

**EXISTING FRAMING PLAN**  
(Existing W36 Composite Girders to Remain)

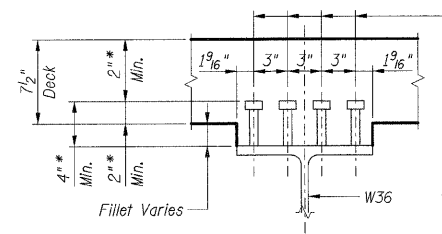
- Notes:**
- For diaphragm details see Sheet S-014.
  - Diaphragms labeled D3 are to be removed and replaced as shown on Sheet S-014.

**INTERIOR GIRDER MOMENT TABLE**

	0.4 Span 1 or 0.6 Span 2	Pier
$I_s$	(in <sup>4</sup> ) 12100	19830
$I_c$ (n)	(in <sup>4</sup> ) 28065	
$I_c$ (3n)	(in <sup>4</sup> ) 20202	
$S_s$	(in <sup>3</sup> ) 664	1030
$S_c$ (n)	(in <sup>3</sup> ) 923	
$S_c$ (3n)	(in <sup>3</sup> ) 825	
Z	(in <sup>3</sup> )	
DL	(k/ft) 0.846	1.350
M DL	(k-ft) 416	1261
sDL	(k/ft) 0.429	
M sDL	(k-ft) 230	
M LL	(k-ft) 613	457
M (Imp)	(k-ft) 145	108
5/3 [M LL + M (Imp)]	(k-ft) 1263	943
M <sub>a</sub>	(k-ft) 2481	2865
M <sub>u</sub>	(k-ft) 2809	
f <sub>s</sub> DL (non-comp)	(ksi) 7.5	14.7
f <sub>s</sub> DL (comp)	(ksi) 3.3	
f <sub>s</sub> 5/3 [M LL + M (Imp)]	(ksi) 16.4	11.0
f <sub>s</sub> (Overload)	(ksi) 27.3	25.7
f <sub>s</sub> (Total)	(ksi)	33.4
VR	(k)	54.0

**INTERIOR GIRDER REACTION TABLE**

	Abutments	Pier
R DL	(k) 40.8	140.0
R LL	(k) 40.4	53.9
Imp.	(k) 9.6	12.7
R (Total)	(k) 90.8	206.6



**EXISTING SHEAR CONNECTOR DETAIL**

\* Stud heights and vertical clearances apply to replacement studs (if required).

Existing 7/8"  $\phi$  stud shear connectors to remain. (total number of existing studs = 7168)  
Shear connectors damaged during deck removal shall be replaced by the Contractor at the Contractor's expense with 7/8"  $\phi$  granular or solid flux filled studs automatically end welded to flange.

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section used in computing  $f_s$  (Total and Overload).  
 $I_c$  and  $S_c$  are the moment of inertia and section modulus of the composite section used in computing  $f_s$  (Total and Overload).  
VR is the maximum Live Load + Impact shear range within the composite portion of the span.  
Z is the plastic section modulus used to determine the fully plastic moments in the non-composite areas.  
 $M_a$  (Applied Moment) = 1.3 [M DL + M sDL + 5/3 (M LL + M (Imp))].  
 $M_u$  is the maximum bending strength. (compact, braced section)  
 $f_s$  (Overload) is the sum of the stresses due to M DL + M sDL + 5/3 (M LL + M (Imp)).  
 $f_s$  (Total) is the sum of the stresses due to 1.3 [M DL + M sDL + 5/3 (M LL + M (Imp))]. (non-compact section)

SHT. S-013

REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION  
F.A.I. ROUTE 57 (INTERSTATE 57)  
112TH PLACE BRIDGE DECK REPLACEMENT  
112TH PLACE OVER I-57  
SN 016-0988 STA. 546+15.56  
COOK COUNTY

STEEL FRAMING PLAN

DATE: 01/16/09

DRAWN BY: CCE  
CHECKED BY: JLR

**TENG**  
TENG & ASSOCIATES, INC.  
ENGINEERS ARCHITECTS/PLANNERS  
205 N. MICHIGAN AVE., CHICAGO, IL 60601  
TELEPHONE: 312.616.0000

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