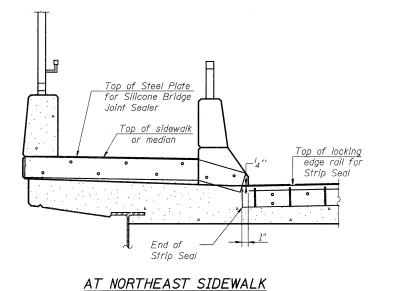
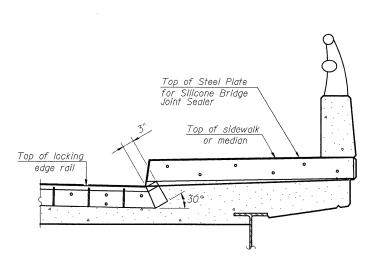
ROUTE NO.	SECTION	COL	COUNTY		SHEET NO.	SHEET NO. 3
FAI 74	*	ROCK ISLAND		246	201	12 sheets
FED. ROAD DIST. NO. 7		ILLINDIS	FED. AID PR	OJECT-		

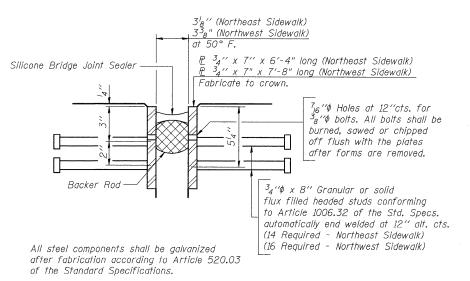
* 81 (1-2, 1, 2-2) RS-1 & M

FED. AID PROJECT-





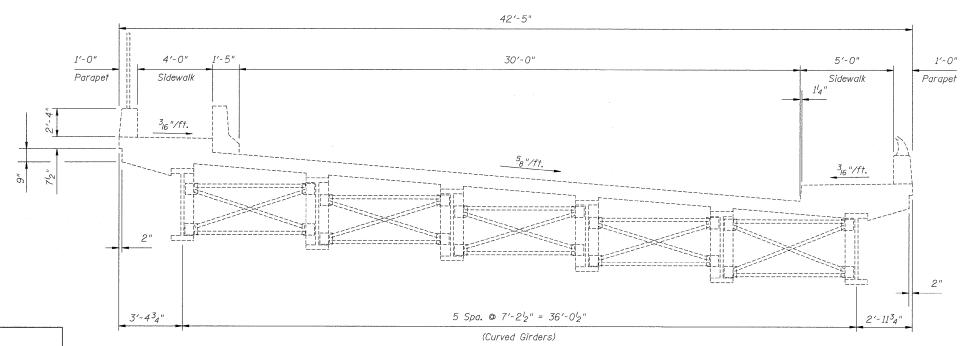
AT NORTHWEST SIDEWALK

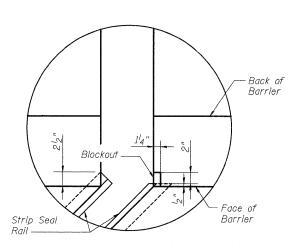


Assembly shown, including material and labor required to fabricate and install Steel Rails shall be included in cost of Silicone Bridge Joint Sealer.

SECTION THROUGH SIDEWALK JOINT

(North Abutment)





NORTHEAST BARRIER STRIP SEAL BLOCKOUT

BRIDGE REPAIR DETAILS

N.B. 27TH ST. OVER F.A.I. 74

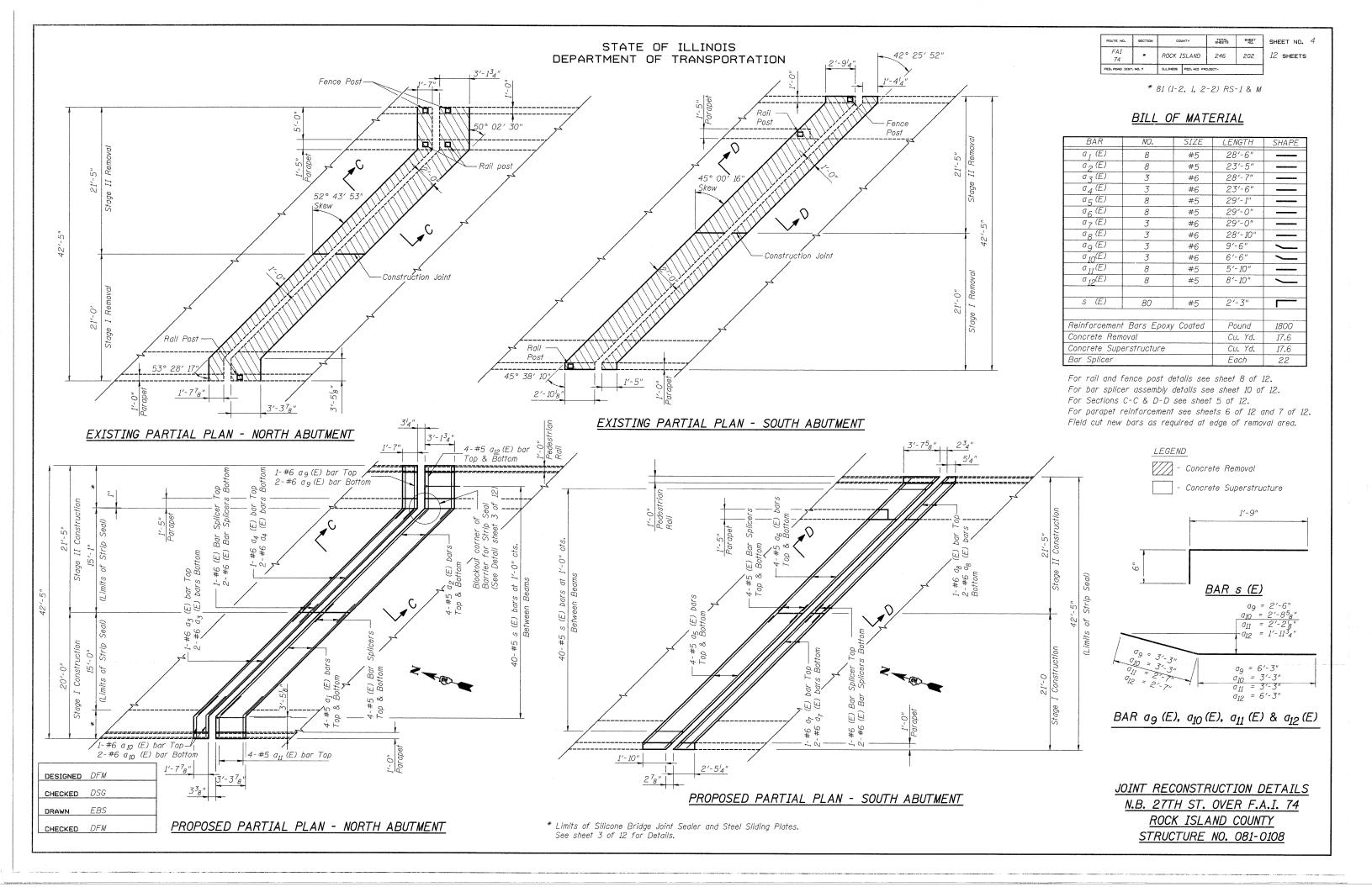
ROCK ISLAND COUNTY

STRUCTURE NO. 081-0108

DESIGNED	DFM
CHECKED	CTH
DRAWN	EBS
CHECKED	DEM

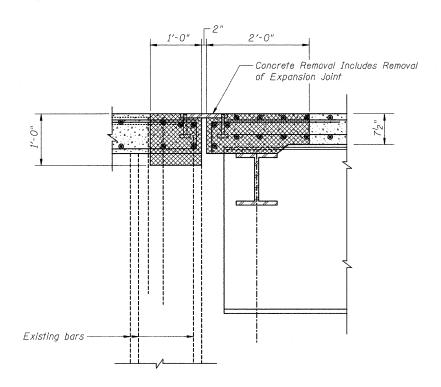
CROSS SECTION

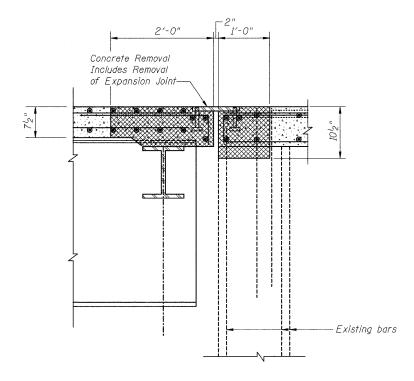
(Looking South)

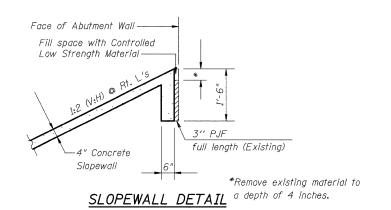


ROUTE NO.	SECTION	COL	COUNTY		SHEET NO.	SHEET NO. 5
FAI 74	*	ROCK ISLAND		246	203	12 SHEETS
FEO. ROAD DIST. NO. 7		ILLINOIS	FED. AID PRO	JECT-		

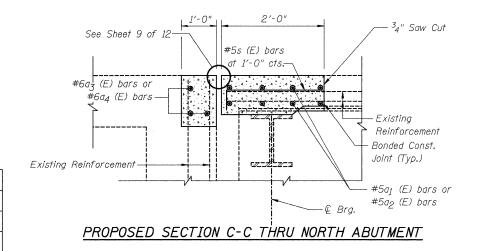
* 81 (1-2, 1, 2-2) RS-1 & M







EXISTING SECTION C-C THRU NORTH ABUTMENT



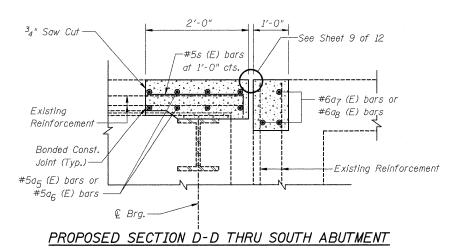
DESIGNED DFM

CHECKED DSG

DRAWN EBS

CHECKED DFM

EXISTING SECTION D-D THRU SOUTH ABUTMENT



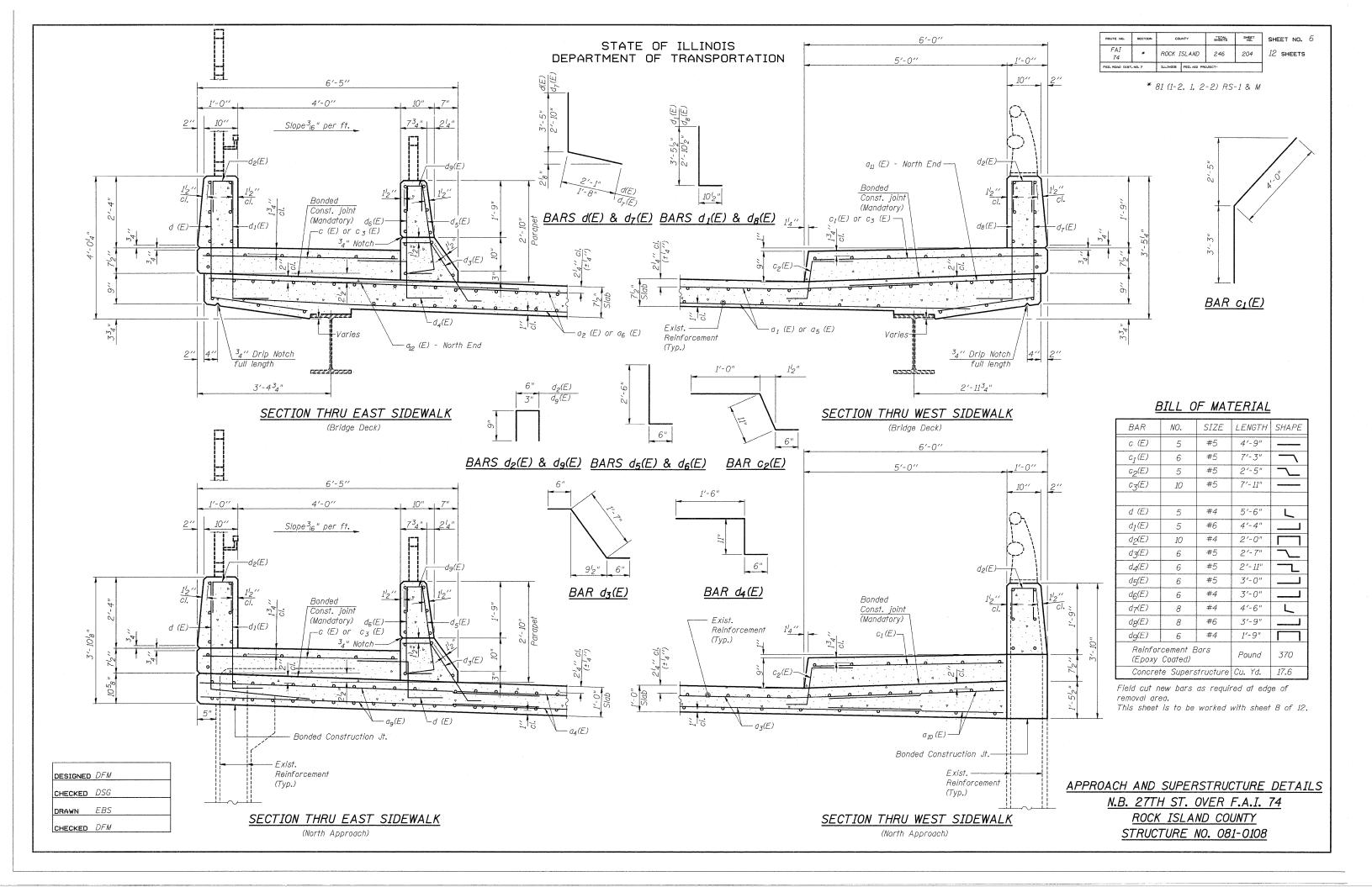
Concrete Removal

Dimensions are based on a Rolled Rail Strip Seal Joint. If the contractor elects to use the Welded Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on sheet 9 of 12.

N.B. 27TH ST. OVER F.A.I. 74

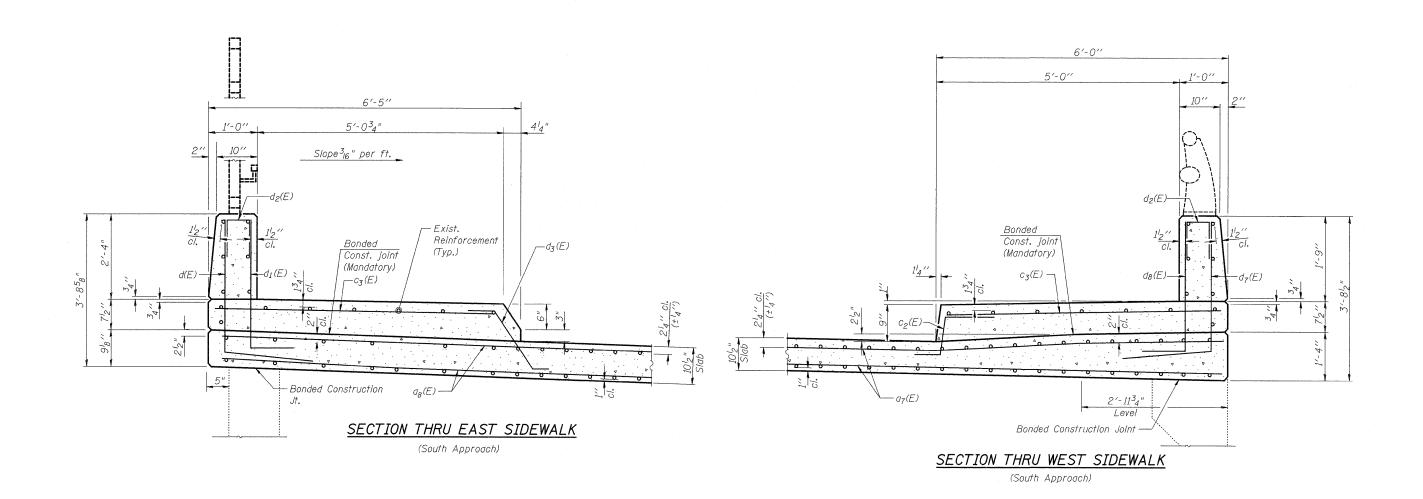
ROCK ISLAND COUNTY

STRUCTURE NO. 081-0108



ROUTE NO.	SECTION	ca	JNTY	TOTAL SHEETS	SHEET NO.	SHEET	NO.	7
FAI 74	*	ROCK ISLAND		246	205	<i>12</i> sı	HEETS	
EED BOAD DIST	NO. 7	TI I TNOTE END ATD PRO		DIECT-				

* 81 (1-2, 1, 2-2) RS-1 & M



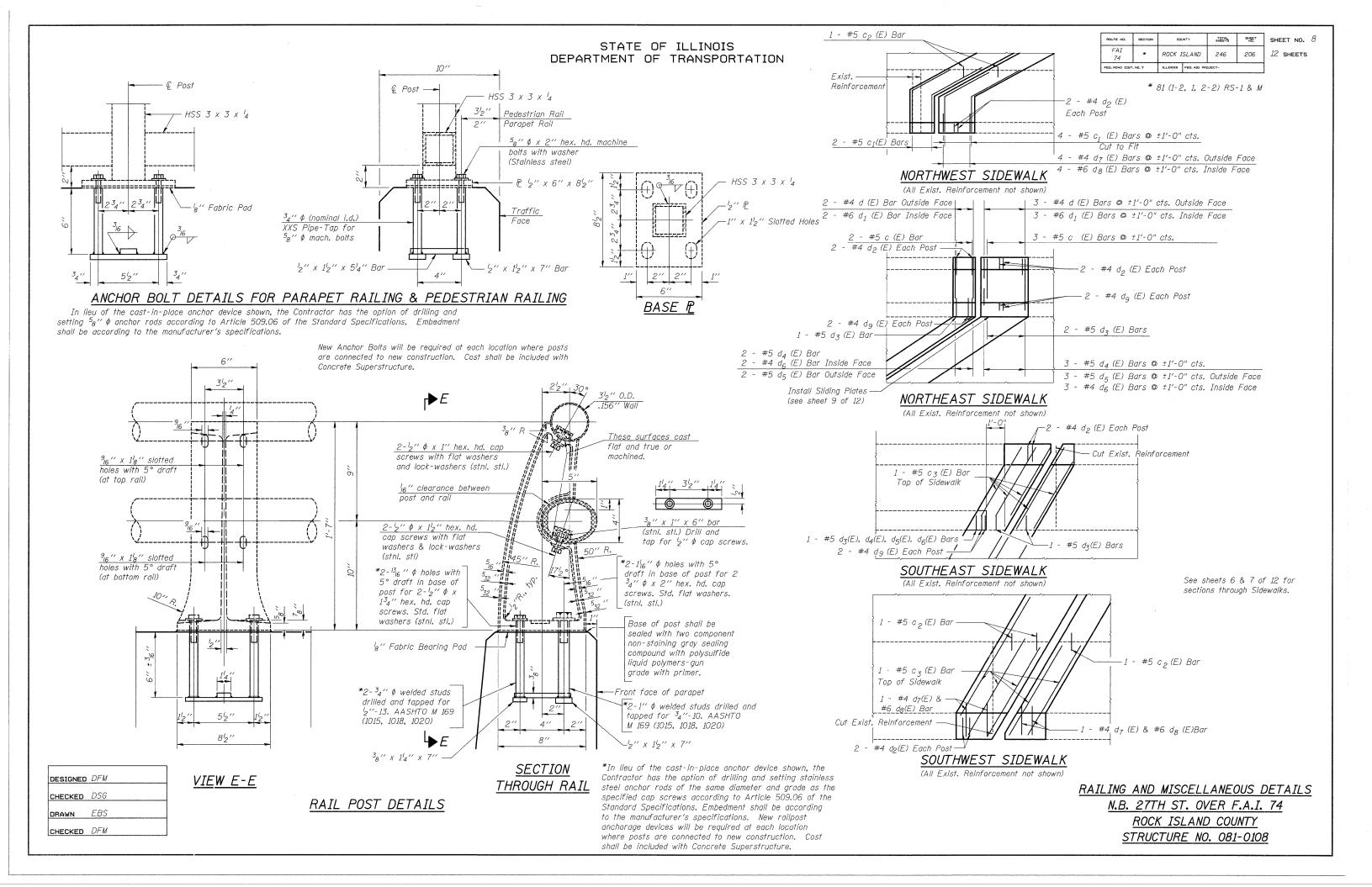
DESIGNED	DFM	
CHECKED	DSG	
DRAWN	EBS	
CHECKED	DFM	

APPROACH AND SUPERSTRUCTURE DETAILS

N.B. 27TH ST. OVER F.A.I. 74

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0108



 $\frac{1'_2"}{at 50^{\circ} F}$ \int Top of slab

ROUTE NO. COUNTY TOTAL SHEETS SHEET NO. SECTION FAI 74 ROCK ISLAND 246 207

SHEET NO. 9 12 SHEETS

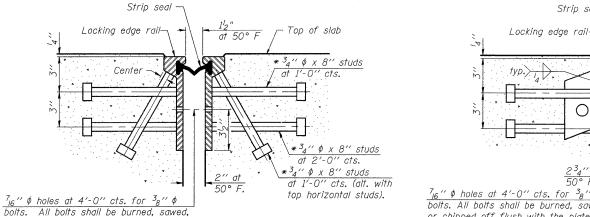
* 81 (1-2, 1, 2-2) RS-1 & M

The strip seal shall be made continuous and shall have a minimum thickness of \(\frac{1}{4} \). The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.



at 2'-0" cts. Anchor plate Place plates at 1'-0" cts. 7 ₁₆ " ϕ holes at 4'-0" cts. for 3 ₈" ϕ (alt. with top horizontal studs) bolts. All bolts shall be burned, sawed, or chipped off flush with the plates after forms are removed, typ.

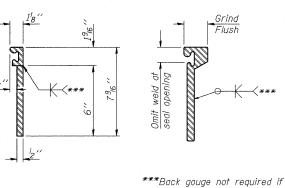
SECTION THRU

WELDED RAIL JOINT

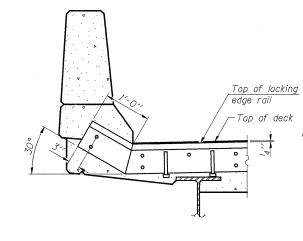
(for welded rail,

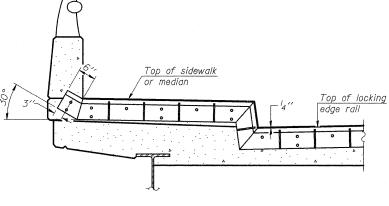
Strip seal-

SECTION THRU ROLLED RAIL JOINT









AT PARAPET

AT SIDEWALK OR MEDIAN

Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

ROLLED EXTRUDED RAIL

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std.

Specs., automatically end welded.

or chipped off flush with the plates

after forms are removed, typ.

WELDED RAIL

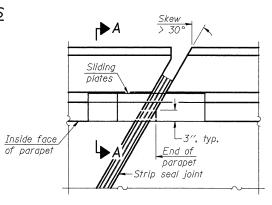
LOCKING EDGE RAIL SPLICE

complete joint penetration

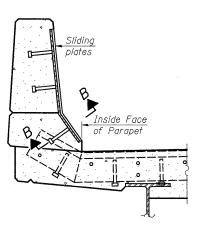
is verified by mock-up.

The inside of the locking edge rail groove shall be free of weld residue.

LOCKING EDGE RAILS



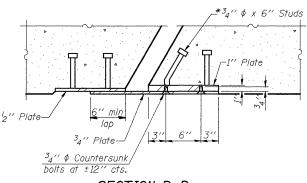
PLAN



SECTION A-A

POINT BLOCK DETAILS (for skews > 30°)

TYPICAL END TREATMENTS



	SE	C7	TON	B-	В
--	----	----	-----	----	---

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	123

EXPANSION JOINT DETAILS N.B. 27TH ST. OVER F.A.I. 74 ROCK ISLAND COUNTY STRUCTURE NO. 081-0108

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

EJ-SSJ

10-1-08



SHEET NO. 10 12 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

(Tension in kips) = 1.25 x fy x A 1

Minimum *Pull-out Strength = $0.66 \times fy \times A$

9'-0"

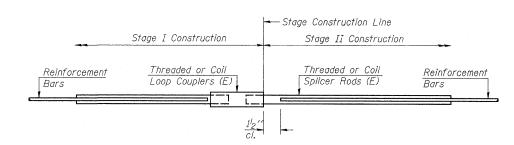
Where fy = Yield strength of lapped reinforcement bars in ksi. A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

#11

	BAR SPLIC	ER ASSEMBLI	ES		
5 0 1		Strength Requirements			
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length		Min. Pull-Out Strength kips - tension		
#4	1'-8''	14.7	7.9		
#5	2'-2"	23.0	12.3		
#6	2'-7''	33.1	17.4		
#7	3′-5′′	45.1	23.8		
#8	4'-6''	58.9	31.3		
#9	5′-9′′	75.0	39.6		
#10	7′-3′′	95.0	50.3		

117.4

61.8



STANDARD

Bar Size	No. Assemblies Required	Location
#5	8	Bridge Deck at South Abutment
#5	8	Bridge Deck at North Abutment
#6	3	South Abutment Backwall
#6	3	North Abutment Backwall

←Stage Construction Line Template <u>"A"</u> Threaded or Coil Forms --Foam Plugs Splicer Rods (E) -Washer Face <u>"B"</u>

BAR SPLICER ASSEMBLY ALTERNATIVES

The diameter of this part is equal or larger than the

diameter of bar spliced.

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

WELDED SECTIONS

ROLLED THREAD DOWEL BAR

** ONE PIECE

- Wire Connector

ŢijŢijijŢ

The diameter of this part

of the bar spliced.

is the same as the diameter

INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

Approach slab

FOR STUB

ABUTMENTS

Abutment hatch block

Threaded or Coil

Loop Couplers (E)

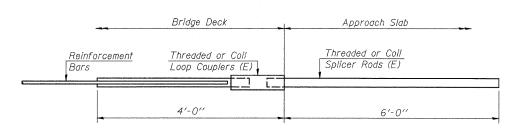
(E): Indicates epoxy coating.

6'-0"

Threaded or Coil

Splicer Rods (E)

-Reinforcement bars



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required =

> Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required =

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

BSD-1

10-1-08

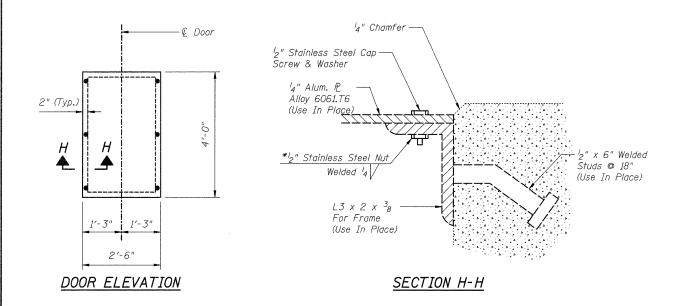
BAR SPLICER ASSEMBLY DETAILS N.B. 27TH ST. OVER F.A.I. 74 ROCK ISLAND COUNTY STRUCTURE NO. 081-0108

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET	SHEET NO	11
FAI 74	*	ROCK ISLAND		246	209	12 SHEETS	
FED. ROAD DIST	NO. 7	ILLINDIS FED. AID PRO		JECT-			

* 81 (1-2, 1, 2-2) RS-1 & M

BILL OF MATERIAL

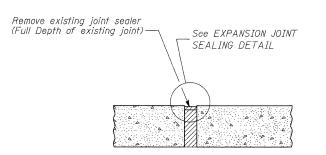
ITEM	NORTH ABUTMENT	SOUTH ABUTMENT	TOTAL
Access Door	1	1	2



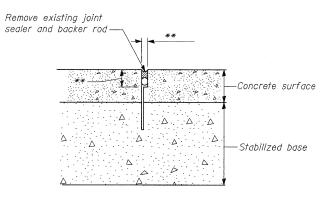
* Replace existing nuts with nuts matching new bolts. Field Weld.
Install cap screw and washer. Total 6 locations for each access door.

ACCESS DOOR DETAILS

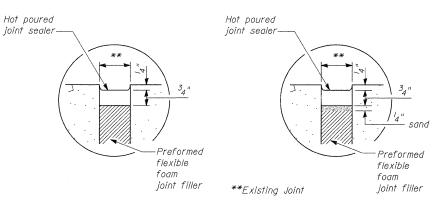
DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DEM



EXISTING EXPANSION JOINT



TRANSVERSE CONTRACTION JOINT



EXPANSION JOINT SEALING DETAIL



Location for joint sealer shown on sheet 1 of 12 is approximate. Approach pavement joint final locations will be determined by Resident Engineer during construction. Actual repair locations shall be shown on as-built plans. Contractor will be paid for the quantity furnished. Joint sealer shall be placed in accordance with the Special Provision.

APPROACH PAVEMENT JOINT DETAILS

ACCESS DOOR AND

APPROACH PAVEMENT JOINT DETAILS

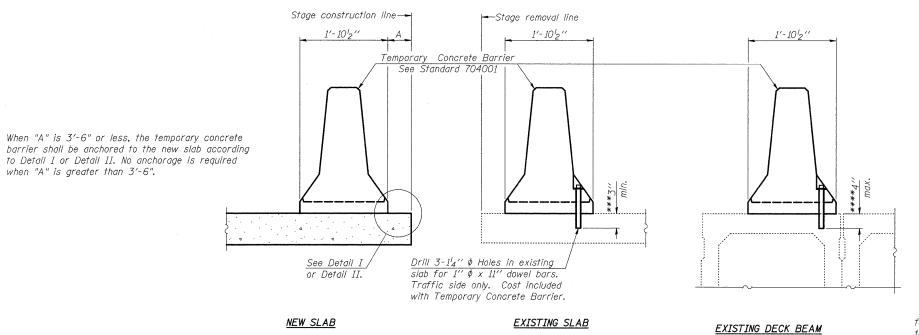
N.B. 27TH ST. OVER F.A.I. 74

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0108

	ROUTE NO.	SECTION	coi	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 12
	FAI 74	*	ROCK 1	SLAND	246	210	<i>12</i> SHEETS
i	FED. ROAD DIST	, NO. 7	ILLINOIS	FED. AID PRO	DJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



NOTES

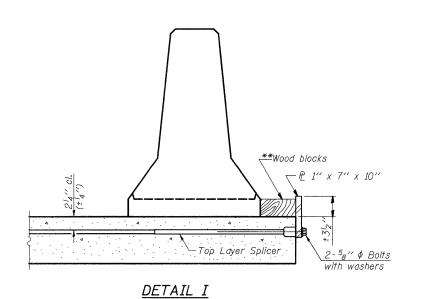
Detail I - With Bar Splicer or Couplers: Connect one (1) 1"x7"x10" steel P to the top layer of couplers with $2^{-5}8'' \phi$ bolts screwed to coupler at approximate © of each barrier panel.

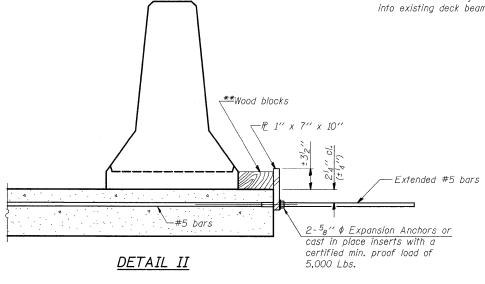
Detail II - With Extended Reinforcement Bars: Connect one (1) 1"x7"x 10" steel 12 to the concrete slab or concrete wearing surface with $2^{-5}8''$ ϕ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate € of each barrier panel.

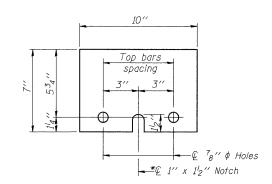
Cost of anchorage is included with Temporary Concrete Barrier. The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

- *** Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.
- **** If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.







STEEL RETAINER P 1" x 7" x 10"

* Required only with Detail II

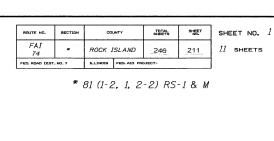
**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

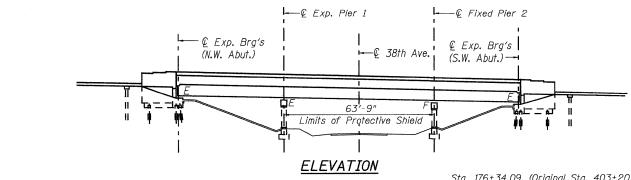
DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

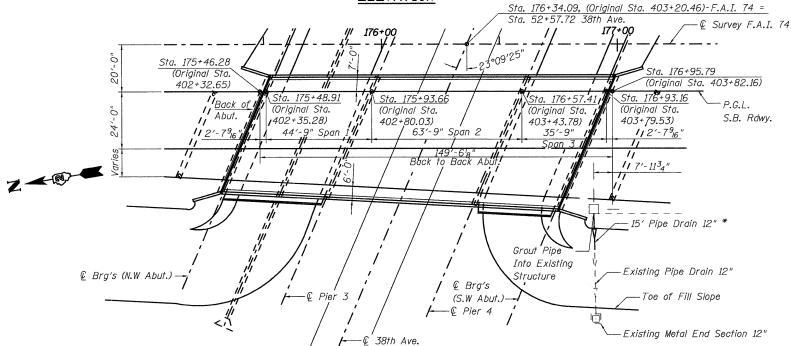
when "A" is greater than 3'-6".

TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION N.B. 27TH ST. OVER F.A.I. 74 ROCK ISLAND COUNTY STRUCTURE NO. 081-0108

R-27

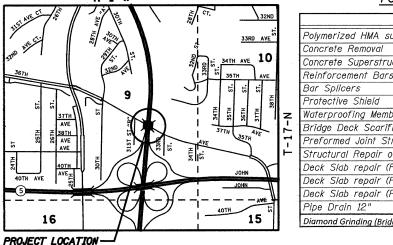






PLAN

TOTAL BILL OF MATERIAL



LOCATION PLAN

DESIGNED DFM

CHECKED DSG

DRAWN EBS

CHECKED DFM

	ITEM	UNIT	QUANTITY
	Polymerized HMA surface Course Mix "E", N70	TON	862
	Concrete Removal	CU. YD.	<i>1</i> 6.6
	Concrete Superstructure	CU. YD.	<i>1</i> 6.6
	Reinforcement Bars, Epoxy Coated	POUND	2230
	Bar Splicers	EACH	22
	Protective Shield	SQ. YD.	<i>354</i>
Z	Waterproofing Membrane System	SQ. YD.	799.2
	Bridge Deck Scarification	SQ. YD.	793.1
1-1	Preformed Joint Strip Seal	F00T	114
—	Structural Repair of Concrete (Depth > 5")	SQ. FT.	18
	Deck Slab repair (Full Depth, Type I)	SQ. YD.	47.5
	Deck Slab repair (Full Depth, Type II)	SQ. YD.	47.5
-14	Deck Slab repair (Partial)	SQ. YD.	10
	Pipe Drain 12"	FOOT	<i>1</i> 5
	Diamond Grinding (Bridge Section)	SQ. yb.	799.2

My registration

0 101/1	
Land Want	D. N. 004 005455
David F. Maxwell, S.E.	Reg. No. 081-005455
My registration expires November :	30, 20/0

GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Existing reinforcement bars extending into the removal areas shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr. 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by an individual acceptable to the Engineer. Any cracks that can not be removed by grinding l_4 in. deep shall be identified and reported to the bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

Quantities of Deck Slab Repair (Full Depth, Type I), Deck Slab Repair (Full Depth, Type II) and Deck Slab Repair (Partial) are approximated. Locations will be determined by the Resident Engineer following removal of the 2" Concrete Overlay. Actual repair locations shall be shown on the as-built plans. Contractor will be paid for the quantities furnished.

Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured of an ambient temperature other than 50° F.

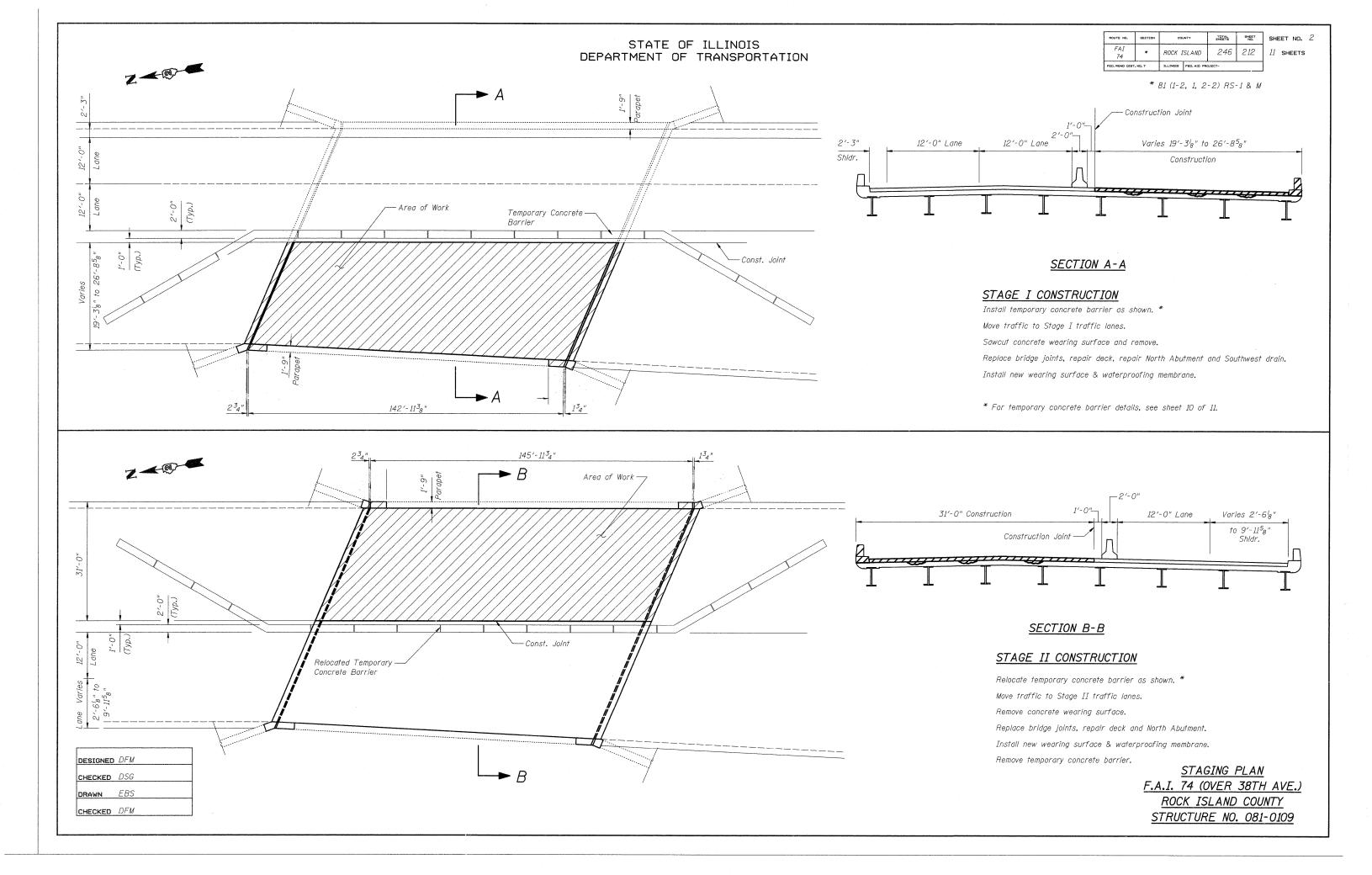
* Quantity of pipe drain 12" is approximate. Field inspection of existing pipe drain shall be done before replacement to ensure remainder of pipe is adequate. Replacement quantity of pipe drain 12" may be adjusted by Resident Engineer based on inspection of remaining pipe. Removal of existing pipe, backfill, installation of new pipe & grouting of inlet is included in pipe drain 12".

PLAN AND ELEVATION

F.A.I. 74 (OVER 38TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0109

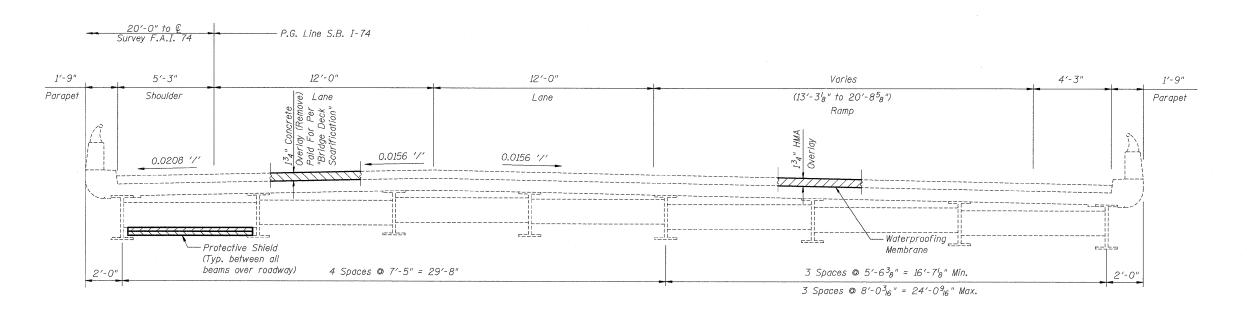


ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHE	EET NO. 3
FAI 74	*	ROCK I	SLAND	246	213	11	SHEETS
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PRO	олест-			

* 81 (1-2, 1, 2-2) RS-1 & M

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N90	TON	86.2
Protective Shield	SQ.YD.	354
Waterproofing Membrane System	SQ.YD.	799.2
Bridge Deck Scarification	SQ.YD.	793.1



DESIGNED DFM

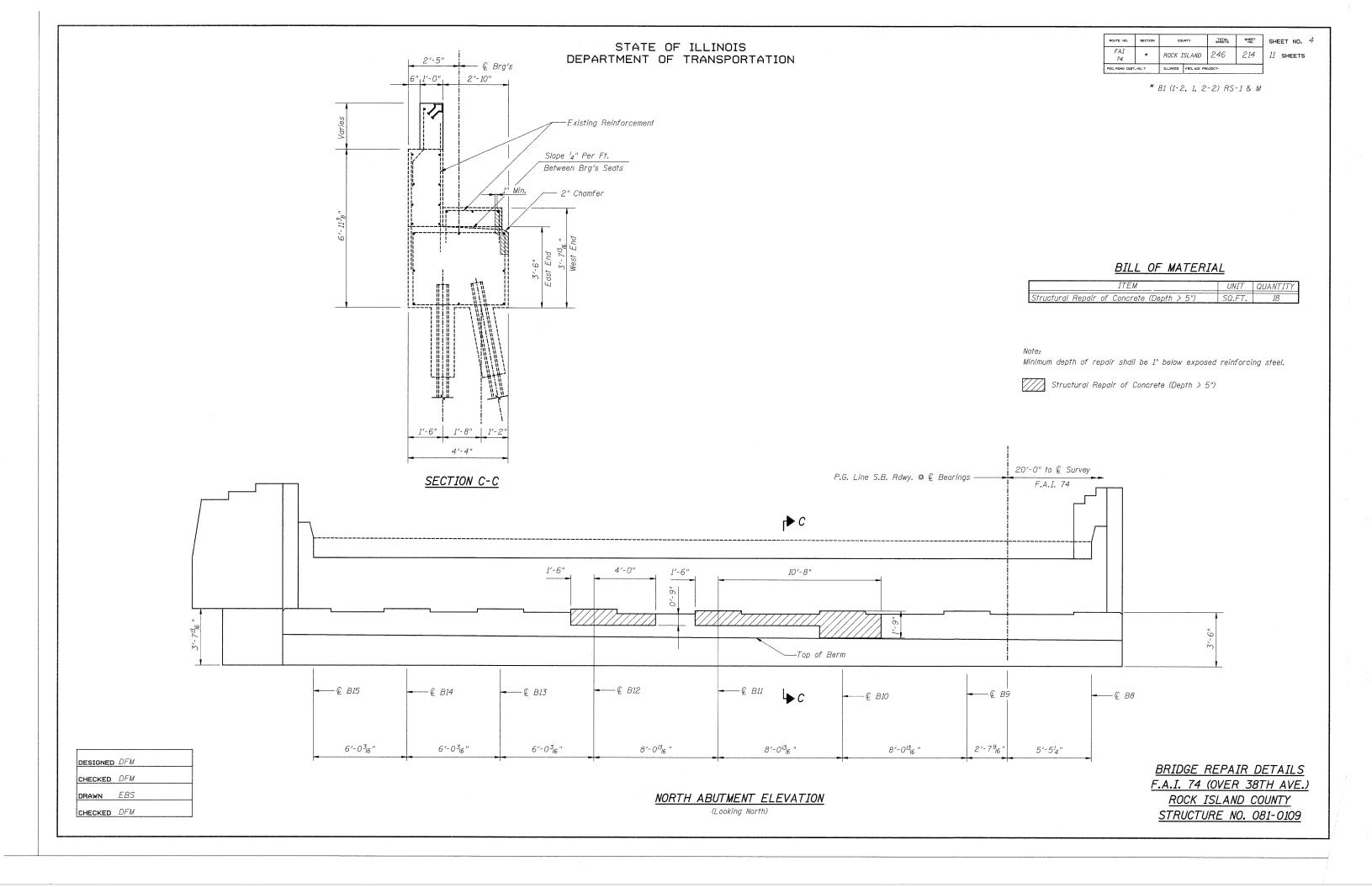
CHECKED DFM

DRAWN EBS

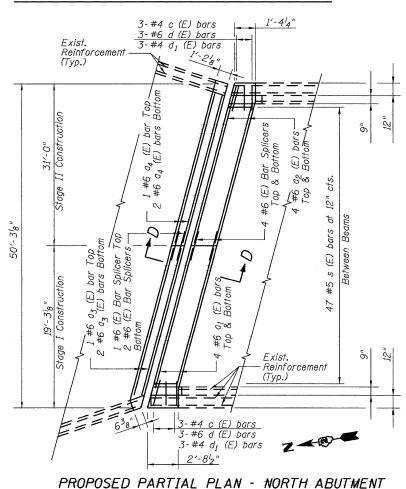
CHECKED DFM

CROSS SECTION
(Looking South)

BRIDGE REPAIR DETAILS
F.A.I. 74 (OVER 38TH AVE.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0109



EXISTING PARTIAL PLAN - NORTH ABUTMENT



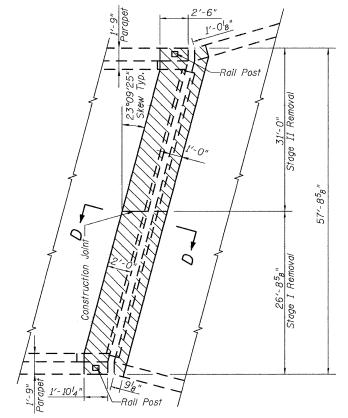
DESIGNED DFM

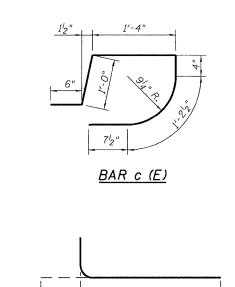
CHECKED DFM

DRAWN EBS

CHECKED DFM

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION





10½" 7½"

 d_1

2'-512"

1'-10¹2"



ROUTE NO. SECTION

ROCK ISLAND

ILLINOIS FED. AID PROJECT-

* 81 (1-2, 1, 2-2) RS-1 & M

BAR S (E)

1'-9"

	BILL OF	MAIL	RIAL	
BAR	NO.	SIZE	LENGTH	SHAPE
a ₁ (E)	8	#6	19'-3"	
a 2 (E)	16	#6	32′-6"	-
a ʒ (E)	3	#6	19′-3"	
0 4 (E)	6	#6	32′-6"	
a 5 (E)	8	#6	27'-4"	
a ₆ (E)	3	#6	27′-5"	
				:
c (E)	12	#4	5′-0"	-2
d (E)	12	#6	3'-4"	<u> </u>
d ₁ (E)	12	#4	2′-6"	<u> </u>
s (E)	101	#5	2′-5"	
				·
Reinforcemen	t Bars Epoxy	Coated	Pound	2230
Concrete Rem	noval		Cu. Yd.	16.6
Concrete Sup	erstructure		Cu. Yd.	16.6
Bar Splicer			Each	22

SHEET NO. 5

11 SHEETS

TOTAL SHEET NO.

246

215

For bar splicer assembly details see sheet 9 of 11.

For Section D-D, see sheet 6 of 11.

For rail post details see sheet 11 of 11.

<u>LEGEND</u>

- Concrete Removal

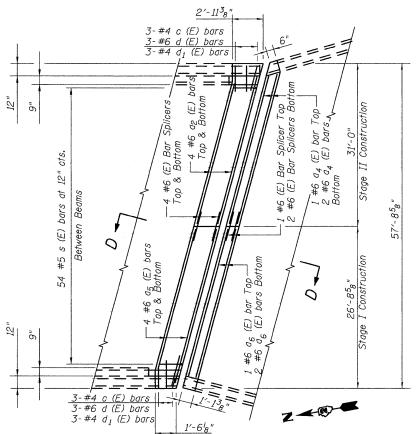
- Concrete Superstructure

F.A.I. 74 (OVER 38TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0109

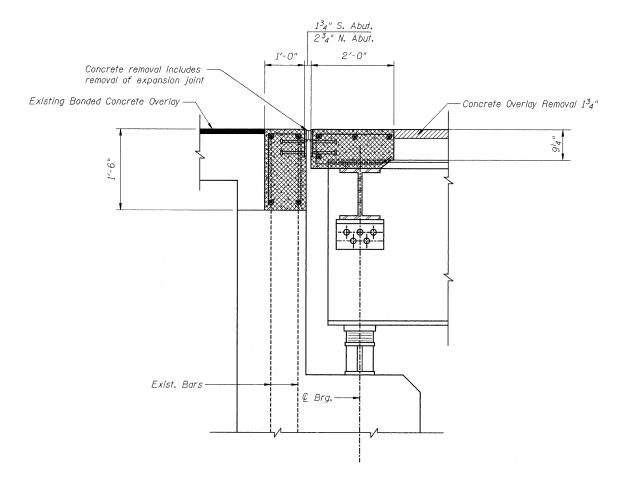
EXISTING PARTIAL PLAN - SOUTH ABUTMENT



PROPOSED PARTIAL PLAN - SOUTH ABUTMENT

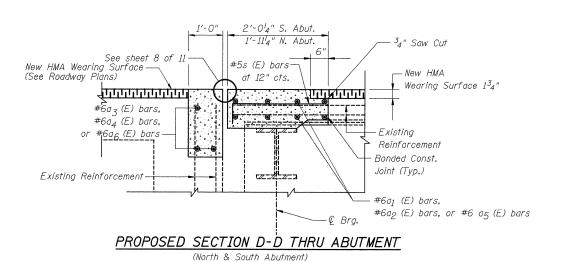


* 81 (1-2, 1, 2-2) RS-1 & M



EXISTING SECTION D-D THRU ABUTMENT

(North & South Abutment)



DESIGNED DFM

CHECKED DFM

DRAWN EBS

CHECKED DFM

LEGEND

Concrete Removal

- Bridge Deck Scarification

JOINT RECONSTRUCTION DETAILS

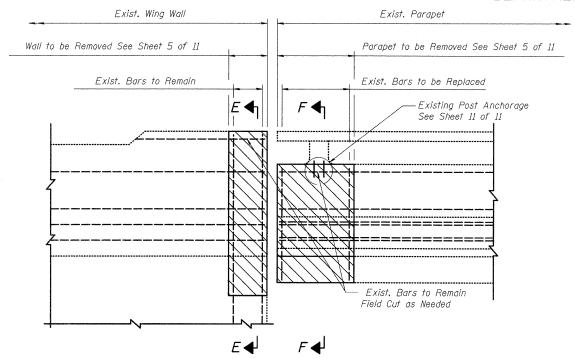
F.A.I. 74 (OVER 38TH AVE.)

ROCK ISLAND COUNTY

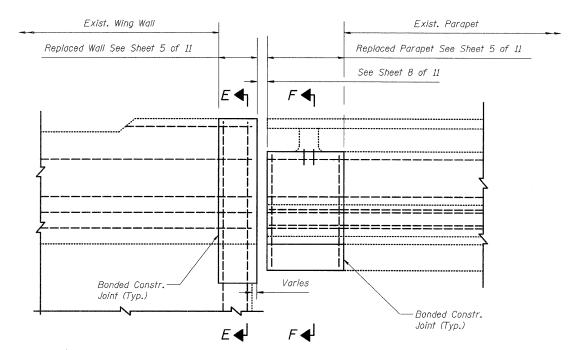
STRUCTURE NO. 081-0109

TOTAL SHEET NO. 7 COUNTY 217 ROCK ISLAND 246 11 SHEETS

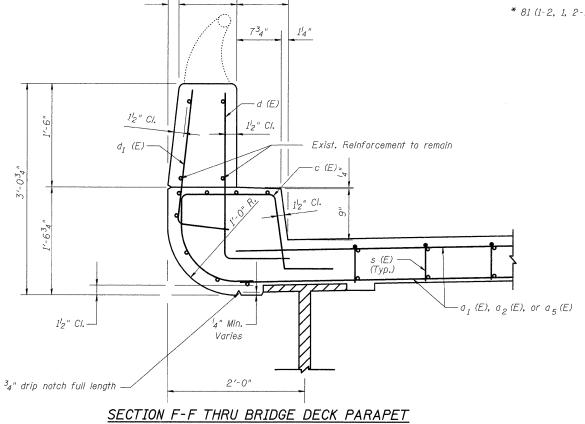
* 81 (1-2, 1, 2-2) RS-1 & M

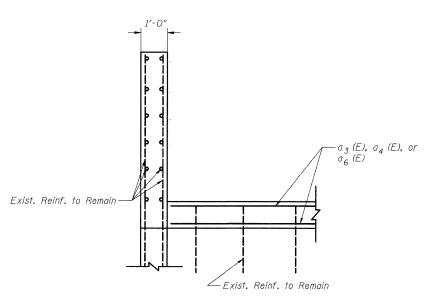


ELEVATION OF CONCRETE REMOVAL THRU PARAPET AND WING WALL



ELEVATION OF CONCRETE REPLACEMENT THRU PARAPET AND WING WALL





SECTION E-E THRU ABUTMENT BACKWALL AND WING WALL

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

Hatched areas indicate concrete sections to be removed and replaced. Perimeters of concrete removal areas shall be saw cut 3_4 " prior to the removal of concrete.

PARAPET AND RETAINING WALL DETAILS F.A.I. 74 (OVER 38TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0109

ROUTE NO. TOTAL SHEETS SHEET NO. SHEET NO. 8218 246 ROCK ISLAND

11 SHEETS

Notes:

TYPICAL END TREATMENTS

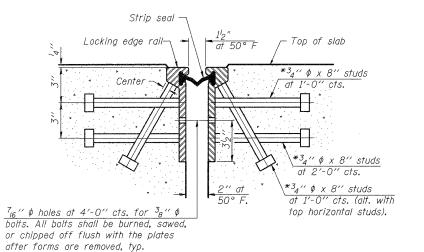
* 81 (1-2, 1, 2-2) RS-1 & M

The strip seal shall be made continuous and shall have a minimum thickness of ${}^{l}_{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.



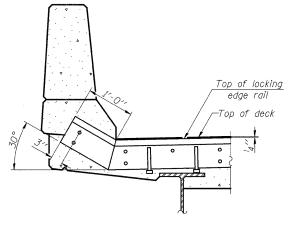
Strip seal-Locking edge rail-Top of slab *3₄'' \$\psi x 8'' studs at 1'-0" cts. at 2'-0" cts. Anchor plate Place plates at 1'-0'' cts. 7_{16} " ϕ holes at 4'-0" cts. for 3_8 " ϕ (alt, with top horizontal studs) bolts. All bolts shall be burned, sawed,

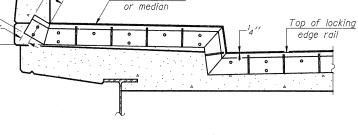
SECTION THRU ROLLED RAIL JOINT

SECTION THRU WELDED RAIL JOINT

or chipped off flush with the plates

after forms are removed, typ.





Top of sidewalk

AT PARAPET

AT SIDEWALK OR MEDIAN

Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

ROLLED EXTRUDED RAIL

*Granular or solid flux filled headed studs conforming to Article 1006,32 of the Std. Specs., automatically end welded.

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

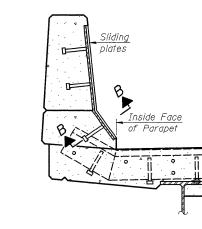
***Back gouge not required if

complete joint penetration

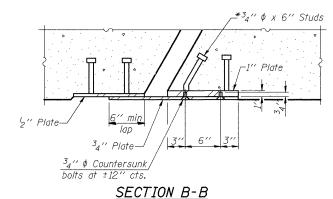
is verified by mock-up.

LOCKING EDGE RAILS

WELDED RAIL



ANCHOR P



BILL OF MATERIAL Unit Total Preformed Joint Strip Seal Foot 114

	ſ►A	Skew > 30°
	Sliding plates	
Inside face/ of parapet	L ▶ A∭—Str	_3", typ. _End of parapet ip seal joint
_		

PLAN

SECTION A-A

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

5-16-08

EJ-SSJ

POINT BLOCK DETAILS (for skews > 30°)

PREFORMED JOINT STRIP SEAL F.A.I. 74 (OVER 38TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0109



11 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

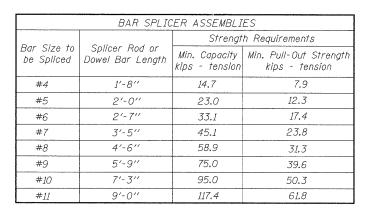
reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval, Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

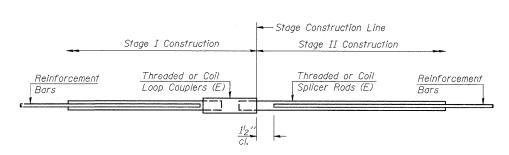
Minimum Capacity (Tension in kips) = 1.25 x fy x A_t

Minimum *Pull-out Strength = $0.66 \times fy \times A_t$ (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

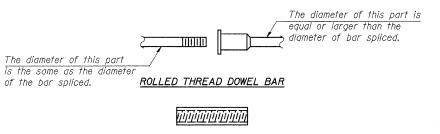




STANDARD

Bar Size	No. Assemblies Required	Location
#5	8	Bridge Deck at South Abutment
#5	8	Bridge Deck at North Abutment
#6	3	South Abutment Backwall
#6	3	North Abutment Backwall
#6	3	North Abutment Backwall

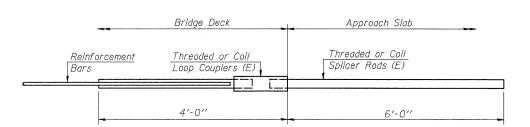
BAR SPLICER ASSEMBLY DETAILS F.A.I. 74 (OVER 38TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0109



** ONE PIECE -Wire Connector Viriiii WELDED SECTIONS

BAR SPLICER ASSEMBLY ALTERNATIVES

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



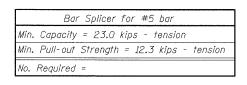
FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

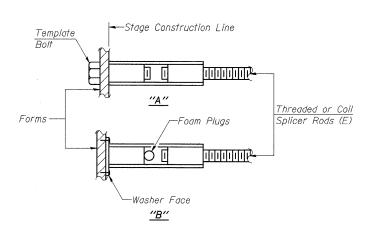
Bar Splicer for #5 bar	
Min. Capacity = 23.0 kips - tension	
Min. Pull-out Strength = 12.3 kips - tension	
No. Required =	

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

BSD-1

5-16-08

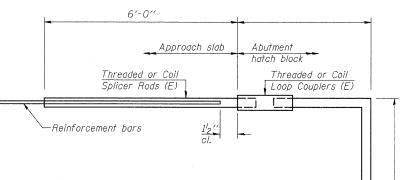




INSTALLATION AND SETTING METHODS

"A" :Set bar splicer assembly by means of a template bolt. "B" :Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.

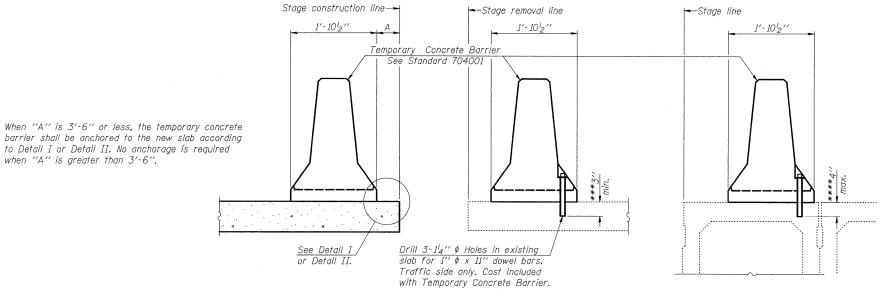


FOR STUB ABUTMENTS

EXISTING DECK BEAM

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHE	EET NO.	10
FAI 74	*	ROCK ISLAND		246	220	11	SHEETS	5
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PRO	OJECT-				

* 81 (1-2, 1, 2-2) RS-1 & M



NOTES

Detail I - With Bar Splicer or Couplers:
Connect one (1) 1"x7"x10" steel 12 to the top layer of couplers with 2-58" \$\phi\$ bolts screwed to coupler at approximate € of each barrier panel.

Detail II - With Extended Reinforcement Bars: Connect one (1) 1"x7"x10" steel P to the concrete slab or concrete wearing surface with $2^{-5}8''$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate © of each barrier panel.

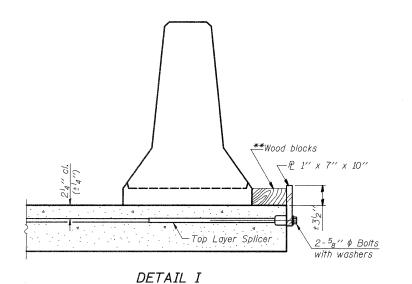
Cost of anchorage is included with Temporary Concrete Barrier. The I'' x 7'' x 10'' plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

EXISTING SLAB

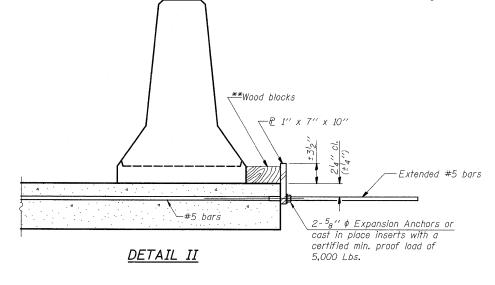
***Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

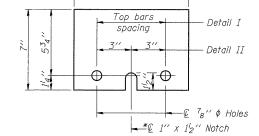
****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



NEW SLAB

when "A" is greater than 3'-6".





STEEL RETAINER P 1" x 7" x 10"

*Required only with Detail II

DESIGNED DFM CHECKED DSG DRAWN EBS

5-*1*6-08

CHECKED DFM

R-27

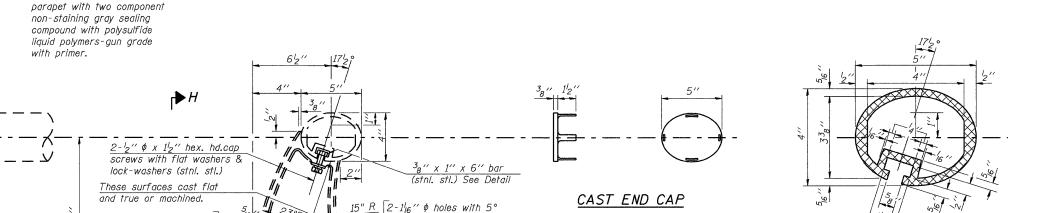
**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION F.A.I. 74 (OVER 38TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0109

ROUTE NO.	SECTION	co	UNTY	TOTAL SHEETS	SHEET NO.	SH
FAI 74	*	ROCK I	SLAND	246	221	11
FEO. ROAD DIST	r. NO. 7	ILLINOIS	FED. AID PR	DJECT-		

SHEET NO. 11

* 81 (1-2, 1, 2-2) RS-1 & M



DRIVE FIT TYPE

draft in base of post for 2

machine bolts. Std. flat washers.

2-1" ϕ welded studs drilled and

tapped for $\frac{3}{4}$ "-10. ASTM

34" \$ x 2" hex. hd.

Caulk perimeter (See note above)

Front face of parapet

A108 (1020)

-½" x ½" x 7"

(stnl. stl.)

SEC. THRU ELLIPTICAL
RAIL SECTION

<u>VIEW H-H</u>

SECTION G-G

RAIL POST DETAILS

 2^{-13}_{16} " ϕ holes with 5°

 $2 - \frac{1}{2}'' \phi \times 1^{3}_{4}'' \text{ hex.}$

draft in base of post for

hd. cap screws. Std. flat washers (stnl. stl.)

' Fabric Bearing Pad

 $2 - \frac{3}{4}$ " ϕ welded studs

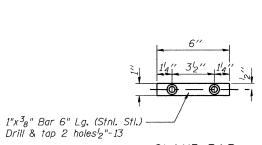
½"-13. AASHTO M 169

drilled and tapped for

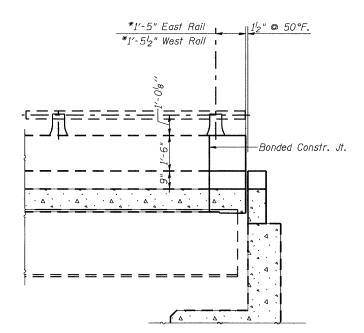
38" x 14" x 7"

(1015, 1018, 1020)

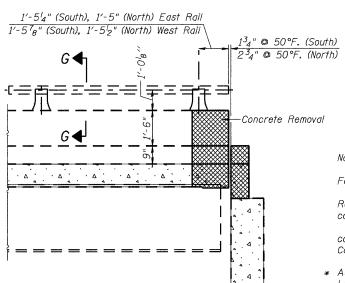
Seal perimeter of post to



CLAMP BAR



INSIDE VIEW AT PROPOSED ABUTMENT



Δ . Δ .

INSIDE VIEW AT EXISTING ABUTMENT

Notes:

For concrete removal limits See Sheet 5 of 11.

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

* Actual distance varies. New joints are skewed through parapet. Length of rail shall be adjusted as detailed with a flush cut and the cast end cap shall be reused.

DESIGNED DFM

CHECKED DSG

DRAWN EBS

CHECKED DFM

 $\frac{9_{16}" \times 1^{l_2}" \text{ slotted}}{\text{holes with } 5^{\circ} \text{ draft}}$

RAILING DETAILS

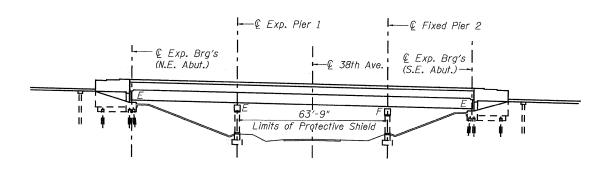
F.A.I. 74 (OVER 38TH AVE.)

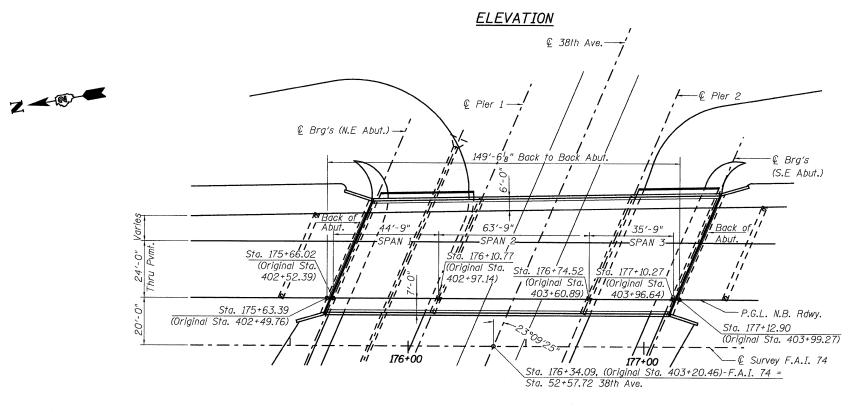
ROCK ISLAND COUNTY

STRUCTURE NO. 081-0109

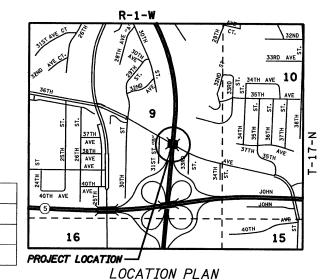
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO
FAI 74	*	ROCK ISLAND	246	222	11 SHEETS
FED. ROAD DIST		ILLINOIS FED. AID PR	OJECT-		Ì

* 81 (1-2, 1, 2-2) RS-1 & M





PLAN



DESIGNED DFM

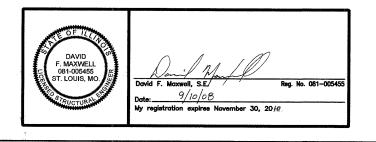
CHECKED DSG

DRAWN EBS

CHECKED DFM

TOTAL BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA surface Course Mix "E", N70	TON	66.6
Concrete Removal	CU. YD.	<i>15.8</i>
Concrete Superstructure	CU. YD.	<i>15.8</i>
Reinforcement Bars, Epoxy Coated	POUND	2080
Bar Splicers	EACH	22
Protective Shield	SQ. YD.	330
Waterproofing Membrane System	SQ. YD.	746.2
Bridge Deck Scarification	SQ. YD.	740.5
Preformed Joint Strip Seal	FOOT	107
Structural Repair of Concrete (Depth > 5")	SQ. FT.	27
Deck Slab repair (Full Depth, Type I)	SQ. YD.	70
Deck Slab repair (Full Depth, Type II)	SQ. YD.	70
Deck Slab repair (Partial)	SQ. YD.	20.
Diamond Grinding (Bridge Section)	SQ. YD.	746.2



GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Existing reinforcement bars extending into the removal areas shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr. 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by an individual acceptable to the Engineer. Any cracks that can not be removed by grinding 1_4 in. deep shall be identified and reported to the bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

Quantities of Deck Slab Repair (Full Depth, Type I), Deck Slab Repair (Full Depth, Type II) and Deck Slab Repair (Partial) are approximated. Locations will be determined by the Resident Engineer following removal of the 2" Concrete Overlay. Actual repair locations shall be shown on the as-built plans. Contractor will be paid for the quantities furnished.

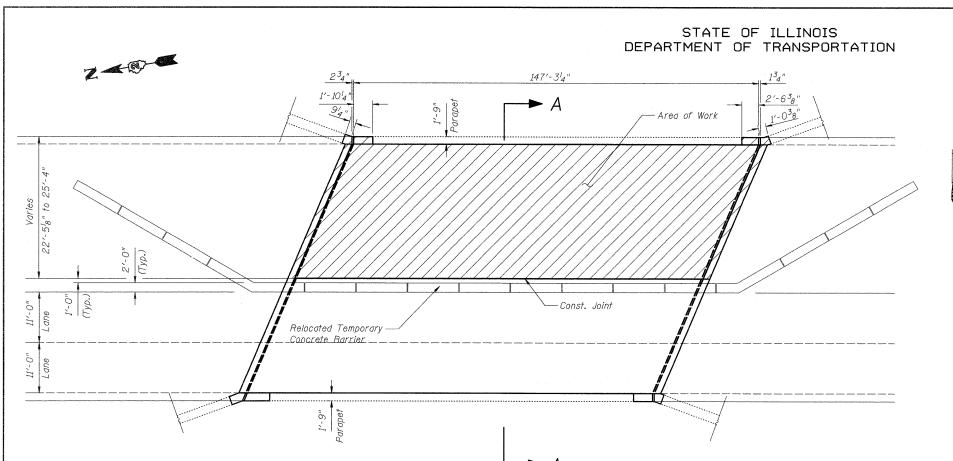
Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured of an ambient temperature other than 50° F.

PLAN AND ELEVATION

F.A.I. 74 (OVER 38TH AVE.)

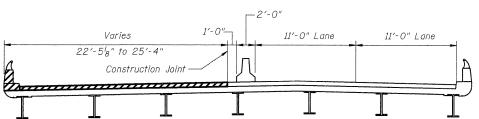
ROCK ISLAND COUNTY

STRUCTURE NO. 081-0110



	ROUTE NO.	SECTION	cou	JNTY	TOTAL SHEETS	SHEET NO.	SHE	EET NO. 2
	FAI 74	*	ROCK I	SLAND	246	223	11	SHEETS
ſ	FED. ROAD DIST.	NO. 7	ILLINOIS	FED. AID PRO	DJECT-			

* 81 (1-2, 1, 2-2) RS-1 & M



SECTION A-A

STAGE I CONSTRUCTION

Install temporary concrete barrier as shown. *

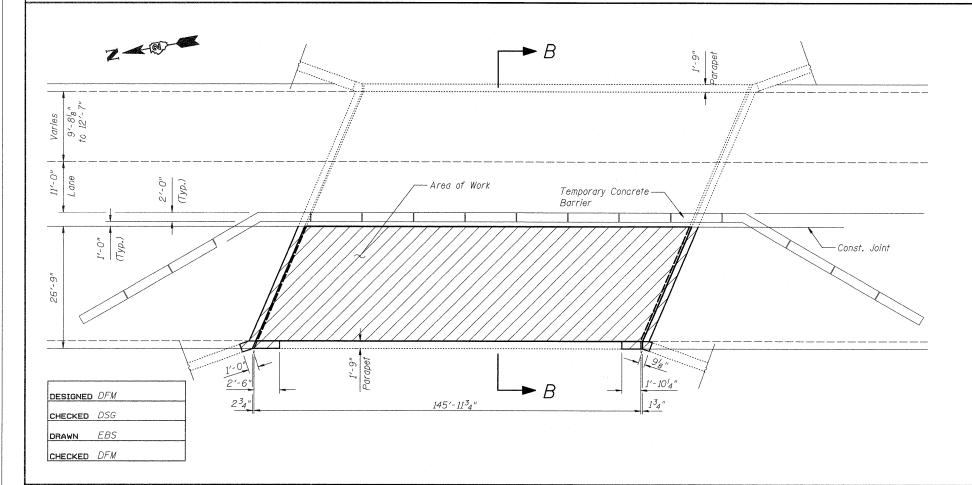
Move traffic to Stage I traffic lanes.

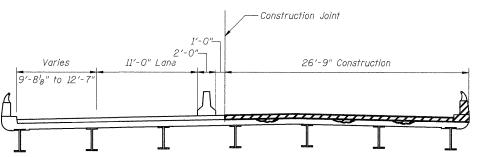
Sawcut concrete wearing surface and remove.

Replace bridge joints and repair deck and North Abutment.

Install new wearing surface & waterproofing membrane.

* For temporary concrete barrier details, see sheet 10 of 11.





SECTION B-B

STAGE II CONSTRUCTION

Relocate temporary concrete barrier as shown. *

Move traffic to Stage II traffic lanes.

Remove concrete wearing surface.

Replace bridge joints and repair deck and North Abutment.

Install new wearing surface & waterproofing membrane.

Remove temporary concrete barrier.

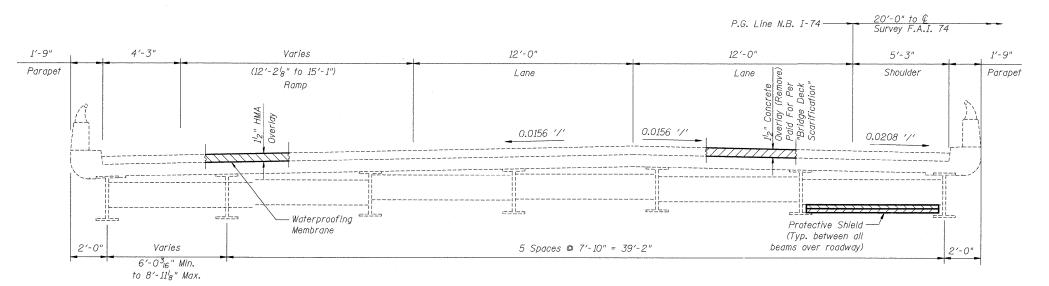
STAGING PLAN
F.A.I. 74 (OVER 38TH AVE.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0110

ROUTE NO.	BECTION	COL	JNTY	TOTAL SHEETS	SHEET NO.	SHE	EET NO. 3
FAI 74	*	ROCK I	SLAND	246	224	11	SHEETS
FED. ROAD DIST	. NO. 7	ILLINOIS	FED. ALD PRI	JECT-			

* 81 (1-2, 1, 2-2) RS-1 & M

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N90	TON	66.6
Protective Shield	SQ.YD.	330
Waterproofing Membrane System	SQ.YD.	746.2
Bridge Deck Scarification	SQ.YD.	740.5



CROSS SECTION

(Looking South)

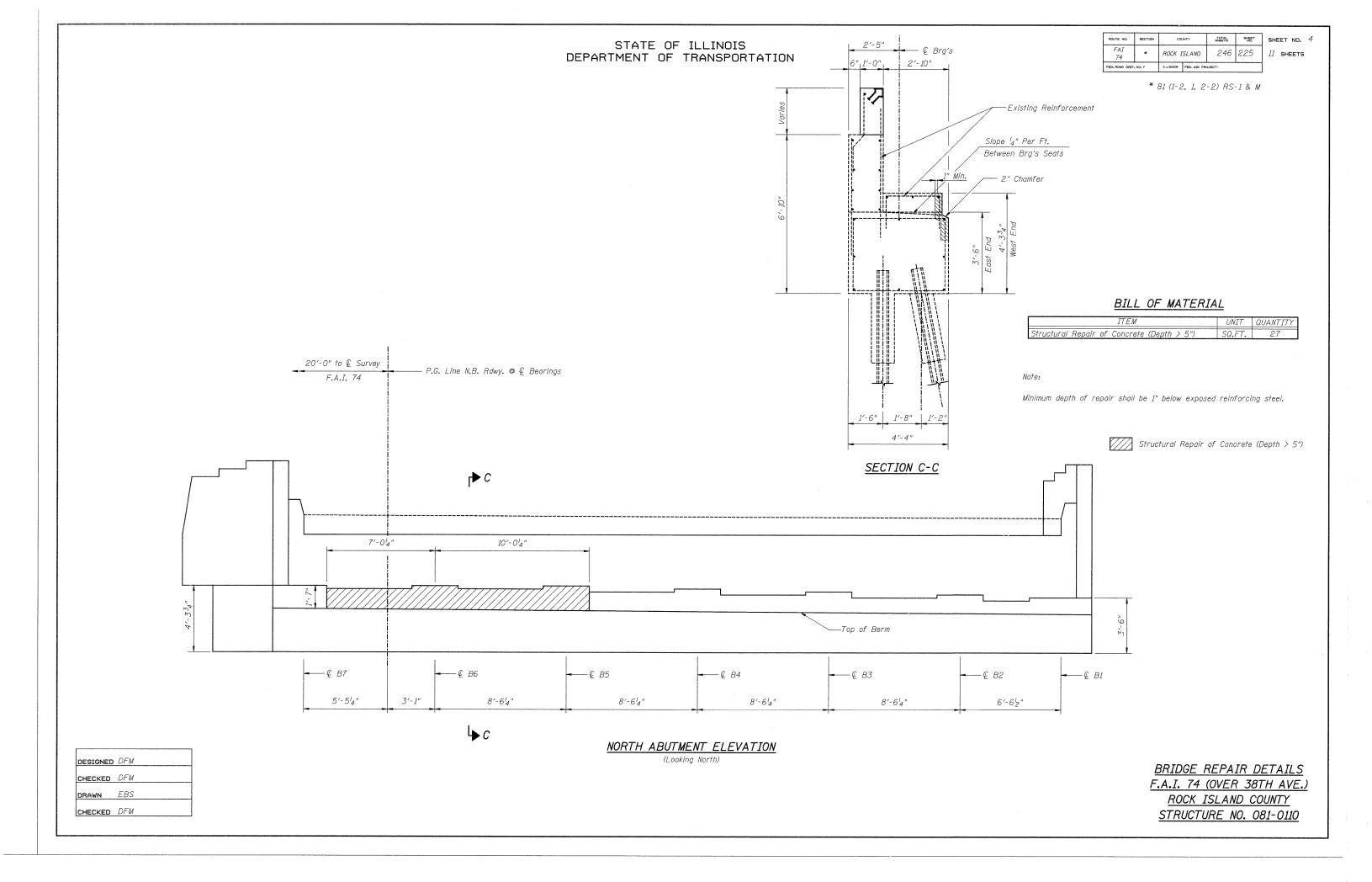
DESIGNED DFM

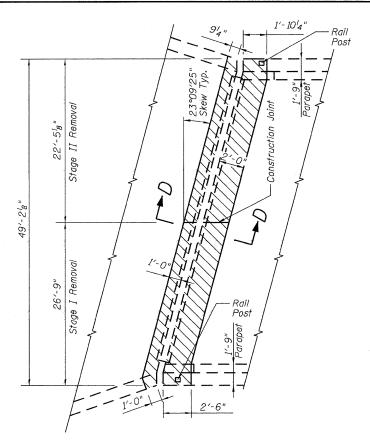
CHECKED DSG

DRAWN EBS

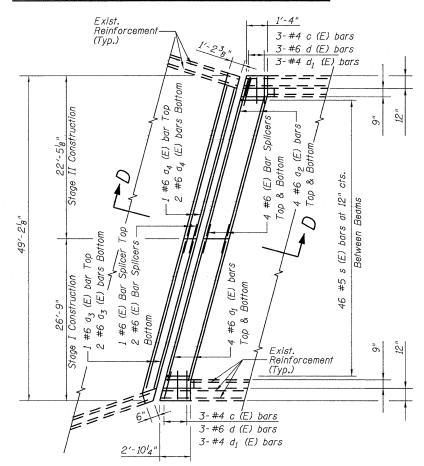
CHECKED DFM

BRIDGE REPAIR DETAILS
F.A.I. 74 (OVER 38TH AVE.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0110

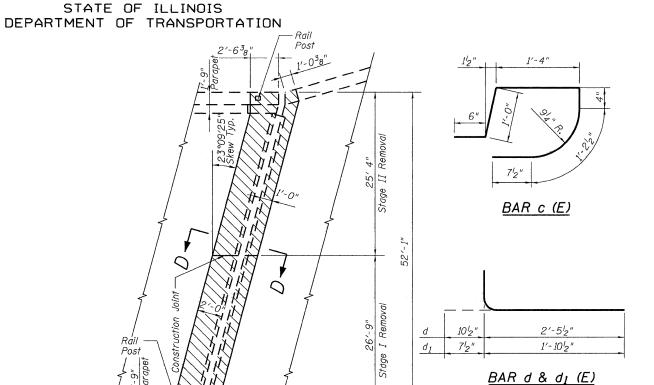




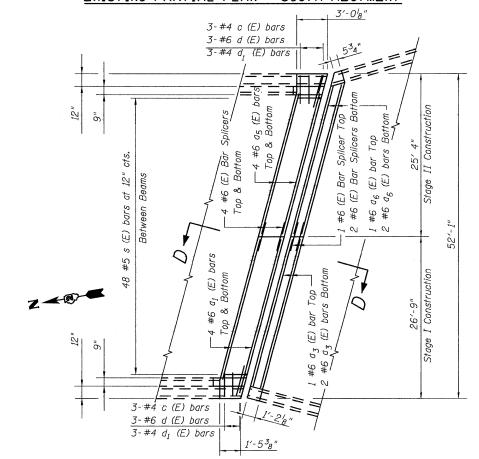
EXISTING PARTIAL PLAN - NORTH ABUTMENT



PROPOSED PARTIAL PLAN - NORTH ABUTMENT



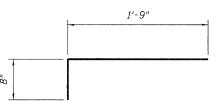
EXISTING PARTIAL PLAN - SOUTH ABUTMENT



PROPOSED PARTIAL PLAN - SOUTH ABUTMENT

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO.
FAI 74	*	ROCK ISLAND	246	226	11 SHEETS
FED. ROAD DIST	. NO. 7	ILLINOIS FED. AID PR	DJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



BAR s (E)

BILL OF MATERIAL

BAR	NO.	SIZE	LENGTH	SHAPE
a ₁ (E)	16	#6	27′-6"	
a ₂ (E)	8	#6	23'-2"	
a 3 (E)	6	#6	27′-5"	
04(E)	3	#6	23'-2"	
a 5 (E)	8	#6	26′-4"	
a 6 (E)	3	#6	26'-4"	
c (E)	12	#4	5′-0"	Ŋ
d (E)	12	#6	3'-4"	<u> </u>
d 1 (E)	12	#4	2'-6"	<u> </u>
s (E)	94	#5	2'-5"	
Reinforcemen	t Bars Epoxy	/ Coated	Pound	2080
Concrete Rem	noval		Cu. Yd.	<i>1</i> 5 . 8
Concrete Sup	erstructure		Cu. Yd.	<i>15.8</i>
Bar Splicer			Each	22

For bar splicer assembly details see sheet 9 of 11.

For Section D-D, see sheet 6 of 11.

For rail post details see sheet 11 of 11.

LEGEND

- Concrete Removal

- Concrete Superstructure

JOINT RECONSTRUCTION DETAILS

F.A.I. 74 (OVER 38TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0110



DESIGNED DFM

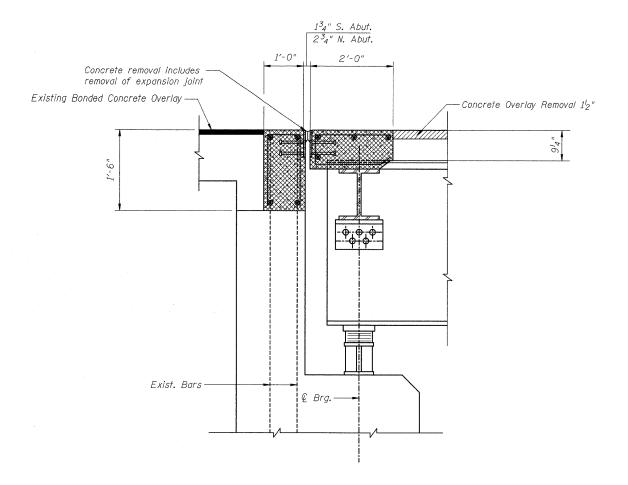
CHECKED DFM

DRAWN EBS

CHECKED DFM

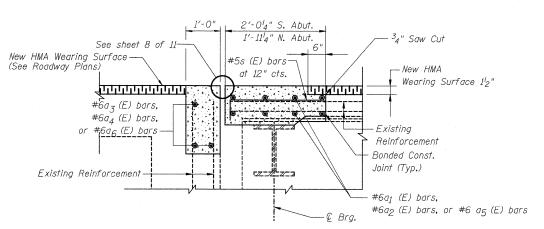


* 81 (1-2, 1, 2-2) RS-1 & M



EXISTING SECTION D-D THRU ABUTMENT

(North & South Abutment)



PROPOSED SECTION D-D THRU ABUTMENT

(North & South Abutment)

DESIGNED DFM

CHECKED DFM

DRAWN EBS

CHECKED DFM

BRIDGE REPAIR DETAILS

F.A.I. 74 (OVER 38TH AVE.)

ROCK ISLAND COUNTY

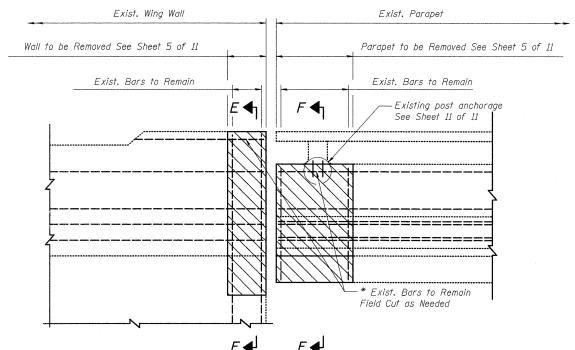
STRUCTURE NO. 081-0110

LEGEND

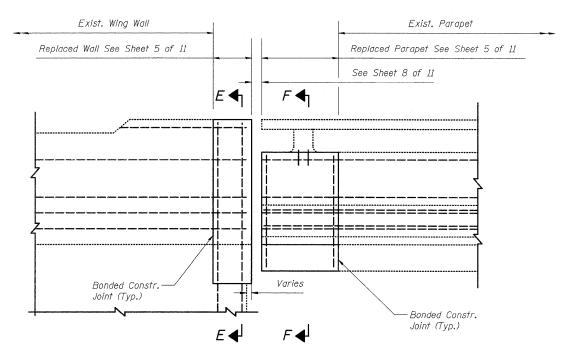
- Concrete Removal

- Bridge Deck Scarification

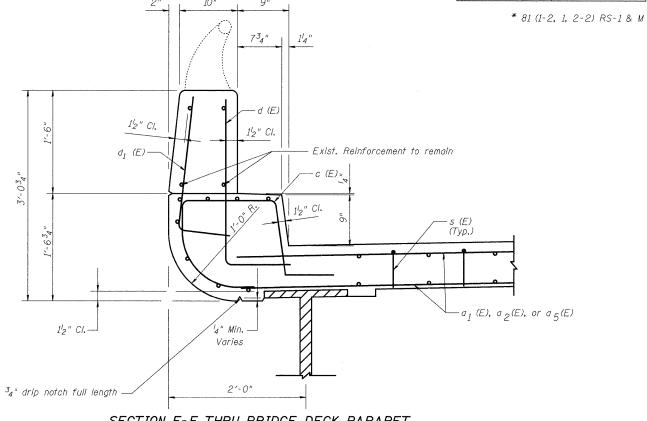
TOTAL SHEET SHEET NO. 7 * ROCK ISLAND 246 228 11 SHEETS



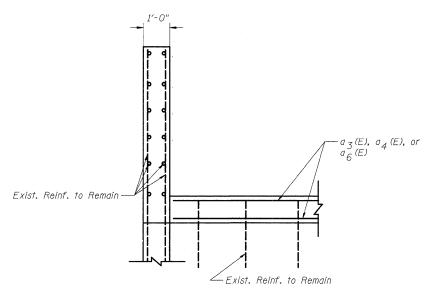
ELEVATION OF CONCRETE REMOVAL THRU PARAPET AND WING WALL



ELEVATION OF CONCRETE REPLACEMENT THRU PARAPET AND WING WALL



SECTION F-F THRU BRIDGE DECK PARAPET



SECTION E-E THRU ABUTMENT BACKWALL AND WING WALL

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

Hatched areas indicate concrete sections to be removed and replaced. Perimeters of concrete removal areas shall be saw cut $\frac{3}{4}$ " prior to the removal of concrete.

PARAPET AND RETAINING WALL DETAILS F.A.I. 74 (OVER 38TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0110

*Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION



SHEET NO. 811 SHEETS

TYPICAL END TREATMENTS

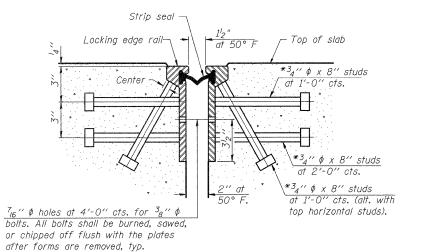
* 81 (1-2, 1, 2-2) RS-1 & M

The strip seal shall be made continuous and shall have a minimum thickness of l_4 ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rai! expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.



Strip seal-_Top of slab Locking edge rail-at 1'-0" cts. at 2'-0" cts. Anchor plate Place plates at 1'-0" cts. 7_{16} " ϕ holes at 4'-0" cts. for 3_8 " ϕ (alt. with top horizontal studs) bolts. All bolts shall be burned, sawed.

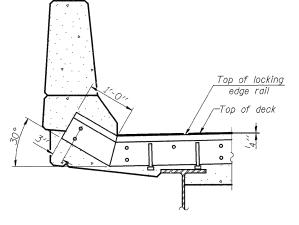
SECTION THRU ROLLED RAIL JOINT

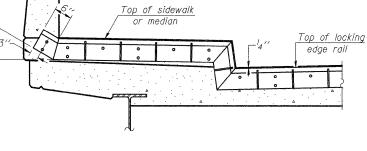


ANCHOR P

or chipped off flush with the plates

after forms are removed, typ.





AT SIDEWALK OR MEDIAN

Shorter plates with a single row of studs

at 12" cts. may be necessary on medians

which are shallower than 9". See

manufacturer's recommendation.

AT PARAPET

ROLLED EXTRUDED RAIL

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

***Back gouge not required if

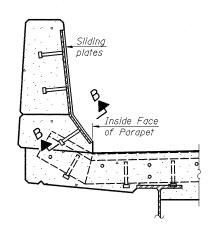
is verified by mock-up.

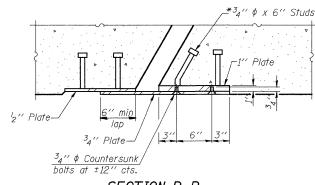
complete joint penetration

Flush

LOCKING EDGE RAILS

WELDED RAIL





SECTION B-B

plates <u>Inside face</u>, *∟3″*, typ. of parapet End of parapet rip seal joint

PLAN

SECTION A-A

POINT BLOCK DETAILS (for skews > 30°)

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	107

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

EJ-SSJ

5-16-08

PREFORMED JOINT STRIP SEAL F.A.I. 74 (OVER 38TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0110



11 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

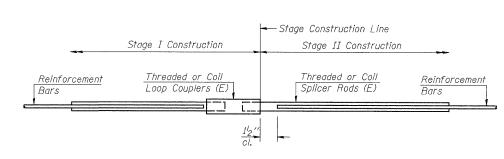
Minimum Capacity (Tension in kips) = 1.25 x fy x A_t

(Tension iii kipo) Minimum *Pull-out Strength = $0.66 \times fy \times A_t$ (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

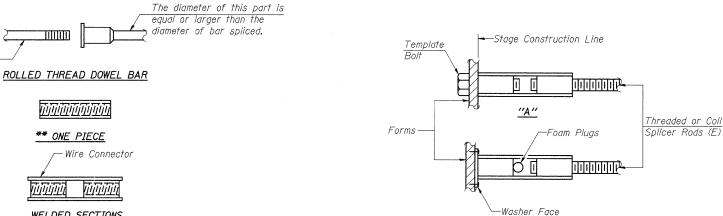
	BAR SPLICER ASSEMBLIES						
		Strength Requirements					
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension				
#4	1'-8''	14.7	7.9				
#5	2'-0"	23.0	12.3				
#6	2'-7''	33.1	17.4				
#7	3′-5″	45.1	23.8				
#8	4'-6''	58.9	31.3				
#9	5′-9′′	75.0	39.6				
#10	7′-3′′	95.0	50.3				
#11	9'-0''	117.4	61.8				



STANDARD

Bar Size	No. Assemblies Required	Location
#5	8	Bridge Deck at South Abutment
#5	8	Bridge Deck at North Abutment
#6	3	South Abutment Backwall
#6	3	North Abutment Backwall

BAR SPLICER ASSEMBLY DETAILS F.A.I. 74 (OVER 38TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0110



BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

The diameter of this part

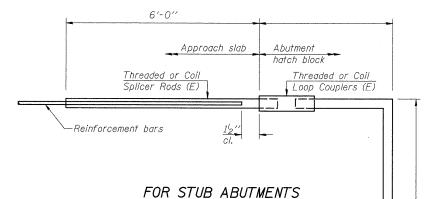
of the bar spliced.

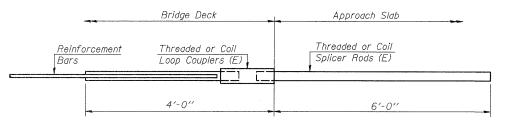
is the same as the diameter

"A" :Set bar splicer assembly by means of a template bolt. "B" :Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.

<u>′′B′′</u>

INSTALLATION AND SETTING METHODS





FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required =

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

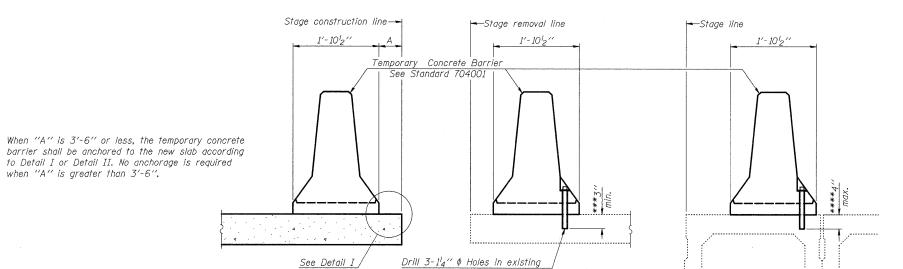
BSD-1 5-16-08

cer for #5 bar
3.0 kips - tension
ngth = 12.3 kips - tension

	Bar Splicer for #5 bar
Min.	Capacity = 23.0 kips - tension
Min.	Pull-out Strength = 12.3 kips - tension
No.	Required =

ROUTE NO.	SECTION	coi	JNTY	TOTAL SHEETS	SHEET ND,	SHI	EET NO. 10	0
FAI 74	*	ROCK 1	SLAND	246	231	11	SHEETS	
FED. ROAD DIST, NO. 7		ILLINOIS	PED, AID PRO	JECT-				

* 81 (1-2, 1, 2-2) RS-1 & M



slab for $1'' \phi \times 11''$ dowel bars.

Traffic side only. Cost included

with Temporary Concrete Barrier.

NEW SLAB

or Detail II.

EXISTING SLAB

EXISTING DECK BEAM

NOTES

Detail I - With Bar Splicer or Coup!ers:

Connect one (1) 1"x7"x10" steel ₱ to the
top layer of couplers with 2-5₈" \$\phi\$ bolts
screwed to coupler at approximate \$\mathbb{Q}\$ of
each barrier panel.

Detail II - With Extended Reinforcement Bars:
 Connect one (I) 1''x7''x10'' steel $mathbb{R}$ to the concrete slab or concrete wearing surface with $2^{-5}g''(\phi)$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate $mathbb{Q}$ of each barrier panel.

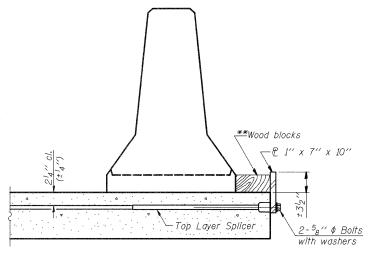
Cost of anchorage is included with Temporary Concrete Barrier.
The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

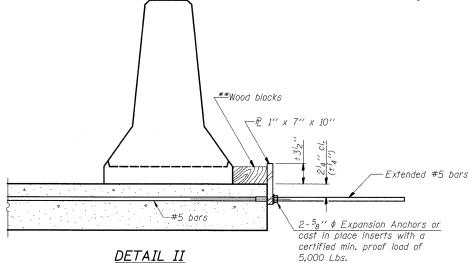
***Dimension shown is minimum required embedment into concrete.

If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

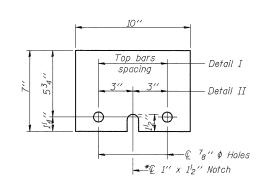
****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



DETAIL I



**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



STEEL RETAINER P 1" x 7" x 10"

*Required only with Detail II

TEMPORARY CONCRETE BARRIER

FOR STAGE CONSTRUCTION

F.A.I. 74 (OVER 38TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0110

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

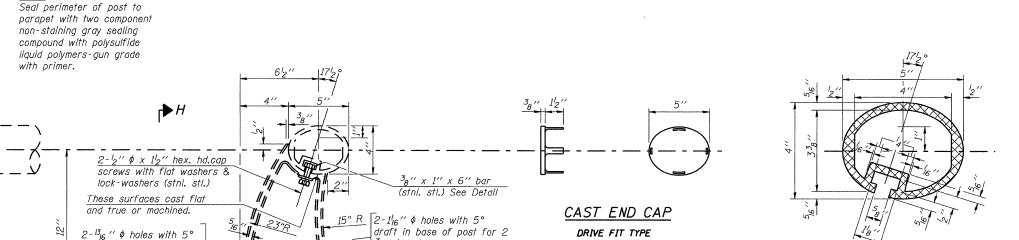
R-27

5-16-08

ROUTE NO.	SECTION	co	UNTY	TOTAL SHEETS	SHEET NO.	SI
FAI 74	*	ROCK I	SLAND	246	232	11
FED. ROAD DIS	r. NO. 7	ILLINGIS	FED. AID PR	OJECT-		

SHEET NO. 11
11 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



SEC. THRU ELLIPTICAL
RAIL SECTION

<u>VIEW H-H</u>

SECTION G-G

RAIL POST DETAILS

draft in base of post for

hd. cap screws. Std. flat

Fabric Bearing Pad

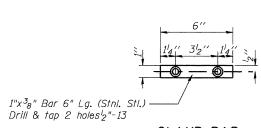
2-34" \phi welded studs

drilled and tapped for '2"-13. AASHTO M 169

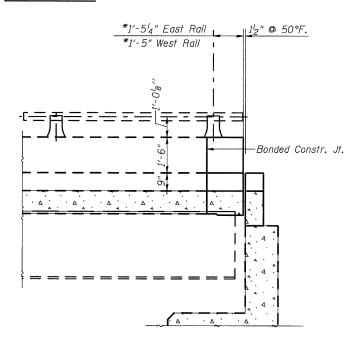
(1015, 1018, 1020) 3₈ " x 1¹4" x 7

 $2^{-\frac{1}{2}''} \phi \times 1^{\frac{3}{4}''}$ hex.

washers (stnl. stl.)



CLAMP BAR



 $\frac{3}{4}$ " $\phi \times 2$ " hex. hd.

Caulk perimeter (See note above)

Front face of parapet

A108 (1020)

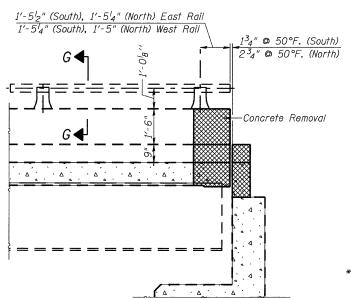
-½" x 1½" x 7"

(stnl. stl.)

machine bolts. Std. flat washers.

2-1'' ϕ welded studs drilled and tapped for $\frac{3}{4}$ "-10. ASTM

INSIDE VIEW AT PROPOSED ABUTMENT



INSIDE VIEW AT EXISTING ABUTMENT

Notes:

For concrete removal limits See Sheet 5 of 11.

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

* Actual distance varies. New joints are skewed through parapet. Length of rail shall be adjusted as detailed with a flush cut and the cast end cap shall be reused.

RAILING DETAILS
F.A.I. 74 (OVER 38TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0110

DESIGNED DFM

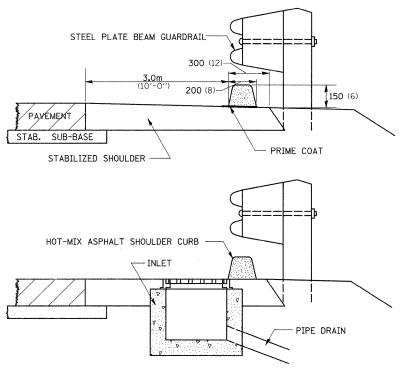
CHECKED DSG

DRAWN EBS

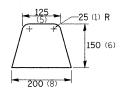
CHECKED DFM

 $\frac{9_{16}^{\prime\prime} \times 1^{l_2^{\prime\prime}} \text{ slotted}}{\text{holes with 5° draft}}$

HOT-MIX ASPHALT SHOULDER CURB



CURB DETAIL



GENERAL NOTES

THIS WORK SHALL BE DONE AS SPECIFIED UNDER SECTION 661 OF THE STANDARD SPECIFICATIONS FOR HOT-MIX ASPHALT CURB.

THE METAL PIPES AND INLETS SHALL BE INSTALLED AS DIRECTED BY THE ENGINEER.

THE BITUMINOUS MATERIALS PRIME COAT SHALL BE CONSIDERED INCLUDED TO THE HOT-MIX ASPHALT SHOULDER CURBS.

ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

HOT-MIX ASPHALT SHOULDER CURB

REVISED - 5-4-94

10.4

DETAILS FOR CURB & GUTTER REPLACEMENT AT INLET

SAWED JOINT (FULL DEPTH)

CURB EXISTING AND GUTTER

CONCRETE CURB AND GUTTER SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 606 OF THE STANDARD SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS, STANDARD 606001 AND THIS DRAWING.

CLASS SI CONCRETE SHALL BE USED THROUGHOUT. A HOLE 40 (1 1/2) IN DIAMETER AND 225 (9) DEEP SHALL BE DRILLED IN THE EXISTING CONCRETE CURB AS SHOWN. A 32×450 (1 1/4 X 18) SMOOTH DOWEL BAR SHALL BE GROUTED IN THE HOLE LONGITUDINALLY.

JOINTS OF A TYPE SIMILAR TO THAT IN THE LINDER-LYING PAVEMENT (EXPANSION OR CONTRACTION) SHALL BE INSTALLED IN THE CONCRETE CURB IN ALIGNMENT WITH THE JOINTS IN THE PAVEMENT.

INLETS ARE NOT TO BE INCLUDED IN THE MEASUREMENT FOR CURB AND GUTTER REPLACEMENT.

THE PROPOSED CONFIGURATION OF THE CURB AND GUTTER SHALL MATCH THAT REMOVED.

THE CONCRETE REQUIRED BETWEEN THE EDGE OF PAVEMENT AND FRAME AND GRATE SHALL BE CONSIDERED INCIDENTAL TO THE CURB AND GUTTER.

THE LOCATION OF THE DOWEL BAR SHALL BE DETERMINED BY THE ENGINEER.

THE COST OF ALL MATERIALS AND LABOR REQUIRED TO INSTALL THE JOINTS AND BARS IN THE CURBS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE PER METER (FOOT) FOR COMBINATION CURB AND GUTTER.

ALL EXISTING TIE BARS IN EDGE OF PAVEMENT SLAB THRU REPLACEMENT AREA SHALL BE CUT OFF.

25 (1) PREFORMED EXPANSION JOINT

FILLER SHALL BE ELIMINATED.

IS WITHIN 1.5m (5'-

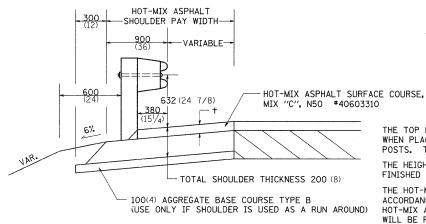
WHEN "A" IS GREATER THAN

50 (2), 2-NO. 15 (NO. 4) BARS SHALL BE PLACED AS SHOWN.

FILLER. IF EXISTING EXPANSION JOINT

17.4

DETAIL OF HOT-MIX ASPHALT SHOULDER AT GUARD RAIL



+ = SEE TYPICAL SECTIONS FOR THICKNESS

GENERAL NOTES

THE TOP LIFT SHALL NOT BE PLACED BEHIND THE GUARDRAIL POSTS. WHEN PLACING THE TOP LIFT THE RAIL MUST BE REMOVED FROM THE POSTS. THE POST SHALL NOT BE REMOVED.

THE HEIGHT OF THE GUARD RAIL SHALL BE SET 632 (24%) FROM THE

THE HOT-MIX ASPHALT SHOULDER SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 482 EXCEPT THE TOP LIFT SHALL BE HOT-MIX ASPHALT SURFACE COURSE, MIXTURE C, N50. THE WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER TON FOR HOT-MIX ASPHALT SURFACE COURSE, MIXTURE "C", N50 AND SQUARE METER (SQUARE YARD) FOR HOT-MIX ASPHALT SHOULDERS OF THE THICKNESS SPECIFIED. THE REMOVAL & REINSTALLATION OF THE GUARDRAIL WILL BE INCLUDED IN THE COST OF THE HOT-MIX ASPHALT SURFACE COURSE, MIXTURE C, N50.

> ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

REVISED - 11-01-07

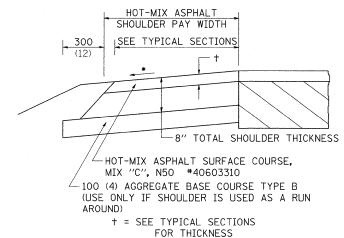
REVISED - 11-01-07

DETAIL OF HOT-MIX ASPHALT SHOULDER AT GUARD RAIL

23.4

DETAILS FOR CURB & GUTTER REPLACEMENT AT INLET

HOT-MIX ASPHALT SHOULDER



GENERAL NOTES

SAME REPAIR AS

INDICATED ON OTHER SIDE OF FRAME AND GRATE.

ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS

-PROPOSED CURB AND GUTTER,

32 (1 1/4) Ø COATED SMOOTH DOWEL BAR WITH CAP TO PROVIDE 25 (1) EXPANSION

FRAME AND GRATE

TYPE M-15,60 (M-6,24)

THE HOT-MIX ASPHALT SHOULDER SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 482 EXCEPT THE TOP LIFT SHALL BE HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N50 #40603310. THE WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER TON FOR HOT-MIX ASPHALT SURFACE COURSE. MIX "C", N50 #40603310 AND SQUARE YARD FOR HOT-MIX ASPHALT SHOULDERS OF THE THICKNESS SPECIFIED.

USE HOT-MIX ASPHALT SURFACE COURSE, MIX "C", N50 #40603310. WHEN RESURFACING EXISTING HOT-MIX ASPHALT SHOULDERS. THE THICKNESS IS SHOWN ON THE TYPICAL SECTIONS. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER TON FOR HOT-MIX ASPHALT SURFACE COURSE. MIX "C", N50 #40603310.

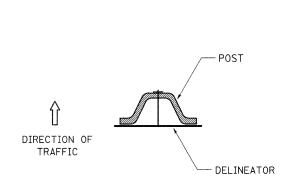
REMOVAL OF MATERIAL FOR PLACEMENT OF THE HOT-MIX ASPHALT SHOULDER TO BE PAID FOR IN UNITS FOR EXCAVATING AND GRADING EXISTING SHOULDERS OR IN CUBIC YARDS FOR EARTH EXCAVATION OR EARTH EXCAVATION WIDENING.

*4% WHEN MAINLINE IS ON TANGENT. FOR CROSS SLOPE ON SUPERELEVATION SECTION, SEE HIGHWAY STANDARD 482001 OR 482006.

> ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

REVISED - 11-01-07		RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
REVISED -	REGION 2 / DISTRICT 2 STANDARD	74	81-(1-2.1,2-2)RS-1&M	ROCK ISLAND		233
REVISED -		''				4A97
REVISED -	SCALE: 50.0000 // IN SHEET NO. OF SHEETS STA. TO STA.	FED. RO	AD DIST. NO. ILLINOIS FED. A		,,,,,	17.0

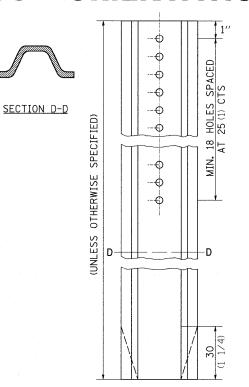
DELINEATOR AND POST ORIENTATION



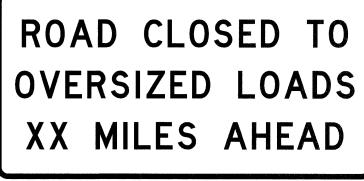
DELINEATORS SHALL BE INSTALLED ACCORDING TO STANDARD 635001 EXCEPT THAT THE POST SHALL BE ROTATED 180°. THE POST WILL HAVE THE WIDE SIDE FACING TRAFFIC AND THE DELINEATOR ATTACHECD AS SHOWN ABOVE.

ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

REVISED - 11-01-07



ROAD CLOSED TO OVERSIZED LOADS



48.1 49.7 34.4 7.1 10.5 6.5 4.8 7.6 10.7 1.7 1.7 25.6 96 7.1 30.5 1.7.6

Permit Loads - Loads Over 13 Feet; 3.0" Radius, 1.3" Border, Black on Orange; IROAD CLOSED TOJ D; IOVERSIZED LOADS] D; IXX MILES AHEADJ D; Table of letter and object lefts.

R 6.5	0 12.5	A 18.7	D 25.9	C 37.6	L 43.6	0 49.2	\$ 55.4	E 61.8	D 67.3	T 79.0	0 84.6		
0 4.8	V 11.0	E 17.6	R 23.1	\$ 29.2	1 35.5	Z 38.2	E 44.3	D 49.8	L 61.5	0 67.1	A 73.3	D 80.5	S 86.5
X	X 12.6	M 26.2	1 22 2	L 25.1	E 40.6	S 46.2	A	H 65.1	E 71.4	A	D 92.7		

All work to furnish and install these signs shall be included in the cost of the Traffic Control Standards and shall not be paid for separately.

ALL DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED.

REVISED - 1-9-08

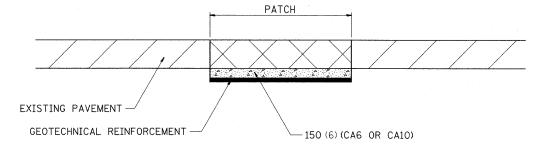
DELINEATOR AND POST ORIENTATION

37.4

ROAD CLOSED TO OVERSIZED LOADS

40.4

SUBGRADE REPLACEMENT



NOTES:

THE CA 6 OR CA 10 SHALL BE COMPACTED IN A MANNER APPROVED BY THE ENGINEER. IF THE MOISTURE CONTENT OF THE MATERIAL IS SUCH THAT COMPACTION SATISFACTORY TO THE ENGINEER CANNOT BE OBTAINED, SUFFICIENT WATER SHALL BE ADDED SO THAT SATISFACTORY COMPACTION CAN BE OBTAINED.

THE CA 6 OR CA 10 WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CU YD FOR GRANULAR SUBGRADE REPLACEMENT

THE GEOTECHNICAL REINFORCEMENT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER SQ YD FOR GEOTECHNICAL REINFORCEMENT

ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

REVISED - 1-09-08

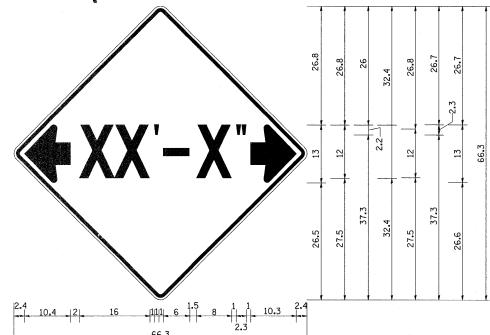
REVISED - REGION 2 / DISTRICT 2 STANDARD

REVISED - REVISED - REVISED - SCALE: 50.0000 // IN SHEET NO. OF SHEETS STA. TO STA. FED. ROAD DIST. NO. ||LLINOIS|FED. ARD PROJECT |

F.A.I. SECTION COUNTY TOTAL SHEET NO. |
RTE. SECTION COUNTY SHEETS NO. |
RTE. SECTION COUNTY SHEETS NO. |
REVISED - SCALE: 50.0000 // IN SHEET NO. OF SHEETS STA. TO STA. |
FED. ROAD DIST. NO. ||LLINOIS|FED. ARD PROJECT |

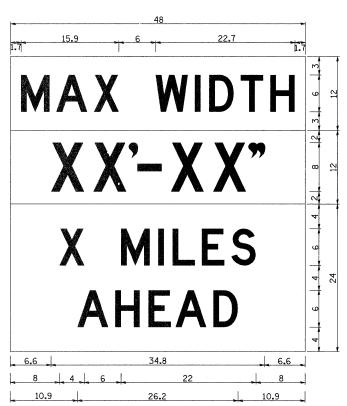
FED. ROAD DIST. NO. ||LLINOIS|FED. ARD PROJECT |

INFORMATIONAL WARNING SIGN (FOR NARROW TRAVEL LANES)



NOTES

W12-2 - Horizontal Clearance Sign 48.0" across sides, 1.9" Radius, 0.8" Border, 0.5" Indent, Black on Orange; Standard Arrow Custom 10.4" X 8.1" 180° Black 11 Inch D Series Lettering; Standard Arrow Custom 10.4" X 8.1" 0°



REVISED - 1-9-08

W12-I103 (Width is 8D); No border, Black on White;

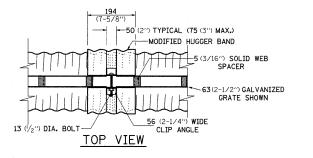
No border, Black on Orange: [XX'-XX''] D:

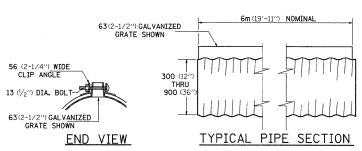
No border, Black on White: [X MILES] D: [AHEAD] D:

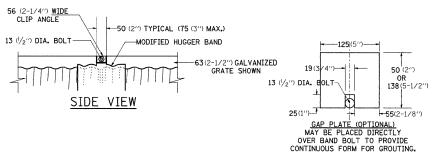
> All work to furnish and install these signs shall be included in the cost of the Traffic Control Standards and shall not be paid for separately.

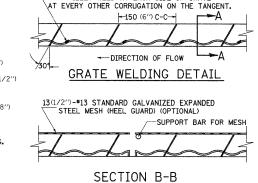
ALL DIMENSIONS ARE IN INCHES UNLESS

SLOTTED DRAIN PIPE

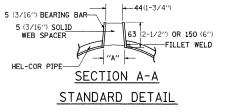








FILLET WELD AT EACH SIDE OF GRATE



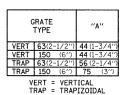
SECTION A-A

DETAIL WITH MESH

(TRAPEZOIDAL GALVANIZED GRATE SHOWN)

STEEL MESH (HEEL GUARD)





- 1. GRATING IS AVAILABLE IN DEPTHS OF 63(2-1/2") AND 150(6").
- VERTICAL GRATING (STRAIGHT SIDES) WITH VERTICAL SPACERS IS ALSO AVAILABLE.
- 3. FOR 150 (6") VERTICAL & TRAPIZOIDAL REQUIREMENTS, THE SLOTTED DRAIN BAND MAY BE FURNISHED WITH THE 100 (4") TECHCO BAND ANGLE.
- 4. DIMENSIONS ARE SUBJECT TO MANUFACTURING TOLERANCES
- REFERENCE CONTECH BAND MANUAL DWG. NO. 1008466 FOR BAND DETAILS.

MANUE	ACTURING	TOLERANCES

- 1. VERTICAL BOW ± 10 (3/8")
- 2. HORIZONTAL BOW ± 16 (5/8")
- 3. TWIST ± 13 (1/2")

NOTES

CLASS SI CONCRETE SHALL EE USED THROUGHOUT.

5 (3/16") BEARING BAR-5 (3/16") SOLID

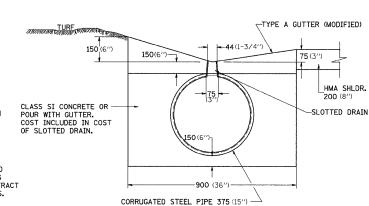
WER SPACER

HEL-COR PIPE-

THE SLOTTED DRAIN SHALL BE CORRUGATED PIPE CULVERT WITH INTEGRAL SLOTTED DRAINS. BEFORE PLACING THE CONCRETE ADJACENT TO THE PIPE, THE SLOT SHALL BE COVERED BY EITHER THIN, FLAT METAL SHEETING OR BY A BOARD NOTCHED TO FIT OVER THE GRATE BARS. THIS COVERING MUST FIT CLOSELY IN THE SLOT TO PREVENT ENTRY OF CONCRETE INTO THE PIPE. PAVING OVER THE SLOTTED DRAIN WILL THEN BE ONE CONTINUOUS OPERATION OVER THE PROTECTED DRAIN, THE PROTECTION FOR THE DRAIN SLOT SHALL THEN BE REMOVED. THE PIPE SHALL DRAIN INTO THE SIDE OF THE INLET. THE OPENING WHERE THE SLOT IS REMOVED SHALL BE COVERED TO PREVENT CONCRETE FROM ENTERING THE PIPE.

THE CORRUGATED STEEL PIPE USED IN THE SLOTTED DRAIN SHALL MEET THE REQUIREMENTS OF AASHTO M-36/ASTM A 760. THE CMP SHALL BE GALVANIZED OR ALUMINIZED STEEL TYPE 2. STEEL GRATING SHALL MEET THE GALVANIZING REQUIREMENTS OF AASHTO M-111. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER FOOT FOR SLOTTED DRAIN PIPE, AND SHALL INCLUDE ELBOWS.

USE APPROVED END CAP TO PREVENT CONCRETE ENTRY INTO THE PIPE DURING GUTTER CONSTRUCTION ON THE UPSTREAM END OF PIPE.



ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) LINESS OTHERWISE NOTED.

REVISEREVISED-08 SECTION COUNTY REVISED REGION 2 / DISTRICT 2 STANDARD ROCK ISLAND 246 235 81-(1-2,1,2-2)RS-1&M REVISED CONTRACT NO. 64A97 REVISED SCALE: 50.0000 // IN SHEET NO. OF SHEETS STA. TO STA.

INFORMATIONAL WARNING SIGNS (FOR NARROW TRAVEL LANES)

SLOTTED DRAIN PIPE

TOTAL SHEE SHEETS NO.

ROUGH GROOVED SURFACE SIGN

ILLINOIS STANDARD W8-I107

SIGN PANEL TYPE 1



COLOR: LEGEND AND BORDER - BLACK NON-RELFLECTIVE BACKGROUND - ORANGE REFLECTORIZED

SIGN			DI	MENSIO	VS.			
SIZE	Α	В	С	D	E	F	G	Н
1200×1200	1200	600	75	850	825	150	325	88
(48×48)	(48.0)	(24.1)	(3.0)	(34.0)	(33.0)	(6.0)	(13.0)	(3.5)

SIGN SIZE		SERIES LINES		MARGIN	BORDER	BLANK STD.	
SIZE	1	2	3			310.	
1200×1200 (48×48)	7C	7C	7C	20 (0 . 8)	30 (1 . 2)	B4-48D	

ALL DIMENSIONS IN INCHES.

GENERAL NOTES

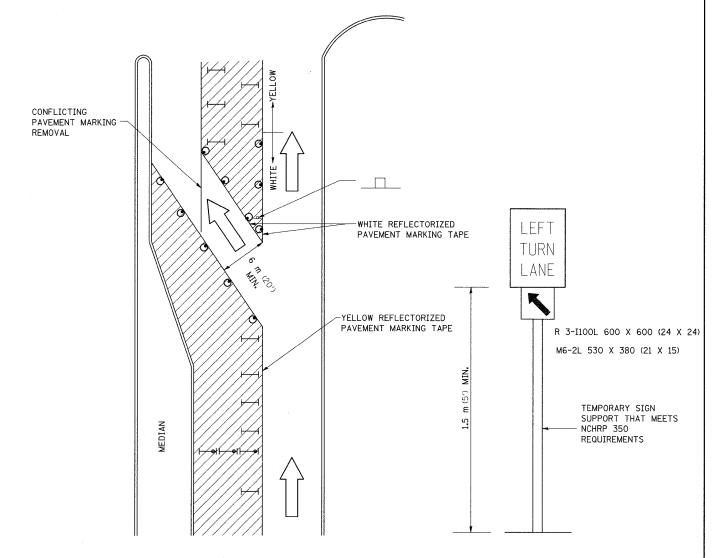
SIGN PANELS AND FACE MATERIALS SHALL BE ACCORDING TO SECTION 720 OF THE STANDARD SPECIFICATIONS

METAL POSTS SHALL BE IN ACCORDANCE WITH STD. 720011.

ALL MOUNTING HARDWARE SHALL BE ALUMINUM, STAINLESS STEEL, ZINC OR CADMIUM PLATED STEEL AND SHALL BE INCLUDED TO THE COST OF THE INSTALLATION.

ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

TRAFFIC CONTROL AND PROTECTION AT TURN BAYS (TO REMAIN OPEN TO TRAFFIC)



LEGEND

WORK AREA

.



TYPE I OR II BARRICADE OR DRUM WITH FLASHING BURNING LIGHT

LANE OPEN TO TRAFFIC



DRUM OR BARRICADE WITH STEADY BURN LIGHT



SIGN (SEE DETAIL)

|----

TYPE I OR II CHECK BARRICADE WITH STEADY LIGHT BURN

GENERAL NOTES

CONES MAY BE SUBSTITUTED FOR BARRICADES OR DRUMS AT HALF THE SPACING DURING DAY OPERATIONS. CONES SHALL BE A MINIMUM OF 710 (28) IN HEIGHT.

STEADY BURNING LIGHTS WILL NOT BE REQUIRED ON BARRICADES OR DRUMS FOR DAY OPERATIONS. ALL LIGHTS WILL BE MONODIRECTIONAL.

REFLECTORIZED TEMPORARY PAYEMENT MARKING TAPE SHALL BE PLACED THROUGHOUT THE BARRICADED AREA OF EACH BAY WHERE THE CLOSURE TIME IS GREATER THAN FOURTEEN DAYS.

THIS APPLICATION ALSO APPLIES WHEN WORK IS BEING PERFORMED IN THE RIGHT LANE(S) AND THE RIGHT TURN BAY IS TO REMAIN OPEN. UNDER THIS CONDITION, "RIGHT TURN LANE" R3-100 600 x 600 (24 X 24) AND M6-2R 530 X 380 (21 X 15) SHALL BE USED.

THESE CONTROLS SHALL SUPPLEMENT MAINLINE TRAFFIC CONTROL FOR LANE CLOSURES. LONGITUDINAL DIMENSIONS MAY BE ADJUSTED TO FIT FIELD CONDITIONS.

TRAFFIC CONTROL AND PROTECTION AT TURN BAYS (TO REMAIN OPEN TO TRAFFIC) SHALL BE INCLUDED IN THE COST SPECIFIED TRAFFIC CONTROL STANDARDS OR ITEMS.

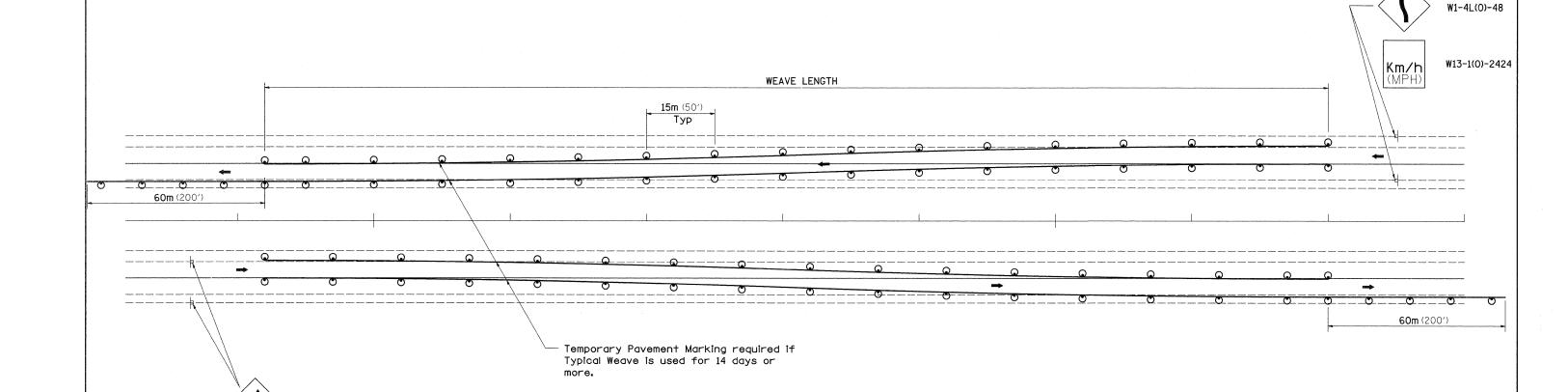
ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

REVISED - 10-15-04
REVISED - REGION 2 / DISTRICT 2 STANDARD

REVISED - REGION 2 / DISTRICT 2 STANDARD

REVISED - REVISED - SCALE: 50.00000 1/ IN SHEET NO. 0F SHEETS STA. TO STA. FED. ROAD DIST. NO. | ILLINOIS FED. AID PROJECT |

TRAFFIC CONTROL TYPICAL WEAVE



G DRUM

G DRUM WITH STEADY BURN MONO-DIRECTIONAL LIGHTS

LEGEND

SIGN ON PERMANENT MOUNT

DESIGNER NOTE:

W1-4R(0)-48

W13-1(0)-2424

- USE ON LONG 4-LANE PROJECTS WHERE THE CONTRACTOR MAY CHANGE A PORTION OF THE WORK TO THE OPPOSITE LANE.
- USE WHERE THE PROJECT IS ADJACENT TO ANOTHER AND THE CONTRACTOR COULD BE WORKING ON DIFFERENT LANES.
- 3. TEMPORARY PAVEMENT MARKING SHALL BE USED WHEN TYPICAL WEAVE IS USED FOR 14 DAYS OR MORE.
- 4. TRAFFIC CONTROL TYPICAL WEAVE SHALL BE INCLUDED IN THE COST OF THE SPECIFIC TRAFFIC CONTROL STANDARDS OF ITEMS.

STANDARD WEAVE CONDITIONS FOR DIFFERENT SPEED LIMITS

POSTED SPEED LIMIT

ADVISORY SPEED LIMIT

WEAVE LENGTH

110 Km/h (65 MPH) 90 Km/h (55 MPH)

80 Km/h (45 MPH) 60 Km/h (35 MPH) 240m (780 FT.)

80 Km/h (45 MPH)

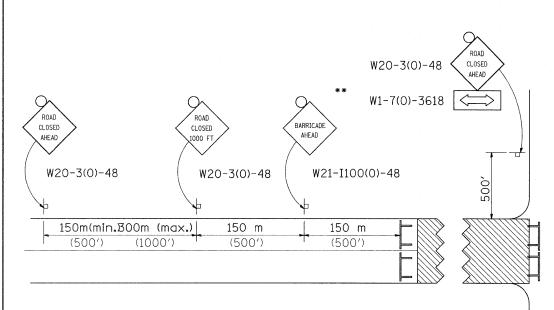
40 Km/h (25 MPH)

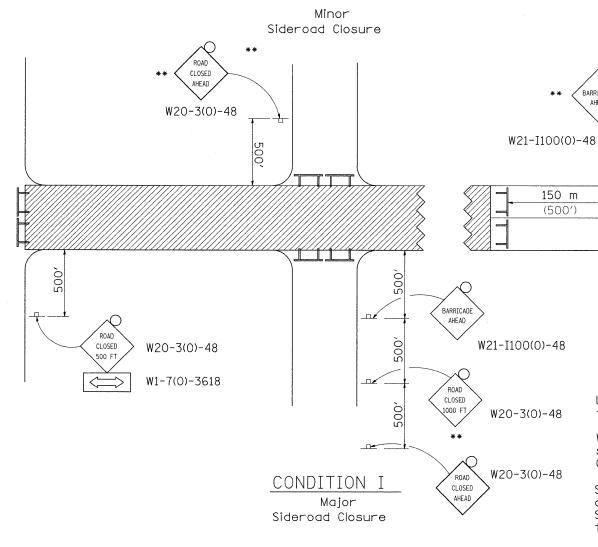
200m (660 FT.) 165m (540 FT.)

ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

FILE NAME =	USER NAME = duncanfa	DESIGNED -	REVISED - 5-30-91			F.A.I. SECTION	COUNTY TOTAL SHEET
c:\pw_work\PWIDOT\DUNCANFA\dms33952\d	105805-sht-ts.dgn	DRAWN -	REVISED -	STATE OF ILLINOIS	REGION 2 / DISTRICT 2 STANDARD	74 81-(1-2.1-2-2)RS-1&M	DOCK TELAND 24C 277
	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED -	DEPARTMENT OF TRANSPORTATION		11 02 12 2720 2710	CONTRACT NO. 64A97
	PLOT DATE = Fr: Feb 06 10:54:57 2009	DATE -	REVISED -		SCALE: SHEET NO. OF SHEETS STA. TO STA.	FED. ROAD DIST. NO. ILLINOIS FED.	

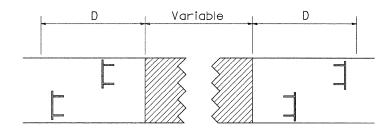
TRAFFIC CONTROL FOR ROAD CLOSURE





CONDITION II

ROAD CLOSED TO THRU TRAFFIC BARRICADE SET UP



Type III Barricades and R11-4-4830 signs shall be as shown in "Road Closed To All Thru Traffic" detail on Highway Standard 701901. If the distance "D" exceeds 600 m (2000") an additional set of barricades and R11-4-4830 shall be placed at each end of the work area.

SYMBOLS



Work area



Type III Barricade with Flashers



Sign with flashing light

GENERAL NOTES

W20-3(0)-48

150 m

(500')

Longitudinal dimensions may be adjusted to fit field conditions.

W20-3(0)-48

(1000')

150m(min.B00m (max.)

(500')

When speed limit is less than 45mph, change sign spacing to 250' and change ROAD CLOSED 1000 FT to ROAD CLOSED 500 FT.

Side roads requiring all three signs as shown in CONDITION I (Major Sideroad Closure), shall be listed in the special provision.

Where local access is to be maintained, barricades are to be set up as shown in Road Closed to thru traffic. Type III Barricades and R11-2-4830 signs shall be as shown in "Road Closed To All Traffic" detail on Highway Standard 701901.

All dimensions are in millimeters (inches) unless otherwise shown.

TYPICAL APPLICATION FOR ROAD CLOSURE

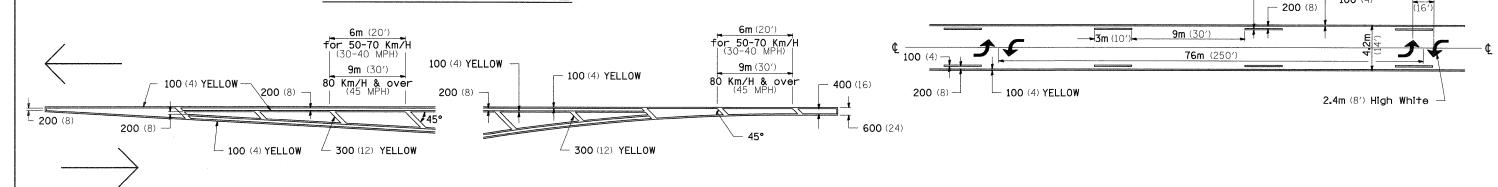
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	PLOT DATE = Fr1 Feb 06 10:54:57 2009	DATE -	REVISED -		SCALE: S	SHEET NO. C	OF S	SHEETS	STA.	TO STA.	FED. ROA	AD DIST, NO. ILLINOIS FED.	AID PROJECT	, ITO. OTAS	\dashv

TYPICAL PAVEMENT MARKINGS

MEDIAN PAVEMENT MARKING

100 (4) YELLOW

TYPICAL PAVEMENT MARKING FOR FLUSH MEDIAN AT LEFT TURN LANE



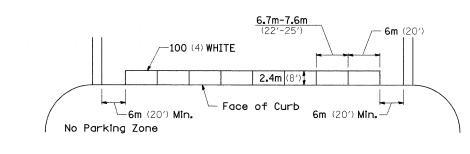
TYPICAL ISLAND OFFSET SHOULDER WIDTH

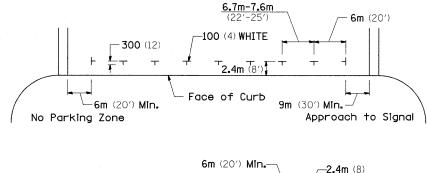
600 (24) WHITE 200 (8) WHITE 200 (8) WHITE 5 Diagonals

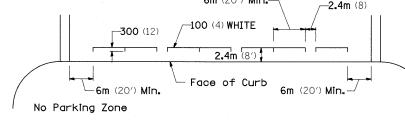
Minimum

-300 (12) WHITE

TYPICAL PARKING SPACING







** ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE NOTED.

STANDARD CROSSWALK MARKING

See Schedules for Locations

			Edge of Pave	<u>men</u> t
)	\-	1 9m (C() Minimum	* 1.2m (4 9m (30	1′) Min. 0′) Max.
→		1.8m (6') Minimum /		
<u>+</u>		600	top Bars (24) WHITE	

 Distance to the nearest edge of the intersecting roadway in the absence of a marked crosswalk.

- 100 (4)

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	PLOT DATE = Fr1 Feb 06 10:54:57 2009	DATE	-	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

REGION 2 / DISTRICT 2 STANDARD

SHEET NO. OF SHEETS STA. TO STA.

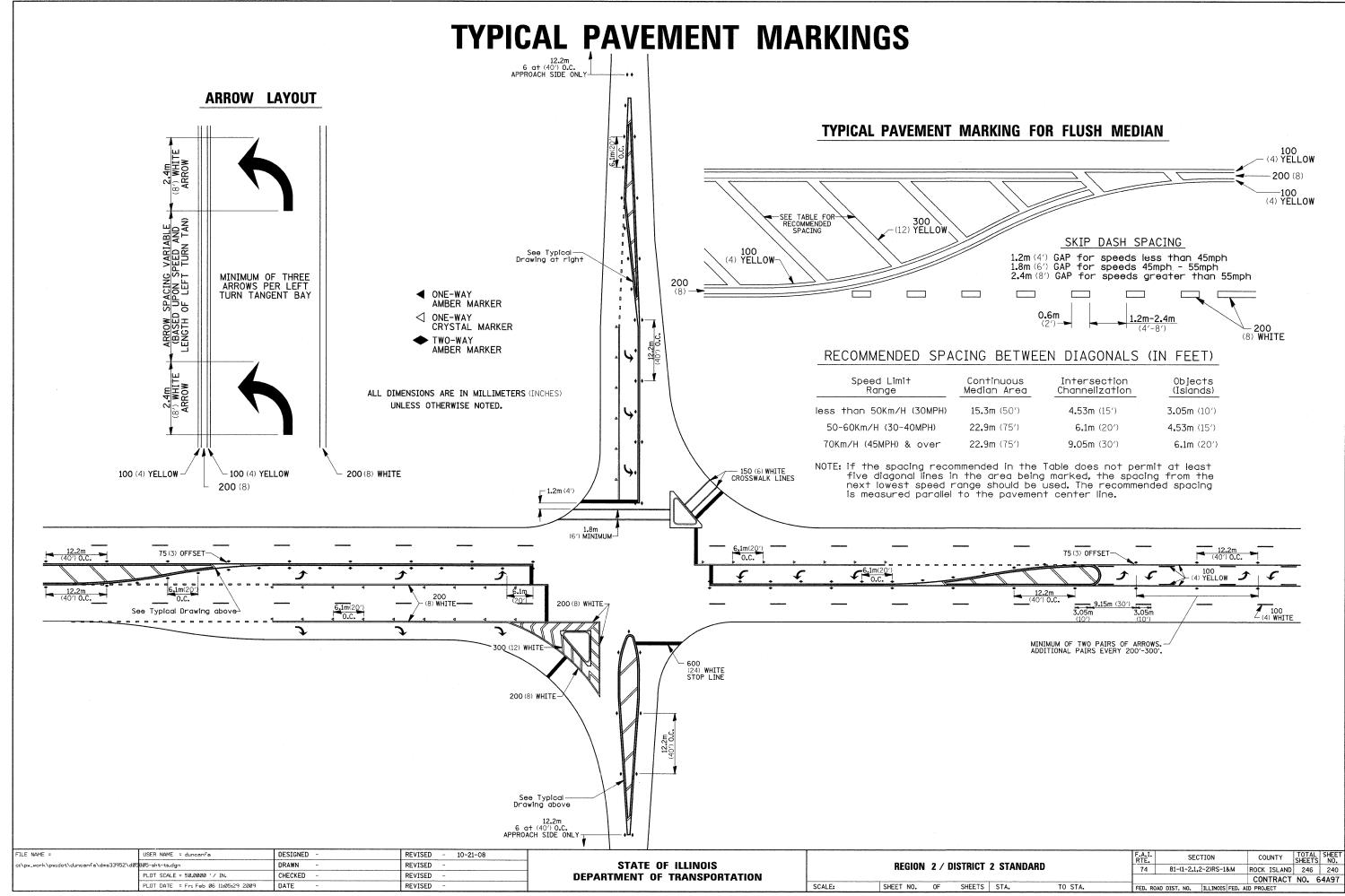
SCALE:

F.A.I. SECTION COUNTY TOTAL SHEETS NO.

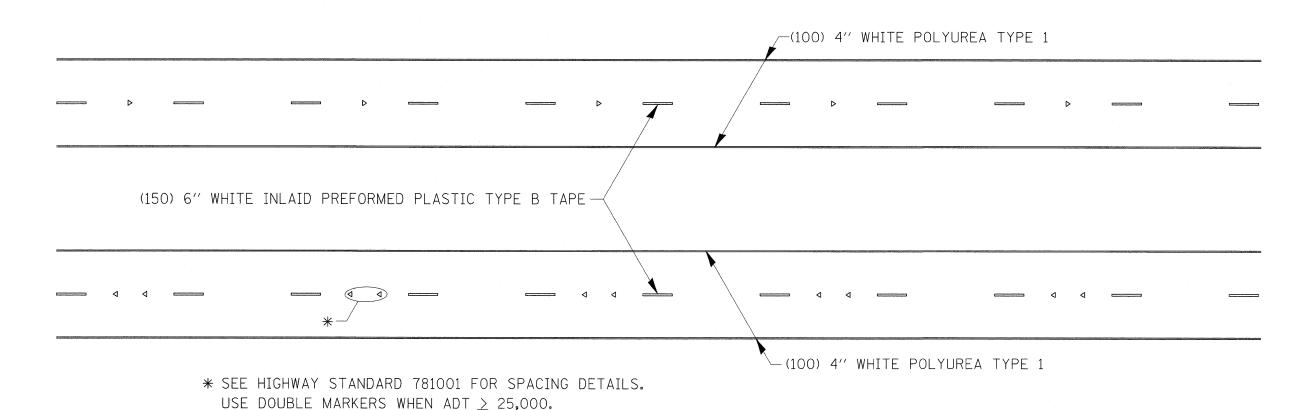
74 81-(1-2,1,2-2)RS-1&M ROCK ISLAND 246 239

CONTRACT NO. 64A97

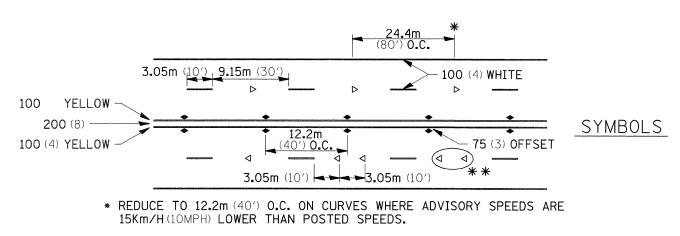
41.1



TYPICAL PAVEMENT MARKINGS



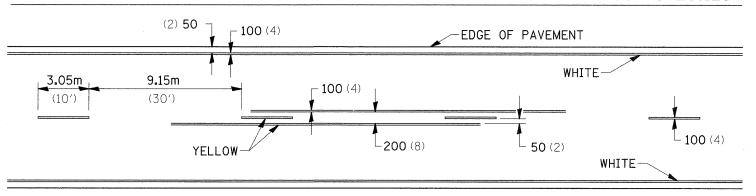
MULTI-LANE / DIVIDED



** USE DOUBLE MARKERS WHEN ADT > 25,000

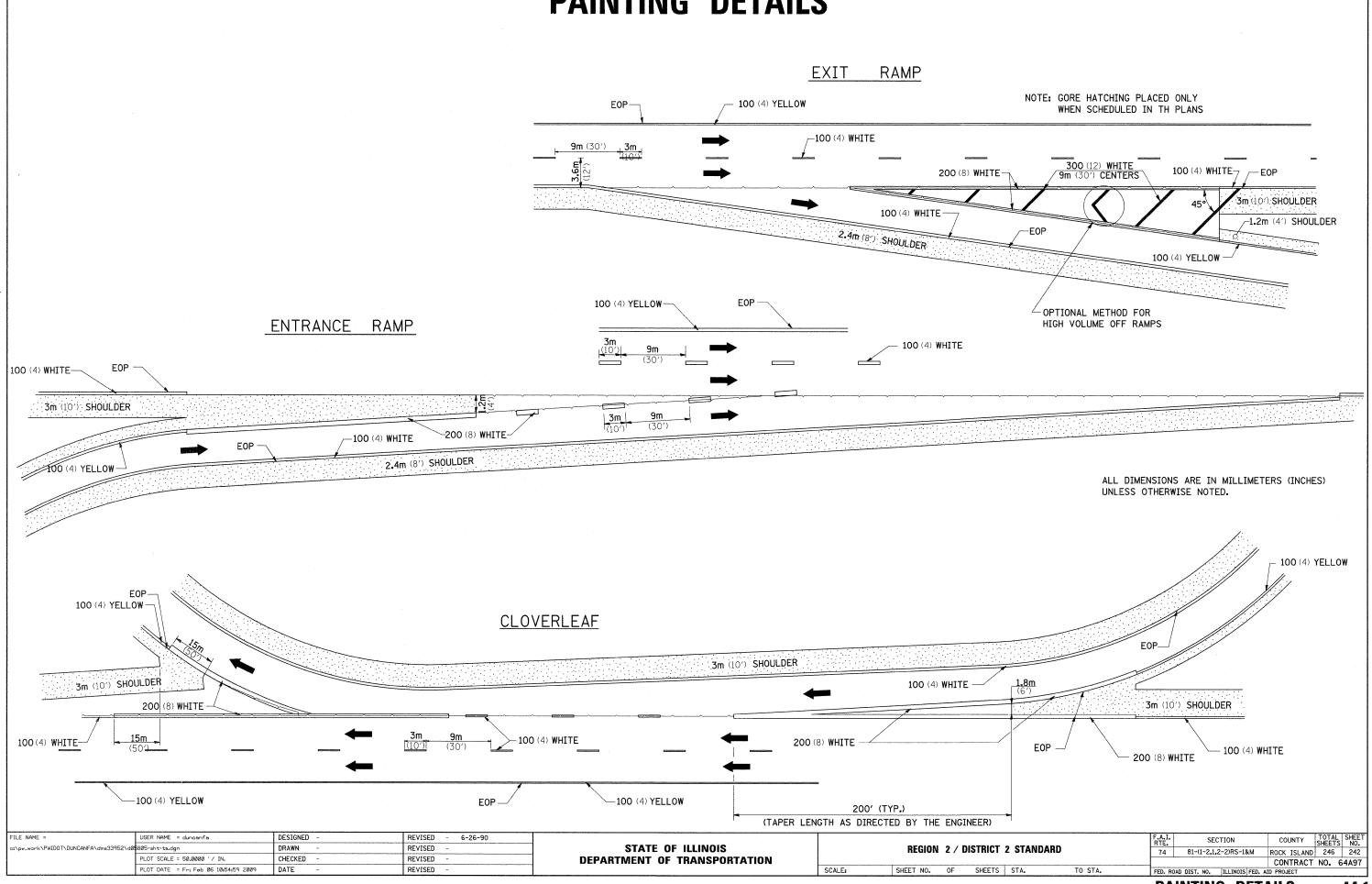
MULTI-LANE / UNDIVIDED

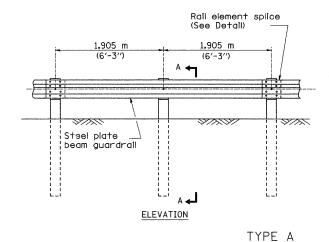
TYPICAL PAVEMENT MARKING FOR TWO LANE SECTION - NO PASSING ZONES

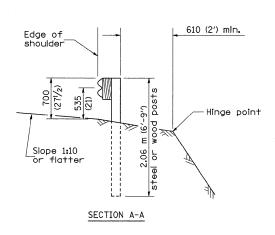


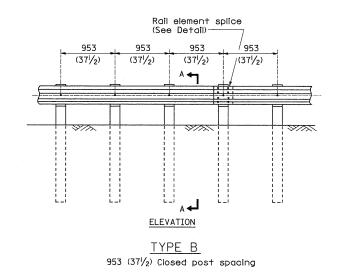
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	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED -	DEPARTMENT OF TRANSPORTATION		74 01 11 2545 2710 1011	CONTRACT NO. 64A97
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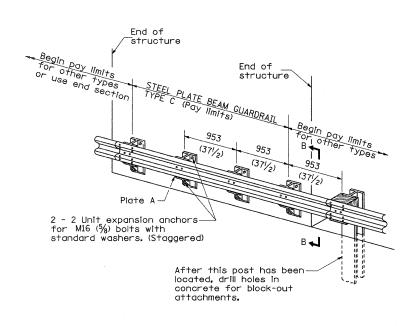
PAINTING DETAILS





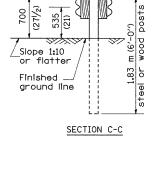


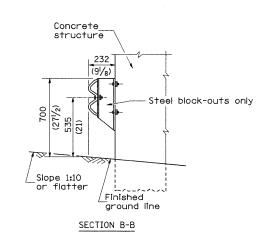


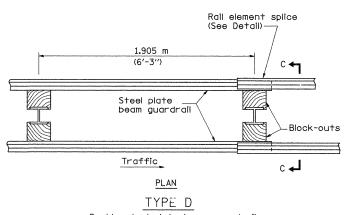




1.905 m (6'-3") Typical post spacing







Double steel plate beam guardrail 1.905 m (6'-3") typical post spacing

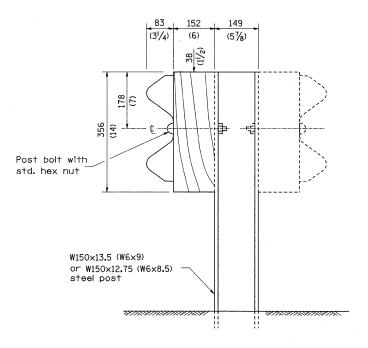
GENERAL NOTES

All slope ratios are expressed as units of vertical displacement to units of horizontal displacement (V:H).

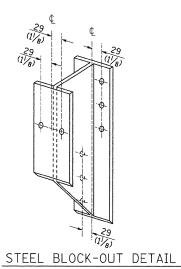
All dimensions are in millimeters (inches) unless otherwise shown.

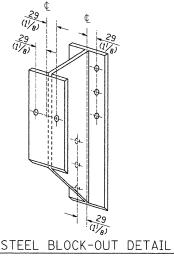
The existing steel posts may be drilled to match the bolt pattern shown herein for the wood block-out, or a new steel post shall be provided. This detail is applicable to the guardrall system used prior to January 1, 2007. For details on the Midwest Guardrall System, see Standard 630001.

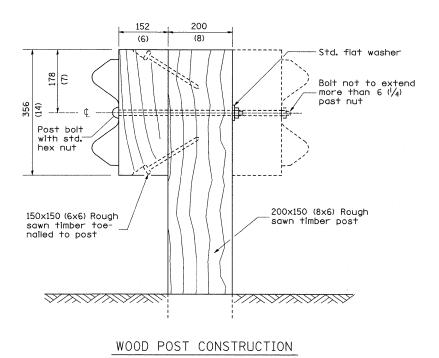
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ar Carlo	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED ~	DEPARTMENT OF TRANSPORTATION					CONTRACT NO. 64A97
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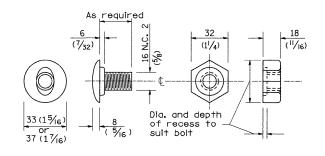


STEEL POST CONSTRUCTION









POST OR SPLICE BOLT & NUT

	305 (12)		
19 Dia. hole (¾)	¢.	310 (12/4)	-¢
	NO ²	TE	

Plate A shall be placed between rail element and block-out at non-splice mounting points only when steel block-outs are used.

PLATE A

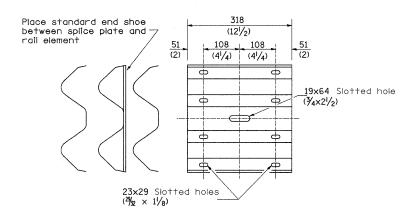
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		PLOT DATE = Fr: Feb 06 10:54:59 2009	DATE -	REVISED -	

STATE	OF	ILLINOIS
DEPARTMENT	OF 1	TRANSPORTATION

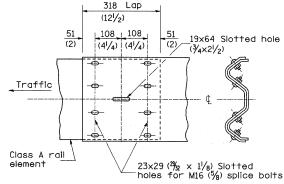
REGION 2/DIS			DIOTRICT			F.A.I. RTE.	SEC	CTION	
		DISTRICT	2 STANDARD		74	81-(1-2,1,2	2-2)RS-1&M	_	
	SHEET NO.	OF	SHEETS	STA.	TO STA.	FED. RO	DAD DIST, NO.	ILLINOIS FED). A

COUNTY TOTAL SHEET NO.

ROCK ISLAND 246 244 CONTRACT NO. 64A97

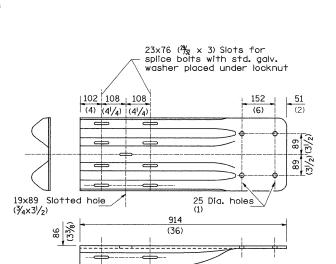


SPLICE PLATE



RAIL ELEMENT SPLICE

REVISED

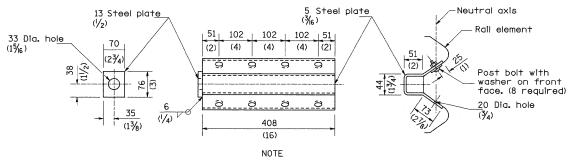


When end shoe is attached to a bridge parapet which has an expansion joint, the bolts shall be provided with a locknut or double nut and shall be tightened only to a point that will allow guardrail movement.

The standard end shoe shall be attached to the concrete with pre-drilled or self-drilling anchor bolts. The anchor cone shall be set flush with the surface of the concrete.

Externally threaded studs protruding from the surface of the concrete

END SHOE



Anchor plate T shall be used to attach cable assembly to guardrail when required on traffic barrier terminals.

ANCHOR PLATE T DETAILS

(7¹/₄)

Ð.

23x29 Slotted Holes

 $(\frac{29}{32} \times 1\frac{1}{8})$

76 108 108

(3) (41/4) (41/4)

 $(22\frac{3}{4})$

 $(\frac{3}{4} \times 2^{1/2})$

ALTERNATE END SHOE

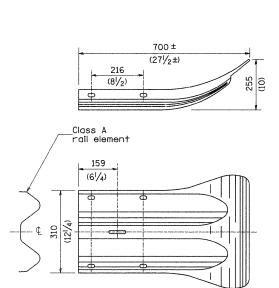
108 108

(41/4) (41/4)

① = 19x64 Slotted hole (optional)

(2)

_25 Dia. holes



END SECTION

PLOT DATE = Fr: Feb Ø6 10:54:59 2009

FILE NAME =

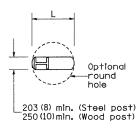
USER NAME = duncanfa DESIGNED REVISED - 1-15-08 :\pw_work\PWIDOT\DUNCANFA\dms33952\d0\$805~sht~ts.dgn DRAWN REVISED PLOT SCALE = 50.0000 '/ IN. CHECKED REVISED

DATE

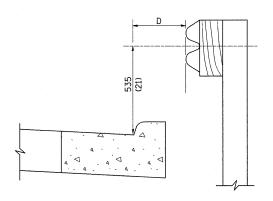
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

REGION	2 /	DISTRICT	2 STANDARD	
 SHEET NO.	0F	SHEETS	STA.	

RIE.	SECTION	COUNTY	SHEETS	SHEET NO.
74	81-(1-2,1,2-2)RS-1&M	ROCK ISLAND	246	245
		CONTRACT	NO. 6	4A97



PLAN

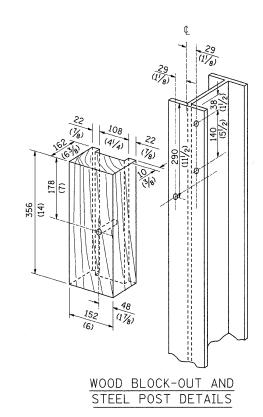


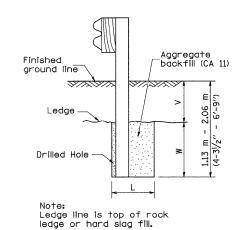
Note:

If it is necessary for D to be more than 300 (12) and less than 3.0 m (10'-0'') type M-5 (M-2)curb and gutter (Std. 606001) shall be used in front of and in advance of the guardrail.

GUARDRAIL PLACED BEHIND CURB

(D = 0 desirable to 300 (12) maximum)



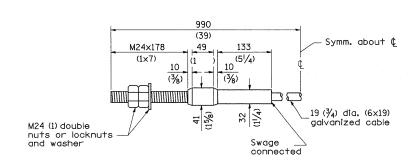


ELEVATION

FOOTING FOR POST WHEN IMPERVIOUS

MATERIAL IS ENCOUNTERED

V	w	L		
V	"	Steel Post	Wood Post	
0 - 460 (0 - 18)	610 (24)	530 (21)	580 (23)	
>460 - 825 (>18 - 41.5)	305 (12)	203 (8)	250 (10)	
>825 - 1.13 m	305 - 0	203	250 (10)	



CABLE ASSEMBLY (18,100 kg (40,000 lbs.) min. breaking strength) Tighten to taut tension.

FILE NAME =	USER NAME = duncanfa	DESIGNED -	REVISED - 1-15-08	
c:\pw_work\PWIDOT\DUNCANFA\dms33952\dØ	805-sht-ts.dgn	DRAWN -	REVISED -	STATE OF ILLINOIS
	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED -	DEPARTMENT OF TRANSPORTATION
	PLOT DATE = Fr: Feb 06 10:55:00 2009	DATE -	REVISED -	

RE	GION	2 /	DISTRICT	2	STANDARD
 SHEET	NO.	0F	SHEETS	1	STA.

 		CONTRACT	NO. E	4A97
74	81-(1-2.1.2-2)RS-1&M	ROCK ISLAND	246	246
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE.