE SKIP-DASH
STA. 384+36.79 TO 386+43.21				206.4	69.0	206.4	206.4	RIGHT EDGE LINE
STAGE I - STANDARD 701321 MARKING REMOVAL				•	409.1			STOP BAR, LEFT EDGE, RIGHT EDGE
STAGE II - STANDARD 701321 MARKING REMOVAL					415.1			STOP BAR, LEFT EDGE, RIGHT EDGE
TOTAL	1089.0	24.0	552.5	472.8	982.2	472.8	472.8	

NOTE

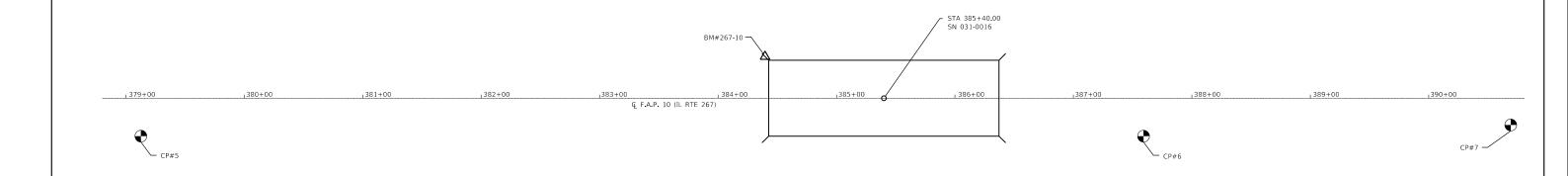
FACTORS USED FOR ESTIMATING PLAN QUANTITIES ARE AS FOLLOWS AND SHALL NOT BE USED FOR THE BASIS OF FINAL QUANTITIES:

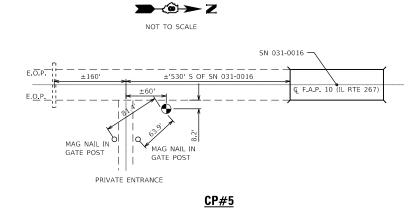
ALL HOT-MIX ASPHALT	112 LB/SQ YD/INCH
BITUMINOUS MATERIALS (PRIME COAT)	
ON AGGREGATE SURFACES	0.25 LB/SQ FT
BITUMINOUS MATERIALS (TACK COAT)	0 05 LB/60 FT
ON MILLED PAVEMENTS	
ALL AGGREGATE	

USER NAME = mmcevers	DESIGNED	-	W. SLEEMAN	REVISED -
	DRAWN	-	W. SLEEMAN	REVISED -
PLOT SCALE = 100.0000 / in.	CHECKED	-	M. McEVERS	REVISED -
PLOT DATE = 1/11/2019	DATE		1/2019	REVISED

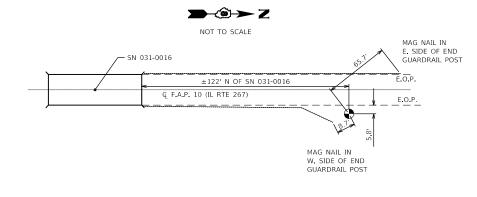
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

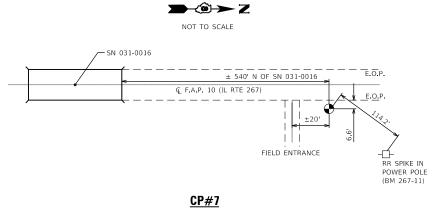
SCHEDULE OF QUANTITIES				F.A.P. RTE	SEC.	SECTION COUNT		COUNTY	TOTAL SHEETS	SHEET NO.			
				10	3BR-1 GREENE		35	11					
											CONTRACT	NO. 76	5K57
SHEET	1	OF 1	l	SHEETS	STA.	TO STA.			ILLINOIS	FED. AI	ID PROJECT		





5/8" REBAR W/ IDOT CAP





CP#6 5/8" REBAR W/ IDOT CAP

5/8" REBAR W/ IDOT CAP

GROUND COORDINATES

DESCRIPTION	NORTHING	EASTING	
FAP RTE. 10 (IL RT :	267)		
CP#5	1012416.8690	285006.4365	
CP#6	1013259.6846	285006.8238	
CP#7	1013677.5719	285009.3205	
POT STA 382+00.00	1012701.9849	284987.0892	
POT STA 385+40.00	1013041.9839	284987.9208	
POT STA 389+00.00	1013401.9829	284988.8012	

BM-267-9 (NOT SHOWN ABOVE)
FOUND CUT "□", IN THE MIDDLE OF THE WEST SIDE
OF A 4' x 4' BOX CULVERT, 0.5 MILES SOUTH OF
LICK CREEK, 11' WEST OF E.O.P.
ELEV=523.886

BM-267-11 (NOT SHOWN ABOVE) FOUND RR SPIKE IN WEST SIDE OF POWER POLE ON THE EAST SIDE OF IL 267, 650' OF THE NORTH END OF SN 031-0016, 70' EAST OF E.O.P. ELEV=536.58

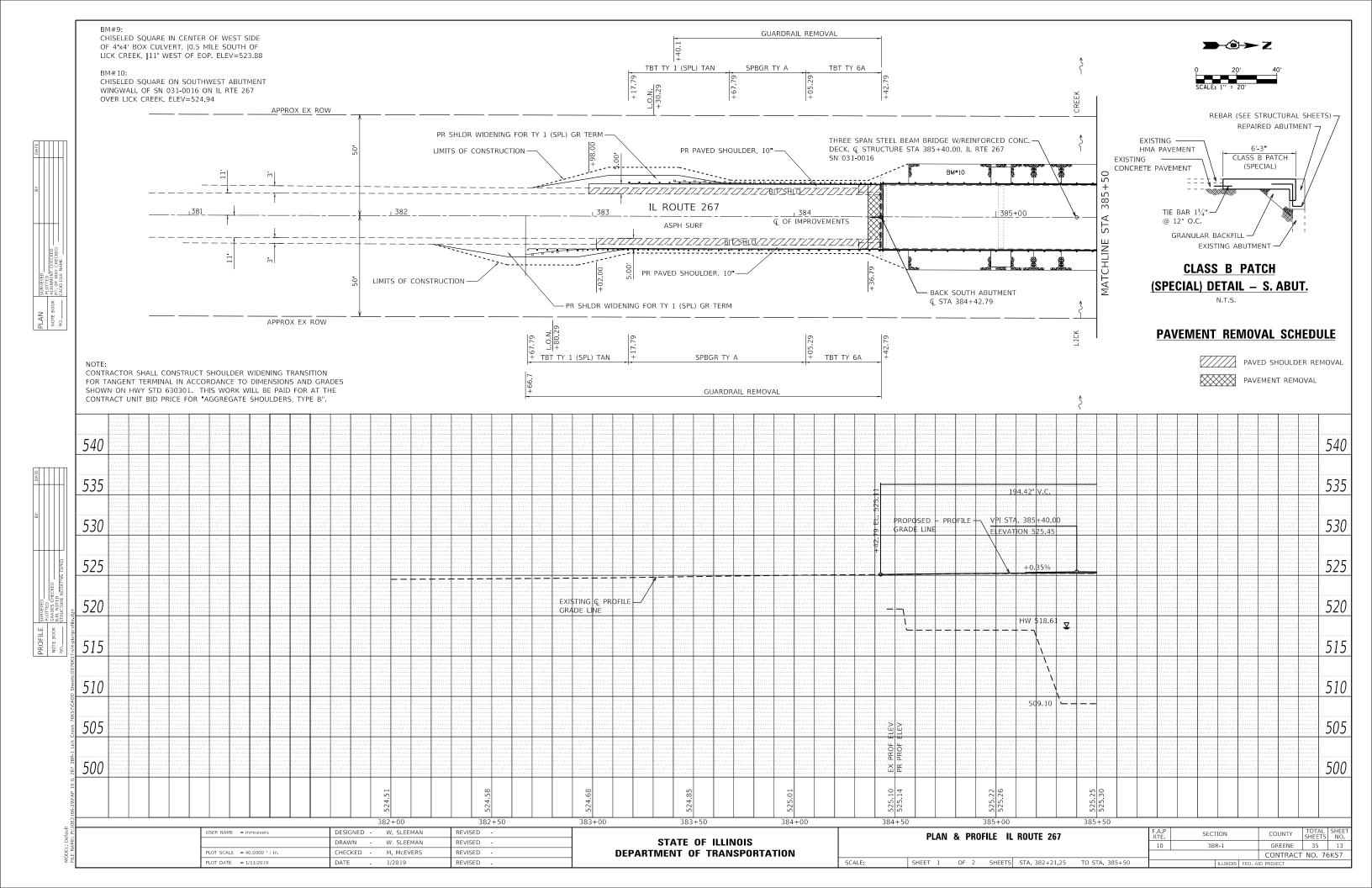
SCALE:

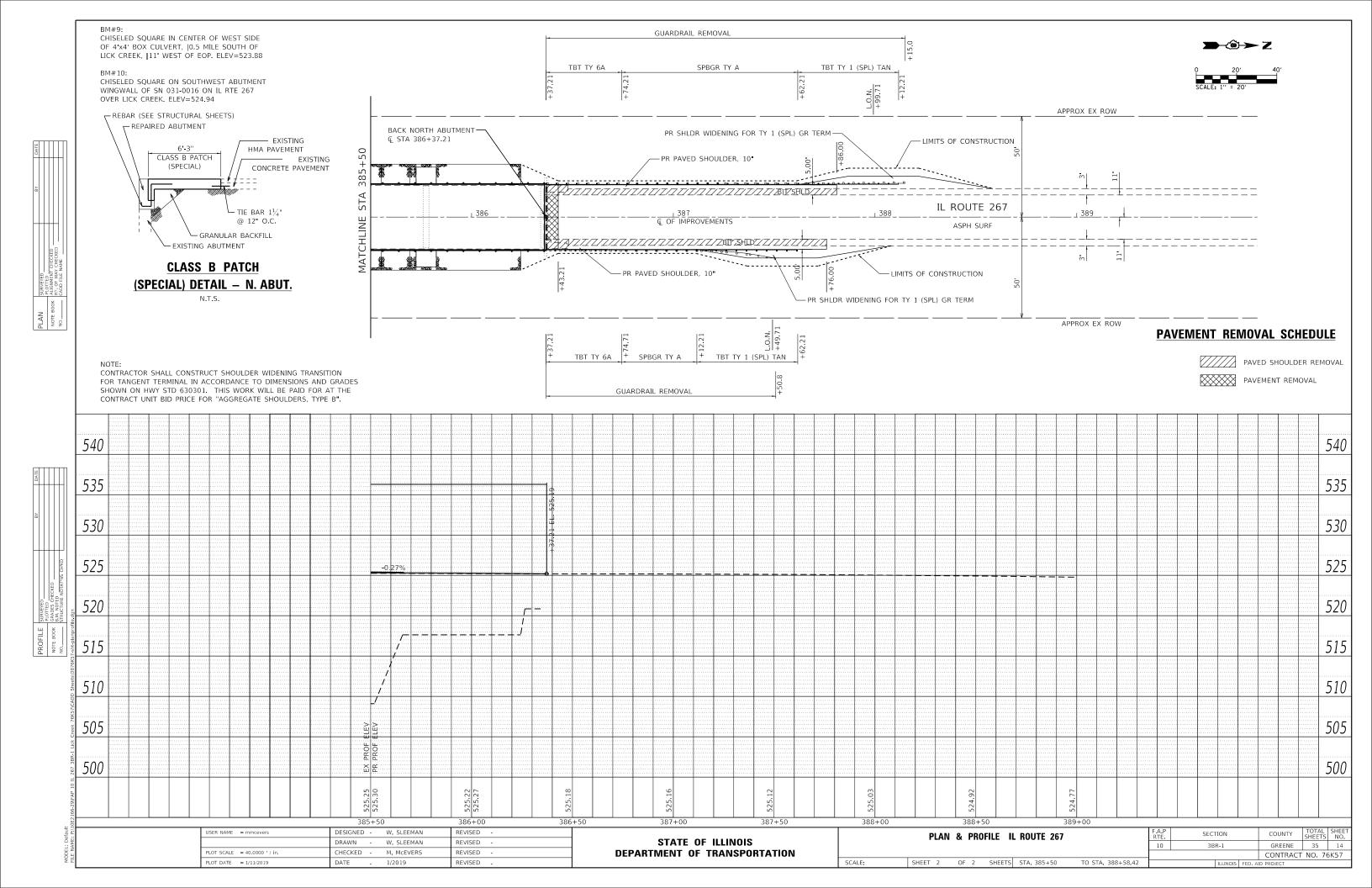
BM-267-10 FOUND CUT "□", ON SW ABUTMENT WINGWALL OF SN 031-0016 ON IL 267 OVER LICK CREEK. ELEV=524.94

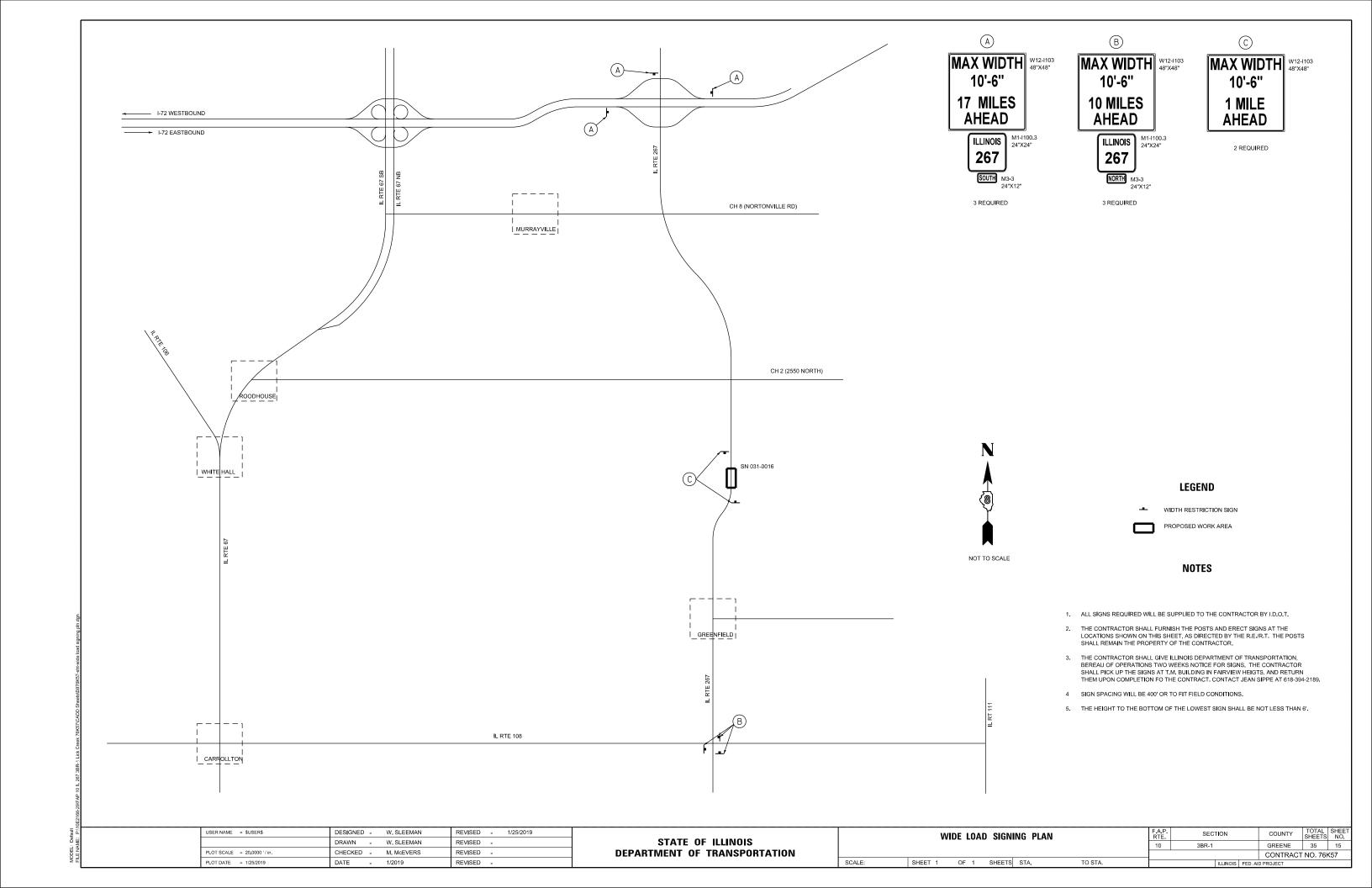
USER NAME = mmcevers	DESIGNED -	W. SLEEMAN	REVISED -
	DRAWN -	W. SLEEMAN	REVISED -
PLOT SCALE = 200.0000 ' / in.	CHECKED -	M. McEVERS	REVISED -
PLOT DATE = 1/11/2019	DATE -	1/2019	REVISED -

STATE	: OF	F ILLINOIS
DEPARTMENT	OF	TRANSPORTATION

			AL	IGNMEN	T, TIES		F.A.P. RTE	SEC ⁻	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.
& BENCHMARKS				10	3B	R-1		GREENE	35	12			
			<u> </u>	DENGIN	IAIIKS						CONTRACT	NO. 76	5K57
SHEET 1		OF	1	SHEETS	STA.	TO STA.			ILLINOIS	FED. AI	ID PROJECT		







B.M.: BM#9; Chiseled Square in center of west side of 4'x4' box culvert ±0.5 mi. South of Lick Creek, ±11' West of EOP, BM#10; Chiseled Square on Southwest abut. wingwall of SN 031-0016 on IL 267 over Lick Creek, Elev. 524.94'

Traffic: Traffic shall be maintained during construction by Staged Construction.

Existing Structure: SN 031-0016, 3-span Reinforced concrete slab on wide flange steel beams, Reinforced concrete piers, Reinforced concrete abutments, Built as F.A.I. 64 Sec. 3-B-F-D Sta. 385+40.00 in 1938, Deck replacement and existing wide flange beams made composite 194'-5" long x 32'-6" wide as F.A.Rte. 164 Sec. 3BY-1 Sta. 385+40.00 in 1969, Deck Overlay & Joint/Bearing Replacement as F.A.Rte. 10 Sec. 3BI-1 in 1987.

Salvage: None.

STATION 385+40.00 RE-BUILT 20 BY STATE OF ILLINOIS F.A.P. RT. 10 SEC. 3BR-1 LOADING HS20 STRUCTURE NO. 031-0016

NAME PLATE See Std. 515001

Existing Name Plate shall be cleaned and relocated next to new Name Plate. Cost included with Name Plates.

SCOPE OF WORK

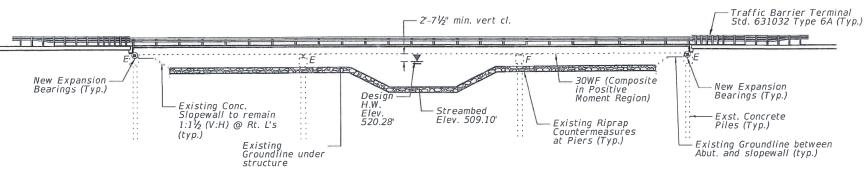
- Pre-Stage Deck Repair, interior diaphragm replacement, and end beam repairs at abutments for Stage I Traffic.
 Remove and replace concrete deck and steel railing.
- Remove and replace deteriorated diaphragms & all end diaphragms. 4) Remove and replace all end bearings
- Repair all beam ends with steel plate retrofitting and adding galvanized angle bearing stiffeners.
 Repair deteriorated parts of concrete substructures.
 Clean Abut. & Pier seats.

- 8) Clean and paint beam ends within 5 ft. of expansion joints and exterior surface and bottom flange of fascia beams.

 9) Install Class A5 riprap from toe of slopewall to toe of slopewall and 10 ft. both upstream and downstream of the superstructure in accordance with Hydraulic Statement in the Bridge Condition Report.

SHEET INDEX General Plan & Elevation General Data Pre-Stage Repairs Staging
Temporary Concrete Barrier
Top of Slab Elevations
Superstructure 6-8 Superstructure
Superstructure Details
Steel Railing, Type SM
Preformed Joint Strip Seal
Abutment Backwall Replacement 10 12 13 14 15-16

18-19 20

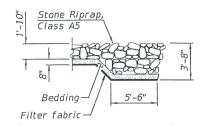


APPROVED

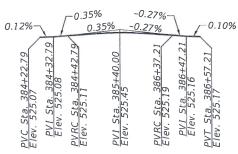
FOR STRUCTURAL ADEQUACY ONLY

Pier Concrete Repair Structural Steel
Bearing Removal
Bearing Details
Bar Splicer Assembly

ENGINEER OF BRIDGES AND STRUCTURES



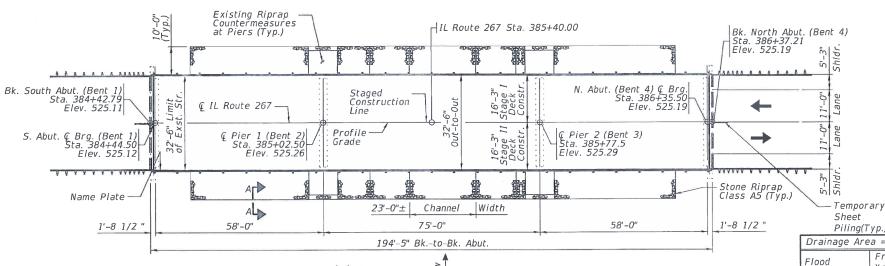
SECTION A-A



L.V.C. = 20.00' L.V.C. = 194.42' L.V.C. = 20.00'

PROFILE GRADE IL ROUTE 267

ELEVATION



PLAN

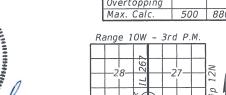
DESIGN SCOUR ELEVATION TABLE

Event / Limit		Design Scour Elevations (ft.)							
State	S. Abut.	Pier 1	Pier 2	N. Abut. (Bent 4)	Itom 113				
	(Bent 1)	(Bent 2)	(Bent 3)	(Bent 4)	item 115				
Q100	516.79	502.15							
Q200	516.79			516.79	5				
Design	516.79	502.15	503.08	516.79					

Temporary

WATERWAY INFORMATION

Drainage Area = 17.73 sq.mi. Low Grade Elev. 522.91 @ Sta. 381+00									
Flood	Freq.	Q	0peni	ng Ft²	Nat.	Head	- Ft.	Headwa	ater El.
F1000	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
	10								
Design	50	5778	834	834	518.61	1.67	1.67	520.28	520.28
Base	100	6682	864	864	518.78	1.94	1.94	520.72	520.72
Overtopping									
Max. Calc.	500	8869			519.24	2.57	2.57	521.81	521.81



081-007375

LICENSED

STRUCTURAL

ENGINEER

OF

LOCATION SKETCH

GENERAL PLAN & ELEVATION IL ROUTE 267 OVER LICK CREEK F.A.P. RTE. 10 - SECTION 3BR-1 GREENE COUNTY IL ROUTE 267 STA. 385+40.00

STRUCTURE NO. 031-0016

DESIGN SPECIFICATIONS

2002 AASHTO Standard Specifications for Highway Bridges

LOADING HS-20-44

Future wearing surface not allowed.

SEISMIC DATA

Seismic Performance Zone (SPZ) = 2Design Spectral Acceleration at 1.0 sec. $(S_{DI}) = .163$ Design Spectral Acceleration at 0.2 sec. $(S_{DS}) = .309$ Soil Site Class = D

DESIGNED - MBH

RHB

CHECKED -

DRAWN

FIELD UNITS (Exist. Const.)

fy = 36,000 psi (M270 Grade 36) (Misc. Steel)

DESIGN STRESSES

FIELD UNITS

f'c = 3,500 psi (Substructure)

fy = 60,000 psi (Reinforcement)

f'c = 5,000 psi (Superstructure)

f'c superstructure = 2,400 psi f'c substructure = 2,400 psi fy substructure = 40,000 psi (Reinforcement) fy = 33,000 psi (A7), Beam Strength limited to yield strength subject to bracing req.

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

GENERAL PLAN & ELEVATION STRUCTURE NO. 031-0016 SHEET 1 OF 20 SHEETS

λ.P. ΓE.	SECTI	ON		COUNTY	TOTAL SHEETS	SHEET NO.
	3BR-1			GREENE	35	16
				CONTRA	CT NO. 7	76K57
	I	LINOIS	FED. All	D PROJECT		

BENTON & ASSOCIATES INC BENTON & ASSOCIATES, INC.

USER NAME =

PLOT SCALE

PLOT DATE =

Consulting Engineers / Land Surveyors 1970 West Lafayette Ave. Jacksonville, IL 62650 Phone: 217-245-4146 Fax: 217-245-4149 IL Design Firm Registration No. 184-000852

STIMES

REVISED REVISED CHECKED -RHB

REVISED -

REVISED - 3/14/19

GENERAL NOTES

- 1. Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts. Bolts 7/8" φ, holes 15/16" φ, unless otherwise noted.
- 2. No field welding is permitted except as specified in the contract documents.
- 3. The Contractor shall test the existing welds by non-destructive methods within 2 ft. of the end of the existing cover plates for cracks after removal of the existing concrete deck. After deck removal, the Contractor shall test all existing top coverplate welds for cracks using non-destructive methods. Dye penetrant (PT), magnetic particle (MT), or other approved testing method shall be performed by qualified personnel approved by the Engineer. If cracks are found, report them to the Bureau of Bridges and Structures for disposition. The cost of testing is included in the Removal of Existing Concrete Deck. The cost of crack repair, if necessary, will be paid for according to Article 109.04 of the Standard Specifications.
- 4. Reinforcement bars designated (E) shall be epoxy coated.
- 5. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 6. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 7. Concrete Sealer shall be applied to the designated areas of the Abutments.
- 8. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 9. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- 10. Structural Steel shall be galvanized, where noted in the plans, in accordance with Hot-Dip Galvanizing for Structural Steel Special Provision. Cost included with Furnishing and Erecting Structural Steel.
- 11. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provisions "Cleaning & Painting Contact Surface Areas of Existing Steel Structures".
- 12. Cleaning and painting of the existing structural steel shall be as specified in the special provision for "Cleaning and Painting Existing Steel Structures". All existing steel shall be cleaned per Near White Blast Cleaning SSPC-SP10. All existing steel shall be painted according to the requirements of Paint System 1 OZ/E/U. The color of the final finish coat for all interior steel surfaces shall be Gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of fascia beams shall be Gray, Munsell No. 5B 7/1.
- 13. "The Organic Zinc Rich Primer/Epoxy/Urethane" paint system shall be used for painting of new non-galvanized structural steel except where otherwise noted. The entire system shall be shop applied, with the exception that the exterior surfaces and bottom of the bottom flange of the fascia beams, masked off connection surfaces, and field installed fasteners, all of which shall be touched up and finish coated in the field. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/1. The color of the final coat for the exterior and bottom flange of the fascia beams shall be Gray, Munsell No. 5B 7/1.

<u>SN 031-0016</u> TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Stone Riprap, Class A5	Sq. Yd.		895	895
Filter Fabric	Sq. Yd.		915	915
Concrete Removal	Cu. Yd.		6.7	6.7
Removal of Existing Concrete Deck	Each	1		1
Structure Excavation	Cu. Yd.		7.0	7.0
Concrete Structures	Cu. Yd.		3.7	3.7
Concrete Superstructure	Cu. Yd.	175		175
Bridge Deck Grooving	Sq. Yd.	470		470
Protective Coat	Sq. Yd.	694		694
Furnishing and Erecting Structural Steel	Pound	4,280		4,280
Reinforcement Bars, Epoxy Coated	Pound	43,080	1,290	44,370
Bar Splicers	Each	16	549	565
Steel Railing, Type SM	Foot	389		389
Name Plates	Each	1		1
Preformed Joint Strip Seal	Foot	65		65
Elastomeric Bearing Assembly, Type I	Each	6		6
Elastomeric Bearing Assembly, Type II	Each	6		6
Anchor Bolts, 5/8"	Each		24	24
Temporary Sheet Piling	Sq. Ft.		103	103
Concrete Sealer	Sq. Ft.		72	72
Granular Backfill for Structures	Cu. Yd.		7.0	7.0
Jack and Remove Existing Bearings	Each		12	12
Structural Steel Removal	Pound	2,960		2,960
Structural Steel Repair	Pound	1,340		1,340
Bridge Curb or Hubguard Repair	Foot	16		16
Containment and Disposal of Lead Paint Cleaning Residues	L. Sum	1		1
Cleaning and Painting Steel Bridge No. 1	L. Sum	1		1
Structural Repair of Concrete (Depth <=5")	Sq. Ft.		9	9
Deck Slab Repair (Full Depth, Type I)	Sq. Yd.	1		1

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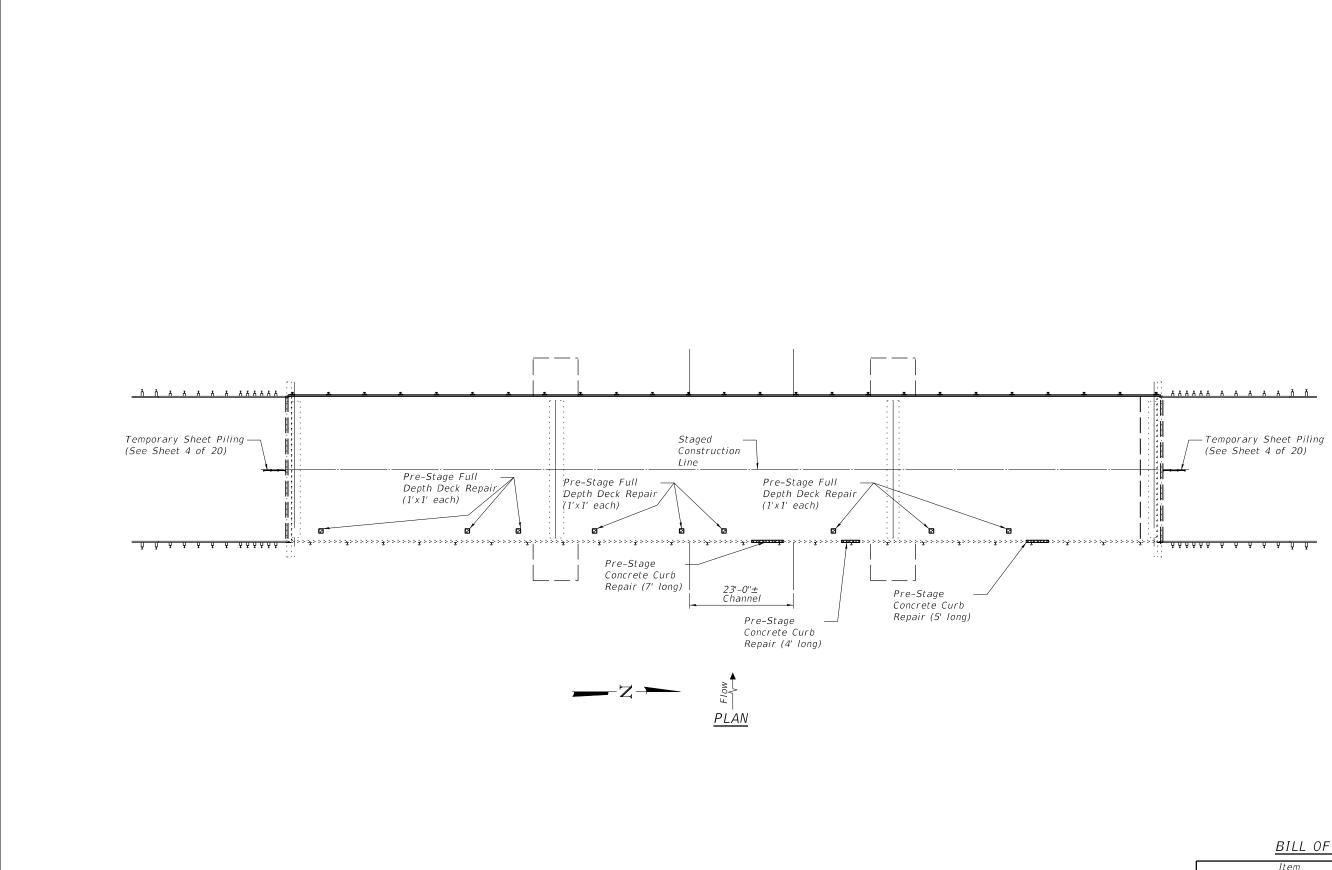
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 PLOT SCALE
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 PLOT DATE
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 RHB
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



BILL OF MATERIAL

Item	Unit	Total
Deck Slab Repair (Full Depth, Type I)	Sq. Yd.	1
Bridge Curb or Hubguard Repair	Ft.	16

 USER NAME
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 PLOT SCALE
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 MBH
 REVISED

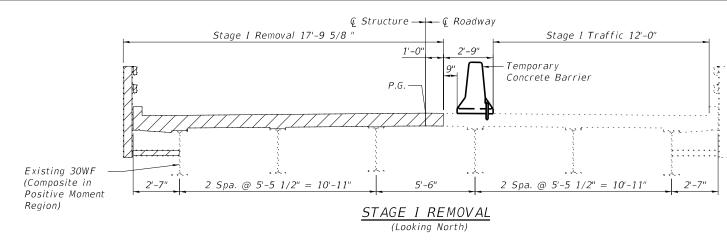
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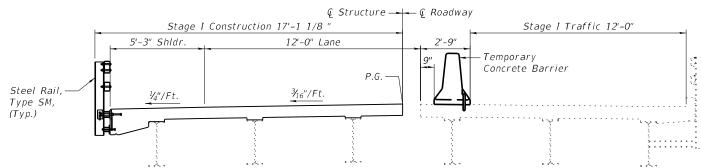
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

 PRE-STAGE PLAN DECK & SUPERSTRUCTURE REPAIRS
 F.A.P. RTE.
 SECTION

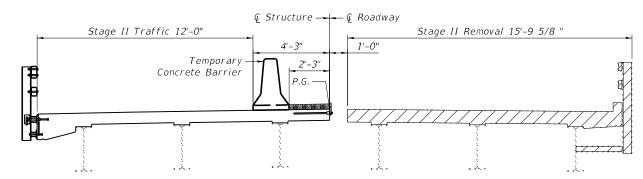
 3 STRUCTURE NO. 031-0016
 10 3BR-1

 3 SHEET 3 OF 20 SHEETS
 11 ILLINOIS



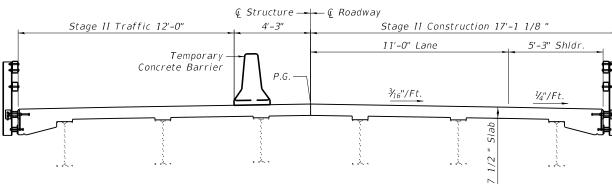


STAGE I CONSTRUCTION (Looking North)



STAGE II REMOVAL

(Looking North)



STAGE II CONSTRUCTION

(Looking North)

Notes:

- 1. See Standard 704001 for details of Temporary Concrete Barrier.
- 2. See Roadway Plans for quantity of Temporary Concrete Barrier.

Temporary Sheet Pile 4'-9" Min. Section Modulus = 2 in³ /ft.

SHEET PILING ELEVATION

(North Abut. shown: South Abut. similar)

Notes:

- Minimum section modulus of temporary sheet piling shall be as shown in the Elevation View of this sheet.
- 2. If the contractor chooses to alter the temporary cantilevered sheet pile design requirements shown on the plans for lesser design requirements, the full design calculations with the required seals shall be submitted to the Department for review and approval.
- 3. The contractor shall connect the first sheet to the existing abutment wall to ensure stability of the sheets driven to the top of the footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling.

BILL OF MATERIAL

L	Item	Unit	Quantity
	Temporary Sheet Piling	Sq. Ft.	103

BENTON & ASSOCIATES, INC. USER NAME = DESIGNED - MBH REVISED -SECTION **STAGING STATE OF ILLINOIS** CHECKED - RHB REVISED -3BR-1 GREENE 35 19 **STRUCTURE NO. 031-0016** PLOT SCALE = REVISED -**DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 76K57 SHEET 4 OF 20 SHEETS PLOT DATE = CHECKED - RHB REVISED -

when "A" is greater than 3'-1". NEW SLAB OR NEW DECK BEAM

barrier shall be restrained to the new slab according

to Detail I, II or III. No restraint is required

-Stage removal line - Stage removal line 1'-101/2" Temporary Concrete Barrier See Standard 704001 min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint

* When hot-mix asphalt wearng surface is present, embedment shall be 3" plus the wearing surface depth.

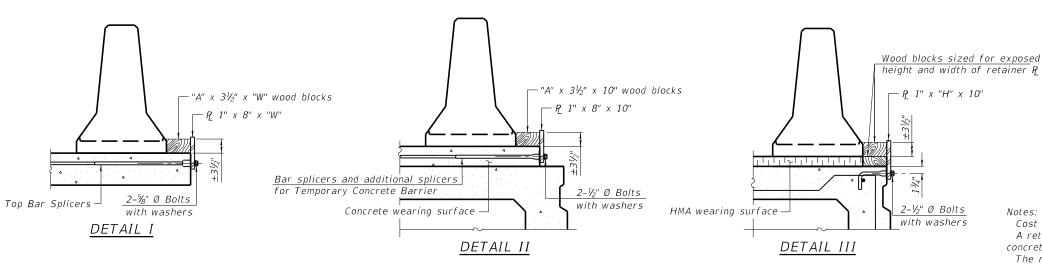
EXISTING DECK BEAM

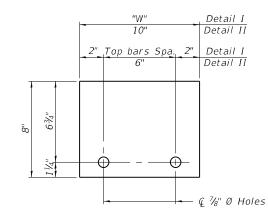
1x8 UNG US Std. 1½6" I.D. x 2½" O.D. x approx. 8 guage thick washer 1" Ø pin RESTRAINING PIN

SECTIONS THRU SLAB OR DECK BEAM

is required when "A" is greater than 3'-1".

EXISTING SLAB





10" → Ç 1/8" Ø Holes

STEEL RETAINER P 1" x "H" x 10" (Detail III)

surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)

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BAR SPLICER FOR #4 BAR - DETAIL III

Notes: Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

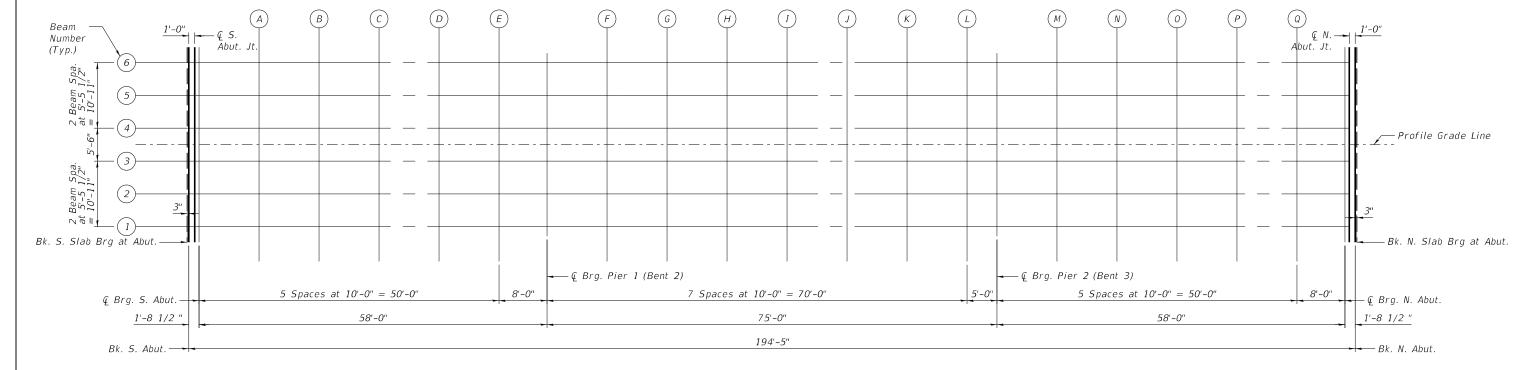
The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate

shall not be removed until just prior to placing the adjacent beam. When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

Detail I - Installation for a new bridge deck or bridge slab.

Detail II - Installation for a new deck beam with an initial concrete wearing

DEPARTMENT OF TRANSPORTATION



<u>PLAN</u>

BENTON & ASSOCIATES, INC.

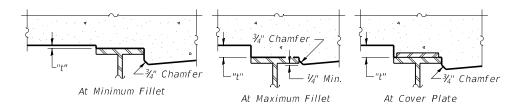
Lay		CHECKED - RHB	REVISED -	STATE OF ILLINOIS	TOP OF SLAB ELEVATIONS STRUCTURE NO. 031-0016	10 RTE.	SECTION 3BR-1	GREENE	SHEETS NO.
DE	PLOT SCALE =	DRAWN - MBH	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 031-0016			CONTRAC	CT NO. 76K57
9 11 9	PLOT DATE =	CHECKED - RHB	REVISED -		SHEET 6 OF 20 SHEETS		ILLINOIS	FED. AID PROJECT	

DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

Note

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS

BEAM 1

	<u>DL</u>	<u> </u>		
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	384+42.79	13.67'	524.88	524.88
€ S. Abut. Jt.	384+43.79	13.67'	524.89	524.89
⊈ Brg. S. Abut.	384+44.50	13.67'	524.89	524.89
А	384+54.50	13.67'	524.92	524.95
В	384+64.50	13.67'	524.95	524.99
С	384+74.50	13.67'	524.98	525.01
D	384+84.50	13.67'	525.00	525.02
Е	384+94.50	13.67'	525.02	525.02
	385+02.50	13.67'	525.03	525.03
F	385+12.50	13.67'	525.05	525.06
G	385+22.50	13.67'	525.06	525.09
Н	385+32.50	13.67'	525.07	525.12
I	385+42.50	13.67'	525.07	525.13
J	385+52.50	13.67'	525.07	525.12
К	385+62.50	13.67'	525.07	525.10
L	385+72.50	13.67'	525.07	525.07
	385+77.50	13.67'	525.06	525.06
М	385+87.50	13.67'	525.06	525.06
N	385+97.50	13.67'	525.04	525.07
0	386+07.50	13.67'	525.03	525.06
Р	386+17.50	13.67'	525.01	525.05
Q	386+27.50	13.67'	524.99	525.01
⊈ Brg. N. Abut.	386+35.50	13.67'	524.96	524.96
ℚ N. Abut. Jt.	386+36.21	13.67'	524.96	524.96
Bk. N. Abut.	386+37.21	13.67'	524.96	524.96

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	384+42.79	8.21'	524.98	524.98
ℚ S. Abut. Jt.	384+43.79	8.21'	524.99	524.99
ℚ Brg. S. Abut.	384+44.50	8.21'	524.99	524.99
А	384+54.50	8.21'	525.02	525.04
В	384+64.50	8.21'	525.05	525.08
С	384+74.50	8.21'	525.08	525.10
D	384+84.50	8.21'	525.10	525.11
Е	384+94.50	8.21'	525.12	525.12
@ Brg. PIER 1 (Bent 2)	385+02.50	8.21'	525.13	525.13
F	385+12.50	8.21'	525.15	525.16
G	385+22.50	8.21'	525.16	525.18
Н	385+32.50	8.21'	525.17	525.20
I	385+42.50	8.21'	525.17	525.21
J	385+52.50	8.21'	525.17	525.20
К	385+62.50	8.21'	525.17	525.19
L	385+72.50	8.21'	525.17	525.17
& Brg. PIER 2 (Bent 3)	385+77.50	8.21'	525.16	525.16
М	385+87.50	8.21'	525.15	525.16
N	385+97.50	8.21'	525.14	525.16
0	386+07.50	8.21'	525.13	525.15
Р	386+17.50	8.21'	525.11	525.13
Q	386+27.50	8.21'	525.08	525.10
⊊ Brg. N. Abut.	386+35.50	8.21'	525.06	525.06
ℚ N. Abut. Jt.	386+36.21	8.21'	525.06	525.06
Bk. N. Abut.	386+37.21	8.21'	525.06	525.06

BEAM 3

	<u> </u>	/ 1/ / _		
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	384+42.79	384+42.79 2.75'		525.07
ℚ S. Abut. Jt.	384+43.79	2.75'	525.07	525.07
ℚ Brg. S. Abut.	384+44.50	2.75'	525.07	525.07
А	384+54.50	2.75'	525.11	525.12
В	384+64.50	2.75'	525.14	525.16
С	384+74.50	2.75'	525.16	525.19
D	384+84.50	2.75'	525.19	525.20
E	384+94.50	2.75'	525.21	525.21
© Brg. PIER 1 (Bent 2)	385+02.50	2.75'	525.22	525.22
F	385+12.50	2.75'	525.23	525.24
G	385+22.50	2.75'	525.24	525.27
Н	385+32.50	2.75'	525.25	525.29
I	385+42.50	2.75'	525.26	525.30
J	385+52.50	2.75'	525.26	525.29
К	385+62.50	2.75'	525.26	525.27
L	385+72.50	2.75'	525.25	525.26
ℚ Brg. PIER 2 (Bent 3)	385+77.50	2.75'	525.25	525.25
М	385+87.50	2.75'	525.24	525.24
N	385+97.50	2.75'	525.23	525.24
0	386+07.50	2.75'	525.21	525.24
Р	386+17.50	2.75'	525.19	525.22
Q	386+27.50	2.75'	525.17	525.18
ℚ Brg. N. Abut.	386+35.50	2.75'	525.15	525.15
ℚ N. Abut. Jt.	386+36.21	2.75'	525.15	525.15
Bk. N. Abut.	386+37.21	2.75'	525.14	525.14
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E-5 BENTON & ASSOCIATES, INC.

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PLOT DATE =	CHECKED	-	RHB	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 TOP OF SLAB ELEVATIONS
 F.A.P. RTE.
 SECTION

 STRUCTURE NO. 031-0016
 10
 3BR-1

 SHEET 7 OF 20 SHEETS
 ILLINOIS

PROFILE GRADE LINE

	TROTTLL	OTT/IDE E		
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	384+42.79	0.00'	525.11	525.11
ℚ S. Abut. Jt.	384+43.79	0.00'	525.11	525.11
ℚ Brg. S. Abut.	384+44.50	0.00'	525.12	525.12
А	384+54.50	0.00'	525.15	525.17
В	384+64.50	0.00'	525.18	525.21
С	384+74.50	0.00'	525.20	525.23
D	384+84.50	0.00'	525.23	525.24
Е	384+94.50	0.00'	525.25	525.25
Brg. PIER 1 (Bent 2)	385+02.50	0.00'	525.26	525.26
F	385+12.50	0.00'	525.28	525.28
G	385+22.50	0.00'	525.29	525.31
Н	385+32.50	0.00'	525.30	525.33
I	385+42.50	0.00'	525.30	525.34
J	385+52.50	0.00'	525.30	525.33
К	385+62.50	0.00'	525.30	525.32
L	385+72.50	0.00'	525.30	525.30
Brg. PIER 2 (Bent 3)	385+77.50	0.00'	525.29	525.29
М	385+87.50	0.00'	525.28	525.29
N	385+97.50	0.00'	525.27	525.29
0	386+07.50	0.00'	525.25	525.28
Р	386+17.50	0.00'	525.23	525.26
Q	386+27.50	0.00'	525.21	525.23
ℚ Brg. N. Abut.	386+35.50	0.00'	525.19	525.19
ℚ N. Abut. Jt.	386+36.21	0.00'	525.19	525.19
Bk. N. Abut.	386+37.21	0.00'	525.19	525.19

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	384+42.79	-2.75'	525.07	525.07
€ S. Abut. Jt.	384+43.79	-2.75'	525.07	525.07
♀ Brg. S. Abut.	384+44.50	-2.75'	525.07	525.07
A	384+54.50	-2.75'	525.11	525.12
В	384+64.50	-2.75'	525.14	525.16
С	384+74.50	-2.75'	525.16	525.19
D	384+84.50	-2.75'	525.19	525.20
Е	384+94.50	-2.75'	525.21	525.21
© Brg. PIER 1 (Bent 2)	385+02.50	-2.75'	525.22	525.22
F	385+12.50	-2.75'	525.23	525.24
G	385+22.50	-2.75'	525.24	525.27
Н	385+32.50	-2.75'	525.25	525.29
I	385+42.50	-2.75'	525.26	525.30
J	385+52.50	-2.75'	525.26	525.29
K	385+62.50	-2.75'	525.26	525.27
L	385+72.50	-2.75'	525.25	<i>525.26</i>
@ Brg. PIER 2 (Bent 3)	385+77.50	-2.75'	525.25	525.25
М	385+87.50	-2.75'	525.24	525.24
N	385+97.50	-2.75'	525.23	525.24
0	386+07.50	-2.75'	525.21	525.24
Р	386+17.50	-2.75'	525.19	525.22
Q	386+27.50	-2.75'	525.17	525.18
⊈ Brg. N. Abut.	386+35.50	-2.75'	525.15	525.15
ℚ N. Abut. Jt.	386+36.21	-2.75'	525.15	525.15
Bk. N. Abut.	386+37.21	-2.75'	525.14	525.14

<u>BEAM 5</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	384+42.79	-8.21'	524.98	524.98
€ S. Abut. Jt.	384+43.79	-8.21'	524.99	524.99
ℚ Brg. S. Abut.	384+44.50	-8.21'	524.99	524.99
А	384+54.50	-8.21'	525.02	525.04
В	384+64.50	-8.21'	525.05	525.08
С	384+74.50	-8.21'	525.08	525.10
D	384+84.50	-8.21'	525.10	525.11
Е	384+94.50	-8.21'	525.12	525.12
© Brg. PIER 1 (Bent 2)	385+02.50	-8.21'	525.13	525.13
F	385+12.50	-8.21'	525.15	525.16
G	385+22.50	-8.21'	525.16	525.18
Н	385+32.50	-8.21'	525.17	525.20
I	385+42.50	-8.21'	525.17	525.21
J	385+52.50	-8.21'	525.17	525.20
К	385+62.50	-8.21'	525.17	525.19
L	385+72.50	-8.21'	525.17	525.17
	385+77.50	-8.21'	525.16	525.16
М	385+87.50	-8.21'	525.15	525.16
N	385+97.50	-8.21'	525.14	525.16
0	386+07.50	-8.21'	525.13	525.15
Р	386+17.50	-8.21'	525.11	525.13
Q	386+27.50	-8.21'	525.08	525.10
ℚ Brg. N. Abut.	386+35.50	-8.21'	525.06	525.06
ℚ N. Abut. Jt.	386+36.21	-8.21'	525.06	525.06
Bk. N. Abut.	386+37.21	-8.21'	525.06	525.06

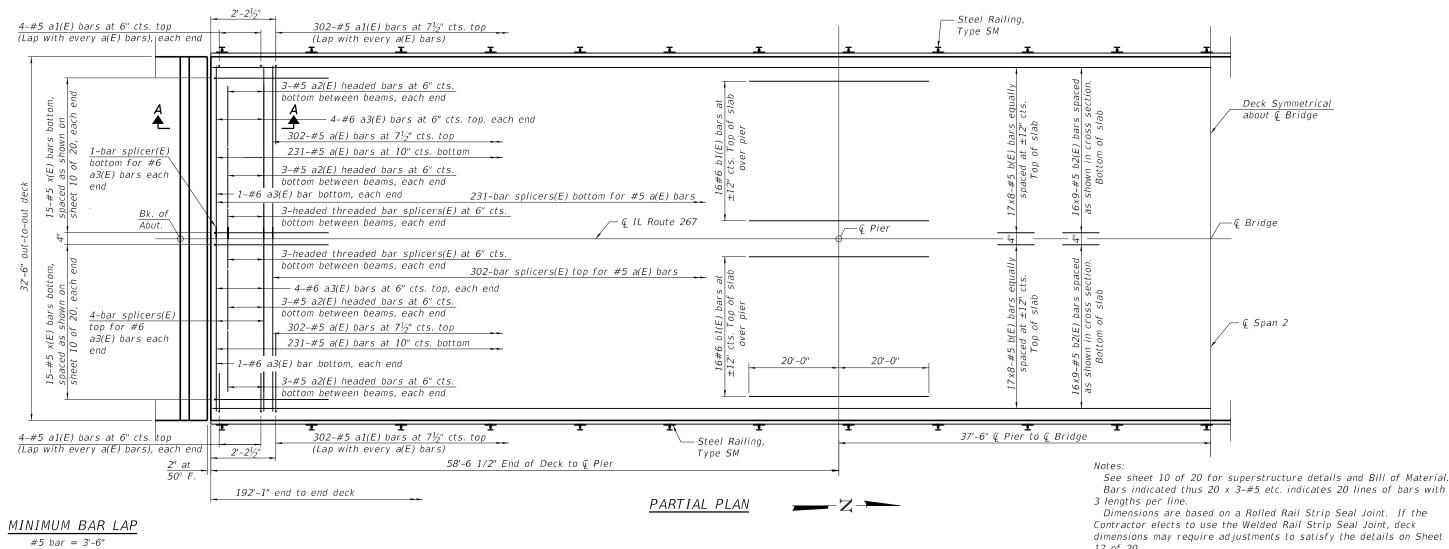
BEAM 6

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Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. S. Abut.	384+42.79	-13.67'	524.88	524.88
€ S. Abut. Jt.	384+43.79	-13.67'	524.89	524.89
ℚ Brg. S. Abut.	384+44.50	-13.67'	524.89	524.89
Α	384+54.50	-13.67'	524.92	524.95
В	384+64.50	-13.67'	524.95	524.99
С	384+74.50	-13.67'	524.98	525.01
D	384+84.50	-13.67'	525.00	525.02
E	384+94.50	-13.67'	525.02	525.02
© Brg. PIER 1 (Bent 2)	385+02.50	-13.67'	525.03	525.03
F	385+12.50	-13.67'	525.05	525.06
G	385+22.50	-13.67'	525.06	525.09
Н	385+32.50	-13.67'	525.07	525.12
I	385+42.50	-13.67'	525.07	525.13
J	385+52.50	-13.67'	525.07	525.12
К	385+62.50	-13.67'	525.07	525.10
L	385+72.50	-13.67'	525.07	525.07
ℚ Brg. PIER 2 (Bent 3)	385+77.50	-13.67'	525.06	525.06
М	385+87.50	-13.67'	525.06	525.06
N	385+97.50	-13.67'	525.04	525.07
0	386+07.50	-13.67'	525.03	525.06
Р	386+17.50	-13.67'	525.01	525.05
Q	386+27.50	-13.67'	524.99	525.01
ℚ Brg. N. Abut.	386+35.50	-13.67'	524.96	524.96
ℚ N. Abut. Jt.	386+36.21	-13.67'	524.96	524.96
Bk. N. Abut.	386+37.21	-13.67'	524.96	524.96

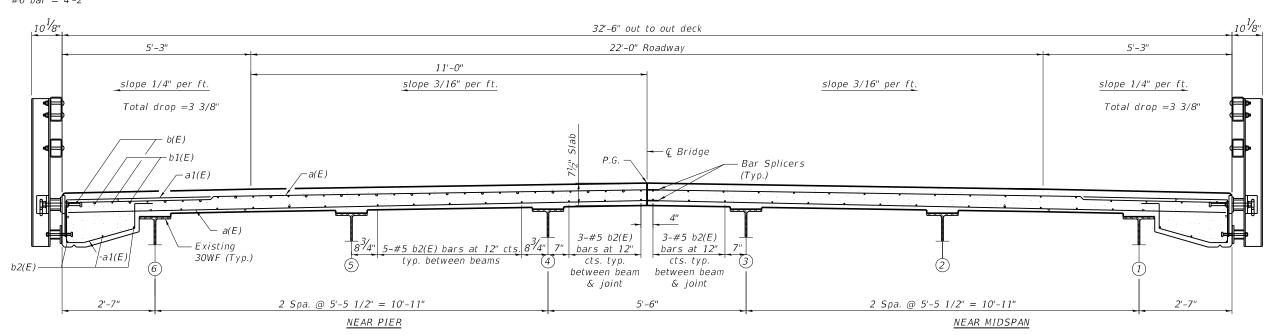
E-S BENTON & ASSOCIATES, INC.

USER NAME =	DESIGNED -	MBH	REVISED -
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PLOT SCALE =	DRAWN -	MBH	REVISED -
PLOT DATE =	CHECKED -	RHB	REVISED -

OP OF SLAB ELEVATIONS 'RUCTURE NO. 031-0016 SHEET 8 OF 20 SHEETS	F.A.P. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 031-0016		3BR-1			GREENE	35	23
311/00101/L 140: 031-0010					CONTRA	CT NO.	76K57
SHEET 8 OF 20 SHEETS			ILLINOIS	FED. All	D PROJECT		



#5 bar = 3'-6" #6 bar = 4'-2"



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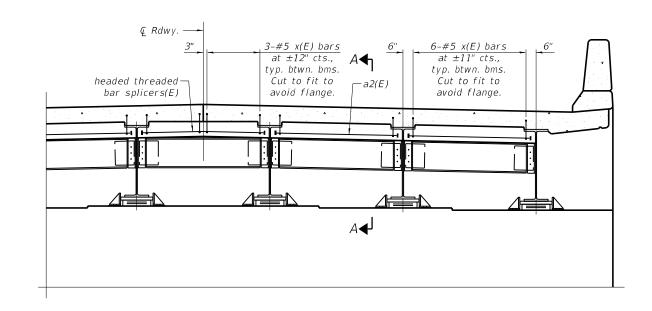
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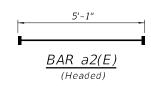
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

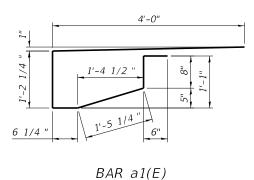
CROSS SECTION (Looking North)

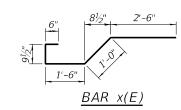
SUPERSTRUCTURE	F.A.P. RTE		
STRUCTURE NO. 031-0016			
311(00101(E1(0:0010010			
SHEET 9 OF 20 SHEETS		_	

TOTAL SHEE NO. SECTION GREENE 35 24 CONTRACT NO. 76K57

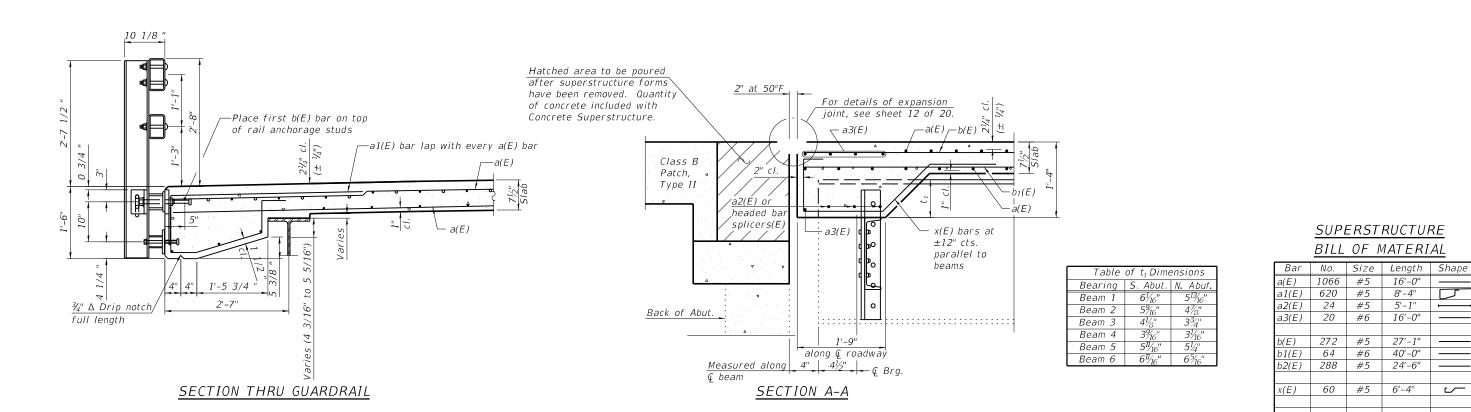








DIAPHRAGM AT ABUTMENT



Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated.

b2(E) 288 #5 24'-6" 6'-4"

8'-4"

5'-1"

16'-0"

Reinforcement Bars, Lbs. 43,080 Epoxy Coated Bar Splicers Each 549 Concrete 172 Superstructure

SUPERSTRUCTURE

BILL OF MATERIAL

1066 #5 16'-0"

620 #5

24 #5

Bars indicated thus 1 x 2-#8 etc. indicates 1 line of bars with 2 lengths per line.

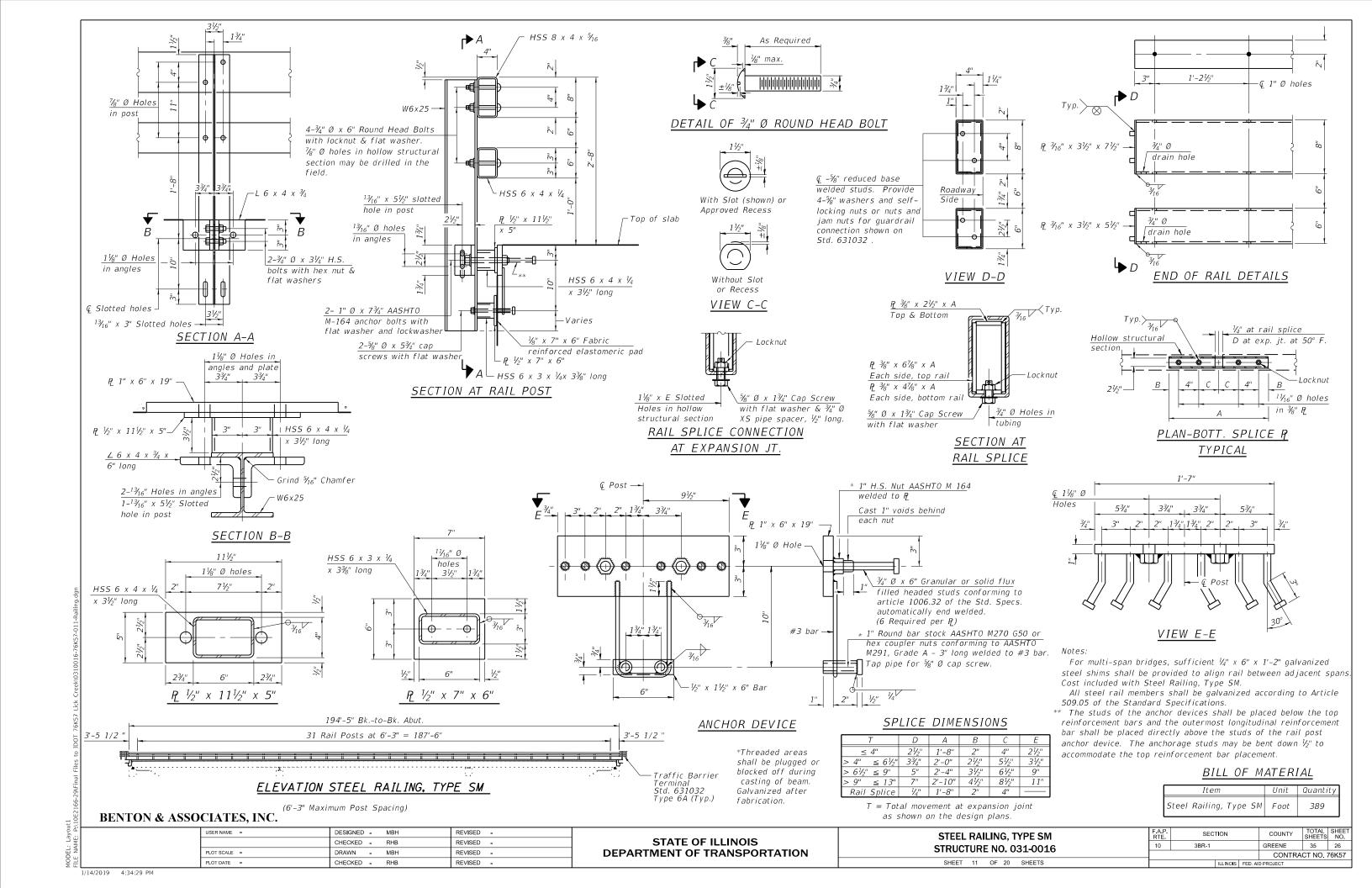
BENTON & ASSOCIATES, INC.

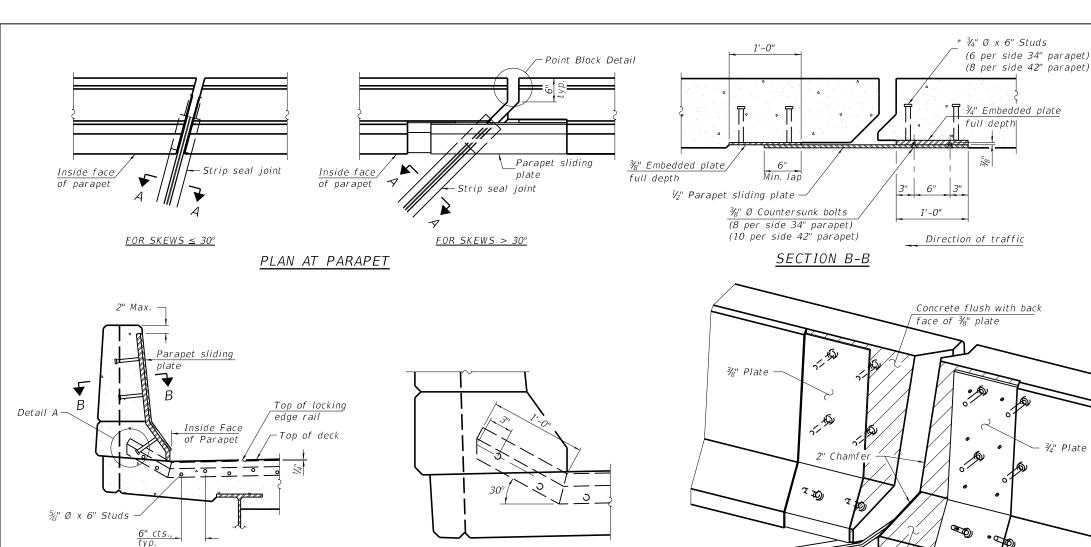
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		CHECKED - RHB	REVISED -
	PLOT SCALE =	DRAWN - MBH	REVISED -
	PLOT DATE =	CHECKED - RHB	REVISED -
TC			

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

				DETAILS 031-0016
SHEET	10	OF	20	SHEETS

SECTION COUNTY 3BR-1 GREENE 35 25 CONTRACT NO. 76K57





ELEVATION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

DETAIL A

Concrete flush with back Concrete flush with back face of ¾" plate

TRIMETRIC VIEW (Showing embedded plates only)

Locking edge rail at 50° F Top of concrete Strip seal at 50° F

SHOWING ROLLED RAIL JOINT

Locking edge railat 50° F Top of concrete -Strip seal * $\frac{1}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs)

%'' ϕ threaded rods in %'' ϕ holes at $\pm 4'$ -0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

SHOWING WELDED RAIL JOINT

<u>ROLLED</u> WELDED RAIL (EXTRUDED) RAIL

Notes:

rated movement of 4 inches.

shall be followed.

rail splice detail.

LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.

on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip

are not permitted. The gland shall be sized for a maximum

The locking edge rails depicted are configured for typical

applications and are conceptual only. The actual configuration

of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails,

however, will not be allowed. Locking edge rails may exceed the

4½" maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. The Maximum space between locking edge rail segments

shall be $\frac{3}{6}$ and sealed with a suitable sealant; however, any

Cost of parapet sliding plates, embedded plates, and

anchorage studs included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based

on the locking edge rail chosen by the Contractor. Deck and

parapet lengths shown elsewhere in the plans are dimensioned

to the concrete opening, not the joint opening, and are based

rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge

The manufacturer's recommended installation methods

according to the manufacturer's recommendation.

seal shall match the configuration of the locking edge

rails. Open or "webbed" strip seal gland configurations

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	65

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

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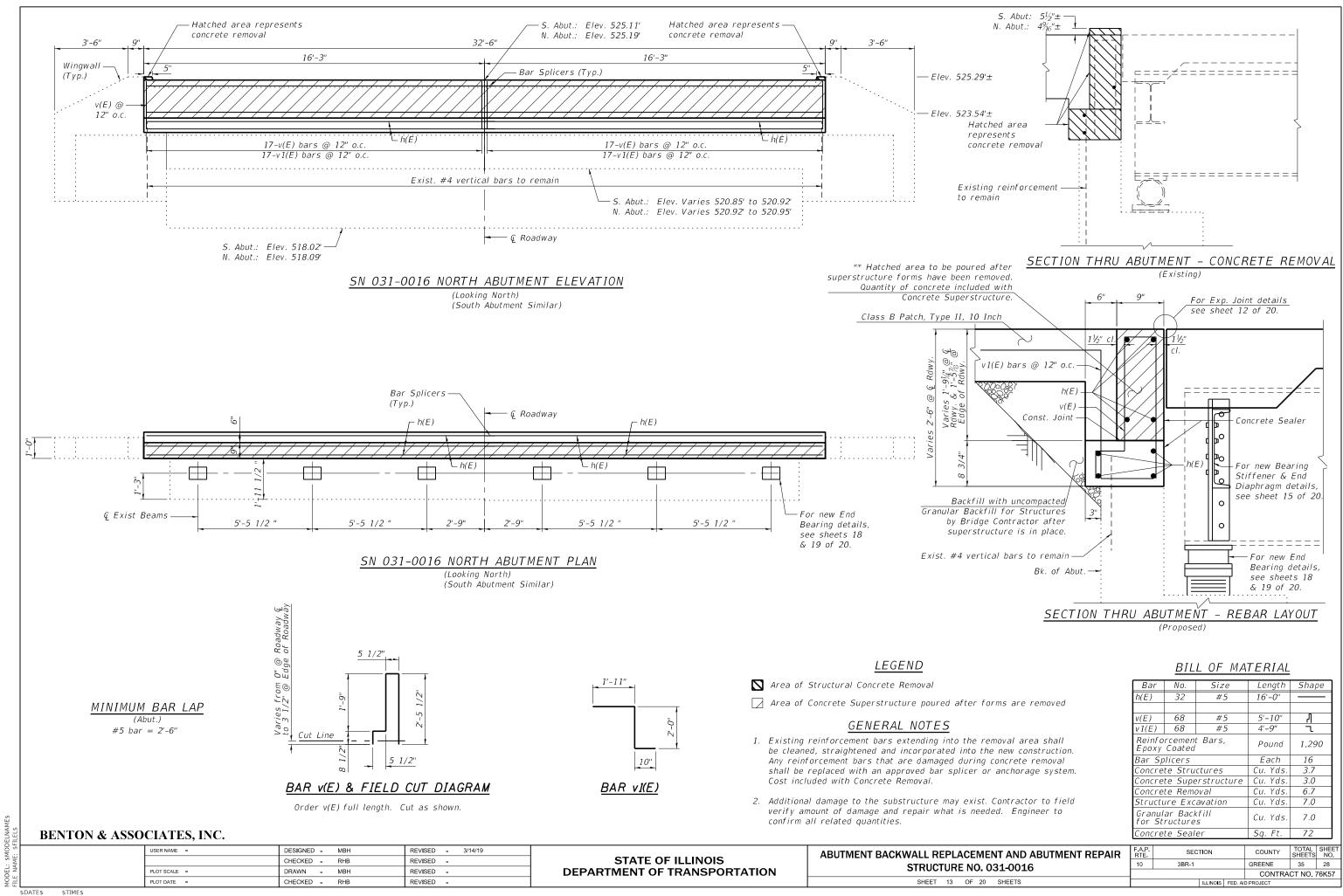
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PLOT SCALE =	DRAWN - MBH	REVISED -
PLOT DATE =	CHECKED - RHB	REVISED -

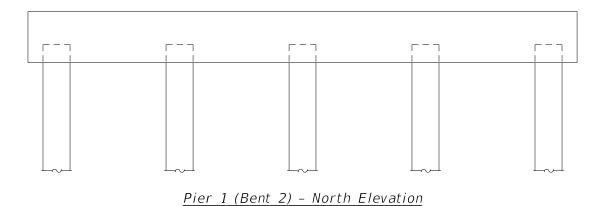
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

PREFORMED JOINT STRIP SEAL STRUCTURE NO. 031-0016						_
	SHEET	12	OF	20	SHEETS	

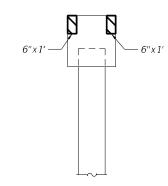
A.P. TE	SEC ⁻	TION		COUNTY	TOTAL SHEETS	SHEET NO.
0	3BR-1			GREENE	35	27
				CONTRA	CT NO.	76K57
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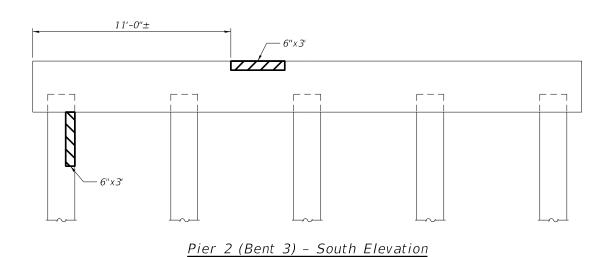




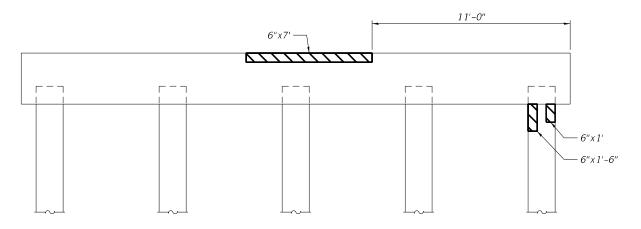
(Looking South)



<u>Pier 1 (Bent 2) - West Elevation</u> (Looking East)



(Looking North)



<u>Pier 2 (Bent 3) - North Elevation</u> (Looking South)

<u>LEGEND</u>

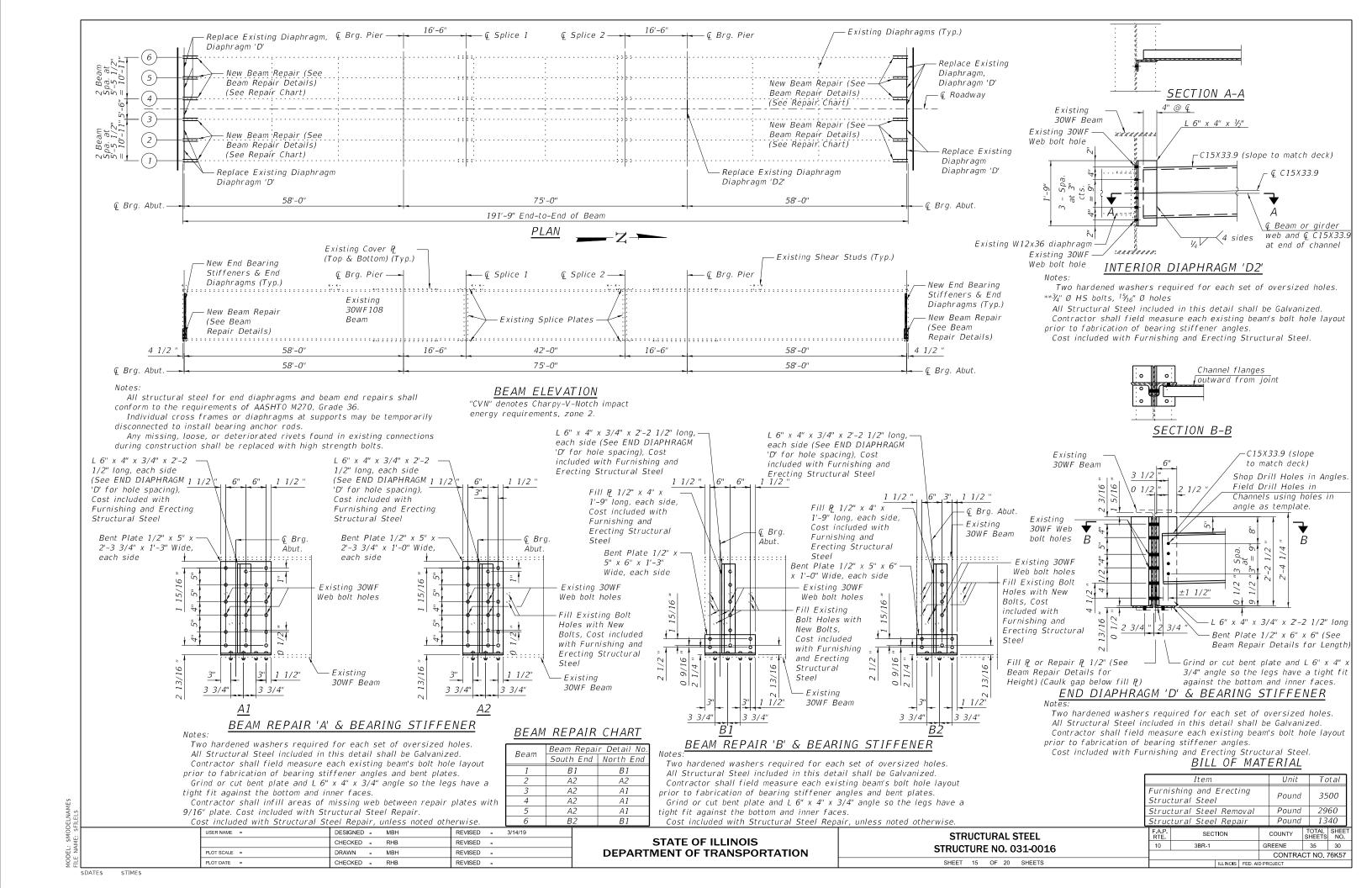
🛮 Area of Structural Concrete Repair

BILL OF MATERIAL

Item	Unit	Total
Structural Repair of Concrete (Depth <= 5")	Sq. Ft.	9

BENTON & ASSOCIATES, INC.

USER NAME =	DESIGNED - MBH	REVISED -		PIER REPAIR	F.A.P.	SECTION	COUNTY	TOTAL SHEET	1
	CHECKED - RHB	REVISED -	STATE OF ILLINOIS		10	3BR-1	GREENE	35 29	1
PLOT SCALE =	DRAWN - MBH	REVISED -	DEPARTMENT OF TRANSPORTATION	STRUCTURE NO. 031-0016	1.4			ACT NO. 76K57	1
PLOT DATE =	CHECKED - RHB	REVISED -		SHEET 14 OF 20 SHEETS	1	III INOIS EED A	ID DBO IECT		-



EXTERIOR GIRDER REACTION TABLE						
	Pier					
R₽	(k)	18.0	66.8			
R Ł	(k)	32.4	40.4			
Rı	(k)	8.9	11.0			
RTotal	(k)	59.3	118.2			

- * Compact section
- ** Braced non-compact and partially braced section

- Is, Ss: Non-composite moment of inertia and section modulus of the steel section used for computing fs(Total and Overload) due to non-composite dead loads (in.4 and in.3).
- Ic(n), Sc(n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing fs(Total and Overload) due to short-term composite live loads (in.⁴ and in.³).
- Ic(3n), Sc(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing fs(Total and Overload) due to long-term composite (superimposed) dead loads (in.4 and in.3).
 - Z: Plastic Section Modulus of the steel section in non-composite
 - ₽: Un-factored non-composite dead load (kips/ft.).
 - MP: Un-factored moment due to non-composite dead load (kip-ft.).
 - $s \, P$: Un-factored long-term composite (superimposed) dead load (kips/ft.)
 - M_S \cite{S} : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
 - M_{L} : Un-factored live load moment (kip-ft.).
 - MI: Un-factored moment due to impact (kip-ft.).
 - Ma: Factored design moment (kip-ft.). 1.3 [MP + MSP + $\frac{5}{3}$ (Mt + MI)]
 - Mu: Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according
- to AASHTO LFD 10.48.1 (kip-ft.). fs (Overload): Sum of stresses as computed from the moments below (ksi). $MP + MsP + \frac{5}{2}(ML + MI)$
- fs (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).
 - 1.3 $[MP + MSP + \frac{5}{3}(ML + MI)]$
 - VR: Maximum\{ + impact shear range within the composite portion of the span for stud shear connector design (kips).

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PLOT DATE =	CHECKED -	RHB	REVISED -

TOTAL SHEE NO.

35 31

BEAM REACTION TABLE

Expansion Bearing Reactions	S. Abut. (kips)	N. Abut. (kips
W30 Dead Load*	2.5	2.5

* Reactions for Dead Load are for dead load of steel only

JACK & REMOVE EXISTING BEARING PROCEDURE

- 1. Removal of existing deck.
- 2. Jacking existing superstructure. Jack capacity provided should be between 50% and 100% greater than maximum expected loading. For reaction table see above.
- 3. Remove bearings.
- 4. Jack existing beams to proposed position and complete construction.

JACKING EXISTING SUPERSTRUCTURE & REMOVING BEARING NOTES

- 1. Jacking existing superstructure shall be done after the existing deck is removed.
- 2. The Contractor shall submit plans for jacking the existing superstructure for approval by the Engineer prior to commencing any work with the bearings. The submittal shall be prepared and sealed by a Licensed Structural Engineer in Illinois.
- 3. It shall be the Contractor's responsibility to verify beam elevations before and after the beams are jacked.
- 4. The lifting of the structure shall be controlled so that the relative elevation between adjacent beams does not vary more than $\frac{1}{4}$ inch from their original elevation differential.
- 5. The relative elevations at adjacent substructure units shall not vary more than $\frac{3}{4}$ inch from the original relative elevations.
- 6. A synchronous lifting system shall be used to control and equalize individual jack pressures to insure that the superstructure is lifted uniformly without exceeding the above stated relative elevation differentials.
- 7. The jack capacity provided shall be between 50% and 100% greater than the maximum expected loading. For reaction table see above.
- 8. The diaphragms shall not be used as load carrying members in the jacking and cribbing system.
- 9. When jacks are placed directly under a beam, the jack shall be centered under the web and a steel plate shall be placed between the top of the jack and the bottom flange of the beam. When web stiffeners bearing on the bottom flange do not exist directly over the location of the jack under a steel beam, hardwood timbers shall be installed tightly between the top and bottom flange to prevent flange rotation. Steel stiffening angles shall be attached to the web of the beam when the beam web thickness is not adequate to carry the jacking load. Steel plates shall be placed under jacks bearing directly on the existing substructure to distribute the jacking load and prevent damage to the existing concrete.
- 10. Jacks shall be placed in a manner and in locations that will ensure that the jacks will be equally loaded and the load will be uniformly distributed to the foundation of the jacking system.
- 11. The following maximum allowable pressures shall be used to determine the area of the timber mats supporting jacking systems.

Supporting Material Max. Allowable Pressure
Natural Ground (Unsaturated)..............0.5 tons/sq. ft.
Conc. Slopewalls & Bit. Shoulders......1.0 tons/sq. ft.
Bituminous Pavements................2.0 tons/sq. ft.
Concrete Pavements............4.0 tons/sq. ft.

BENTON & ASSOCIATES, INC.

Existing Bearing Assembly —

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	CHECKED -	RHB	REVISED -
PLOT SCALE =	DRAWN -	MBH	REVISED -
PLOT DATE =	CHECKED -	RHB	REVISED -

-— € Brg.

EXISTING BEARING REMOVAL DETAIL

Prior to ordering any material the Contractor shall verify in the field all bearing height and shim thickness dimensions.

<u>Ex</u>._W<u>30</u>__

— Existing Plate to be removed using the air-arc method and grind smooth all weld material

Burn existing anchor bolts flush with existing concrete surface.

Grind existing anchor bolt smooth and seal with epoxy.

Cost is included with "Jack and Remove Existing Bearings".

remaining on the bottom flange.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BEARING REMOVAL DETAILS STRUCTURE NO. 031-0016

Unit

Each

Total

12

BILL OF MATERIAL

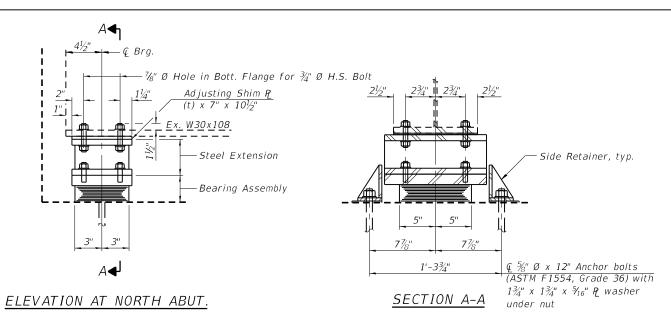
Item

Jack and Remove

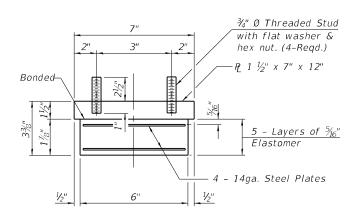
Existing Bearings

\10E2166-29\Final Files to IDOT 76K57 Lick Creek\0310016-76K57-017-Removal.dgn

ODEL: Layout1 LE NAME: P:\10E2166-2



TYPE I ELASTOMERIC EXP. BRG.



BEARING ASSEMBLY

Note:

Shim plates shall not be placed under Bearing Assembly.

lotes:

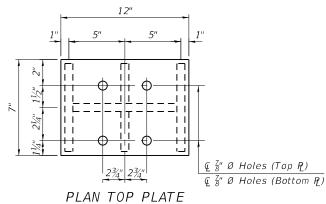
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

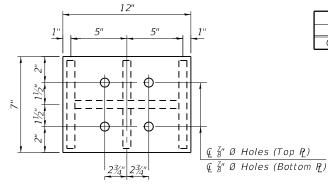
Two $\frac{1}{8}$ in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

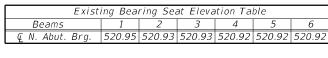
Prior to ordering any material the Contractor shall verify in the field all bearing height and shim thickness dimensions.

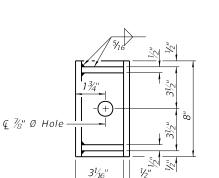


Bearing Properties					
Dimensions	а	b	С		
ℚ N. Abut. Brg.	31/8"	5½"	3½"		

Bearing Shim P_ Thickness - t (inches)						
Beams 1 2 3 4 5 6						
© N. Abut. Brg. 1/8" 58" 58" 0 11/16" 15/16"						

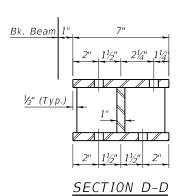




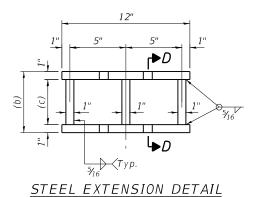


SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



<u>PLAN BOTTOM PLATE</u>



BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	6
Anchor Bolts, 5/8"	Each	12
Furnishing and Erecting Structural Steel	Pound	440

BENTON & ASSOCIATES, INC.

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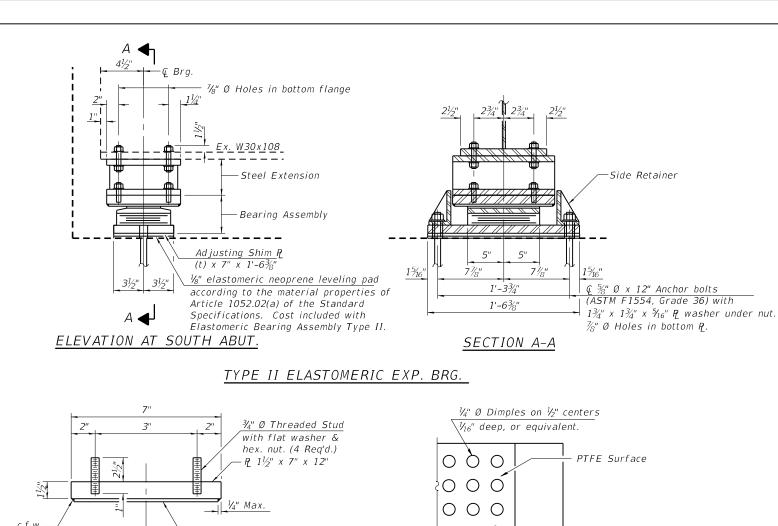
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

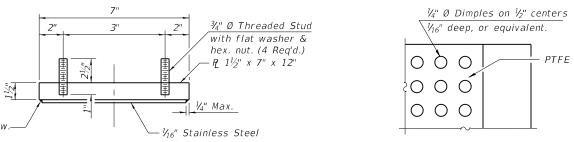
BEARING DETAILS	F.A.P. RTE	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 031-0016	10	3BR-1			GREENE	35	33
311001011E 110: 031-0010					CONTRA	CT NO.	76K57
SHEET 18 OF 20 SHEETS			ILLINOIS	FED AL	D PROJECT		

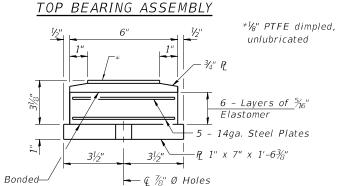
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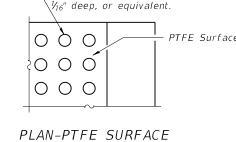
1/4" Stainless steel plate, A240, Type 304,

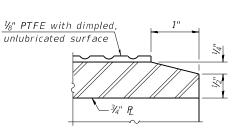
No. 1 finish



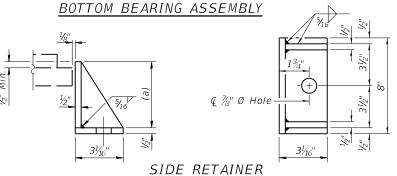




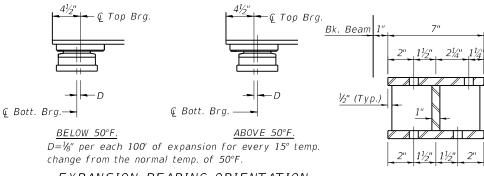








Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



EXPANSION BEARING ORIENTATION

The above diagrams are for informational purposes only to show the amount of expected offset "D" for the current temperature in the field.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION D-D

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type II.

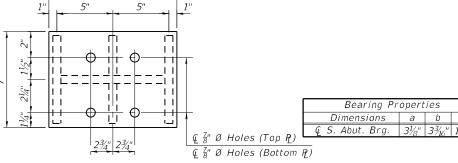
The $\frac{1}{8}$ " PTFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.

Bonding of 1/8" PTFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.

Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

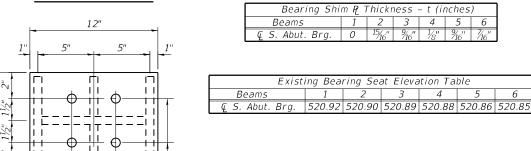
Two $rac{1}{8}$ in. adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

Prior to ordering any material the Contractor shall verify in the field all bearing height and shim thickness dimensions.



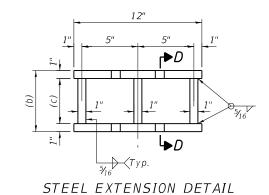
PLAN TOP PLATE

12"



 $Q \frac{Z}{8}$ Ø Holes (Top P_2) \mathcal{Q}_{8}^{7} Ø Holes (Bottom P₂)

PLAN BOTTOM PLATE



BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type II	Each	6
Anchor Bolts, 5/8"	Each	12
Furnishing and Erecting Structural Steel	Pound	340

BENTON & ASSOCIATES, INC.

	USER NAME =	DESIGNED - MBH	REVISED -
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	PLOT SCALE =	DRAWN - MBH	REVISED -
	PLOT DATE =	CHECKED - RHB	REVISED -

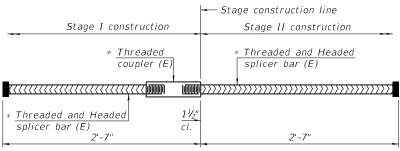
TOTAL SHEE NO. COUNTY **BEARING DETAILS** 3BR-1 GREENE 35 34 **STRUCTURE NO. 031-0016** CONTRACT NO. 76K57 SHEET 19 OF 20 SHEETS

STANDARD BAR SPLICER ASSEMBLY

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar	No. assemblies	Minimum
Location	size	required	lap length
Superstructure	#5	533	3'-6"
Superstructure	#6	10	4'-2"
Abutments	#5	16	2'-6"

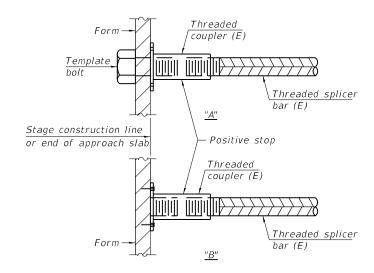


HEADED BAR SPLICER ASSEMBLY

Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706.

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

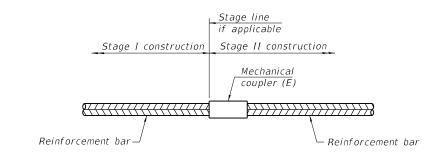
Location	Bar	No. assemblies
	size	required
Superstructure	#5	6



INSTALLATION AND SETTING METHODS

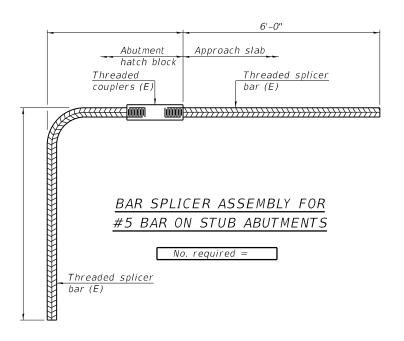
(E): Indicates epoxy coating.

"A" : Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BENTON & ASSOCIATES, INC.

USER NAME =	DESIGNED - MBH	REVISED -	
	CHECKED - RHB	REVISED -	
PLOT SCALE =	DRAWN - MBH	REVISED -	
PLOT DATE =	CHECKED - RHB	REVISED -	

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS 3BR-1 **STRUCTURE NO. 031-0016** SHEET 20 OF 20 SHEETS

COUNTY GREENE 35 35 CONTRACT NO. 76K57