INTERIOR GIRDER MOMENT TABLE

		0.4 Sp. 1	Pier #1	0.5 Sp. 2	Pier #2	0.6 Sp. 3
I_s	(in^4)	7350	7350	7350	7350	7350
$I_c(n)$	(in^4)	25868	-	25868	-	25868
$I_c(3n)$	(in^4)	17806	-	17806	-	17806
S_s	(in^3)	442	442	442	442	442
$S_c(n)$	(in^3)	779	-	779	-	779
$S_c(3n)$	(in^3)	683	-	683	-	683
Z	(in^3)	-	-	-	-	-
ϱ	$(k/')$	0.85	1.26	0.85	1.25	0.85
$M\varrho$	$('k)$	66.0	363.0	197.8	363.0	66.0
$s\varrho$	$(k/')$	0.41	-	0.41	-	0.41
$M_s\varrho$	$('k)$	49.2	-	138.9	-	49.2
M_{ϱ}	$('k)$	233.6	151.9	389.7	151.9	233.6
M_{IM}	$('k)$	70.1	45.6	116.9	45.6	70.1
$\frac{5}{3}[M_{\varrho}+_I]$	$('k)$	506.2	329.2	844.33	329.2	506.2
M_a	$('k)$	807.8	899.9	1535.34	899.9	807.8
* M_u	$('k)$	2261	-	2114	-	2261
$f_s\,\varrho\,non\text{-}comp$	(ksi)	1.8	9.9	5.4	9.9	1.8
$f_s\,\varrho\,(comp)$	(ksi)	0.9	-	2.4	-	0.9
$f_s\,\frac{5}{3}[M_{\varrho}+M_I]$	(ksi)	7.8	8.9	13.0	8.9	7.8
$f_s\,(Overload)$	(ksi)	10.5	18.8	20.8	18.8	10.5
** $f_s\,(Total)$	(ksi)	-	24.4	-	24.4	-
VR	(k)	44.5	-	47.3	-	44.6

INTERIOR GIRDER REACTION TABLE

		*** W. Abut.	Pier #1	Pier #2	*** E. Abut.
$R\varrho$	(k)	17.2	76.8	76.8	17.2
R_{ϱ}	(k)	35.4	42.7	42.7	35.4
R_I	(k)	10.6	12.8	12.8	10.6
R_{Total}	(k)	63.3	132.4	132.4	63.3

*   Compact section
** Braced non-compact and partially braced section
*** These reactions include 3 kips per foot dead load from the Approach Pavement.

I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads ($in.^4$ and $in.^3$).

$I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads ($in.^4$ and $in.^3$).

$I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio. "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads ($in.^4$ and $in.^3$).

ϱ: Un-factored non-composite dead load (kips/ft.).

$M\varrho$: Un-factored moment due to non-composite dead load (kip-ft.).

$s\varrho$: Un-factored long-term composite (superimposed) dead load (kips/ft.)

$M_s\varrho$: Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).

M_{ϱ}: Un-factored live load moment (kip-ft.).

M_I: Un-factored moment due to impact (kip-ft.).

M_a: Factored design moment (kip-ft.).
$1.3\,[\,M\varrho + M_s\varrho + \frac{5}{3}\,(M_{\varrho} + M_I)\,]$

M_u: Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).

f_s (Overload): Sum of stresses as computed from the moments below (ksi).
$M\varrho + M_s\varrho + \frac{5}{3}\,(M_{\varrho} + M_I)$

f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).
$1.3\,[M\varrho + M_s\varrho + \frac{5}{3}\,(M_{\varrho} + M_I)]$

VR: Maximum $_{\varrho}$ + impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).

BEAM MOMENT & REACTION TABLES
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