TBM 04/14/08B - RR spike in E face of 18" maple tree, 17.1' Lt. of Sta. 12+22.8 - Elev. 541.02

Concrete -

SOUTH ABUTMENT

Steel Piles, HP12x53-

Embankment

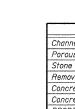
Berm El. 541.10

Excavation

on closed timber abutments and timber pier with concrete caps, 52' Bk, to Bk, abutments, 22.5' Out to Out of deck. Existing Structure Number 026-3313 To be removed. See Special Provisions.

ROUTE SECTION SHEETS NO. TR 518 FAYETTE 08-08122-00-BR 11 FED. ROAD DIST, NO. 7 ILLINOIS

CONTRACT NO. 95576



RIPRAP ANCHOR DETAIL

-Curled End Section, Typ. All 4 Corners See Sheet 8 for det

2'-0"

NORTH ABUTMENT

Encasemen

Steel Piles, HP12x53

Embankment

BILL OF MATERIALS (BRIDGE ONLY)

ITEM	UNIT	SUPER	SUB	TOTAL
Channel Excavation	Cu Yd	-	270	270
Porous Granular Embankment	Ton	-	76	76
Stone Dumped Riprap, Class A4	Ton	-	125	125
Removal of Existing Structures	Each	-	-	1
Concrete Structures	Cu Yd	-	21.0	21.0
Concrete Encasement	Cu Yd	-	2.8	2.8
PPCDB (33" Depth)	Sq Ft	1648	-	1648
Reinforcement Bars	Pound	-	3420	3420
Steel Railing, Type S1	Foot	140	-	140
Furnishing Steel Piles HP12x53	Foot	-	243	243
Driving Piles	Foot	-	243	243
Test Pile Steel HP12x53	Each	-	1	1
Name Plates	Each	-	1	1
				1

GENERAL NOTES

See Section 502 of the Standard Specifications for Structural Excavation.

Layout of siope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

Channel excavation shall be excavated as shown within the limits of the proposed bridge, then tapered to the existing channel at the ROW line. If the Engineer deems the material satisfactory, it may be used to construct the roadway embankment.

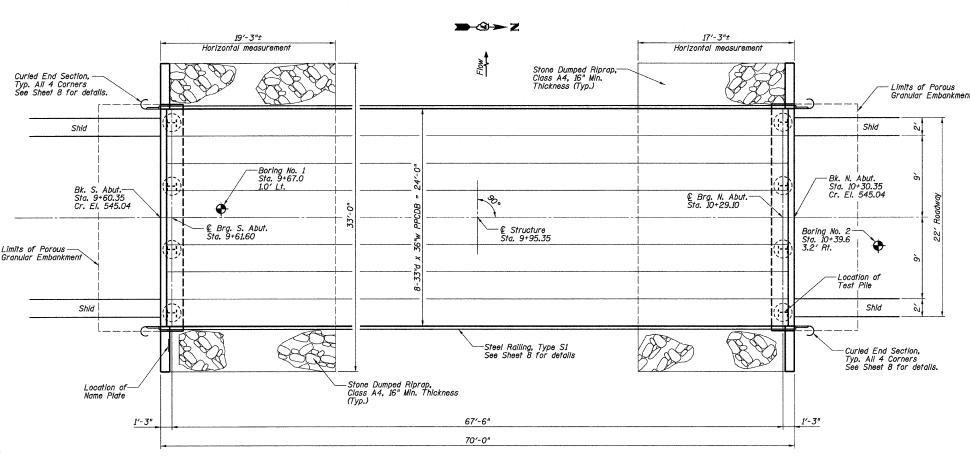
See Specifications for Soil Borings.

Do not scale these drawings.

The Steel H-piles shall be according to AASHTO M270 Grade 50.

In addition to all other requirements of Section 512 of the Standard Specifications, splices for Steel H-piles shall develop the full capacity of the steel's cross sectional area of the pile for tension, shear and bending forces. One approved method of achieving this requirement is full penetration but welding of the entire cross section. Other types of splices meeting the full capacity requirement may be allowed subject to the approval of the Engineer. Any proposal by the Contractor to use an alternate splice method must include adequate documentation demonstrating that the full tension, shear and bending capacities will be met. Appropriate welder qualifications will be required for the positions and processes used in splicing all piles. Nondestructive testing of completed welds will be limited to visual inspection.

The abutment bearing seat surfaces for the precast prestressed concrete deck beams shall be adjusted by shimming to assure firm and even bearing. As required, by fabric adjusting shims of the dimensions of the Exterior Bearing Pad shall be



DHW15 EL. 541.10

-Riprap Anchor (Typ.)

Existing

ELEVATION

Structure

El. 530.27±

0

33"d x 36"w PPCDB (Typical)

Channel -

BUILT 200_ BY FAYETTE COUNTY SEC. 08-08122-00-BR LOADING HL-93 STRUCTURE NO. 026-3443

NAME PLATE (See State Standard 515001 for details)

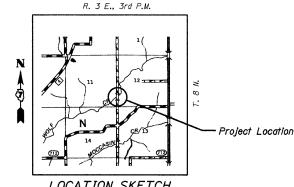
PROFILE GRADE ACROSS STRUCTURE Alona & Roadway

I certify that to the best of my information, knowledge, and belief, this bridge is structurally adequate for the design loading shown on plans. The design is an economical one for the structure and complies with requirements of the current AASHTO Standard Specifications for Highway Bridges.



Gary L. Hahn

Illinois Licensed Structural Engineer No. 81-4853 Expires Nov. 30, 2010



LOCATION SKETCH

GENERAL PLAN AND ELEVATION PROPOSED BRIDGE OVER WOLF CREEK

FAYETTE COUNTY, ILLINOIS

TR 518 SECTION 08-08122-00-BR

Sheet

Job No. 50608

Flood

Design

Max. Calc

3ase

RHUTASEL and ASSOCIATES, INC. CONSULTING ENGINEERS • LAND SURVEYORS FREEBURG, ILLINOIS

6059 380

100 9723 412

Drainage Area = 51.44 Sq. Mi.

WATERWAY DATA

459

Low Grade Elev. 538.37 @ Sta. 6+69.97

Opening Sq. Ft. Natural Head - Ft. Headwater El.

Exist. | Prop. | H.W.E. | Exist. | Prop. | Exist. | Prop.

541.10 0.12 0.10 541.22

504 541.79 0.12 0.11 541.91 541.90

DESIGN STRESSES

PRECAST PRESTRESSED UNITS

= 60,000 psi (reinforcement)

PLAN

FIELD UNITS $f_{o}' = 3,500 \text{ psi}$ = 60,000 psi (reinforcement)

 $f_c' = 6,000 \, psi$ $f'_{ci} = 5,000 psi$ 270,000 psi $\binom{l_2}{\prime\prime}$ ϕ low lax. strands) $f_{pbt} = 201,960 \text{ psi } (\frac{1}{2})'' \text{ p low lax. strands})$

DESIGN SPECIFICATIONS

2007 AASHTO LRFD with all applicable interims

LOADING HL-93

25#/sq. ft. included in dead load for future wearing surface.