

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.P.	I-HBR-2	WINNEBAGO	57	30
FED. RD. DES. NO. 7		ILLINOIS	FED. A.D. PROJECT-	

Contract #64D50

SHEET NO. 11
27 SHEETS

INTERIOR GIRDER MOMENT TABLE

	0.4 Sp. 1 or 0.6 Sp. 4	Piers 1 & 3	0.5 Sp. 2 or 0.5 Sp. 3	Pier 2
I_s (in ⁴)	4,090	4,090	4,090	4,090
$I_c(n)$ (in ⁴)	11,981	-	11,981	-
$I_c(3n)$ (in ⁴)	8,666	-	8,666	-
S_s (in ³)	299	299	299	299
$S_c(n)$ (in ³)	462	-	462	-
$S_c(3n)$ (in ³)	414	-	414	-
Z (in ³)	-	-	-	-
M_{DC1} ('k')	0.773	0.773	0.773	0.773
M_{DC2} ('k')	0.15	0.15	0.15	0.15
M_{DC2} ('k')	19.2	39.1	22.6	43.9
M_{DW} ('k')	0.267	0.267	0.267	0.267
M_{DW} ('k')	34.1	69.6	40.2	78.2
$M_L + IM$ ('k')	413	369	451	414
$M_u(\text{Strength } I)$ ('k')	921	1,051	1,023	1,179
$\phi_f M_n, \phi_f M_{nc}$ ('k')	4,022	1,232	4,022	1,232
$f_s DC1$ (ksi)	3.97	8.11	4.7	9.11
$f_s DC2$ (ksi)	0.56	1.57	0.66	1.77
$f_s DW$ (ksi)	0.99	2.79	1.17	3.14
$f_s 1.3(L+IM)$ (ksi)	13.95	19.25	15.23	21.6
$f_s (\text{Service } II)$ (ksi)	19.47	31.72	21.76	35.62
$f_s (\text{Total})(\text{Strength } I)$ (ksi)	-	-	-	-
V_f ('k')	13.9	-	13.3	-

* Compact sections

** Non-Compact and slender sections

INTERIOR GIRDER REACTION TABLE
HL93 Loading

	N & S Abut.	Piers 1 & 3	Pier 2
R_{DC1} ('k')	12.4	43.8	46.0
R_{DC2} ('k')	2.38	8.46	8.93
R_{DW} ('k')	4.22	15.04	15.87
$M_L + IM$ ('k')	57.9	111.5	111.5
R_{Total} ('k')	76.9	178.8	182.3

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in⁴ and in³).

Z: Plastic Section Modulus of the steel section in non-composite areas. Omit line in Moment Table if not used in design calculations (in³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

$M_L + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

1.25 ($M_{DC1} + M_{DC2}$) + 1.5 M_{DW} + 1.75 $M_L + IM$

$\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

$\phi_f M_{nc}$: Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).

f_s (Service II): Sum of stresses as computed from the moments below (ksi).

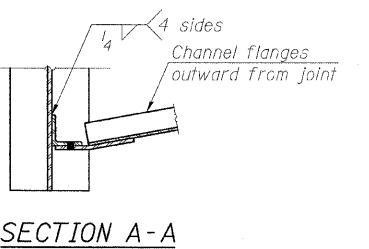
$M_{DC1} + M_{DC2} + M_{DW} + 1.5 M_L + IM$

f_s (Total)(Strength I): Sum of stresses as computed from the moments below on non-compact section (ksi).

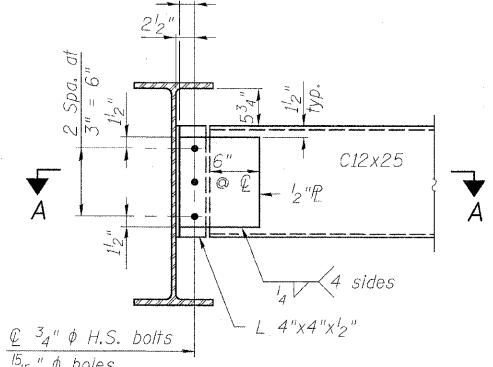
1.25 ($M_{DC1} + M_{DC2}$) + 1.5 M_{DW} + 1.75 $M_L + IM$

V_f : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

NOTE:
Furnishing Structural Steel is
not included in this contract.

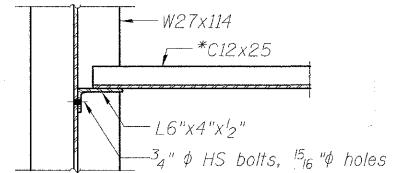


SECTION A-A

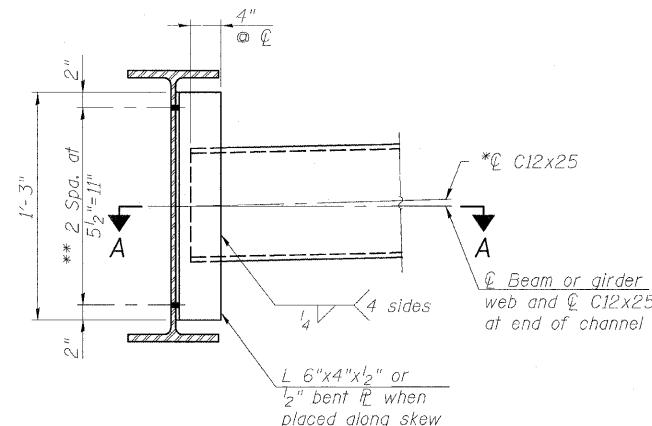


END DIAPHRAGM (D)

Note:
Two hardened washers required for each set of oversized holes.



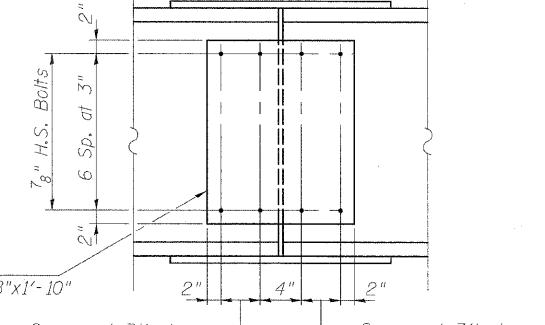
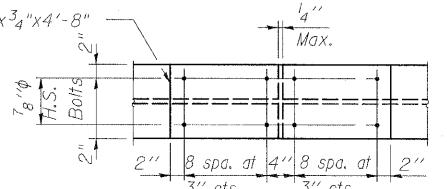
SECTION A-A



INTERIOR DIAPHRAGM (D1)

Note:
Two hardened washers required for each set of oversized holes.

*C12x30 channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section.
** 3/4 inch diameter H.S. bolts, 15/16 inch center-to-center.



SPLICE DETAIL

M270 Grade 50 (N.T.R.)
(18 Required)

STRUCTURAL STEEL DETAILS
MERIDIAN ROAD OVER US 20

F.A.P. 301 (US 20) - SEC. 1-HBR-2

WINNEBAGO COUNTY

STATION 100+00

STRUCTURE NO. 101-0096

DESIGNED	GUN / OAO
CHECKED	FCO
DRAWN	TCS / GUN
CHECKED	FCO