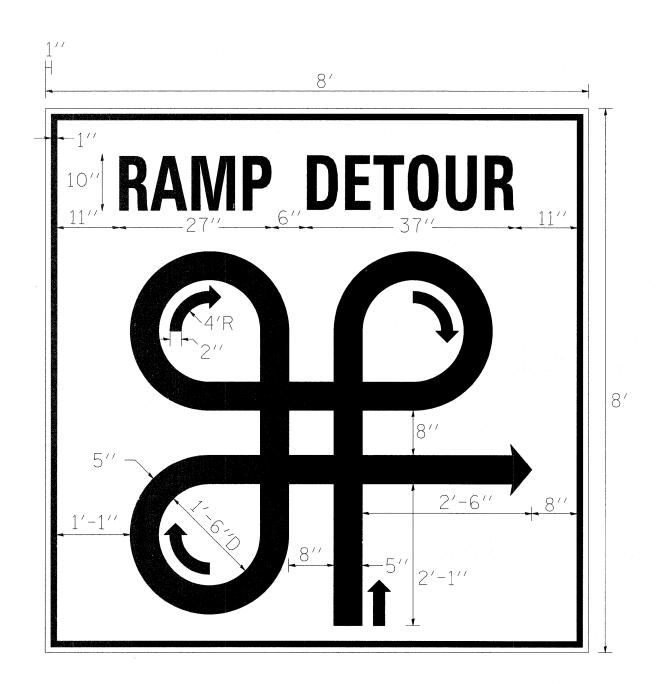


SPECIAL SIGN DETAILS



RAMP DETOUR

10" GOTHIC C FONT LETTERS

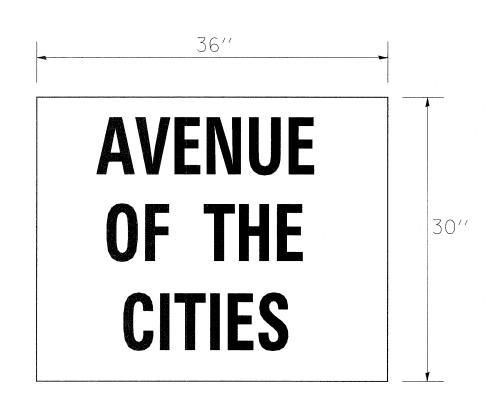
ORANGE - BACKGROUND

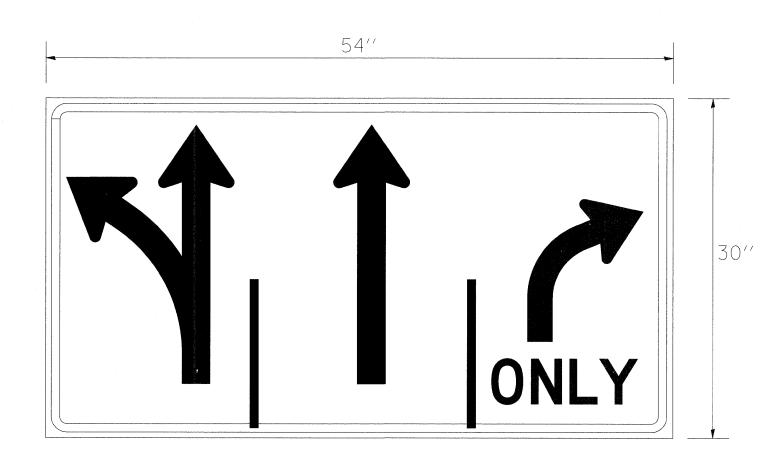
BLACK - LETTERS AND SYMBOLS

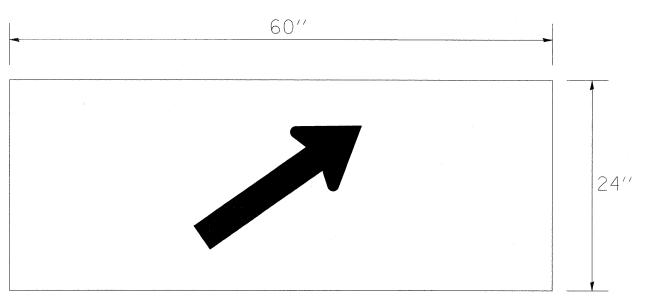
NOTE: SEE SPECIAL PROVISIONS FOR MOUNTING DETAILS

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State Control	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED -	DEPARTMENT OF TRANSPORTATION	SPECIAL SIGN DETAILS						CONTRAC	10 10	7		
	PLOT DATE = Fr1 Feb 06 10:54:46 2009	DATE -	REVISED -		SCALE:	SHEET NO.	0F	SHEETS	STA.	TO STA.	1	ILLINOIS FED.	AID PROJECT	, 1101 0 1113	
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SPECIAL SIGN DETAILS



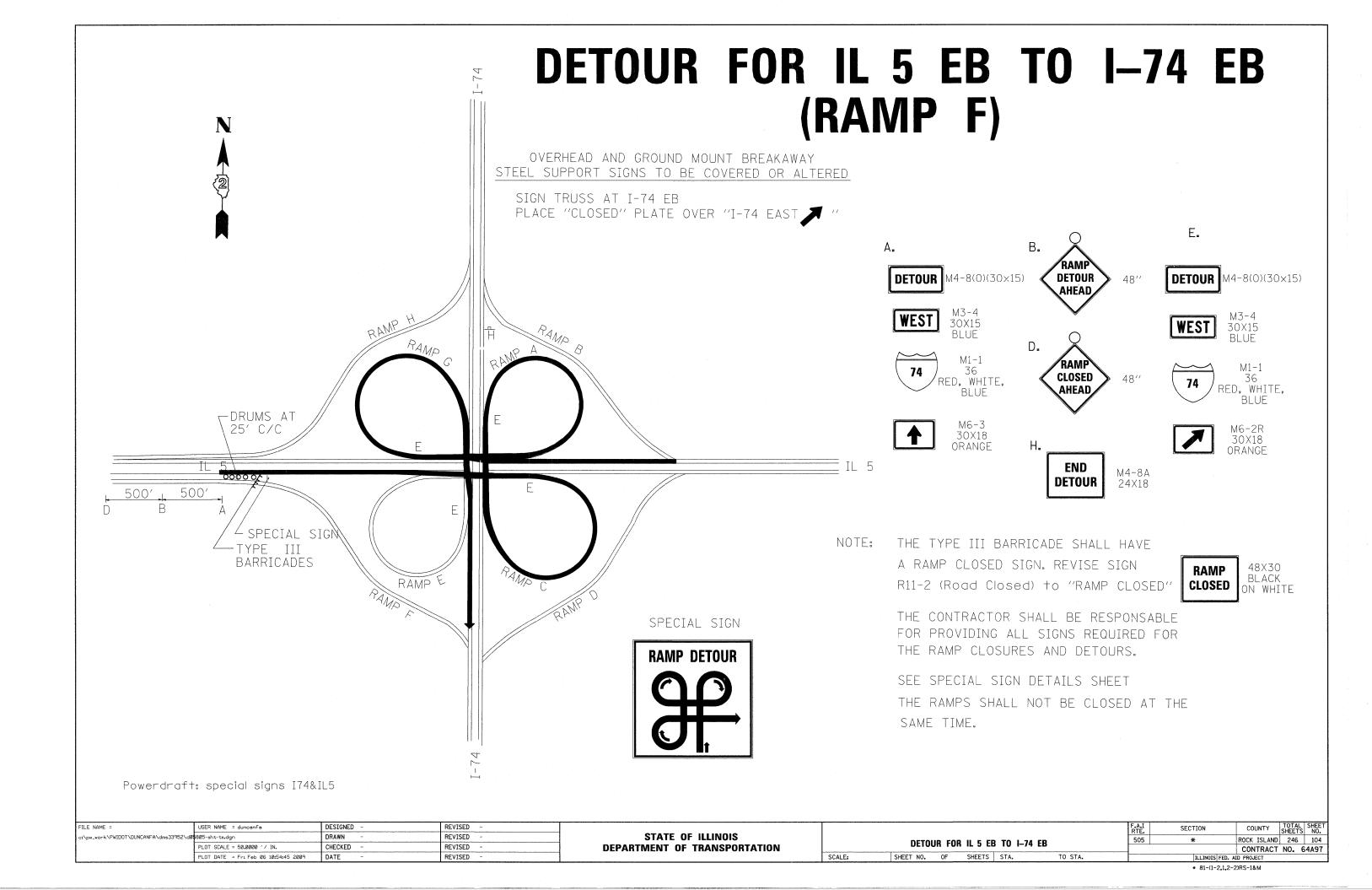


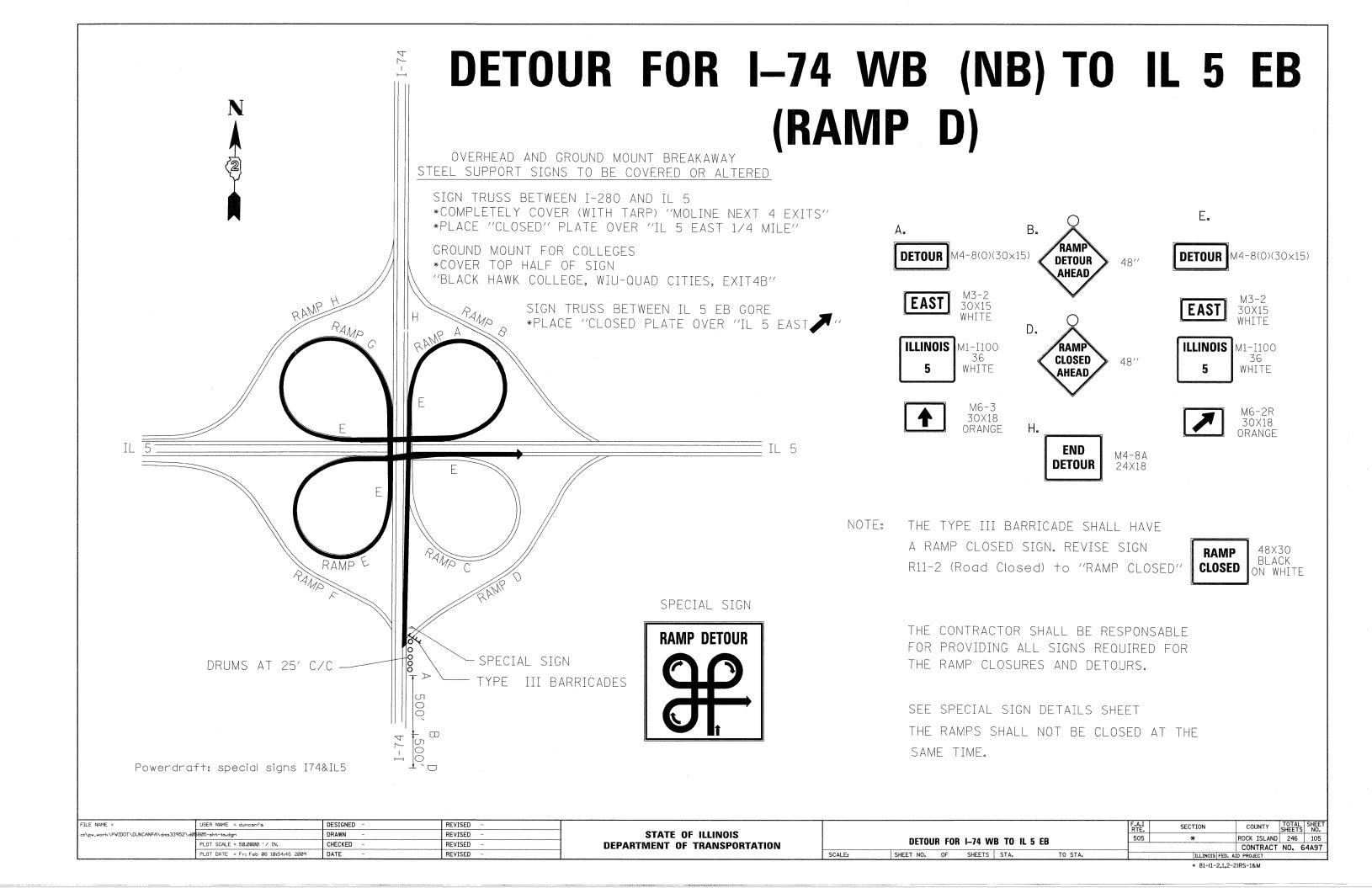


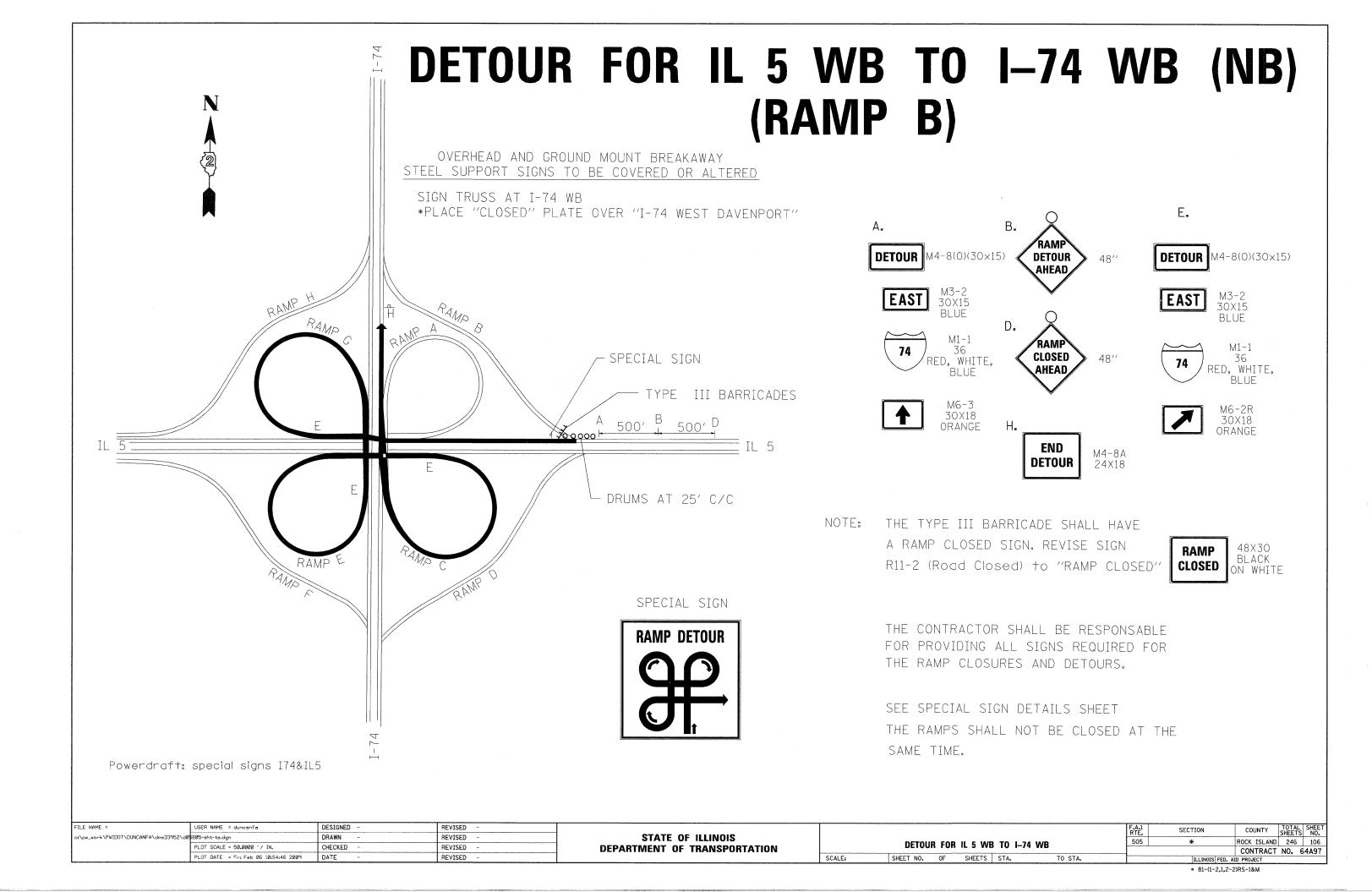
SIGN DESIGNS WILL BE GIVEN AT PRE-CONSTRUCTION MEETING.

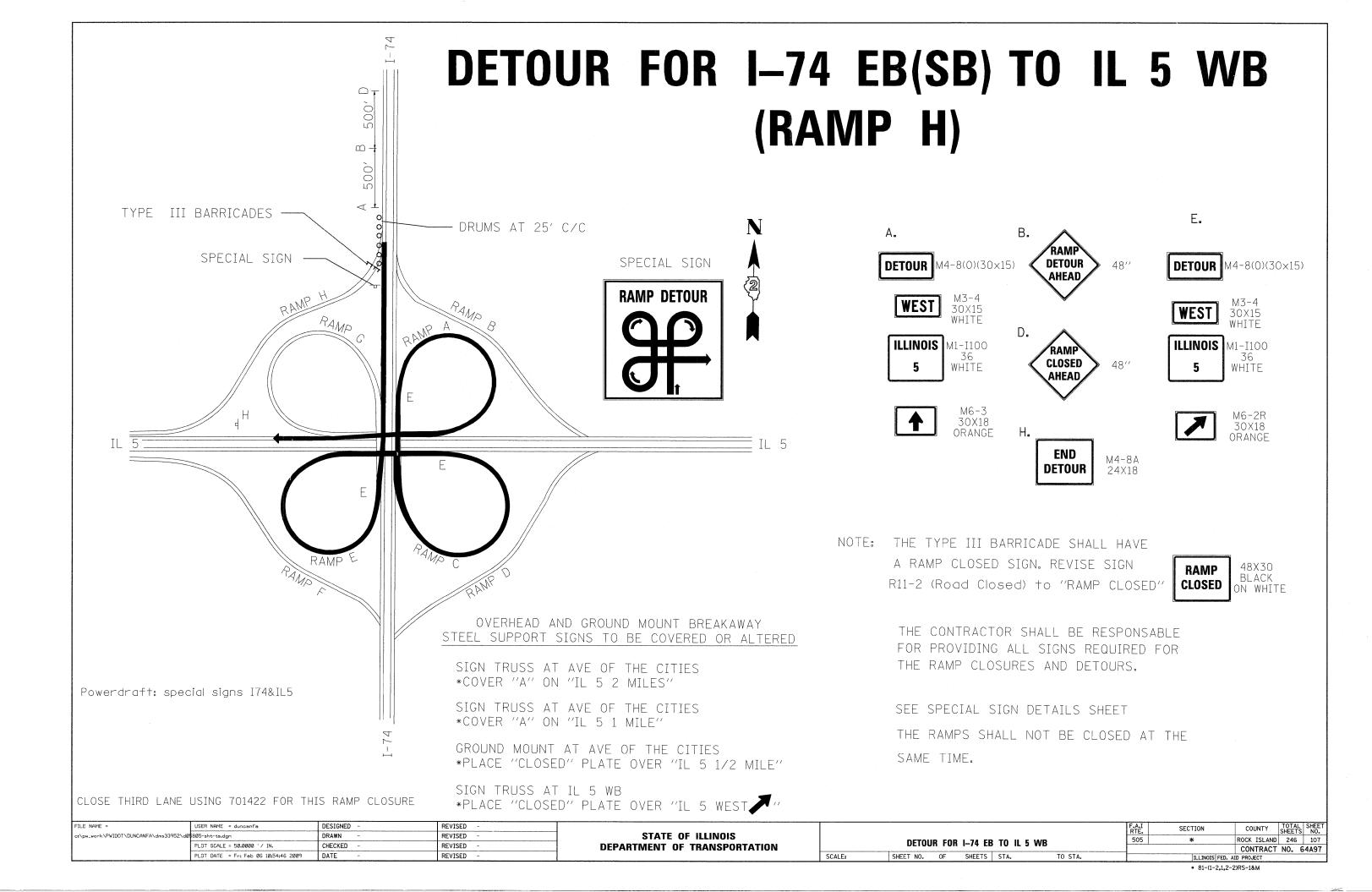
NOTE: SEE SPECIAL PROVISIONS FOR MOUNTING DETAILS

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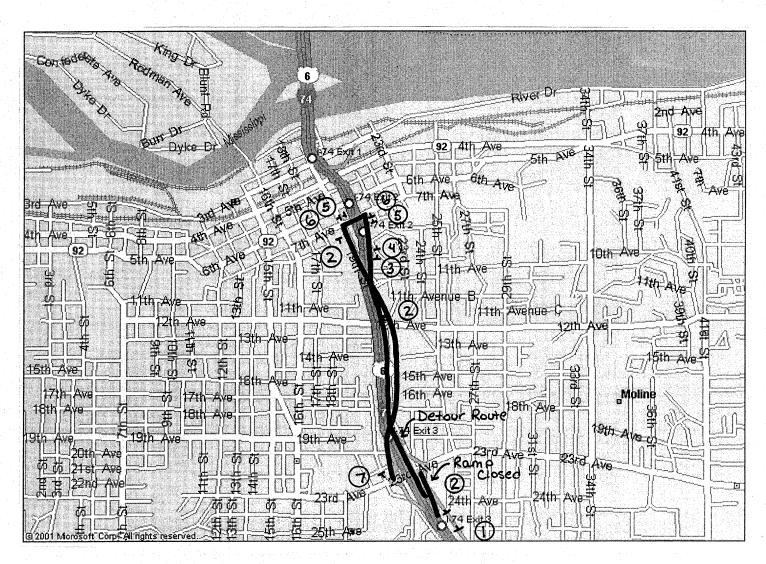
DETOUR ROUTE:

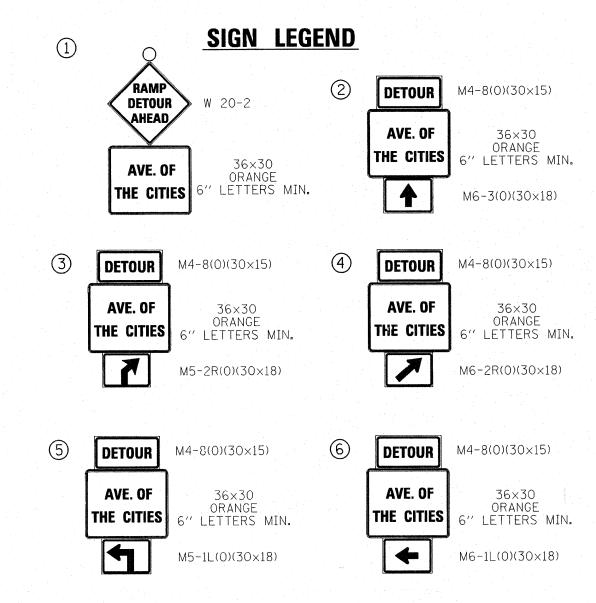
CONTINUE NORTH ON I-74

EXIT AT 7TH AVENUE WEST ON 7TH AVENUE

SOUTH ON 19TH STREET

DETOUR FOR I-74 WB(NB) TO AVENUE OF THE CITIES





NOTE:

OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

- SIGN TRUSS BETWEEN I-280 AND IL 5
 COMPLETELY COVER (WITH TARP) "MOLINE NEXT 4 EXITS"
- SIGN TRUSS AT IL 5 WB GORE
 PLACE "CLOSED" PLATE OVER "AVENUE OF THE CITIES 1 1/2 MILES"
- GROUND MOUNT "AVENUE OF THE CITIES 1/2 MILE" PLACE "CLOSED" PLATE OVER SIGN
- GROUND MOUNT AT AVENUE OF THE CITIES GORE PLACE "CLOSED" PLATE OVER SIGN

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

	DETOUR FO	R I-74	WB TO	AVE. OF	THE CITIES	
SCALE:	SHEET NO.	0F	SHEETS	STA.	TO STA.	

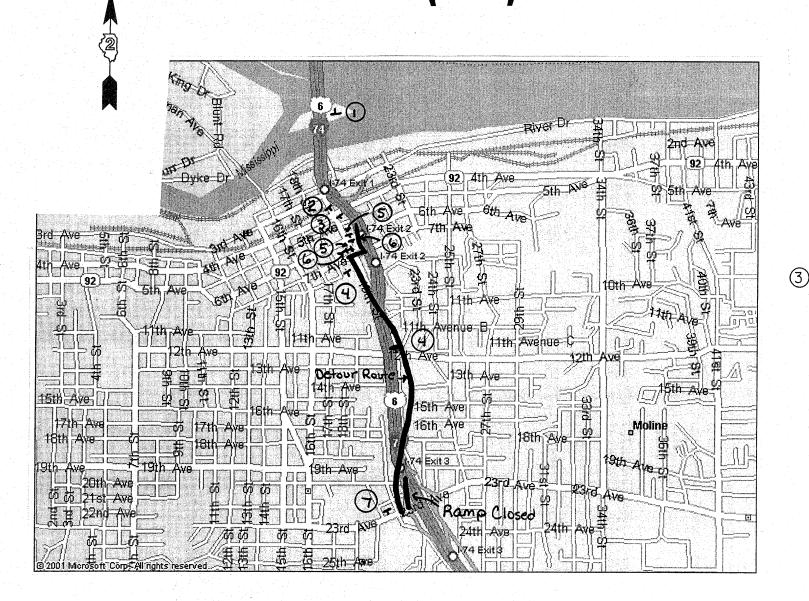
END M4-8A (0) 2418

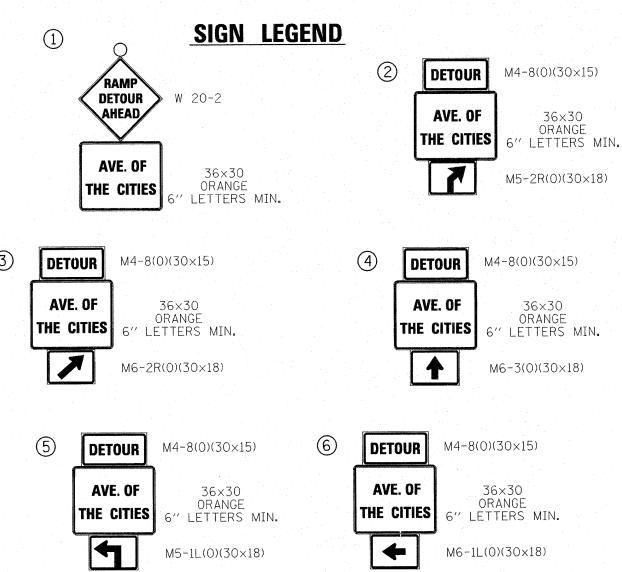
(7)

	ILLINOIS FED. A	D PROJECT		
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505	* **	ROCK ISLAND	246	108
F.A.I RTE.	SECTION	COUNTY	SHEETS	NO.

• 81-(1-2,1,2-2)RS-1&M

DETOUR FOR I-74 EB(SB) TO AVENUE OF THE CITIES





NOTE: OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

-SIGN TRUSS AT 7TH AVENUE GORE

COVER "1 MILE" ON "AVENUE OF THE CITIES 1 MILE"

INSTALL

8'x24" ORANGE IN LOWER LEFT CORNER OF THE SIGN

-CANTILEVER "AVENUE OF THE CITIES 1/2 MILE" PLACE "CLOSED" PLATE OVER SIGN

-SIGN TRUSS BETWEEN 7TH AVENUE AND AVENUE OF THE CITIES
PLACE "CLOSED" PLATE OVER "AVENUE OF THE CITIES EXIT ♣ ONLY"

-SIGN TRUSS AVENUE OF THE CITIES GORE
PLACE "CLOSED" PLATE OVER "AVENUE OF THE CITIES ■ "

DETOUR ROUTE: 7TH AVENUE OFF RAMP WEST ON 7TH AVENUE SOUTH ON 19TH STREET

				TEACE CEOSED FEATE O	VEIV AVEIV
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1	DETOUR FOR I	I-/4 EB IU A	WE. UF	INE CITIES			CONTRACT	NO. 64A97
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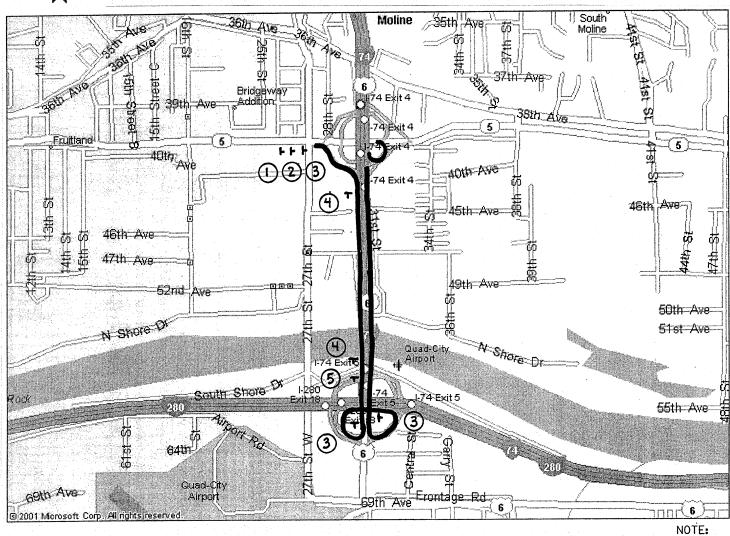
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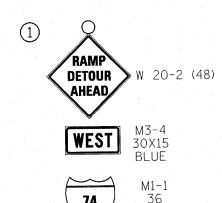
DETOUR

M4-8A (0) 2418

DETOUR FOR IL 5 EB TO I-74 WB (NB)







RED, WHITE,

BLUE





30X15





30X18



WEST 30X15 BLUE





M4-8(0)(30×15) **DETOUR**

WEST 30X15 BLUE





.M6-3 30X18 ORANGE

RAMP DETOUR

SEE SPECIAL SIGN DESIGN

OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

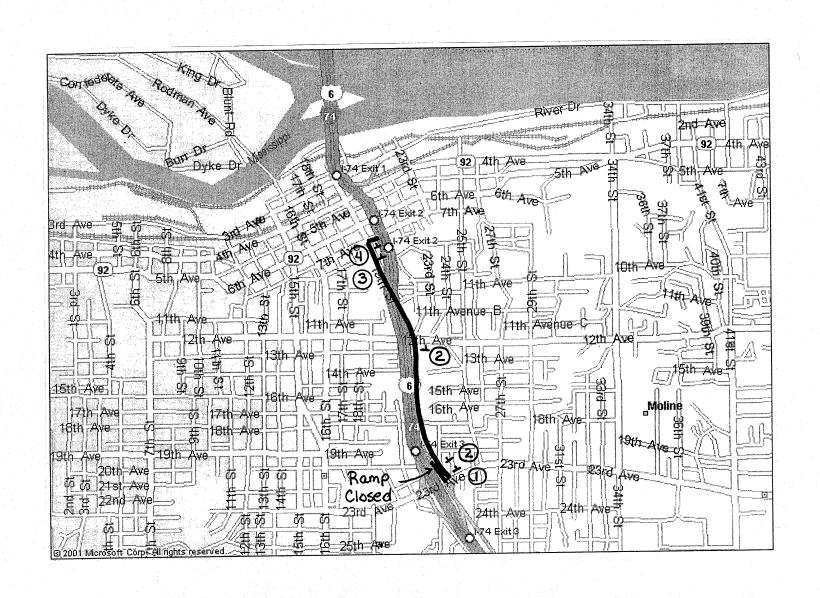
-SIGN TRUSS AT I-74 EB (SB) GORE
PLACE "CLOSED" PLATE OVER "I-74 WEST DAVENPORT NEXT RIGHT"

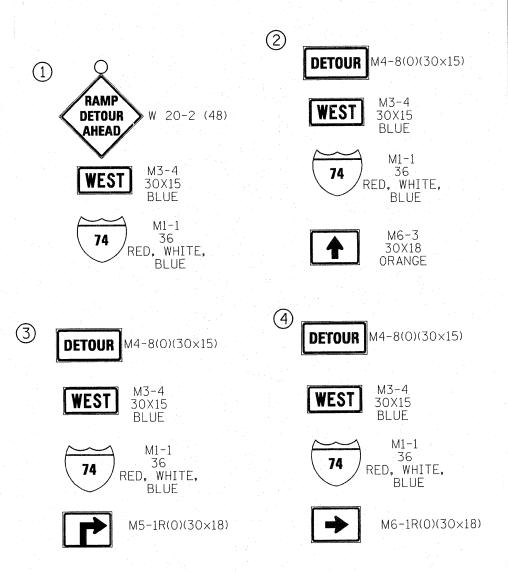
-CANTILEVER AT I-74 WB(NB) GORE PLACE "CLOSED" PLATE OVER SIGN

DETOUR ROUTE: SOUTH ON I-74 TAKE 2 INSIDE RAMPS AT I-280

	TAKE 2 INSIDE RAMI	-3 AT 1-200					F.A.I SECTION COUNTY TOTAL SHEET
F	ILE NAME =	USER NAME = duncanfa	DESIGNED -	REVISED -	OTATE OF HANDIO		RTE. SECTION SINCE 13 NO. 1505 ** ROCK ISLAND 246 110
	Newwork\PWIDOT\DUNCANFA\dms33952\d0	805-sht-ts-dgn	DRAWN	REVISED -	STATE OF ILLINOIS	DETOUR FOR IL 5 EB TO I-74 WB	CONTRACT NO. 64A97
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		PLOT DATE = Fr: Feb 06 10:54:49 2009	DATE -	REVISED -		JOREEN	• 81-(1-2,1,2-2)RS-1&M

DETOUR FOR AVENUE OF THE CITIES TO 1-74 WB(NB)





NOTE: OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

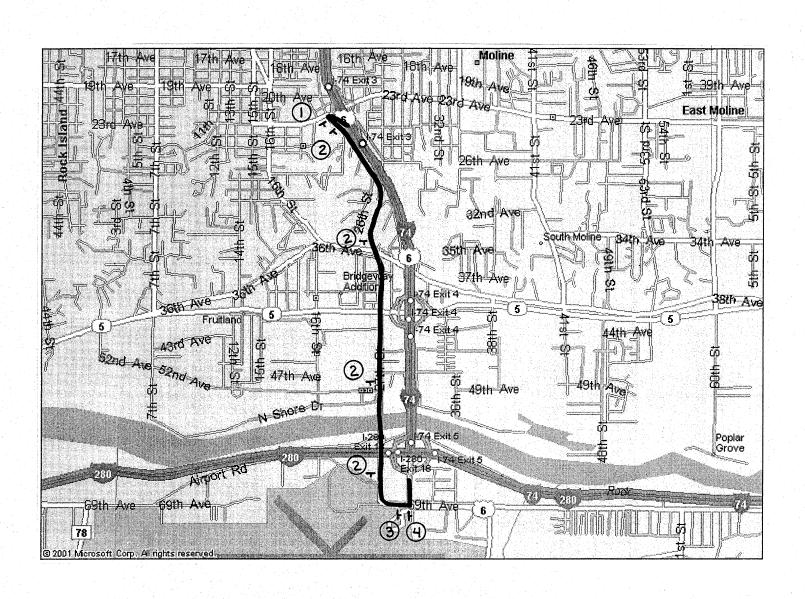
- GROUND MOUNT AT I-74 WB (NB) GORE PLACE "CLOSED" PLATE OVER SIGN

DETOUR ROUTE: NORTH ON 19TH STREET TO 7TH STREET EAST ON 7TH STREET

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DETOUR FOR AVENUE OF THE CITIES TO I-74 EB (SB)

SIGN LEGEND



DETOUR W 20-2 (48) AHEAD.

EAST

EAST 30X15 BLUE

DETOUR M4-8(0)(30×15)

30X15

M1-136 RED, WHITE, BLUE

M1-136 RED, WHITE, BLUE

DETOUR M4-8(0)(30×15) **DETOUR** M4-8(0)(30×15)

30X18 ORANGE

30X15 BLUE

EAST 30X15

36 RED. WHITE. BLUE

M1-136 RED. WHITE, BLUE

 $M5-1L(0)(30\times18)$

 $M6-1L(0)(30\times18)$

OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

- GROUND MOUNT AT I-74 EB (SB) GORE PLACE "CLOSED" PLATE OVER SIGN

DETOUR ROUTE: SOUTH ON 19TH ST. WHICH TURNS INTO 27TH ST. WHICH TURNS INTO 69TH AVE. NORTH ON I-74 STUB

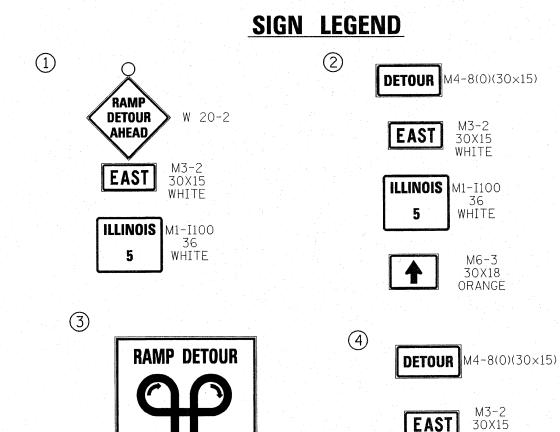
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	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED -	DEPARTMENT OF TRANSPORTATION			C′	ONTRACT NO. 64A97
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DETOUR FOR I-74 EB (SB) TO IL 5 EB



South 38th Ave Ramp Closed 46th Ave -46th Ave Detour Route 50th Ave -N-Shore Of 51st Ave A Shore Dr South Shore Dr 55th Ave Frontage Rd 69th Ave

DETOUR ROUTE: CONTINUE SOUTH ON I-74 TAKE 2 INSIDE LOOPS TO I-280 NORTH ON I-74



NOTE: OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

-SIGN TRUSS AT AVENUE OF THE CITIES GORE COVER THE "B" ON THE IL 5 2 MILES" SIGN

SEE SPECIAL SIGN DESIGN

-SIGN TRUSS "IL 5 1 MILE" SIGN COVER THE "B"

-SIGN TRUSS AT IL 5 WB GORE
PLACE "CLOSED" PLATE OVER "IL 5 EAST NEXT RIGHT" SIGN

-CANTILEER AT IL 5 EB GORE
PLACE "CLOSED" PLATE OVER SIGN

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	PLOT SCALE = 50.0000 '/ IN.	CHECKED -	REVISED -	DEPARTMENT OF TRANSPORTATION		DETOUR FOR 1-74 EB TO IL 3 EB	CONTRACT NO. 64A97
	PLOT DATE = Fr1 Feb 06 10:54:51 2009	DATE -	REVISED -		SCALE:	SHEET NO. OF SHEETS STA. TO STA.	ILLINOIS FED. AID PROJECT

ILLINOIS M1-I100

36

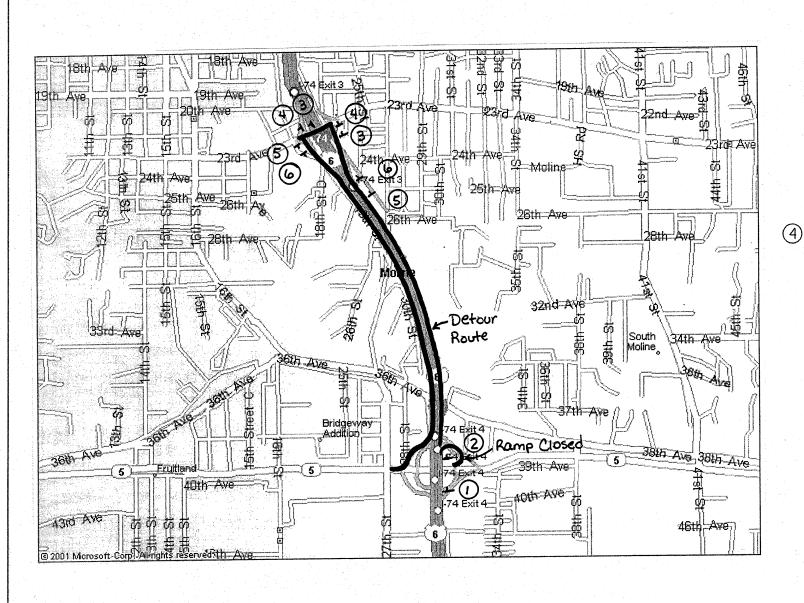
WHITE

30X18

DETOUR FOR I-74 WB (NB) TO IL 5 WB



SIGN LEGEND



RAMP **DETOUR** W 20-2 (48) AHEAD

> WEST 30X15 WHITE

ILLINOIS M1-I100 36 5 WHITE

DETOUR

WEST

 $M4-8(0)(30\times15)$

30X15 WHITE

ILLINOIS M1-I100 5 WHITE

30X18 ORANGE **DETOUR** M4-8(0)(30×15)

30X15 WHITE

ILLINOIS

M1-I100 36 WHITE



M5-1L(0)(30×18)

M4-8(0)(30×15) **DETOUR**

> WEST 30X15 WHITE

ILLINOIS M1-I100 36 WHITE

> 30X18 **ORANGE**

M4-8(0)(30×15) **DETOUR**

> WEST 30X15

ILLINOIS M1-I100 36 WHITE

M5-2R(0)(30×18)

DETOUR M4-8(0)(30×15)

WEST

30X15 WHITE

ILLINOIS M1-I100 36 WHITE

M6-2R 30X18 ORANGE

NOTE: OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

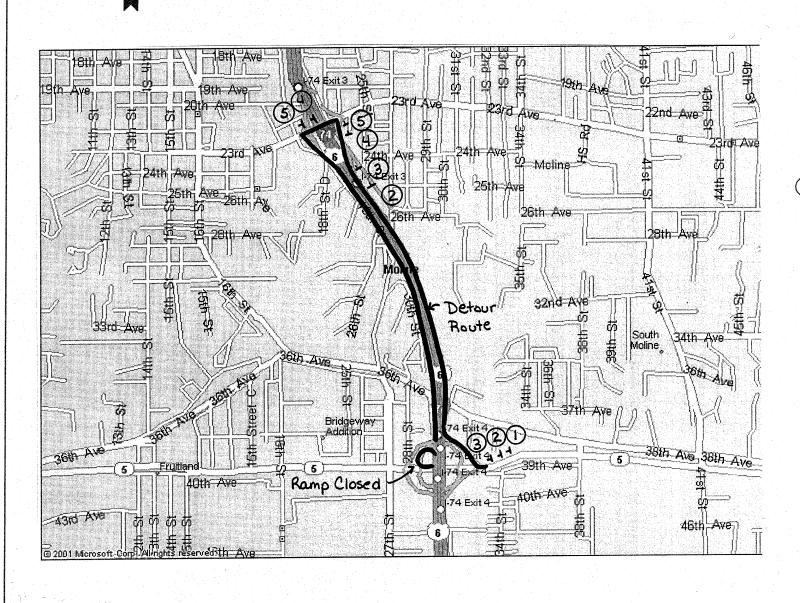
- SIGN TRUSS BETWEEN I-280 AND IL 5 COMPLETELY COVER (WITH TARP) "MOLINE NEXT 4 EXITS"
- GROUND MOUNT FOR COLLEGES COVER THE BOTTOM HALF OF SIGN "AUGUSTANA COLLEGE EXIT 4A"
- SIGN TRUSS AT IL 5 EB PLACE "CLOSED" PLATE OVER "IL 5 WEST NEXT RIGHT" SIGN
- CANTILEER AT IL 5 WB PLACE "CLOSED" PLATE OVER SIGN

DETOUR ROUTE: CONTINUE NORTH ON I-74 EXIT AT AVE. OF THE CITIES WEST ON AVE. OF THE CITIES SOUTH ON I-74

<u> </u>								
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N A 2

DETOUR FOR IL 5 WB TO I-74 EB (SB)





EAST M3-2 30X15 WHITE

M1-1 36 RED, WHITE, BLUE **DETOUR** M4-8(0)(30×15)

EAST M3-4 30X15 BLUE

M1-1 36 RED, WHITE, BLUE

M5-2R(0)(30×18)

DETOUR M4-8(0)(30×15)

EAST M3-2 30X15 BLUE

> M1-1 36 RED, WHITE, BLUE

M6-2R 30X18 ORANGE (4) **DETOUR** M4-8(0)(30×15)

M3-2 30X15 BLUE

M1-1 36 RED, WHITE, BLUE

M5-1L(0)(30×18)

DETO

DETOUR M4-8(0)(30×15)

EAST M3-2 30X15

M1-1 36 RED, WHITE, BLUE

4

M6-1L(0)(30×18)

NOTE

OVERHEAD AND GROUND MOUNT BREAKAWAY STEEL SUPPORT SIGNS TO BE COVERED OR ALTERED.

-CANTILEVER AT I-74 EB GORE PLACE "CLOSED" PLATE OVER SIGN

-SIGN TRUSS AT I-74 WB GORE
"QUAD CITY AIRPORT", COMPLETELY COVER (WITH TARP) SIGN
ON THE "I-74 EAST NEXT RIGHT" PLACE "CLOSED" PLATE OVER SIGN

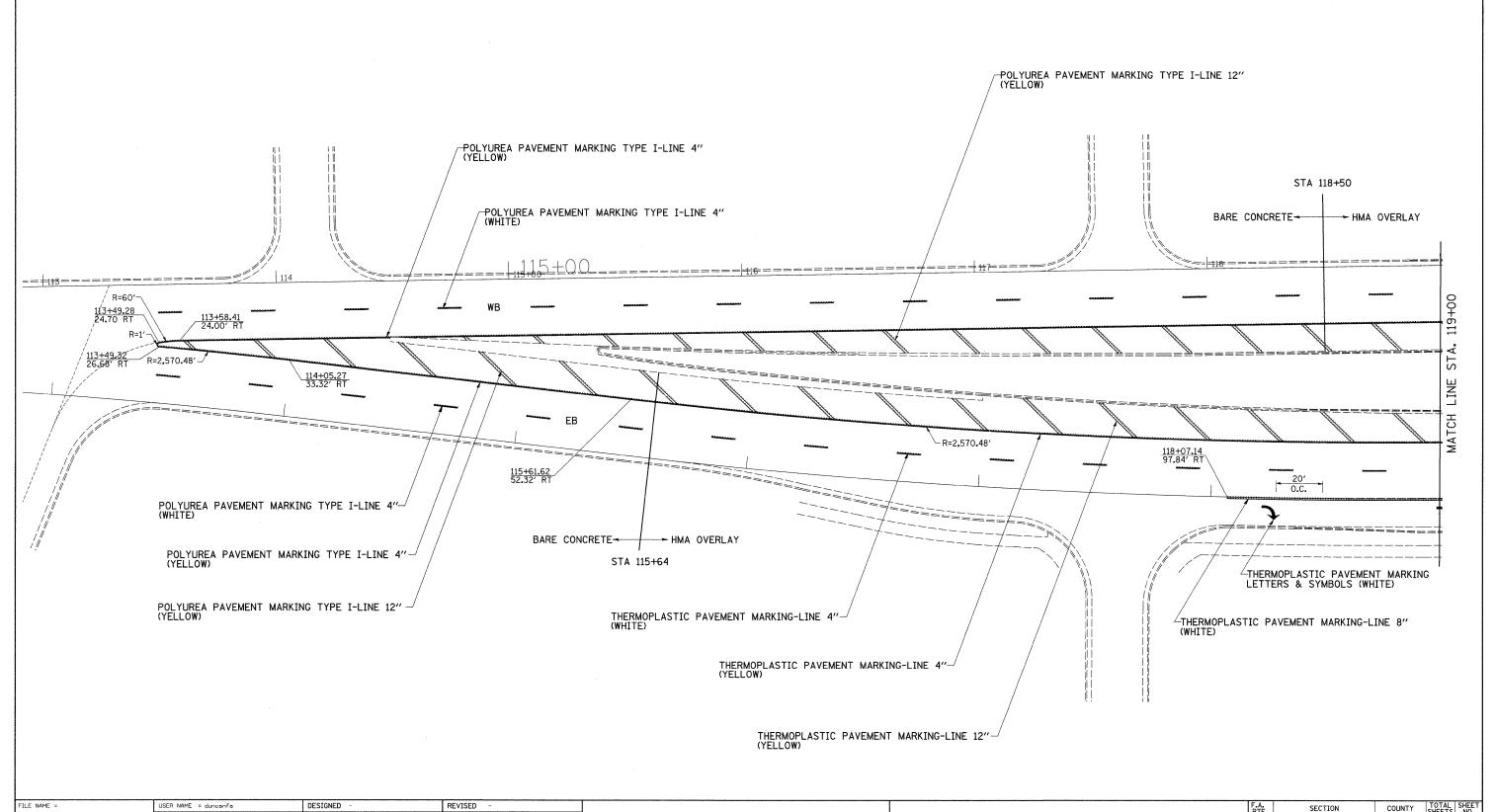
DETOUR ROUTE:
NORTH ON I-74
EXIT AT AVENUE OF THE CITIES
WEST ON AVENUE OF THE CITIES
SOUTH OF I-74

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					SILET NO. OF SILETO STA	TO STM		ILLINOIS FED. AID PROJECT

PAVEMENT MARKING DETAILS



CONTRACT NO.



STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

PAVEMENT MARKING DETAILS

SHEET NO. OF SHEETS STA.

DRAWN

DATE

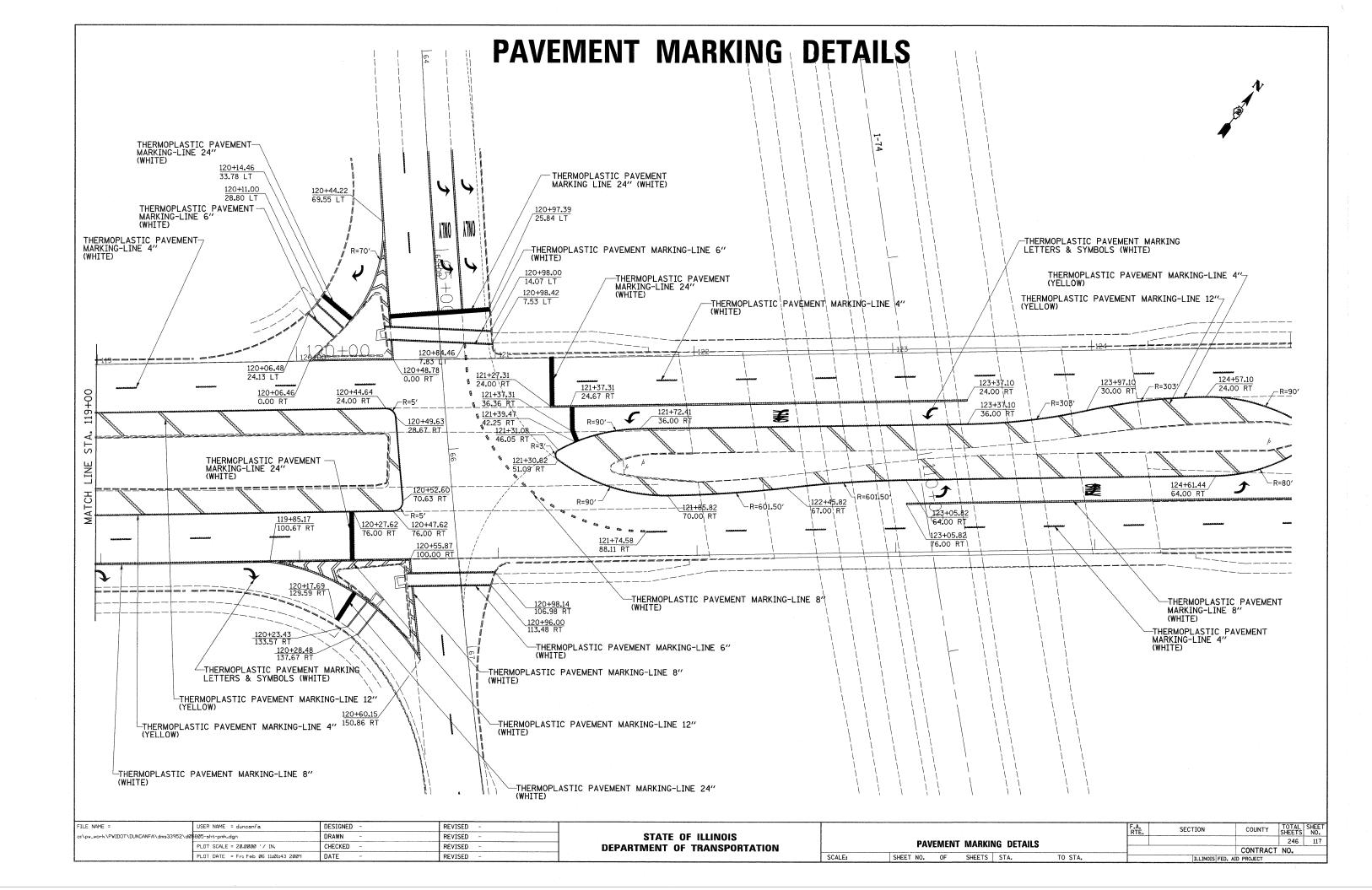
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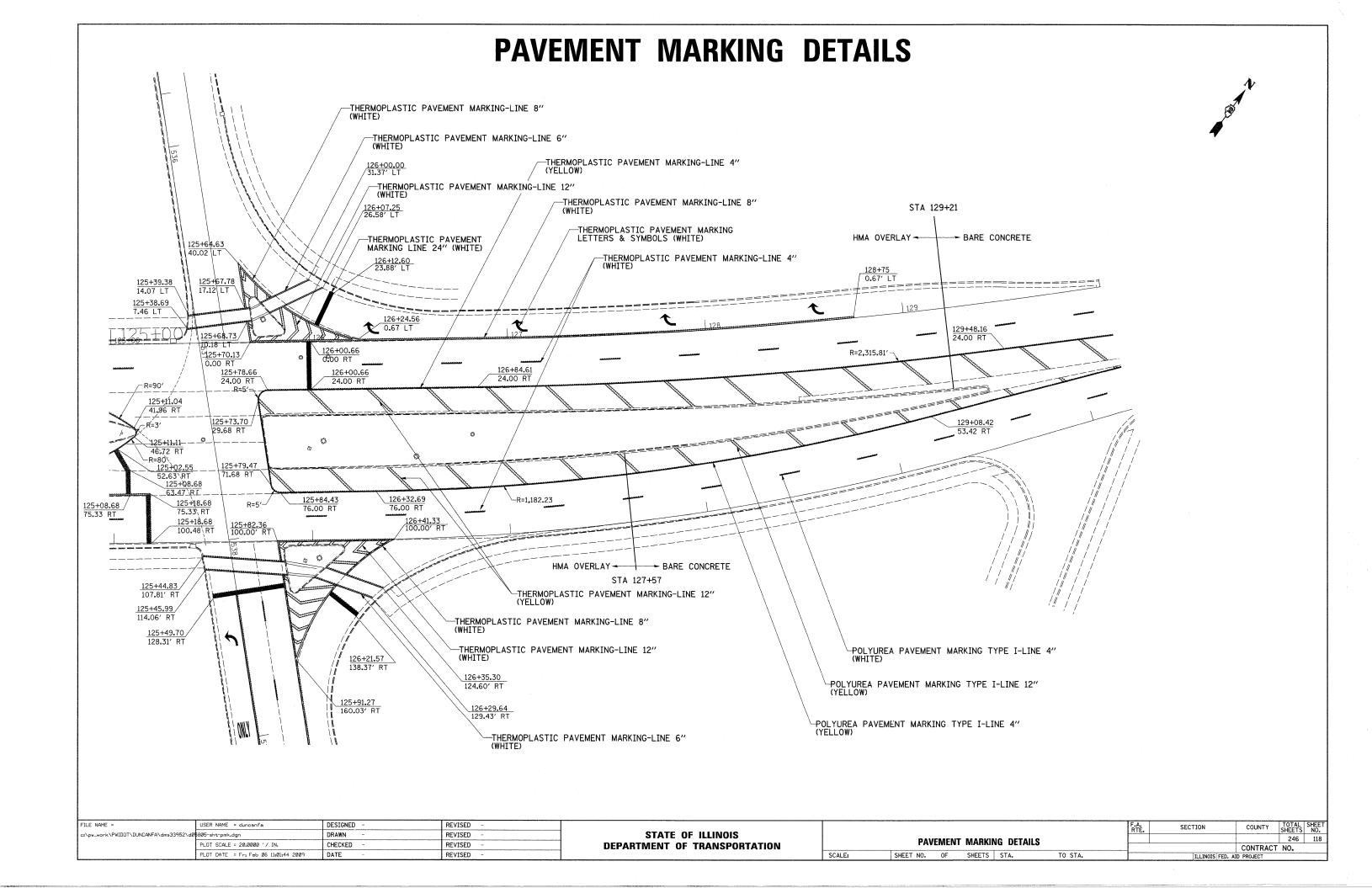
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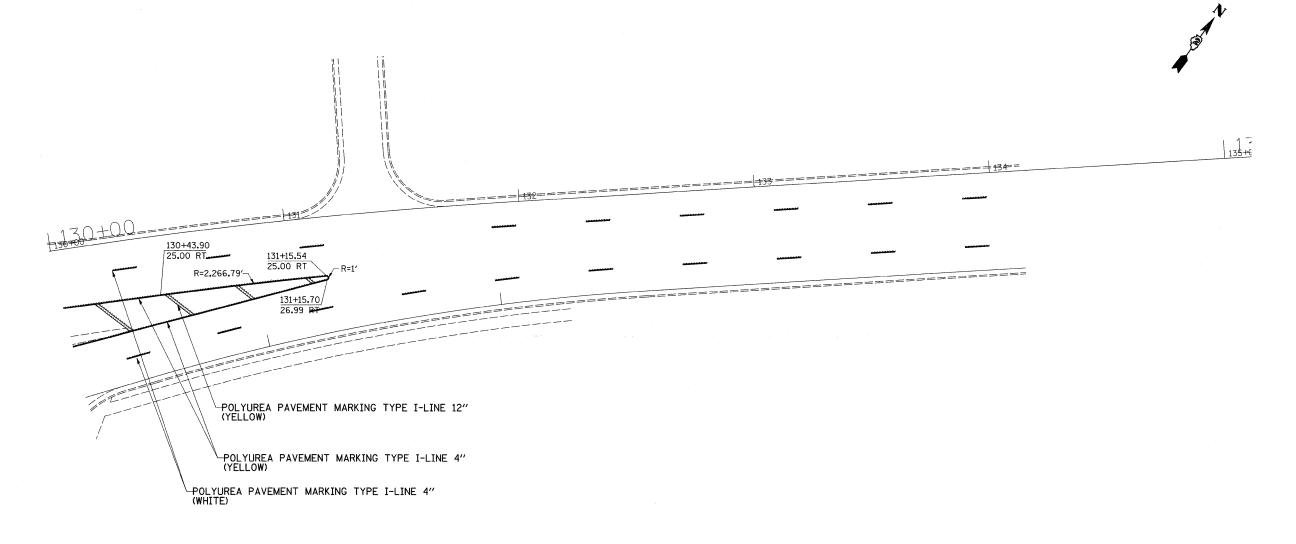
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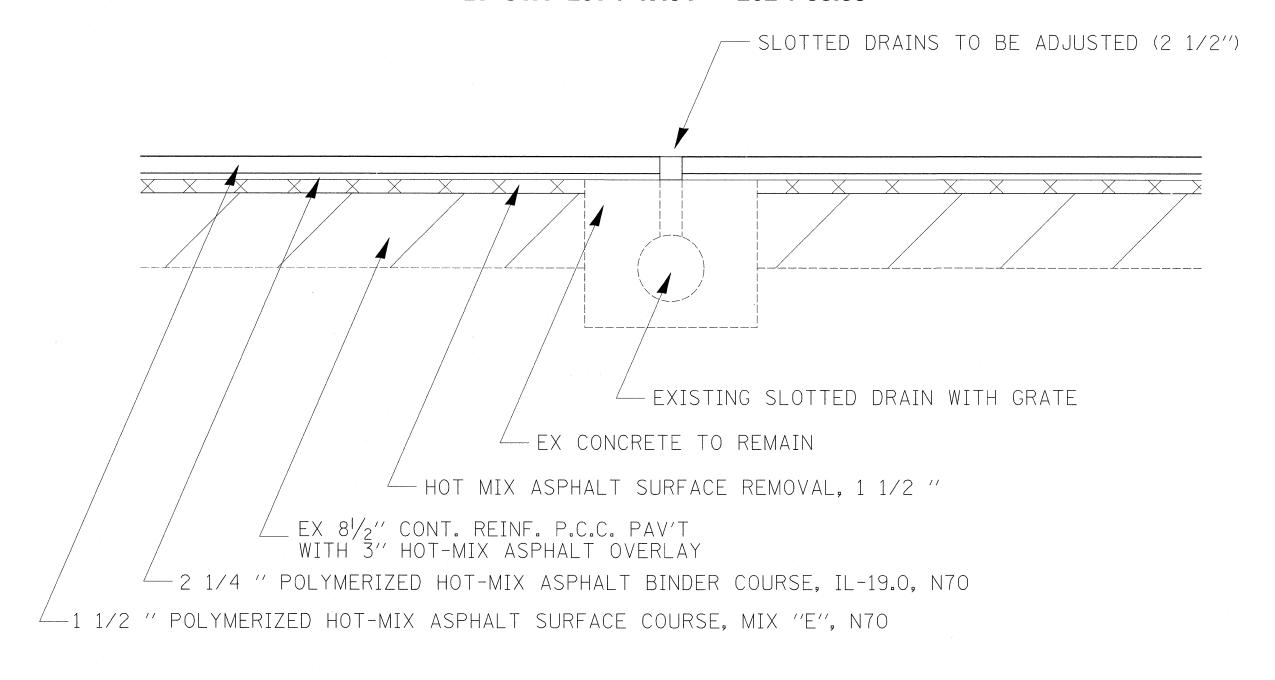
PAVEMENT MARKING DETAILS



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	PLOT DATE = Fr1 Feb 06 11:01:46 2009	DATE -	REVISED -		SCALE:	SHEET NO.	0F	SHEETS	STA.	TO STA.		ILLINOIS FED. AID PRO		

SLOTTED DRAINS TO BE ADJUSTED

LT STA 201 + 47.64 - 202 + 58.35

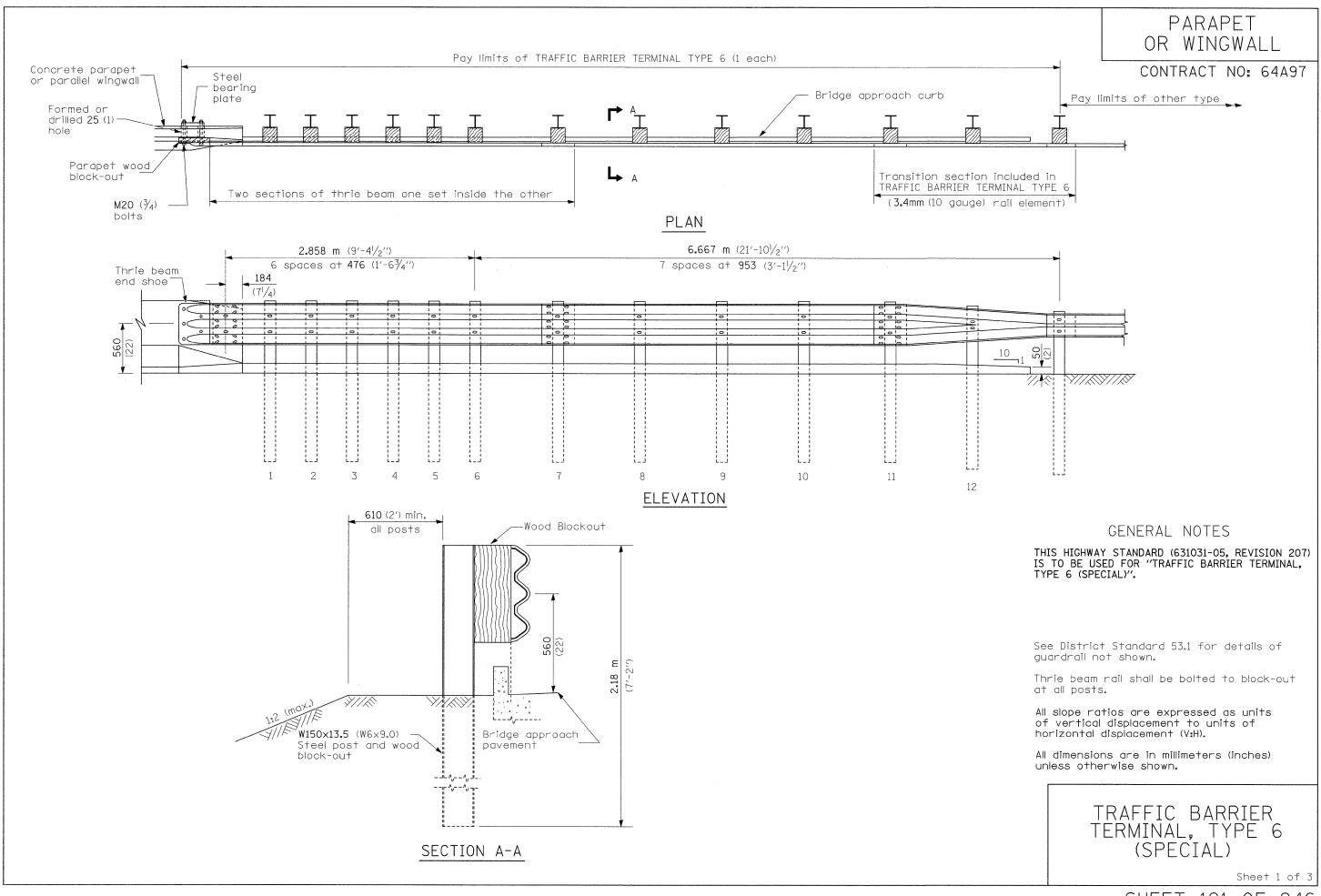


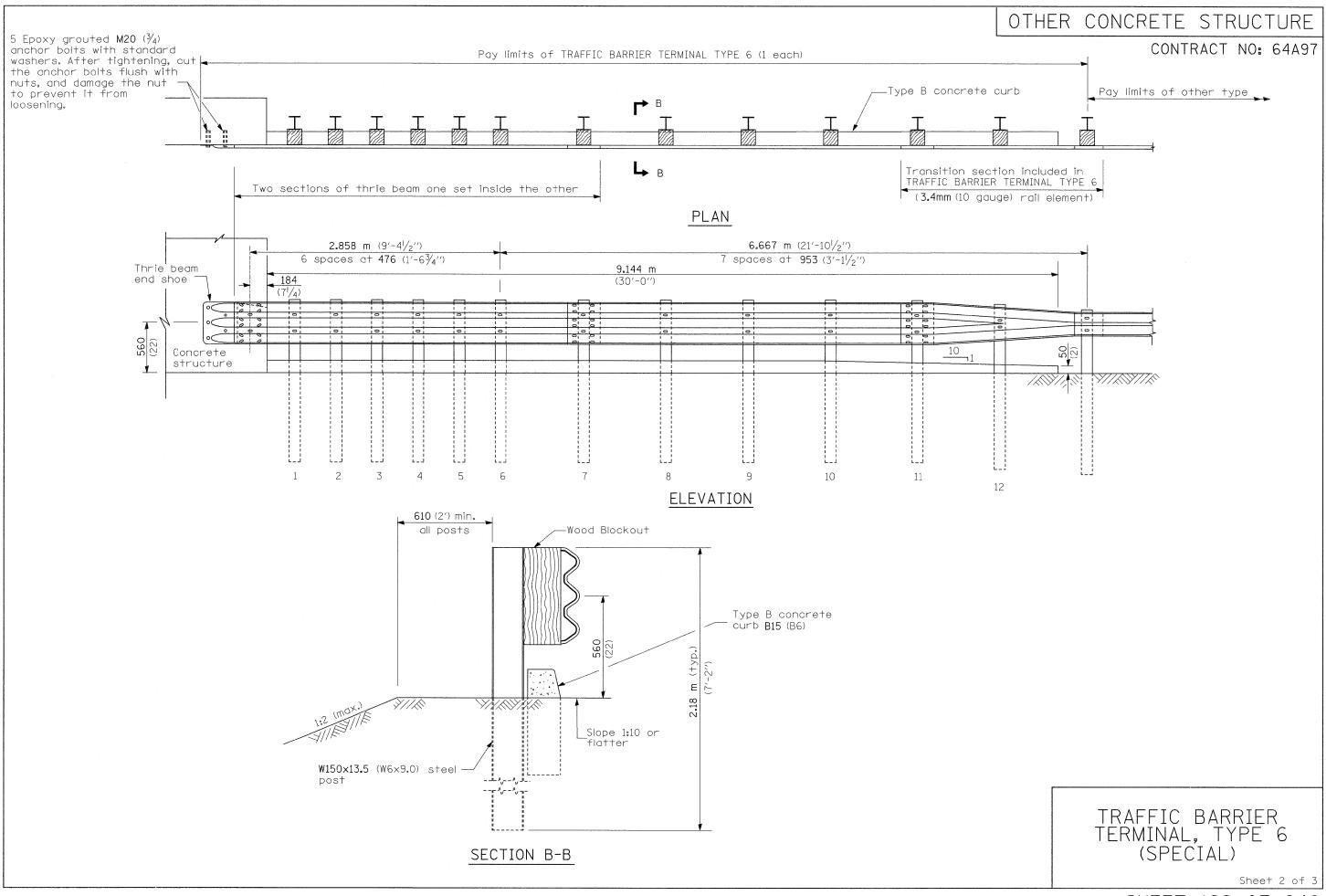
NOTES:

A 2 1/2" galvanized grate extension shall be welded to the existing slotted drain grate according to the manufacturer's requirements. The grate shall conform to District Standard 61.2. If the existing slotted drain grate is damaged the contractor shall replace it at their expence.

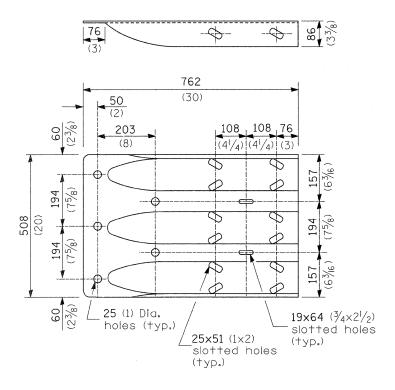
This work shall be measured lengthwise along the center of the drain. This work shall be paid for at the contract unit price per Foot for SLOTTED DRAINS TO BE ADJUSTED which shall include the grate extension.

FILE NAME =	USER NAME = dunconfa	DESIGNED -	REVISED -								F.A.I.	SECTION	COUNTY	TOTAL SHEET
c:\pw_work\PWIDOT\DUNCANFA\dms33952\dØ	8Ø5-sht-ts.dgn	DRAWN -	REVISED -	STATE OF ILLINOIS	SLOTTED DRAINS TO BE ADJUSTED				74	81-(1-2, 1, 2-2)RS-1&M	ROCK ISLAND	D 246 120		
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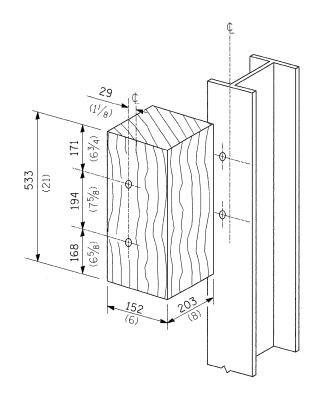




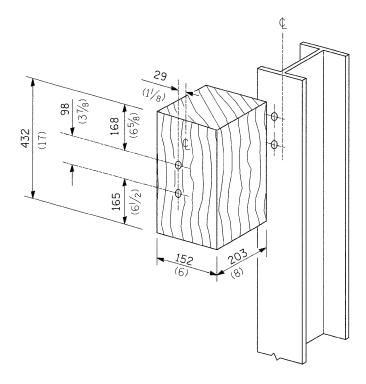
CONTRACT NO: 64A97



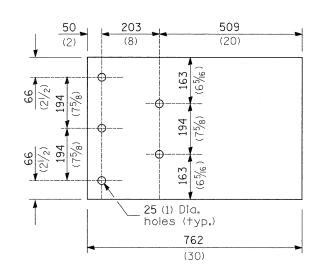
THRIE BEAM END SHOE DETAIL

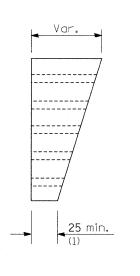


POSTS 1-11 WOOD BLOCKOUT DETAIL

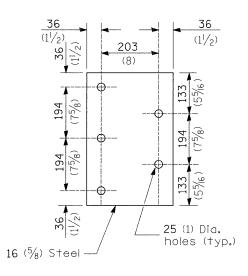


POST 12 WOOD BLOCKOUT DETAIL









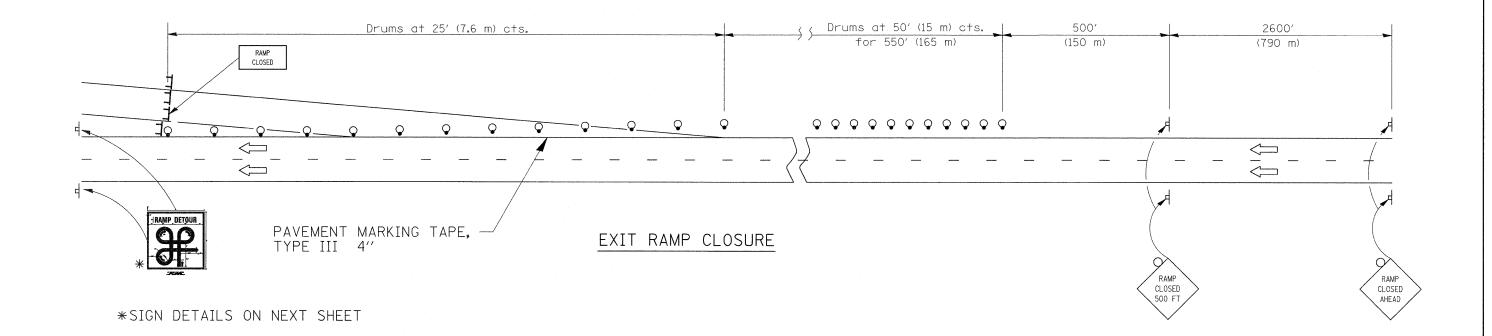
PARAPET STEEL BEARING PLATE DETAIL

(5 each individual 125x125x16 (5x5x $\frac{5}{8}$) steel plates with centered 25 (1) holes may be substituted for the plate shown.)

TRAFFIC BARRIER TERMINAL, TYPE 6 (SPECIAL)

Sheet 3 of 3

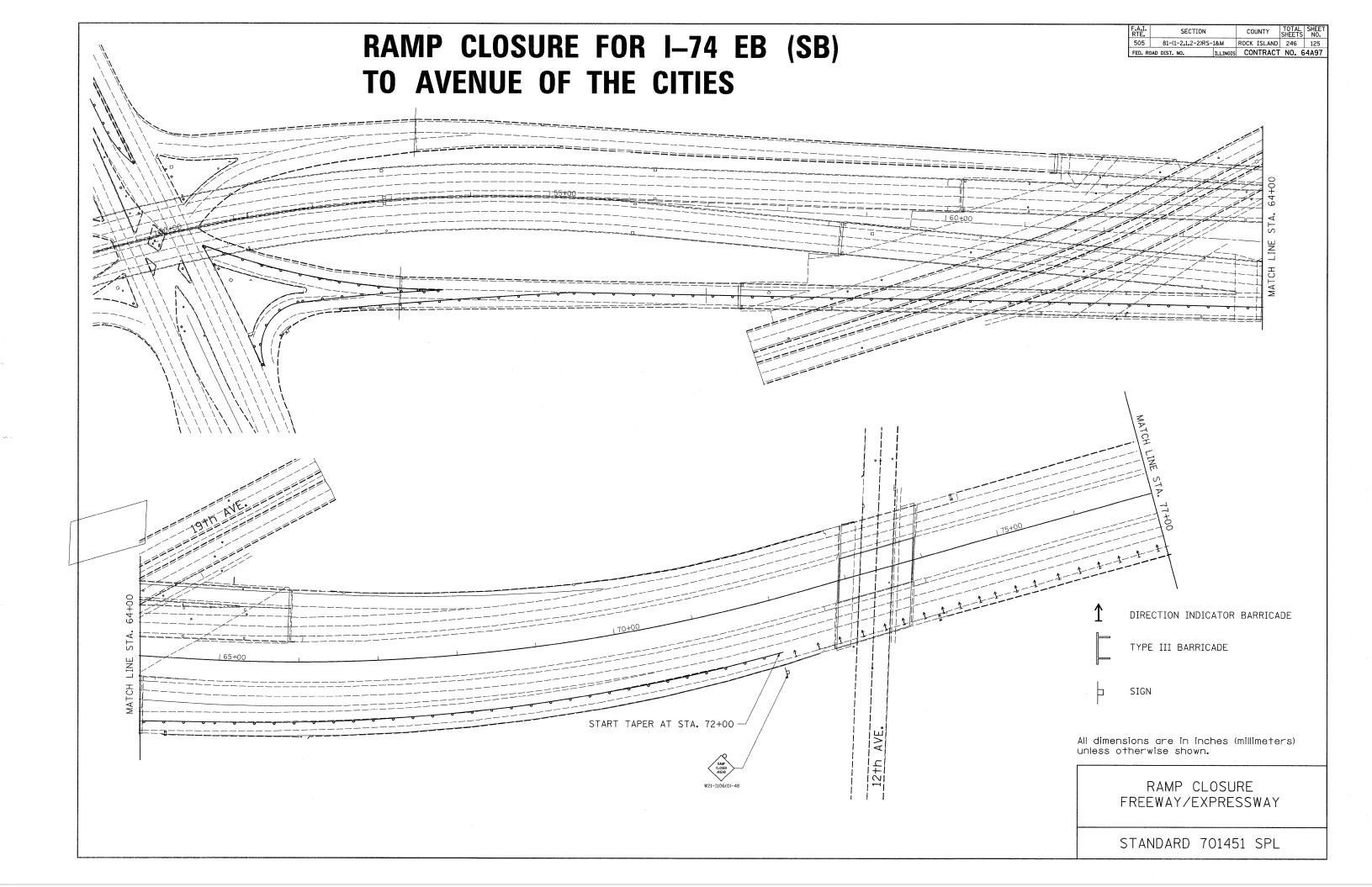
-	F.A.I. RTE.	SECTION		со	UNTY	TOTAL SHEETS	SHEET NO.
	505	81-(1-2,1,2-2)RS-	ROCK	ISLAND	246	124	
	FED. RO	DAD DIST. NO.	ILLINOIS	CO	NTRACT	NO. 6	4A97



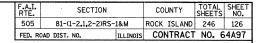
All dimensions are in inches (millimeters) unless otherwise shown.

RAMP CLOSURE FREEWAY/EXPRESSWAY

STANDARD 701451 SPL

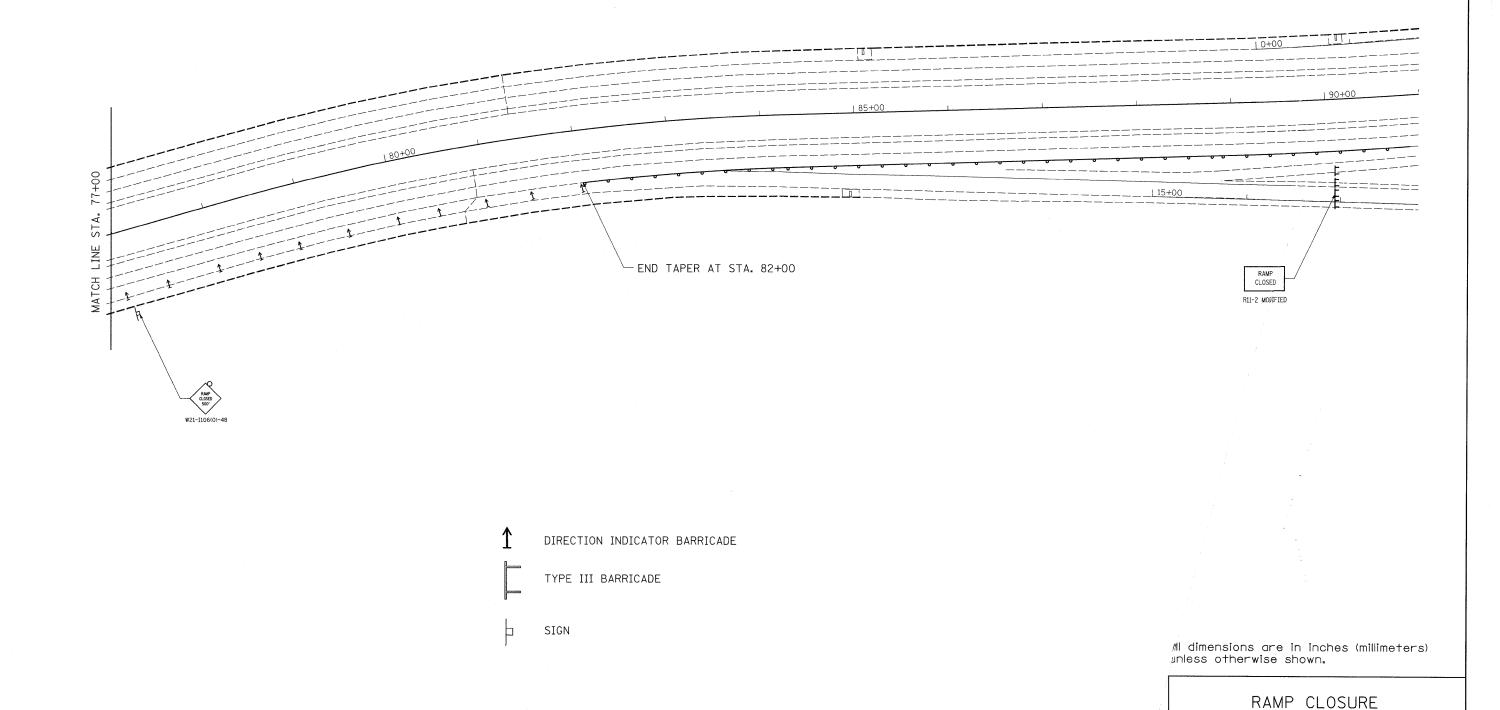


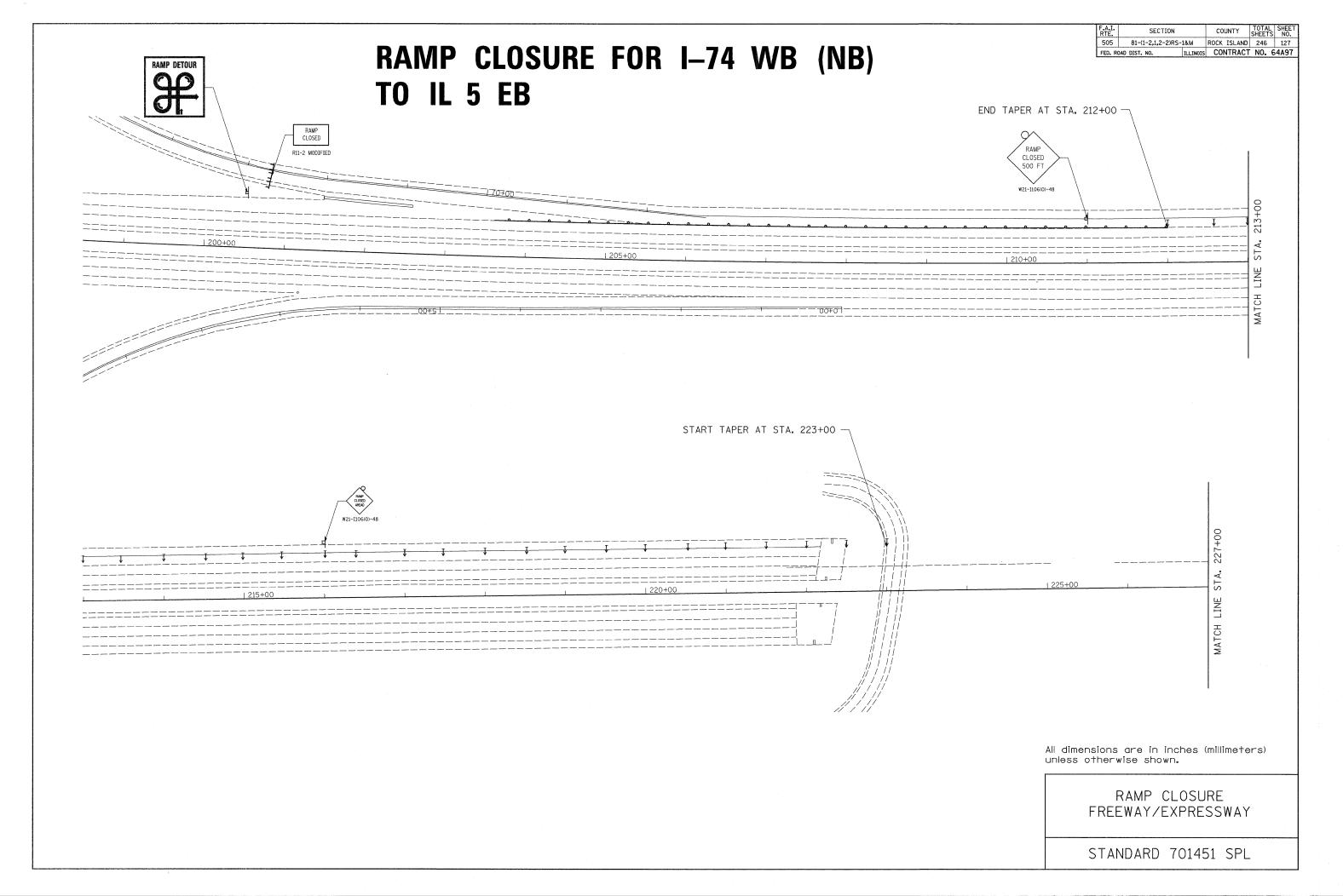
RAMP CLOSURE FOR I-74 EB (SB) TO AVENUE OF THE CITIES



FREEWAY/EXPRESSWAY

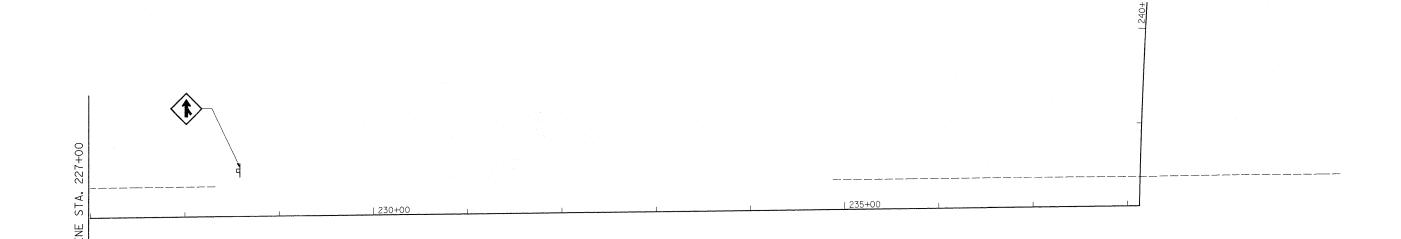
STANDARD 701451 SPL





RAMP CLOSURE FOR I-74 WB (NB) TO IL 5 EB





All dimensions are in inches (millimeters) unless otherwise shown.

RAMP CLOSURE FREEWAY/EXPRESSWAY

STANDARD 701451 SPL

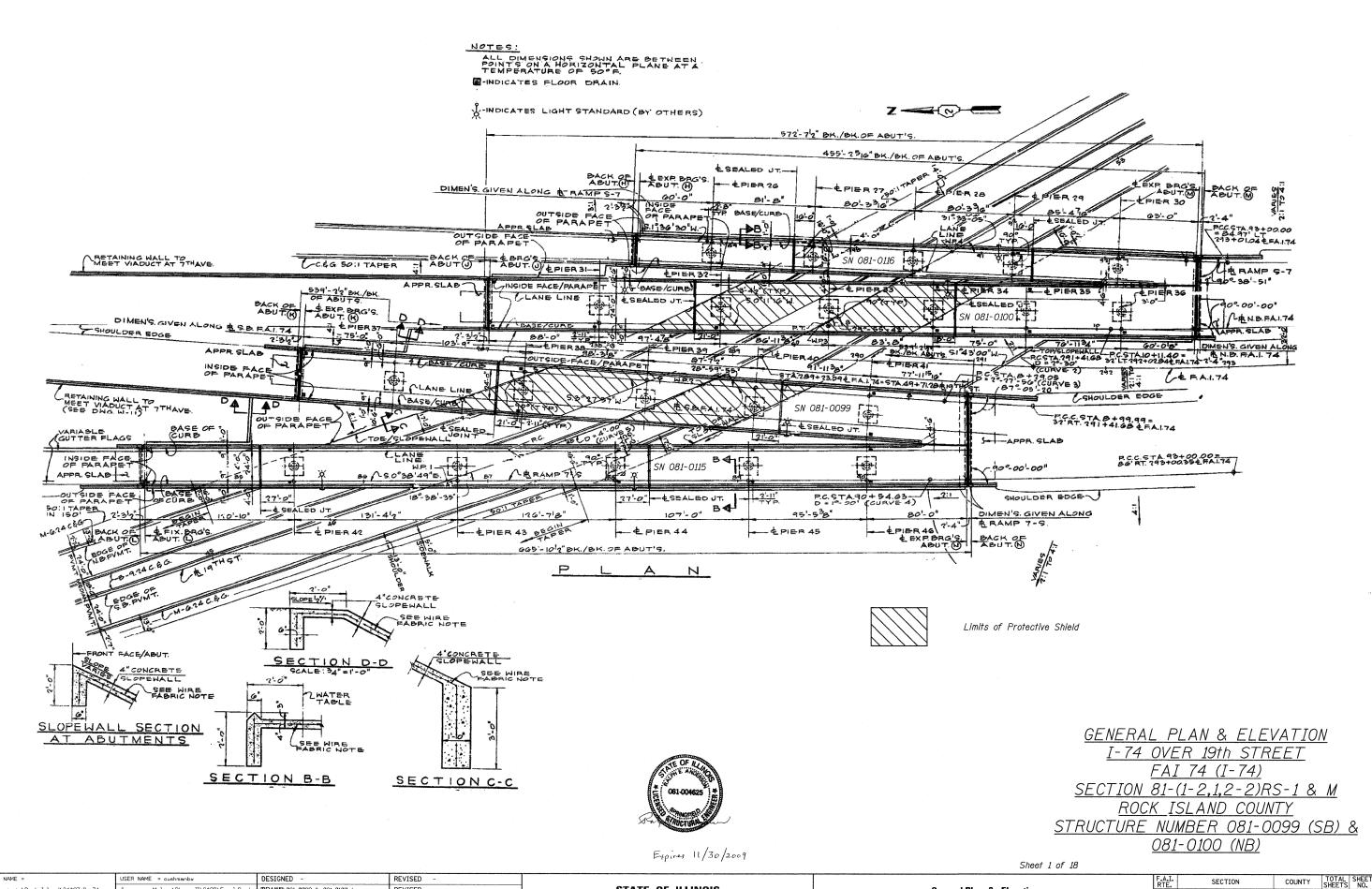
SUMMARY OF STRUCTURAL PLANS

CODE NUMBER	ITEM	UNIT	SN 081-0099	SN 081-0100	SN 081-0101	SN 081-0102	SN 081-0103	SN 081-0104	SN 081-0105	SN 081-0108	SN 081-0109	SN 081-0110	TOTALS
28100101	STONE RIPRAP, CLASS A1	SQ YD	***							29.9			30
40603565	POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, MIX "E", N70	TON	252.00	268.00	102.00	74.00	122.00	96.00	348.00		862.00	66.60	2,191
50102400	CONCRETE REMOVAL	CU YD	37,5	37 . 5	16.2	12.4	29.9	21.8	27.3	17.6	16.6	15.8	232.6
50104650	SLOPE WALL REMOVAL	SQ YD							554				554
50157300	PROTECTIVE SHIELD	SQ YD	1,011	820	354	252	305	232	1,179		354	330	4,837
50300255	CONCRETE SUPERSTRUCTURE	CU YD	37.5	37.5	16.2	12.4	29.9	21.8	27.3	17.6	16.6	15.8	232.6
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	4530	4580	1,850	1,390	2,720	2,100	3,770	2,170	2,230	2,080	27420
50800515	BAR SPLICERS	EACH	70	70	22	22	22	22	44	22	22	22	338
51100100	SLOPE WALL 4 INCH	SQ YD							554				554
52000110	PREFORMED JOINT STRIP SEAL	FOOT	170	170	117	86	184	139	240	112	114	107	1,439
58100200	WATERPROOFING MEMBRANE SYSTEM	SQ YD	2,245	2,389	570.7	414.3	1,170	924.0	2,340.0		799.2	746.2	11,598
59300100	CONTROLLED LOW-STRENGTH MATERIAL	CU YD			*******			No. at an	****	1.0			1.0
60100945	PIPE DRAINS 12"	F00T		NO 600							15		15
63300405	REMOVAL AND REINSTALLATION OF EXISTING TRAFFIC BARRIER TERMINAL, TYPE1	EACH	3	3									6
X0301424	SILICONE JOINT SEALER	FOOT							234	14			248
X0321316	ACCESS DOOR	EACH								2			2
X0324865	DIAMOND GRINDING (BRIDGE SECTION)	SQ YD	2,245	2,389	5 7o.T	414.3	1170	924	2340		799.2	746.2	11598
X0325303	STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES)	SQ FT				***			162		18	27	207
Z0006200	BRIDGE DECK SCARIFICATION	SQ YD	2,323	2,466	564.5	409.8	1,160	917	2,330		793.1	740.5	11,704
Z0016001	DECK SLAB REPAIR (FULL DEPTH, TYPE I)	SQ YD	25	25	32	22.7	162.5	25			47.5	70	410
Z0016002	DECK SLAB REPAIR (FULL DEPTH, TYPE II)	SQ YD	400	500	32	22.7	162.5	25	3		47.5	70	1,263
Z0016200	DECK SLAB REPAIR (PARTIAL)	SQ YD	450	450	64	50.5	120	120	27		10	20	1,312
Z0032470	JOINT SEALER	FOOT								62			62

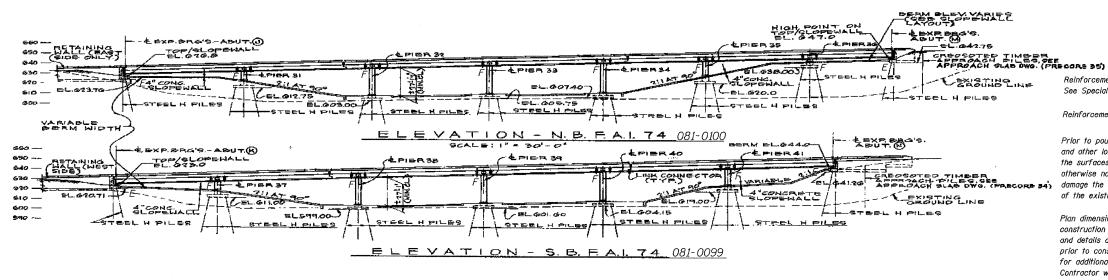
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	PLOT DATE = Fr1 Feb 06 10:36:10 2009	DATE -	REVISED -

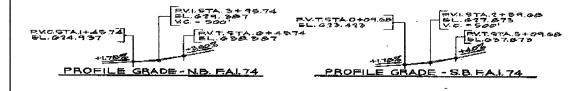
STATE OF ILLINOIS	
DEPARTMENT OF TRANSPORTATION	

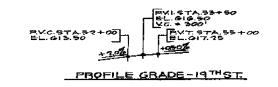
STATE OF ILLINOIS

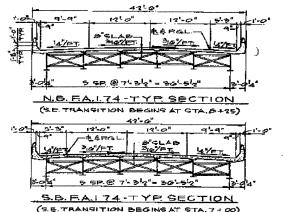


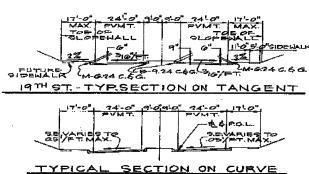
FILE NAME = STATE OF ILLINOIS rfacing in Moline\Phase II\CADD\Final Brid DRAWN\081-0099 & 081-0100.dgr REVISED **General Plan & Elevation** 81-(1-2,1,2-2)RS-1 & M Rock Island 246 130 PLOT SCALE = 50.0000 '/ IN. CHECKED REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 64A97 PLOT DATE = Fri Feb Ø6 13:16:25 2009 DATE SCALE: SHEET NO. OF SHEETS STA. TO STA.











General Notes

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50° F.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL	081-0099	081-0100
CONCRETE SUPERSTRUCTURE	CU YD	75	37.5	37.5
CONCRETE REMOVAL	CU YD	75	37.5	37.5
REINFORCEMENT BARS, EPOXY COATED	POUND	9110	4530	4580
PREFORMED JOINT STRIP SEAL	FOOT	340	170	170
BAR SPLICERS	EACH	140	70	70
REMOVAL AND REINSTALLATION OF EXISTING TRAFFIC BARRIER TERMINAL, TYPE 1	EACH	6	3	3
WATERPROOFING MEMBRANE SYSTEM	SQ YD	4634	2245	2389
BRIDGE DECK SCARIFICATION	SQ YD	4789	2323	2466
DECK SLAB REPAIR (FULL DEPTH, TYPE I)	SQ YD	50	25	25
DECK SLAB REPAIR (FULL DEPTH, TYPE II)	SQ YD	900	400	500
DECK SLAB REPAIR (PARTIAL)	SQ YD	900	450	450
POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, MIX "E", N70	TON	520	252	268
PROTECTIVE SHIELD	SQ YD	1831	1011	820
DIAMOND GRINDING (BRIDGE SECTION)	SQ YD	4634	2245	2389

GENERAL PLAN & ELEVATION

I-74 OVER 19th STREET

FAI 74 (I-74)

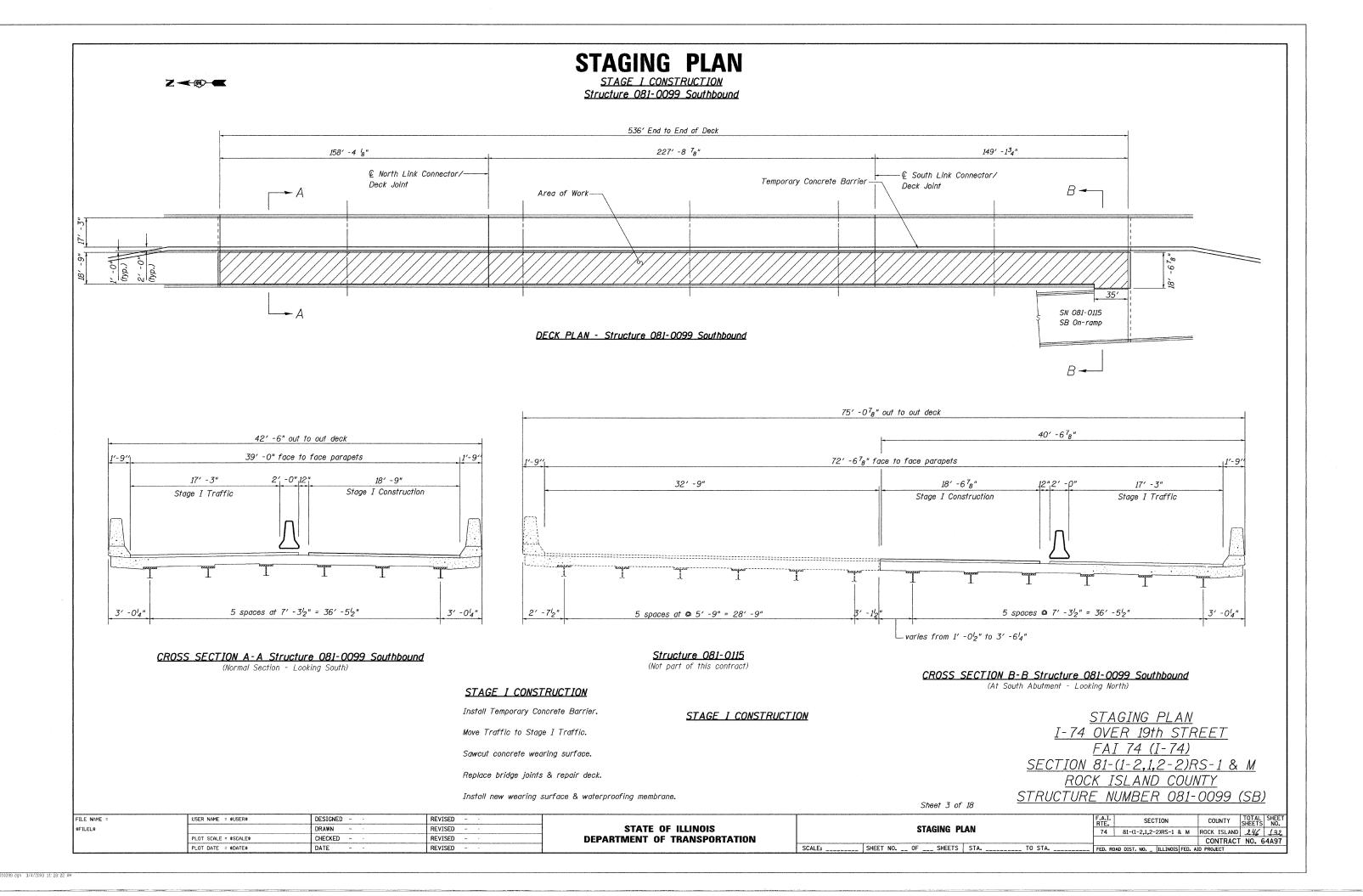
SECTION 81-(1-2,1,2-2)RS-1 & M

ROCK ISLAND COUNTY

STRUCTURE NUMBER 081-0099 (SB) & 081-0100 (NB)

Sheet 2 of 18

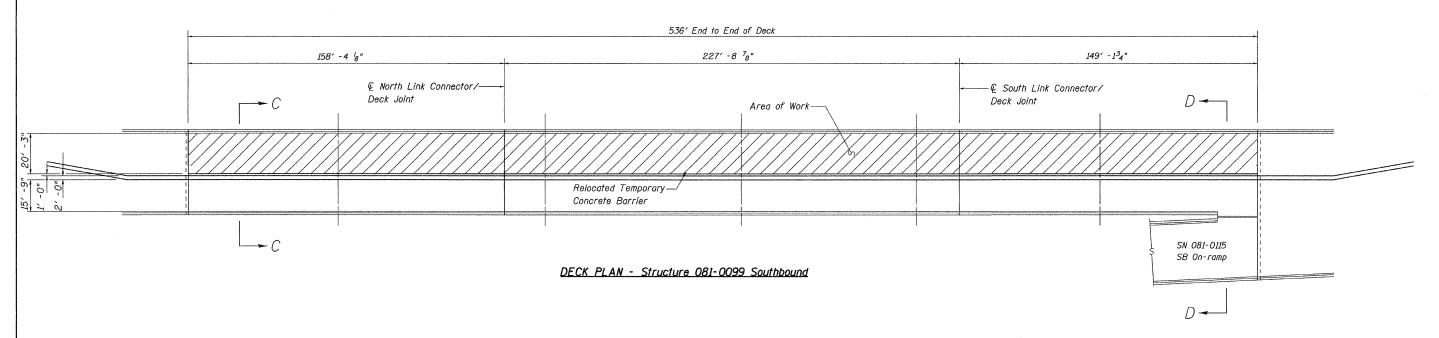
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FILE NAME =	USER NAME = \$USER\$	DESIGNED -	REVISED - ·			F.A.I. SECTION	COUNTY TOTAL SHEET
\$FILEL\$		DRAWN ~ ·	REVISED - ·	STATE OF ILLINOIS	GENERAL PLAN & ELEVATION	74 81-(1-2.1.2-2)RS-1 & M	ROCK ISLAND 246 131
	PLOT SCALE = \$SCALE\$	CHECKED - ·	REVISED -	DEPARTMENT OF TRANSPORTATION		11 1 2 12 LANG 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	CONTRACT NO. 64A97
	PLOT DATE = \$DATE\$	DATE	REVISED - ·		SCALE: SHEET NO OF SHEETS STA TO STA	FED. ROAD DIST. NO ILLINOIS FED.	

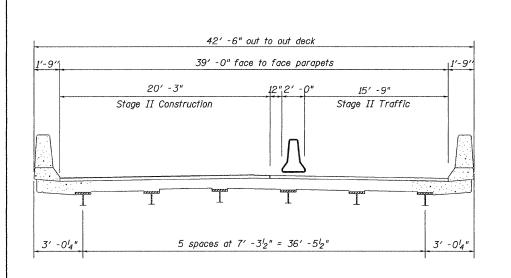


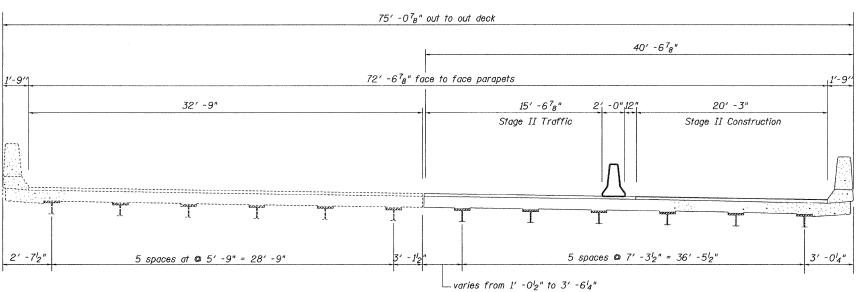
Z - (3) - (4)

STAGING PLAN

STAGE II CONSTRUCTION
Structure 081-0099 Southbound







CROSS SECTION C-C Structure 081-0099 Southbound

(Normal Section - Looking South)

PLOT DATE = \$DATE\$

STAGE II CONSTRUCTION

Relocate Temporary Concrete Barrier.

Move Traffic to Stage II Traffic.

Remove concrete wearing surface.

Replace bridge joints & repair deck.

Install new wearing surface & waterproofing membrane.

Remove temporary concrete barrier.

CROSS SECTION D-D Structure 081-0099 Southbound

(At South Abutment - Looking North)

STAGING PLAN

I-74 OVER 19th STREET

FAI 74 (I-74)

SECTION 81-(1-2,1,2-2)RS-1 & M

ROCK ISLAND COUNTY

STRUCTURE NUMBER 081-0099 (SB)

Sheet 4 of 18

 USER NAME = #USER#
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 PLOT SCALE = #SCALE#
 CHECKED - ...
 REVISED - ...

REVISED

DATE

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

Structure 081-0115

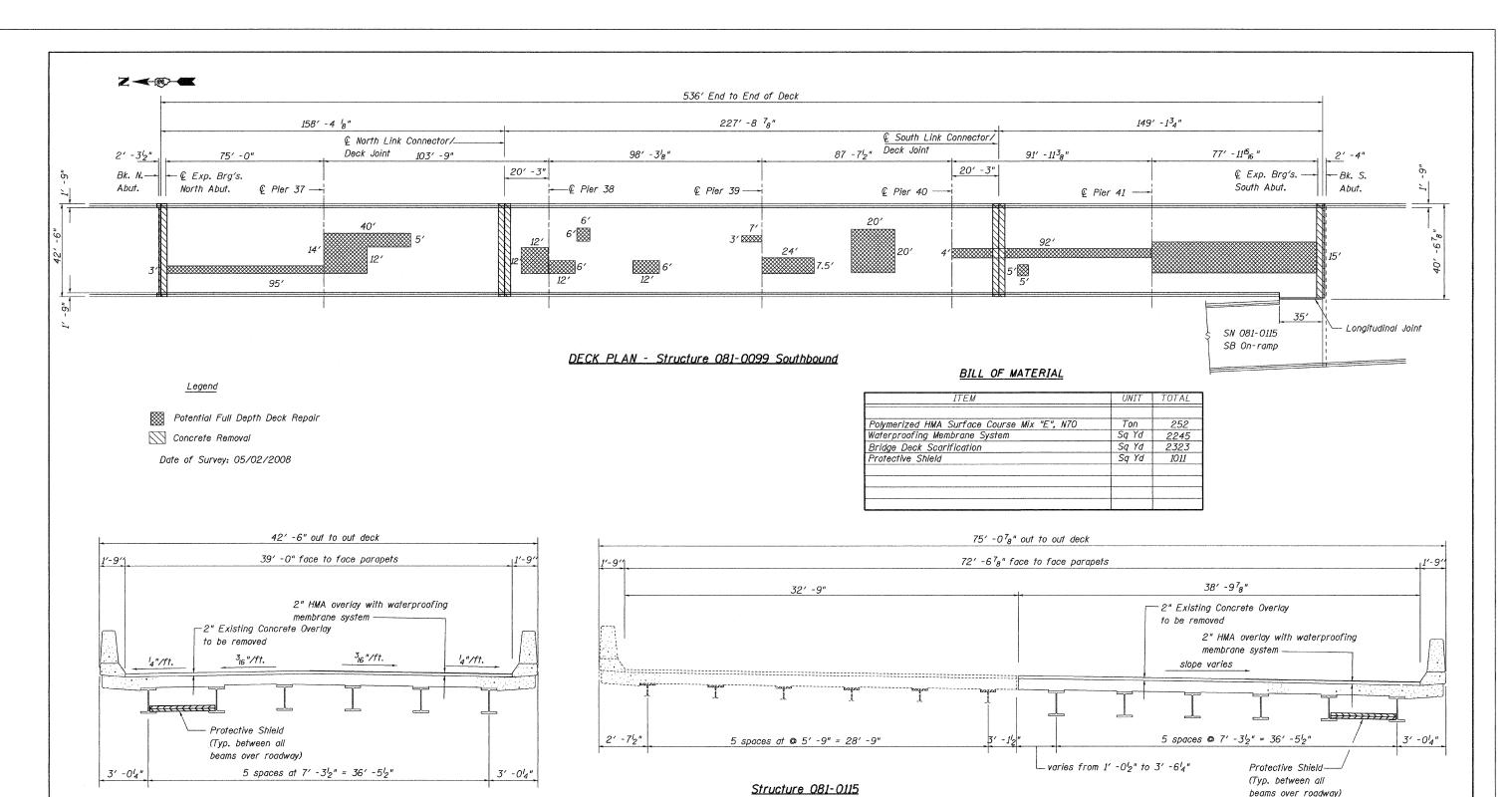
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CROSS SECTION - Structure 081-0099 Southbound (Normal Section - Looking South)

CHECKED

DATE

REVISED

REVISED

PLOT SCALE = \$SCALE\$

PLOT DATE = \$DATE\$

(Not part of this contract)

SHEET NO. __ OF ___ SHEETS STA.

CROSS SECTION - Structure 081-0099 Southbound (At South Abutment - Looking North)

TO STA.

DECK PLAN & CROSS-SECTION I-74 OVER 19th STREET FAI 74 (I-74) SECTION 81-(1-2,1,2-2)RS-1 & M

STRUCTURE NUMBER 081-0099 (SB)

FED. ROAD DIST. NO. _ | ILLINOIS| FED. AID PROJECT

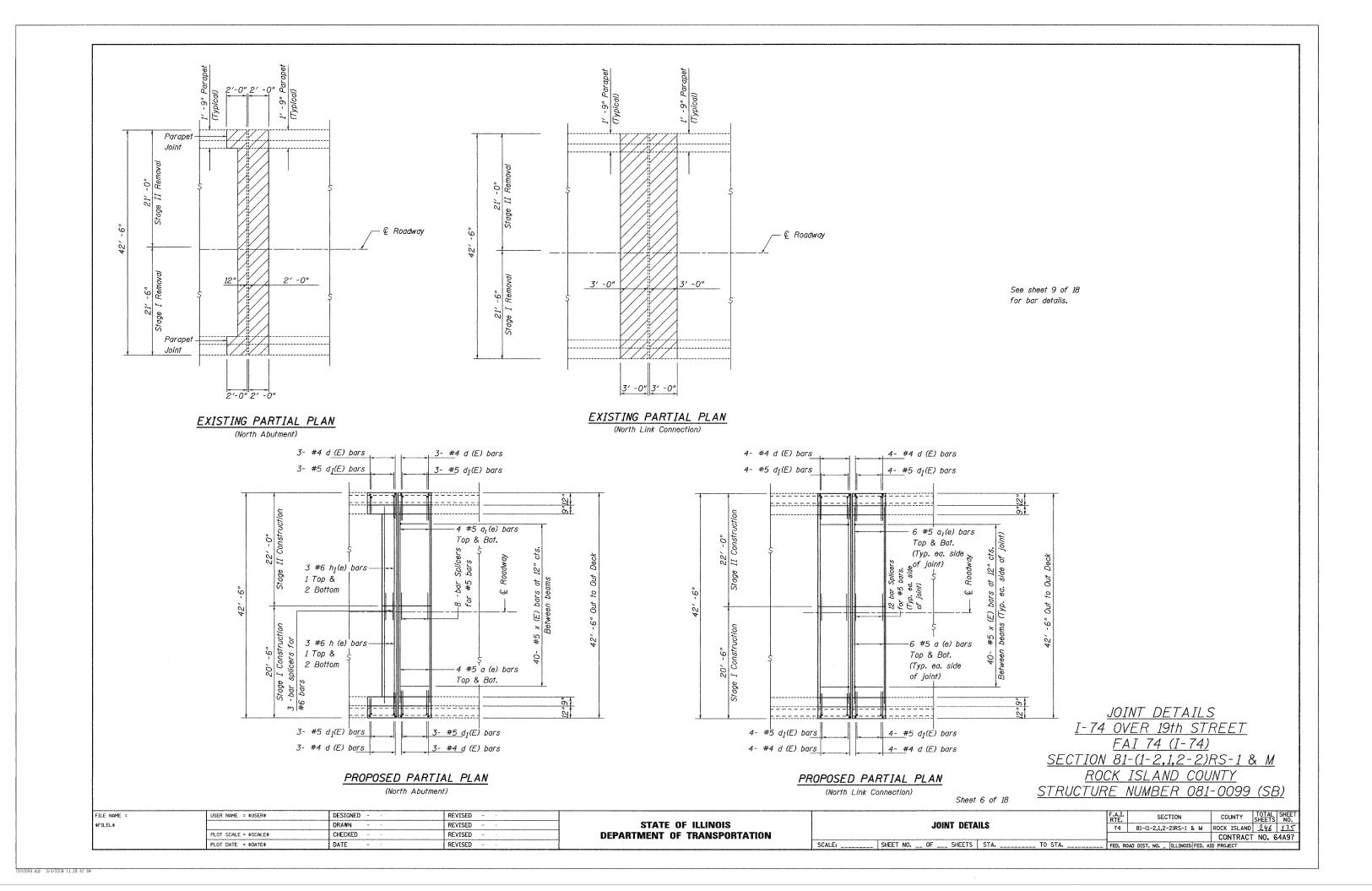
ROCK ISLAND COUNTY Sheet 5 of 18 DESIGNED LISER NAME = \$LISER\$ REVISED STATE OF ILLINOIS **Deck Plan & Cross Sections** 74 81-(1-2,1,2-2)RS-1 & M ROCK ISLAND 246 134 DRAWN REVISED

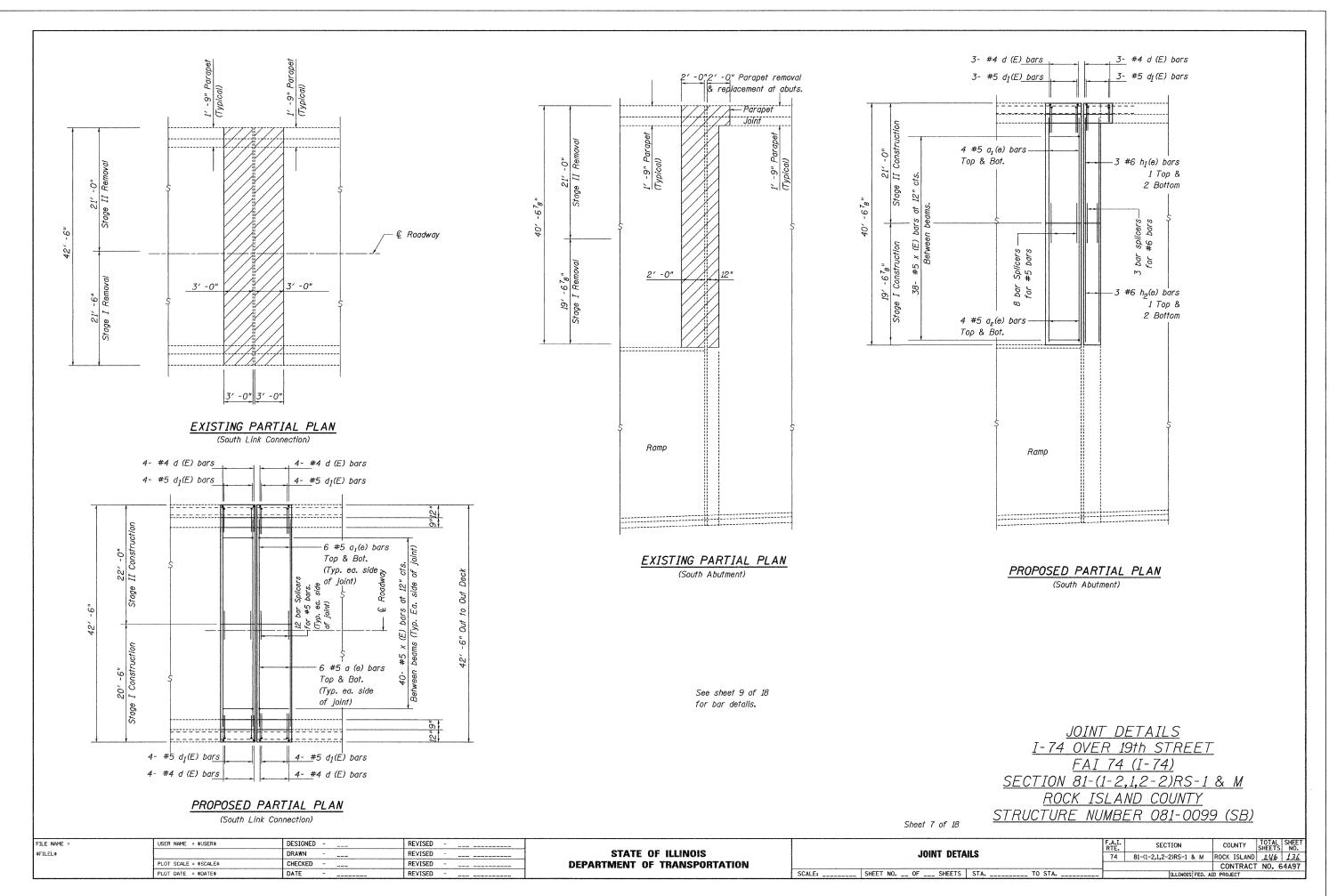
DEPARTMENT OF TRANSPORTATION

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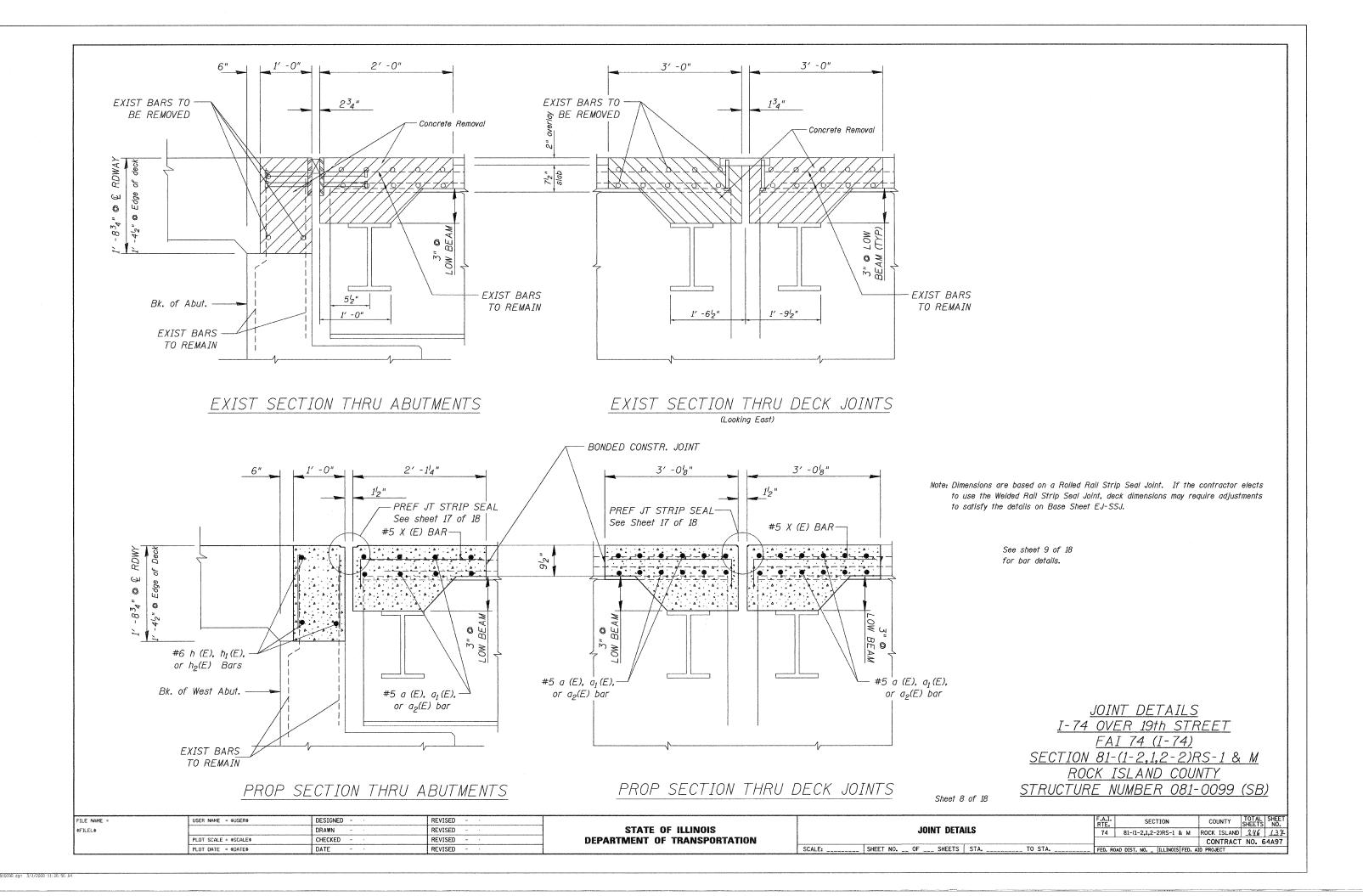
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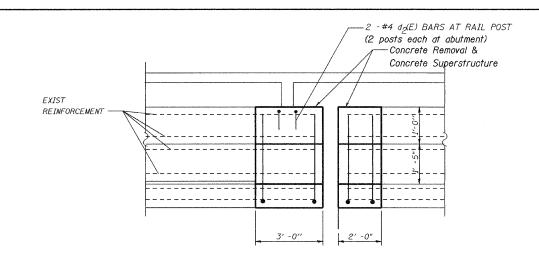
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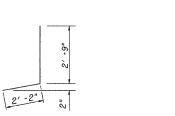


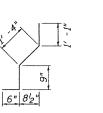
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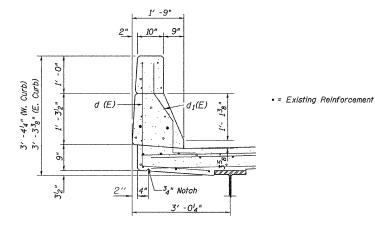




NOTE: ALL COST FOR REMOVAL OF EXISTING ALUMINUM RAILING AND RAIL ELEMENTS, IF REQUIRED, AND RE-INSTALLATION SHALL BE CONSIDERED INCLUDED IN CONCRETE REMOVAL







#4 d (E) BAR

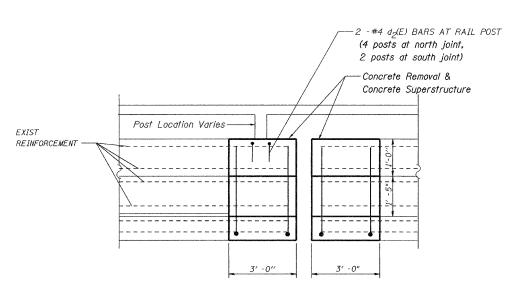
#5 d1(E) BAR

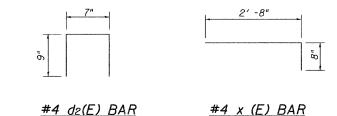
TYPICAL SECTION THRU PARAPET

Bill of Material

BAR	NO.	SIZE	LENGT	ГН		SHAPE
a (E)	56	5	20' -2	2"	-	
a ₁ (E)	64	5	21' -	8"	-	
a ₂ (E)	8	5	18′ - 1	2"	-	
d (E)	50	4	4' -11			
d ₁ (E)	50	5	3′ -8			ل
dg(E)	20	4	2' -1	н		П
h (E)	6	6	20′ -2	2"	_	***********
h1(E)	6	6	21' -8		_	
hg(E)	6					
x (E)	238	5	3' -4	4 "		
CONCRETE	SUPERSTRU	ICTURE		CU)	'D	37.5
001/0057	- 0//0550550/	OTUDE		0// \	<u> </u>	77.
CONCRETE	<i>REMOVAL</i>			CU Y	D'	37.5
REINFORC	CEMENT BARS	, EPOXY COA	TED	POUI	V D	4530
PREFORM	ED JOINT STA	RIP SEAL		F00T		170
I ILLI OTTINE		EACH				
BAR SPL.				EAC	Η	70
BAR SPL.		Τ6		EAC EAC		
BAR SPL.	ICERS	Т6				
BAR SPL.	ICERS	Τ6				
BAR SPL.	ICERS	Τ6				
BAR SPL.	ICERS	T6				
BAR SPL.	ICERS	76				70

INSIDE VIEW OF PARAPET AT ABUTMENTS

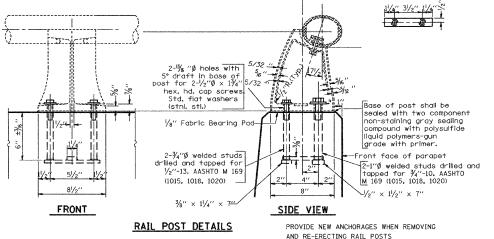




INSIDE VIEW OF PARAPET AT LINK CONNECTOR JOINTS

Notes:

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

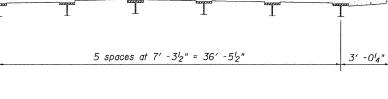


DECK DETAILS I-74 OVER 19th STREET FAI 74 (I-74) SECTION 81-(1-2,1,2-2)RS-1 & M ROCK ISLAND COUNTY STRUCTURE NUMBER 081-0099 (SB)

Sheet 9 of 18

FILE NAME =	USER NAME = \$USER\$	DESIGNED - ·	REVISED - ·			F.A.I. SECTION	COUNTY TOTAL SHEET
\$FILEL\$		DRAWN - ·	REVISED -	STATE OF ILLINOIS	JOINT DETAILS	74 81-(1-2.1.2-2)RS-1 & M	ROCK ISLAND 246 138
	PLOT SCALE = \$SCALE\$	CHECKED - ·	REVISED ~ ·	DEPARTMENT OF TRANSPORTATION			CONTRACT NO. 64A97
	PLOT DATE = *DATE*	DATE	REVISED - ·		SCALE; SHEET NO OF SHEETS STA TO STA	FED. ROAD DIST. NO ILLINOIS FED. AI	D PROJECT

STAGING PLAN Z STAGE I CONSTRUCTION Structure 081-0100 Northbound 569' -358" End to End of Deck 165' -0" 209′ -5⁵8″ 194′ -7³8" B --€ South Link Connector/ € North Link Connector/-Deck Joint Deck Joint Area of Work-SN 081-0116 84' -0" NB Off-ramp A -Temporary Concrete Barrier DECK PLAN - Structure 081-0100 Northbound 42' -6" out to out deck 67' -434" out to out deck at abutment 39' -0" face to face parapets 1'-9" 22' -7³4" at abutment 41' -3" at abutment 2' -0" 12" 17' - 3" 17' - 3" Stage I Traffic 18' -9" 21' -0" Stage I Construction Stage I Construction Stage I Traffic at abutment



CROSS SECTION A-A Structure 081-0100 Northbound (Normal Section - Looking North)

CROSS SECTION B-B Structure 081-0100 Northbound

5 spaces at @ 7' -312" = 36' -512"

(At South Abutment - Looking North)

SCALE:

Structure 081-0116 (Not part of this contract)

3 spaces @ 6' -8" = 20' -0"

STAGE I CONSTRUCTION

Install Temporary Concrete Barrier.

Move Traffic to Stage I Traffic.

Sawcut concrete wearing surface.

Replace bridge joints & repair deck.

Install new wearing surface & waterproofing membrane.

STAGING PLAN I-74 OVER 19th STREET FAI 74 (I-74)

3' -0"

Sheet 10 of 18

3'-64

__ varies

SECTION 81-(1-2,1,2-2)RS-1 & M ROCK ISLAND COUNTY STRUCTURE NUMBER 081-0100 (NB)

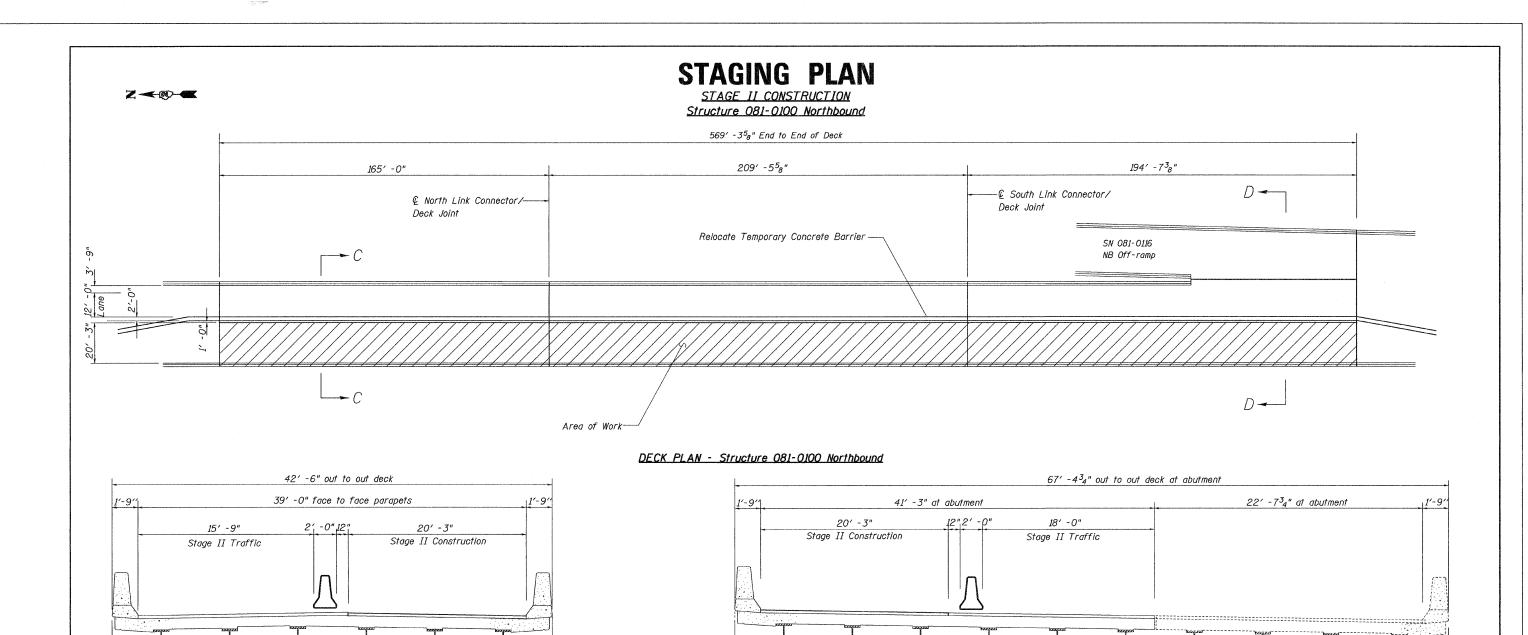
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	PLOT DATE = \$DATE\$	DATE	REVISED - ·

STATI	E OF	ILLINOIS
DEPARTMENT	OF	TRANSPORTATION

3' -04"

				F.A.			SEC	TION		COUNTY	· ;	TOTAL SHEETS	SHEET NO.
STAG	NG PLAN			74		81-(1-:	2,1,2-	2)RS-1	& M	ROCK ISL	AND	246	139
										CONTR	ACT	NO. 6	54A97
SHEET NO OF S	HEETS STA	TO ST	TA	FED.	ROAD	DIST.	NO	ILLINOIS	FED. A	ID PROJECT			

3' -04"



CROSS SECTION C-C Structure 081-0100 Northbound

5 spaces at $7' - 3l_2'' = 36' - 5l_2''$

3' -04"

(Normal Section - Looking South)

STAGE II CONSTRUCTION

3' -04"

Relocate Temporary Concrete Barrier.

Move Traffic to Stage II Traffic.

Remove concrete wearing surface.

Replace bridge joints & repair deck.

Install new wearing surface & waterproofing membrane.

Remove temporary concrete barrier.

CROSS SECTION D-D Structure 081-0100 Northbound

5 spaces at @ 7' -3'2" = 36' -5'2"

(Near South Abutment - Looking North)

Structure 081-0116

3 spaces @ 6' -8" = 20' -0"

3' -0"

(Not part of this contract)

STAGING PLAN I-74 OVER 19th STREET FAI 74 (I-74) ROCK ISLAND COUNTY

Sheet 11 of 18

3'-64"

__ varies

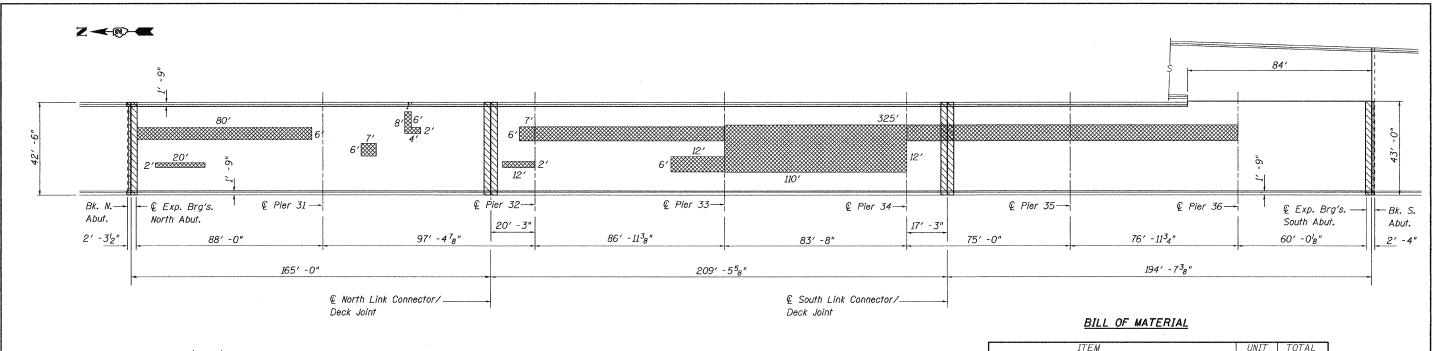
SECTION 81-(1-2,1,2-2)RS-1 & M STRUCTURE NUMBER 081-0100 (NB)

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	\$FILEL\$		DRAWN - ·	REVISED - ·
		PLOT SCALE = \$SCALE\$	CHECKED - ·	REVISED - ·
		PLOT DATE = \$DATE\$	DATE	REVISED - ·

STATI	E OF	ILLINOIS
DEPARTMENT	OF	TRANSPORTATION

3' -04"

											F.A.I. RTE.		SE	CTION	***************************************	COI	JNTY	TOTAL	SHEE NO.	1
			S	TAGI	NG P	.AN					74	81-	(1-2,1,2	2-2)RS	-1 & M	ROCK	ISLAND	246	140	
														CON	NTRACT	NO.	64A97	7		
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Legend

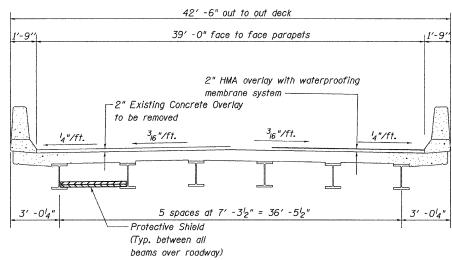
Potential Full Depth Deck Repair

Concrete Removal

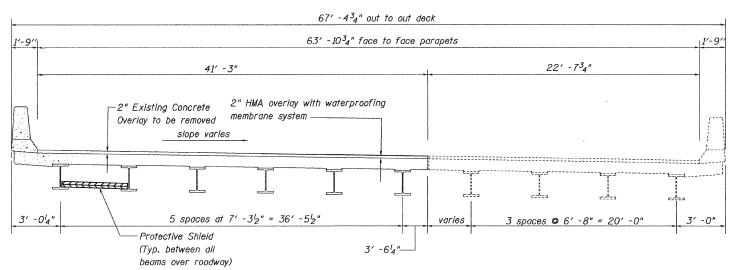
Date of Survey: 05/02/2008

DECK PLAN - Structure 081-0100 Northbound

ITEM	UNIT	TOTAL
Polymerized HMA Surface Course Mix "E", N70	Ton	268
Waterproofing Membrane System	Sq Yd	2389
Bridge Deck Scarification	Sq Yd	2466
Protective Shield	Sq Yd	820



CROSS SECTION - Structure 081-0100 Northbound
(Normal Section - Looking North)



CROSS SECTION - Structure 081-0100 Northbound

(At South Abutment - Looking North)

DECK PLAN & CROSS-SECTION

I-74 OVER 19th STREET

FAI 74 (I-74)

SECTION 81-(1-2,1,2-2)RS-1 & M

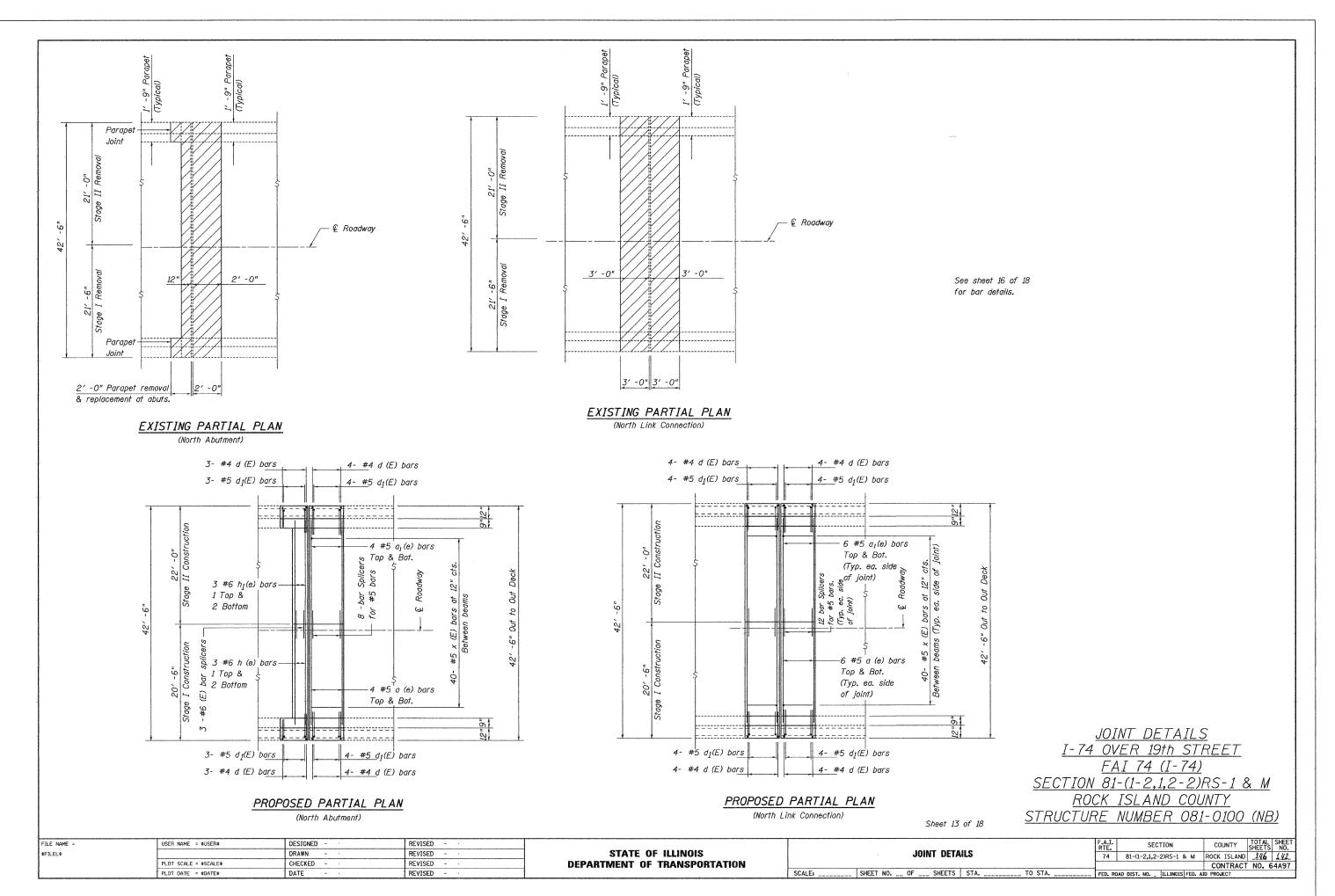
ROCK ISLAND COUNTY

STRUCTURE NUMBER 081-0100 (NB)

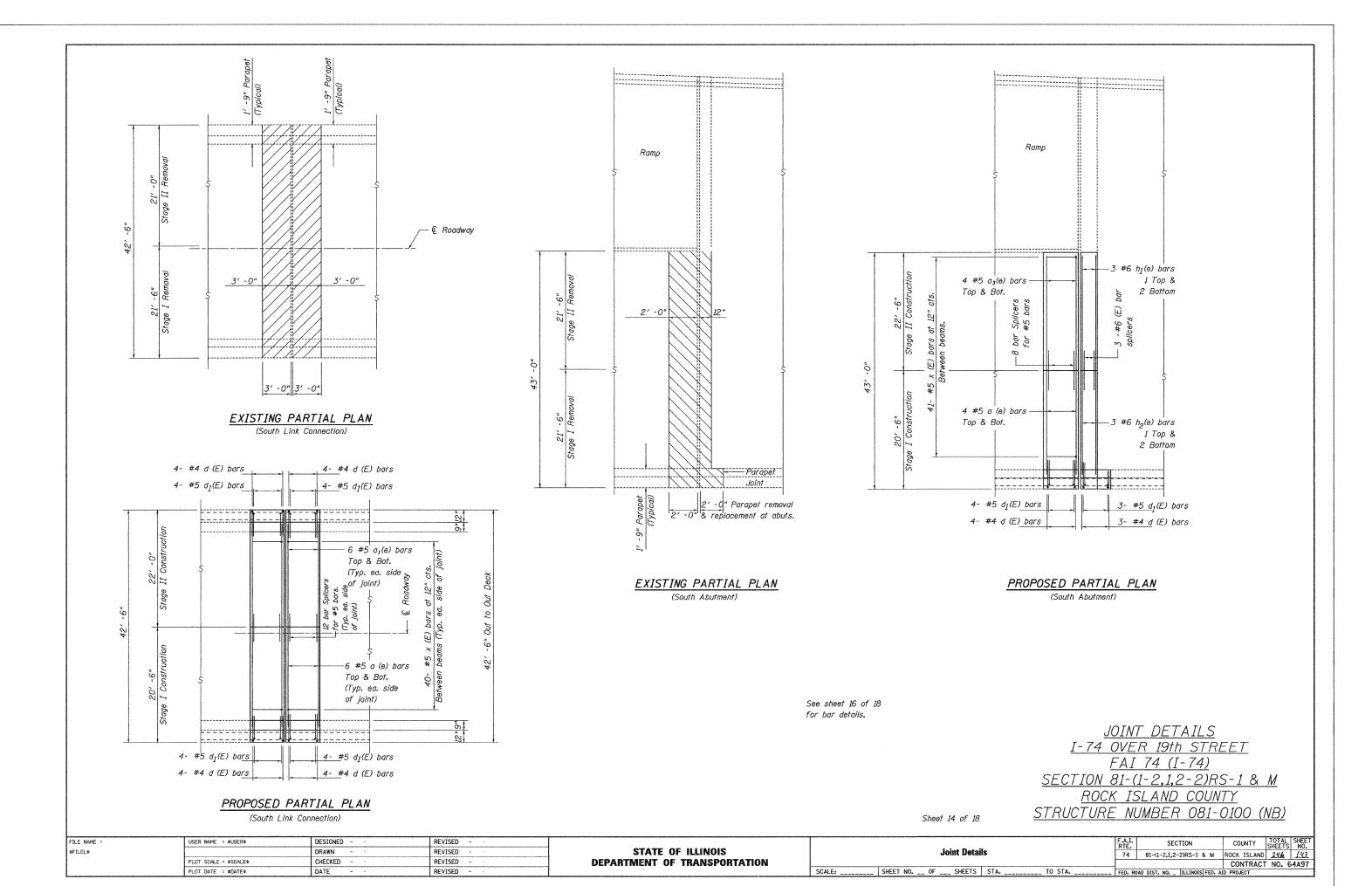
Sheet 12 of 18

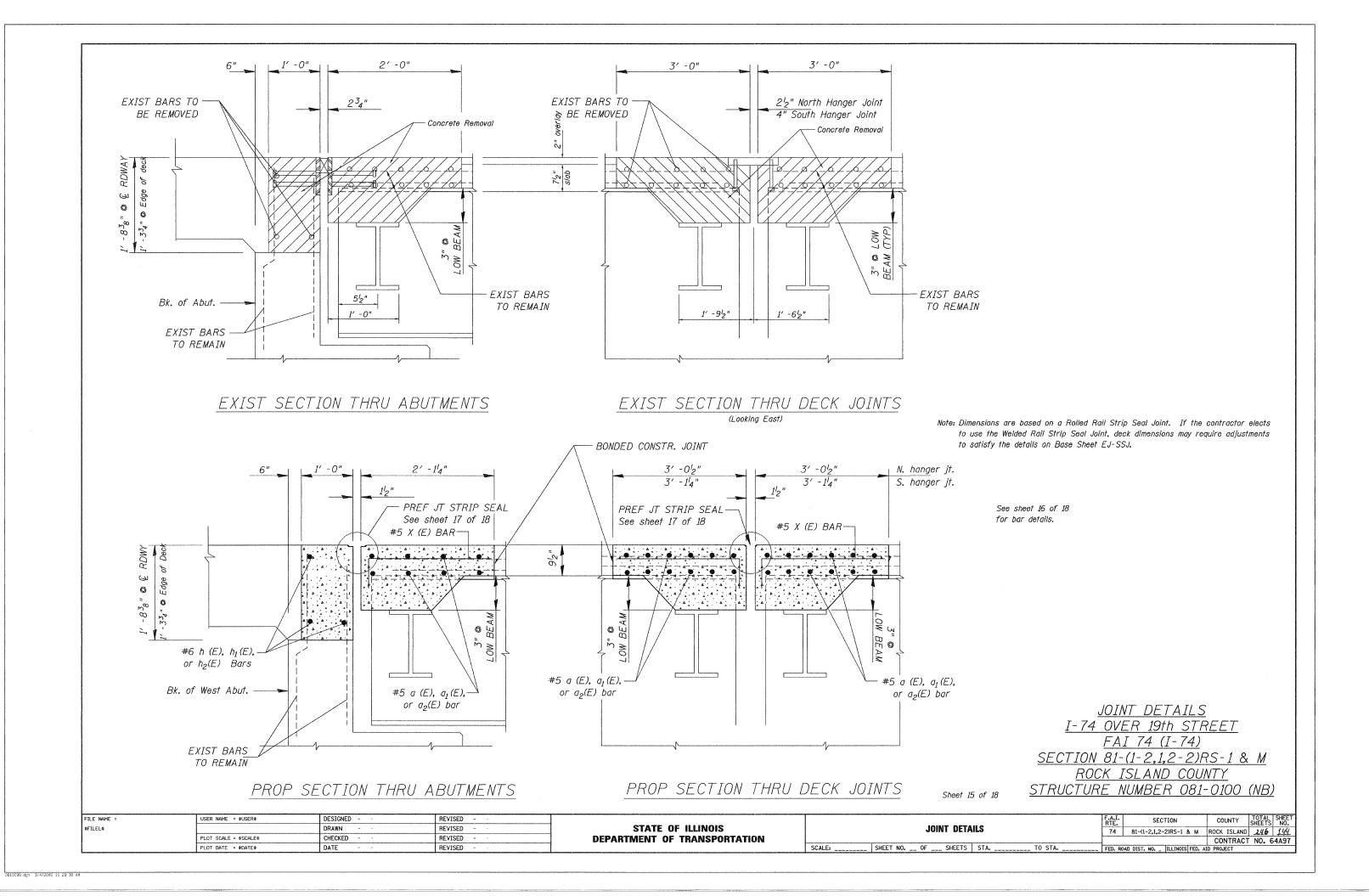
FILE NAME =	USER NAME = \$USER\$	DESIGNED - ·	REVISED - ·			F.A.I. SECTION COUNTY SHEETS NO.
\$FILEL\$		DRAWN - ·	REVISED - ·	STATE OF ILLINOIS	STAGING PLAN	74 81-(1-2.1,2-2)RS-1 & M ROCK ISLAND 2 46 141
	PLOT SCALE = \$SCALE\$	CHECKED - ·	REVISED - ·	DEPARTMENT OF TRANSPORTATION		CONTRACT NO. 64A97
	PLOT DATE = \$DATE\$	DATE	REVISED - ·		SCALE: SHEET NO OF SHEETS STA TO STA	FED. ROAD DIST. NO ILLINOIS FED. AID PROJECT

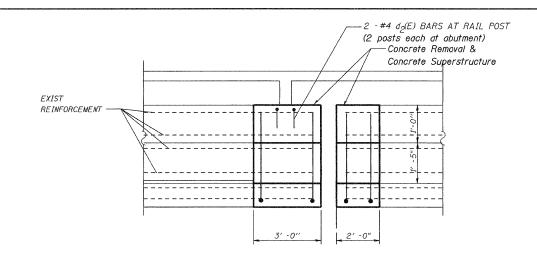
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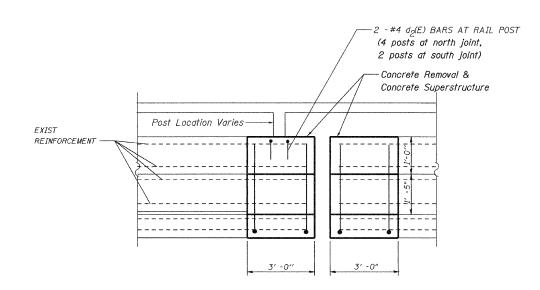
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INSIDE VIEW OF PARAPET AT ABUTMENTS



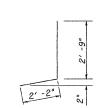
INSIDE VIEW OF PARAPET AT LINK CONNECTOR JOINTS

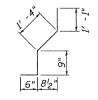
Notes:

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

REVISED

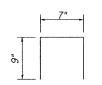
NOTE: ALL COST FOR REMOVAL OF EXISTING ALUMINUM RAILING AND RAIL ELEMENTS, IF REQUIRED, AND RE-INSTALLATION SHALL BE CONSIDERED INCLUDED IN CONCRETE REMOVAL

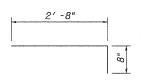




#4 d (E) BAR #

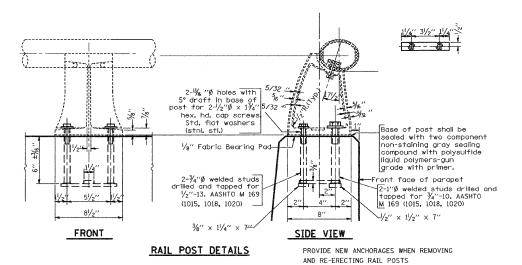
#5 d1(E) BAR





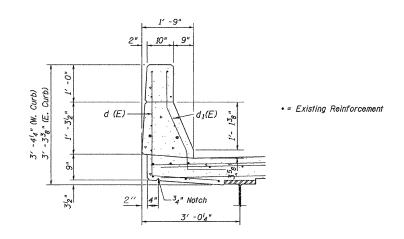
#4 d2(E) BAR

#4 x (E) BAR



SCALE:

Sheet 16 of 18



TYPICAL SECTION THRU PARAPET

Bill of Material

(Northbound structure)

BAR	NO.	SIZE	LENG	ГН		SHAPE		
a (E)	64	5	20' -		-			
a ₁ (E)	56	5	21' -	8"	-			
a3(E)	8	5	22' -	2"				
d (E)	53	<u>4</u> 5	4' - 11	II .	J			
d ₁ (E)	53		3' -8 2' -1	п		_}		
d ₂ (E)	20	4	2' -1	"		П		
h (E)	6	6	20' -	2"				
hj(E)	6	6	21' -8		_			
hg(E)	6	6	18' - 2					
(5)	0.44	5						
x (E)	241		3'	4	<u> </u>			
	SUPERSTRU	CTURE		CU Y		37.5		
	REMOVAL			CU Y		37.5		
REINFORC	EMENT BARS	, EPOXY COA	TED	POU		4580		
PREFORME	ED JOINT STA	RIP SEAL		F00		170		
BAR SPLI	CERS			EAC		70		
REM REIN	IT B TERM	Τ6		EAC	Ή	3		
						L		

DECK DETAILS

I-74 OVER 19th STREET

FAI 74 (I-74)

SECTION 81-(1-2,1,2-2)RS-1 & M

ROCK ISLAND COUNTY

STRUCTURE NUMBER 081-0100 (NB)

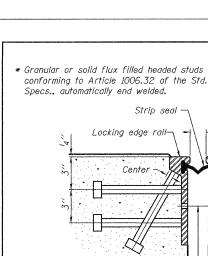
DATE

PLOT DATE = \$DATE\$

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
DECK DETAILS	74	81-(1-2,1,2-2)RS-1 & M	ROCK ISLAND	_246	145
			CONTRACT	NO. 6	4A97
SHEET NO. OF SHEETS STA. TO STA.	FFD. RO	AD DIST. NO. THE INDISPED. AT	D PROJECT		

08:0099.dgn 3/4/2009 11: 29: 36 AM



or chipped off flush with the plates

after forms are removed, typ.

1¹₂" at 50° F - Top of slab * 34" \$ x 8" studs at 1'-0" cts. at 2'-0" cts. * ³4′′ ¢ x 8′′ studs 1'-0" cts. (alt. with top horizontal studs). $\frac{7_{16}^{\prime\prime}}{bolts}$ ϕ holes at 4'-0'' cts. for $\frac{3}{8}^{\prime\prime}$ ϕ bolts. All bolts shall be burned, sawed,

Strip seal-1½"at 50° F Top of slab Locking edge rail *34" \$ x 8" studs t 1'-0'' cts. at 2'-0" cts. Anchor plate Place plates at 1'-0" cts. $\frac{7_{16}{''}}{}$ \$\phi\$ holes at 4'-0'' cts. for $\frac{3}{8}$ " \$\phi\$ bolts. All bolts shall be burned, sawed, (alt. with top horizontal studs) or chipped off flush with the plates

SECTION THRU

WELDED RAIL JOINT

ANCHOR P

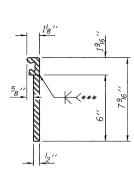
The strip seal shall be made continuous and shall have a minimum thickness of ${}^{l}_{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

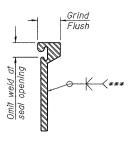
The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

SECTION THRU ROLLED RAIL JOINT

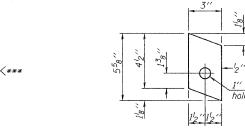


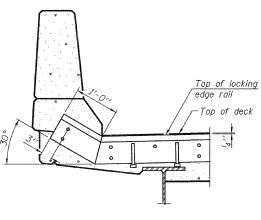


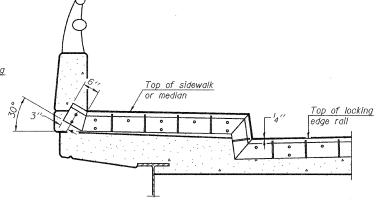
***Back gouge not required if complete joint penetration

is verified by mock-up.

after forms are removed, typ.







AT PARAPET

SCALE: N

AT SIDEWALK OR MEDIAN Shorter plates with a single row of studs at 12" cts. may be necessary on medians

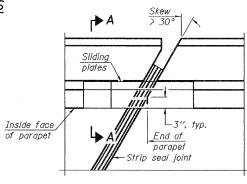
which are shallower than 9". See manufacturer's recommendation.

ROLLED EXTRUDED RAIL WELDED RAIL

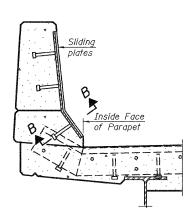
LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

LOCKING EDGE RAILS



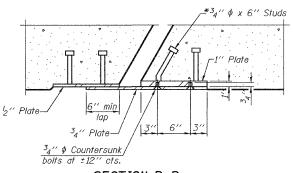
PLAN



SECTION A-A

POINT BLOCK DETAILS (for skews > 30°)

TYPICAL END TREATMENTS



SECTION B-B

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	340

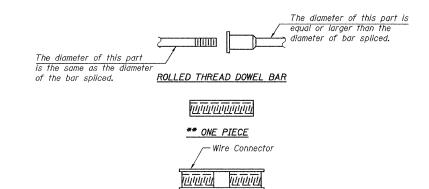
STRIP SEAL JOINT I-74 OVER 19th STREET FAI 74 (I-74) SECTION 81-(1-2,1,2-2)RS-1 & M ROCK ISLAND COUNTY STRUCTURE NUMBER 081-0099 (SB) and 081-0100 (NB)

Sheet 17 of 18

EJ-55J	10-1-08		
FILE NAME =	USER NAME = \$USER\$	DESIGNED -	 REVISED
\$FILEL\$		DRAWN -	 REVISED
	PLOT SCALE = \$SCALE\$	CHECKED -	 REVISED
	PLOT DATE = \$DATE\$	DATE -	 REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

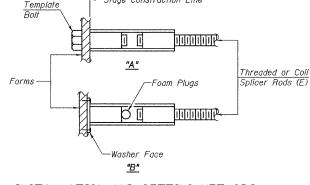
			**************************************	F.A.L. RTE.	SECTION	***************************************	COUNTY	TOTAL SHEETS	SHEET NO.
	Preformed Joi	nt Strip Seal		74	81-(1-2,1,2-2)RS-1	& M	ROCK ISLAND	246	146
						ILLINOIS	CONTRACT	NO. (64A97
None	SHEET NO. 4_ OF 5 SHEE	TS STA.	TO STA	FED. R	OAD DIST. NO ILLINO	IS FED. A	D PROJECT		



BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

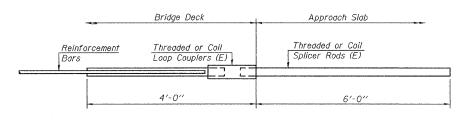
**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



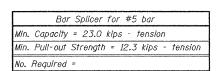
-Stage Construction Line

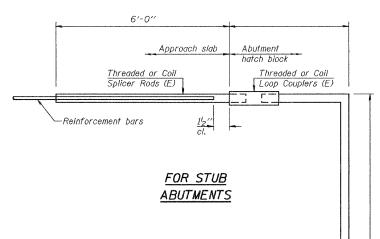
INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS





Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 12.3 kips - tension No. Required =

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

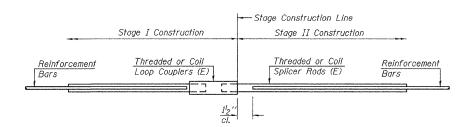
Minimum Capacity (Tension in kips) = $1.25 \times fy \times A_t$

(Tension in kips)
Minimum *Pull-out Strength = $0.66 \times fy \times A_f$

Where fy = Yield strength of lapped reinforcement bars in ksi.

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

BAR SPLICER ASSEMBLIES										
		Strength Requirements								
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension							
#4	1′-8′′	14.7	7.9							
#5	2'-2"	23.0	12.3							
#6	2'-7''	33.1	17.4							
#7	3′-5″	45.1	23.8							
#8	4'-6''	58.9	31.3							
#9	5′-9′′	75.0	39.6							
#10	7′-3′′	95.0	50.3							
#11	9'-0"	117.4	61.8							



STANDARD

Bar Size	No. Assemblies Required	Location		
#5	128	Deck		
#6	12	Abutments		

BAR SPLICER ASSEMBLY DETAILS I-74 OVER 19th STREET FAI 74 (I-74) SECTION 81-(1-2,1,2-2)RS-1 & M ROCK ISLAND COUNTY STRUCTURE NUMBER 081-0099 (SB) and

Sheet 18 of 18

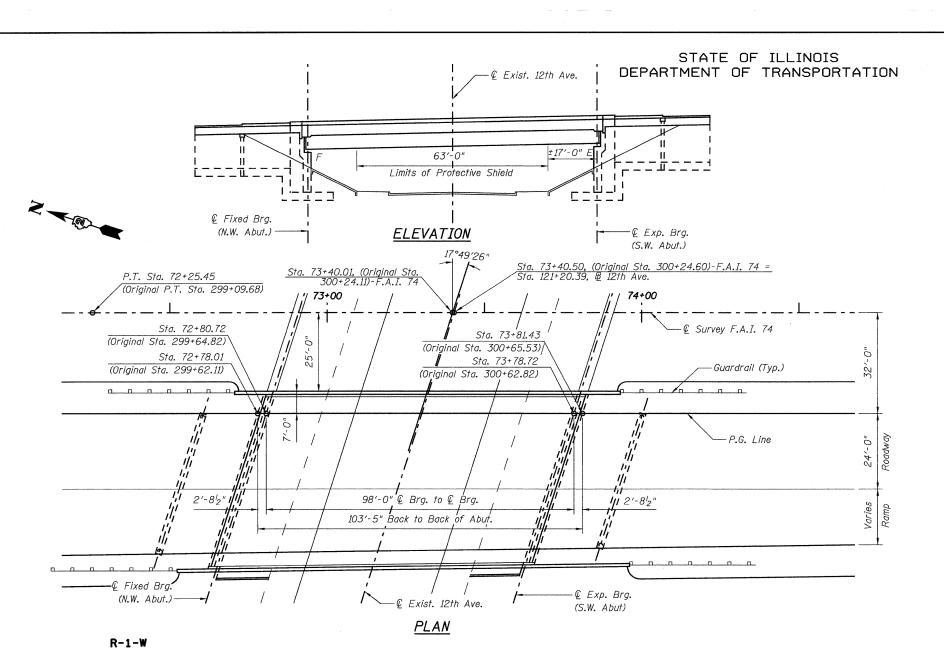
081-0100 (NB)

BSD-1	10-1-08		
FILE NAME =	USER NAME = \$USER\$	DESIGNED	REVISED
\$FILEL\$		DRAWN	REVISED
	PLOT SCALE = \$SCALE\$	CHECKED	REVISED
	PLOT DATE = \$DATE\$	DATE -	REVISED
	PLOT DATE = \$DATE\$	DATE	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

	F.A.L. RTE.	SECTION	COUNTY	TOTAL SHEET SHEETS NO.
Bar Splicers	74	81-(1-2,1,2-2)RS-1 & M	Rock Island	246 647
		ILLINOIS	CONTRACT	NO. 64A97
SCALE: NODE SHEET NO. 5_ OF 5_ SHEETS STA TO STA	FED. RO	DAD DIST. NO ILLINOIS FED. AL	ID PROJECT	

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GENERAL NOTES

Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars extending into the new construction. Any reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars extending into the new construction.

dash No field welding is permitted except as specified in the contract documents.

Reinforcement bars shall conform to the requirements of ASTM A706 Grade 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by an individual acceptable to the Engineer. Any cracks that can not be removed by grinding 1_4 " deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

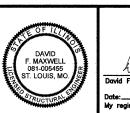
The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

Quantities of Deck Slab Repair (Full Depth, Type I), Deck Slab Repair (Full Depth, Type II) and Deck Slab Repair (Partial) are approximated. Locations will be determined by the Resident Engineer following removal of the Concrete Deck Overlay. Actual repair locations shall be shown on the as-built plans. Contractor will be paid for the quantities furnished.

Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured at an ambient temperature other than 50° F.

ROUTE NO.	SECTION	SECTION COUNTY TOTAL SHEET NO.		SHEET NO.	SHEET NO. 1
FAI 74	*	ROCK ISLAND	246	148	10 SHEETS
FED. ROAD DIST	r. NO. 7	ILLINDIS FED. AID PR	OJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



David F. Maxwell, S.E. Reg. No. 081–005455

Date: 9/10/0 B

My registration expires November 30, 2010

TOTAL BILL OF MATERIAL

TOTAL DILL OF MATE	./11/12	
ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N70	TON	102
Concrete Removal	CU. YD.	16.2
Protective Shield	SQ. YD.	354
Concrete Superstructure	CU. YD.	16.2
Reinforcement Bars, Epoxy Coated	POUND	1850
Bar Splicers	EACH	22
Preformed Joint Strip Seal	FOOT	117
Waterproofing Membrane System	SQ. YD.	570.7
Bridge Deck Scarification	SQ. YD.	564.5
Deck Slab Repair (Full Depth, Type I)	SQ. YD.	32
Deck Slab Repair (Full Depth, Type II)	SQ. YD.	32
Deck Slab Repair (Partial)	SQ. YD.	64
Diamond Grinding (Bridge Section)	SQ. YD.	570.7

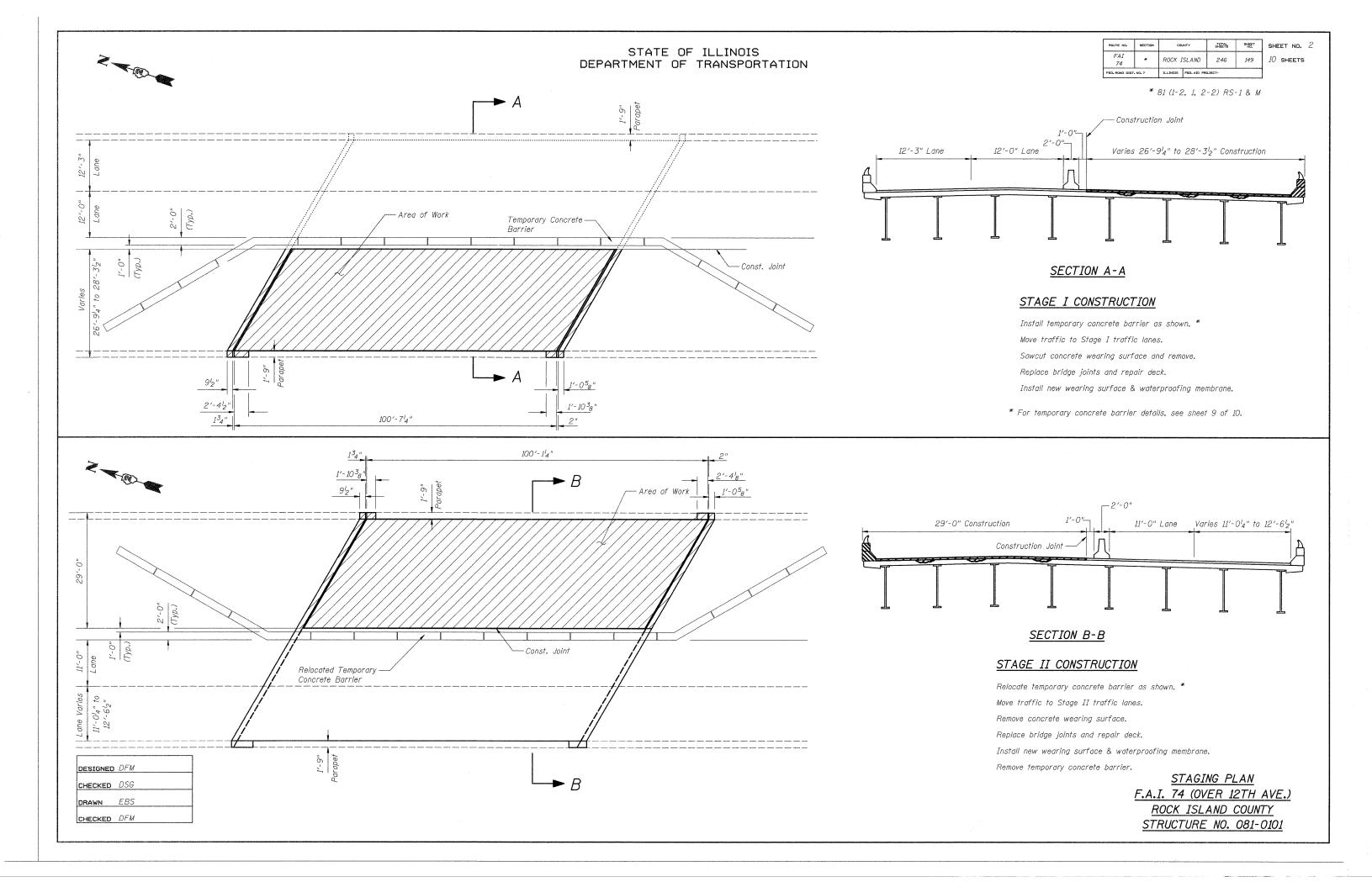
PLAN AND ELEVATION
F.A.I. 74 (OVER 12TH AVE.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0101

	ST IST IS	1	ST. "A" # 19	2 1.1	9"	15)	TEPH- SHARE PARK AVE	18TH ST "A"	, 11 11	TH_	CE ON AV CT.	3 AV	9TH 3 10TI	AVE	26TH	= \$\frac{\display}{5} = \frac{\display}{5} \frac{2887H}{51.}	N-81-1
			L	13TH	5		Ц	AVE	$I\!\!L$	T			3/1	\vdash	13TH		-17
			15TH	16TH	14TH		퇿	AVE		1			2	-			1
	14TH	15TH		25TH	ST.	ST.	AVE	1		1	-	14TH	AVE?	\	14TH	AVE	
	-	15	-	AVE		16TH	I AVE I	/ _r		II	1 1	5TH_	AVE	\uparrow	15TH	AVE	
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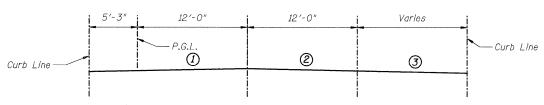
LOCATION PLAN

DESIGNED	DFM	
CHECKED	DSG	
DRAWN	EBS	
CHECKED	DFM	



ROUTE NO.	SECTION	cou	INTY	TOTAL SHEETS	SHEET NO.	SHEE	T NO.	3
FAI 74	*	ROCK 1	SLAND	246	150	<i>10</i> s	HEETS	
FED. ROAD DIST	NO. 7	ILLINOIS	FED. AID PR	JECT-	· · · · · · · · · · · · · · · · · · ·			

* 81 (1-2, 1, 2-2) RS-1 & M



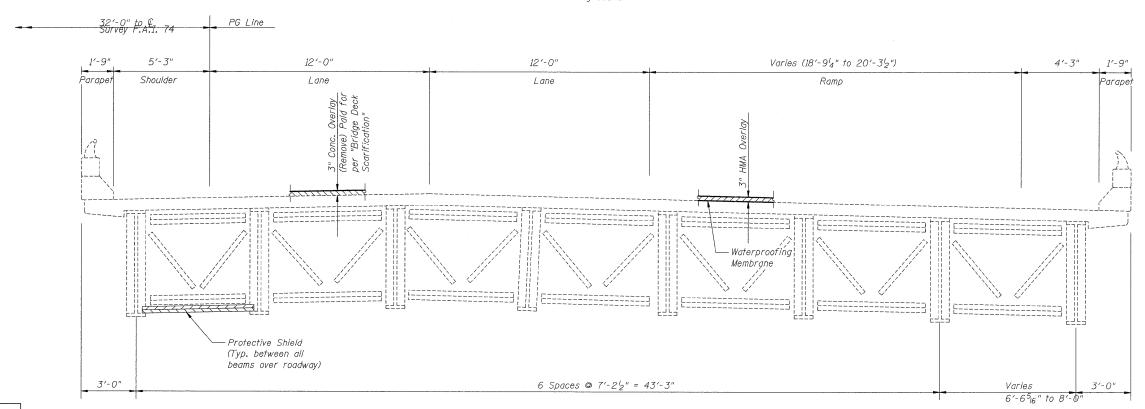
CURRENT STATION	ORIGINAL STATION	SLOPE ①	SLOPE ②	SLOPE ③
72+65.90	299+50	0.032	0.015	-0.0156
73+15.90	300+00	0.0225	0.00	-0.017
73+65.90	300+50	0.017	-0.010	-0.019
73+90.90	300+75	0.0163	-0.013	-0.0196

BILL OF MATERIAL

ITEM	UNIT	QUANTITY	
Polymerized HMA Surface Course Mix "E", N90	TON	102	
Protective Shield	SQ. YD.	354	
Waterproofing Membrane System	SQ. YD.	570.7	
Bridge Deck Scarification	SQ. YD.	564.5	
Diamond Grindina (Bridge Section)	SQ. YD.	570.7	

EXISTING SLOPE CONFIGURATION

(Looking South)



DESIGNED DFM

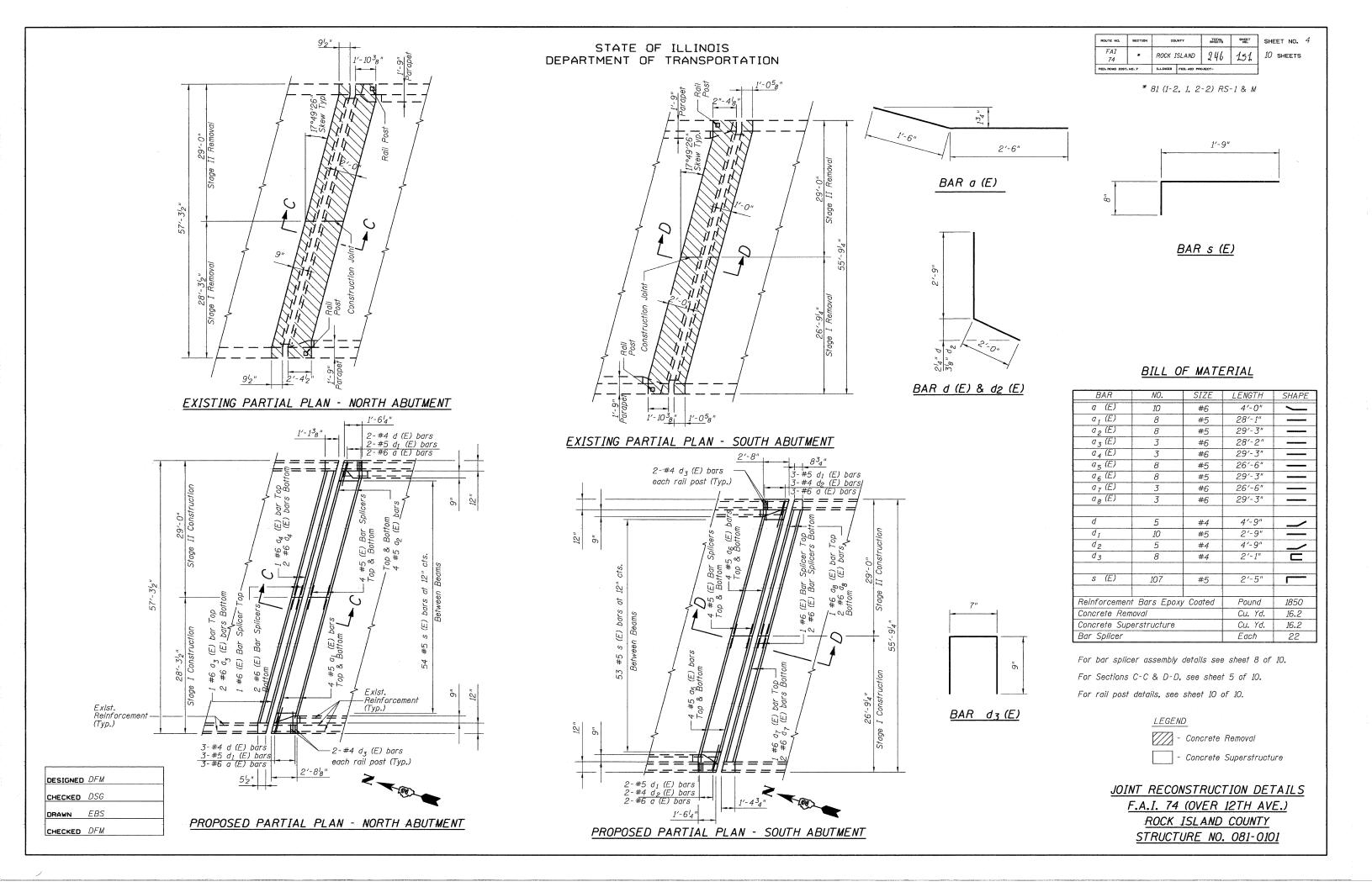
CHECKED DSG

DRAWN EBS

CHECKED DFM

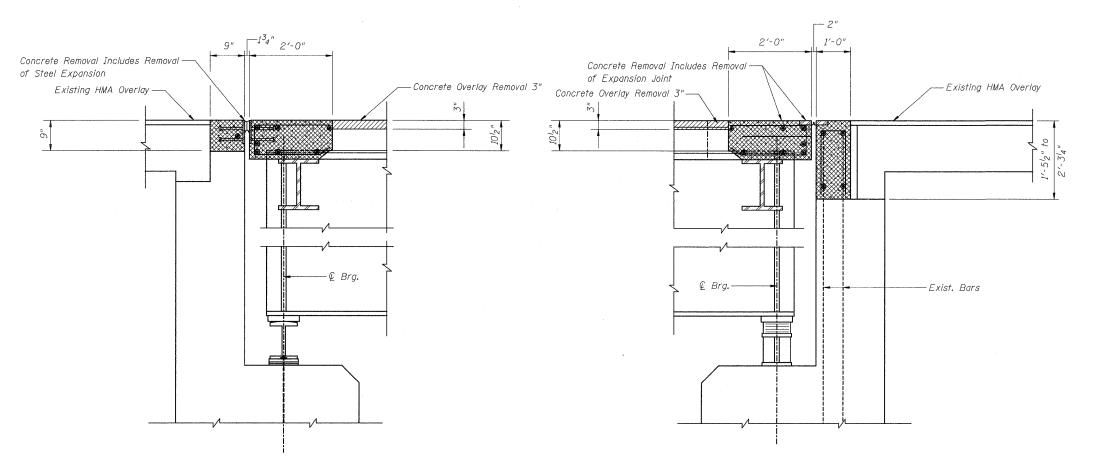
CROSS SECTION
(Looking South)

BRIDGE REPAIR DETAILS
F.A.I. 74 (OVER 12TH AVE.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0101



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 5
FAI 74	*	ROCK ISLAND	246	152	10 sheets
FED. ROAD DIST	ND. 7	ILLINGIS FED. AID PR	OJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



LEGEND

- Concrete Removal

- Bridge Deck Scarification

EXISTING SECTION C-C THRU NORTH ABUTMENT

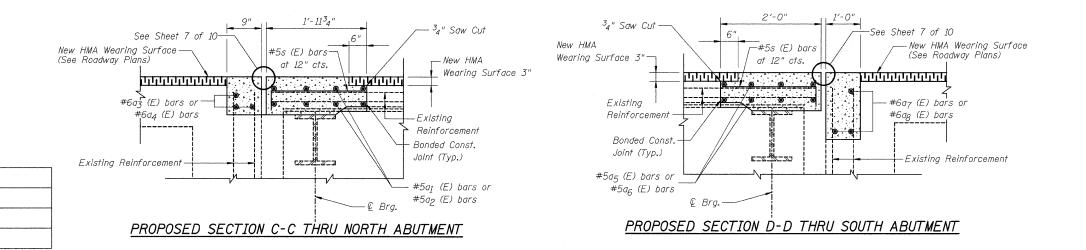
DESIGNED DFM

CHECKED DSG

DRAWN EBS

CHECKED DFM

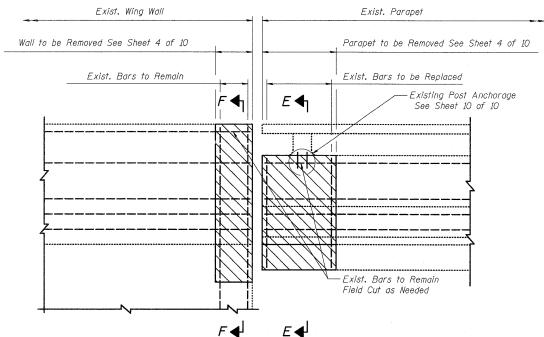
EXISTING SECTION D-D THRU SOUTH ABUTMENT



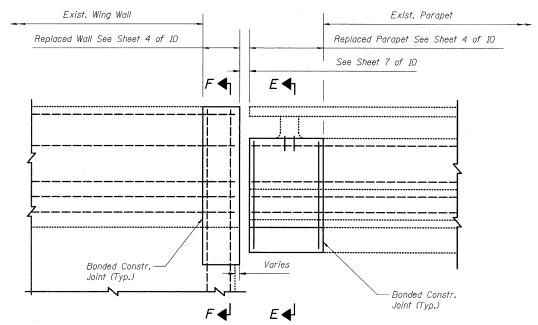
JOINT RECONSTRUCTION DETAILS F.A.I. 74 (OVER 12TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0101

ROUTE NO.	SECTION	cou	NTY	TOTAL SHEETS	SHEET NO.	SHE	ET NO.
FAI 74	*	ROCK I.	SLAND	246	153	10	SHEETS
FED. ROAD DIST.	NO. 7	ILLINDIS	FED. AID PR	DJECT-			

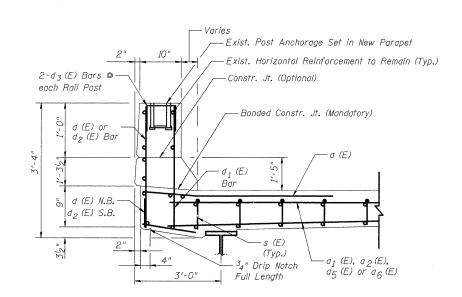
* 81 (1-2, 1, 2-2) RS-1 & M



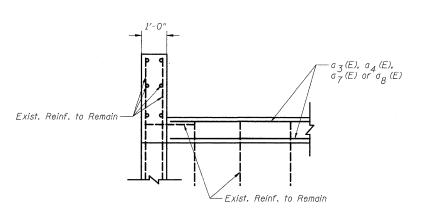
ELEVATION OF CONCRETE REMOVAL THRU PARAPET AND WING WALL



ELEVATION OF CONCRETE REPLACEMENT THRU PARAPET AND WING WALL



SECTION E-E THRU BRIDGE DECK PARAPET



SECTION F-F THRU ABUTMENT BACKWALL AND WING WALL

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

Hatched areas indicate concrete sections to be removed and replaced. Perimeters of concrete removal areas shall be saw cut 34 " prior to the removal of concrete.

PARAPET AND RETAINING WALL DETAILS

F.A.I. 74 (OVER 12TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0101



10 SHEETS

Top of locking edge rail

Notes:

* 81 (1-2, 1, 2-2) RS-1 & M

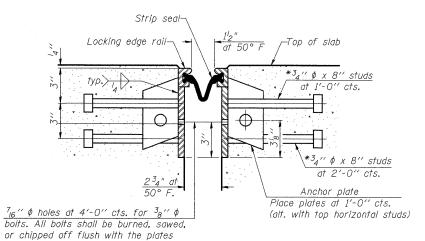
The strip seal shall be made continuous and shall have a minimum thickness of ${}^{l}_{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

> Top of sidewalk or median

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.



SECTION THRU ROLLED RAIL JOINT

at 50° F

— Top of slab

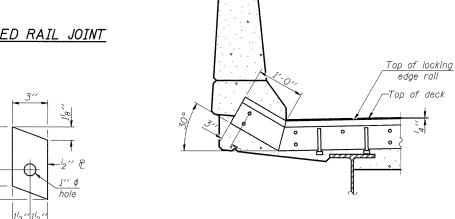
'-0'' cts.

at 2'-0" cts.

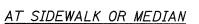
at 1'-0" cts, (alt, with

top horizontal studs).

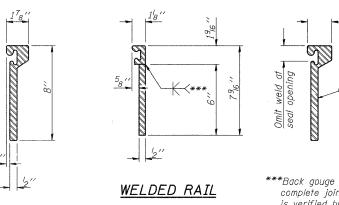
SECTION THRU WELDED RAIL JOINT



AT PARAPET



Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.



***Back gouge not required if complete joint penetration is verified by mock-up.

after forms are removed, typ.

ANCHOR P

ROLLED EXTRUDED RAIL

*Granular or solid flux filled headed studs

Locking edge rail-

 $^{7}_{16}$ " ϕ holes at 4'-0" cts. for $^{3}_{8}$ " ϕ

bolts. All bolts shall be burned, sawed,

or chipped off flush with the plates

after forms are removed, typ.

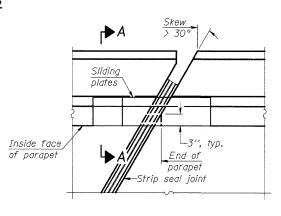
conforming to Article 1006.32 of the Std. Specs., automatically end welded.

Strip seal

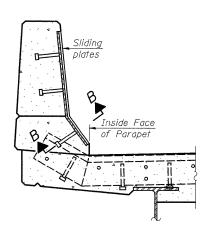
LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

LOCKING EDGE RAILS



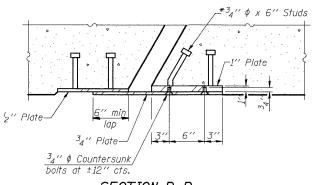
PLAN



SECTION A-A

POINT BLOCK DETAILS (for skews > 30°)

TYPICAL END TREATMENTS



SECTION	B-B
	-

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	117

PREFORMED JOINT STRIP SEAL F.A.I. 74 (OVER 12TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0101

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

EJ-SSJ

5-16-08



10 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

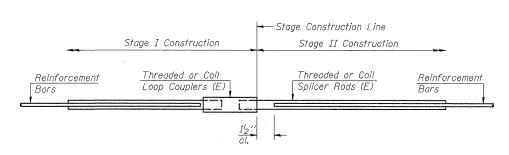
Minimum Capacity (Tension in kips) = 1.25 x fy x A_t

(Tension ii kipo) Minimum *Pull-out Strength = 0.66 x fy x A_t (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

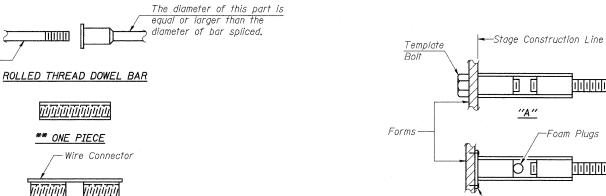
BAR SPLICER ASSEMBLIES Strength Requirements Splicer Rod or Bar Size to Min. Capacity Min. Pull-Out Strength Dowel Bar Length be Spliced kips - tension kips - tension 7.9 #4 1'-8" 14.7 #5 2'-0" 23.0 12.3 2'-7" 33.1 17.4 #6 #7 3′-5′′ 45.1 23.8 58.9 4'-6' #8 31.3 #9 5′-9″ 75.0 39.6 7'-3" 95.0 50.3 #10 117.4 61.8 #11



STANDARD

Bar Size	No. Assemblies Required	Location
#5	8	Bridge Deck at South Abutment
#5	8	Bridge Deck at North Abutment
#6	3	South Abutment Backwall
#6	3	North Abutment Backwall

BAR SPLICER ASSEMBLY DETAILS F.A.I. 74 (OVER 12TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0101



BAR SPLICER ASSEMBLY ALTERNATIVES

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

Bridge Deck

4'-0"

Threaded or Coil

Loop Couplers (E) .

WELDED SECTIONS

INSTALLATION AND SETTING METHODS "A" :Set bar splicer assembly by means of a template bolt.

6'-0"

Threaded or Coil

Splicer Rods (E)

Reinforcement bars

Washer Face

<u>''B''</u>

"B" :Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.

Approach slab

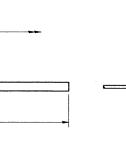
FOR STUB ABUTMENTS

Abutment hatch block

> Threaded or Coil Loop Couplers (E)

Threaded or Coil

Splicer Rods (E)



Approach Slab

Threaded or Coil

Splicer Rods (E)

FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required =

	Bar Splicer for #5 bar
Min.	Capacity = 23.0 kips - tension
Min.	Pull-out Strength = 12.3 kips - tension

	Bar Splicer for #5 bar
Min.	Capacity = 23.0 kips - tension
Min.	Pull-out Strength = 12.3 kips - tension
No.	Required =

DESIGNED	DFM	
CHECKED	DSG	
DRAWN	EBS	
CHECKED	DFM	
RSD	- 1	5-

5-16-08

The diameter of this part

is the same as the diamete

Reinforcement

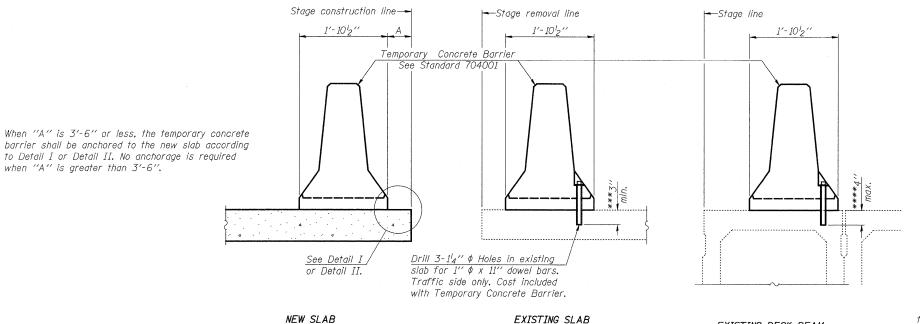
Bars

of the bar spliced.

EXISTING DECK BEAM

ROUTE NO.	SECTION	co	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 9
FAI 74	*	ROCK 1	SLAND	246	156	10 sheets
FED. ROAD DIST	, NO. 7	ILLINOIS	FED. AID PRI	DJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



NOTES

Detail I - With Bar Splicer or Couplers: Connect one (1) 1"x7"x10" steel ₱ to the top layer of couplers with 2-58" \$\phi\$ bolts screwed to coupler at approximate € of each barrier panel.

Detail II - With Extended Reinforcement Bars: Connect one (1) 1"x7"x10" steel P to the concrete slab or concrete wearing surface with 2-58"\$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate € of each barrier panel. Cost of anchorage is included with Temporary Concrete Barrier.

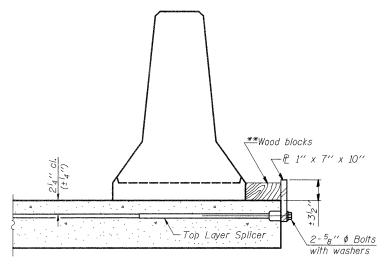
The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

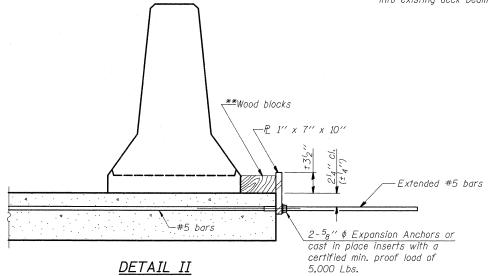
***Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment

shall be in addition to wearing surface depth.

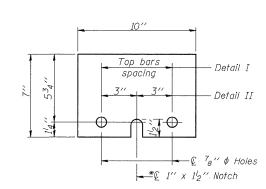
*****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



DETAIL I



**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



STEEL RETAINER P 1" x 7" x 10"

*Required only with Detail II

TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION F.A.I. 74 (OVER 12TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0101

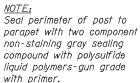
DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM
·	

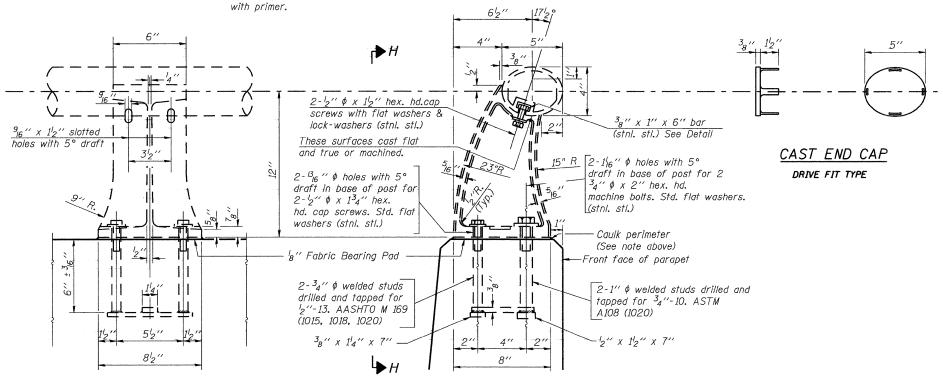
R-27 5-16-08

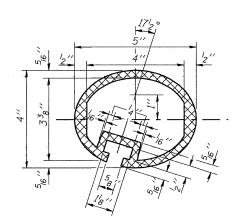
| ROUTE NO. | SECTION | COUNTY | SHEET'S | COUNTY |

SHEET NO. 10 10 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M





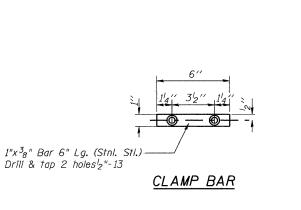


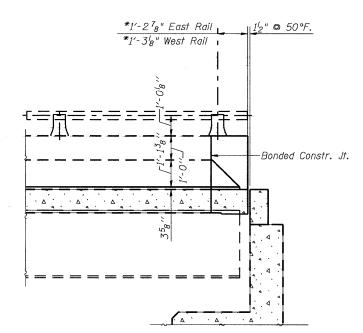
SEC. THRU ELLIPTICAL
RAIL SECTION

VIEW H-H

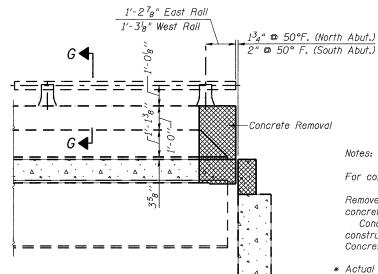
SECTION G-G

RAIL POST DETAILS





INSIDE VIEW AT PROPOSED ABUTMENTS



INSIDE VIEW AT EXISTING ABUTMENTS

For concrete removal limits See Sheet 4 of 10.

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

* Actual distance varies. New Joints are skewed through Parapet. Length of rail shall be adjusted as necessary with a flush cut and the cast end cap shall be reused.

DESIGNED DFM

CHECKED DSG

DRAWN EBS

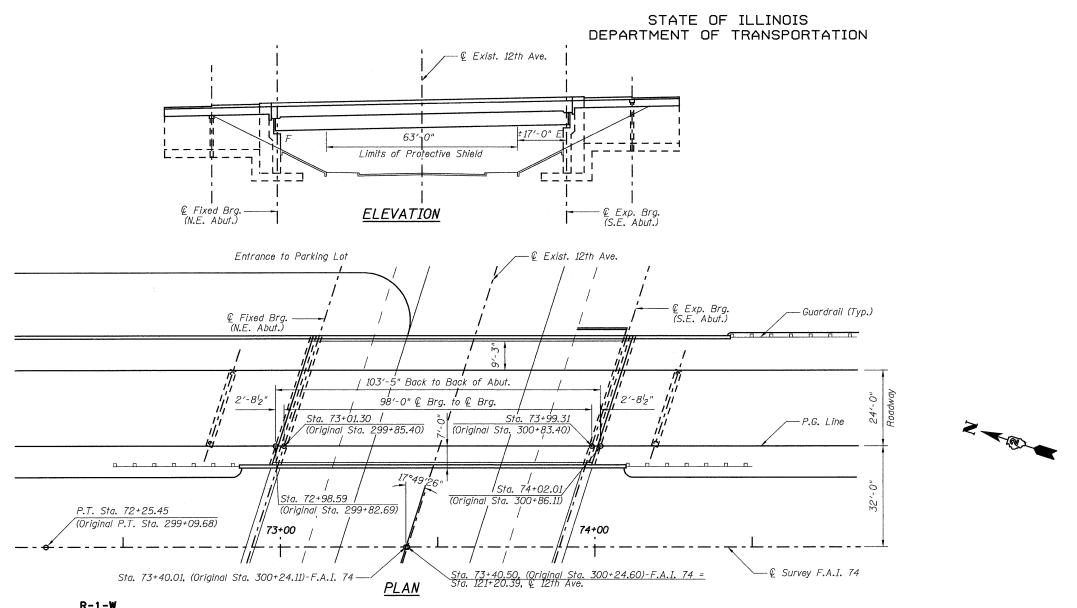
CHECKED DFM

RAILING DETAILS

F.A.I. 74 (OVER 12TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0101



GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.

Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with approved bar splicer or anchorage system. Cost included with Concrete Removal.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by an individual acceptable to the Engineer. Any cracks that can not be removed by grinding \(^4\) deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

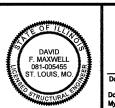
The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

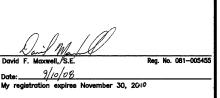
Quantities of Deck Slab Repair (Full Depth, Type I), Deck Slab Repair (Full Depth, Type II) and Deck Slab Repair (Partial) are approximated. Locations will be determined by the Resident Engineer following removal of the Concrete Deck Overlay. Actual repair locations shall be shown on the as-built plans. Contractor will be paid for the quantities furnished.

Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured at an ambient temperature other than 50° F.

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 1
FAI 74	*	ROCK ISLAND	246	158	10 SHEETS
FED. ROAD DIST	r. NO. 7	ILLINOIS FED. AID PR	OJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M





TOTAL BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N70	TON	74
Concrete Removal	CU. YD.	12.4
Protective Shield	SQ. YD.	252
Concrete Superstructure	CU. YD.	12.4
Reinforcement Bars, Epoxy Coated	POUND	1390
Bar Splicers	EACH	22
Preformed Joint Strip Seal	FOOT	86
Waterproofing Membrane System	SQ. YD.	414.3
Bridge Deck Scarification	SQ. YD.	409.8
Deck Slab Repair (Full Depth, Type I)	SQ. YD.	22.7
Deck Slab Repair (Full Depth, Type II)	SQ. YD.	22.7
Deck Slab Repair (Partial)	SQ. YD.	50.5
Diamond Grinding (Bridge Section)	SQ. YD.	414.3

PLAN AND ELEVATION

F.A.I. 74 (OVER 12TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0102

Si jus	BUTTERMORTH CS &	TO THE	2 Han 15 Han 15	911	SOMAN I I I I I I I I I I I I I I I I I I I	л 18тн Isт ″. 18тн Isт ″.	11TH 11TH	CONTROL AVE	VE AVE	10TH	AVE 11TH	261	10TH
		15TH	13TH 13TH	5 14TH	lig	(AV		1	4	AVE P		13TH	
14TH	15TH	15.7	L5TH AVE	ST		VE /E	$\neg \mathbb{N}$	11	STH_	AVE AVE	# <u>§</u>	15TH 16TH	A

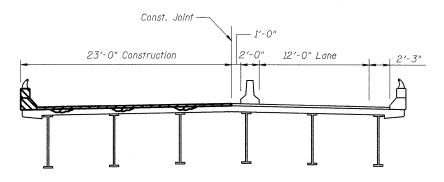
LOCATION PLAN

DESIGNED	DFM	 ***********	
CHECKED	DSG		
DRAWN	EBS		
CHECKED	DFM		

STATE OF ILLINOIS

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHEET NO. 2	2
FAI 74	*	ROCK I	SLAND	246	159	10 SHEETS	
FED. ROAD DIST	NO. 7	ILLINOIS	FED. AID PRO	DJECT-			

* 81 (1-2, 1, 2-2) RS-1 & M



SECTION A-A

STAGE I CONSTRUCTION

Install temporary concrete barrier as shown. *

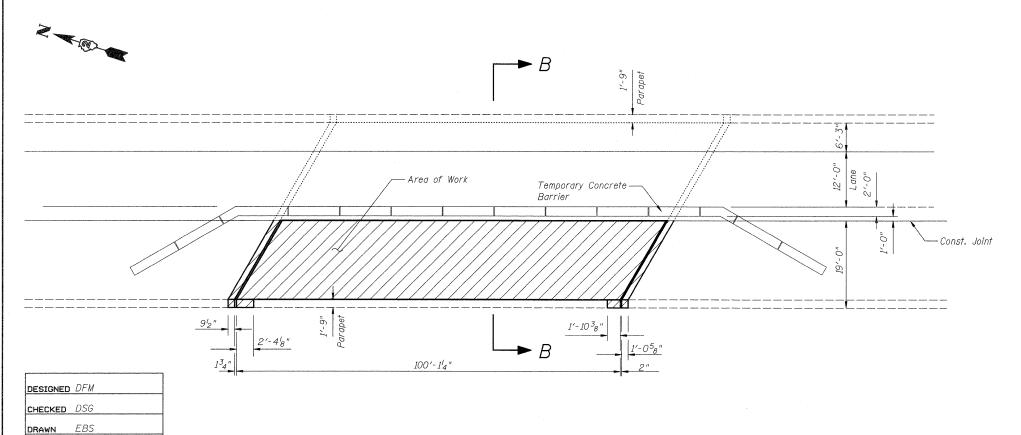
Move traffic to Stage I traffic lane.

Sawcut concrete wearing surface and remove.

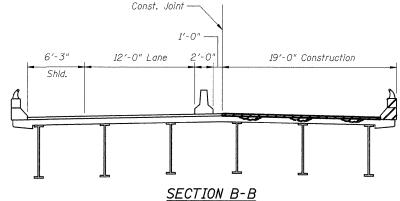
Replace Bridge Joints and repair deck.

Install new wearing surface & waterproofing membrane.

* For temporary concrete barrier details, see sheet 9 of 10.



CHECKED DFM



STAGE II CONSTRUCTION

Relocate temporary concrete barrier as shown. *

Move traffic to Stage II traffic lane.

Remove concrete wearing surface.

Replace bridge joints and repair deck.

Install new wearing surface & waterproofing membrane.

Remove temporary concrete barrier.

STAGING PLAN

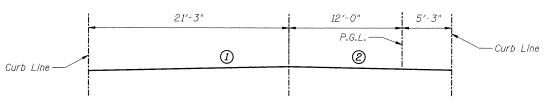
F.A.I. 74 (OVER 12TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0102

	ROUTE NO.	SECTION	co	JNTY	TOTAL SHEETS	SHEET NO.	SHE	ET NO.	
	FAI 74	*	ROCK 1	SLAND	246	160	10	SHEETS	
ŀ	FED. ROAD DIST	NO. 7	ILLINGIS	FED. AID PRI	DIECY-				

* 81 (1-2, 1, 2-2) RS-1 & M



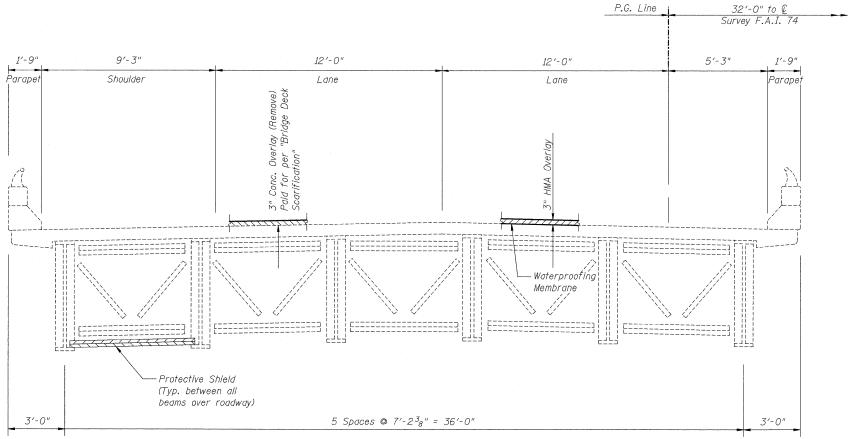
	ORIGINAL STATION	SLOPE ①	SLOPE ②
72+90.90	299+75	0.0238	0.007
73+15.90	300+00	0.0192	-0.0007
73+40.90	300+25	0.0167	-0.008
73+65.90	300+50	0.0158	-0.012
73+90.90	<i>300+75</i>	0.0156	-0.0156

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N90	TON	74
Protective Shield	SQ. YD.	252
Waterproofing Membrane System	SQ. YD.	414.3
Bridge Deck Scarification	SQ. YD.	409.8

EXISTING SLOPE CONFIGURATION

(Looking South)



CROSS SECTION

(Looking South)

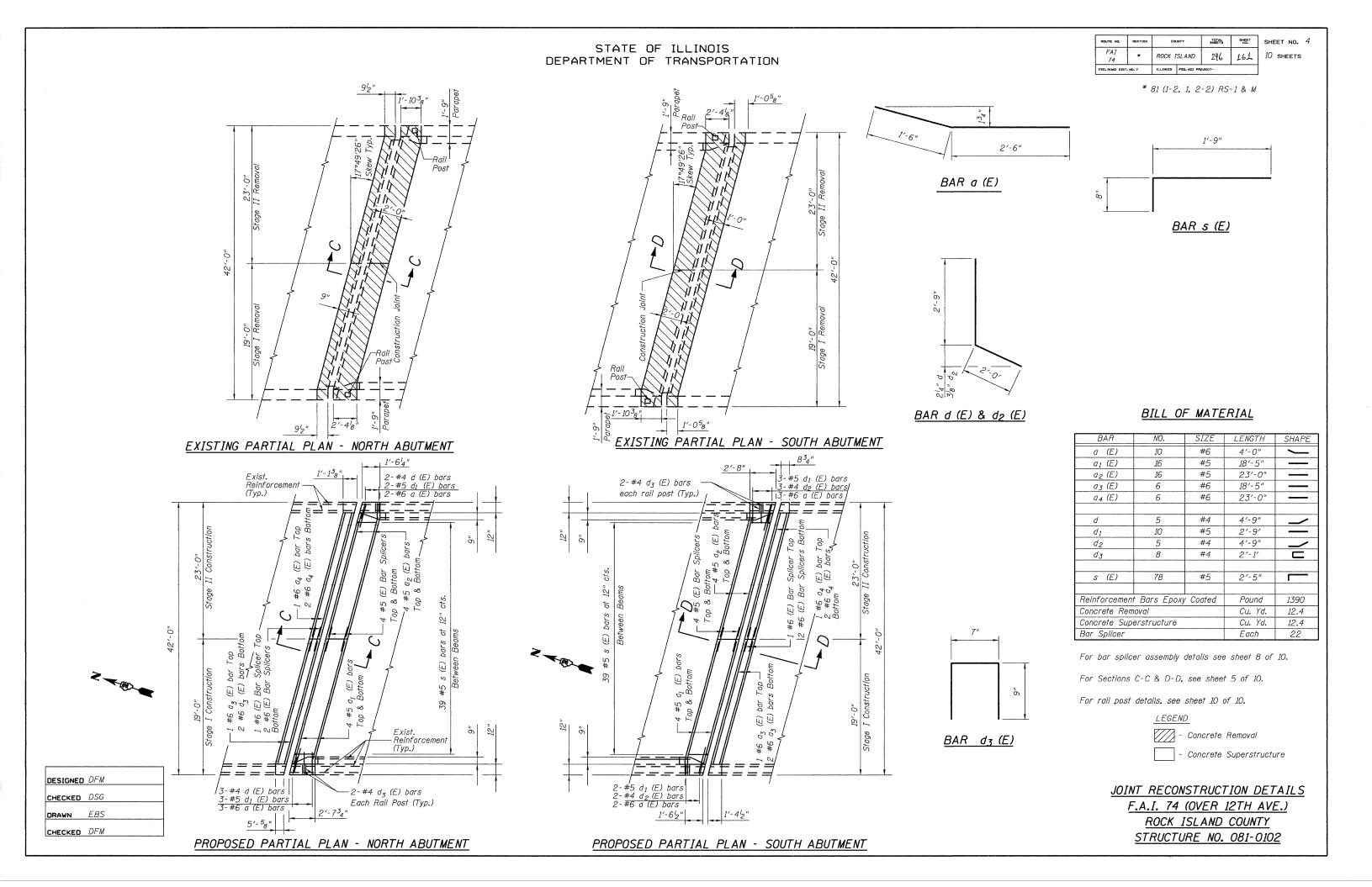
BRIDGE REPAIR DETAILS
F.A.I. 74 (OVER 12TH AVE.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0102

DESIGNED DFM

CHECKED DSG

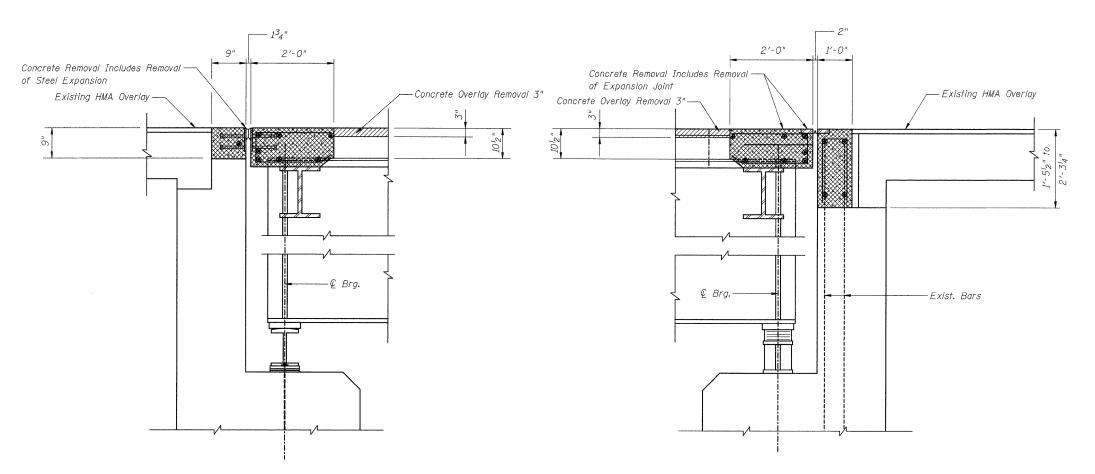
DRAWN EBS

CHECKED DFM



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 5
FAI 74	*	ROCK ISLAND	246	162	10 sheets
FED. ROAD DIST	. NO. 7	ILLINOIS FED. AID PR	DJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



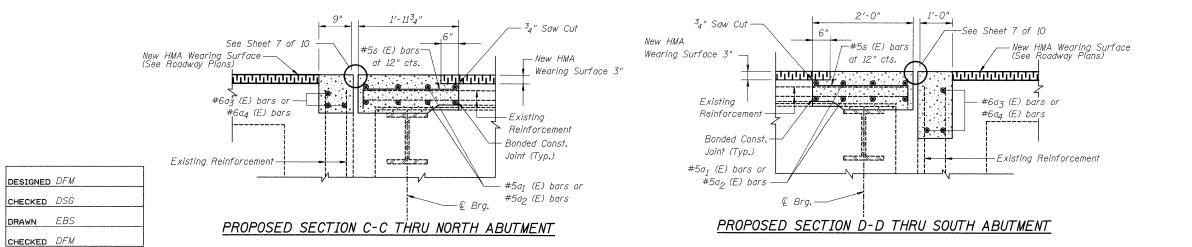
LEGEND

Concrete Removal

- Bridge Deck Scarification

EXISTING SECTION C-C THRU NORTH ABUTMENT

EXISTING SECTION D-D THRU SOUTH ABUTMENT



JOINT RECONSTRUCTION DETAILS

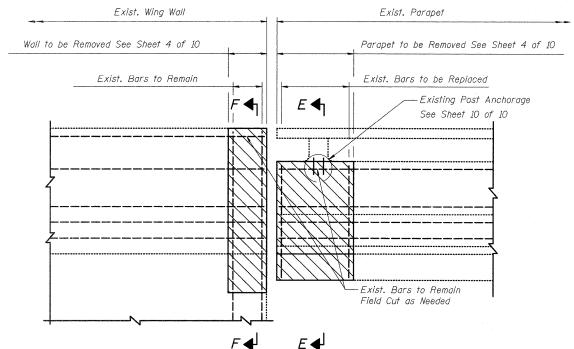
F.A.I. 74 (OVER 12TH AVE.)

ROCK ISLAND COUNTY

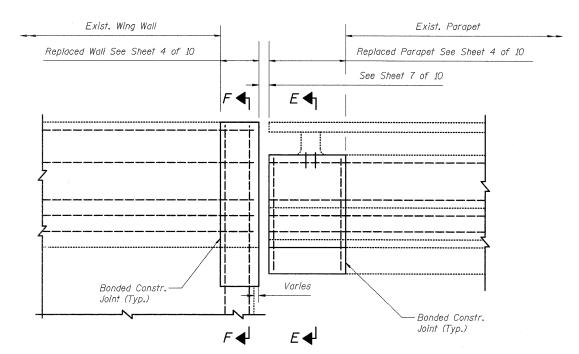
STRUCTURE NO. 081-0102

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHEET NO. 6
FAI 74	*	ROCK ISLAND		246	163	10 SHEETS
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PR	олест-		

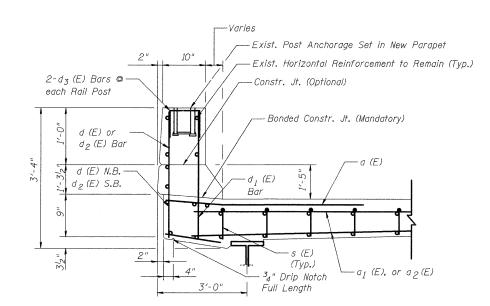
* 81 (1-2, 1, 2-2) RS-1 & M



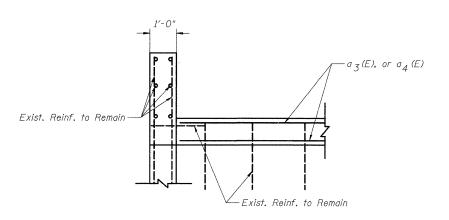
ELEVATION OF CONCRETE REMOVAL THRU PARAPET AND WING WALL



ELEVATION OF CONCRETE REPLACEMENT THRU PARAPET AND WING WALL



SECTION E-E THRU BRIDGE DECK PARAPET



SECTION F-F THRU ABUTMENT BACKWALL AND WING WALL

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

Hatched areas indicate concrete sections to be removed and replaced. Perimeters of concrete removal areas shall be saw cut $\frac{3}{4}$ " prior to the removal of concrete.

PARAPET AND RETAINING WALL DETAILS F.A.I. 74 (OVER 12TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0102

ROUTE NO. TOTAL SHEET NO. ROCK ISLAND 246 164

SHEET NO. 7

Notes:

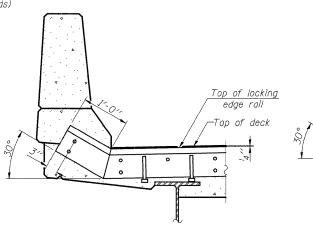
* 81 (1-2, 1, 2-2) RS-1 & M

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

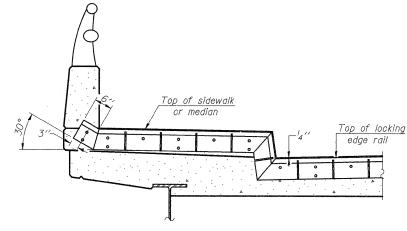
The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.



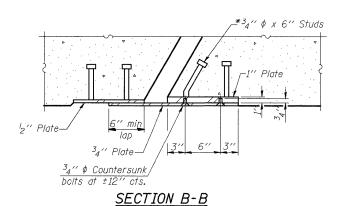




AT SIDEWALK OR MEDIAN

Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

TYPICAL END TREATMENTS



BILL OF MATERIAL

	Unit	lotal
Preformed Joint Strip Seal	Foot	86

PREFORMED JOINT STRIP SEAL

F.A.I. 74 (OVER 12TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0102

Strip seal-Locking edge rail-Top of slab at 1'-0'' cts. at 2'-0" cts. Anchor plate
Place plates at 1'-0'' cts. $7_{16}^{\prime\prime}$ ϕ holes at 4'-0" cts. for $3_8^{\prime\prime}$ ϕ (alt. with top horizontal studs) bolts. All bolts shall be burned, sawed, or chipped off flush with the plates

SECTION THRU WELDED RAIL JOINT

after forms are removed, typ. SECTION THRU ROLLED RAIL JOINT

at 50° F

— Top of slab

at 2'-0" cts.

at 1'-0" cts. (alt. with

top horizontal studs).

cts.

*Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

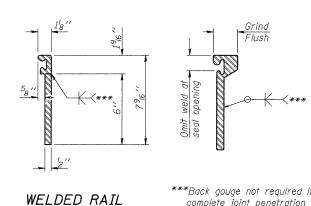
Locking edge rail-

 $^{7}_{16}$ " ϕ holes at 4'-0" cts. for $^{3}_{8}$ " ϕ

bolts. All bolts shall be burned, sawed,

or chipped off flush with the plates

Strip seal



***Back gouge not required if complete joint penetration is verified by mock-up.

after forms are removed, typ.

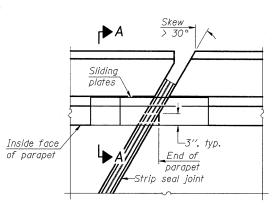
ANCHOR P

ROLLED EXTRUDED RAIL

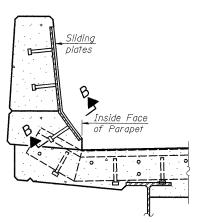
LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

LOCKING EDGE RAILS



PLAN



SECTION A-A

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM EJ-SSJ 5-16-08

POINT BLOCK DETAILS (for skews > 30°)



SHEET NO. 810 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

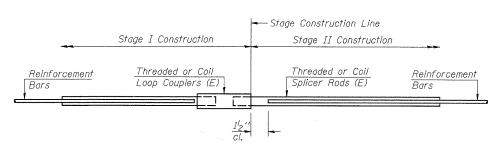
Minimum Capacity = $1.25 \times fy \times A_t$

Minimum *Pull-out Strength = 0.66 x fy x A_t (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

 \dot{A}_{t} = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

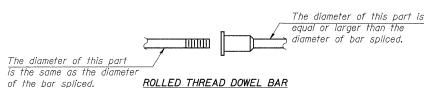
BAR SPLICER ASSEMBLIES Strength Requirements Bar Size to Splicer Rod or Min. Capacity | Min. Pull-Out Strength be Spliced Dowel Bar Length kips - tension kips - tension #4 1'-8'' 14.7 7.9 #5 2'-0" 12.3 23.0 #6 2'-7" 33.1 17.4 23.8 #7 3'-5" 45.1 58.9 #8 4'-6' 31.3 5'-9" 75.0 #9 39.6 7'-3" 95.0 #10 50.3 9'-0" 117.4 #11 61.8



STANDARD

Bar Size	No. Assemblies Required	Location
#5	8	Bridge Deck at South Abutment
#5	8	Bridge Deck at North Abutment
#6	3	South Abutment Backwall
#6	3	North Abutment Backwall

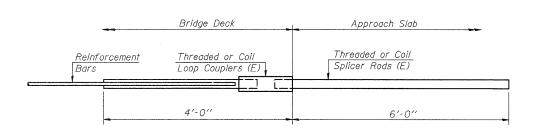
BAR SPLICER ASSEMBLY DETAILS F.A.I. 74 (OVER 12TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0102



** ONE PIECE -Wire Connector WELDED SECTIONS

BAR SPLICER ASSEMBLY ALTERNATIVES

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

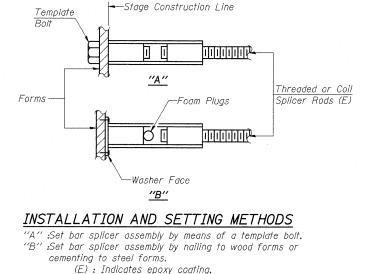


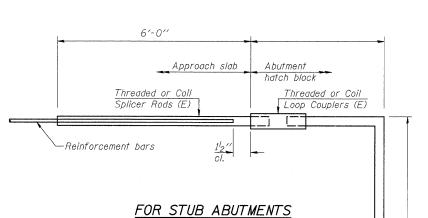
FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required =

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DEM

CHECKED BSD-1 5-16-08

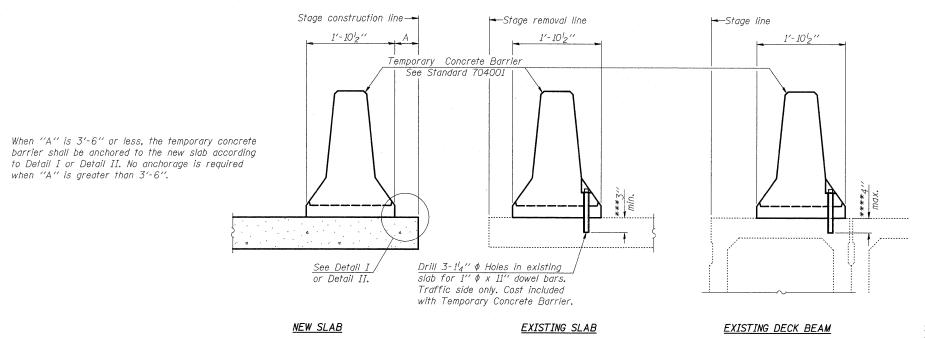




Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required =

	ROUTE NO.	SECTION	cou	anty	TOTAL SHEETS	SHEET ND.	SHEET NO. 9
	FAI 74	*	ROCK 1	SLAND	246	166	10 sheets
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PR	DJECT~			

* 81 (1-2, 1, 2-2) RS-1 & M



NOTES

Detail I - With Bar Splicer or Couplers:

Connect one (1) 1"x7"x10" steel P_c to the top layer of couplers with $2^{-5}8$ " ϕ bolts screwed to coupler at approximate Q_c of each barrier panel.

Detail II - With Extended Reinforcement Bars: Connect one (1) 1''x7''x10'' steel f_{-}^{p} to the concrete slab or concrete wearing surface with $2^{-5}g''\phi$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate f_{-}^{p} of each barrier panel.

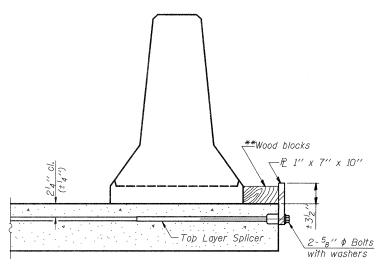
Cost of anchorage is included with Temporary Concrete Barrier.
The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

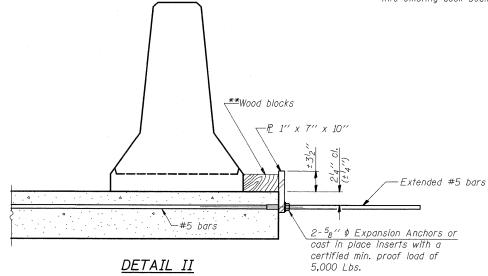
***Dimension shown is minimum required embedment into concrete.

If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

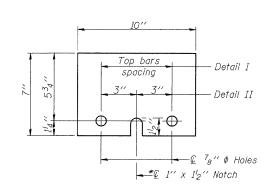
****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



<u>DETAIL I</u>



***Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



STEEL RETAINER P 1" x 7" x 10"

*Required only with Detail II

TEMPORARY CONCRETE BARRIER

FOR STAGE CONSTRUCTION

F.A.I. 74 (OVER 12TH AVE.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0102

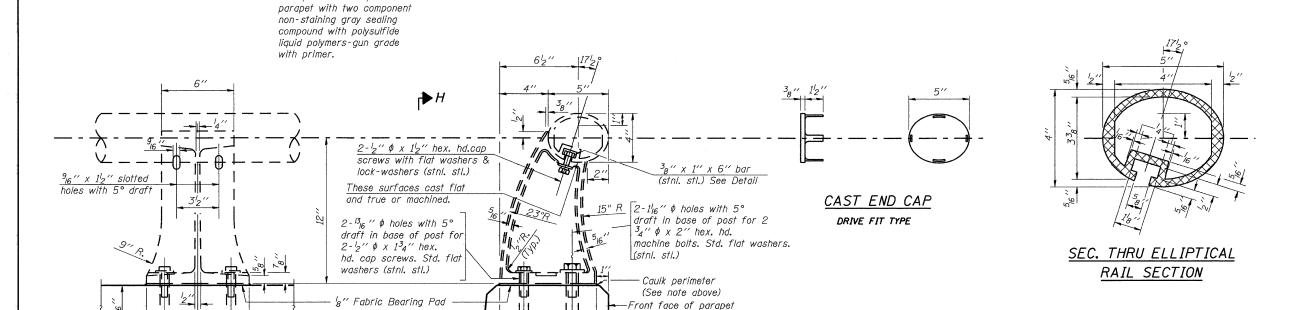
DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

R-27 5-16-08

FAI * ROCK ISLAND 246 1	67

HEET NO. 10 10 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



VIEW H-H

1"x³g" Bar 6" Lg. (Stnl. Stl.) Drill & tap 2 holes¹2"-13

SECTION G-G

RAIL POST DETAILS

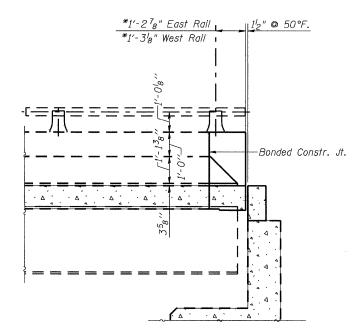
2-34" ø welded studs

drilled and tapped for

(1015, 1018, 1020) 38" x 14" x 7"

5"-13. AASHTO M 169

Seal perimeter of post to



INSIDE VIEW AT PROPOSED ABUTMENTS

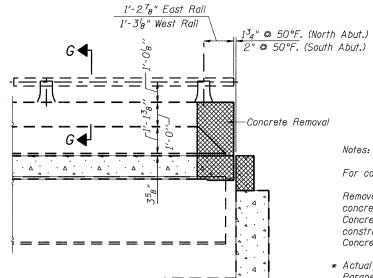
2-1" \$\phi\$ welded studs drilled and

tapped for $\frac{3}{4}$ "-10. ASTM

A108 (1020)

-12" x 112" x 7"

CLAMP BAR



Δ . . Δ .

For concrete removal limits See Sheet 4 of 10.

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

* Actual distance varies. New Joints are skewed through Parapet. Length of rail shall be adjusted as necessaary with a flush cut and the cast end cap shall be reused.

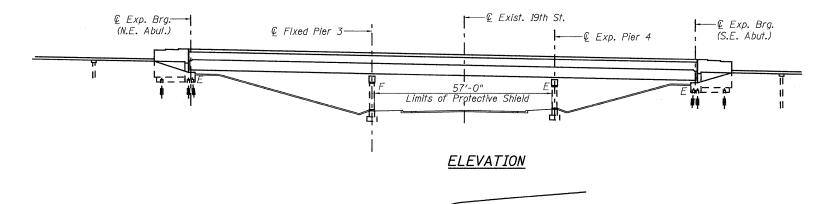
DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM INSIDE VIEW AT EXISTING ABUTMENTS

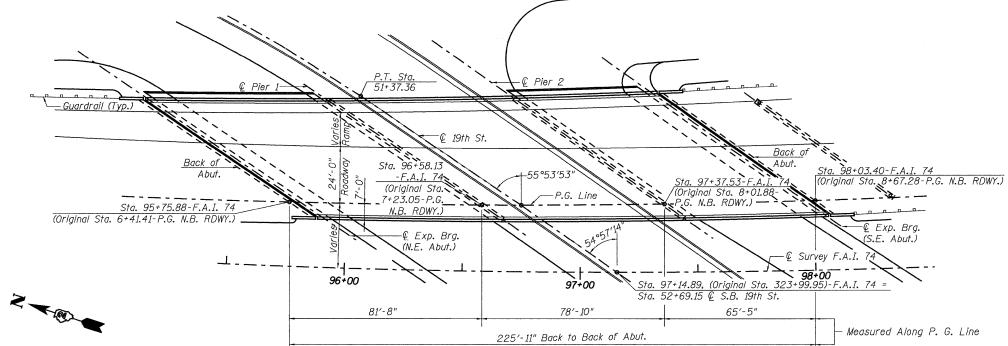
RAILING DETAILS F.A.I. 74 (OVER 12TH AVE.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0102



SHEET NO. 110 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M





PLAN

R-1-W PROJECT LOCATION

LOCATION PLAN

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars shall conform to the requirements of ASTM A706 Grade 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.

Exist. reinforcement bars extending into the removal areas shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with

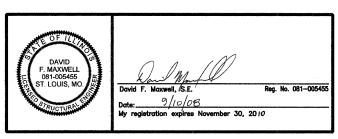
Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by an individual acceptable to the Engineer. Any cracks that can not be removed by grinding $\frac{1}{4}$ in. deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

Quantities of Deck Slab Repair (Full Depth, Type II), Deck Slab Repair (Full Depth, Type II) and approval bar splicer or anchorage system. Cost included with Concrete Removal. Deck Slab Repair (Partial) are approximated. Locations will be determined by the Resident Engineer following removal of the Concrete Deck Overlay. Actual repair locations shall be shown on the as-built plans. Contractor will be paid for the quantities furnished.

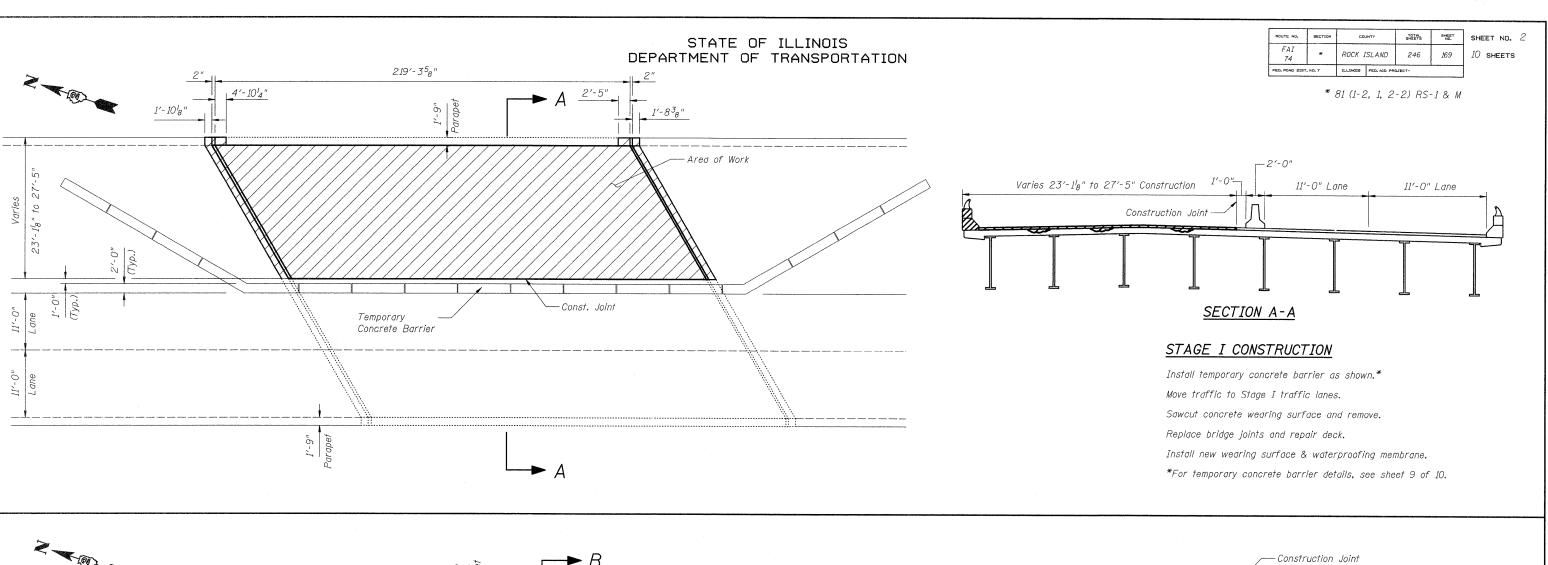
> Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured at an ambient temperature other than 50° F.

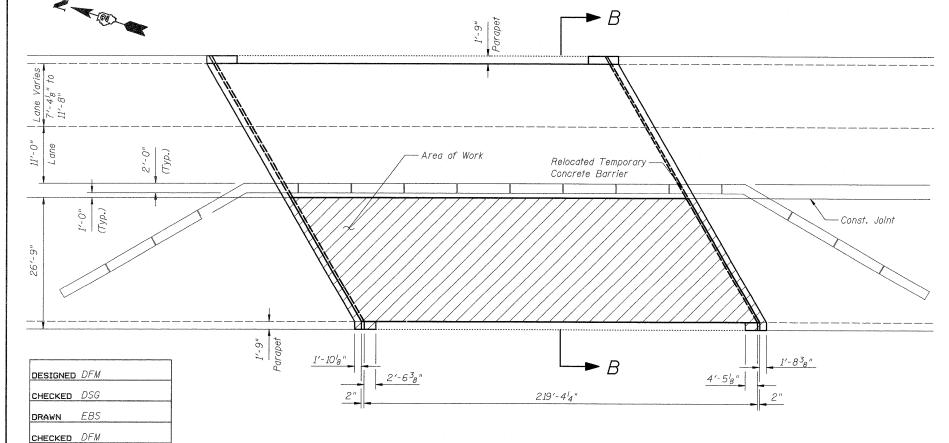


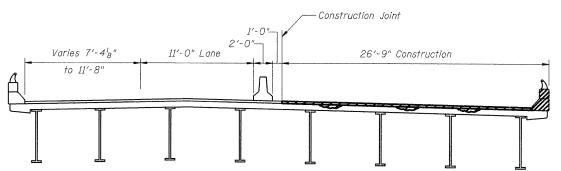
TOTAL BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N70	TON	122
Concrete Removal	CU.YD.	29.9
Protective Shield	SQ.YD.	305
Concrete Superstructure	CU.YD.	29.9
Reinforcement Bars, Epoxy Coated	POUND	2720
Bar Splicers	EACH	22
Preformed Joint Strip Seal	FOOT	184
Waterproofing Membrane System	SQ.YD.	1170
Bridge Deck Scarification	SQ.YD.	1160
Deck Slab Repair (Full Depth Type I)	SQ.YD.	<i>162.</i> 5
Deck Slab Repair (Full Depth Type II)	SQ.YD.	162.5
Deck Slab Repair (Partial)	SQ.YD.	120
Diamond Grinding (Bridge Section)	SO YD.	1170

PLAN AND ELEVATION F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0103







<u>SECTION B-B</u>

STAGE II CONSTRUCTION

Relocate temporary concrete barrier as shown.*

Move traffic to Stage II traffic lanes.

Remove concrete wearing surface.

Replace bridge joints and repair deck.

Install new wearing surface & waterproofing membrane.

Remove temporary concrete barrier.

STAGING PLAN
F.A.I. 74 (OVER 19TH ST.)

ROCK ISLAND COUNTY

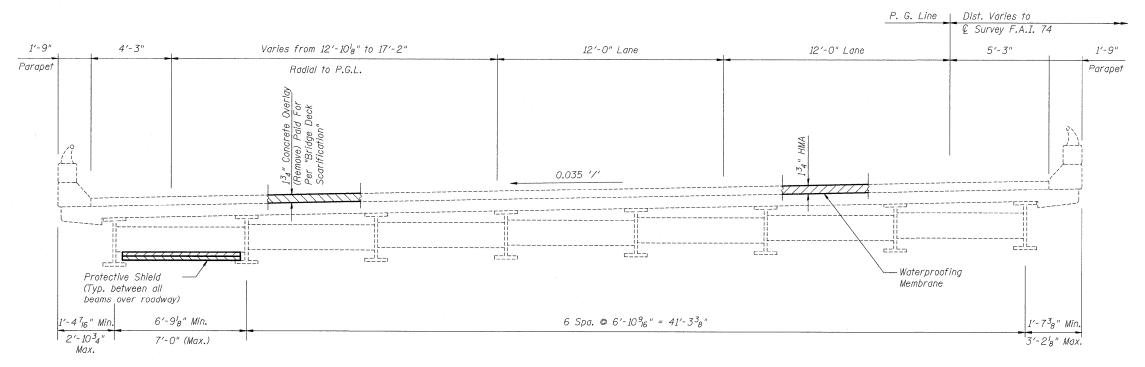
STRUCTURE NO. 081-0103

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 3
FAI 74	*	ROCK ISLAND	246	170	10 sheets
FED, ROAD DIST	ND. 7	ILLINOIS FED. AID F	PROJECT~		

* 81 (1-2, 1, 2-2) RS-1 & M

BILL OF MATERIAL

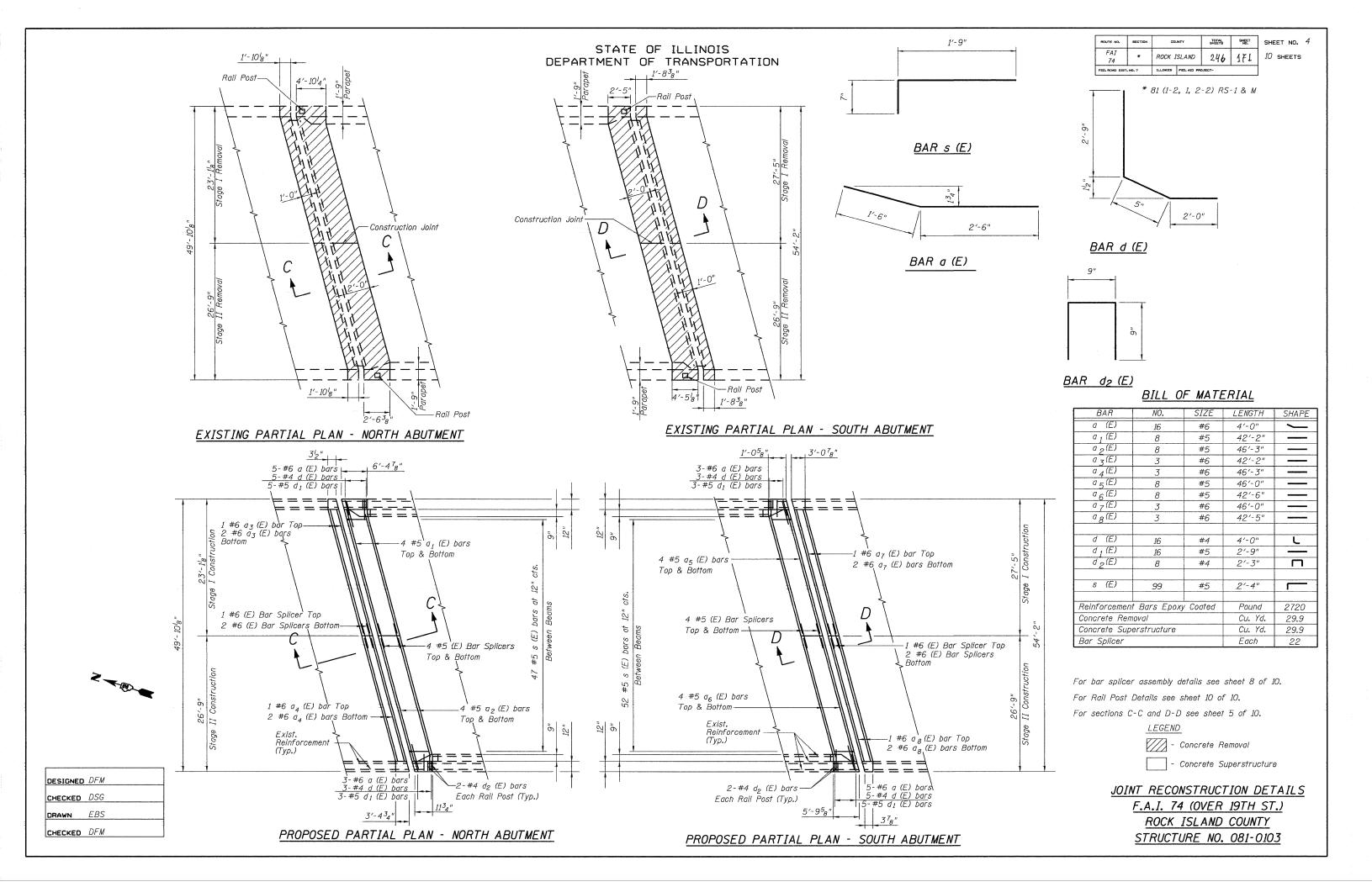
ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N90	TON	122
Protective Shield	SQ.YD.	305
Waterproofing Membrane System	SQ.YD.	1170
Bridge Deck Scarification	SQ.YD.	1160



DESIGNED DFM CHECKED DFM DRAWN EBS CHECKED DFM

CROSS SECTION (Looking South)

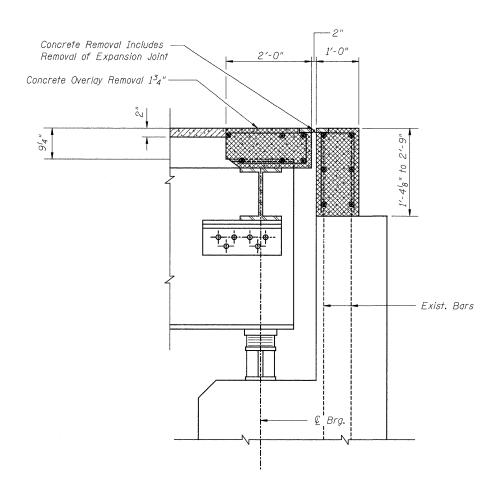
BRIDGE REPAIR DETAILS F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0103





10 sheets

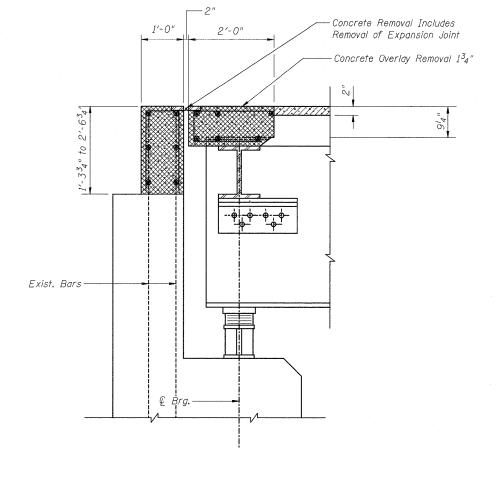
* 81 (1-2, 1, 2-2) RS-1 & M



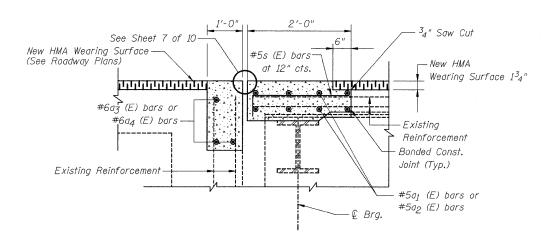
<u>LEGEND</u>

Concrete Removal

Bridge Deck Scarification



EXISTING SECTION C-C THRU NORTH ABUTMENT



PROPOSED SECTION C-C THRU NORTH ABUTMENT

2'-0"	111'-0"
3 ₄ " Saw Cut — 6".	See Sheet 7 of 10
New HMA#5s (E) ba	New HMA Wearing Surface
Wearing Surface 1^3_4 "— at 12" cts.	11 / 1 / (See Roadway Mains)
1 11111111/2020	-(-)
Existing Existing	#6a7 (E) bars or
Reinforcement // / / / / / / / / / / / / / / / / /	#6a ₈ (E) bars
1///	
Bonded Const. —/ //	
Joint (Typ.)	Existing Reinforcement
#5a ₅ (E) bars or	
#5a ₆ (E) bars ——	•
€ Brg.	

EXISTING SECTION D-D THRU SOUTH ABUTMENT

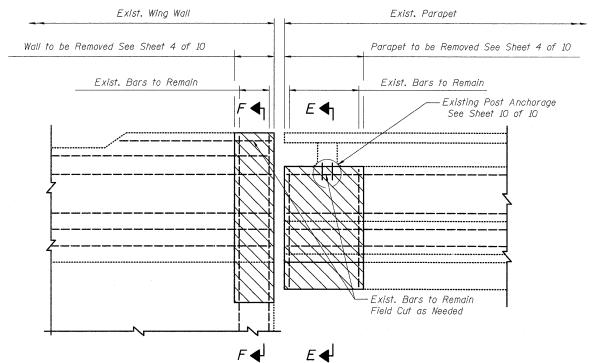
PROPOSED SECTION D-D THRU SOUTH ABUTMENT

o DFM
DFM
EBS
DFM
DFM EBS

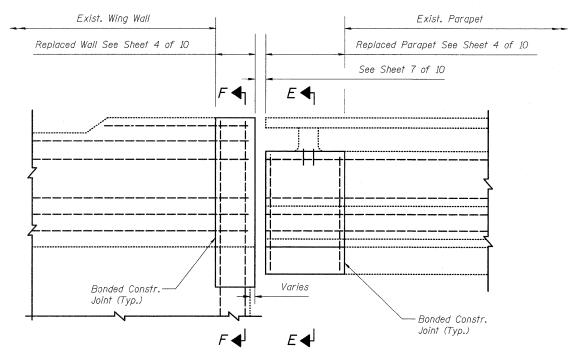
JOINT RECONSTRUCTION DETAILS F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0103

ROUTE NO.	SECTION	coi	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO.	6
FAI 74	*	* ROCK IS		246	173	10 SHEETS	
FEG. ROAD DIST	. NO. 7	ILLINOIS FED. AID PR		D.TECT~			

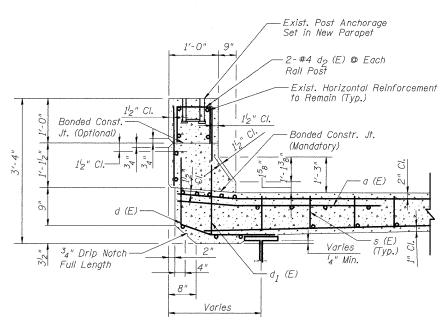
* 81 (1-2, 1, 2-2) RS-1 & M



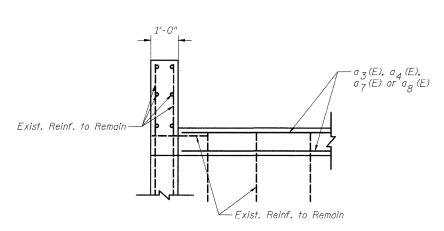
ELEVATION OF CONCRETE REMOVAL THRU PARAPET AND WING WALL



ELEVATION OF CONCRETE REPLACEMENT THRU PARAPET AND WING WALL



SECTION E-E THRU BRIDGE DECK PARAPET



SECTION F-F THRU ABUTMENT BACKWALL AND WING WALL

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

Note:

Hatched areas indicate concrete sections to be removed and replaced. Perimeters of concrete removal areas shall be saw cut $\frac{3}{4}$ " prior to the removal of concrete.

PARAPET AND RETAINING WALL DETAILS F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0103

_Top of slab

at 1'-0" cts.



SHEET NO. 7 10 SHEETS

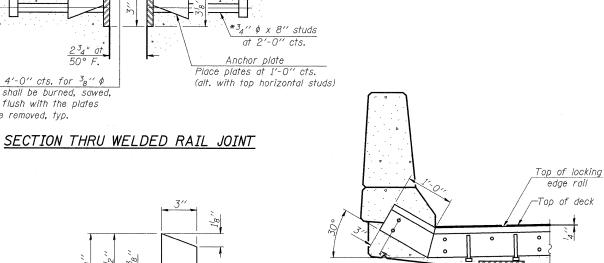
* 81 (1-2, 1, 2-2) RS-1 & M

The strip seal shall be made continuous and shall have a minimum thickness of ${}^{l}_{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

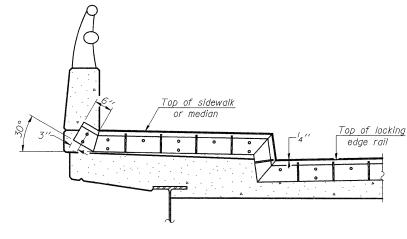
The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.



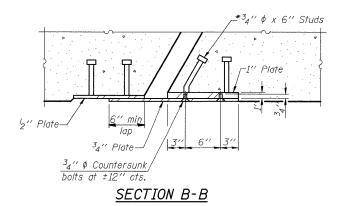




AT SIDEWALK OR MEDIAN

Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

TYPICAL END TREATMENTS



Item	Unit	Total
Preformed Joint Strip Seal	Foot	184

BILL OF MATERIAL

Strip seal Locking edge railat 50° F — Top of slab Cente. nt 1'-0" cts. at 2'-0" cts. at 1'-0" cts. (alt. with $^{7}_{16}$ " ϕ holes at 4'-0" cts. for $^{3}_{8}$ " ϕ top horizontal studs). bolts. All bolts shall be burned, sawed, or chipped off flush with the plates

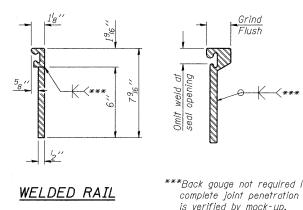
*Granular or solid flux filled headed studs

Specs., automatically end welded.

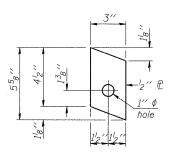
after forms are removed, typ.

conforming to Article 1006.32 of the Std.

SECTION THRU ROLLED RAIL JOINT



***Back gouge not required if is verified by mock-up.



Strip seal-

Locking edge rail-

 $\frac{7_{16}^{\prime\prime}}{\phi}$ holes at 4'-0'' cts. for $\frac{3}{8}$ '' ϕ bolts. All bolts shall be burned, sawed,

or chipped off flush with the plates

after forms are removed, typ.

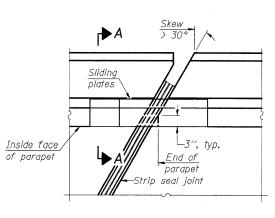
ANCHOR P

ROLLED EXTRUDED RAIL

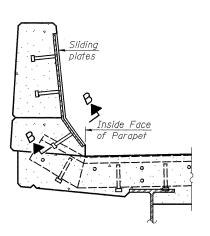
LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

LOCKING EDGE RAILS



PLAN



SECTION A-A

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM EJ-SSJ 5-16-08

POINT BLOCK DETAILS (for skews > 30°)

PREFORMED JOINT STRIP SEAL F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0103

Stage Construction Line

"A"

Template

Forms-



SHEET NO. 810 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

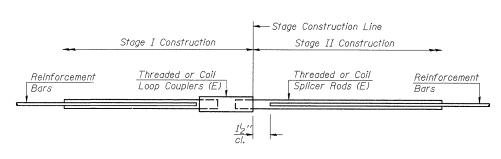
Minimum Capacity (Tension in kips) = 1.25 x fy x A_t

Minimum *Pull-out Strength = $0.66 \times fy \times A_t$ (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

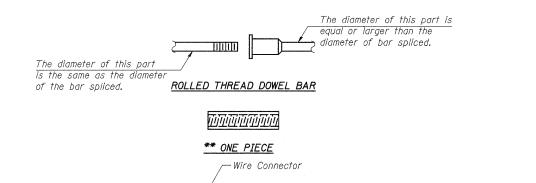
DAD COLICED ACCEUDITEC								
	BAR SPLICER ASSEMBLIES							
D C' /-	6 11 5 1	Strengt	h Requirements					
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min, Pull-Out Strength kips - tension					
#4	1'-8''	14.7	7.9					
#5	#5 2'-0'' #6 2'-7''	23.0	12.3					
#6		33.1	17.4					
#7	3′-5′′	45.1	23.8					
#8	4'-6''	<i>58.</i> 9	31.3					
#9	#9 5′-9′′		39.6					
#10	7′-3′′	95.0	50,3					
#11	9'-0''	117.4	61.8					



STANDARD

Bar Size	No. Assemblies Required	Location
#5	8	Bridge Deck at South Abutment
#5	8	Bridge Deck at North Abutment
#6	3	South Abutment Backwall
#6	3	North Abutment Backwall

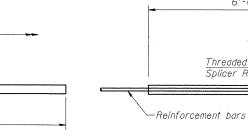
BAR SPLICER ASSEMBLY DETAILS F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0103



BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.





Threaded or Coil

Splicer Rods (E)

	Bridge Deck	Approach Slab	
Reinforcement Bars	Threaded or Coil Loop Couplers (E)	Threaded or Coil Splicer Rods (E)	
Name of the State	4'-0"	6′-0′′	

FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required =

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

5-16-08

BSD-1

Min. Capacity Min. Pull-out	= 23
Min. Pull-out	Stren
No. Required	=

"A" :Set b	ar splicer ar splicer	assembly to	y means of	METHODS a template bo	o/t.	
cemei		teel forms. cates epoxy	coating.			

-Foam Plugs

Threaded or Coil

Splicer Rods (E)

hatch block

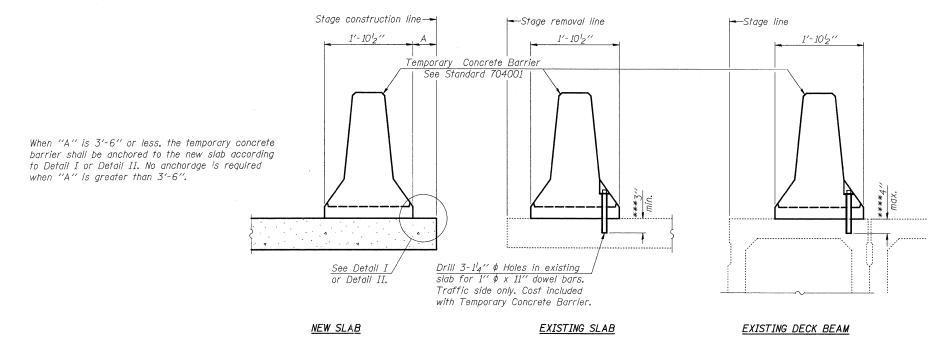
Threaded or Coil

Loop Couplers (E)

	Bar Splicer for ≠	‡5 bar
Min.	Capacity = 23.0 kips	- tension
Min.	Pull-out Strength = 12	.3 kips - tension
No.	Required =	

ROUTE NO.	SECTION	co	UNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 9
FAI 74	*	ROCK 1	SLAND	246	176	10 sheets
FEO. ROAD DIST, NO. 7		ILLINOIS	FED. AID PR	JECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



NOTES

Detail I - With Bar Splicer or Couplers:

Connect one (1) 1"x7"x10" steel $\mathbb R$ to the top layer of couplers with $2^{-5}8$ " ϕ bolts screwed to coupler at approximate $\mathbb Q$ of each barrier panel.

Detail II - With Extended Reinforcement Bars:

Connect one (1) 1''x7''x10'' steel № to the concrete slab or concrete wearing surface with 2-5₈'' ©

Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate ② of each parrier panel.

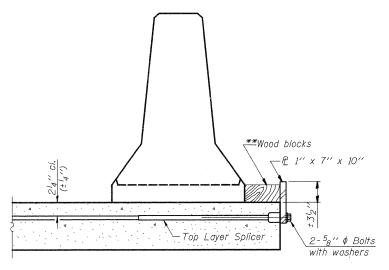
Cost of anchorage is included with Temporary Concrete Barrier. The $1^{\prime\prime}$ x $7^{\prime\prime}$ x $10^{\prime\prime}$ plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

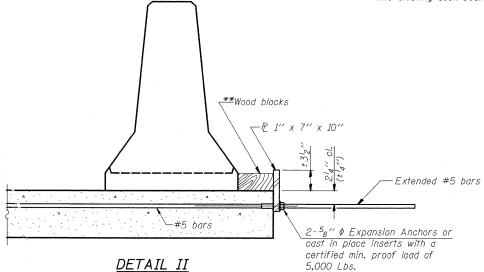
***Dimension shown is minimum required embedment into concrete.

If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

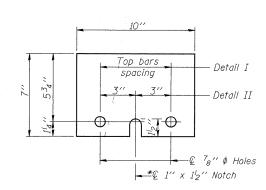
****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



<u>DETAIL I</u>



**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



STEEL RETAINER P 1" x 7" x 10"

*Required only with Detail II

TEMPORARY CONCRETE BARRIER

FOR STAGE CONSTRUCTION

F.A.I. 74 (OVER 19TH ST.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0103

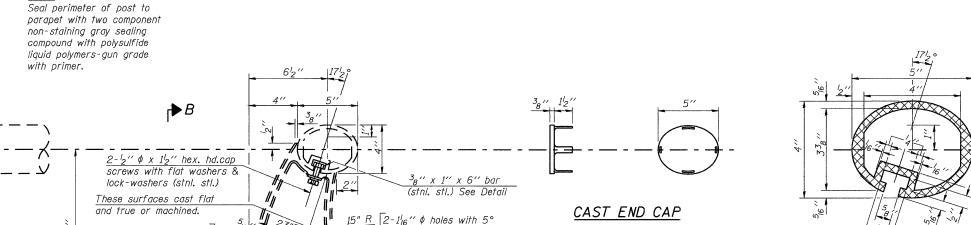
DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

R-27 5-16-08

DRIVE FIT TYPE

SHEET NO. 10

* 81 (1-2, 1, 2-2) RS-1 & M



draft in base of post for 2

machine bolts. Std. flat washers.

2-1" \$\phi\$ welded studs drilled and

tapped for $\frac{3}{4}$ "-10. ASTM

 $\frac{3}{4}$ " ϕ x 2" hex. hd.

Caulk perimeter (See note above)

Front face of parapet

A108 (1020)

- 2" x 12" x 7"

(stnl. stl.)

SEC. THRU ELLIPTICAL
RAIL SECTION

<u>VIEW B-B</u>

SECTION A-A

RAIL POST DETAILS

 2^{-13} ₁₆ " ϕ holes with 5° draft in base of post for

hd. cap screws. Std. flat washers (stnl. stl.)

Fabric Bearing Pad

 $2-\frac{3}{4}$ " ϕ welded studs

drilled and tapped for

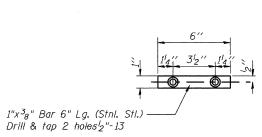
38" x 14" x 7"

 \mathbf{B}

(1015, 1018, 1020)

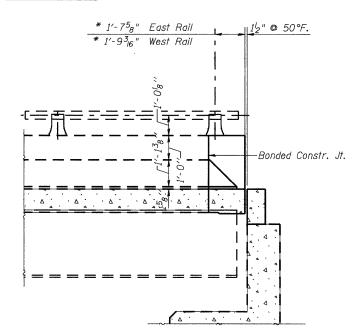
½"-13. AASHTO M 169

 $2 - \frac{1}{2}$ " $\phi \times 1^{3}$ 4" hex.

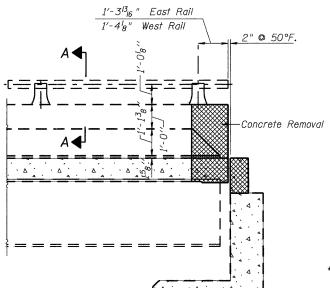


NOTE:

CLAMP BAR



INSIDE VIEW AT PROPOSED ABUTMENTS



INSIDE VIEW AT EXISTING ABUTMENTS

Notes:

For concrete removal limits See Sheet 4 of 10.

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

* Actual distance varies. New Joints are skewed through Parapet. Length of rail shall be adjusted as necessary with a flush cut and the cast end cap shall be reused.

RAILING DETAILS
F.A.I. 74 (OVER 19TH ST.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0103

DESIGNED DFM

CHECKED DSG

DRAWN EBS

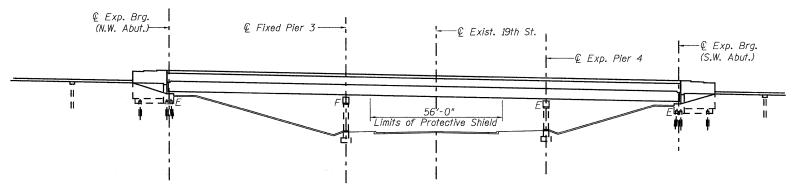
CHECKED DFM

9₁₆'' x 1¹2'' slotted holes with 5° draft



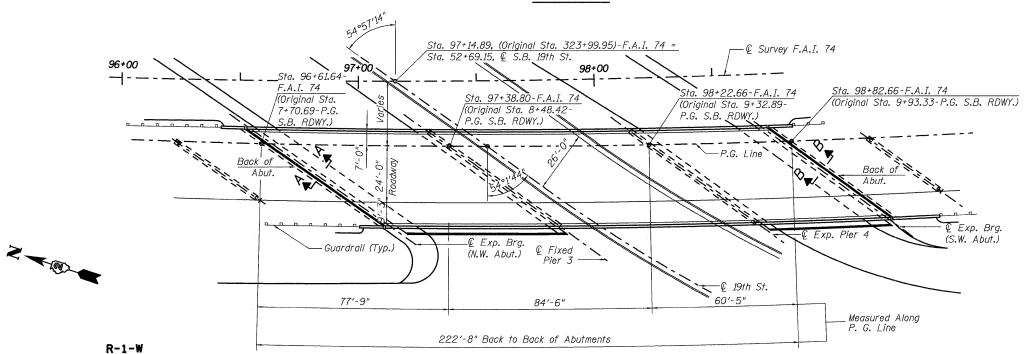
10 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



ELEVATION

PLAN



GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars shall conform to the requirements of ASTM A706 Grade 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.

Exist. reinforcement bars extending into the removal areas shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with approval bar splicer or anchorage system. Cost included with Concrete Removal.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by an individual acceptable to the Engineer. Any cracks that can not be removed by grinding $\frac{1}{4}$ in. deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

Quantities of Deck Slab Repair (Full Depth, Type I), Deck Slab Repair (Full Depth, Type II) and Deck Slab Repair (Partial) are approximated. Locations will be determined by the Resident Engineer following removal of the Concrete Deck Overlay. Actual repair locations shall be shown on the as-built plans. Contractor will be paid for the quantities furnished.

Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured at an ambient temperature other than 50° F.

0111	N
Soul Ma Lell	/
David F. Maxwell, S/E.	Reg. No. 08100545
Date: 9/10/08	
My registration expires November	30 2010

TOTAL BILL OF MATERIAL

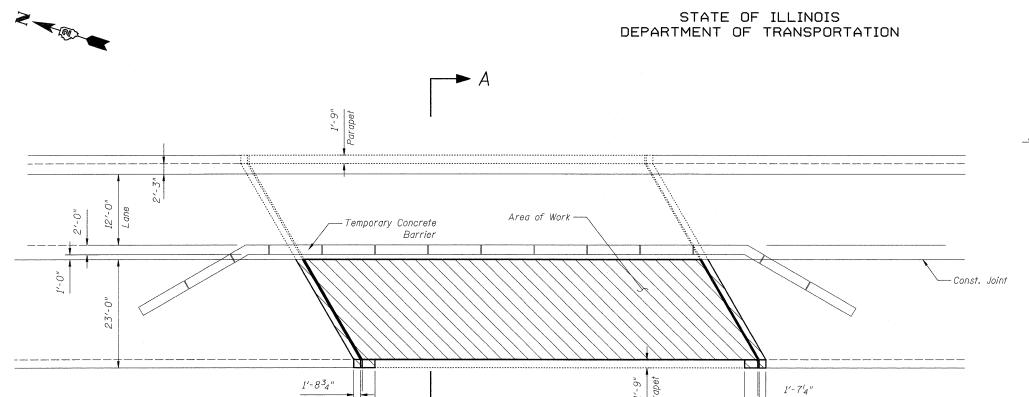
ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N70	TON	96
Concrete Removal	CU. YD.	21.8
Protective Shield	SQ. YD.	232
Concrete Superstructure	CU. YD.	21.8
Reinforcement Bars, Epoxy Coated	POUND	2100
Bar Splicers	EACH	22
Preformed Joint Strip Seal	F00T	139
Waterproofing Membrane System	SQ. YD.	924
Bridge Deck Scarification	SQ. YD.	917
Deck Slab Repair (Full Depth Type I)	SQ. YD.	25
Deck Slab Repair (Full Depth Type II)	SQ. YD.	25
Deck Slab Repair (Partial)	SQ. YD.	120
Diamond Grinding (Bridge Section)	SR YD.	924

PLAN AND ELEVATION F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0104

PARK 23RD 26TH AVE PROJECT LOCATION

LOCATION PLAN

DESIGNED	DFM	
CHECKED	DSG	
DRAWN	EBS	
CHECKED	DFM	

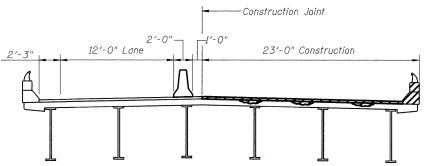


213'-778"

4'-178"

ROUTE NO.	SECTION	CO	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 2
FAI 74	*	ROCK ISLAND		246	179	10 SHEETS
FED. ROAD DIST, NO. 7		ILLINOIS	FED. AID PRI	DJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M



SECTION A-A

STAGE I CONSTRUCTION

Install temporary concrete barrier as shown.*

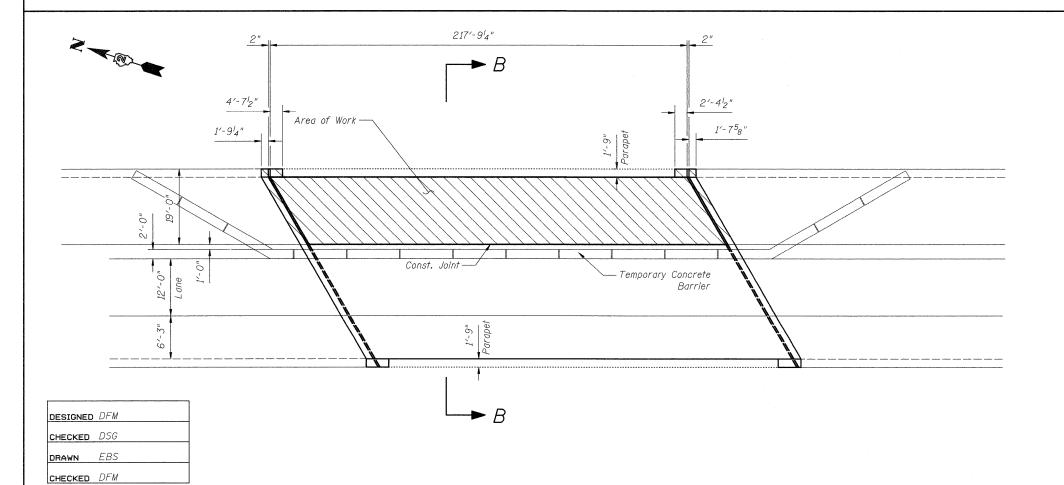
Move traffic to Stage I traffic lanes.

Sawcut concrete wearing surface and remove.

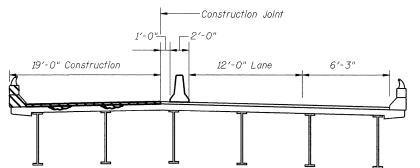
Replace bridge joints and repair deck.

Install new wearing surface & waterproofing membrane.

*For temporary concrete barrier details, see sheet 9 of 10.



2'-5³8"



SECTION B-B

STAGE II CONSTRUCTION

Relocate temporary concrete barrier as shown.*

Move traffic to Stage II traffic lanes.

Remove concrete wearing surface.

Replace bridge joints and repair deck.

Install new wearing surface & waterproofing membrane.

Remove temporary concrete barrier.

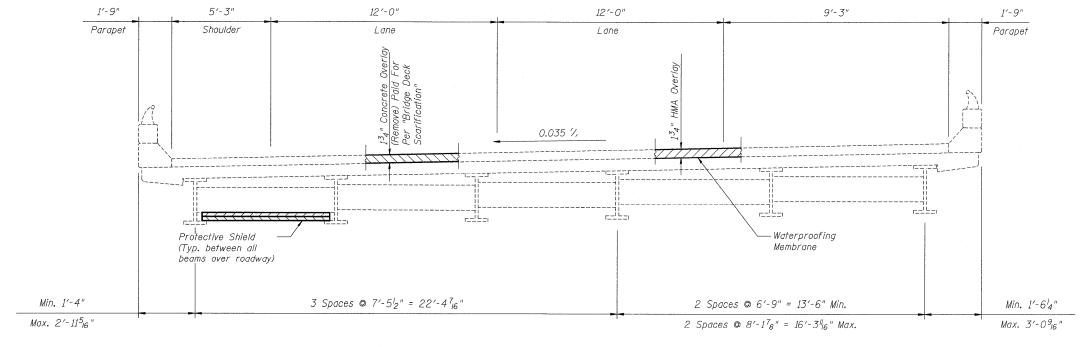
STAGING PLAN F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0104

ROUTE NO.	SECTION	co	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 3
FAI 74	*	ROCK I	SLAND	246	180	10 SHEETS
END BOAD DIES	. NO 7	TI I THOTO	EED AVD BB	O VECT-		

* 81 (1-2, 1, 2-2) RS-1 & M

BILL OF MATERIAL

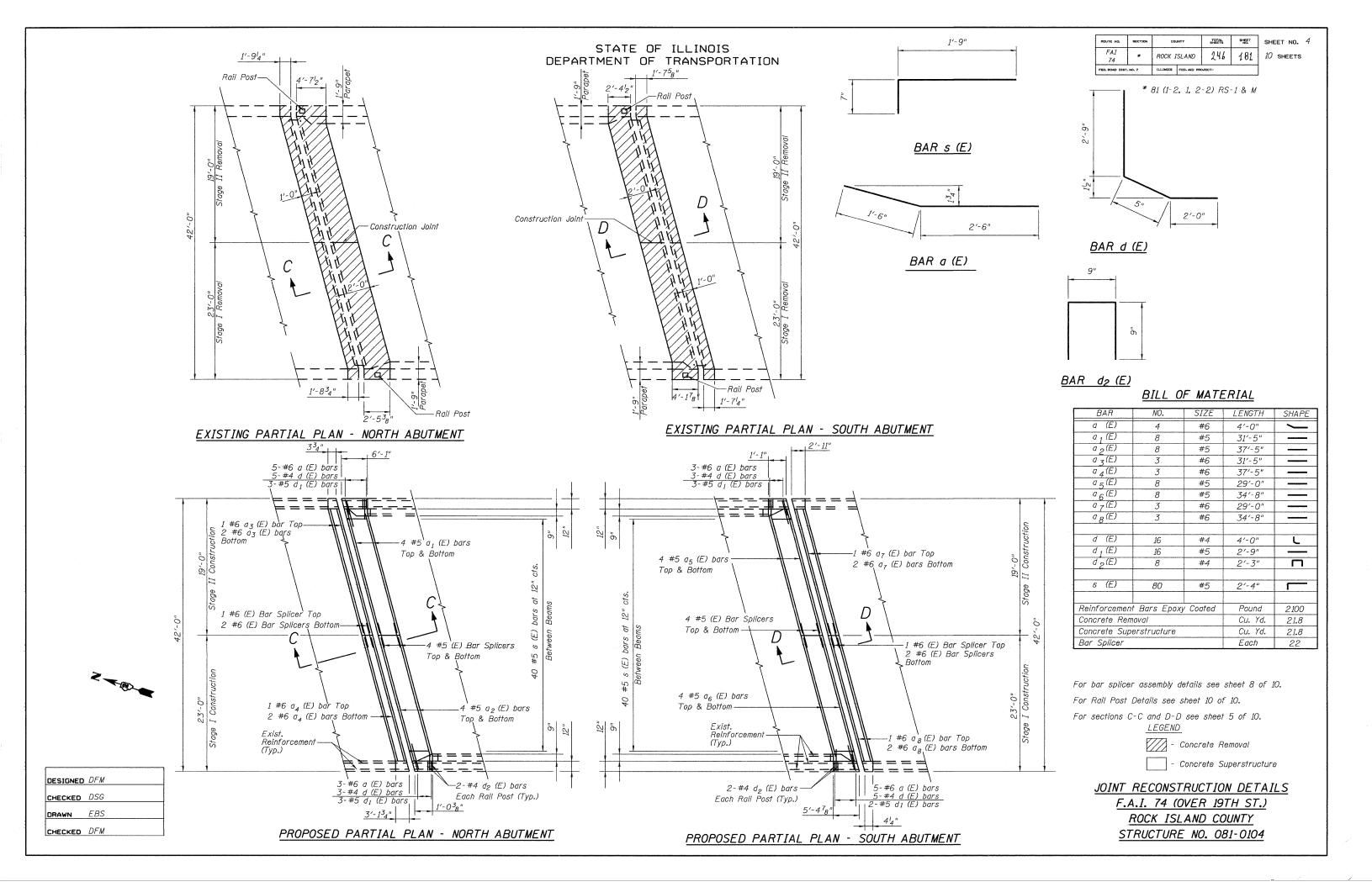
ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N90	TON	96
Protective Shield	SQ.YD.	232
Waterproofing Membrane System	SQ.YD.	924
Bridge Deck Scarification	SQ.YD.	917



DESIGNED DFM CHECKED DFM DRAWN EBS CHECKED DFM

CROSS SECTION (Looking South)

BRIDGE REPAIR DETAILS F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0104



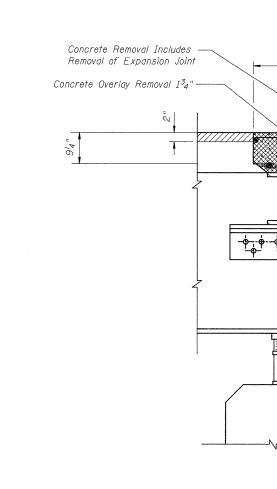
-Concrete Removal Includes

Removal of Expansion Joint

Concrete Overlay Removal 1^3_4 "

RGUTE NO.	SECTION	co	UNTY	TOTAL SHEETS	SHEET NO.	SHEET	NO.	5
FAI 74	*	ROCK ISLAND		246	182	<i>10</i> sн	EETS	
ED. ROAD DIST	, NO. 7	ILLINOIS	FED. AID PR	DJECT-				

* 81 (1-2, 1, 2-2) RS-1 & M



LEGEND

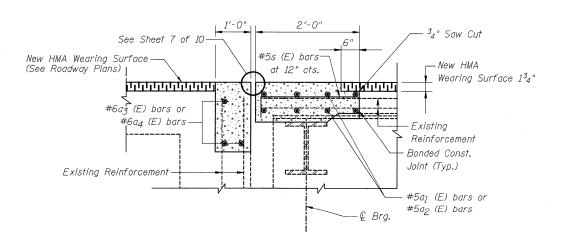
Concrete Removal

Bridge Deck Scarification

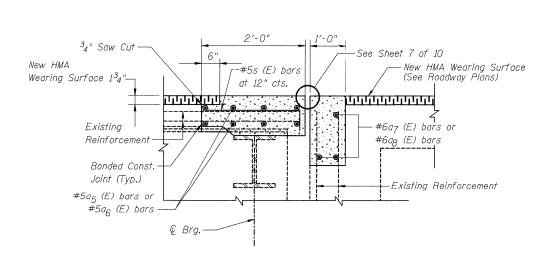
EXISTING SECTION C-C THRU NORTH ABUTMENT

Exist. Bars —

2'-0"



PROPOSED SECTION C-C THRU NORTH ABUTMENT



EXISTING SECTION D-D THRU SOUTH ABUTMENT

1'-0"

— Exist. Bars

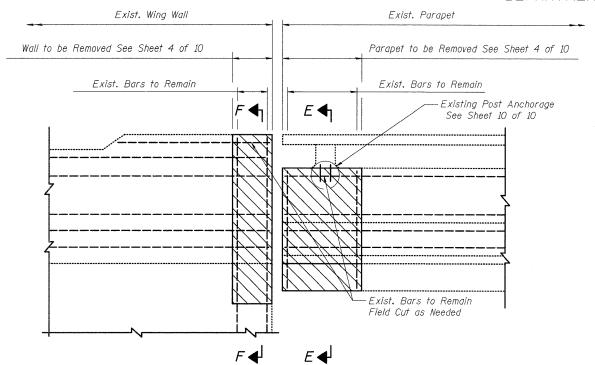
PROPOSED SECTION D-D THRU SOUTH ABUTMENT

JOINT RECONSTRUCTION DETAILS F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0104

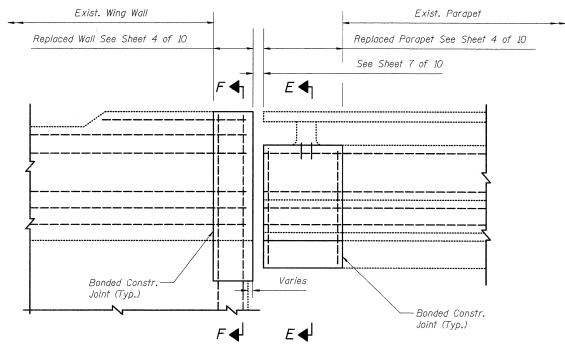
Γ	· · · · · · · · · · · · · · · · · · ·
DESIGNED	DFM
CHECKED	DFM
DRAWN	EBS
CHECKED	DFM

ROUTE NO.	SECTION	ca	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 6
FAI 74	*	ROCK 1	SLAND	246	183	10 sheets
FED. ROAD DIST, NO. 7		ILLINOIS	FED. AID PR	OJECY-		

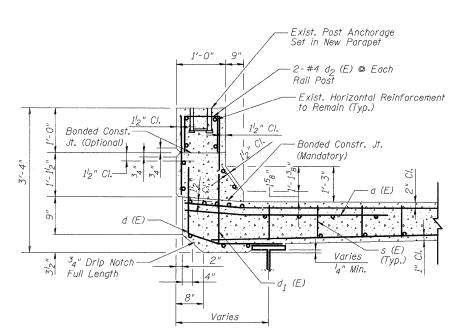
* 81 (1-2, 1, 2-2) RS-1 & M



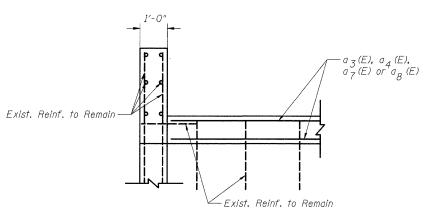
ELEVATION OF CONCRETE REMOVAL THRU PARAPET AND WING WALL



ELEVATION OF CONCRETE REPLACEMENT THRU PARAPET AND WING WALL



SECTION E-E THRU BRIDGE DECK PARAPET



SECTION F-F THRU ABUTMENT BACKWALL AND WING WALL

PRINCES	
DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

Note:

Hatched areas indicate concrete sections to be removed and replaced. Perimeters of concrete removal areas shall be saw cut 3_4 " prior to the removal of concrete.

PARAPET AND RETAINING WALL DETAILS

F.A.I. 74 (OVER 19TH ST.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0104

_Top of slab

*3₄′′ ∮ x 8′′ studs

at 1'-0" cts.

Anchor plate

Place plates at 1'-0'' cts.

at 2'-0" cts.

Strip seal—

SECTION THRU WELDED RAIL JOINT

ANCHOR P

Locking edge rail-

 $\frac{7_{16}}{''}$ ϕ holes at 4'-0" cts. for $\frac{3}{8}$ " ϕ bolts. All bolts shall be burned, sawed,

or chipped off flush with the plates

after forms are removed, typ.



SHEET NO. 7 10 SHEETS

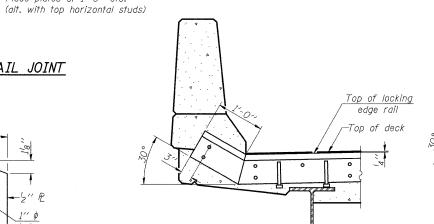
* 81 (1-2, 1, 2-2) RS-1 & M

The strip seal shall be made continuous and shall have a minimum thickness of \(\frac{1}{4}'' \). The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

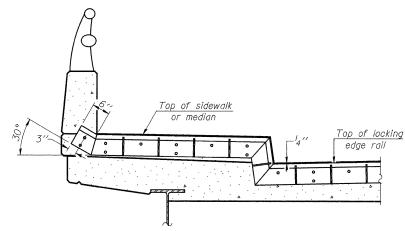
The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.





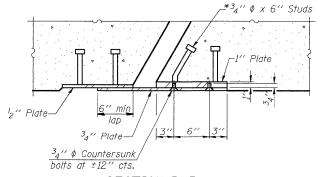


AT SIDEWALK OR MEDIAN

Shorter plates with a single row of studs at 12" cts, may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

Preformed Joint Strip Seal

TYPICAL END TREATMENTS



SECTION B-B

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

5-16-08

EJ-SSJ

*Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

Locking edge rail-

Cente.

 $\frac{7_{16}{''}}{}$ \$\phi\$ holes at 4'-0'' cts. for $\frac{3}{8}$ '' \$\phi\$ bolts. All bolts shall be burned, sawed,

ROLLED EXTRUDED RAIL

or chipped off flush with the plates

after forms are removed, typ.

Strip seal -

at 50° F

SECTION THRU ROLLED RAIL JOINT

— Top of slab

-0" cts.

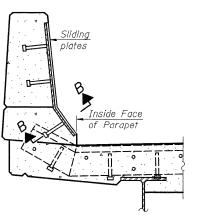
at 2'-0" cts.

*3₄'' \(\psi \) x 8'' studs

at 1'-0" cts. (alt. with

top horizontal studs).

plates L3", typ. Inside face of parapet End of parapet trip seal joint PLAN



SECTION A-A

POINT BLOCK DETAILS (for skews > 30°)

PREFORMED JOINT STRIP SEAL F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0104

BILL OF MATERIAL

Unit

Foot

Total

139

LOCKING EDGE RAIL SPLICE The inside of the locking edge

***Back gouge not required if

complete joint penetration

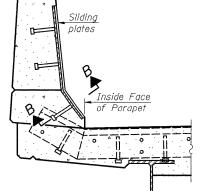
is verified by mock-up.

Flush

rail groove shall be free of weld residue.

LOCKING EDGE RAILS

WELDED RAIL





10 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

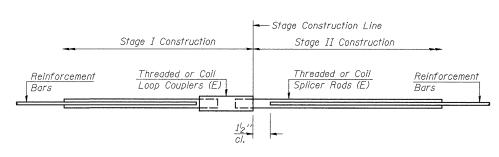
Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- Minimum Capacity (Tension in kips) = 1.25 x fy x A_t
- Minimum *Pull-out Strength = 0.66 x fy x A_t (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

- A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

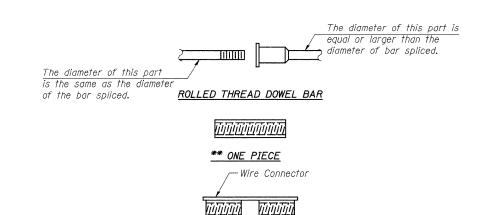
	BAR SPLICER ASSEMBLIES								
		Strength Requirements							
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension						
#4	1'-8''	14.7	7.9						
#5	2'-0"	23.0	12.3						
#6	2'-7"	33.1	17.4						
#7	3′-5″	45.1	23.8						
#8	4′-6′′	58.9	31.3						
#9	5′-9′′	75.0	39.6						
#10	7′-3′′	95.0	50.3						
#11	9'-0''	117.4	61.8						



STANDARD

Bar Size	No. Assemblies Required	Location
#5	8	Bridge Deck at South Abutment
#5	8	Bridge Deck at North Abutment
#6	3	South Abutment Backwall
#6	3	North Abutment Backwall

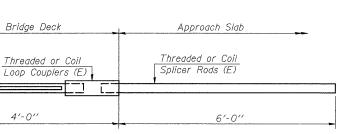
BAR SPLICER ASSEMBLY DETAILS F.A.I. 74 (OVER 19TH ST.) ROCK ISLAND COUNTY STRUCTURE NO. 081-0104



BAR SPLICER ASSEMBLY ALTERNATIVES

WELDED SECTIONS

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

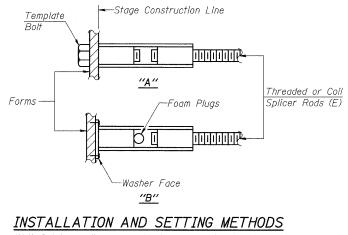
Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required =

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
	DEM

CHECKED DFM BSD-1 5-16-08

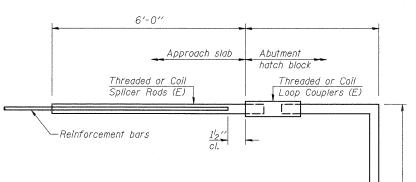
<u>Reinforcement</u>

Bars



"A" :Set bar splicer assembly by means of a template bolt. "B" :Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



FOR STUB ABUTMENTS

	Bar Splicer for #5 bar
Min.	Capacity = 23.0 kips - tension
Min.	Pull-out Strength = 12.3 kips - tension
No.	Required =

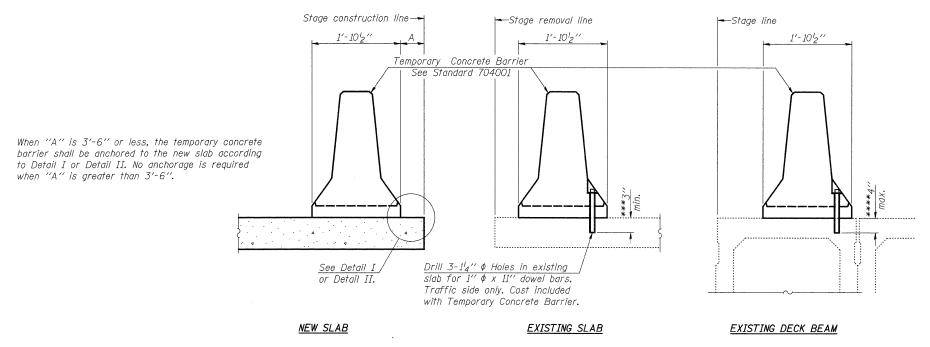
**Wood blocks

─#5 bars

DETAIL II

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO.
FAI 74	*	ROCK ISLAND	246	186	10 SHEETS

* 81 (1-2, 1, 2-2) RS-1 & M



NOTES

Detail I - With Bar Splicer or Couplers:

Connect one (1) 1"x7"x10" steel P_c to the top layer of couplers with $2^{-5}8$ " ϕ bolts screwed to coupler at approximate Q_c of each barrier panel.

Detail II - With Extended Reinforcement Bars: Connect one (1) 1"x7"x10" steel ℓ to the concrete slab or concrete wearing surface with $2^{-5}8''\phi$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate ℓ of each barrier panel.

Cost of anchorage is included with Temporary Concrete Barrier.

The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

***Dimension shown is minimum required embedment into concrete.

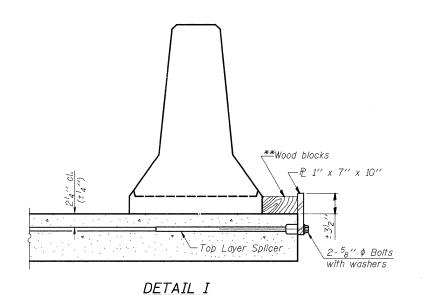
If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.

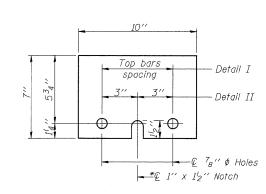
Extended #5 bars

 $2^{-5}8''$ ϕ Expansion Anchors or cast in place inserts with a certified min. proof load of

5.000 Lbs.



**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



STEEL RETAINER P 1" x 7" x 10"

*Required only with Detail II

TEMPORARY CONCRETE BARRIER
FOR STAGE CONSTRUCTION
F.A.I. 74 (OVER 19TH ST.)
ROCK ISLAND COUNTY
STRUCTURE NO. 081-0104

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

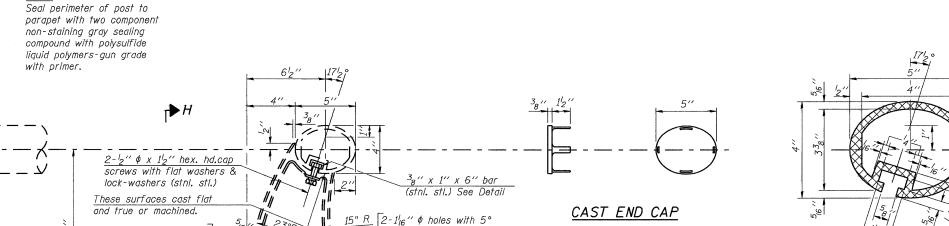
R-27 5-16-08

DRIVE FIT TYPE

ROUTE NO.	SECTION	coi	UNTY	TOTAL SHEETS	SHEET NO.	SH
- FAI 74	*	ROCK 1	SLAND	246	187	10
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PR	DJECY-		

10 sheets

* 81 (1-2, 1, 2-2) RS-1 & M



draft in base of post for 2

machine bolts. Std. flat washers.

2-1" ϕ welded studs drilled and

tapped for $\frac{3}{4}$ "-10. ASTM

 3_{Δ} " ϕ x 2" hex. hd.

Caulk perimeter (See note above)

Front face of parapet

A108 (1020)

-12" x 112" x 7"

(stnl. stl.)

SEC. THRU ELLIPTICAL
RAIL SECTION

<u>VIE₩ H-H</u>

SECTION G-G

RAIL POST DETAILS

2-13₁₆ " \$\phi\$ holes with 5°

 $2^{-\frac{1}{2}''} \phi \times 1^{\frac{3}{4}''} \text{ hex.}$

draft in base of post for

hd. cap screws. Std. flat washers (stnl. stl.)

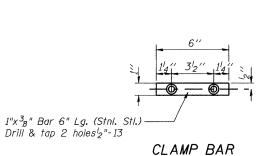
Fabric Bearing Pad

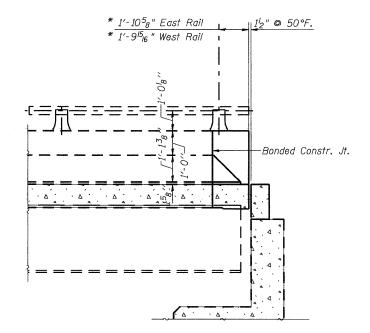
 $2^{-\frac{3}{4}''}$ ϕ welded studs

drilled and tapped for

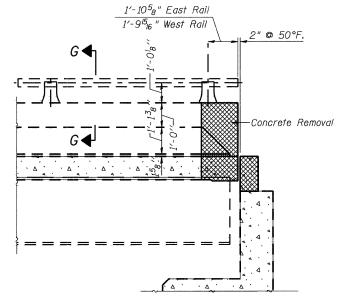
(1015, 1018, 1020) ³8″ x 1¹4″ x 7″

2"-13. AASHTO M 169





INSIDE VIEW AT PROPOSED ABUTMENT



INSIDE VIEW AT EXISTING ABUTMENT

Notes:

For concrete removal limits See Sheet 4 of 10.

Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. Cost included with Concrete Superstructure. If anchors are damaged during construction, they shall be replaced and are included with Concrete Removal.

* Actual distance varies. New Joints are skewed through Parapet. Length of rail shall be adjusted as necessary with a flush cut and the cast end cap shall be reused.

DESIGNED DFM

CHECKED DSG

DRAWN EBS

CHECKED DFM

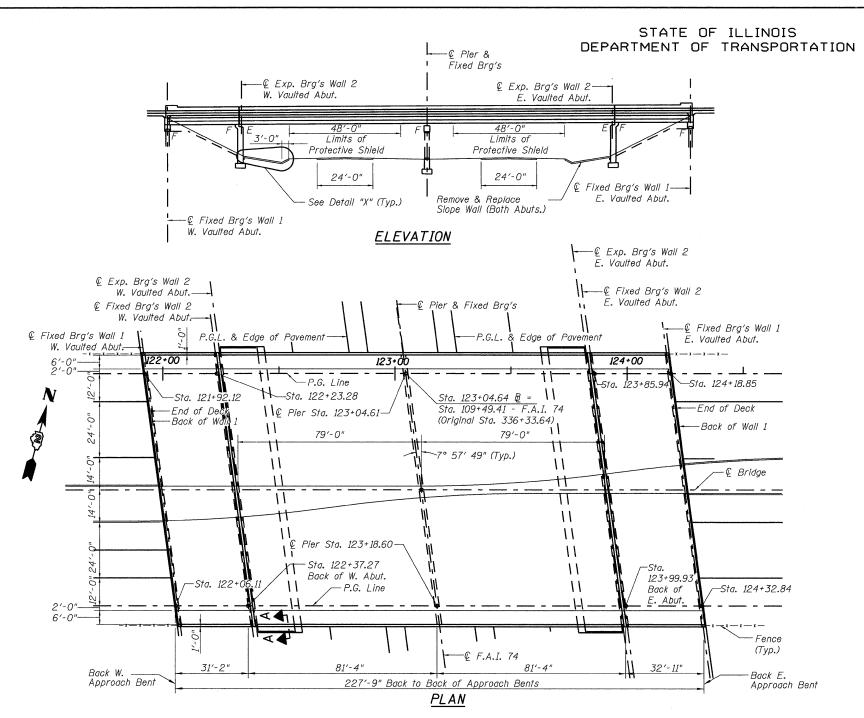
9₁₆" x 1¹2" slotted holes with 5° draft

RAILING DETAILS

F.A.I. 74 (OVER 19TH ST.)

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0104



DESIGNED DFM

CHECKED DSG

DRAWN EBS

CHECKED DFM

TOTAL BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N 90	TON	348
Concrete Removal	CU.YD.	27.3
Slope Wall Removal	SQ.YD.	554
Protective Shield	SQ.YD.	1179
Concrete Superstructure	CU.YD.	27.3
Reinforcement Bars, Epoxy Coated	POUND	3770
Bar Splicers	EACH	44
Slope Wall (4 inch)	SQ.YD.	554
Waterproofing Membrane System	SQ.YD.	2340
Silicone Joint Sealer	FOOT	234
Preformed Joint Strip Seal	FOOT	240
Structural Repair of Concrete (Depth >5")	SQ.FT.	162
Bridge Deck Scarification	SQ.YD.	2330
Deck Slab Repair (Full Depth Type II)	SQ.YD.	3
Deck Slab Repair (Partial Depth)	SQ.YD.	27
Diamond Grinding (Bridge Section)	SQ. YD	2340



* 81 (1-2, 1, 2-2) RS-1 & M

GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Reinforcement bars shall conform to the requirements of ASTM A706 Grade 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

Quantities of Deck Slab Repair (Full Depth, Type II), and Deck Slab Repair (Partial Depth) are approximated and are intended for sidewalk only. Locations along the sidewalk will be determined by the resident Engineer. Actual repair locations shall be shown on the as-built plans.

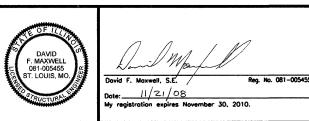
The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured of an ambient temperature other than 50° F.

Existing reinforcement bars extending into the removal areas shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with approved bar splicer or anchorage system. Cost included with concrete removal.

Note:

For Section A-A, Detail "X" & Bridge Sidewalk Repair see Sheet 3 of 11.

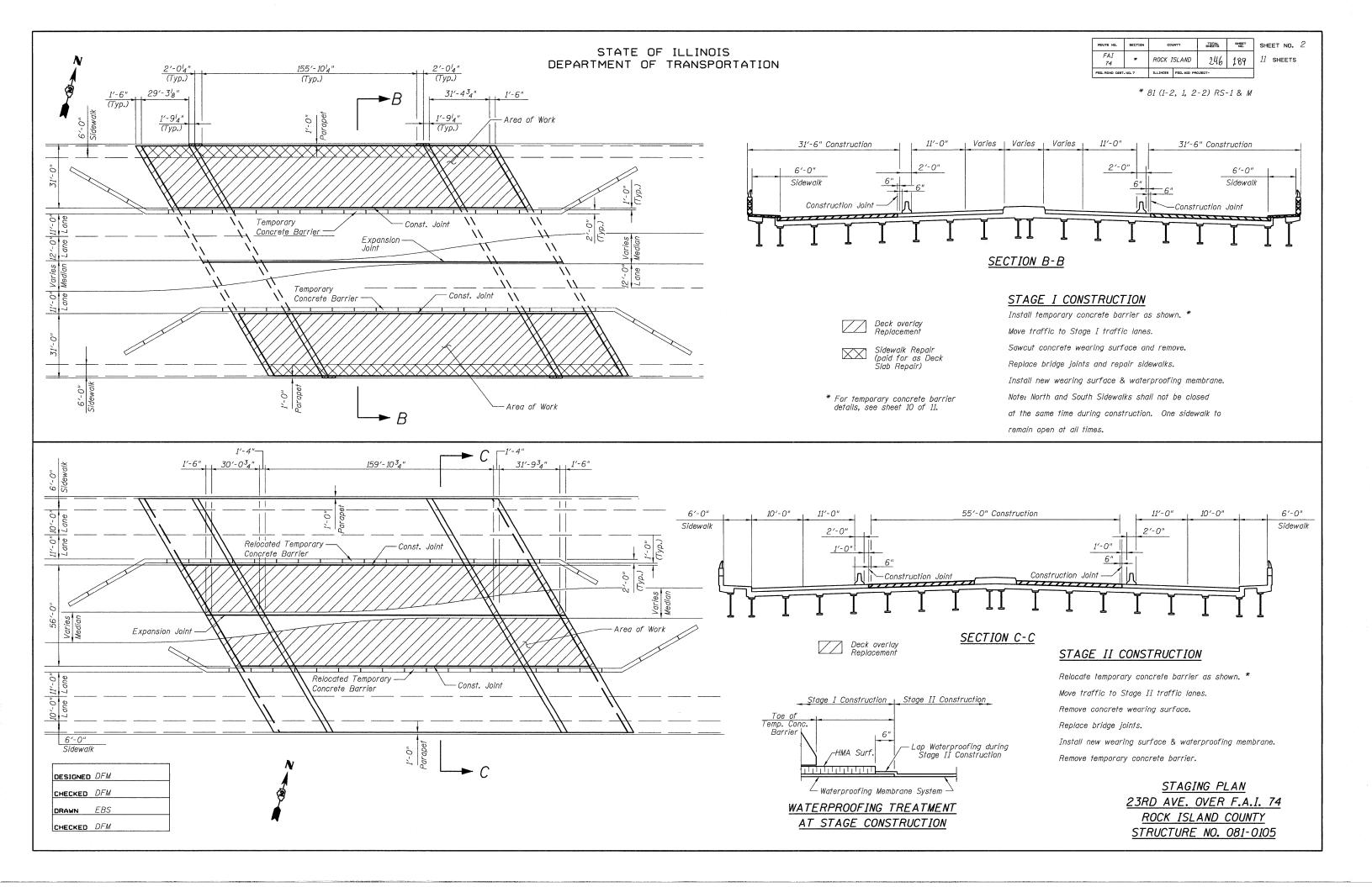


PLAN AND ELEVATION

23RD AVE. OVER F.A.I. 74

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0105



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 3
FAI 74	*	ROCK ISLAND	246	190	11 SHEETS
FED. ROAD DIST	NO. 7	ILLINOIS FED. AID PE	OJECT-		

* 81 (1-2, 1, 2-2) RS-1 & M

Note A:

Slopewall shall be reinforced with welded wire fabric, 6" x 6" - W 4.0 x W 4.0, weighing 58 pounds per 100 Sq. Ft., included in contract unit price for Slopewall (4 inch).

Quantities for sidewalk repair are approximate. Locations will be determined by the Resident Engineer during Stage I Construction. Actual repair locations shall be shown on as-built plans. Contractor will be paid for the quantity furnished.

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Polymerized HMA Surface Course Mix "E", N90	TON	348
Protective Shield	SQ. YD.	1179
Waterproofing Membrane System	SQ. YD.	2340
Bridge Deck Scarification	SQ. YD.	2330
Deck Slab Repair (Full Depth Type II)	SQ. YD.	3
Deck Slab Repair (Partial Depth)	SQ. YD.	27

See Note A

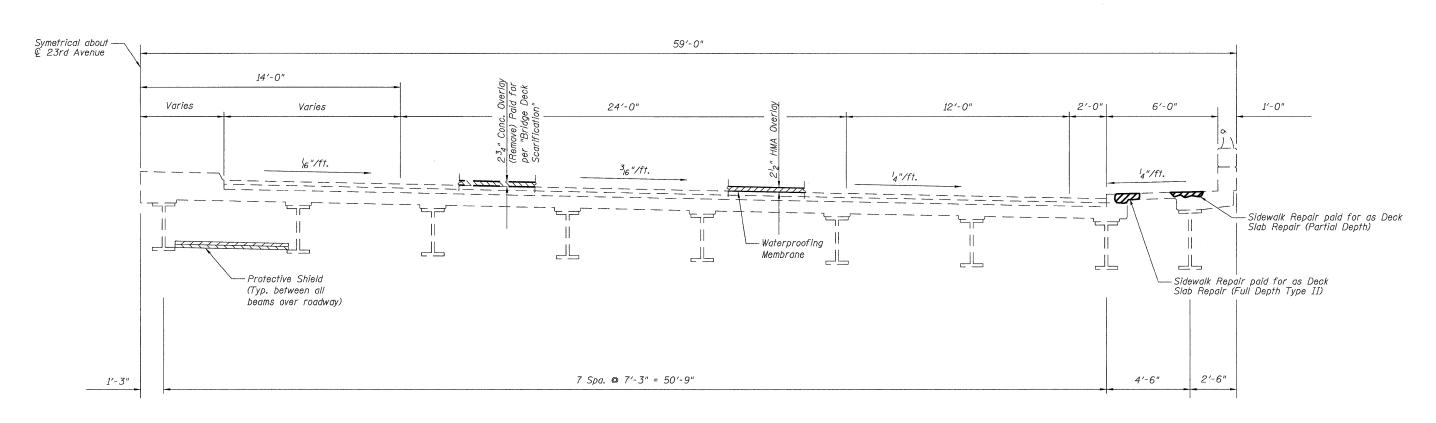
DETAIL "X"

CONCRETE SLOPE PROTECTION

SECTION A-A

Edge of Deck

2'-0"



HALF CROSS SECTION

(Looking West)

DESIGNED DFM

CHECKED DFM

DRAWN EBS

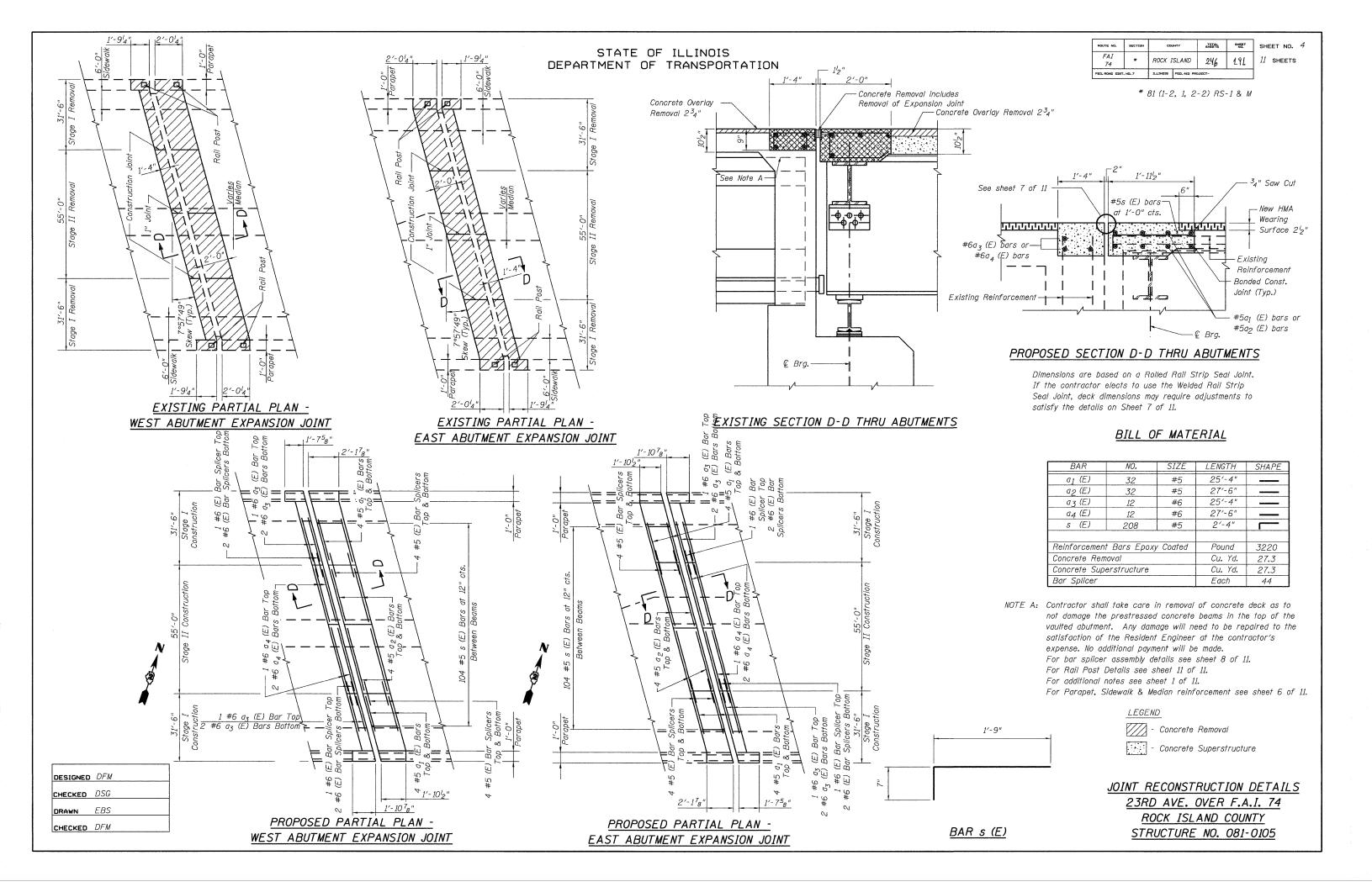
CHECKED DFM

BRIDGE REPAIR DETAILS

23RD AVE. OVER F.A.I. 74

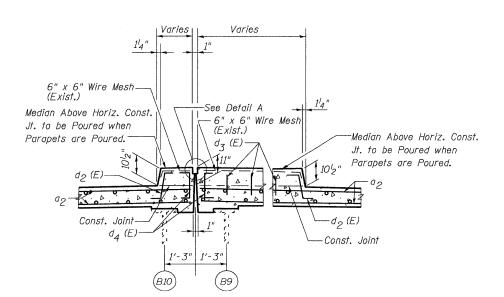
ROCK ISLAND COUNTY

STRUCTURE NO. 081-0105

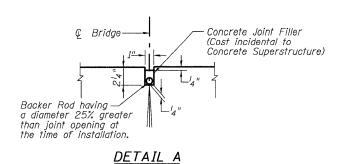


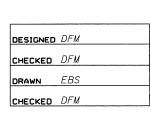
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NG,	SHEET NO. 5
FAI 74	*	ROCK ISLAND	246	192	11 SHEETS
FED. ROAD DIST. NO. 7		ILLINOIS FED. AID PR	DJECT~		

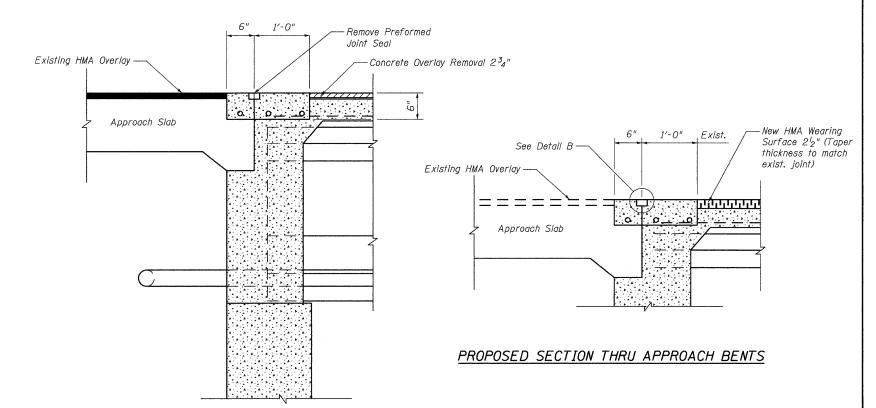
* 81 (1-2, 1, 2-2) RS-1 & M



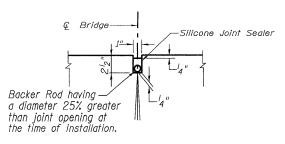
MEDIAN SECTION E-E







EXISTING SECTION THRU APPROACH BENT



DETAIL B

For additional notes see sheet 1 of 11.

LEGEND

- Concrete Removal

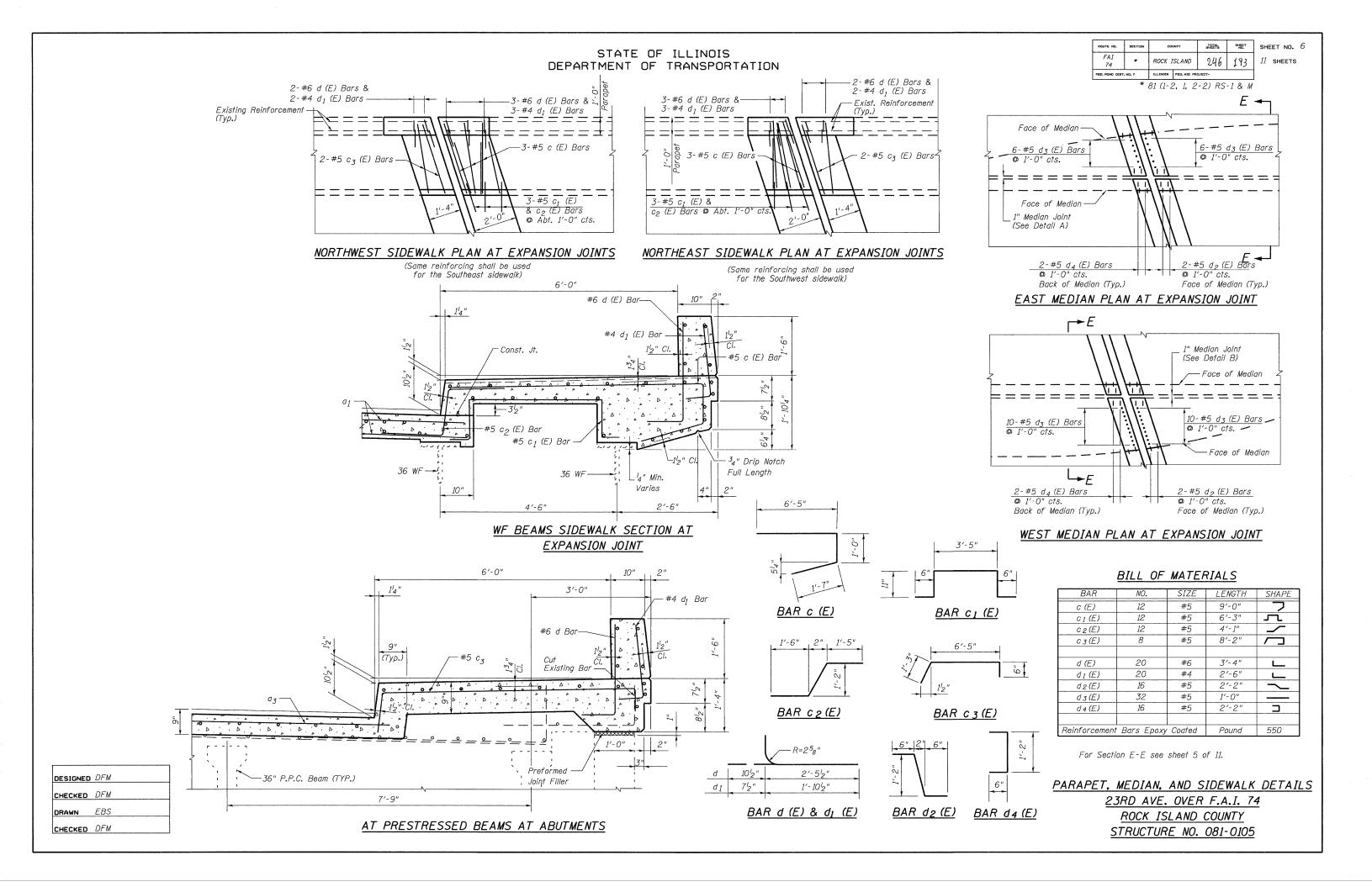
- Concrete Superstructure

JOINT RECONSTRUCTION DETAILS

23RD AVE. OVER F.A.I. 74

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0105



ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHE	EE.
FAI 74	*	ROCK ISLAND		246	194	11	s
FED. ROAD DIST	. NO. 7	ILLINOIS	FED. ALD PR	OJECT-			

T NO. 7 SHEETS

Notes:

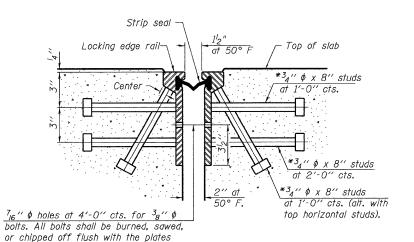
* 81 (1-2, 1, 2-2) RS-1 & M

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.



*Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

Strip seal- $\frac{1^{l_2}"}{at 50^{\circ} F}$ Locking edge rail-_Top of slab at 1'-0" cts. at 2'-0" cts. Anchor plate Place plates at 1'-0" cts. $^{7}_{16}$ " ϕ holes at 4'-0" cts. for $^{3}_{8}$ " ϕ (alt. with top horizontal studs) bolts. All bolts shall be burned, sawed,

SECTION THRU ROLLED RAIL JOINT

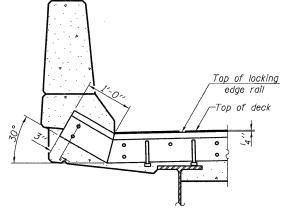
SECTION THRU WELDED RAIL JOINT

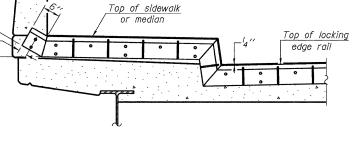
ANCHOR P

(for welded rail.

or chipped off flush with the plates

after forms are removed, typ.





AT PARAPET

AT SIDEWALK OR MEDIAN

Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

ROLLED EXTRUDED RAIL

after forms are removed, typ.

LOCKING EDGE RAIL SPLICE

***Back gouge not required if

is verified by mock-up.

complete joint penetration

_Grind Flush

The inside of the locking edge rail groove shall be free of weld residue.

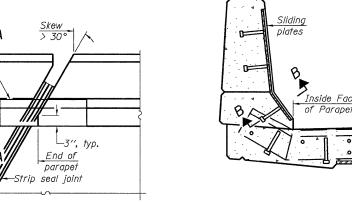
plates

PLAN

Inside face of parapet

LOCKING EDGE RAILS

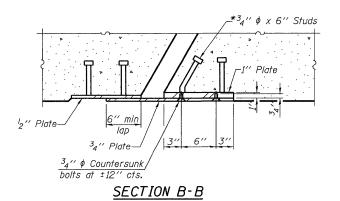
WELDED RAIL



SECTION A-A

POINT BLOCK DETAILS (for skews > 30°)

TYPICAL END TREATMENTS



Item	Unit	Total
Preformed Joint Strip Seal	Foot	240

BILL OF MATERIAL

DESIGNED DFM CHECKED DSG DRAWN EBS CHECKED DFM

5-16-08

EJ-SSJ

EXPANSION JOINT DETAILS 23RD OVER F.A.I. 74 ROCK ISLAND COUNTY STRUCTURE NO. 081-0105

Stage Construction Line

<u>"A"</u>

-Foam Plugs

Threaded or Coil

Splicer Rods (E)

Template

Forms-



* 81 (1-2, 1, 2-2) RS-1 & M

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and fied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

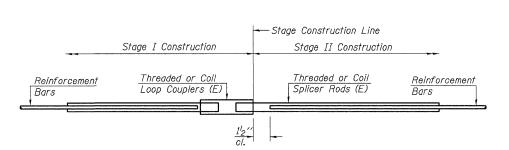
- Minimum Capacity (Tension in kips) = 1.25 x fy x A_t
- (Tension in kips)

 Minimum *Pull-out Strength = $0.66 \times fy \times A_t$ (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

- A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

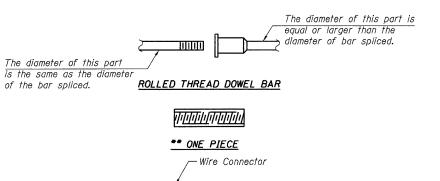
	BAR SPLICER ASSEMBLIES							
		Strengt	h Requirements					
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension					
#4	1'-8''	14.7	7.9					
#5	2'-0"	23.0	12.3					
#6	2'-7''	33.1	17.4					
#7	3′-5″	45.1	23.8					
#8	4'-6''	58.9	31.3					
#9	5′-9′′	75.0	39.6					
#10	7′-3′′	95.0	50.3					
#11	9'-0''	117.4	61.8					



STANDARD

Bar Size	No. Assemblies Required	Location
#5	16	Bridge Deck at East Abutment
#5	16	Bridge Deck at West Abutment
#6	6	East Abutment
#6	6	West Abutment

BAR SPLICER ASSEMBLY DETAILS 23RD AVE. OVER F.A.I. 74 ROCK ISLAND COUNTY STRUCTURE NO. 081-0105



WELDED SECTIONS

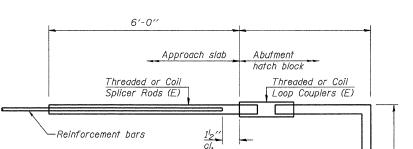
BAR SPLICER ASSEMBLY ALTERNATIVES

**Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



-Washer Face

<u>"B"</u>



FOR STUB ABUTMENTS

**	Bridge Deck	Approach Slab
Reinforcement Bars	Threaded or Coil Loop Couplers (E)	Threaded or Coil Splicer Rods (E)
	4'-0''	6′-0″

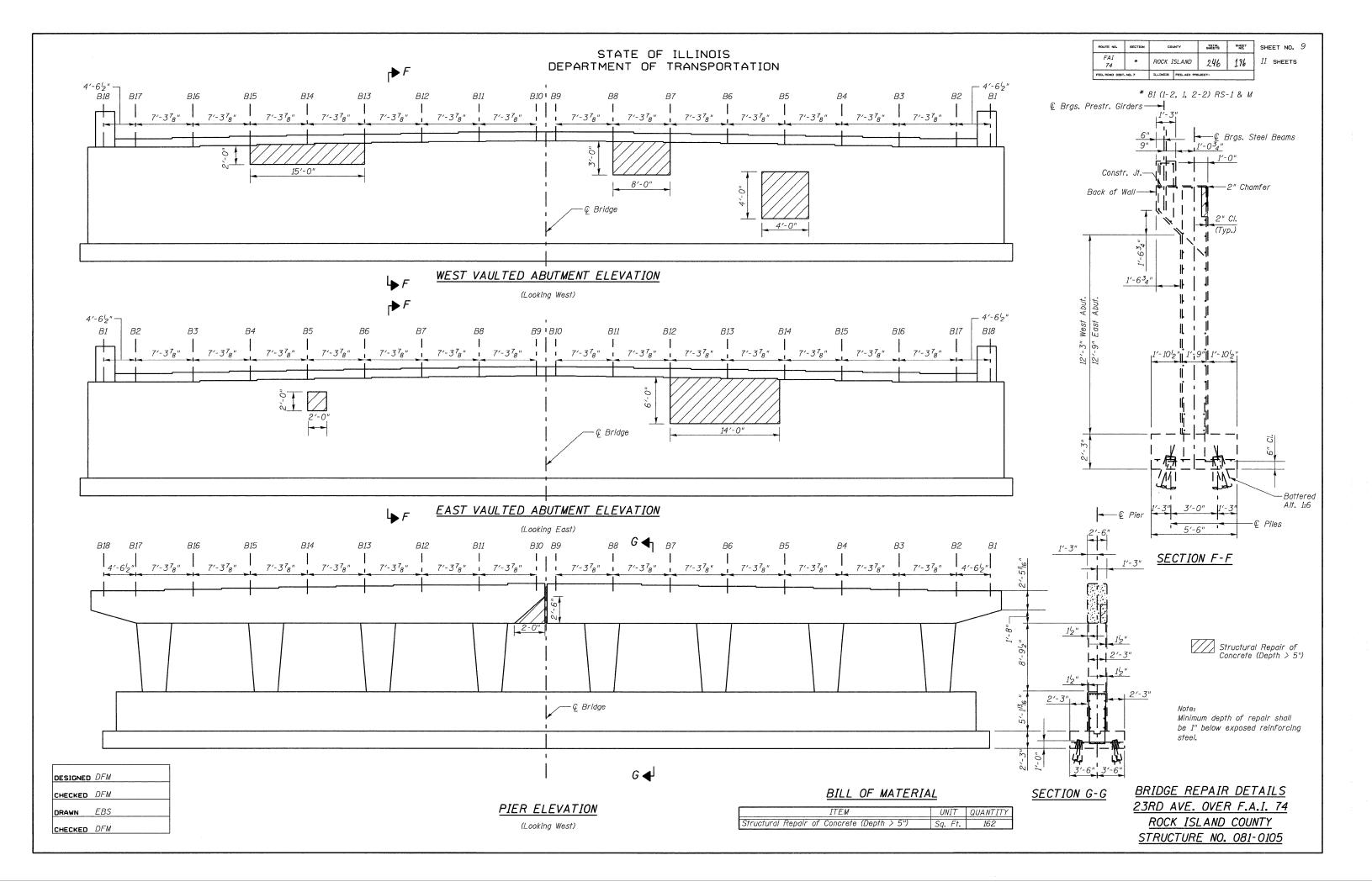
FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

	Bar	Splicer	for	#5	bar		
Min.	Capacity	= 23.0	kips	5 - 1	ensior	7	
Min.	Pull-out	Strength	=	12.3	kips	-	tension
No.	Required	=					

DESIGNED	DFM
CHECKED	DSG
DRAWN	EBS
CHECKED	DFM

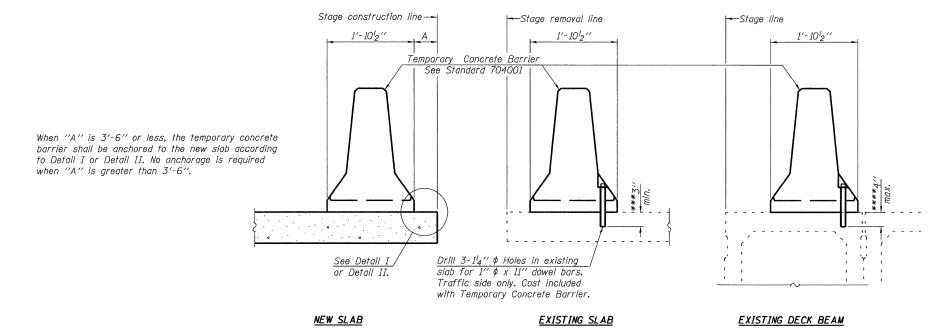
	Bar	Splicer	for #5	bar	
Min.	Capacity	= 23.0	kips - 1	ension	
Min.	Pull-out	Strength	= 12.3	kips -	tension
No.	Required	=			-

BSD-1 5-16-08



ROUTE NO.	SECTION	COL	INTY	TOTAL SHEETS	SHEET NO.	SHEET	NO.	10
FAI 74	*	ROCK I	SLAND	246	197	<i>11</i> sı	HEETS	
FED. ROAD DIST	. NO. 7	ILLINOIS	FED. AID PR	DJECT-				

* 81 (1-2, 1, 2-2) RS-1 & M



NOTES

Detail I - With Bar Splicer or Couplers:

Connect one (1) 1"x7"x10" steel P_c to the top layer of couplers with $2^{-5}8$ " ϕ bolts screwed to coupler at approximate Q_c of each barrier panel.

Detail II - With Extended Reinforcement Bars:

Connect one (1) 1''x7''x10'' steel £ to the concrete slab or concrete wearing surface with 2-5g'' \$\phi\$

Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate £ of each barrier panel.

Cost of anchorage is included with Temporary Concrete Barrier.

The 1'' x 7'' x 10'' plate shall not be removed until stage II constructed.

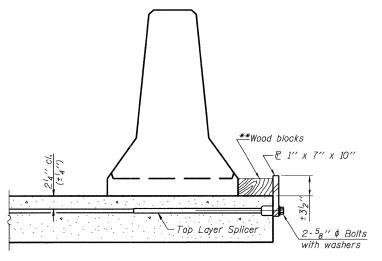
The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

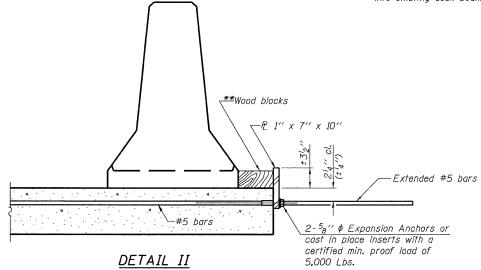
***Dimension shown is minimum required embedment into concrete.

If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

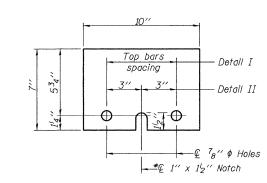
****If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



<u>DETAIL I</u>



**Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.



STEEL RETAINER & 1" x 7" x 10"

*Required only with Detail II

TEMPORARY CONCRETE BARRIER

FOR STAGE CONSTRUCTION

23RD AVE. OVER F.A.I. 74

ROCK ISLAND COUNTY

STRUCTURE NO. 081-0105

DESIGNED	DFM
CHECKED	DFM
DRAWN	EBS
CHECKED	DFM

R-27

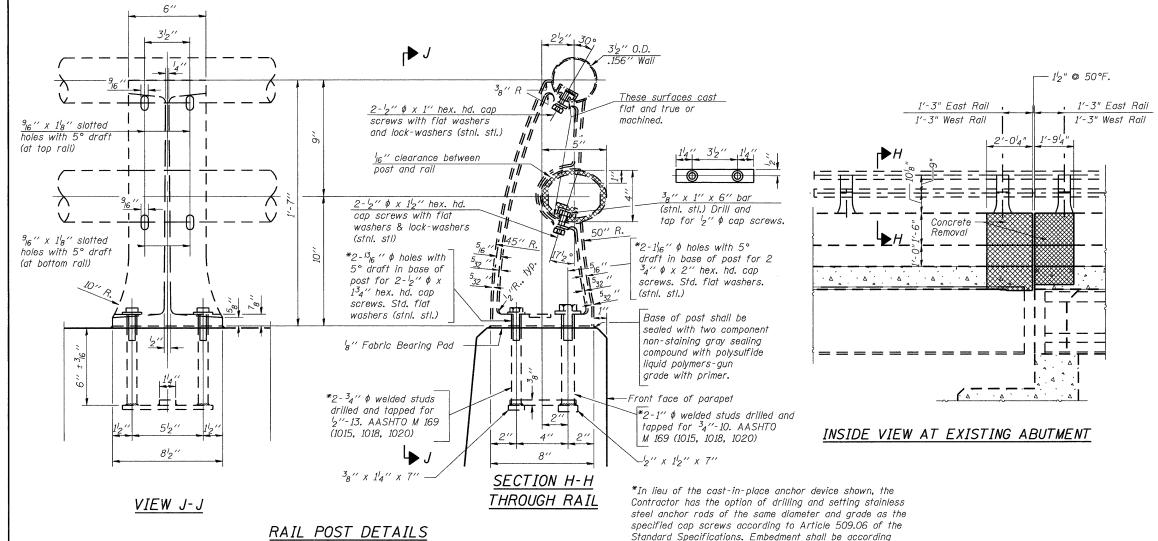
5-16-08

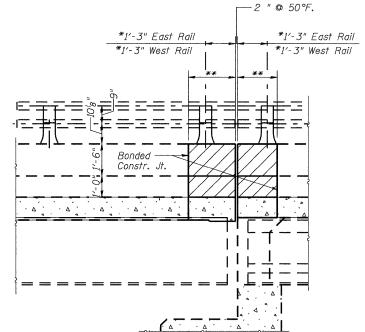
to the manufacturer's specifications.

** See sheet 4 of 12 for dimension.

ROUTE NO.	SECTION	co	JNTY	YOTAL SHEETS	SHEET NO.	SHEET NO. 11
FAI 74	*	ROCK I	SLAND	246	198	11 SHEETS
FED. ROAD DIST	NO. 7	ILLINOIS	FED. AID PRO	DJECT~		

* 81 (1-2, 1, 2-2) RS-1 & M





INSIDE VIEW AT PROPOSED ABUTMENT

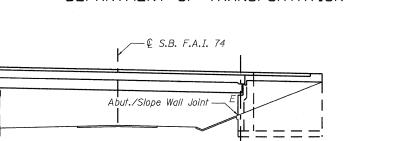
Notes.

For concrete removal limits See Sheet 4 of 12. Remove and reuse railing, including posts and anchors, where concrete removal affects existing rail posts. If anchors are damaged during construction, they shall be replaced and are included with concrete removal.

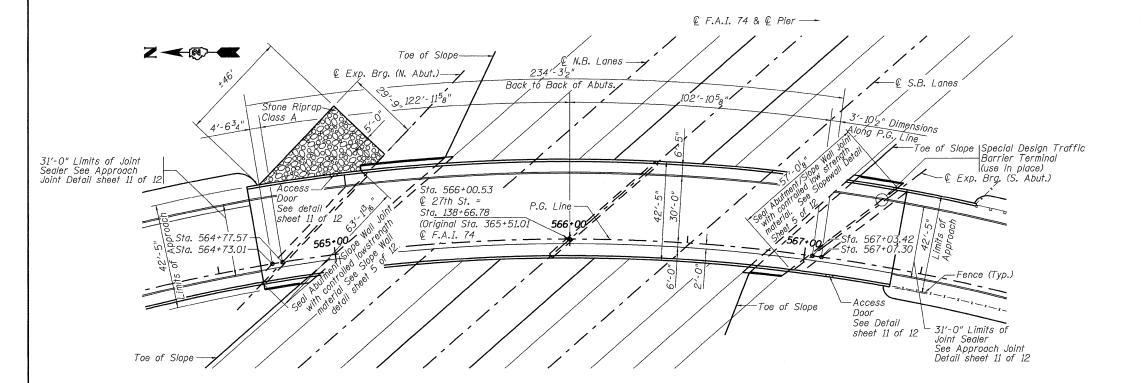
Length of rail shall be adjusted as detailed with a flush cut and the cast end cap shall be reused.

> RAILING DETAILS 23RD AVE. OVER F.A.I. 74 ROCK ISLAND COUNTY STRUCTURE NO. 081-0105

DESIGNED DFM CHECKED DFM DRAWN EBS CHECKED DFM



€ Exp. Brg. (S. Abut.)



ELEVATION

€ N.B. F.A.I. 74

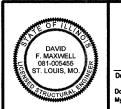
— Abut./Slope Wall Joint

€ Exp. Brg. (N. Abut.)-

PLAN

TOTAL BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Stone Riprap, Class A1	SQ. YD.	29.9
Concrete Removal	CU. YD.	17.6
Concrete Superstructure	CU. YD.	17.6
Reinforcement Bars, Epoxy Coated	POUND	2170
Bar Splicers	EACH	22
Controlled Low Strength Material	CU. YD.	1
Preformed Joint Strip Seal	FOOT	112
Silicone Joint Sealer	FOOT	14
Access Door	EACH	2
Joint Sealer	FOOT	62



Dovid F. Maxwell S.E. Reg. No. 081-005455

Dote: 12/30/08

My registration expires November 30, 2010.

* 81 (1-2, 1, 2-2) RS-1 & M

GENERAL NOTES

No field welding is permitted except as specified in the contract documents.

Existing reinforcement bars extending into the removal areas shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with approved bar splicer or anchoring system. Cost included with concrete removal.

Reinforcement bars shall conform to the requirements of ASTM A 706 Grade 60. See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Layout of proposed slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and all other loose, potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this project.

Joint openings shall be adjusted according to article 520.04 of the standard specifications when the deck is poured of an ambient temperature other than 50° F.

PLAN AND ELEVATION

N.B. 27TH ST. OVER F.A.I. 74

ROCK ISLAND COUNTY

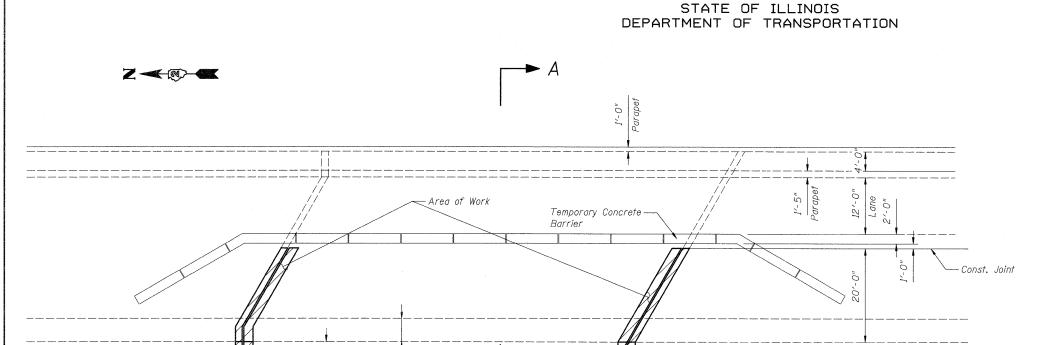
STRUCTURE NO. 081-0108

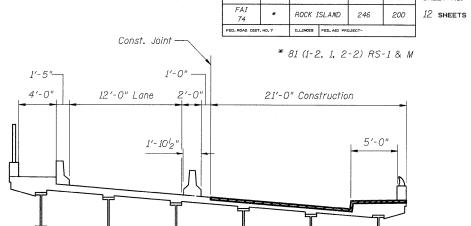
DESIGNED DFM

CHECKED DSG

DRAWN EBS

CHECKED DFM





COUNTY

TOTAL SHEET SHEET NO. 2

STAGE I CONSTRUCTION

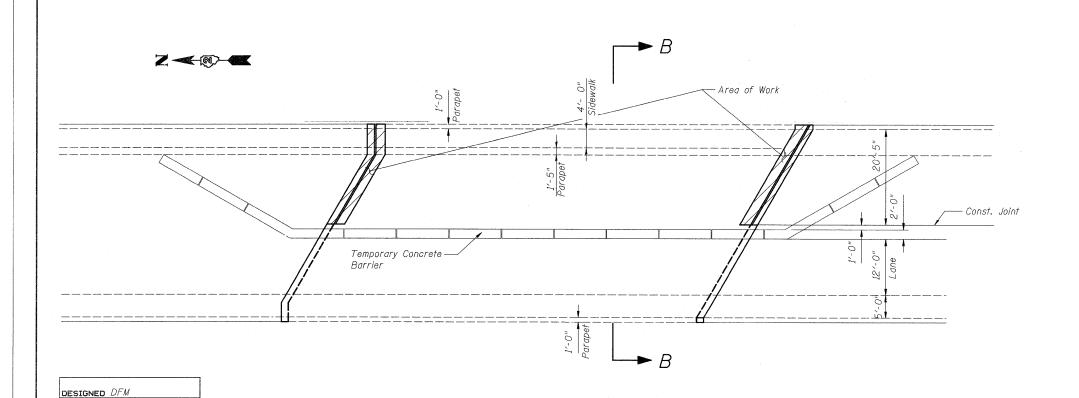
SECTION A-A

Install temporary concrete barrier as shown. *

Move traffic to Stage I traffic lane.

Replace Bridge Joints & Seal Approach Joints.

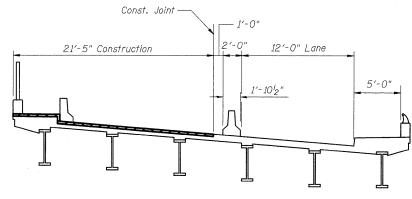
* For temporary concrete barrier details, see sheet 12 of 12.



CHECKED DSG

DRAWN EBS

CHECKED DFM



SECTION B-B

STAGE II CONSTRUCTION

Relocate temporary concrete barrier as shown. *

Move traffic to Stage II traffic lane.

Replace bridge joints & Seal Approach Joints.

Remove temporary concrete barrier.

<u>BRIDGE REPAIR DETAILS</u>

<u>N.B. 27TH ST. OVER F.A.I. 74</u>

<u>ROCK ISLAND COUNTY</u>

<u>STRUCTURE NO. 081-0108</u>