DESCRIPTION

STATE OF ILLINOIS STANDARDS:

NUMBER

1401010111	
280001-04	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS TEMPORARY EROSION CONTROL SYSTEMS PAVEMENT JOINTS
420101-04	24' (7-2m) JOINTED PCC PAVEMENT
420401-06	BRIDGE APPROACH PAVEMENT
420601-05	24' (7.2m) PAVEMENT
421001-02	BAR REINFORCEMENT FOR CRC PAVEMENT
515001-02	NAME PLATE FOR BRIDGES
542401	METAL END SECTION FOR PIPE CULVERTS
604051-02	FRAME AND GRATE TYPE 11
606001-03	CONCRETE CURB TYPE B AND COMBINATION
	CONCRETE CURB AND GUTTER
630001-07	STEEL PLATE BEAM GUARDRAIL
631026-04	TRAFFIC BARRIER TERMINAL, TYPE 5
664001-01	CHAIN LINK FENCE
667101	PERMANENT SURVEY MARKERS
701901	TRAFFIC CONTROL DEVICES
	TYPICAL PAVEMENT MARKINGS
720001	

REGULATORY SPEED LIMITS TRAFFIC DATA (ADT)

DESIGN DESIGNATION

DESCRIPTION OF BRIDGE REHABILITATION: OLD ELM ROAD BRIDGE-STRUCTURE NO. 049-6870

REPLACEMENT OF AN EXISTING T-BEAM BRIDGE (049-6863) WITH ASSOCIATED SITE WORK. THE WORK INCLUDES REMOVAL OF THE EXISTING BRIDGE AND FOUNDATION, INSTALLATION OF NEW CONCRETE SHAFTS AND CAP, CONSTRUCTING NEW ABUTMENT AND WINGWALLS INSTALLING A NEW SEMI-INTEGRAL STEEL BEAM BRIDGE INSTALLING AN AESTHETIC BRIDGE RAILING AND SIDEWALK TEXTURING, INSTALLING STONE RIP-RAP SLOPE PROTECTION, AND RECONSTRUCTING THE ROADWAY AND SIDEWALK APPROACHES TO THE BRIDGE. THE PROJECT ALSO INCLUDES STORM AND SANITARY SEWER WORK.

FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES, REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

Design/MICROSTATION-F P.E. (847)705-4169

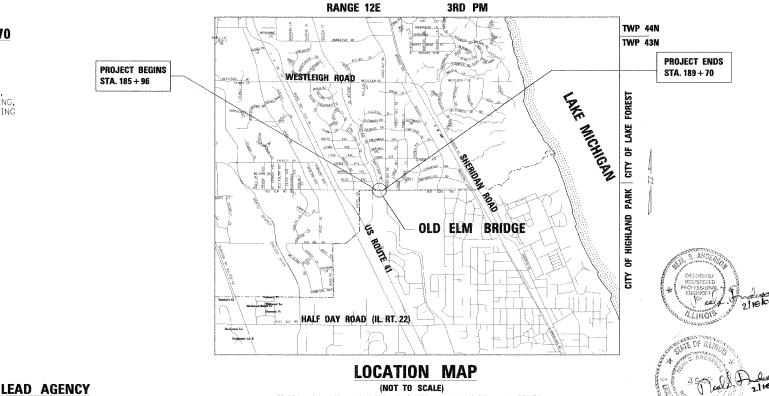
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123

CONTRACT NO. 63020

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS**

PLANS FOR PROPOSED FEDERAL AID URBAN PROJECT

F.A.U. ROUTE 1248 OLD ELM ROAD BRIDGE OVER THE SKOKIE RIVER SECTION 99-00080-00-BR **PROJECT BRM-8003(103)** CITY OF LAKE FOREST CITY OF HIGHLAND PARK LAKE COUNTY JOB NO. C-91-405-01



GROSS AND NET LENGTH OF PROJECT = 374 LIN. FT = .071 MILES

THE CITY OF

LAKE FOREST

847-234-2600

LAKE FOREST

110 EAST LAUREL

LAKE FOREST, IL. 60045

Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Pfingsten Road, Northbrook, II 60062 (847) 272-7400. Fax: (847) 291-4813

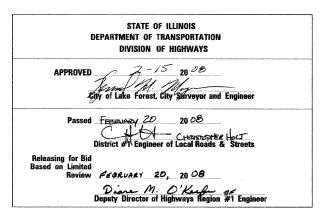
NEAL S. ANDERSON, PE, SE ILLINOIS LICENSED PROFESSIONAL ENGINEER NO. 062-045757 EXPIRES 11/30/2008

ILLINOIS LICENSED STRUCTURAL ENGINEER EXPIRES 11/30/2009

RTE. SECTION COUNTY TOTAL SHEETS 1248 99-00080-00-BR LAKE 46

CONTRACT NO. 63020





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INDEX	0F	SHEETS
NUMBE	R	DESCRIPTION

TITLE SHEET INDEX OF SHEETS GENERAL NOTES SUMMARY OF QUANTITIES TEMPORARY STREAM DIVERSION PLAN AND ELEVATION DETOUR PLAN AND SIGNAGE BENCH MARK AND ALIGNMENT TIES PROPOSED PLAN & PROFILE STA. 185+96 TO STA. 189+70 EXISTING AND PROPOSED TYPICAL SECTIONS EXISTING AND PROPOSED DRAINAGE AND UTILITY PLAN SIDEWALK PLAN SIDEWALK AND PRIVATE ENTRANCE DETAILS GENERAL PLAN & ELEVATION GENERAL NOTES & TOTAL BILL OF MATERIAL 14. TYPICAL CROSS SECTION DRILLED SHAFT CAP LAYOUT TOP OF SLAB ELEVATION LOCATIONS TOP OF SLAB ELEVATION 19. DECK SLAB DETAILS TYPICAL CROSS SECTION PARAPET PLAN & ELEVATION 21. PARAPET PRECAST CONCRETE CAP PLAN & ELEVATION 22. ABUTMENT DIAPHRAGM DETAILS SUPERSTRUCTURE DETAILS AND BILL OF MATERIAL FRAMING PLAN 26. BEAM ELEVATION 27. BEARING DETAILS

CANTILEVER FORMING BRACKETS 40. SOIL BORING LOGS-I SOIL BORING LOGS-II 41. PLAN BRIDGE APPROACH PAVEMENT (SPECIAL) 42.

36.

37.

ANCHOR BOLT DETAILS FOR BEARINGS EAST ABUTMENT DRILLED SHAFT CAP PLAN EAST ABUTMENT STEM WALL PLAN EAST ABUTMENT ELEVATION

EAST ABUTMENT & WINGWALL DETAILS WEST ABUTMENT DRILLED SHAFT CAP PLAN WEST ABUTMENT STEM WALL PLAN WEST ABUTMENT STEM WALL ELEVATION

WEST ABUTMENT & WINGWALL DETAILS

BRIDGE APPROACH PAVEMENT (SPECIAL) SECTIONS & DETAILS 43.

SUBSTRUCTURE BILL OF MATERIAL & DETAILS

ABUTMENTS & WINGWALLS DRILLED SHAFT DETAILS

- BRIDGE APPROACH PAVEMENT JOINT SYSTEM-EXPANSION
- EXISTING & PROPOSED CROSS SECTION STA. 185+00 TO STA. 188+50.00 EXISTING & PROPOSED CROSS SECTION STA. 189+00.00 TO STA. 190+00.00



Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Pfingsten Road, Northbrook, Il 60062 (647) 272-7400, Fax: (647) 291-4613 WJE No. 2006.2324



THE CITY OF LAKE FOREST CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

<u> </u>				-	OLD FLM ROAL	O OVER EAST SKOKIE DITCH
					TITLE:	
					IND	EX OF SHEETS
					SCALE: N.T.S	DRAWN BY: IMG
REV. NO.	DATE	REVISION RECORD	MADE BY	CHECKED BY	DATE: JANUARY 2008	CHECKED BY: NSA

2. It shall be the Contractor's responsibility to verify all dimensions and conditions existing in the field prior to construction and ordering materials.

3. Any reference to standards in the plans or special provisions shall be interpreted to be the latest Standard Specifications of the Department listed in the plans with the latest revision numbers.

4. Before starting any excavation work, the Contractor shall call "JULIE" at 800-892-0123 for field locations of buried electric, telephone, cable and gas facilities (48 hours notification is required).

5. The City of Lake Forest Department of Public Works shall be notified at least 48 hours prior to commencing construction.

6. The Contractor shall obtain a construction permit from the Illinois Department of Natural Resources (IDNR), Office of Water Resources for any temporary construction activity placed in the water except cofferdams. This shall include the placement of material for run-arounds, causeways, etc. Any permit application by the Contractor shall refer to the IDNR permit number which was issued for the permanent construction.

7. All utilities, school districts, local police and fire departments should be notified by the Contractor prior to the start of the construction.

8. The Contractor will be allowed to set up a yard and/or field office at the west end of the project limits on the closed portion of Old Elm Road. The Engineer's field office will be paid for under the contract unit price per calendar month for ENGINEER'S FIFLD OFFICE TYPE A

9. The Contractor shall provide traffic control in conformance with the "Manual on Uniform Traffic Control Devices for Streets and Highways," State of Illinois, and Section 107.14 of the Standard Specifications. Barricades and other required traffic control will be paid under the lump sum price for TRAFFIC CONTROL AND compressive strength (fc) of 4,000 psi at 28 days. The concrete mix shall have PROTECTION

10. No work shall commence until the detour route is established and traffic control requirements are met.

11. The Contractor shall limit his construction activities to the work areas designated in the job plans. The Contractor at his own expense shall repair any damage to areas outside of these limits to the satisfaction of the Engineer. 12. Special care shall be taken in excavating the grading near utilities and

trees to be saved in order to avoid unnecessary damage 13. Silt fence shall be installed at low points adjacent to the river, downslope

from areas susceptible to erosion during construction. 14. Layout of slope protection system may be varied in the field to suit ground

conditions as directed by the Engineer. 15. It is the Contractor's responsibility to insure pipe outlets from all catch. basins are free from blockages and debris. The storm sewer drainage flow on the east abutment pipe shall be maintained during the project duration.

16. During construction operations, loose material deposits that obstruct the flow of water in the river channel shall be removed before the end of each workday. At the conclusion of construction operations, all drainage structures shall be free from all dirt and debris. This work will not be paid for separately but shall be included in the contract.

17. All trees with diameters larger than six-inches (6") shall have their trunks protected from construction activity.

18. The Contractor shall protect all identified trees during construction. At the Contractor's expense, all broken tree limbs over one-inch (1") in diameter shall be made flush with the next large branch. All limbs, branches, scrub brush and other debris shall be disposed of by the Contractor outside the limits of the project.

19. All existing grass areas disturbed by the Contractor shall be reseeded and protected from erosion with erosion control blankets.

20. During the construction, the contractor will be required, at his expense, to have available a water truck or similar equipment to control dust. If necessary the Contractor shall be required to control dust during non-working hours.

21. The Contractor shall make all reasonable and necessary attempts to open Old otherwise Elm prior to the required completion date. Several work activities can be and are expected to be finished under traffic (notified of reduced speed) using daily shoulder closures or occasional flagmen. Representative work activities include but are not limited to topsoil placement, seeding, finish grading outside the railing walls and miscellaneous site work. Daily traffic control, if used, shall be incidental to the TRAFFIC CONTROL pay item.

Utility Coordination

1. Utilities in the vicinity of the project are shown on Sheets 8 and 12 of this plan set. The Contractor shall be responsible for coordinating his construction activities with the appropriate utility company.

2. The location and elevation of existing utilities are approximate and are provided by the owners. The exact locations and elevations are to be verified by the Contractor through the owner of the utility.

3. The Contractor shall maintain pipe flow throughout the course of removal and replacement of sanitary sewer.

4. Existing sanitary shall be removed and replaced using auger pits. No open excavation shall be performed in the channel for the sanitary sewer work.

Demolition Notes

1. The Contractor shall exercise extreme care with demolition activities on the existing foundation. Vibrations from demolition of concrete in direct contact with the ground may be too excessive for the buried North Shore Sanitary District line and shall be avoided. Damage to the 42 in. diameter concrete pipe due to demolition or demolition vibrations shall be the Contractor's sole responsibility. Costs for pipe repair and effluent clean-up shall be borne by the Contractor, at his expense

2. The Contractor may elect to sawcut or otherwise partition the existing concrete footings, abutments and wingwalls for removal from the excavation area. Concrete break-up and disposal would be performed away from the existing utilities to avoid damage. The work activity will not be paid for separately but shall be considered incidental to the concrete removal cost.

3. The extent of the demolition work is as shown on, and reasonably inferable from, the drawings.

4. Provide for off-site disposal of all demolished materials.

Concrete General Notes

1. Reinforcing bars shall conform to the requirements of ASTM A706 Gr. 60. See special provisions

2. Reinforcing bars designed (E) shall be epoxy-coated. Damage to the epoxy coating during handling, placement, etc. shall be repaired with a compatible

3. All accessories including bolsters, chairs, tie wire, etc. used to tie or support the epoxy-coated bars shall be epoxy-coated.

4. Structural Concrete: Concrete shall be IDOT Class SI having a minimum an air content between 5 and 8 percent of the volume of the concrete. The Contractor shall submit concrete mix designs to the Engineer for approval.

5. The concrete for bridge decks finished according to Article 503.16(a) of the Standard Specifications shall be placed and compacted parallel to the skew in uniform increments along centerline of bridge. The machine used for finishing shall be set parallel to the skew for striking off and screeding.

6. All construction joints shall be bonded.

7. The Contractor shall make allowance for the lateral deflection of forms, shrinkage and settlement of falsework or braces, in addition to allowance for deadlock deflection. Forms for deck slabs shall be removed prior to placement of bridge approach pavement.

8. Design and construction of formwork shall be the responsibility of the Contractor and shall be performed with accordance with ACI 347 and the Standard Specifications.

9. The Contractor shall use cantilever forming brackets on the exterior beams or girders, and brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06 of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations

10. The Contractor shall use a form-lined textured surface when casting the walls for this project. The selected textured surface, approved by the Engineer, shall match as close as possible to a random ashlar limestone masonry pattern. 11. The form-lined textured surface shall be carried a minimum of 2 ft. below

the proposed finished grade elevation or as indicated on the drawings. At vertical wingwall and abutment surfaces facing the river, the form lined textured surface shall be as shown on the drawings.

12. The back face of the wingwalls and retaining walls shall be waterproofed. according to 503.18 of the Standard Specifications

Steel Notes

1. All structural steel members shall be AASHTO M270 Grade 50, unless noted

2. Stainless steel shall conform to ASTM A276 Type 316.

3. Load carrying member components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.

4. The structural steel bearing plates of the Elastomeric Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50.

5. All fasteners shall be high strength bolts AASHTO M164 Type 3, ASTM A325, unless noted otherwise. All nuts shall be type DH and coated with a visible lubricant

6. Calculated weight of structural steel = 27.500 lbs.

7. All welding shall be performed by certified welders and shall comply with the latest edition of the bridge Welding Code ANSI/AASHTO/AWS D1.5. 8. Welding electrodes shall be E70XX.

9. Field welding of construction accessories will not be permitted to beams or girders, except as specified in the Contract Documents.

10. All cross frames and diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.

Hot Dip Galvanizing

nuts, and washers shall be galvanized according to AASHTO M111 or M232 (as

2. Safeguard products against steel embrittlement in conformance with ASTM 143. 3. Handle all articles to be galvanized in such a manner as to avoid any

mechanical damage and to avoid distortion.

4. Repair of damaged coating:

4.1. The maximum area to be repaired is defined in accordance with ASTM 123 Section 4.6 current edition.

4.2. Repair areas damaged by welding, flame cutting or during handling, transport or erection by one of the approved methods in accordance with ASTM A780 whenever damage exceeds 3/16" in width. Minimum thickness requirements for the repair are those described in ASTM A123 Section 4.6 current edition.

to the steel cost. Moisture Protection

1. Construction joints designated on the drawings will be filled with a 2component sealant supported by closed cell form backer rod. The joint sealant is specified as follows:

5. Cost incurred for touch-up cold galvanizing all new steel shall be incidental

1.1. Silicone Sealant - Use for buried joints or where long term water exposure is anticipated.

1.2. Polyurethane Sealant - Architectural joint sealant for exposed-to-view surfaces. Color shall match the substrate

2. Refer to Special Provisions for any additional information on joint sealant.

3. Joint sealant to be used in horizontal, nearly level applications shall be self-leveling (SL) formulation. Joints to receive the self-leveling sealant shall be identified by the Engineer in the field.

4. Joint sealant shall be applied as specified in the construction plans and as determined by the Engineer

5. Designated concrete surface shall be coated with a silane surface sealer. The sealer shall be isobutyl trimethoxy silane in a 40% solids solution of anhydrous isopropyl alcohol. Surfaces to receive silane include the bridge deck. concrete sidewalk, bridge approach pavement, gutter sections and other Engineer designated surfaces.

6. The silane surface sealer shall be considered an alternate to the Department's protective coat requirements for concrete cast in the fall.

Sedimentation and Erosion Control Notes

1. Soil disturbances shall be conducted in such a manner as to minimize erosion. Soil stabilization measures shall consider the time of year, site conditions and the use of temporary or permanent measures.

2. Soil erosion and sediment control features shall be constructed prior to the commencement of hydrologic disturbance of upland areas.

3. Disturbed areas shall be stabilized with temporary or permanent measures within 14 calendar days of the end of active hydrologic disturbance or redistribution. 4. All temporary erosion and sediment control measures shall be removed within

30 days after final site stabilization is achieved or after the temporary measures are no longer needed.

5. All temporary and permanent erosion control measures must be maintained and repaired as needed.

6. Soil stockpiles shall not be located in a flood prone area or a designated Buffer Protecting Waters of the United States or Isolated waters of Lake County.

7. The erosion control measures indicated on the plans are the minimum requirements. Additional measures may be required, as directed by the Engineer or Governing Agency

Expansion Joint Seal

direction of stationing.

1 Perform all shop welding in accordance with the Bridge Welding Code ANSI/AASHTO/AWS D1.5. Do not weld to surfaces in contact with the elastomeric seal or the top surface (riding surface) except as shown in the shop splice detail. Do not weld inside seal cavity.

2. Fabricate edge rail assemblies in one piece including upturns, except where the length or configuration prohibits shipping or proper installation or where phase construction requires separate assemblies. Shop splice sections of edge rail to obtain the required length by partial penetration double v-groove welds on prepared beveled edges and seal welds. Weld all around the joint as far as practical to achieve a watertight seal. Do not use short pieces of edge rail less than 6 ft - 0 in. long unless required at curbs, sidewalks or phase construction locations

3. Hot-dip galvanize (HDG) edge rail assemblies after shop fabrication in accordance with the Special Provisions and the manufacturer's recommendations. 4. Furnish temporary or sacrificial support brackets, bolts, clamps, etc. that are capable of resisting shipping, handling and construction forces without damage to the edge rail assemblies or galvanized coating and are adjustable to account for variable temperature settings. Do not use temporary or sacrificial support brackets, bolts, clamps, etc. between the faces of the edge rails. 5. Clearly match mark corresponding edge rail assemblies with joint location and

1. All structural steel members, bearing plates, side retainers, bolts.

6. Submit shop drawings showing all joint materials and project specific details and dimensions. Include name of manufacturer, seal model number, and seal

> movement range 7. After galvanizing has been completed, do not weld within 2 in. of edge rail surfaces that will be exposed in the completed structure. Do not weld expansion joint components to or electrically ground to reinforcing steel or structural steel. Seal field butt joints and empty shipping and erection holes with caulk

8. Protect galvanized edge rail assemblies during screeding operations per the manufacturer's recommendations. Provide temporary blocking material in the edge rail seal cavities to prevent concrete intrusion during approach slab placement and finishing.

9. Loosen any temporary or sacrificial support brackets, bolts, clamps, etc. that span across the joint after initial set of concrete, but not later than two hours after conclusion of concrete placement.

10. Install elastomeric seal after completion of approach slab casting. Remove all joint form material and blocking material prior to installing elastomeric seal. Field install elastomeric seal in accordance with manufacturer's recommendations. Thoroughly coat all contact surfaces between the elastomeric seal and the edge rail seal cavities with an adhesive lubricant before setting elastomeric seal in place.

Reference Standards

before placing deck concrete.

1. Illinois Department of Transportation (IDOT)

1.1. Standard Specifications for Road and Bridge Construction (Adopted January 1, 2007)

1.2. Supplemental Specifications and Recurring Special Provisions (Adopted January 1, 2008)

1.3 Highway Standards

2. American Association of State Highway and Transportation Officials (AASHTO)

2.1. Standard Specifications for Highway Bridges (LRFD) 2.2. Standard Specifications for Transportation Materials and Methods of

Sampling and Testing

2.3. Bridge Welding Code (ANSI/AASHTO/AWS D1.5)

2.4. M111 Standard Specifications for Zinc (Hot Dip Galvanized) Coating on Iron

2.5. M232 Standard Specification for Zinc Coating (Hot Dip) on Iron and Steel Hardware

3. American Society for Testing and Materials (ASTM)

3.1. A123 Standard Specifications for Zinc (Hot Dip Galvanized) Coating on Iron

3.2. A143 Standard Practice for Safeguarding Against Embrittlement of Hot Dip Galvanized Structural Steel Products and Procedure for Detecting Embrittlement 3.3. A153 Standard Specification for Zinc Coating (Hot Dip) on Iron and Steel

3.4. A384 Standard Recommended Practice for Safeguarding Against Warpage and Distortion During Hot Dip Galvanizing of Steel Assemblies

3.5. A385 Standard practice for Providing High Quality Zinc Coatings (Hot Dip) 3.6. A780 Standard Practice for Repair of Damaged Hot Dip Galvanized Coatings

4 City of Lake Forest

4.1. Engineering and Construction Standards

REVISION RECORD

DATE



Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Prinsten Road, Northbrook, II 60062 (847) 272-7400, Fax: (847) 291-4813 W.IF No. 2006 2324



THE CITY OF LAKE FOREST CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

SECTION

1248 99-00080-00-BR

STA. 185+96

COUNTY

LAKE

TO STA. 189+70

FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

63020

46

OLD ELM ROAD OVER EAST SKOKIE DITCH

GENERAL NOTES

DRAWN BY: IMG DATE: JANUARY 2008 CHECKED BY: NSA

CALE: N.T.S MADE CHECKED

TOM-2A VO71-2A

			I000-2A	X071-2A	
CODE NUMBER	PAY ITEM	UNIT	ROADWAY	BRIDGE	TOTAL
20100110	TREE REMOVAL (6 TO 15 UNITS DIAMETER)	Unit	110		110
20101100	TREE TRUNK PROTECTION	Each	20		20
20201200		Cu. Yd.			+
	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL	Cu.Yd.		1180	1180
20300100	CHANNEL EXCAVATION		180		180
20700400	POROUS GRANULAR EMBANKMENT (SPECIAL)	Cu. Yd.		110	110
21001000	GEOTECHNICAL FABRIC FOR GROUND STABILIZATION	Sq. Yd.	250	No. 100	250
21101615	TOPSOIL FURNISH AND PLACE, 4"	Sq. Yd.	750		750
25000210	SEEDING, CLASS 2A	Acre	0.2		0.2
25100630	EROSION CONTROL BLANKET	Sq. Yd.	1000		1000
25200200	SUPPLEMENTAL WATERING	Unit	10		10
28000900	FENCE (EROSION CONTROL)	Foot	150		150
28100107	STONE RIPRAP, CLASS A4	Sq. Yd.		425	425
28200200	FILTER FABRIC	Sq. Yd.	T	425	425
31200100	STABILIZED SUB-BASE 4"	Sq. Yd.	405		405
40600100	BITUMINOUS MATERIALS (PRIME COAT)	Gal	220		220
40603335	HOT-MIX ASPHALT SURFACE COURSE, MIX "D", N50	Ton	100		100
42001400	BRIDGE APPROACH PAVEMENT (SPECIAL)	Sa. Yd.	245		245
42100100	CONT. REINF. PORTLAND CEMENT CONC. PAVEMENT 8"	Sq. Yd.			
		Sq. Yd.	135	-	135
42100700	PAVEMENT REINFORCEMENT 8"	1 "	135		135
42300400	PORTLAND CEMENT CONCRETE DRIVEWAY PAVEMENT, 8"	Sq. Yd.	40		40
42400430	PORTLAND CEMENT CONCRETE SIDEWALK, 5" (SPECIAL)	Sq. Ft.	520		520
44000100	PAVEMENT REMOVAL	Sq. Yd.	330		330
44000161	HOT MIX ASPHALT REMOVAL 3"	Sq. Yd.	725		725
44000200	DRIVEWAY PAVEMENT REMOVAL	Sq. Yd.	40		40
44000500	COMBINATION CURB AND GUTTER REMOVAL	Foot	680		680
44000600	SIDEWALK REMOVAL	Sq. Ft.	660		660
50100100	REMOVAL OF EXISTING STRUCTURES	Each		1	1
50200100	STRUCTURE EXCAVATION	Cu.Yd.		125	125
50300100	FLOOR DRAINS	Each		4	4
50300225	CONCRETE STRUCTURES	Cu. Yd.		205	205
50300255	CONCRETE SUPERSTRUCTURES	Cu. Yd.		90	90
50401205	PRECAST CONCRETE CAPS	Foot		82	82
XX007358	FURNISHING AND ERECTING STRUCTURAL STEEL (GALVANIZED)	L Sum		1	1
50500505	STUD SHEAR CONNECTORS	Each	Marie Andrews	880	880
50800105	REINFORCEMENT BARS	Pound			24785
50800103		Pound		24785	
	REINFORCEMENT BARS, EPOXY COATED	Each		63640	63640
51500100	NAME PLATES	Cu. Yd.		1	1
51603000	DRILLED SHAFT IN SOIL			130	130
52100010	ELASTOMERIC BEARING ASSEMBLY TYPE 1	Each		16	16
521005 2 0	ANCHOR BOLTS, 1"	Each		32	32
54200457	PIPE CULVERTS, TYPE 1 RCCP 42"	Foot	15		15
5421A036	PIPE CULVERTS, CLASS A, TYPE 1 36" (TEMPORARY)"	Foot	159		159
54213891	STEEL END SECTIONS 36"	Each	6		6
55101800	STORM SEWER REMOVAL 42"	Foot	15		15
59100100	GEOCOMPOSITE WALL DRAIN	Sq. Yd.		175	175
60109582	PIPE UNDERDRAINS FOR STRUCTURES 6"	Foot		140	140
60236800	INLETS, TYPE A, TYPE 11 FRAME AND GRATE	Each	2		2
60255 6 00	MANHOLES TO BE ADJUSTED (SPECIAL)	Each	2		2
60260200	INLETS TO BE ADJUSTED (SPECIAL)	Each	2		2
60603800	COMBINATION CONCRETE CURB AND GUTTER (TYPE B-6.12)	Foot	460		460
60606200	COMBINATION CONCRETE CURB AND GUTTER (TYPE B-9.12)	Foot	100		100
63000000	STEEL PLATE BEAM GUARD RAIL, TYPE A	Foot	160	401.50	160
63100070		Each			4
~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	TRAFFIC BARRIER TERMINAL, TYPE 5		4		+
63200310	GUARDRAIL REMOVAL	Foot	160		160
66411900	TEMPORARY FENCE	Foot	200		200
66700 10 5	PERMANENT SURVEY MARKERS (SPECIAL)	Each	1		1
67000400	ENGINEER'S FIELD OFFICE - TYPE A	Cal. Mo.	5		5
67100100	MOBILIZATION	L Sum	1		1
70102550	TRAFFIC CONTROL AND PROTECTION FOR TEMPORARY DETOUR	Each	1		1
1 70000000	THERMOPLASTIC PAVEMENT MARKING - LINE 4"	Foot	375		375
78000200					1
78000200	PREFORMED THERMOPLASTIC PAVEMENT MARKING - LINE 4"	Foot	375	***	375
		Foot Each	375 4		375
78006110	PREFORMED THERMOPLASTIC PAVEMENT MARKING - LINE 4"				

SUMMARY OF QUANTITIES

IO00-2A X071-2A

CODE NUMBER ROADWAY BRIDGE TOTAL PAY ITEM UNIT XX000503 SPLIT RAIL FENCE (SPECIAL) 110 Foot 110 XX001429 FORM LINER TEXTURED SURFACE (SPECIAL) 290 290 Sq. Yd. XXOC7289 JOINT SEALANT (SPECIAL)
XXOO7290 SILANE SURFACE SEALER (SPECIAL) XX00**7289** 600 600 Foot 465 465 Sq. Yd. XX004699 SANITARY SEWER REMOVAL AND REPLACEMENT, 12" (SPECIAL) 80 80 Foot XX005128 STRIP SEAL EXPANSION JOINT ASSEMBLY Foot 75 75 290 XX007359 STAINING CONCRETE STRUCTURES (SPECIAL) Sq. Yd. 290 XX007287 STAINLESS STEEL RUB PLATE ASSEMBLY (SPECIAL) Each 4 4 XX007288 LIGHT WEIGHT AGGREGATE GRANULAR BACKFILL (SPECIAL) 880 880 Cu. Yd.

* SPECIALTY ITEMS

V A080

COUNTY TOTAL SHEET NO. RTE. SECTION 1248 99-00080-00-BR LAKE STA. 185+96 TO STA. 189+70 FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

63020

Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists
330 Pfingsten Road, Northbrook, II 60062
(847) 272-7400, Fax: (847) 291-4813
WJE No. 2006.2324



REVISION RECORD

THE CITY OF LAKE FOREST

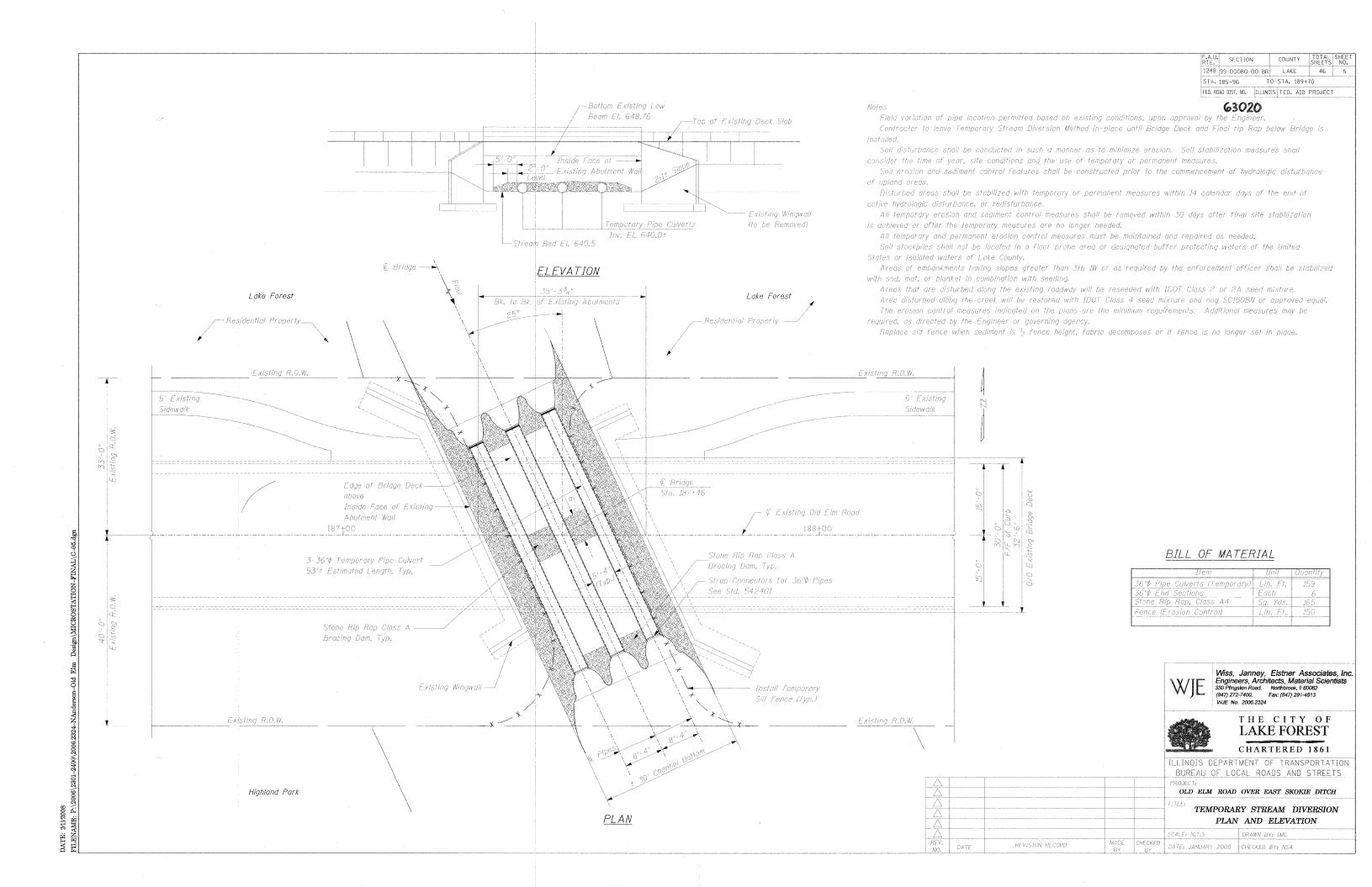
CHARTERED 1861

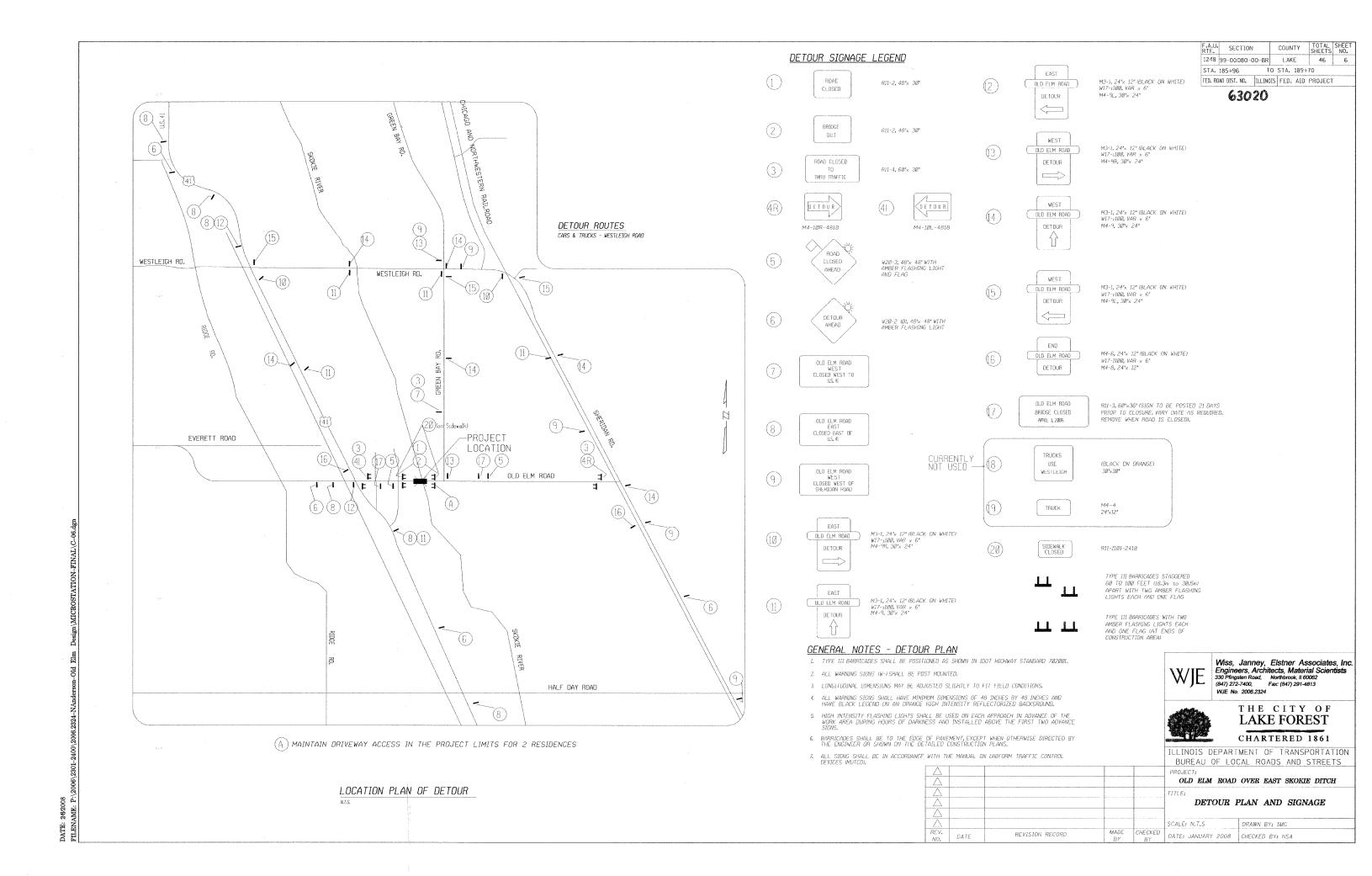
ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

OLD ELM ROAD OVER EAST SKOKIE DITCH

SUMMARY OF QUANTITIES

SCALE: N.T.S DRAWN BY: IMG MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA





BENCHMARK:

(2) MON NO. 24

BRASS DISK IN CONCRETE SOUTH OF THE INTERSECTION OF OLD ELM RD. AND GREEN BAY RD. IN LINE WITH THE CENTER LINE OF GREEN BAY RD. EXTENDES SOUTH APPROXIMATE 3.5 FT. OF SOUTH BACK OF CURB. NOTE, A SMALLER DISK IS SET *.5 FT. NE OF BENCHMARK. ELEV. = 72.17, MSL = 652.33

F.A.U. SECTION COUNTY 1248 99-00080-00-BR LAKE STA, 185+96 TO STA. 189+70 FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

63020

"CONTROL STATION RECORD" 0F

MONUMENT SITUATED IN SECTION 9 TOWNSHIP 43 NORTH, RANGE 12 EAST OF THE 3RD PRINCIPAL MERIDIAN, CITY OF LAKE FOREST, LAKE COUNTY, ILLINOIS MONUMENT DESCRIPTION AND REMARKS STANDARD CITY OF LAKE FOREST CONTROL MONUMENT-BRASS DISK IN CONCRETE ELEVATION 72.169 FEET, REFERRED TO THE CITY OF LAKE FOREST DATUM PLANE ILLINOIS STATE PLANE COORDINATES-EAST ZONE: NAO27 NORTHING (Y)=2022360.9188 EASTING(X)=634374,2731 GRID AZIMUTH MARKS: MARK DESCRIPTION AZIMUTH FROM NORTH 89 34 0 06 (1) MON NO. 16 17

10

REVISION RECORD

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THE CITY OF LAKE FOREST CHARTERED 1861

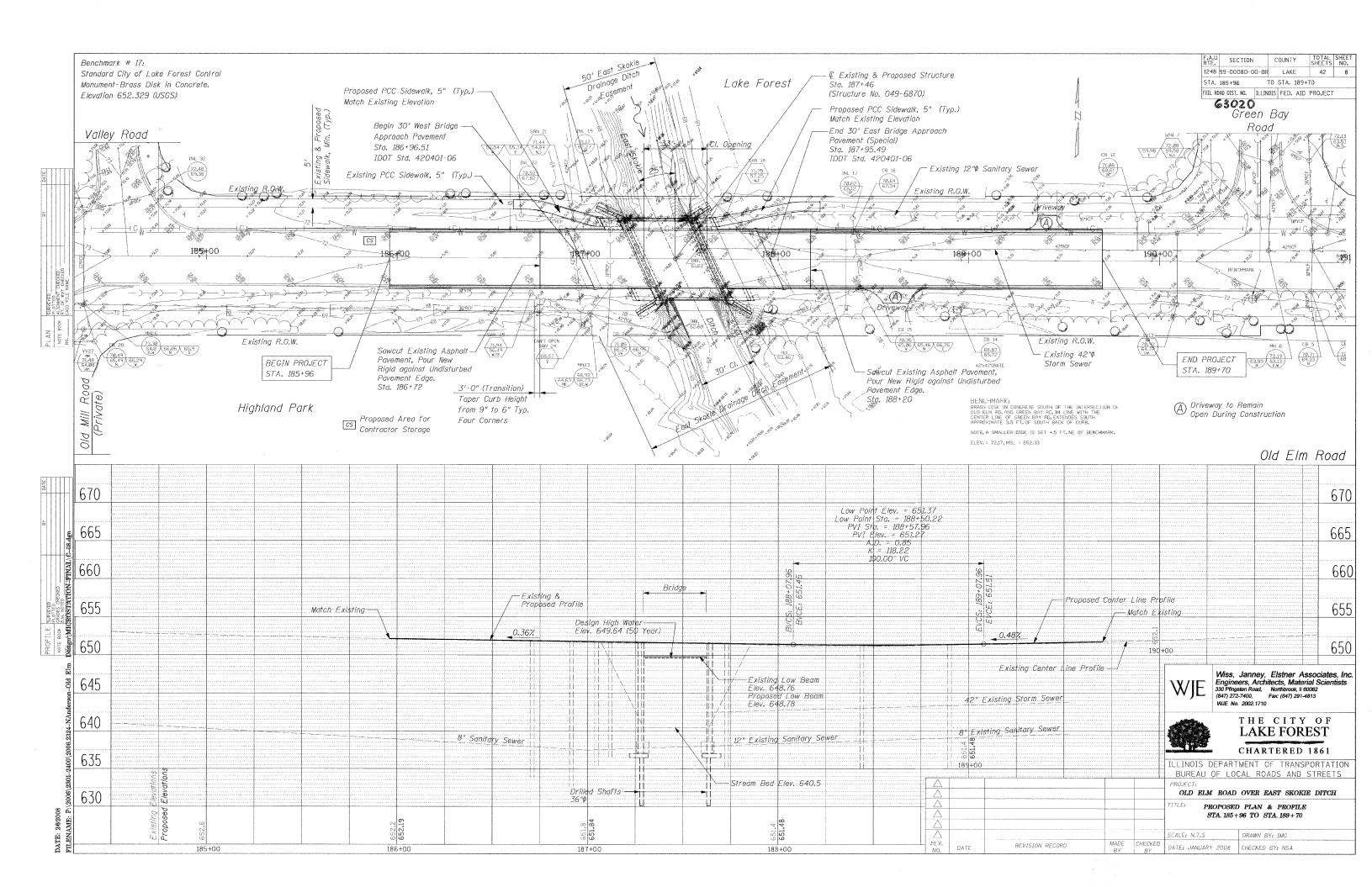
ILLINOIS DEPARTMENT OF TRANSPORTATION

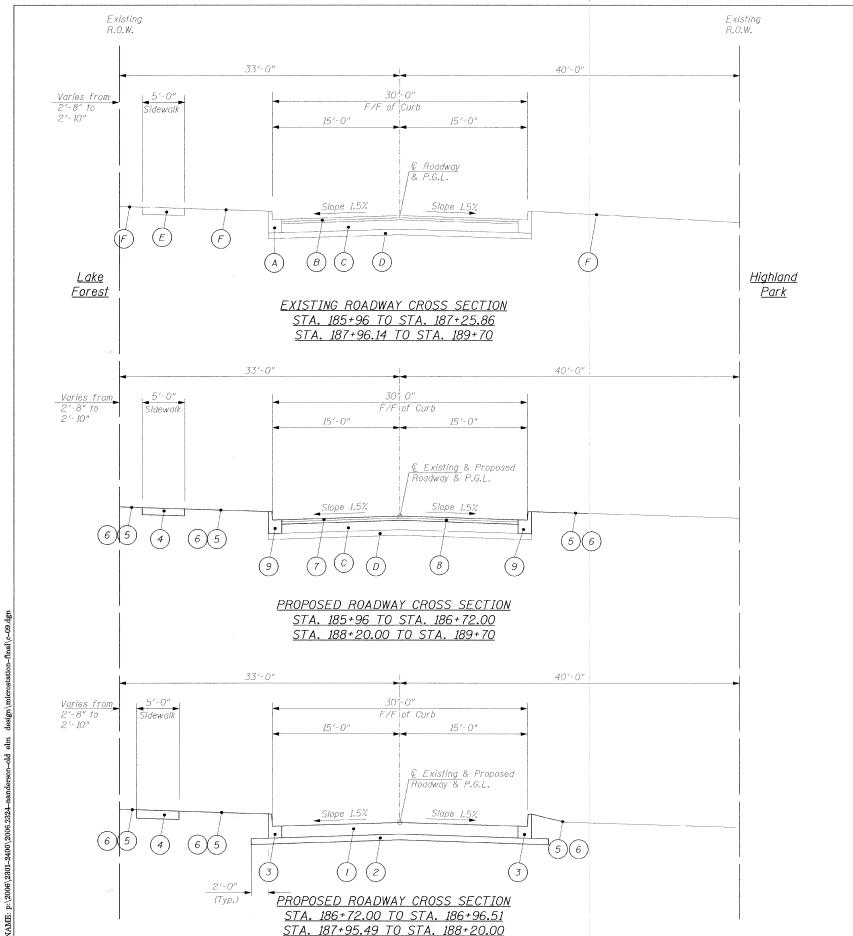
BUREAU OF LOCAL ROADS AND STREETS OLD ELM ROAD OVER EAST SKOKIE DITCH

> BENCHMARK AND ALIGNMENT TIES

DRAWN BY: IMG

SCALE: N.T.S MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA





EXISTING LEGEND

RTE. SECTION 1248 99-00080-00-BR LAKE TO STA, 189+70 STA. 185+96 FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

63020

Combination Concrete Curb & Gutter, Type B-6.12 (Typ.)

3"± Existing Bituminous Resurfacing

Bituminous Aggregate Mixture Base Course-(61/2")

Sub-Base Granular Material-Type B (4")

Existing P.C.C. Sidewalk

Existing Ground Line

PROPOSED LEGEND

Continuously Reinforced Concrete Pavement, 8"

Stabilized Sub-Base, 4"

Combination Concrete Curb and Gutter, Type B-9.12

P.C.C. Sidewalk, 5" (For Limits of Sidewalk Removal and Replacement See Sheet No. 11)

(5) Topsoil Excavation and Placement, 4"

Seeding, Class 2A

Removal (Milling) of Existing Surface, 3"

Proposed Hot-Mix Asphalt Surface Course, Mix "D", N50

Combination Concrete Curb and Gutter, Type B-6.12

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THE CITY OF LAKE FOREST CHARTERED 1861

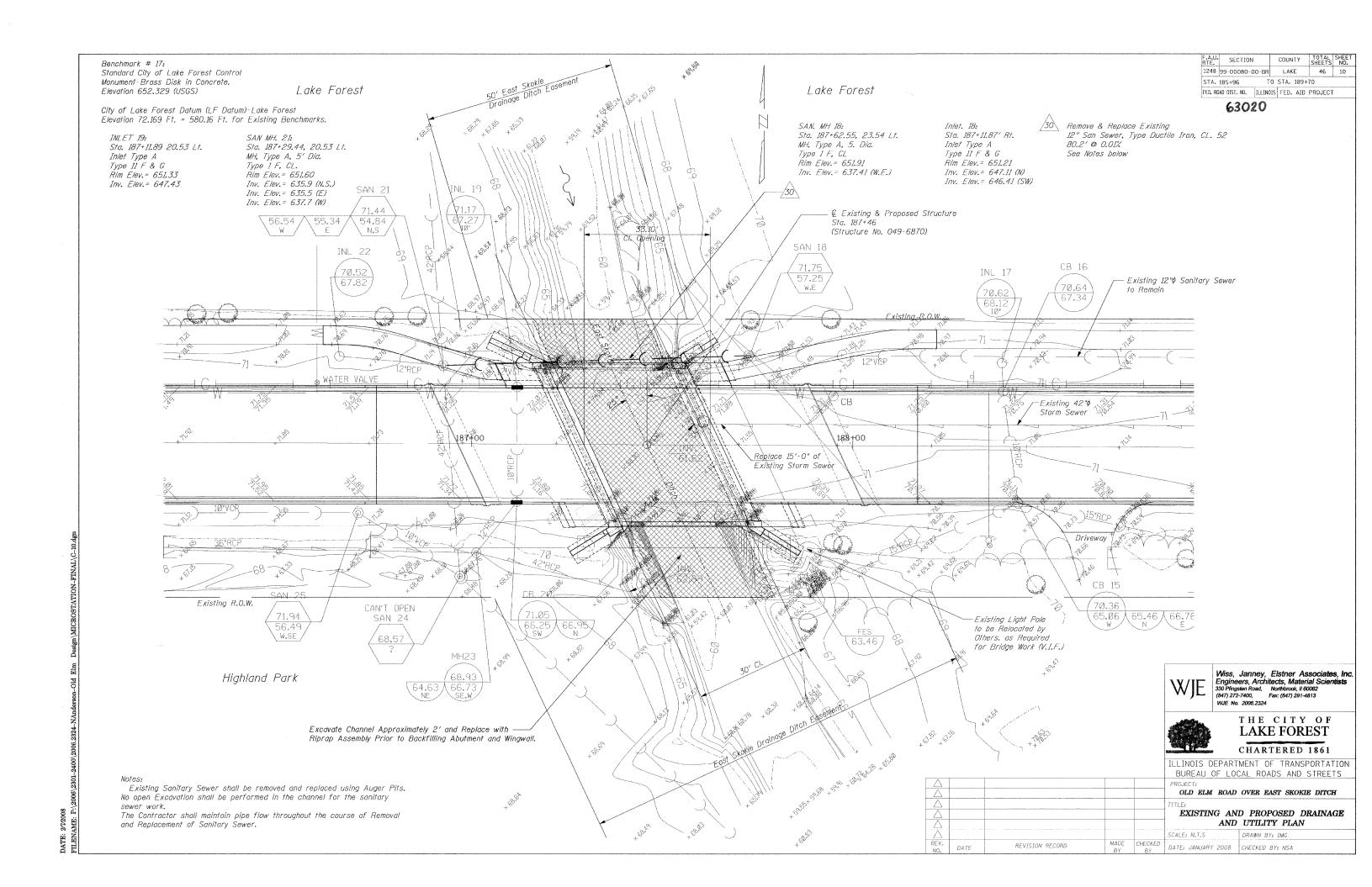
ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

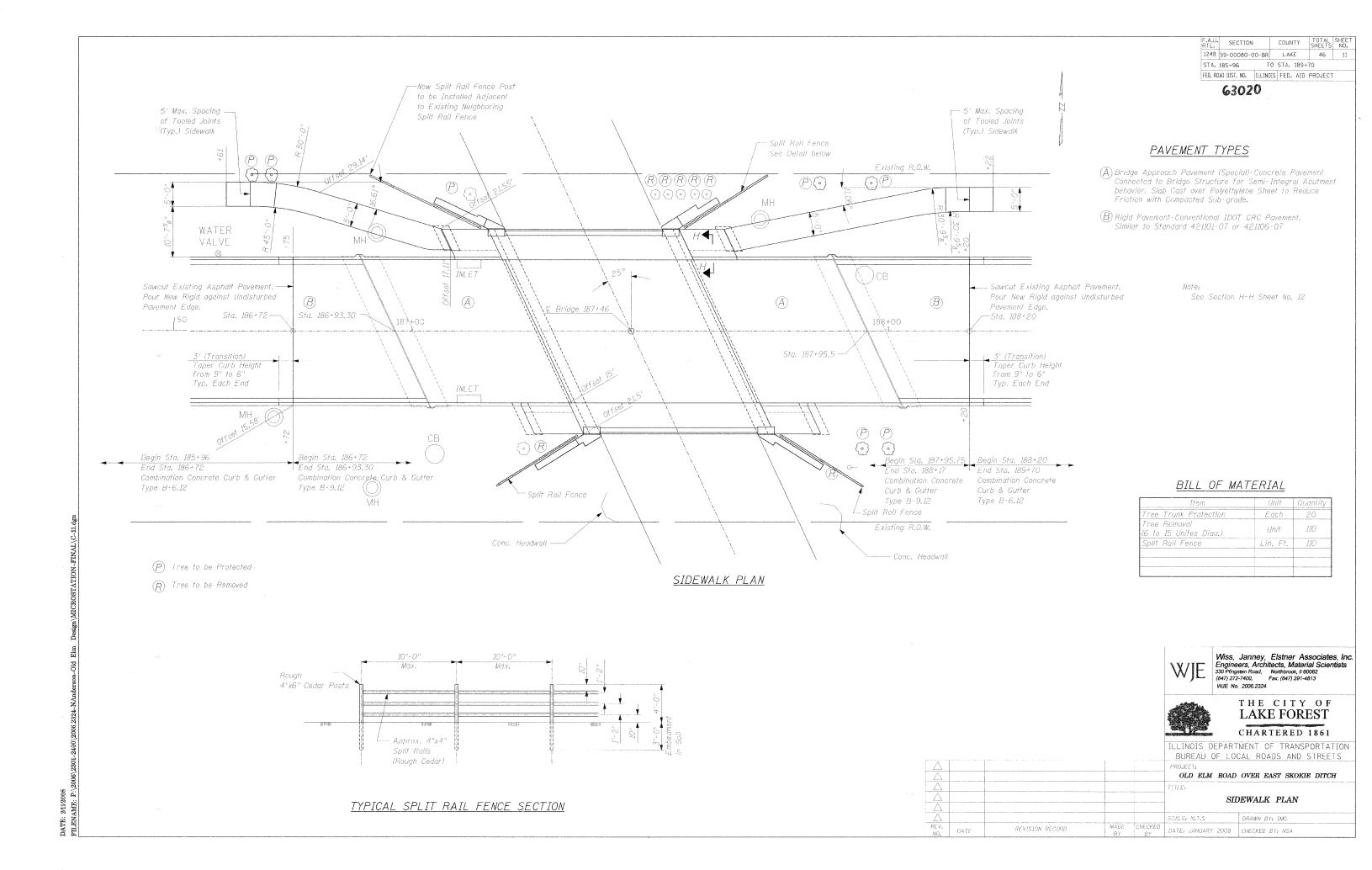
OLD ELM ROAD OVER EAST SKOKIE DITCH ITLE: EXISTING & PROPOSED TYPICAL SECTIONS STA. 185+96.00 TO STA. 189+70.00

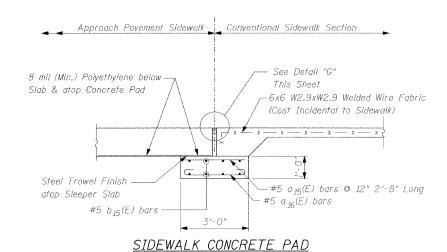
REVISION RECORD

CALE: N.T.S MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA

DRAWN BY: IMG



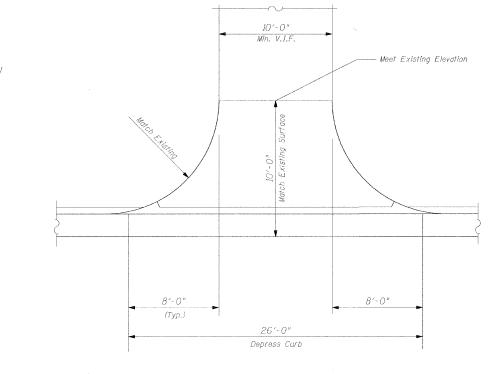




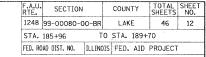
Cost of Sidewalk Concrete Pad Included with Bridge Approach Pavement (Special)

- Silicone Seal - Backer Rod

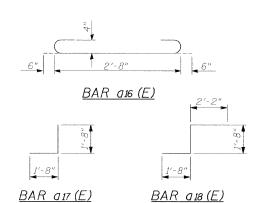
DETAIL "G"



TYPICAL PRIVATE ENTRANCE



63020



BILL OF MATERIAL

Item	Unit	Total
P.C.C Sidealk, 5" (Special)	Sq. Yd.	520
P.C.C Driveway Pavement, 8"	Sq. Yd.	40

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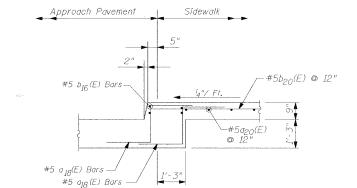


THE CITY OF LAKE FOREST CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION

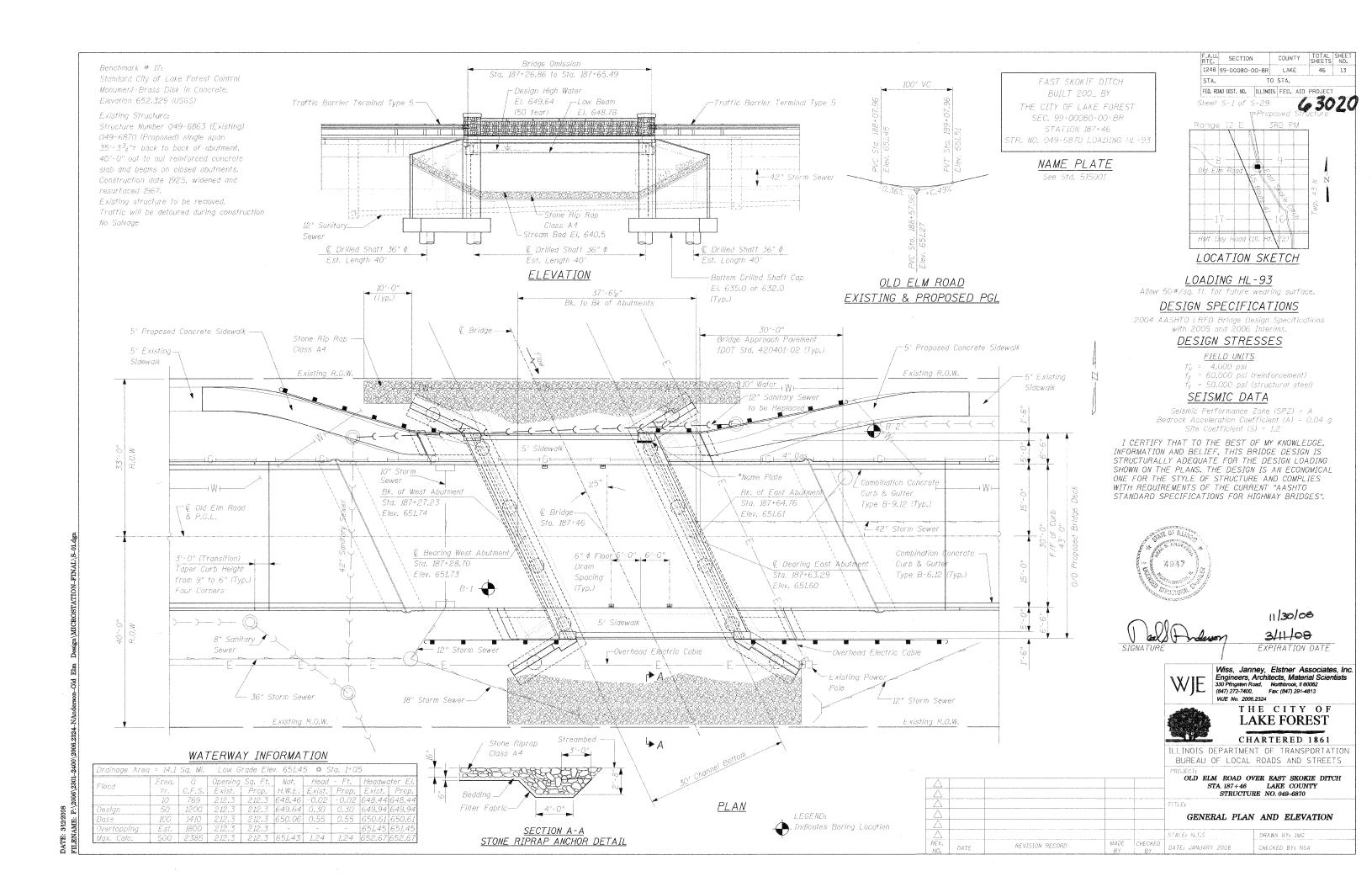
BUREAU OF LOCAL ROADS AND STREETS

					OLD ELM ROAD	OVER EAST SKOKIE DITCH
Δ					TITLE: CITIETIA	LK & PRIVATE
\triangle						
Δ					ENTRA	ANCE DETAILS
\triangle					SCALE: N.T.S	DRAWN BY: IMG
REV.	DATE	REVISION RECORD	MADE	CHECKED	DATE: JANUARY 2008	CHECKED BY: NSA
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SECTION H-H SIDEWALK AT APPROACH PAVEMENT

For Location of Section H-H See Sheet No. 11. Cost of 9" Sidewalk and Curb at Approach Slab Included with Bridge Approach Pavement (Special)



Sheet S-2 of S-29 **63020**

TOTAL BILL OF MATERIAL

TOTAL BILL	,			,
ITEM	UNIT	SUPER	SUB	TOTAL
Removal and Disposal of Unsuitable Material	Cu. Yd.	-	1180	1180
Porous Granular Embankment (Special)	Cu. Yd.	-	110	110
Light Weight Aggregate Granular Backfill (Special)	Cu. Yd.	-	880	880
Stone Riprap, Class A4	Sq. Yd.	-	425	425
Filter Fabric for Use with Riprap	Sg. Yd.	-	425	425
Removal of Existing Structures	L. Sum	-	1	1
Structure Excavation	Cu. Yd.	-	125	125
Floor Drains	Each	4	-	4
Concrete Structures	Cu. Yd.	-	205	205
Concrete Superstructures	Cu. Yd.	90	-	90
Precast Concrete Caps	Foot	82	-	82
Furnishing and Erecting Structural Steel (Galvanized)	L. Sum	1	-	1
Stud Shear Connectors	Each	880	-	880
Reinforcement Bars	Pound	-	24785	2478
Reinforcement Bars, Epoxy Coated	Pound	15590	48050	6364
Name Plates	Each	1	-	1
Drilled Shaft in Soil	Cu. Yd.	-	130	130
Elastomeric Bearing Assembly, Type 1	Each	16	-	16
Anchor Bolts, 1"	Each	32	-	32
Geocomposite Wall Drain	Sq. Yd.	-	175	175
Pipe Underdrains for Structures, 6"	Foot	-	140	140
Form Liner Textured Surface (Special)	Sq. Yd.	60	230	290
Staining Concrete Structures (Special)	Sq. Yd.	60	230	290
Stainless Steel Rub Plate Assembly (Special)	Each	4	-	4

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Engineers, Architects, Material Scientists
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(847) 272-7400, Fax: (847) 291-4813 WJE No. 2006.2324



REVISION RECORD

THE CITY OF LAKE FOREST

CHARTERED 1861 ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

GENERAL NOTES & TOTAL BILL OF MATERIAL

SCALE: N.T.S DRAWN BY: IMG MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA

GENERAL NOTES:

Calculated weight of Structural Steel = 27,500 Lbs.

All Construction joints shall be bonded.

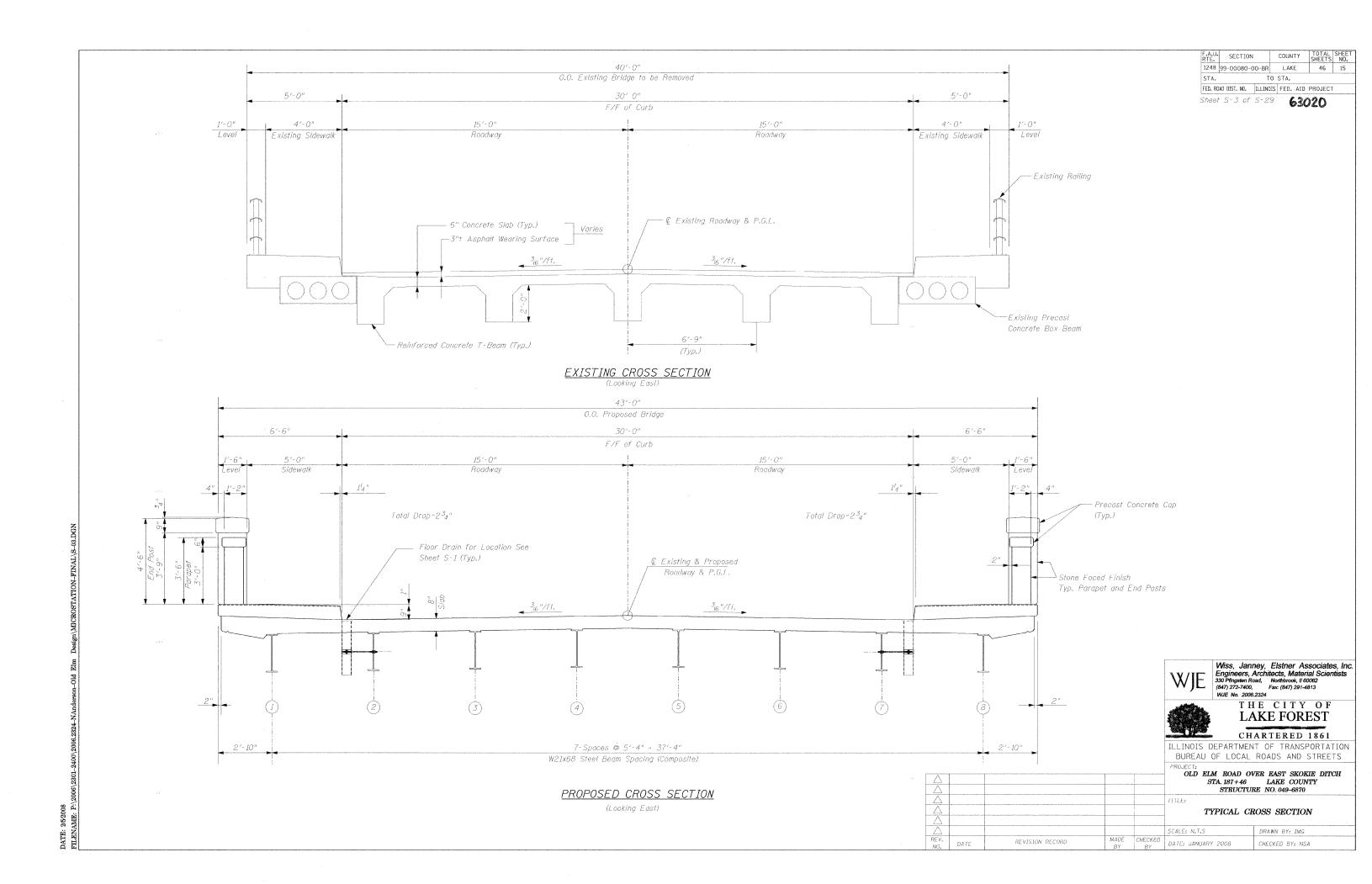
The Contractor shall take appropriate precautions to deal with the presence of lead if encountered on this project.

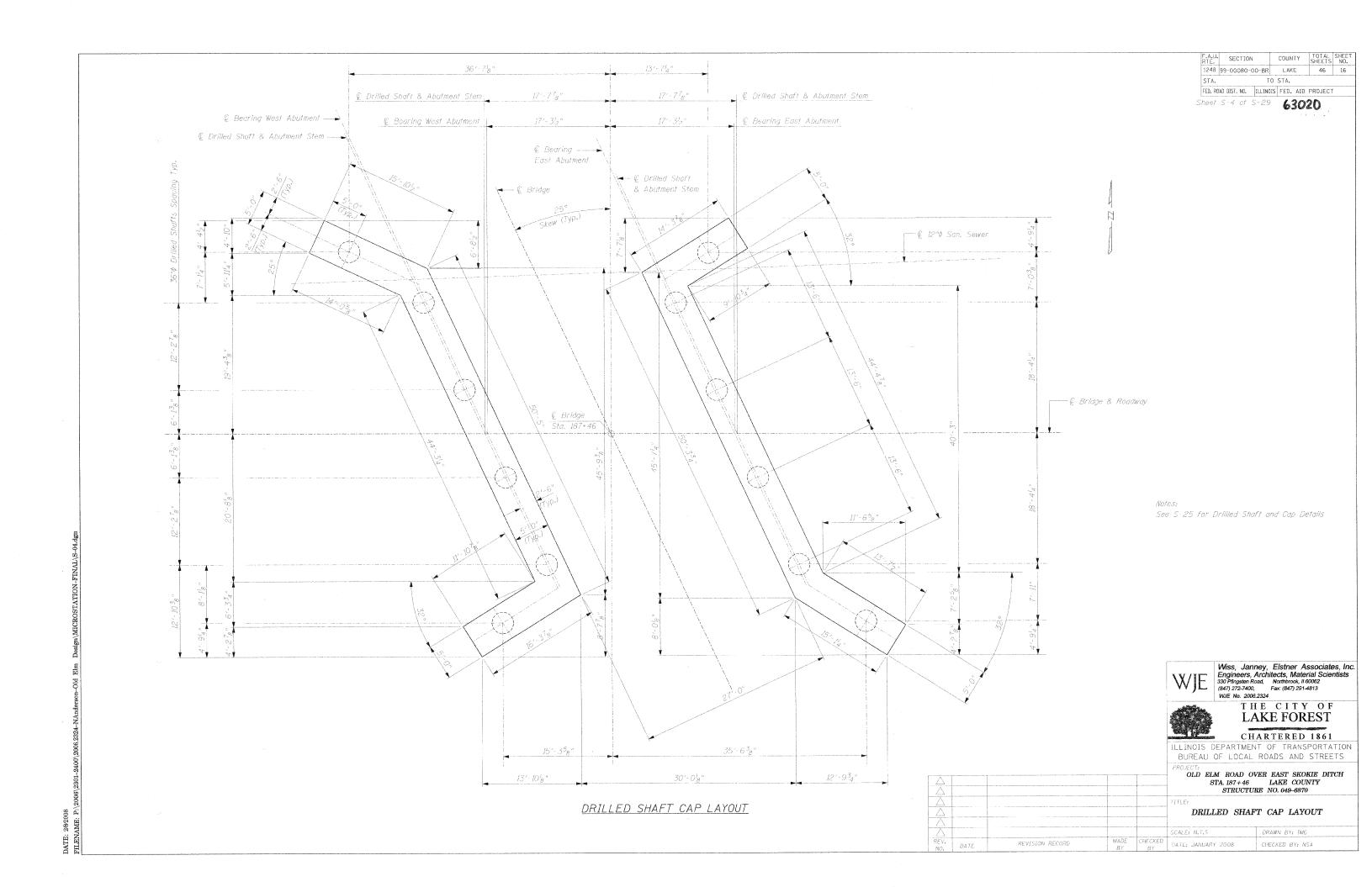
Excavation behind existing abutment walls shall be done before removing the existing superstructure. The Contractor shall sawcut the existing abutments at the stage removal line before stage I removal.

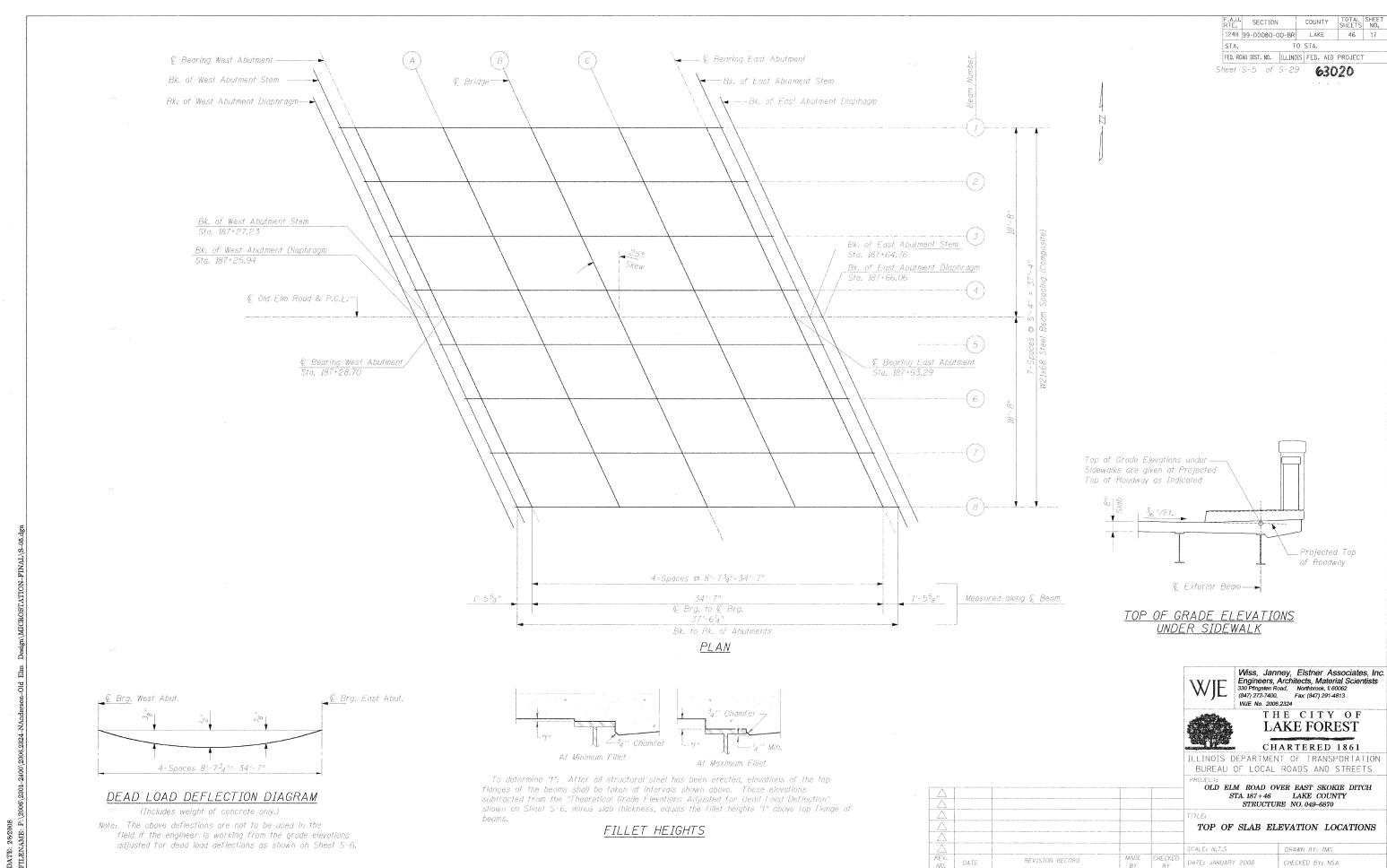
Bearing seat surfaces shall be constructed or adjusted to the designated "elevations within a tolerance of $^{l}_{8}$ inch. Adjustment shall be made either by grinding the surface or by shimming the bearing. Two 's' adjusting shims, of the dimensions of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims. (For Type I Elastomeric Bearings, two $^{l}8^{\prime\prime}$ adjusting shims shall be provided for each bearing and placed as detailed).

Field welding of construction accessories will not be permitted to beams or girders.

The structural steel bearing plates of the Elastomeric Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50 (Galvaniaed - See Special Provision)







BEAM NO. 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of West Diaphragm	187+17.24	- 18.67	651.47	651.47
© Bearing West Abutment	187+20.01	- 18.67	651.46	651.46
A B C	187+28.65 187+37.30 187+45.95	- 18.67 - 18.67 - 18.67	651.43 651.40 651.37	651.46 651.44 651.40
	187+54.59	- 18.67	651.34	651.34
Bk. of East Diaphragm	187+57.36	- 18.67	651.33	651.33

BEAM NO. 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of West Diaphragm	187+19.72	- 13, 34	651.55	<i>651</i> . 55
© Bearing West Abutment	187+22.49	- 13.34	651.54	651.54
A B C	187+31.13 187+39.78 187+48.43	- 13.34 - 13.34 - 13.34	651.51 651.47 651.44	651.54 651.52 651.47
₽ Bearing East Abutment	187+57.07	-13.34	651.41	651.41
Bk. of Cast Diaphragm	187+59.84	- 13.34	651.40	651.40

BEAM NO. 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevalions Adjusted For Dead Load Deflection
Bk. of West Diaphragm	187+22.21	-8.00	651.62	651.62
	187+24.98	-8.00	651.61	651.61
A B C	187+33.62 187+42.27 187+50.92	-8.00 -8.00 -8.00	651.61 651.59 651.55	651.61 651.59 651.55
© Bearing East Abutment	187+59.56	-8.00	651.49	651.49
Bk, of East Diaphragm	187+62.33	-8.00	651.48	651.48

BEAM NO. 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead I oad Deflection
Bk. of West Diaphragm	187+24.70	-2.67	651.70	651.70
	187+27.47	-2.67	651.69	651.69
A B C	187+36.11 187+44.76 187+53.41	-2.67 -2.67 -2.67	651.65 651.62 651.59	651.69 651.66 651.62
	187+62.05	-2.67	651.56	651.56
Bk. of East Diaphragm	187+64.82	-2.67	651.55	651.55

© BRIDGE & P.G.L.

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of West Diaphragm	187+25.94	0,00	651.73	651.73
© Bearing West Abutment	187+28.71	0.00	651.72	651.72
A B C	187+37.35 187+46.00 187+54.65	0,00 0.00 0.00	651.69 651.66 651.63	651.72 651.70 651.66
© Bearing East Abutment	187+63.29	0.00	651.60	651.60
Bk. of East Diaphragm	187+66.06	0.00	651.59	651.59

BEAM NO. 5

Location	Station	Offsel	Theoretical Grade Elevations	Thcoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of West Diaphragm	187+27.18	+2.67	651.69	651.69
© Bearing West Abutment	187+29.95	+2.67	651.68	651.68
A B C	187+38.59 187+47.24 187+55.89	+2.67 +2.67 +2.67	651.65 651.61 651.58	651.68 651.66 651.61
© Bearing East Abutment	187+64.53	+2.67	651.55	651.55
Bk. of East Diaphragm	187+67.30	+2.67	651.54	651.54

BEAM NO. 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elovations Adjusted For Dead Load Deflection
Bk. of West Diaphragm	187+29,67	+8,00	651.59	651.59
@ Bearing West Abutment	187+32.44	+8.00	651.58	651.58
A B C	187+41.08 187+49.73 187+58.38	+8.00 +8.00 +8.00	651.55 651.52 651.49	651.58 651.56 651.52
© Bearing East Abutment	187+67.02	+8.00	651.46	651.46
Bk. of East Diaphragm	187+69.79	+8.00	651.45	651.45

BEAM NO. 7

DETIM NO. 1										
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection						
Bk. of West Diaphragm	187+32.16	+ 13.34	651.50	651.50						
© Bearing West Abutment	187+34.93	+13.34	651.49	651.49						
A B C	187+43.57 187+52.22 187+60.87	+13.34 +13.34 +13.34	651.46 651.43 651.40	651.49 651.47 651.43						
	187+69.51	+13.34	651.37	651.37						
Bk. of East Diaphragm	187+72.28	+13.34	651,36	651.36						

BEAM NO. 8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
Bk. of West Diaphragm	187+34.64	+18.67	651.41	651.41		
© Bearing West Abutment	187+37.41	+18.67	651.40	651.40		
A B C	187+46.05 187+54.70 187+63.35	+18.67 +18.67 +18.67	651.37 651.34 651.31	651.40 651.38 651.34		
	187+71.99	+ 18.67	651.28	651.28		
Bk. of East Diaphragm	187+74.76	+18.67	651.27	651.27		

Notes: See S-5 for Elevation Locations

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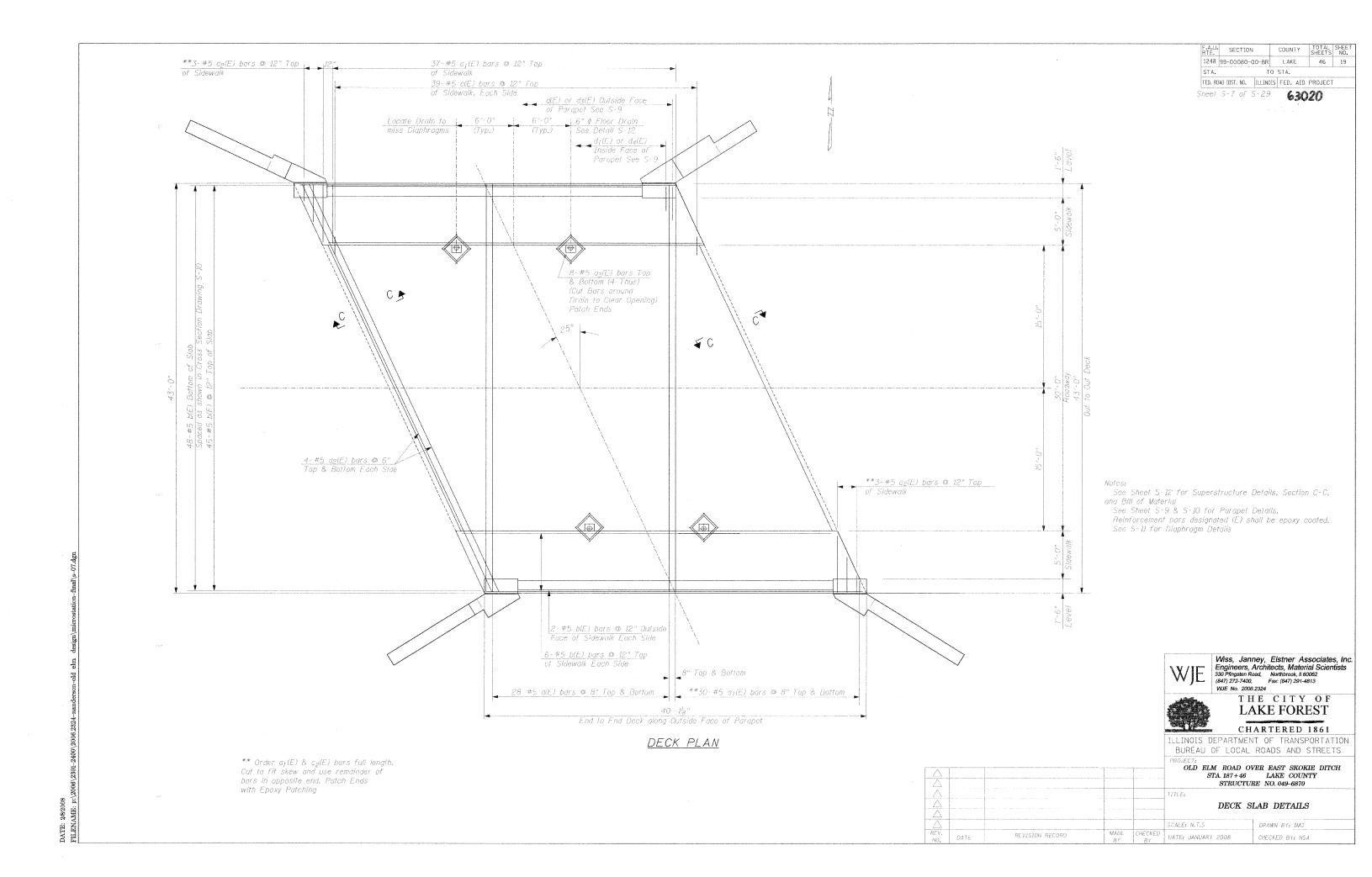
PROJECT:
OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

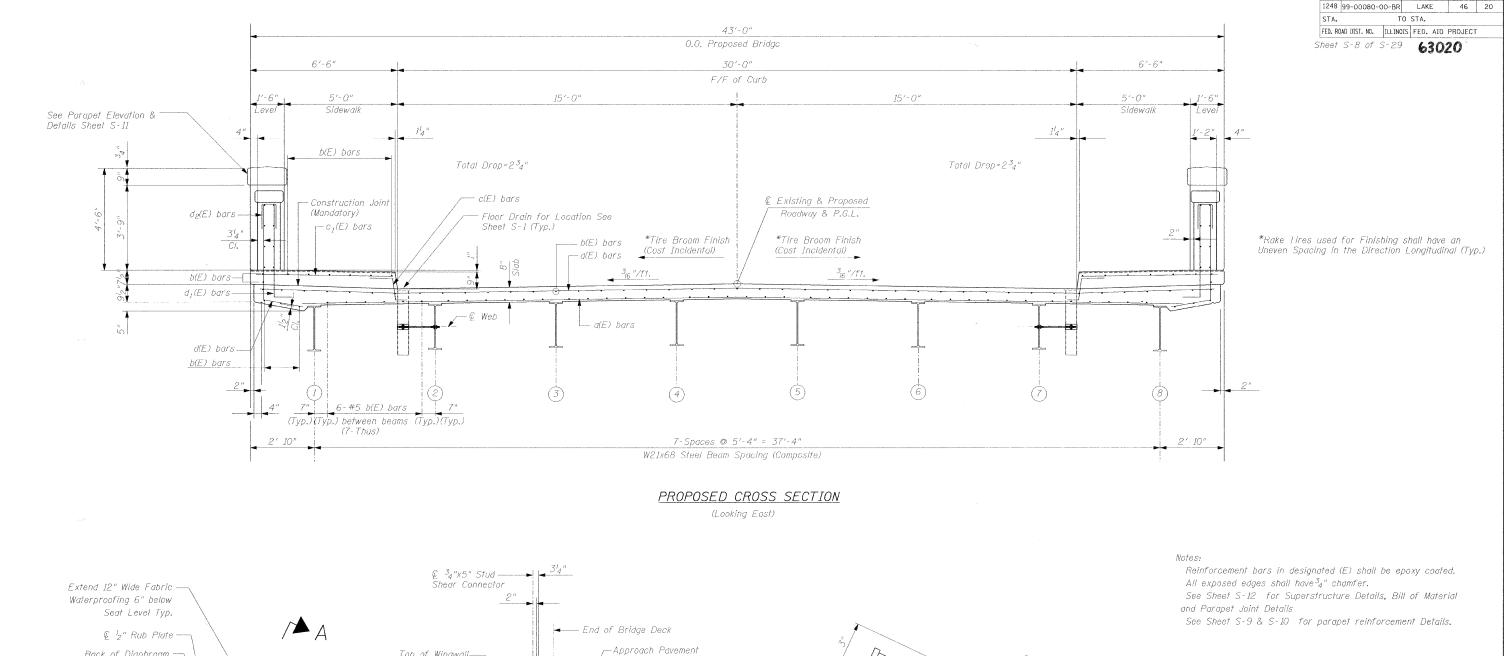
TOP OF SLAB ELEVATION

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REVISION RECORD

SCALE: N.T.S





RTE. SECTION

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ILLINOIS DEPARTMENT OF TRANSPORTATION

BUREAU OF LOCAL ROADS AND STREETS

OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46
LAKE COUNTY

**COUNTY NO. 049-6870

TITLE:

MADE CHECKED BY BY

REVISION RECORD

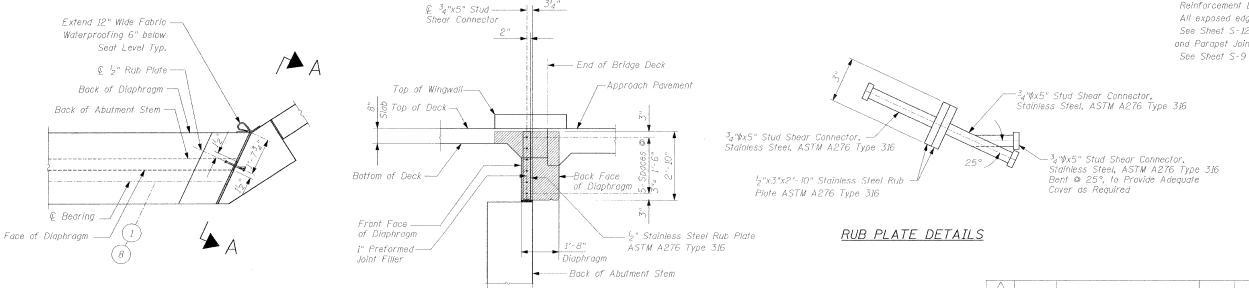
SCALE: N.T.S

DATE: JANUARY 2008

STRUCTURE NO. 049-6870

TYPICAL CROSS SECTION

WJE No. 2006.2324

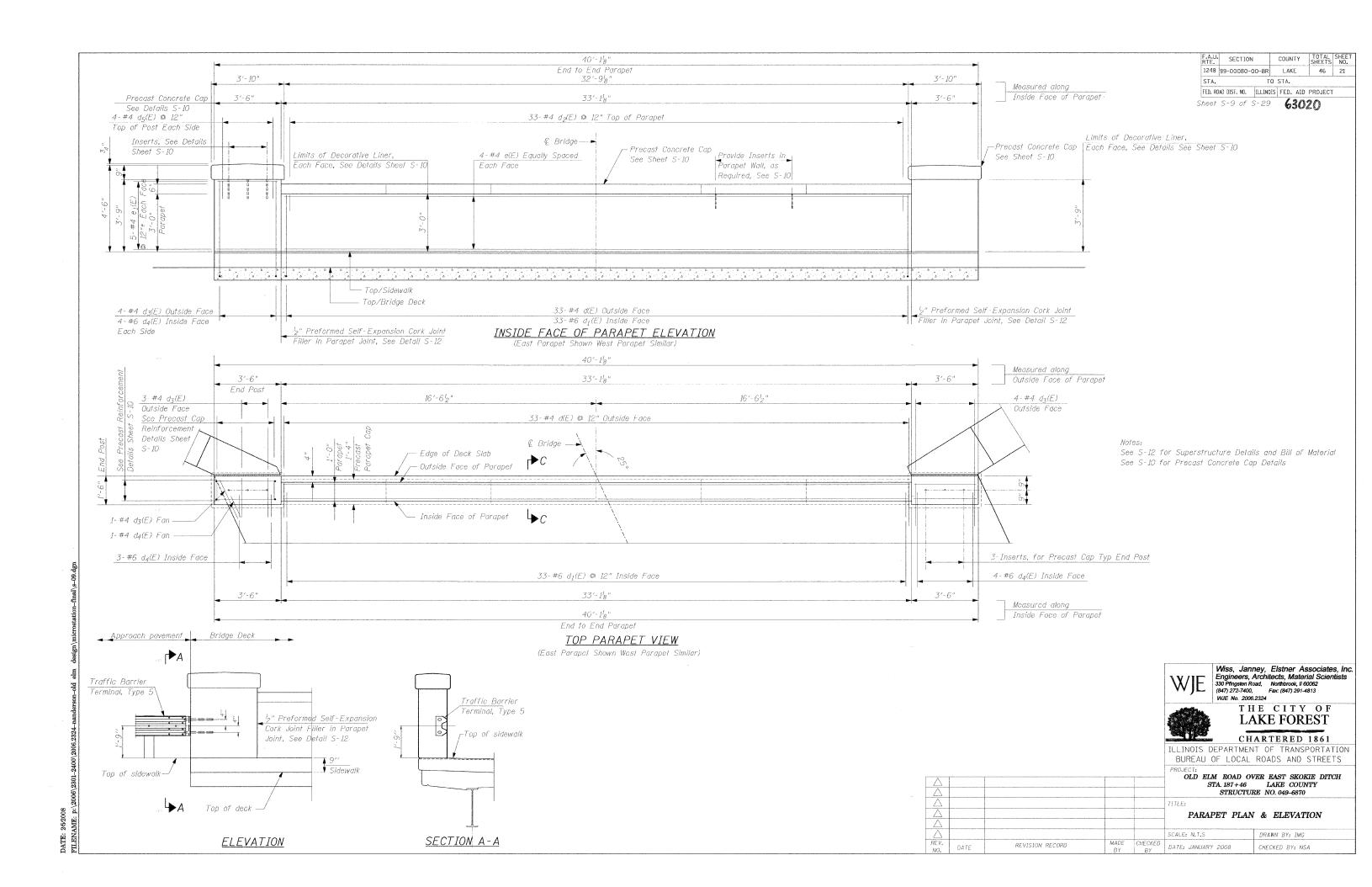


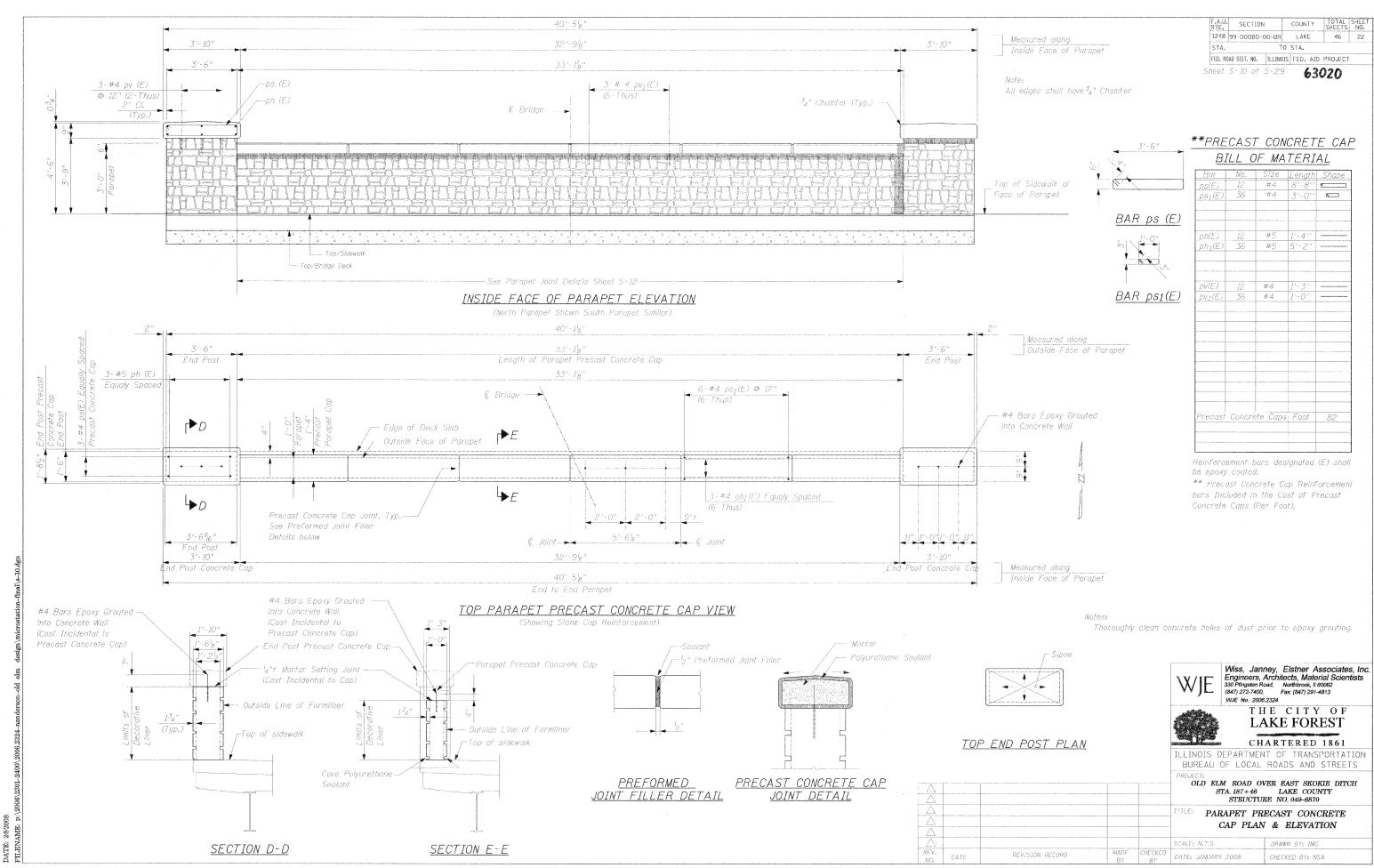
2'-0" Abutment Stem

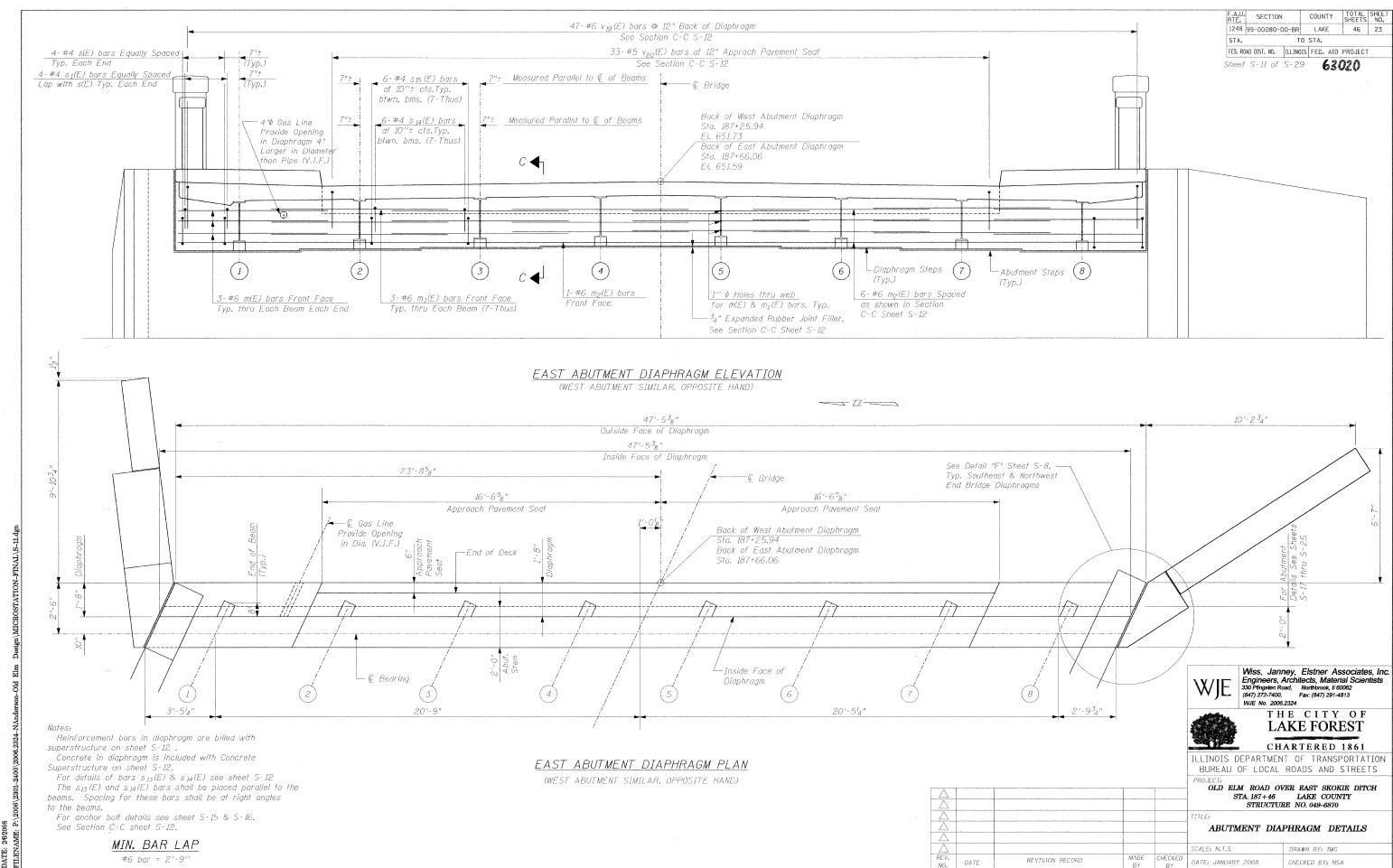
DETAIL A-A

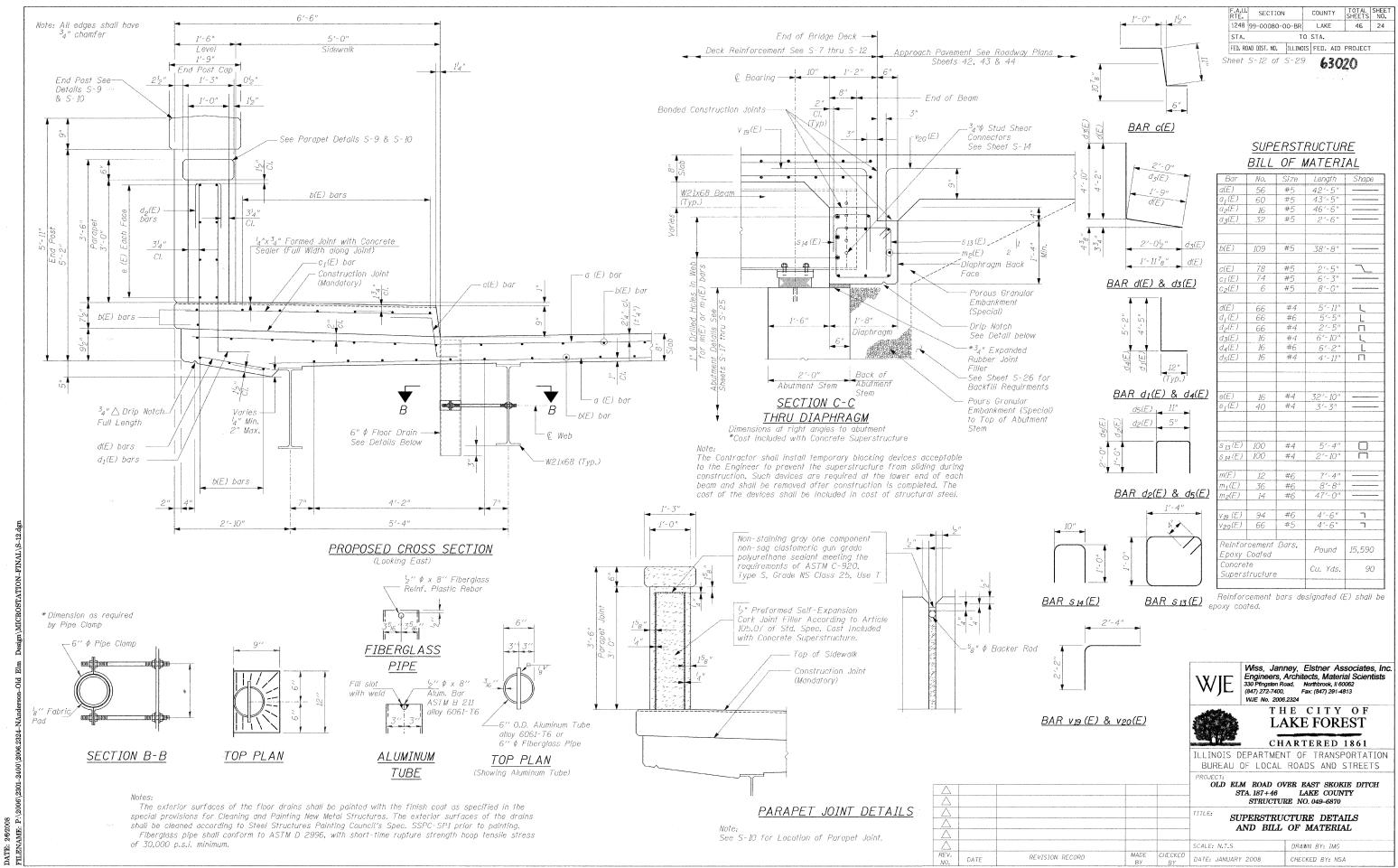
Dimensions at right angles to abutment *Cost included with Concrete Superstructure

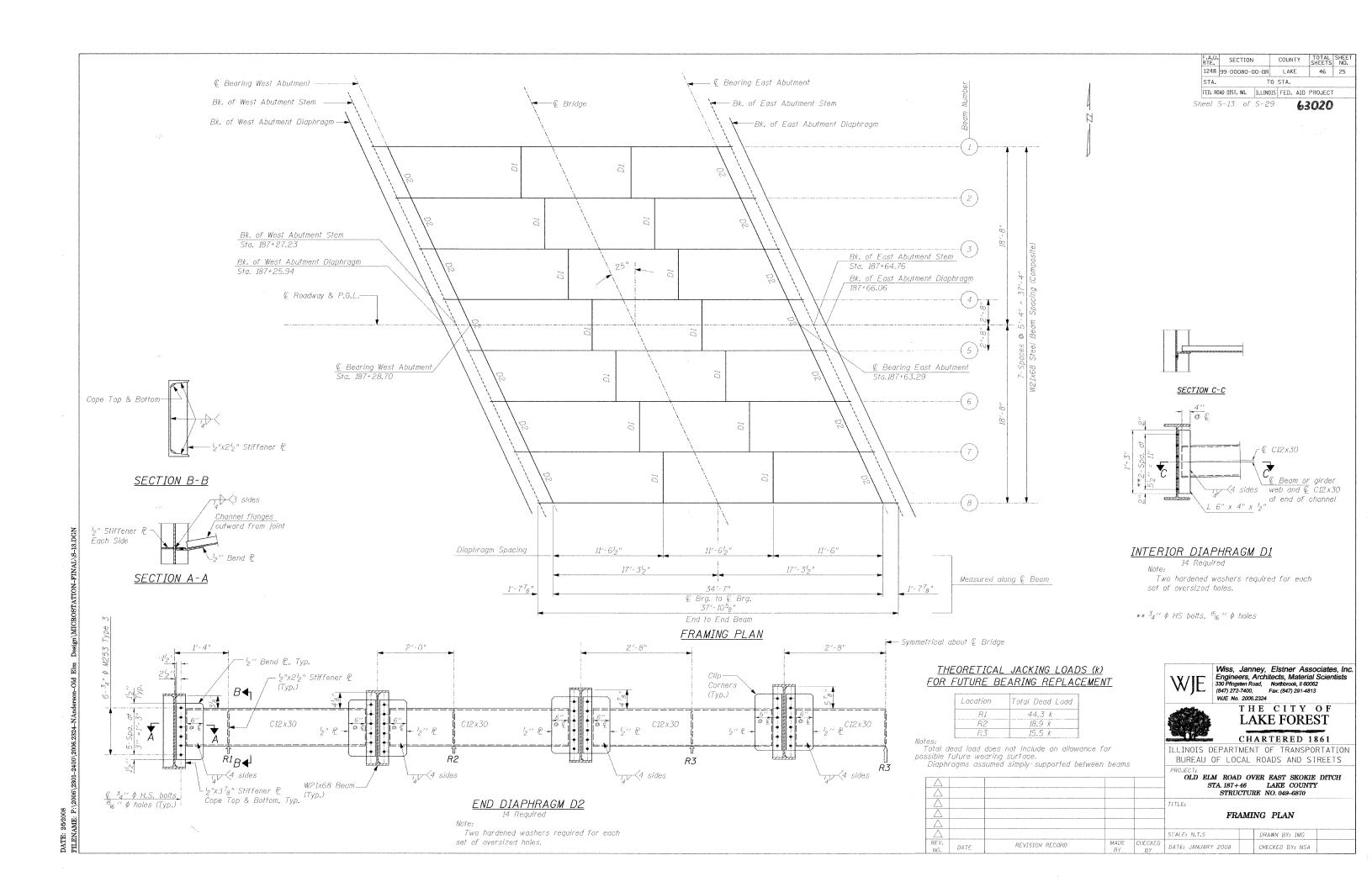
DETAIL "F"



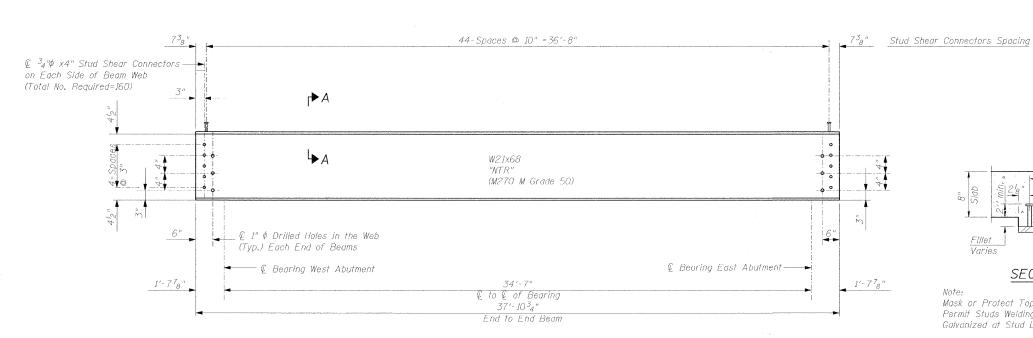












INTERIOR GIRDER MOMENT TABLE							
		0.5 Span					
Is	(in4)	1480					
Ic (n)	(in4)	4829					
Ic (3n)	(in4)	3617					
Ss	(in ³)	140					
Sc (n)	(in ³)	227					
Sc (3n)	(in ³)	204					
Z	(in ³)	160					
DC1	(k/')	0.61					
M DC1	('k)	91.2					
DC2	(k/')	0.41					
M DC2	('k)	61.3					
DW	(k/')	0.27					
M DW	('k)	40.4					
M 4+Imp	('k)	364					
Mu (Strength I)	('k)	888					
¢f Mn, ¢r Mnc	('k)	1258					
fs DC1	(ksi)	7.8					
fs DC2	(ksi)	3.6					
fs DW	(ksi)	2.4					
fs 1.3(½+I)	(ksi)	25.0					
fs (Service II)	(ksi)	38.8					
fs (Total)(Strength I)	(ksi)	51.5					
Vf	(k)	51.4					

Is and Ss are the moment of inertia and section modulus of the steel section used in computing is due to noncomposite loads.

Ic(n) and Sc(n) are the moment of inertia and section modulus of the composite section used in computing fs due to short-term composite loads.

Ic(3n) and Sc(3n) are the moment of inertia and section modulus of the composite section used in computing fs due to long-term composite loads.

Z is the plastic section modulus used to determine the fully plastic moments in the non-composite areas.

DC1 is the dead load acting on the non-composite section.

DC2 is the dead load acting on the long-term composite

DW is the dead load acting on the long-term composite section due to wearing surface.

Mu (Strength I)=1.25 M(DCI+DC2)+1.5 M DW +1.75 M(\(\frac{1}{2}\)+Imp) ♦ Mn is the Compact positive moment capacity computed in accordance with 6.10.7.1.

fs (Service II) is the sum of the stresses due to DC1+DC2+DW+1,3(4+Imp)

fs (Total) (Strength I) (Non-Compact Section) is the sum of the stresses due to 1.25(DC1+DC2)+1.5DW+1.75(4+Imp)

Vf is the maximum shear range computed in accordance with Article 6.10.10

BEAM ELEVATION

"NTR" denotes elements to which notch toughness requirements are applicable.

INTERIOR GIRDER REACTION TABLE

HL93 Loading

55.5

DW

All steel beams, plates, channels & anchors shall be galvanized after shop fabrication according to AASHTO M 111 or AASHTO M232 and ASTM A 385 and Standard Specification for Steel Hardware M234 except stainless steel bolts.

³₄′′ ∮ Granular or solid flux filled headed studs, automatically end welded to flange. (720 Req'd.) Fillet

F.A.U. SECTION

STA.

1248 99-00080-00-BR

Sheet S-14 of S-29

COUNTY TOTAL SHEE SHEETS NO.

46 26

LAKE

FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

SECTION A-A

Mask or Protect Top Flange from Galvanizing to Permit Studs Welding, Alternate-Grid Galvanized at Stud Locations to Permit Stud Welding.

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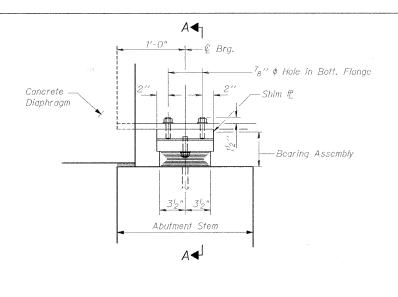
CHARTERED 1861

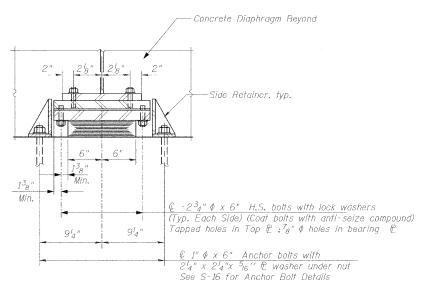
DRAWN BY: IMG MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS OLD ELM ROAD OVER EAST SKOKIE DITCH STA. 187+46 LAKE COUNTY STRUCTURE NO. 049-6870 TITLE: BEAM ELEVATION

REVISION RECORD

SCALE: N.T.S

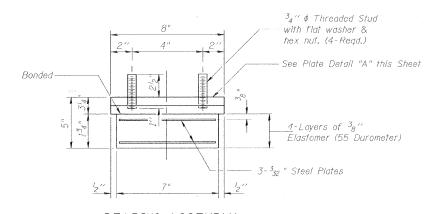




ELEVATION AT ABUT.

SECTION A-A

TYPE I ELASTOMERIC EXP. BRG.



BEARING ASSEMBLY

Note: Shim plates shall not be placed under Bearing Assembly.

© 1½" ♦ Hole SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

Notes:

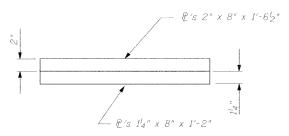
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (Fy=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.

Anchor bolts for side retainers may be cast in place or installed in holes drilled before or after members are in

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Side retainers and other steel members required for the bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.



PLATES DETAIL "A"

F.A.U. RTE.	SECTION	4	COUN	ΤΥ	TOTAL SHEETS	SHEET NO.
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FED. RO	AD DIST. NO.	TLLINO	IS FFD.	AID	PROJECT	

Sheet S-15 of S-29 63020

BILL OF MATERIAL

<i>Item</i>	Unit	Total
Elastomeric Bearing Assembly Type I	Each	16
Anchor Bolts 1" Ø	Each	32

Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Pfingsten Road, Northbrook, Il 60062 (847) 272-7400, Fax: (847) 291-4813



WJE No. 2006.2324 THE CITY OF LAKE FOREST

CHECKED BY: NSA

CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

ROJECT:
OLD ELM ROAD OVER EAST SKOKIE DITCH STA. 187+46 LAKE COUNTY STRUCTURE NO. 049-6870

T!TLE: BEARING DETAILS SCALE: N.T.S DRAWN BY: IMG

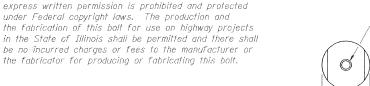
REVISION RECORD

MADE CHECKED DATE: JANUARY 2008

I-2-E1 11-1-06

Sheet S-16 of S-29

63020



"d" Holes with zerk

Top of base plate

for epoxy grout

134" 1411 1/8" 138" 16 Anchor Bolt (See Bearing Details 158'' 15/6 " 2/8" 12" for number, size and length.)

278" 113/6 25" 258" 256" 338''

End of

coil lock

le" at Bottom

PLAN-COIL WIRE

The Illinois Coil-Lock Anchor Bolt is a proprietary

item which is the property of the Illinois Department of

Transportation. Use, reproduction or disclosure without

under Federal copyright laws. The production and

the fabricator for producing or fabricating this bolt.

28"

End of groove

532 wide x 32 deep groove

in anchor bolt with 18" \$

ILLINOIS COIL-LOCK ANCHOR BOLT

MATERIALS FOR ILLINOIS COIL-LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold árawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 519, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coil wire shall be made of any suitable soft steel wire.

The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed. The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type I, Grade 1 and of a Class suitable for the temperature at installation.

INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

- 1. With the coil wire in place, the bott shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete bearing seat.
- 2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

- 1. A threaded rod stud with nut and washer of the type specified.
- 2. A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical.

L ocation	Туре

ASTM F 1554 Grade 105, ASTM A 449 and AASHTO M 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above.

GENERAL NOTES

Holes in the masonry for anchor bolts shall be drilled through the base plates to the diameter and depth shown or according to the manufacturer's recommendation after beams or girders have been erected and adjusted.

Prior to setting the bolts, the holes shall be dry and all dust and loose particles shall be removed by the use of compressed air or vacuuming.

The anchor bolts, furnished and installed and including the epoxy grout or capsules shall not be paid for separately but shall be included in the unit bid price for Furnishing and Erecting Structural Steel.

ANCHOR BOLT DETAILS FOR BEARINGS

Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Pfingsten Road, Northbrook, Il 60062 (647) 227-7400, Fax: (647) 291-4613 WJE No. 2006.2324



THE CITY OF LAKE FOREST CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

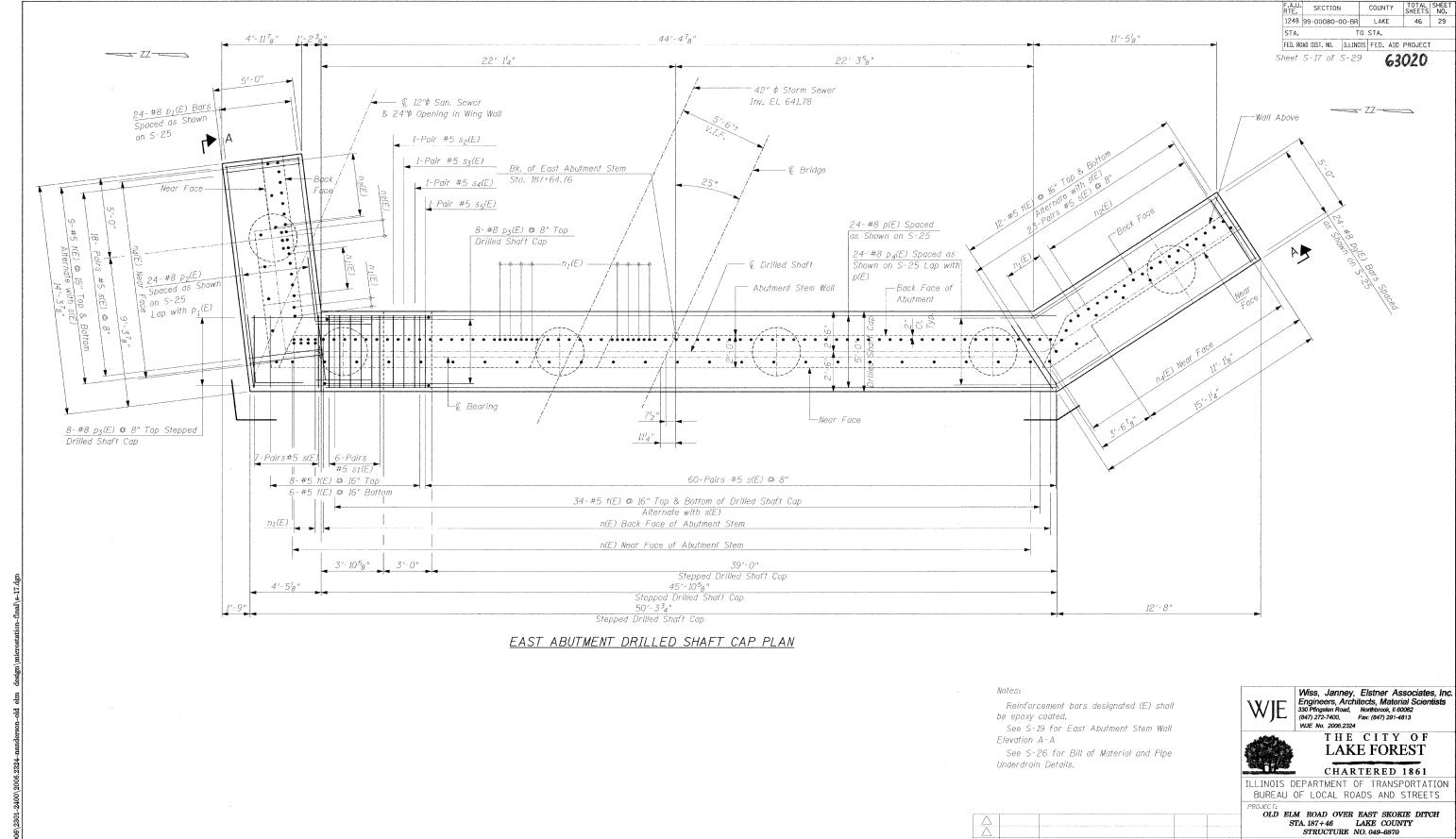
OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

ANCHOR BOLT DETAILS FOR BEARINGS

DRAWN BY: IMG

SCALE: N.T.S MADE CHECKED REVISION RECORD DATE: JANUARY 2008 CHECKED BY: NSA

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EAST ABUTMENT DRILLED SHAFT CAP PLAN

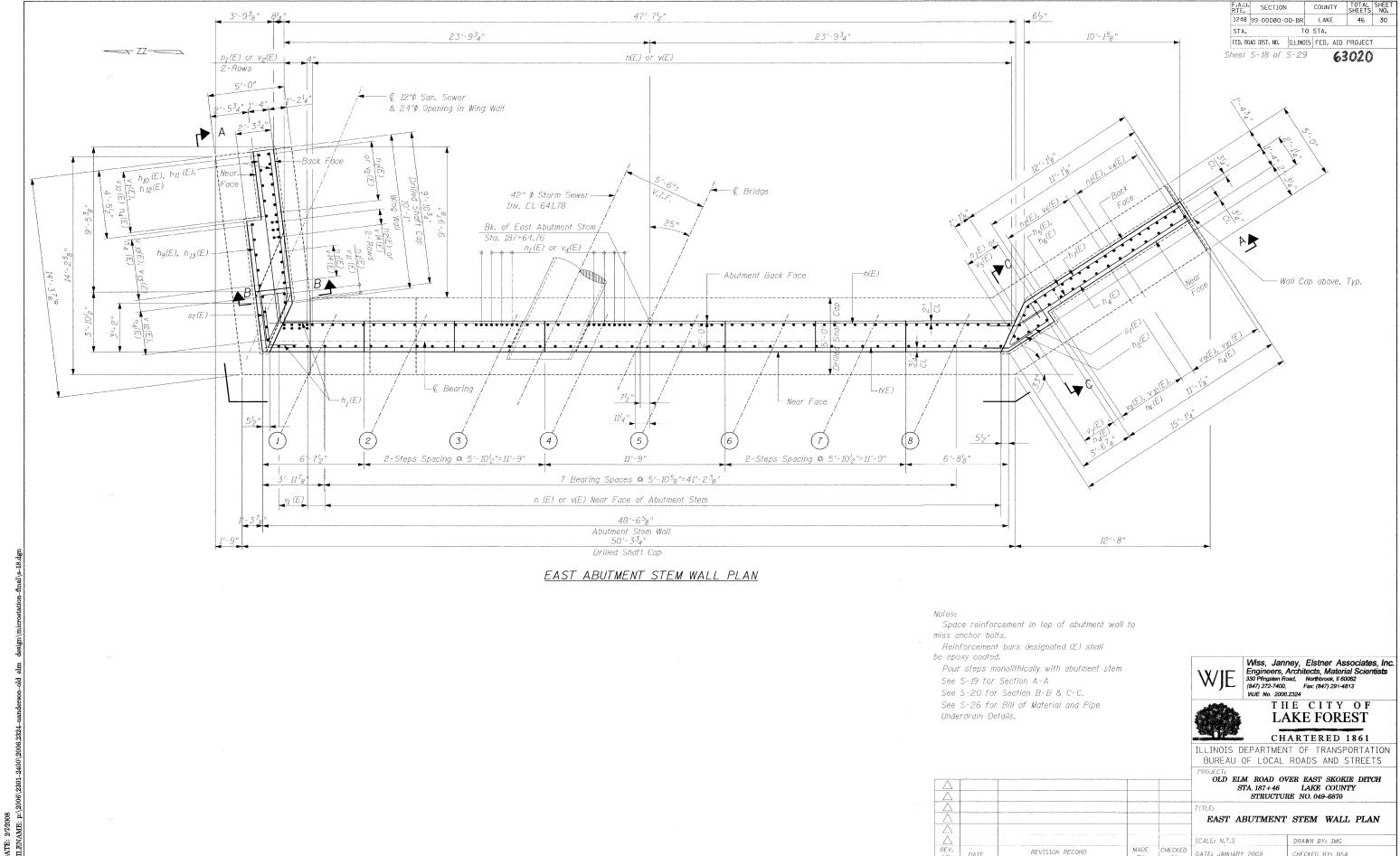
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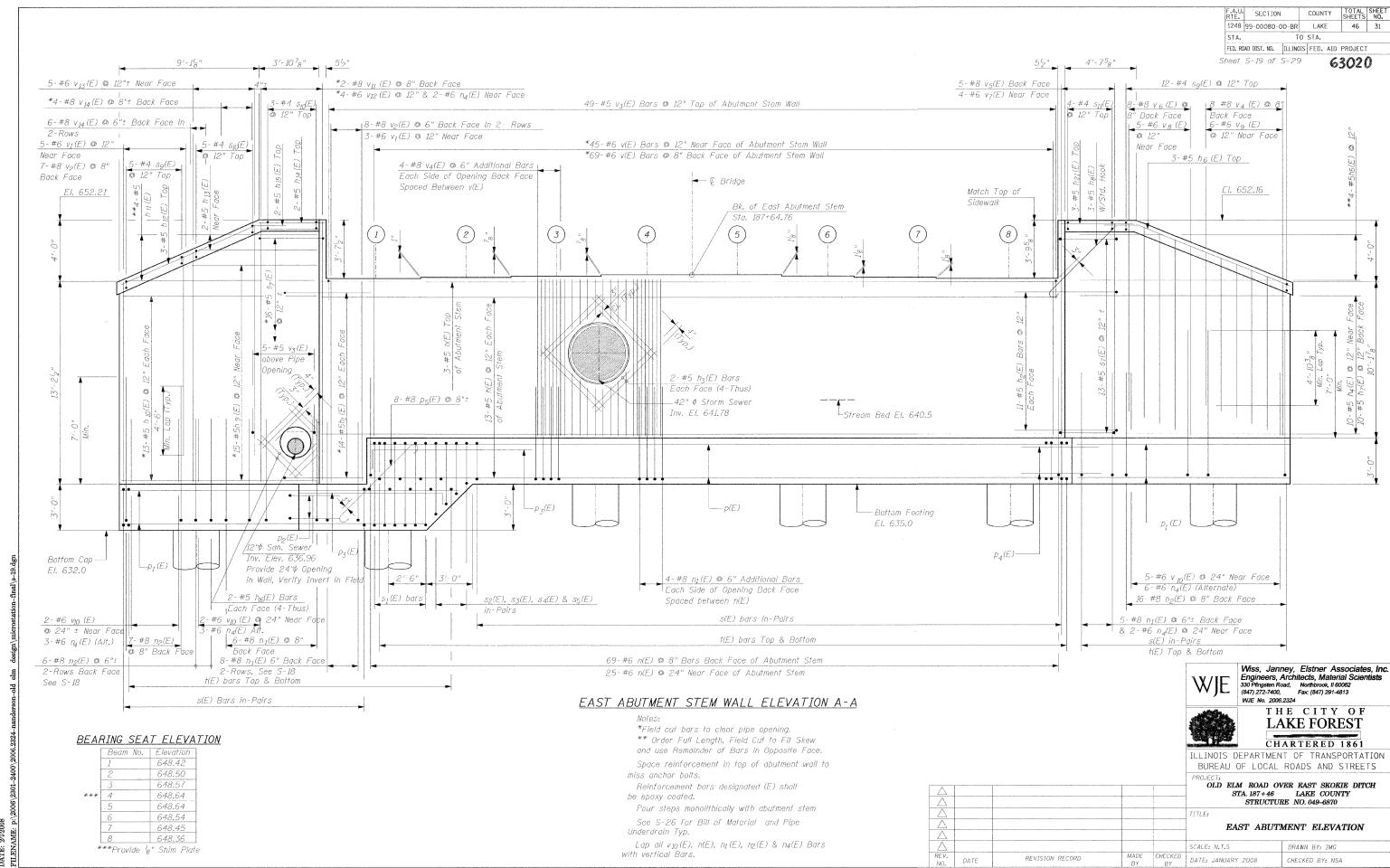
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MADE CHECKED DATE: JANUARY 2008

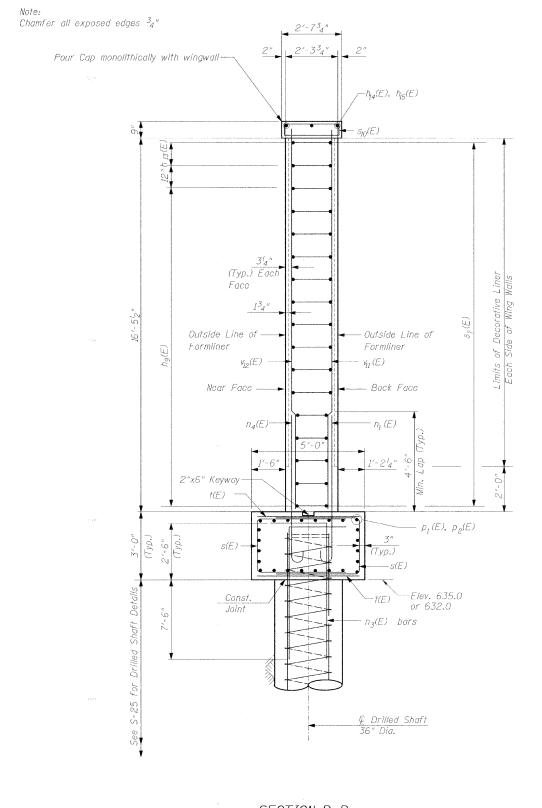
REVISION RECORD



MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA

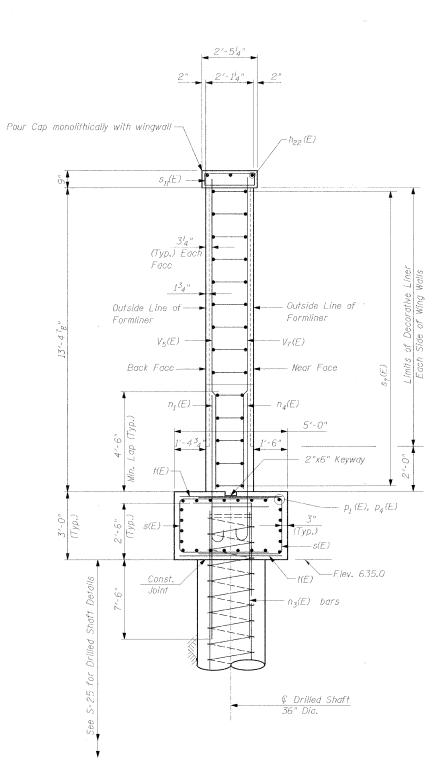


DATE: 9/7/9008



SECTION B-B Showing Reinforcement

EAST ABUTMENT



SECTION C-C Showing Reinforcement

See S-18 for Location of Section B-B and Section C-C. See S-26 for Bill of Material & Pipe Underdrain Details. Provide Shear Key in Top of Caisson Cap and Roughen Surface to 4" Amplitude at Locations where Concrete will be Placed for the Abutment and Wingwall.

SECTION

1248 99-00080-00-BR

Sheet S-20 of S-29

STA.

COUNTY

LAKE

FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

46 32

63020

Wiss, Janney, Elstner Associates, Inc.
Engineers, Architects, Material Scientists
330 Pfingsten Road. Northbrook, Il 60062
(847) 227-2400, Fax: (847) 291-4813
WJE No. 2006,2324



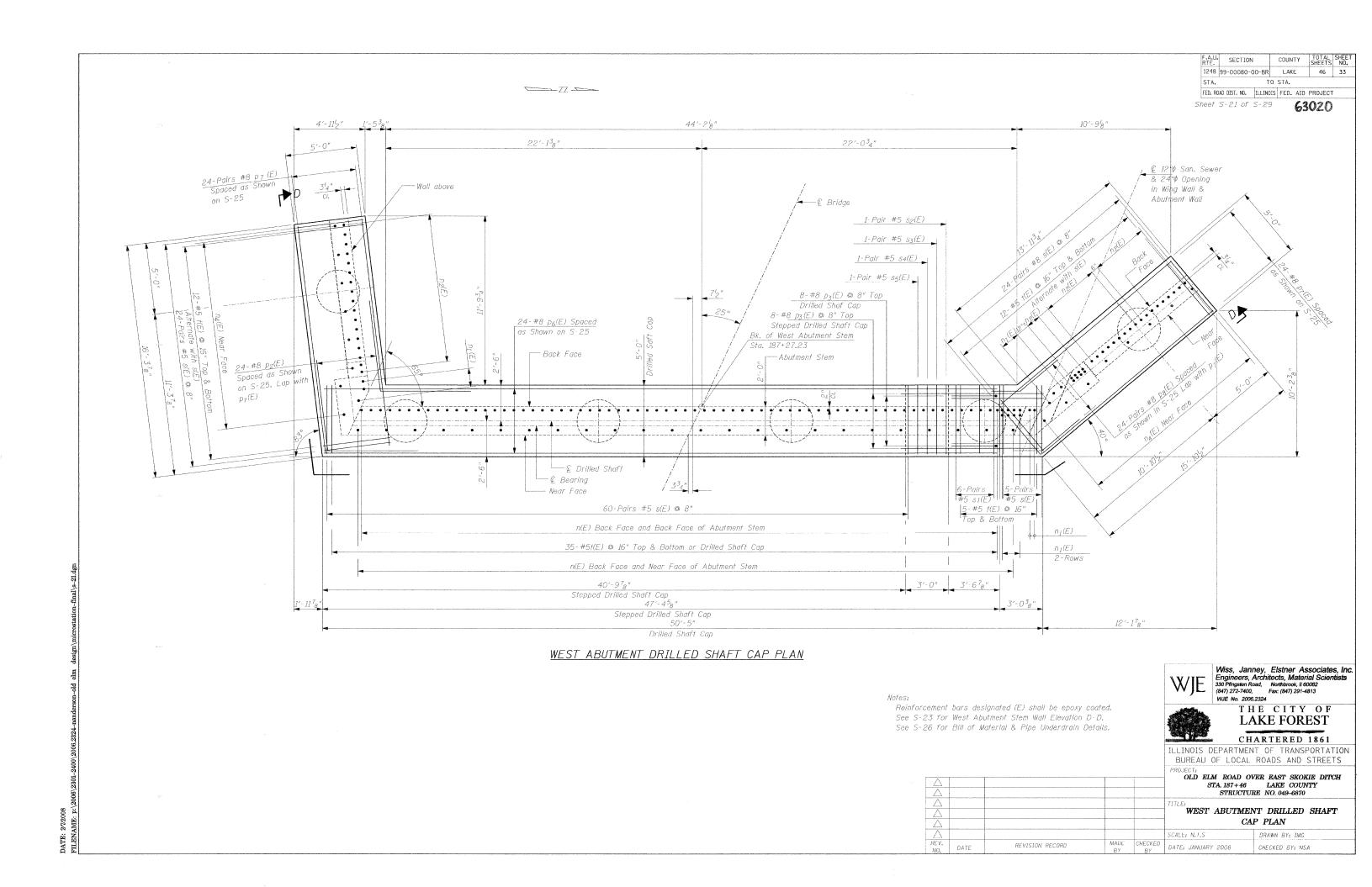
THE CITY OF LAKE FOREST CHARTERED 1861

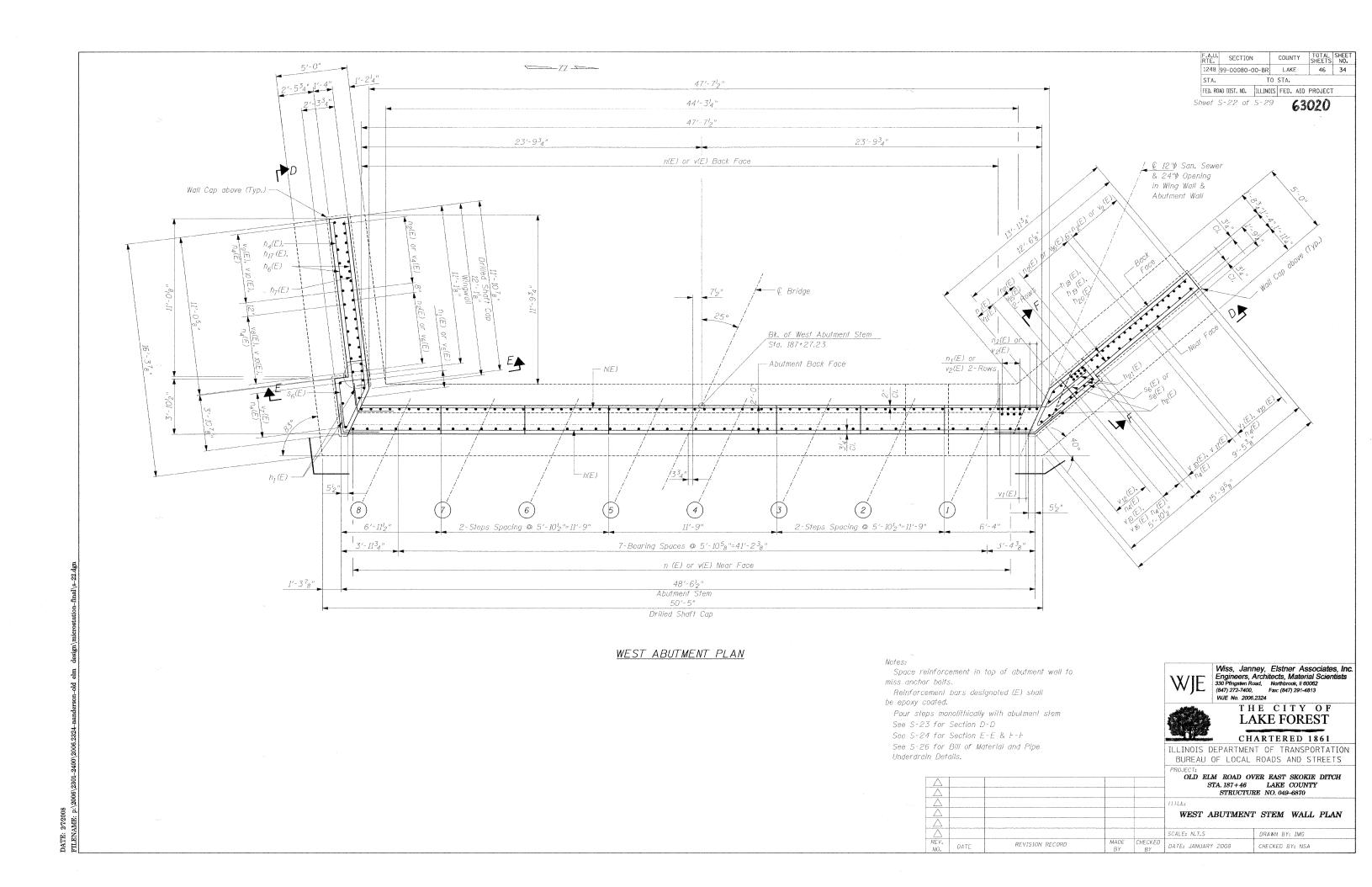
ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

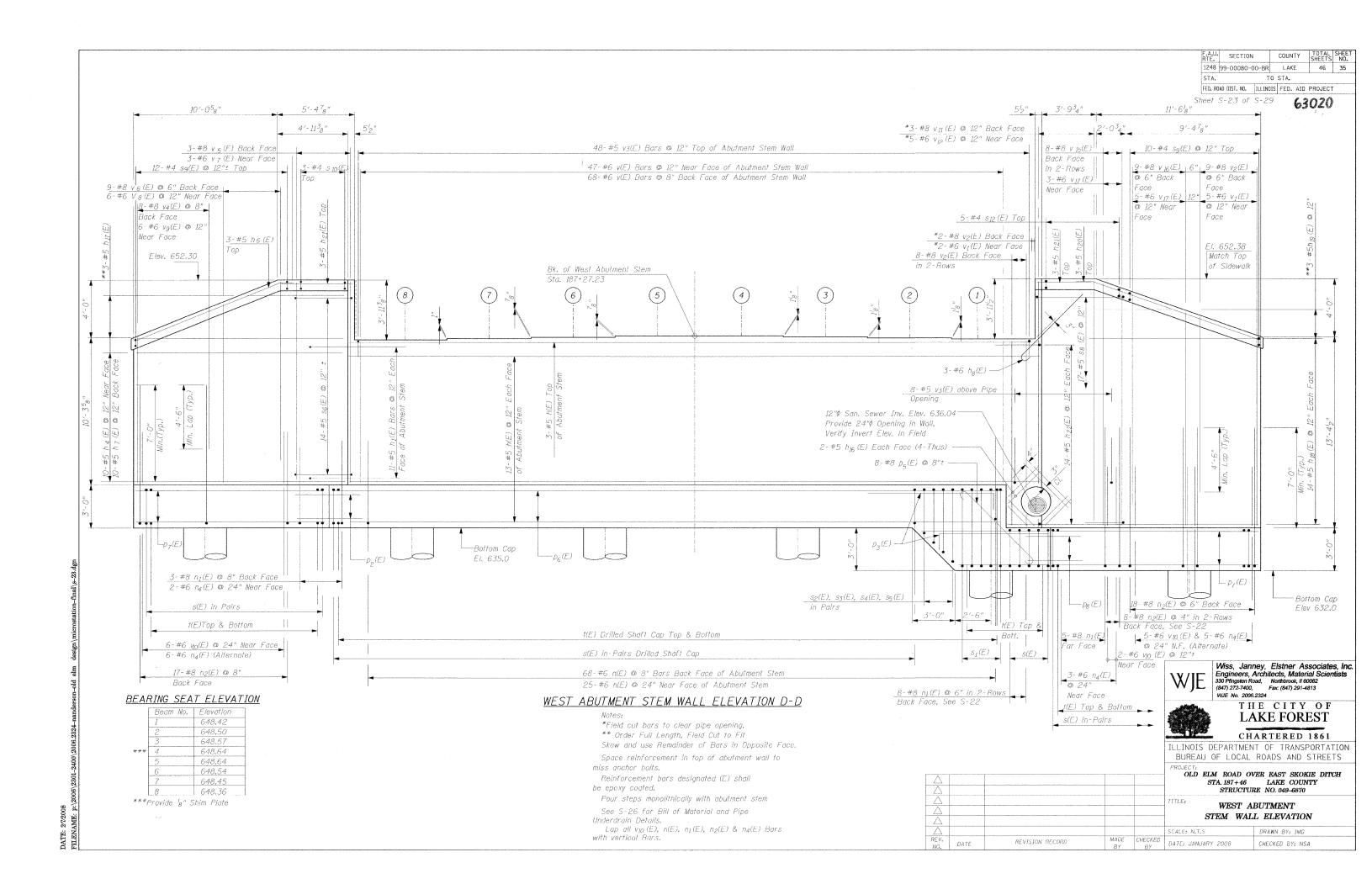
OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

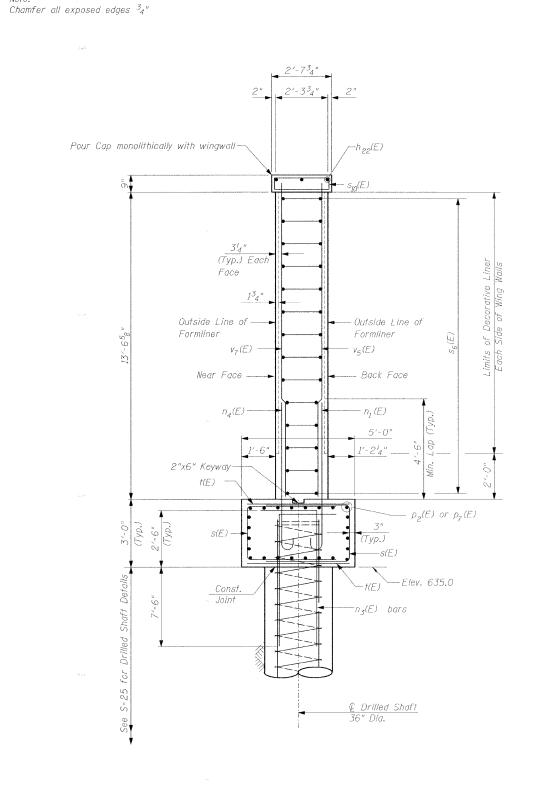
EAST ABUTMENT & WINGWALL DETAILS

DRAWN BY: IMG SCALE: N.T.S MADE CHECKED DATE: JANUARY 2008 REVISION RECORD CHECKED BY: NSA



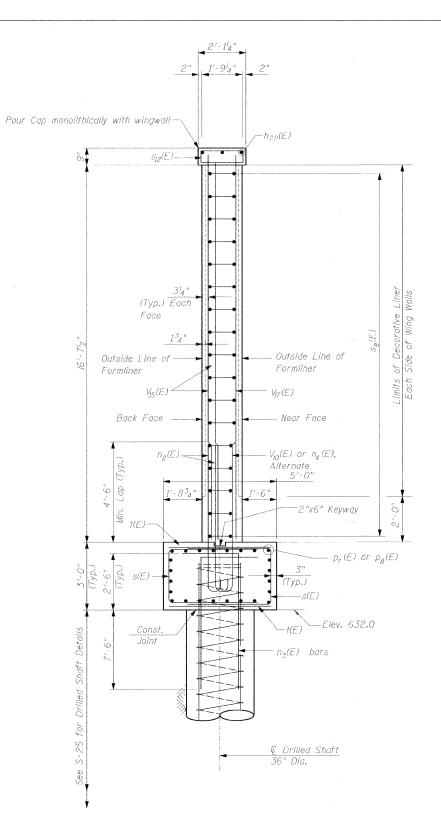






SECTION E-E Showing Reinforcement

WEST ABUTMENT



SECTION F-F Showing Reinforcement

REVISION RECORD

See S-22 for Location of Section E-E and Section F-F. See S-26 for Bill of Material and Pipe Underdrain Details. Provide Shear Key in Top of Caisson Cap and Roughen Surface to ${}^{l}_{4}{}^{n}$ Amplitude at Locations where Concrete will be Placed for the Abutment and Wingwall.

RTE. SECTION 1248 99-00080-00-BR

Sheet S-24 of S-29

STA.

LAKE 46 36

TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

Wiss, Janney, Elstner Associates, Inc.
Engineers, Architects, Material Scientists
330 Pfingsten Road, Northbrook, Il 60062
(847) 272-7400, Fax: (847) 291-4813



THE CITY OF LAKE FOREST

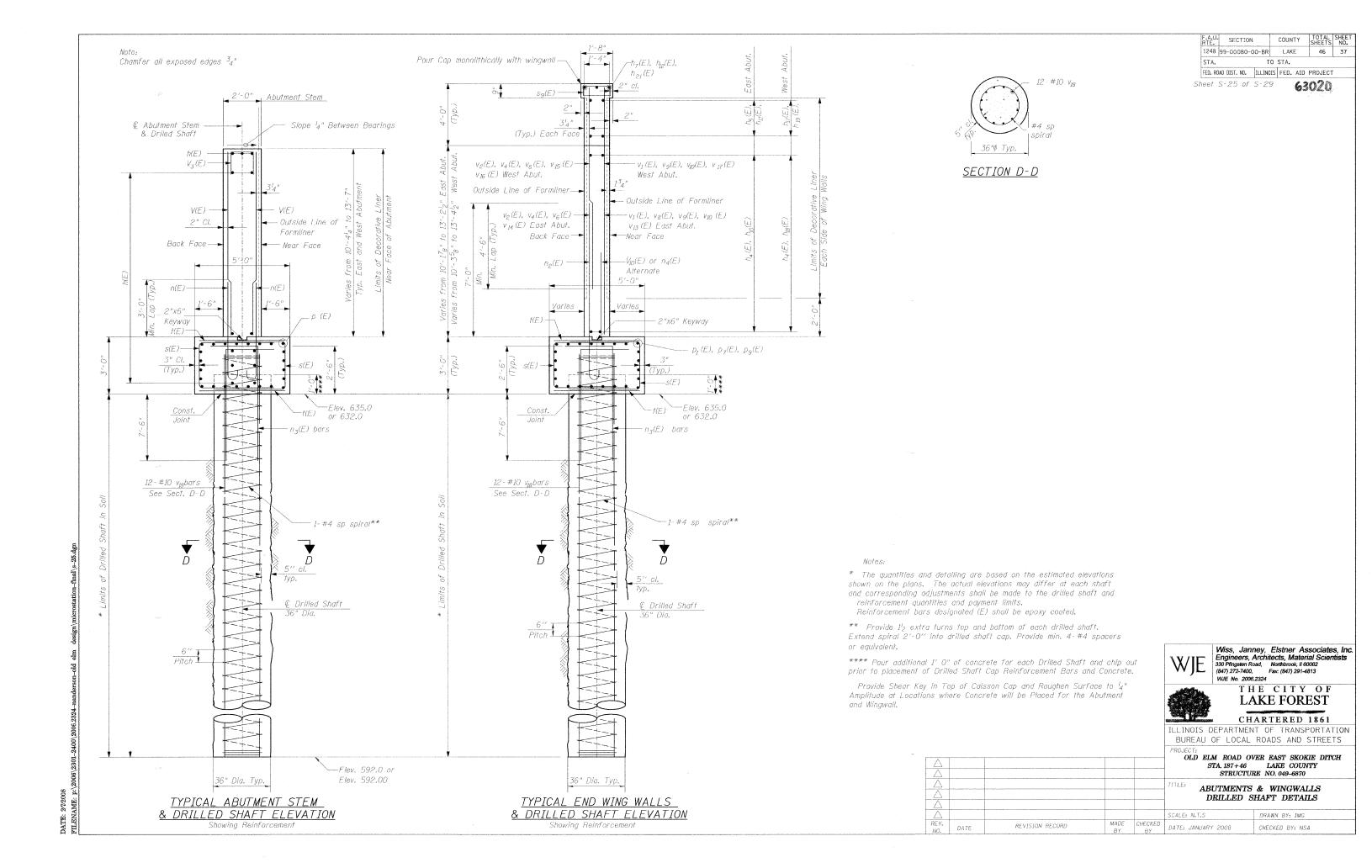
CHARTERED 1861

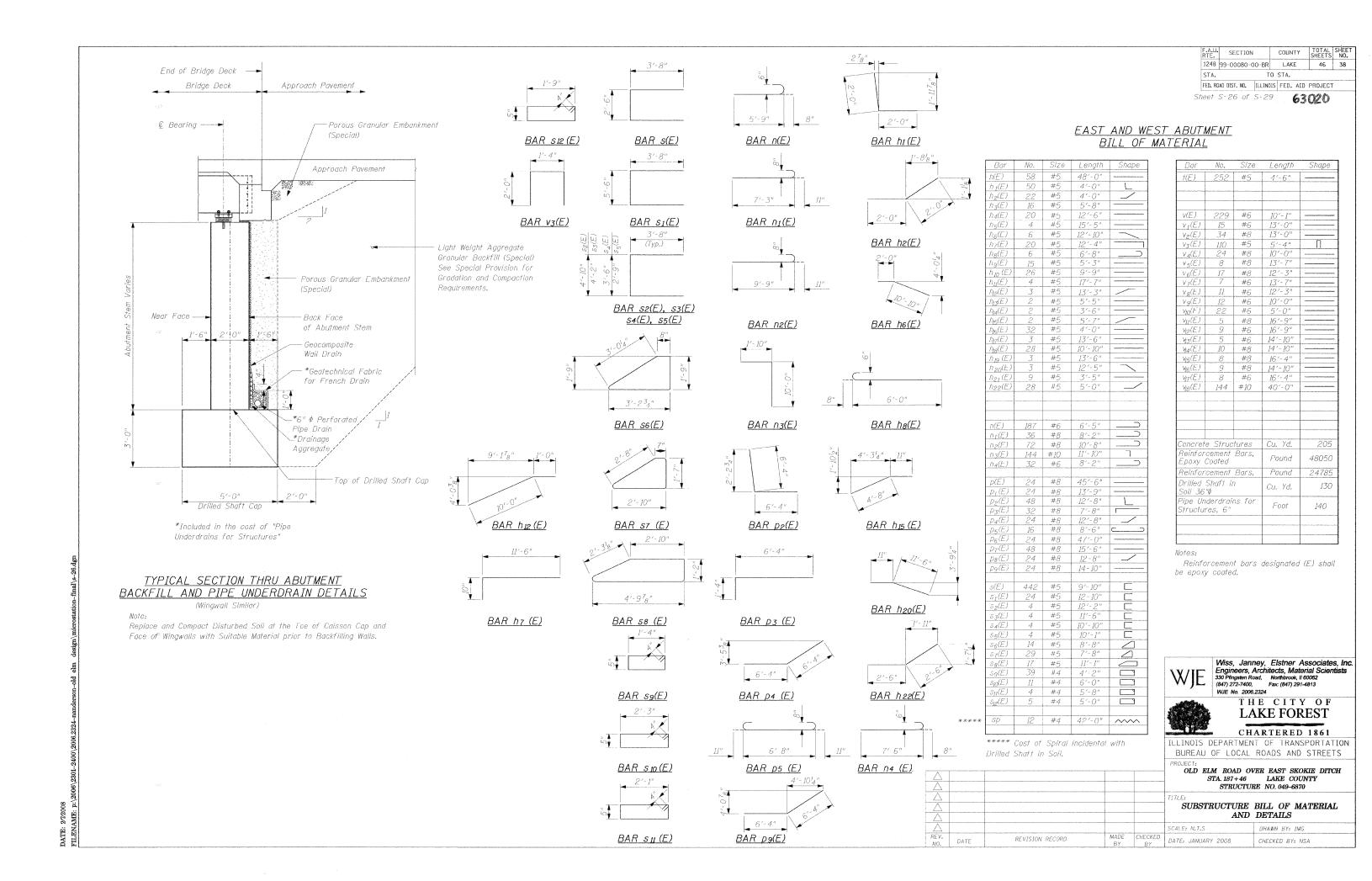
ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

OLD CITE OLD ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

TITLE: WEST ABUTMENT & WINGWALL DETAILS

DRAWN BY: IMG SCALE: N.T.S MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA





Sheet S-27 of S-29

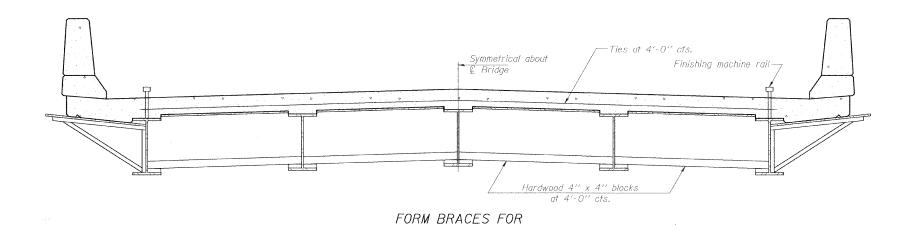
63020

When cantilever forming brackets are used, the work shall be done according to Article 503.06, except as modified below and in the details shown on this sheet.

The finishing machine rails shall be placed on the top flange of the exterior beams.

The beams or girders, supporting cantilever forming brackets, shall be tied together at 4 foot intervals.

For Standard construction, or Stage Construction the Hardwood bracing materials shall be placed as shown between webs of beams in each bay.



STANDARD CONSTRUCTION

CANTILEVER FORMING BRACKETS FOR SUPERSTRUCTURES WITH W27 BEAMS AND SMALLER

Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists
330 Pfingsten Road, Northbrook, II 60062
(847) 272-7400, Fax: (847) 291-4813
WJE No. 2006.2324



MADE CHECKED DATE: JANUARY 2008

REVISION RECORD

THE CITY OF LAKE FOREST CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

PROJECT:
OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

CANTILEVER FORMING BRACKET SCALE: N.T.S DRAWN BY: IMG

CHECKED BY: NSA

SB-1

11-1-06

F.A.U. RTE.	SECTION	V	COUN	TY	TOTAL	SHEET NO.
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Sheet S-28 of S-29

63020

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ROUTE DESCRIPTION	ON Q	ld Eir	n Roc	ıd Bri	dgeLOGGED	BY DA	2	
SECTION 9 U	OCAT	ION .	Lake	Forest	llinois			
COUNTY Lake DRILLIN	IG M	ETHO	O HS	<u> </u>	HAMMER TYPE 1	40-LB		
STRUCT. NO	D E P	M.	e S	M 0 - S		L	ucs	30 - S
Station 187+21 Offset 11' R Ground Surface Elev.0.0 ft.	H	5	Qu	Ť	First Encounter Dry 11.	S	Qu	T
	(11)	/6)(13f)	(X)	After ft. (f)[/6"	(tst)	(%)
ASPHALTIC CONCRETE _0.90 Medium Stiff, Brown, Sity CLAY PILL With Little Grove!	-	3 3		13.8		4 8 4	0.8	14.4
Side Side	_	3 3 35	-	16.6	Very Stiff to Hard, Gray, Silty CLAY	- 5 10	5,1	18.2
de jan	5				-	20 14	-	
	-	2 2 3		19.7]	van de la composition della co	
Very Stiff, Brown, Silty CLAY		2 5 8	3.3	17.2		3 5	2.3	20.4
-11.50		ę. 8:	3.5	15.4				
Very Stiff, Gray, Sifty CLAY NOTE: Encountered Large Gravel at		8						
18.5'20'		4 7 10	3.8	16.7	-	10 0	2.6	19.7
,		6	3.5	17.2		1		·
·		4 7	2.3	17.0		1,	2.7	16.2
- Age	-20	7					-	
	_	6 7	2.2	17.0	-	1		
Stiff, Gray, Silty CLAY	- 1	4 4 5	0.9	16.2		3 5 80 8	2.8	25.3
Stitl, Gray, Siry, Clar. NOTE: With Occasional Sand Seams at 23'-28'					Very Stiff, Gray, Sifty CLAY			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Buige, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (rev. 8-99)

(PS) Illinois	s Depar nsporta	ţm	ent		01	NI DODING LOG	Pag	e 2	of 2
Division of highwa transis Dept. of Tra	ys anspertation					OIL BORING LOG			01-03
SECTION 9						dge LOGGED	BY DAG	<u> </u>	
COUNTY Lake						HAMMER TYPE 14		teritorio managione.	
STRUCT. NO. Station BORING NO. B-1 Station 182+21 Offset 11' R Ground Surface Elec		D E P T H (ff)	8 U W S	U C S Qu (tsf)	M 0 1 S 1 (%)		0 W	U C S Qu (tsf)	M 0 5 1 (%)
Very Stiff, Groy, Silty CLAY	-53.50		3 4	2.2	27.3	(continued)		ANT OR ANT	
		55	5 3 4	2.3	26.3				
		60	7						
END OF BORING ® 55.0'	-65.00	-85	6 8	2.9	22.8			\$\$	The special control of the special particles and the special s
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The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO 7206)
BBS, from 137 (rev. 8-99)

Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Pfingsten Road, Northbrook, Il 60062 (847) 272-7400, Fax: (847) 291-4813 WJE No. 2006.2324



TITLE:

REVISION RECORD

THE CITY OF LAKE FOREST CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

PROJECT:
OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

SOIL BORING LOGS-I

DRAWN BY: IMG MADE CHECKED DATE: JANUARY 2008 CHECKED BY: NSA

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FED. RO	DAD DIST. NO.	ILLINO	IS FED.	AID	PROJECT	
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1248	99-00080-0	00-BR	LAKE		46	41
F.A.U. RTE.	SECTION	1	COUNT	Υ	TOTAL SHEETS	SHEET NO.

(Illinois Depa of Transport	rtm	ent	:	S	OIL BORING LOG	Pa	ge 1	of 3		
Division of highways lilinois Dept. of Transportation					Date 05-0					
SECTION 9					idge LOGGED) BY <u>D</u> i	NG			
COUNTY Loke DRILL	ING M	ETHO	10 H2	A	HAMMER TYPE_	140-LB				
STRUCT. NO. Station BORING NO. B-2 Station:188+02 Offset 22' L Ground Surface Elev. Q.Q. R	0 E P T H	8 0 W S	U C S Qu	M 0 - S T	Groundwater Elev. First Encounter Dry ft. Upon Completion Dry ft.	D 8 L P 0 H S	C S Qu	M 0 1 5 1		
PORTLAND CEMENT 0.00	- K.	1/4	(131)	(10)	After Hrs ft. (continued)	ft)(/6'	f(12)	(7)		
CONCRETE (FILL) -0.50/ Loose, Brown, Moist, SAND and GRAVEL FILL	<u></u>	5 4 4			_	4 6 7	1.9	20.8		
With Silty Clay (FILL)						1	<u> </u>			
Very Stiff, Brown, Silty CLAY LOAM (CL) -5.50		3 3 5	2.25		Stiff, Gray, Silty CLAY	3 -30 6	1.4	20.5		
Hard, Brown, Silty CLAY (CL)		7 12 15	4.5	15.0	to Silty CLAY LOAM (CL) -					
	-10	6 11 14	5.6	14.5	-	4 5 -35 8	1.8	20.0		
Very Stiff to Hard,		6 8 10	2.6	14.1	Hord, Groy, Silty CLAY		The state of the s			
Groy, Silty CLAY (CL)	-15	5 6 9	3.5	15.7		5 8 -40 11	4.1	18.5		
		4 8 12	3.1	16.0	-42.00 Very Dense, Gray, Moist,					
***	-20	4 7 8	2.8	14.1	SILT (ML) -	11 36 -45 48		13.0		
-21.00 Stiff, Groy, Silty CLAY to Silty CLAY LOAM (CL) NOTE: With Occasional		4 6 6	1.3	22.3	-					
Silt Seams at 23'-28'	-25	3 4 6	1.2	20.8	Very Stiff, Gray, Silty CLAY (CL)	5 8 -50 12	3.5	19.2		
·	크				-	∄				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (rev. 8-99)

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ROUTE	DESCRIPTIO	N Q	ld Elr	n Roc	od Bri	dgeLOGGED	BY DAG
SECTION 9	u	CAT	ION	Lake	Forest	Misois	
COUNTY Loke	DRILLIN	G M	ETHO	O HS	A	HAMMER TYPE_	40-LB
STRUCT. NO		D E P H	8 L O ₩ S	Qu (tsf)	M 0 1 S T	Stream Bed Elev	D 8 U M E L C O P O S I T W S H S Qu I
		-55	S 8		12.3	(continued)	
Very Stiff, Gray, Silty	-60,00		4 8 10	2.8	20.1		555
very stiff, Gray, Sifty		- 55	5 7 10	2.2	23.4		900
			4 7	2.5	22.5	,	
END OF BORING @ 70.0'	70.00 	70	8	A THE PARTY OF THE			95
		-75 	· · · · ·				22

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (rev. 8-99)

Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Pfingsten Road, Northbrook, II 60062 (847) 272-7400, Fax: (847) 291-4813 WJE No. 2006.2324



THE CITY OF LAKE FOREST

CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

PROJECT:
OLD ELM ROAD OVER EAST SKOKIE DITCH
STA. 187+46 LAKE COUNTY
STRUCTURE NO. 049-6870

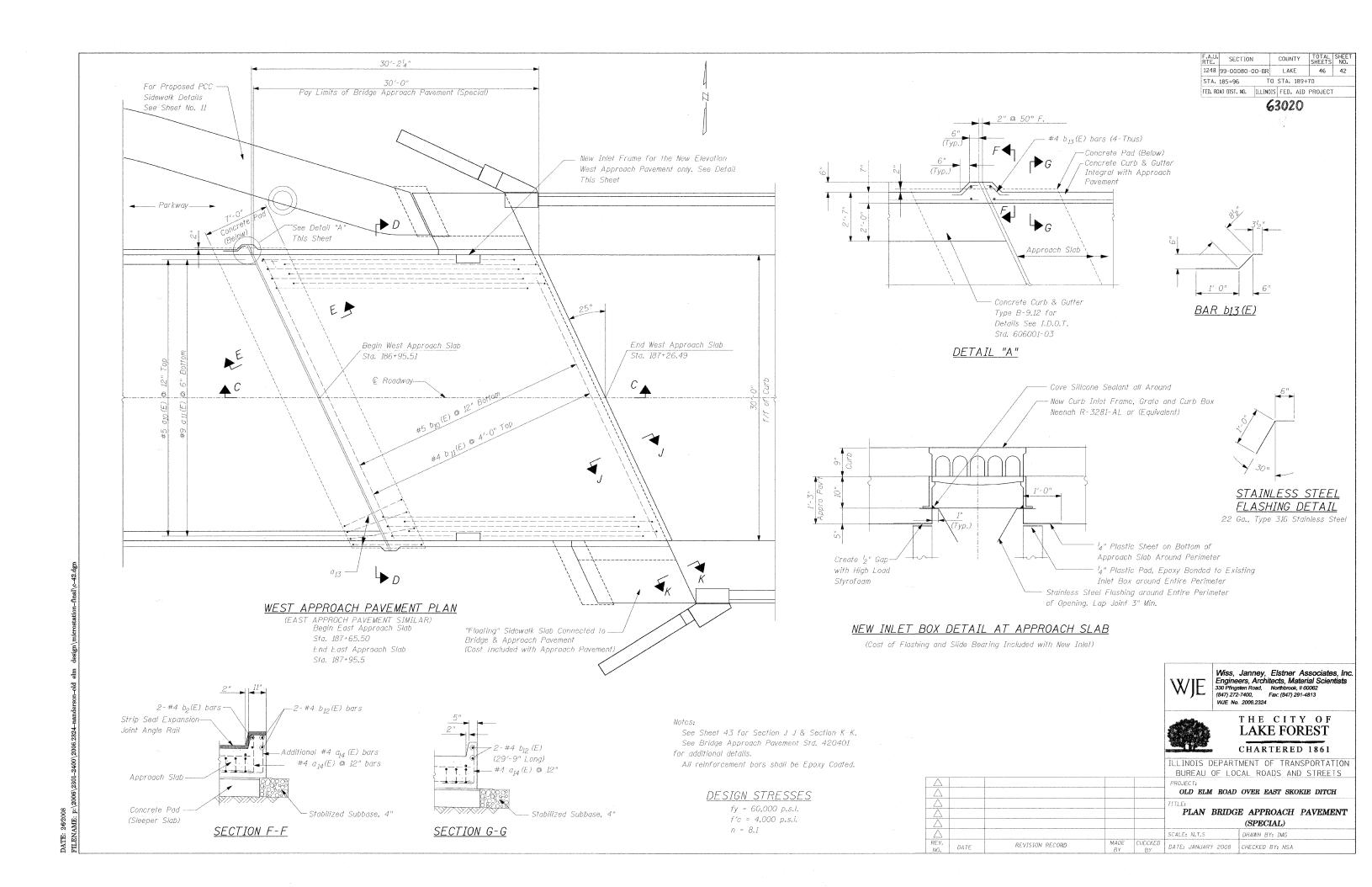
SOIL BORING LOGS-II

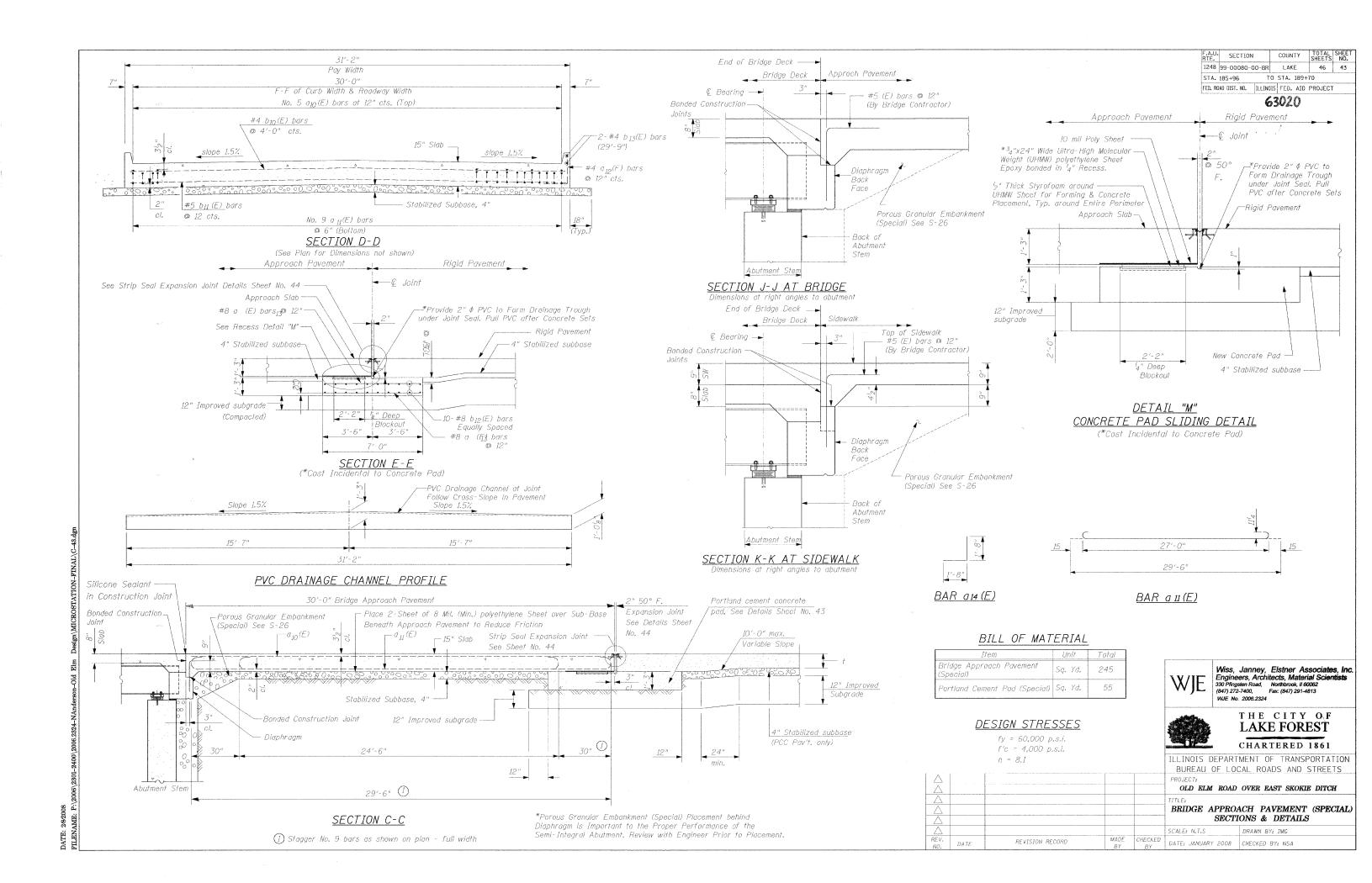
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REVISION RECORD

MADE CHECKED DATE: JANUARY 2008

CHECKED BY: NSA





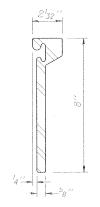
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SECTION THRU ROLLED RAIL EXP. JOINT

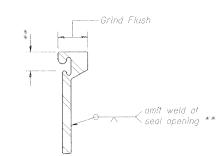
(Studs Reauired)

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



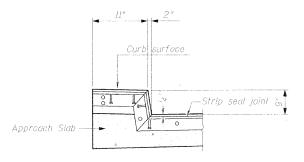
ROLLED (EXTRUDED) RAIL

(Galvanized Finish)



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.



CURB DETAILS AT EXPANSION JOINT*

* Shorter plates with a single row of stude at 12" centers may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

TYPICAL END TREATMENTS

F.A.U. RTE.	SECTION			COUN	TY	TOTAL SHEETS	SHEET NO.
1248	99-00080-00-BR			LAK	E	46	44
STA.	185+96	Т	0	STA.	189+	70	
FED. RO	AD DIST. NO.	ILLING	IS	FED.	AID	PROJECT	

63020

GENERAL NOTES

The strip seal shall be made continuous and shall have a minimum thickness of ${}^{l}_{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Ralls.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed.

Locking Edge Roils may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed.

The joint opening and deck dimensions detailed on the superstructure are based on a preformed joint seal. If the contractor elects to use the alternate strip seal joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

APPROACH PAVEMENT JOINT SYSTEM - EXPANSION (STRIP SEAL)



Wiss, Janney, Elstner Associates, Inc. Engineers, Architects, Material Scientists 330 Pfingsten Road, Northbrook, Il 60062 (647) 272-7400, Fax: (647) 291-4613 WJE No. 2006,2324



THE CITY OF LAKE FOREST CHARTERED 1861

ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF LOCAL ROADS AND STREETS

				- NOOLC 13	
				OLD ELM ROAD	OVER EAST SKOKIE DITCH
 					PPROACH PAVEMENT YSTEM-EXPANSION
				SCALE: N.I.S	DRAWN BY: IMG
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