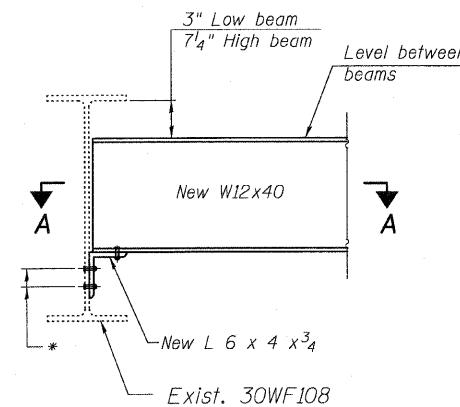
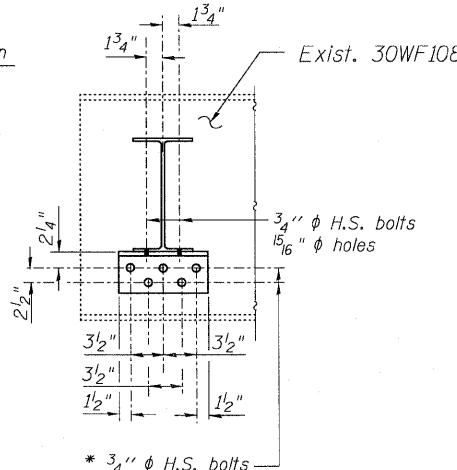


SECTION A-A



* Field drill $15/16''$ ϕ holes in new angle $6 \times 4 \times 3/4$ and connect with $3/4''$ ϕ H.S. bolts. Use holes in new angle as a template to field drill holes in existing beam. Cost included with Furnishing and Erecting Structural Steel.



I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads ($in.^4$ and $in.^3$).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, " n ", used for computing f_s (Total and Overload) due to short-term composite live loads ($in.^4$ and $in.^3$).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, " $3n$ ", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads ($in.^4$ and $in.^3$).

$I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite dead loads ($in.^4$ and $in.^3$).

Q : Un-factored non-composite dead loads (kips/ft.).

M_Q : Un-factored moment due to non-composite dead load (kip-ft.).

S_Q : Un-factored long-term composite (superimposed) dead load (kips/ft.).

M_{S_Q} : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).

M_L : Un-factored live load moment (kip-ft.).

M_I : Un-factored moment due to impact (kip-ft.).

M_o : Factored design moment (kip-ft.).

$1.3[M_Q + M_{S_Q} + \frac{5}{3}(M_L + M_I)]$

M_u : Compact composite moment capacity according to AASHTO LFD 10.50.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).

f_s (Overload): Sum of stresses as computed from the moments below (ksi). $M_Q + M_{S_Q} + \frac{5}{3}(M_L + M_I)$

f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi). $1.3[M_Q + M_{S_Q} + \frac{5}{3}(M_L + M_I)]$

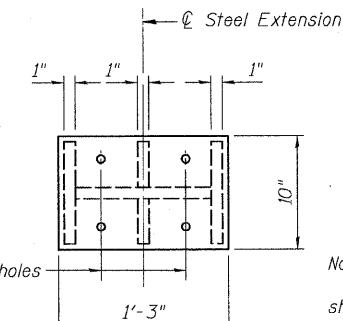
VR : Maximum $L_e +$ impact shear range within the composite portion of the span for stud shear connector design (kips).

INTERIOR BEAM MOMENT TABLE			
	0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or Pier 2	0.5 Sp. 2
I_s (in^4)	4,470	4,470	4,470
$I_c(n)$ (in^4)	12,309	-	12,309
$I_c(3n)$ (in^4)	9,316	-	9,316
I_{cr} (in^4)	-	6,471	-
S_s (in^3)	299	299	299
$S_c(n)$ (in^3)	442	-	442
$S_c(3n)$ (in^3)	403	-	403
S_{cr} (in^3)	-	354	-
Q (k'/f')	0.85	0.85	0.85
M_Q ($'K'$)	65.7	181.3	117.0
S_Q (k'/f')	0.34	0.34	0.34
M_{S_Q} ($'K'$)	24.7	68.5	44.1
M_L ($'K'$)	213.8	199.0	258.7
M_I ($'K'$)	64.2	59.0	77.7
$S_3[M_L + M_I]$ ($'K'$)	463.3	430.0	560.7
M_o ($'K'$)	719.9	883.7	938.3
M_u ($'K'$)	1802	-	1802
f_{sQ} (non-comp) (ksi)	2.64	7.28	4.70
f_{sQ} (comp) (ksi)	0.67	2.32	1.20
$f_{sL} S_3 [M_L + M_I]$ (ksi)	12.58	14.58	15.22
f_{sI} (Overload) (ksi)	15.89	24.17	21.11
f_{sT} (Total) (ksi)	-	31.4	-
VR (K)	52.6	58.5	44.5

** Compact Section

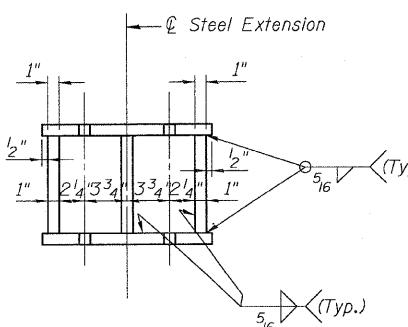
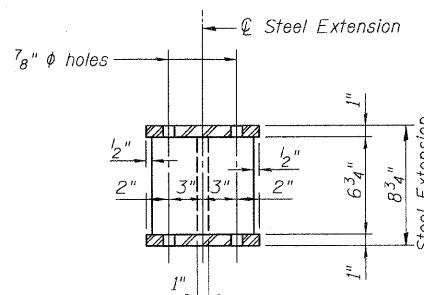
INTERIOR BEAM REACTION TABLE	
	Abut. Pier
R_Q (K)	48.4 ***
R_L (K)	36.1
R_I (K)	10.8
R_{Total} (K)	84.6

*** Includes total reaction from approach pavement divided by 6 girders.



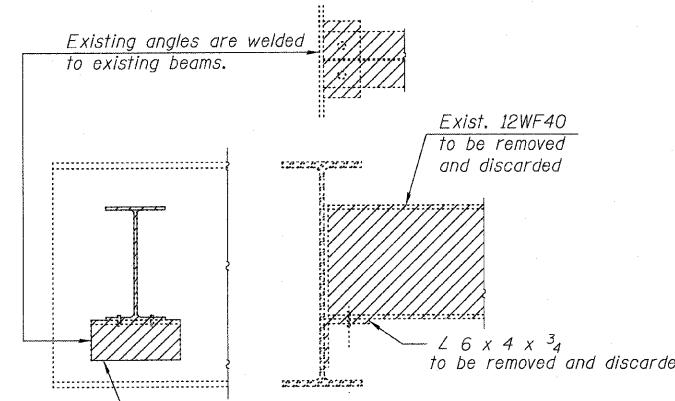
Note:
Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions.

PLAN STEEL EXTENSION



BILL OF MATERIAL

Item	Unit	Total
Structural Steel Removal	Pound	3730



END DIAPHRAGM "D" REMOVAL DETAIL

Remove 10 Diaphragms and 20 Connection Angles

ELEVATION STEEL EXTENSION: END VIEW ELEVATION STEEL EXTENSION

Fabricated from 1" Plate - Cost included with Fabrication and Erection of Structural Steel