

April 9, 2020

SUBJECT: FAU Route 297 (US 6) Project STP-1YJA(627) Section 33N-2(12) Will County Contract No. 60V40 Item No. 173, April 24, 2020 Letting Addendum A

NOTICE TO PROSPECTIVE BIDDERS:

Attached is an addendum to the plans or proposal. This addendum involves revised and/or added material.

- 1. Revised the Schedule of Prices
- 2. Revised page iii of the Table of Contents to the Special Provisions
- 3. Revised pages 186-191 of the Special Provisions
- 4. Added pages 230-240 to the Special Provisions
- 5. Revised sheets 5, 12, 76, 77 & 78 of the Plans

Prime contractors must utilize the enclosed material when preparing their bid and must include any changes to the Schedule of Prices in their bid.

Very truly yours,

CLEG

Jack A. Elston, P.E. Bureau Chief, Design and Environment

CONSTRUCTION AIR QUALITY - DIESEL RETROFIT (BDE)	159
DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION (BDE)	161
DISPOSAL FEES (BDE)	171
ELECTRIC SERVICE INSTALLATION (BDE)	172
EMULSIFIED ASPHALTS (BDE)	173
ENGINEER'S FIELD OFFICE AND LABORATORY (BDE)	177
EQUIPMENT PARKING AND STORAGE (BDE)	180
FUEL COST ADJUSTMENT (BDE)	180
GEOTECHNICAL FABRIC FOR PIPE UNDERDRAINS AND FRENCH DRAINS (BDE)	
MANHOLES, VALVE VAULTS, AND FLAT SLAB TOPS (BDE)	191
MOBILIZATION (BDE)	192
PORTLAND CEMENT CONCRETE (BDE)	193
REMOVAL AND DISPOSAL OF REGULATED SUBSTANCES (BDE)	193
SILT FENCE, INLET FILTERS, GROUND STABILIZATION AND RIPRAP FILTER FABR	IC (BDE)
	205
STEEL COST ADJUSTMENT (BDE)	
	211
STEEL COST ADJUSTMENT (BDE)	211 214
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE)	211 214 214 215
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE)	211 214 214 215
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE)	211 214 214 215 215
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE)	211 214 214 215 215 218 218
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE)	211 214 214 215 215 218 218
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE)	211 214 214 215 215 218 218 218 219
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE) TRAINING SPECIAL PROVISIONS (BDE)	211 214 214 215 215 218 218 219 I222
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE) TRAINING SPECIAL PROVISIONS (BDE) IDOT TRAINING PROGRAM GRADUATE ON-THE-JOB TRAINING SPECIAL PROVISION	211 214 214 215 215 218 218 219 I222 224
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE) TRAINING SPECIAL PROVISIONS (BDE) IDOT TRAINING PROGRAM GRADUATE ON-THE-JOB TRAINING SPECIAL PROVISION TRAVERSABLE PIPE GRATE FOR CONCRETE END SECTIONS (BDE)	211 214 214 215 215 218 218 218 219 I222 224 225
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE) TRAINING SPECIAL PROVISIONS (BDE) IDOT TRAINING PROGRAM GRADUATE ON-THE-JOB TRAINING SPECIAL PROVISION TRAVERSABLE PIPE GRATE FOR CONCRETE END SECTIONS (BDE) WARM MIX ASPHALT (BDE)	211 214 214 215 215 218 218 218 219 I222 224 225 227
STEEL COST ADJUSTMENT (BDE) STEEL PLATE BEAM GUARDRAIL MANUFACTURING (BDE) SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) TEMPORARY PAVEMENT MARKING (BDE) TRAFFIC BARRIER TERMINAL, TYPE 1 SPECIAL (BDE) TRAFFIC CONTROL DEVICES - CONES (BDE) TRAINING SPECIAL PROVISIONS (BDE) IDOT TRAINING PROGRAM GRADUATE ON-THE-JOB TRAINING SPECIAL PROVISION TRAVERSABLE PIPE GRATE FOR CONCRETE END SECTIONS (BDE) WARM MIX ASPHALT (BDE) WEEKLY DBE TRUCKING REPORTS (BDE)	211 214 214 215 215 218 218 218 219 I222 227 227 227 227

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MANHOLES, VALVE VAULTS, AND FLAT SLAB TOPS (BDE)

Effective: January 1, 2018 Revised: March 1, 2019

Description. In addition to those manufactured according to the current standards included in this contract, manholes, valve vaults, and flat slab tops manufactured prior to March 1, 2019, according to the previous Highway Standards listed below will be accepted on this contract:

Product	Pre	evious Standar	ds
Precast Manhole Type A, 4' (1.22 m) Diameter	602401-05	602401-04	602401-03
Precast Manhole Type A, 5' (1.52 m) Diameter	602402-01	602402	602401-03
Precast Manhole Type A, 6' (1.83 m) Diameter	602406-09	602406-08	602406-07
Precast Manhole Type A, 7' (2.13 m) Diameter	602411-07	602411-06	602411-05
Precast Manhole Type A, 8' (2.44 m) Diameter	602416-07	602416-06	602416-05
Precast Manhole Type A, 9' (2.74 m) Diameter	602421-07	602421-06	602421-05
Precast Manhole Type A, 10' (3.05 m) Diameter	602426-01	602426	
Precast Valve Vault Type A, 4' (1.22 m) Diameter	602501-04	602501-03	602501-02
Precast Valve Vault Type A, 5' (1.52 m) Diameter	602506-01	602506	602501-02
Precast Reinforced Concrete Flat Slab Top	602601-05	602601-04	

The following revisions to the Standard Specifications shall apply to manholes, valve vaults, and flat slab tops manufactured according to the current standards included in this contract:

HOT-MIX ASPHALT BINDER AND SURFACE COURSE (D-1)

Effective: November 1, 2019 Revised: February 2, 2020

<u>Description</u>. This work shall consist of constructing a hot-mix asphalt (HMA) binder and/or surface course on a prepared base. Work shall be according to Sections 406 and 1030 of the Standard Specifications, except as modified herein.

Materials. Revise Article 1004.03(c) to read:

" (c) Gradation. The coarse aggregate gradations shall be as listed in the following table.

Use	Size/Application	Gradation No.
Class A-1, A-2, & A-3	3/8 in. (10 mm) Seal	CA 16 or CA 20
Class A-1	1/2 in. (13 mm) Seal	CA 15
Class A-2 & A-3	Cover Coat	CA 14
	IL-19.0;	CA 11 ^{1/}
	Stabilized Subbase IL-19.0	
	SMA 12.5 ^{2/}	CA 13 ^{4/} , CA 14, or CA 16
HMA High ESAL	SMA 9.5 ^{2/}	CA 13 ^{3/4/} or CA 16 ^{3/}
	IL-9.5	CA 16, CM 13 ^{4/}
	IL-9.5FG	CA 16
	IL-19.0L	CA 11 ^{1/}
HMA Low ESAL	IL-9.5L	CA 16

9/ CA 16 or CA 13 may be blended with the CA 11.

- 10/ The coarse aggregates used shall be capable of being combined with stone sand, slag sand, or steel slag sand meeting the FA/FM 20 gradation and mineral filler to meet the approved mix design and the mix requirements noted herein.
- 11/ The specified coarse aggregate gradations may be blended.
- 12/ CA 13 shall be 100 percent passing the 1/2 in. (12.5mm) sieve."

Revise Article 1004.03(e) of the Supplemental Specifications to read:

"(e) Absorption. For SMA the coarse aggregate shall also have water absorption ≤ 2.0 percent."

HMA Nomenclature. Revise the "High ESAL" portion of the table in Article 1030.01 to read:

"High ESAL	Binder Courses	IL-19.0, IL-9.5, IL-9.5FG, IL-4.75, SMA 12.5, Stabilized Subbase IL-19.0
-	Surface Courses	IL-9.5, IL-9.5FG, SMA 12.5, SMA 9.5"

Revise Article 1030.02 of the Standard Specifications and Supplemental Specifications to read:

"1030.02 Materials. Materials shall be according to the following.

Item	Article/Section
(a) Coarse Aggregate	
(b) Fine Aggregate	
(c) RAP Material	
(d) Mineral Filler	
(e) Hydrated Lime	
(f) Slaked Quicklime (Note 1)	
(g) Performance Graded Asphalt Binder (Note 2)	
(h) Fibers (Note 3)	
(i) Marm Mix Asphalt (MMA) Technologies (Note	(1)

(i) Warm Mix Asphalt (WMA) Technologies (Note 4)

Note 9. Slaked quicklime shall be according to ASTM C 5.

- Note 10. The asphalt binder shall be an SBS PG 76-28 when the SMA is used on a full-depth asphalt pavement and SBS PG 76-22 when used as an overlay, except where modified herein. The asphalt binder shall be a SBS PG 76-22 for IL-4.75, except where modified herein. The elastic recovery shall be a minimum of 80.
- Note 11. A stabilizing additive such as cellulose or mineral fiber shall be added to the SMA mixture according to Illinois Modified AASHTO M 325. The stabilizing additive shall meet the Fiber Quality Requirements listed in Illinois Modified AASHTO M 325. Prior to approval and use of fibers, the Contractor shall submit a notarized certification by the producer of these materials stating they meet these requirements. Reclaimed Asphalt Shingles (RAS) may be used in Stone Matrix Asphalt (SMA) mixtures designed with an SBA polymer modifier as a fiber additive if the mix design with RAS included meets AASHTO T305 requirements. The RAS shall be from a certified source that produces either Type I or Type 2. Material shall meet requirements noted herein and the actual dosage rate will be determined by the Engineer.

Note 12. Warm mix additives or foaming processes shall be selected from the Department's Qualified Producer List, "Technologies for the Production of Warm Mix Asphalt (WMA)"."

<u>Mixture Design</u>. Revise Article 1030.04(a)(1) of the Standard Specifications and the Supplemental Specifications to read:

High ESAL, MIXTURE COMPOSITION (% PASSING) ^{1/}										
Sieve	IL-19.0	mm	SMA	12.5	SMA	A 9.5	IL-	9.5mm	IL-4.7	75 mm
Size	min	max	min	max	min	max	min	max	min	max
1 1/2 in (37.5 mm)										
1 in. (25 mm)		100								
3/4 in. (19 mm)	90	100		100						
1/2 in. (12.5 mm)	75	89	80	100		100		100		100
3/8 in. (9.5 mm)				65	90	100	90	100		100
#4 (4.75 mm)	40	60	20	30	36	50	34	69	90	100
#8 (2.36 mm)	20	42	16	24 ^{4/}	16	324/	34 ^{5/}	52 ^{2/}	70	90
#16 (1.18 mm)	15	30					10	32	50	65
#30 (600 μm)			12	16	12	18				
#50 (300 μm)	6	15					4	15	15	30
#100 (150 μm)	4	9					3	10	10	18
#200 (75 μm)	3	6	7.0	9.0 ^{3/}	7.5	9.5 ^{3/}	4	6	7	9 ^{3/}
#635 (20 μm)			≤ (3.0	≤ (3.0				
Ratio Dust/Asphalt Binder		1.0		1.5		1.5		1.0		1.0

11/ Based on percent of total aggregate weight.

12/ The mixture composition shall not exceed 44 percent passing the #8 (2.36 mm) sieve for surface courses with Ndesign = 90.

- 13/ Additional minus No. 200 (0.075 mm) material required by the mix design shall be mineral filler, unless otherwise approved by the Engineer.
- 14/ When establishing the Adjusted Job Mix Formula (AJMF) the percent passing the #8 (2.36 mm) sieve shall not be adjusted above the percentage stated on the table.
- 15/ When establishing the Adjusted Job Mix Formula (AJMF) the percent passing the #8 (2.36 mm) sieve shall not be adjusted below 34 percent.

Revise Article 1030.04(b)(1) of the Standard Specifications to read:

"(1) High ESAL Mixtures. The target value for the air voids of the HMA shall be 4.0 percent, for IL-4.75 it shall be 3.5 percent and for Stabilized Subbase it shall be 3.0 percent at the design number of gyrations. The voids in the mineral aggregate (VMA) and voids filled with asphalt binder (VFA) of the HMA design shall be based on the nominal maximum size of the aggregate in the mix and shall conform to the following requirements.

	VOLUMETRIC REQUIREMENTS High ESAL				
	Voids in the Mineral Aggregate (VMA), % minimum			Voids Filled with Asphalt Binder	
Ndesign	IL-19.0; Stabilized Subbase IL- 19.0	IL-9.5	IL-4.75 ^{1/}	(VFA), %	
50			18.5	65 – 78 ^{2/}	
70	13.5	15.0		65 - 75	
90	10.0	10.0		00 - 75	

- 5/ Maximum draindown for IL-4.75 shall be 0.3 percent.
- 6/ VFA for IL-4.75 shall be 72-85 percent."

Revise the table in Article 1030.04(b)(3) to read:

"VOLUMETRIC REQUIREMENTS, SMA 12.5 $^{\rm 1/}$ and SMA 9.5 $^{\rm 1/}$				
NdesignDesign Air Voids Target %Voids in the Mineral Aggregate (VMA), % min.Voids Filled with Asphalt (VFA), %				
80 4/	3.5	17.0 ^{2/} 16.0 ^{3/}	75 - 83	

- 9/ Maximum draindown shall be 0.3 percent. The draindown shall be determined at the JMF asphalt binder content at the mixing temperature plus 30 °F.
- 10/ Applies when specific gravity of coarse aggregate is ≥ 2.760 .
- 11/ Applies when specific gravity of coarse aggregate is < 2.760.
- 12/ Blending of different types of aggregate will not be permitted. For surface course, the coarse aggregate can be crushed steel slag, crystalline crushed stone or crushed sandstone. For binder course, coarse aggregate shall be crushed stone (dolomite), crushed gravel, crystalline crushed stone, or crushed sandstone.

Add to the end of Article 1030.05 (d) (2) a. of the Standard Specifications:

"During production, the Contractor shall test SMA mixtures for draindown according to AASHTO T305 at a frequency of 1 per day of production."

Revise the last paragraph of Article 1102.01 (a) (5) of the Standard Specifications to read:

"IL-4.75 and Stone Matrix Asphalt (SMA) mixtures which contain aggregate having absorptions greater than or equal to 2.0 percent, or which contain steal slag sand, shall have minimum surge bin storage plus haul time of 1.5 hours."

<u>Quality Control/Quality Assurance (QC/QA)</u>. Revise the third paragraph of Article 1030.05(d)(3) to read:

"If the Contractor and Engineer agree the nuclear density test method is not appropriate for the mixture, cores shall be taken at random locations determined according to the QC/QA document "Determination of Random Density Test Site Locations". Core densities shall be determined using the Illinois Modified AASHTO T 166 or T 275 procedure."

Add the following paragraphs to the end of Article 1030.05(d)(3):

- "Longitudinal joint density testing shall be performed at each random density test location. Longitudinal joint testing shall be located at a distance equal to the lift thickness or a minimum of 4 in. (100 mm), from each pavement edge (i.e. for a 5 in. (125 mm) lift the near edge of the density gauge or core barrel shall be within 5 in. (125 mm) from the edge of pavement). Longitudinal joint density testing shall be performed using either a correlated nuclear gauge or cores.
- a. Confined Edge. Each confined edge density shall be represented by a oneminute nuclear density reading or a core density and shall be included in the average of density readings or core densities taken across the mat which represents the Individual Test.
- b. Unconfined Edge. Each unconfined edge joint density shall be represented by an average of three one-minute density readings or a single core density at the given density test location and shall meet the density requirements specified herein. The three one-minute readings shall be spaced 10 ft (3 m) apart longitudinally along the unconfined pavement edge and centered at the random density test location.

When a longitudinal joint sealant (LJS) is applied, longitudinal joint density testing will not be required on the joint(s) sealed."

	"DENSITY CONTROL LIMITS				
Mixture Composition	Parameter	Individual Test (includes confined edges)	Unconfined Edge Joint Density, minimum		
IL-4.75	Ndesign = 50	93.0 – 97.4 % ^{1/}	91.0%		
IL-9.5FG	Ndesign = 50 - 90	93.0 – 97.4 %	91.0%		
IL-9.5	Ndesign = 90	92.0 – 96.0 %	90.0%		
IL-9.5, IL-9.5L,	Ndesign < 90	92.5 – 97.4 %	90.0%		
IL-19.0	Ndesign = 90	93.0 – 96.0 %	90.0%		
IL-19.0, IL-19.0L	Ndesign < 90	93.0 ^{2/} – 97.4 %	90.0%		
SMA	Ndesign = 80	93.5 – 97.4 %	91.0%		

Revise the second table in Article 1030.05(d)(4) and its notes to read:

1/ Density shall be determined by cores or by correlated, approved thin lift nuclear gauge.

2/ 92.0 % when placed as first lift on an unimproved subgrade."

Equipment. Add the following to Article 1101.01 of the Standard Specifications:

- "(h) Oscillatory Roller. The oscillatory roller shall be self-propelled and provide a smooth operation when starting, stopping, or reversing directions. The oscillatory roller shall be able to operate in a mode that will provide tangential impact force with or without vertical impact force by using at least one drum. The oscillatory roller shall be equipped with water tanks and sprinkling devices, or other approved methods, which shall be used to wet the drums to prevent material pickup. The drum(s) amplitude and frequency of the tangential and vertical impact force shall be approximately the same in each direction and meet the following requirements:
 - (1) The minimum diameter of the drum(s) shall be 42 in. (1070 mm);
 - (2) The minimum length of the drum(s) shall be 57 in. (1480 mm);
 - (3) The minimum unit static force on the drum(s) shall be 125 lb/in. (22 N/m); and
 - (4) The minimum force on the oscillatory drum shall be 18,000 lb (80 kN)."

Construction Requirements.

Add the following to Article 406.03 of the Standard Specifications:

Revise the third paragraph of Article 406.05(a) to read:

"All depressions of 1 in. (25 mm) or more in the surface of the existing pavement shall be filled with binder. At locations where heavy disintegration and deep spalling exists, the area shall be cleaned of all loose and unsound material, tacked, and filled with binder (hand method)."

Revise Article 406.05(c) to read.

"(c) Binder (Hand Method). Binder placed other than with a finishing machine will be designated as binder (hand method) and shall be compacted with a roller to the satisfaction of the Engineer. Hand tamping will be permitted when approved by the Engineer."

Revise the special conditions for mixture IL-4.75 in Article 406.06(b)(2)e. to read:

"e. The mixture shall be overlaid within 5 days of being placed."

Revise Article 406.06(d) to read:

"(d) Lift Thickness. The minimum compacted lift thickness for HMA binder and surface courses shall be as follows.

MINIMUM COMPACTED LIFT THICKNESS				
Mixture Composition	Thickness, in. (mm)			
IL-4.75	3/4 (19) - over HMA surfaces ^{1/} 1 (25) - over PCC surfaces ^{1/}			
IL-9.5FG	1 1/4 (32)			
IL-9.5, IL-9.5L	1 1/2 (38)			
SMA 9.5	1 3/4 (45)			
SMA 12.5	2 (51)			
IL-19.0, IL-19.0L	2 1/4 (57)			

1/ The maximum compacted lift thickness for mixture IL-4.75 shall be 1 1/4 in. (32 mm)."

Revise Table 1 and Note 3/ of Table 1 in Article 406.07(a) of the Standard Specifications to read:

"TABLE 1 - MINIMUM ROLLER REQUIREMENTS FOR HMA				
	Breakdown Roller (one of the following)	Intermediate Roller	Final Roller (one or more of the following)	Density Requirement
Binder and Surface ^{1/}	V _D , P ^{3/} , T _B , 3W, O _T , O _B	Р ^{3/} , О _т , О _в	Vs, Tb, T _f , Ot	As specified in Articles: 1030.05(d)(3), (d)(4), and (d)(7).
IL-4.75 and SMA $^{\rm 4/5/}$	Т _{В,} ЗW, От		T_F , 3W, O_T	
Bridge Decks ^{2/}	Тв		TF	As specified in Articles 582.05 and 582.06.

3/ A vibratory roller (V_D) or oscillatory roller (O_T or O_B) may be used in lieu of the pneumatictired roller on mixtures containing polymer modified asphalt binder."

Add the following to EQUIPMENT DEFINITION in Article 406.07(a) contained in the Errata of the Supplemental Specifications:

- "OT Oscillatory roller, tangential impact mode. Maximum speed is 3.0 mph (4.8 km/h) or 264 ft/min (80 m/min).
- O_B Oscillatory roller, tangential and vertical impact mode, operated at a speed to produce not less than 10 vertical impacts/ft (30 impacts/m)."

Delete last sentence of the second paragraph of Article 1102.01(a) (4) b. 2.

Add to the end of Article 1102.01 (a) (4) b. 2.:

"As an option, collected dust (baghouse) may be used in lieu of manufactured mineral filler according to the following:

- (e.) Sufficient collected dust (baghouse) is available for production of the SMA mix for the entire project.
- (f.) A mix design was prepared based on collected dust (baghouse).

Revise Article 1030.04 (d) of the Standard Specifications to read:

"(d) Verification Testing. High ESAL, IL-4.75, and SMA mix designs submitted for verification will be tested to ensure that the resulting mix designs will pass the required criteria for the Hamburg Wheel Test (IL mod AASHTO T-324) and the Tensile Strength Test (IL mod AASHTO T-283). The Department will perform a verification test on gyratory specimens compacted by the Contractor. If the mix fails the Department's verification test, the Contractor shall make the necessary changes to the mix and resubmit compacted specimens to the Department for verification. If the mix fails again, the mix design will be rejected.

All new mix designs will be required to be tested, prior to submittal for Department verification and shall meet the following requirements:

(3)Hamburg Wheel Test criteria. The maximum allowable rut depth shall be 0.5 in. (12.5 mm). The minimum number of wheel passes at the 0.5 in. (12.5 mm) rut depth criteria shall be based on the high temperature binder grade of the mix as specified in the mix requirements table of the plans.

Illinois Modified AASHTO T 324 Requirements ^{1/}

Asphalt Binder Grade	# Repetitions	Max Rut Depth (mm)
PG 70 -XX (or higher)	20,000	12.5
PG 64 -XX (or lower)	10,000	12.5

- 3/ When produced at temperatures of 275 ± 5 °F (135 ± 3 °C) or less, loose Warm Mix Asphalt shall be oven aged at 270 ± 5 °F (132 ± 3 °C) for two hours prior to gyratory compaction of Hamburg Wheel specimens.
- Note: For SMA Designs (N-80) the maximum rut depth is 6.0 mm at 20,000 repetitions. For IL 4.75mm Designs (N-50) the maximum rut depth is 9.0mm at 15,000 repetitions.
- (2) Tensile Strength Criteria. The minimum allowable conditioned tensile strength shall be 60 psi (415 kPa) for non-polymer modified performance graded (PG) asphalt binder and 80 psi (550 kPa) for polymer modified PG asphalt binder. The maximum allowable unconditioned tensile strength shall be 200 psi (1380 kPa)."

<u>Production Testing</u>. Revise first paragraph of Article 1030.06(a) of the Standard Specifications to read:

"(a) High ESAL, IL-4.75, WMA, and SMA Mixtures. For each contract, a 300 ton (275 metric tons) test strip, except for SMA mixtures it will be 400 ton (363 metric ton), will be required at the beginning of HMA production for each mixture at the beginning of each construction year according to the Manual of Test Procedures for Materials "Hot Mix Asphalt Test Strip Procedures". At the request of the Producer, the Engineer may waive the test strip if previous construction during the current construction year has demonstrated the constructability of the mix using Department test results."

Add the following after the sixth paragraph in Article 1030.06 (a) of the Standard Specifications:

"The Hamburg Wheel test shall also be conducted on all HMA mixtures from a sample taken within the first 500 tons (450 metric tons) on the first day of production or during start up with a split reserved for the Department. The mix sample shall be tested according to the Illinois Modified AASHTO T 324 and shall meet the requirements specified herein. Mix production shall not exceed 1500 tons (1350 metric tons) or one day's production, whichever comes first, until the testing is completed and the mixture is found to be in conformance. The requirement to cease mix production may be waived if the plant produced mixture demonstrates conformance prior to start of mix production for a contract.

If the mixture fails to meet the Hamburg Wheel criteria, no further mixture will be accepted until the Contractor takes such action as is necessary to furnish a mixture meeting the criteria"

Method of Measurement:

Add the following after the fourth paragraph of Article 406.13 (b):

"The plan quantities of SMA mixtures shall be adjusted using the actual approved binder and surface Mix Design's Gmb."

Basis of Payment. Replace the second through the fifth paragraphs of Article 406.14 with the following:

"HMA binder and surface courses will be paid for at the contract unit price per ton (metric ton) for MIXTURE FOR CRACKS, JOINTS, AND FLANGEWAYS; HOT-MIX ASPHALT BINDER COURSE (HAND METHOD), of the Ndesign specified; HOT-MIX ASPHALT BINDER COURSE, of the mixture composition and Ndesign specified; HOT-MIX ASPHALT SURFACE COURSE, of the mixture composition, friction aggregate, and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT BINDER COURSE (HAND METHOD), of the Ndesign specified; POLYMERIZED HOT-MIX ASPHALT BINDER COURSE (HAND METHOD), of the Ndesign specified; POLYMERIZED HOT-MIX ASPHALT BINDER COURSE, of the mixture composition and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT BINDER COURSE, of the mixture composition and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, of the mixture composition, friction aggregate, and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT, of the mixture composition and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT, of the mixture composition and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT, of the mixture composition and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT, of the mixture composition and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, STONE MATRIX ASPHALT, of the mixture composition and Ndesign specified; POLYMERIZED HOT-MIX ASPHALT, of the mixture composition, friction aggregate, and Ndesign specified."