

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	1

INDEX OF SHEETS 02-28-14 LETTING ITEM 058

- 1 COVER SHEET, INDEX OF SHEETS
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PROPOSED PLANS FOR FEDERAL AID HIGHWAY

T.R. 428 (RICHTON ROAD)
RICHTON ROAD OVER PLUM CREEK
BRIDGE RECONSTRUCTION
PROJECT BROS-0197(124)
SECTION 11-02118-01-BR
CRETE TOWNSHIP
WILL COUNTY
JOB NO.: C-91-403-12

CONTRACT NO: 61A02



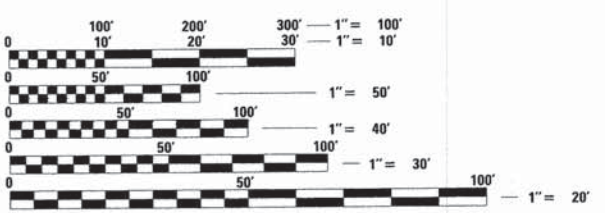
**THIS PROJECT IS LOCATED
IN CRETE TOWNSHIP**

DESIGN DESIGNATION AND TRAFFIC DATA

**2013 ADT VEHICLES
RICHTON RD = 1350**

FUNCTIONAL CLASSIFICATION: LOCAL ROAD

**DESIGN SPEED 30 MPH
POSTED SPEED 30 MPH**

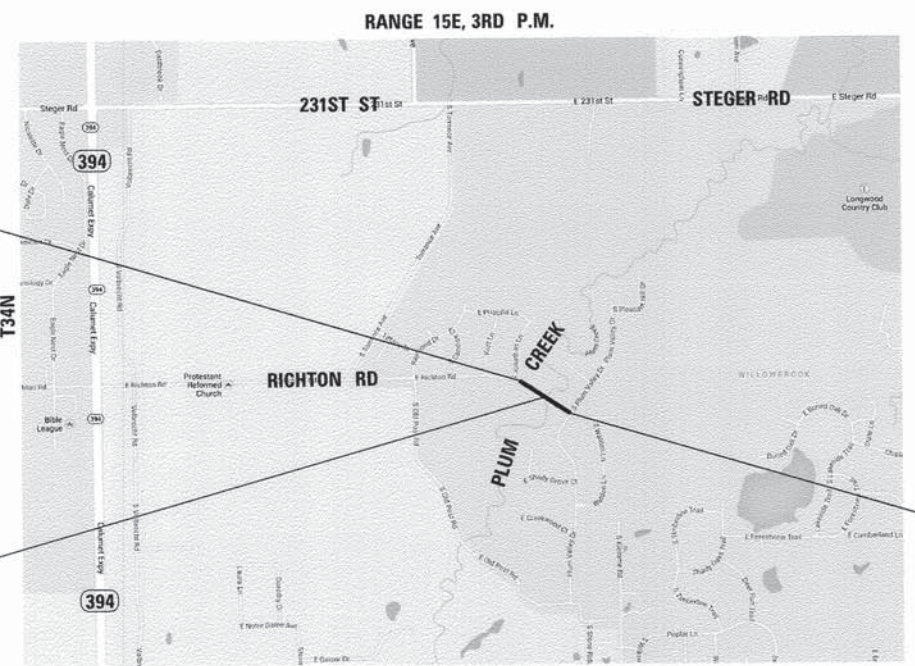


FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123 OR 811

**BEGIN IMPROVEMENT
STA 10 + 50.18**

**PROP STRUCTURE
SN 099-3286**



LOCATION MAP

SCALE: NTS

GROSS LENGTH = 493.33 FT. = 0.093 MILE
NET LENGTH = 493.33 FT. = 0.093 MILE



**END IMPROVEMENT
STA 15 + 43.51**

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

APPROVED December 24th, 2013
Anthony Perrotti
CRETE TOWNSHIP HIGHWAY COMMISSIONER

PASSED December 17, 2013
Carl J. Hart
DISTRICT 1 ENGINEER LOCAL ROADS AND STREETS

RELEASING FOR BID
BASED ON LIMITED
REVIEW December 17, 2013
John Fortmann
DEPUTY DIRECTOR OF HIGHWAYS, REGION 1 ENGINEER

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OF THE STATE OF ILLINOIS**

PROGRAM AND OFFICE ENGINEER: CHARLES F. RIDDLE, P.E., 847-705-4406, SCHAUMBURG, IL

IDOT DISTRICT 1 STANDARDS

- BD-32 BUTT JOINT AND HMA TAPER DETAILS
- BD-34 DETAILS FOR DEPRESSED CURB & GUTTER AND SHOULDER TREATMENT AT TBT TY 1 SPL
- BD-51 BENCHING DETAIL FOR EMBANKMENT WIDENING

IDOT HIGHWAY STANDARDS

- 000001-06 STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
- 280001-07 TEMPORARY EROSION CONTROL SYSTEMS
- 420401-10 BRIDGE APPROACH PAVEMENT CONNECTOR
- 515001-03 NAME PLATE FOR BRIDGES
- 601101-01 CONCRETE HEADWALL FOR PIPE DRAIN
- 606001-05 CONCRETE CURB TYPE B AND COMBINATION CONCRETE CURB AND GUTTER
- 630001-10 STEEL PLATE BEAM GUARDRAIL
- 630201-06 PCC/HMA STABILIZATION AT STEEL PLATE BEAM GUARDRAIL
- 630301-06 SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
- 631031-12 TRAFFIC BARRIER TERMINAL, TYPE 6
- 635006-03 REFLECTOR AND TERMINAL MARKER PLACEMENT
- 635011-02 REFLECTOR MARKER AND MOUNTING DETAILS
- 701901-03 TRAFFIC CONTROL DEVICES

UTILITY CONTACT INFORMATION

AQUA ILLINOIS
MICHAEL CAP
1000 SOUTH SCHUYLER AVE
KANKAKEE, IL 60901
OFFICE: (815) 614-2029
CELLULAR: (815) 450-8502

AT&T
STEVE PESOLA
1000 COMMERCE DRIVE
OAK BROOK, IL 60523
OFFICE: (630) 573-5703
CELLULAR: (815) 412-5255

COMCAST
TED WYMAN
688 INDUSTRIAL DRIVE
ELMHURST, IL 60126
OFFICE: (630) 600-6349

COMED
BRAD SCHINADARGAR
200 S. GOVERNORS HIGHWAY
UNIVERSITY PARK, IL 60466
OFFICE: (708) 235-2692
CELLULAR: (708) 625-2399

NICOR
DUC LE
1844 FERRY RD
NAPERVILLE, IL 60563
OFFICE: (630) 317-2242


GENERAL NOTES

1. SEEDING SHALL NOT BE PERMITTED AT ANY TIME WHEN THE GROUND IS FROZEN, WET, OR IN AN UNTILLABLE CONDITION. LOCATIONS TO BE SEEDED WILL BE DETERMINED BY THE ENGINEER.
2. ALL ELEVATIONS REFER TO U.S.G.S. MEAN SEA LEVEL DATUM.
3. ALL DAMAGE TO TOWNSHIP, CITY, COUNTY OR STATE OWNED UNDERGROUND FACILITIES, CAUSED BY THE CONTRACTOR SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER AT THE CONTRACTOR'S EXPENSE. THIS SHALL INCLUDE ALL TEMPORARY REPAIRS REQUIRED TO KEEP THE FACILITY OPERATIONAL WHILE MATERIAL IS BEING OBTAINED TO MAKE PERMANENT REPAIRS. SPLICING OF ELECTRICAL CABLE SHALL NOT BE ALLOWED. ELECTRIC CABLE SHALL BE REPLACED FROM POLE TO POLE OR CONTROLLER.
4. EXCEPT WHERE MODIFIED BY THE SPECIAL PROVISIONS INCLUDED IN THE CONTRACT DOCUMENTS OR THE DETAILS IN THE PLANS, ALL CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH THE "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION", ADOPTED JANUARY 1, 2012; THE "SUPPLEMENTAL SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS", ADOPTED JANUARY 1, 2014; THE LATEST EDITION OF THE "ILLINOIS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS", THE "STANDARD SPECIFICATIONS FOR WATER AND SEWER MAIN CONSTRUCTION IN ILLINOIS" MAY 1996 FIFTH EDITION; THE "DETAILS" IN THE PLANS; AND THE "SPECIAL PROVISIONS" IN THE CONTRACT DOCUMENTS.

ANY REFERENCE TO STANDARDS THROUGHOUT THE PLANS OR SPECIAL PROVISIONS SHALL BE INTERPRETED AS THE LATEST IDOT STANDARDS.
5. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO UNDERSTAND THE SOIL AND GROUNDWATER CONDITIONS AT THE SITE. THE STRUCTURE GEOTECHNICAL REPORT IS INCLUDED IN THE PROJECT SPECIFICATIONS FOR REVIEW AND INFORMATION.
6. BEFORE STARTING ANY EXCAVATION, THE CONTRACTOR SHALL CALL "JULIE" AT 1-800-892-0123 FOR FIELD LOCATIONS OF BURIED ELECTRIC, TELEPHONE AND GAS FACILITIES (48 HOUR NOTIFICATION IS REQUIRED).
7. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH UTILITY COMPANIES, WILL COUNTY, AND CRETE TOWNSHIP.
8. THE CONTRACTOR WILL NOT BE ALLOWED TO SET UP A YARD OR FIELD OFFICE ON TOWNSHIP, CITY, COUNTY OR STATE PROPERTY WITHOUT WRITTEN PERMISSION FROM THE CRETE TOWNSHIP HIGHWAY DEPARTMENT.
9. THE ILLINOIS DEPARTMENT OF TRANSPORTATION IS NOT THE OWNER OF RECORD FOR THIS BRIDGE. THOSE SEEKING HISTORIC, AS-BUILT OR OTHER EXISTING DOCUMENTS MUST CONTACT THE OWNER OF RECORD TO MAKE ARRANGEMENTS FOR ACCESS TO THIS INFORMATION. OWNER OF RECORD:
CRETE TOWNSHIP HIGHWAY DEPARTMENT
ANTHONY RECUPITO, HIGHWAY COMMISSIONER
25405 S. STATE ST., CRETE, IL. 60417
10. THE CONTRACTOR SHALL BE REQUIRED TO PROVIDE ACCESS TO ABUTTING PROPERTY AT ALL TIMES DURING THE CONSTRUCTION OF THIS PROJECT.
11. DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS. STATIONS ARE PROVIDED ON THE PLANS.
12. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND CONDITIONS EXISTING IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING OF MATERIALS.
13. THE CONTRACTOR SHALL USE CARE IN REMOVING OR EXCAVATING NEAR ALL EXISTING ITEMS WHICH WILL REMAIN. ANY DAMAGE DONE TO EXISTING ITEMS BY THE CONTRACTOR SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.
14. THE CONTRACTOR SHALL COMPLY WITH ALL REQUIREMENTS AT THE DIRECTION OF THE ENGINEER. THIS WORK IS INCIDENTAL TO THE CONTRACT.
15. CLEARING SHALL BE DONE TO THE CONSTRUCTION LIMITS OF THE PROJECT. SEE SECTION 201.01(A) OF THE STANDARD SPECIFICATIONS. CLEARING WILL NOT BE MEASURED FOR PAYMENT.
16. THE PROJECT HYDRAULIC REPORT CAN BE OBTAINED FROM THE CRETE TOWNSHIP HIGHWAY DEPARTMENT.
17. THE CONTRACTOR MUST RETURN ALL EXISTING MOT SIGNS TO CRETE TOWNSHIP FREE OF DAMAGE AFTER THE COMPLETION OF THE PROJECT. THIS WORK IS INCIDENTAL TO TRAFFIC CONTROL AND PROTECTION, (SPECIAL).
18. THE CONTRACTOR SHALL MAINTAIN AT ALL TIMES A 10 FOOT MINIMUM HORIZONTAL CLEARANCE FROM THE OVERHEAD COMED 12KV LINES. IF THIS MINIMUM CLEARANCE CANNOT BE MAINTAINED, THE CONTRACTOR SHALL REQUEST FROM COMED A POWER OUTAGE. COMED REQUIRES UP TO 16 WEEKS OF LEAD TIME FOR ANY OUTAGE. ANY NECESSARY OUTAGE WILL BE AT THE EXPENSE OF THE CONTRACTOR AND SHALL NOT IMPACT SCHEDULE.
19. PROJECT WORK HOURS:
7 AM TO 5 PM - MONDAY THRU FRIDAY
9 AM TO 3 PM - SATURDAY
NO WORK ON SUNDAY

EROSION CONTROL GENERAL NOTES

1. THE CONSTRUCTION LIMITS WILL BE STAKED BY THE CONTRACTOR PRIOR TO COMMENCING CONSTRUCTION. PAYMENT FOR THIS WORK WILL BE INCLUDED WITHIN THE ITEM CONSTRUCTION LAYOUT, LS. THE CONSTRUCTION LIMITS MAY BE ADJUSTED BY THE ENGINEER TO PRESERVE TREES AND NO ADDITIONAL COMPENSATION WILL BE PAID TO THE CONTRACTOR FOR CHANGED CONSTRUCTION LIMITS. ALL WORK AND EQUIPMENT SHALL REMAIN WITHIN THESE LIMITS THROUGHOUT THE DURATION OF THIS PROJECT.
2. SEDIMENT AND EROSION CONTROL DEVICES SHALL BE FUNCTIONAL BEFORE THE PROJECT SITE IS OTHERWISE DISTURBED.
3. UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES WILL BE CONSTRUCTED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS IN THE ILLINOIS URBAN MANUAL REVISED 2012.
4. THE WILL-SOUTH COOK SOIL AND WATER CONSERVATION DISTRICT (WSCSWCD) MUST BE NOTIFIED ONE WEEK PRIOR TO THE PRE-CONSTRUCTION CONFERENCE, ONE WEEK PRIOR TO THE COMMENCEMENT OF LAND DISTURBING ACTIVITIES, AND ONE WEEK PRIOR TO THE FINAL INSPECTION.
5. A COPY OF THE APPROVED EROSION AND SEDIMENT CONTROL PLAN AND SWPPP SHALL BE MAINTAINED ON SITE AT ALL TIMES.
6. PRIOR TO COMMENCING LAND-DISTURBING ACTIVITIES IN AREAS OTHER THAN INDICATED ON THESE PLANS (INCLUDING BUT NOT LIMITED TO, ADDITIONAL PHASES OF DEVELOPMENT AND OFF-SITE BORROW OR WASTE AREAS), A SUPPLEMENTARY EROSION CONTROL PLAN SHALL BE SUBMITTED TO THE OWNER FOR REVIEW BY WSCSWCD.
7. THE CONTRACTOR IS RESPONSIBLE FOR INSTALLATION OF ANY ADDITIONAL EROSION CONTROL MEASURES NECESSARY TO PREVENT EROSION AND SEDIMENTATION AS DETERMINED BY THE WSCSWCD. THESE ADDITIONAL ITEMS ARE INCIDENTAL TO THE CONTRACT.
8. DURING DEWATERING OPERATIONS, WATER WILL BE PUMPED INTO SEDIMENT BASINS OR SILT TRAPS. DEWATERING DIRECTLY INTO FIELD TILES OR STORMWATER STRUCTURE IS PROHIBITED.
9. IT IS THE RESPONSIBILITY OF THE LANDOWNER AND/OR GENERAL CONTRACTOR TO INFORM ANY SUB-CONTRACTOR(S) WHO MAY PERFORM WORK ON THIS PROJECT, OF THE REQUIREMENTS IN IMPLEMENTING AND MAINTAINING THESE EROSION CONTROL PLANS AND THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEMS (NPDES) PERMIT REQUIREMENTS SET FORTH BY THE ILLINOIS EPA.
10. PERIMETER EROSION CONTROL BARRIER SHALL BE ERECTED ADJACENT TO TEMPORARY CONSTRUCTION FENCE. THE RESIDENT ENGINEER SHALL HAVE FINAL DETERMINATION OF THE PLACEMENT AND LOCATION OF THE PERIMETER EROSION CONTROL BARRIER.
11. COMPLETED SLOPES (SECTION OF ROAD EMBANKMENT) SHALL BE SEEDED AND BLANKETED AS THE EXCAVATION PROCEEDS TO THE EXTENT CONSIDERED DESIRABLE AND PRACTICAL. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR PROLONG FINAL GRADING AND SHAPING SO THAT THE ENTIRE PROJECT CAN BE PERMANENTLY SEEDED AT ONE TIME.
12. CLEANING OF VEHICLES AND EQUIPMENT SHALL BE PERFORMED IN A MANNER TO REDUCE THE AMOUNT OF POLLUTANTS TRIBUTARY TO STORM SEWERS AND OPEN WATERS TO THE MAXIMUM EXTENT POSSIBLE.
13. REFER TO STORM WATER POLLUTION PREVENTION PLAN FOR INSPECTION AND MAINTENANCE SCHEDULES, AND SEQUENCE OF ACTIVITIES.
14. ALL NECESSARY MEASURES SHALL BE TAKEN TO CONTAIN ANY FUEL OR POLLUTION RUN OFF. LEAKY EQUIPMENT OR SUPPLIES SHALL BE IMMEDIATELY REPAIRED OR REMOVED FROM THE SITE.
15. WHEN STREAM SHORELINE DISTURBANCE IS NECESSARY, THE STREAM SHORELINE SHALL BE STABILIZED WITHIN FORTY EIGHT (48) HOURS AFTER DISTURBANCE IS COMPLETED OR WORK IS INTERRUPTED.
16. WORK ALONG THE STREAM BANKS OF PLUM CREEK SHALL BE LIMITED TO PERIODS OF LOW FLOW. IF WORK MUST TAKE PLACE UNDER FLOWING CONDITIONS, THEN ADDITIONAL EROSION CONTROL MEASURES WILL BE NECESSARY AS DETERMINED BY THE WSCSWCD. THESE ADDITIONAL ITEMS WILL BE CONSIDERED INCIDENTAL TO THE CONTRACT.
17. THE USE OF HAY BALES IS NOT ALLOWED FOR EROSION CONTROL.

FILE NAME =	DESIGNED - EJA	REVISED -	 <p>STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p>GENERAL NOTES, EROSION CONTROL NOTES, AND IDOT STANDARDS</p>			T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
#FILE#	DRAWN - JLW	REVISED -					428	11-02118-01-BR	WILL	50	2
USER NAME = #USER#	CHECKED - MPM	REVISED -		SCALE: NTS			SHEET NO. 1 OF 1 SHEETS			CONTRACT NO. 61A02	
PLOT DATE = #DATE#	DATE - DECEMBER 9, 2013	REVISED -		STATION			TO STA.		[ILLINOIS] FED. AID PROJECT		

SUMMARY OF QUANTITIES				CONSTRUCTION CODE		
				80% FED 20% LOCAL ROADWAY 0004	80% FED 20% LOCAL BRIDGE 0011	100% LOCAL BRIDGE 0011
CODE	ITEM	UNIT	URBAN 0004 TOTAL QUANTITY	URBAN	URBAN	URBAN
20100110	TREE REMOVAL (6 TO 15 UNITS)	UNIT	100	100		
20101000	TEMPORARY FENCE	FOOT	258	258		
20101100	TREE TRUNK PROTECTION	EACH	3	3		
20200100	EARTH EXCAVATION	CU YD	1935	1935		
21101505	TOPSOIL EXCAVATION AND PLACEMENT	CU YD	650	650		
25000312	SEEDING, CLASS 4A	ACRE	0.25	0.25		
25000320	SEEDING, CLASS 5	ACRE	0.25	0.25		
25000350	SEEDING, CLASS 7	ACRE	0.25	0.25		
25000400	NITROGEN FERTILIZER NUTRIENT	POUND	23	23		
25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	23	23		
25000700	AGRICULTURAL GROUND LIMESTONE	TON	0.5	0.5		
25100630	EROSION CONTROL BLANKET	SQ YD	994	994		
28000400	PERIMETER EROSION BARRIER	FOOT	923	923		
28200200	FILTER FABRIC	SQ YD	707		707	

* SPECIALITY ITEM

FILE NAME =	DESIGNED - EJA	REVISED -
#FILE#	DRAWN - JLW	REVISED -
USER NAME = jworthington	CHECKED - MPM	REVISED -
PLOT DATE = 12/5/2013	DATE - DECEMBER 9, 2013	REVISED -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES

SCALE: SHEET NO. OF SHEETS STA. TO STA.

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	3
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

SUMMARY OF QUANTITIES				CONSTRUCTION CODE		
				URBAN 0004	80% FED 20% LOCAL ROADWAY 0004	80% FED 20% LOCAL BRIDGE 0011
CODE	ITEM	UNIT	TOTAL QUANTITY	URBAN	URBAN	URBAN
30300112	AGGREGATE SUBGRADE IMPROVEMENT 12"	SO YD	1106	1106		
40600100	BITUMINOUS MATERIALS (PRIME COAT)	GALLON	120	120		
40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SO YD	153	153		
40603080	HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	TON	270	270		
40603335	HOT-MIX ASPHALT SURFACE COURSE, MIX "D" N50	TON	67	67		
42001430	BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)	SO YD	49	49		
48203037	HOT-MIX ASPHALT SHOULDERS, 10"	SO YD	444	444		
50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1		1	
50200100	STRUCTURE EXCAVATION	CU YD	49		49	
50300225	CONCRETE STRUCTURES	CU YD	65.3		65.3	
50300255	CONCRETE SUPERSTRUCTURE	CU YD	290.8		290.8	
50300260	BRIDGE DECK GROOVING	SO YD	526		526	
50300300	PROTECTIVE COAT	SO YD	674		674	
50400745	FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BULB T-BEAMS 72"	FOOT	542		542	

* SPECIALITY ITEM

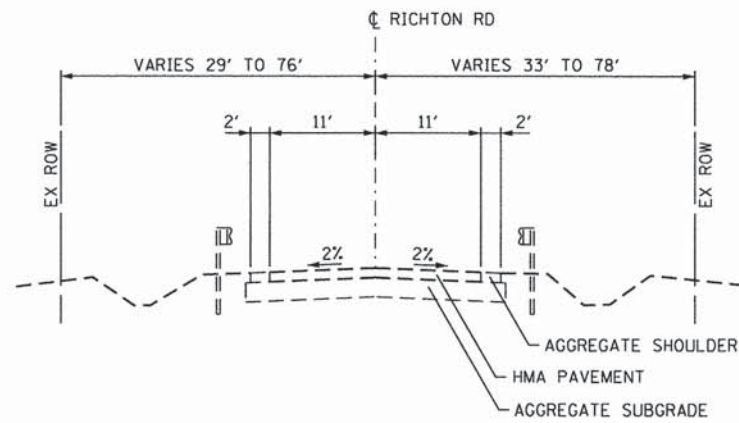
SUMMARY OF QUANTITIES				CONSTRUCTION CODE		
				URBAN 0004	80% FED 20% LOCAL ROADWAY 0004	80% FED 20% LOCAL BRIDGE 0011
CODE	ITEM	UNIT	TOTAL QUANTITY	URBAN	URBAN	URBAN
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	66070		66070	
51201800	FURNISHING STEEL PILES HP14X73	FOOT	650		650	
51202305	DRIVING PILES	FOOT	650		650	
51203800	TEST PILE STEEL HP14X73	EACH	2		2	
51204650	PILE SHOES	EACH	12		12	
51500100	NAME PLATES	EACH	1		1	
59100100	GEOCOMPOSITE WALL DRAIN	SO YD	109		109	
60608250	COMBINATION CONCRETE CURB AND GUTTER, TYPE M-2.06	FOOT	58	58		
* 63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	FOOT	25	25		
* 63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4	4		
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	4	4		
63200310	GUARDRAIL REMOVAL	FOOT	303	303		
67100100	MOBILIZATION	L SUM	1	1		
* 78200410	GUARDRAIL MARKERS, TYPE A	EACH	5	5		

* SPECIALITY ITEM

SUMMARY OF QUANTITIES				CONSTRUCTION CODE		
				URBAN 0004	80% FED 20% LOCAL ROADWAY 0004	80% FED 20% LOCAL BRIDGE 0011
CODE	ITEM	UNIT	TOTAL QUANTITY	URBAN	URBAN	URBAN
* 78200420	GUARDRAIL MARKERS, TYPE B	EACH	4	4		
* 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4	4		
Z0013797	STABILIZED CONSTRUCTION ENTRANCE	SQ YD	73	73		
Z0013798	CONSTRUCTION LAYOUT	L SUM	1	1		
Z0018010	DRAINAGE SCUPPERS, DS-33	EACH	4		4	
Z0046304	PIPE UNDERDRAINS FOR STRUCTURES, 4"	FOOT	142		142	
# Z0076600	TRAINEES	HOUR	1000	1000		
# Z0076604	TRAINEES TRAINING PROGRAM GRADUATE	HOUR	1000	1000		
X2810108	STONE RIPRAP, CLASS A4 (SPECIAL)	SQ YD	707		707	
X4404400	PAVEMENT REMOVAL (SPECIAL)	SQ YD	777	777		
X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	293		293	
X6700410	ENGINEER'S FIELD OFFICE, TYPE A (SPECIAL)	CAL MO	6			
X7010216	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	L SUM	1	1		
XX005642	GATEWAY MONUMENT SIGN COMPLETE	EACH	2			2

* SPECIALITY ITEM

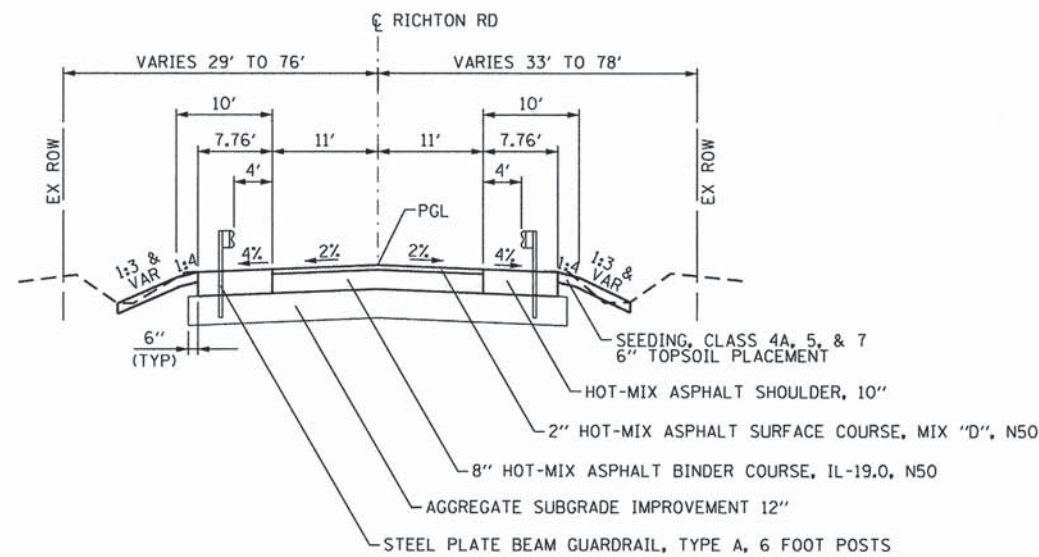
CONSTRUCTION TYPE CODE 0042



RICHTON RD EXISTING TYPICAL SECTION

(LOOKING EAST)
STA. 10+80.17 TO STA. 15+13.51

STRUCTURAL DESIGN TRAFFIC:	YEAR 2023	
PV = 1471	SU = 0	MU = 46
ROAD/STREET CLASSIFICATION:	CLASS III	
PERCENT OF STRUCTURAL DESIGN TRAFFIC IN DESIGN LANE:		
P = 50%	S = 50%	M = 50%
TRAFFIC FACTOR:	ACTUAL TF = 0.18	AC TYPE= *
	MINIMUM TF = 3.16	
PG GRADE: BINDER = PG64-22	SURFACE=SBS-PG64-28	
SUBGRADE SUPPORT RATING:		
	SSR = POOR (STA. 10+50.17 TO STA. 15+43.51)	



RICHTON RD PROPOSED TYPICAL SECTION

(LOOKING EAST)
STA. 10+80.17 TO STA. 12+24.50
STA. 13+93.50 TO STA. 15+13.51
BRIDGE & APPROACH SLABS OMISSION STA. 12+24.50 TO STA. 13+93.50

HOT-MIX ASPHALT REQUIREMENTS

MIXTURE TYPE	THICKNESS	AIR VOIDES @ NDES
HOT-MIX ASPHALT SURFACE COURSE, MIX "D", N50 (IL-9.5mm)	2"	4% @ 50 Gyr
HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	8" (2 LIFTS)	4% @ 50 Gyr
HOT-MIX ASPHALT SHOULDER, 10" (HMA BINDER IL-19mm)	10" (3 LIFTS)	4% @ 50 Gyr

UNIT WEIGHT USED TO CALCULATE ALL HOT-MIX ASPHALT MIXTURES IS 112 LBS/SQ YD/IN.
* THE "AC TYPE" FOR POLYMERIZED HMA MIXES SHALL BE "SBS/SBR PG 70-22" AND FOR NON-POLYMERIZED HMA THE "AC TYPE" SHALL BE "PG 64-22" UNLESS MODIFIED BY DISTRICT ONE SPECIAL PROVISIONS. FOR "PERCENT OF RAP" SEE DISTRICT ONE SPECIAL PROVISIONS. FOR USE OF RECYCLED MATERIALS, SEE SPECIAL PROVISIONS.

SCHEDULES

EARTHWORK SCHEDULE

(1) STAGE & LOCATION	(2) EARTH EXCAVATION (CU. YD.)	(3) EXCAVATION TO BE USED IN EMBANKMENT ADJUSTED FOR SHRINKAGE (25% FACTOR) (CU. YD.)	(4) EMBANKMENT (CU. YD.)	(5) EARTHWORK BALANCE WASTE (+) OR BORROW (-) (CU. YD.)	(6) TOPSOIL EXCAVATION (CU. YD.)	(7) TOPSOIL PLACEMENT (CU. YD.)
STA. 10+80.17 TO STA. 12+24.50	395	296	63	233	185	95
STA. 13+93.50 TO STA. 15+13.51	1,035	776	952	-176	270	170
BRIDGE OMISSION AREA, STA. 12+24.50 TO STA. 13+93.50	505	379	290	89	195	80
TOTAL	1,935			145	650	345

PAVEMENT AND SHOULDERS

LOCATION	AGGREGATE SUBGRADE IMPROVEMENT, 12"	BITUMINOUS MATERIALS (PRIME COAT)	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	HOT-MIX ASPHALT BINDER COURSE, IL 19.0, N50	HOT-MIX ASPHALT SURFACE COURSE, MIX "D" N50	BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)	HOT-MIX ASPHALT SHOULDERS, 10"	COMBINATION CONCRETE CURB AND GUTTER, TYPE M-2.06
	SQ YD	GAL	SQ YD	TON	TON	SQ YD	SQ YD	SQ YD
BRIDGE	0	0	0	0	0	0	0	0
WEST OF BRIDGE	1106	66	79	149	37	24	226	0
EAST OF BRIDGE	0	54	75	121	30	24	218	58
TOTAL	1106	120	153	270	67	49	444	58

LANDSCAPING AND EROSION CONTROL

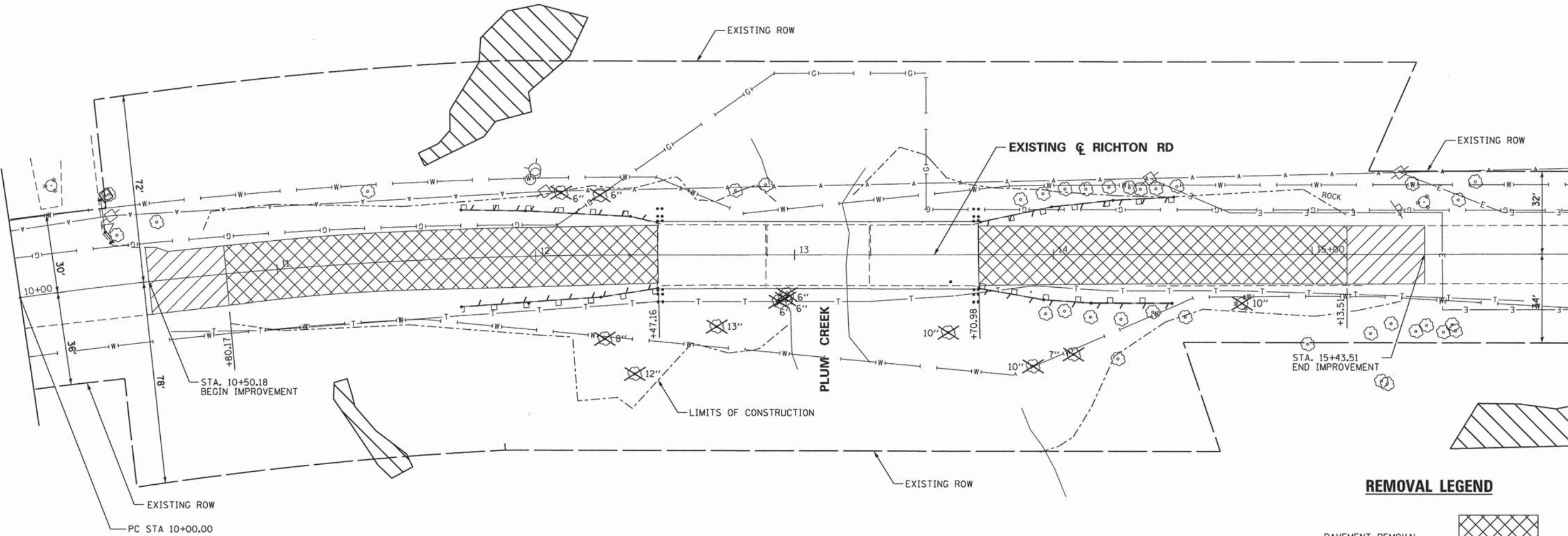
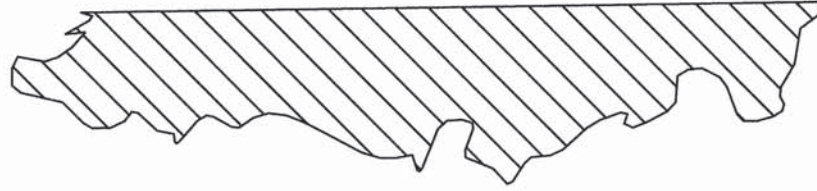
LOCATION	TEMPORARY FENCE	SEEDING, CLASS 4A	SEEDING, CLASS 5	SEEDING, CLASS 7	NITROGEN FERTILIZER NUTRIENT	POTASSIUM FERTILIZER NUTRIENT	AGRICULTURAL GROUND LIMESTONE	EROSION CONTROL BLANKET	PERIMETER EROSION BARRIER	PERIMETER EROSION BARRIER, ROLLED EXCELSIOR	STABILIZED CONSTRUCTION ENTRANCE
	FOOT	ACRE	ACRE	ACRE	POUND	POUND	TON	SQ YD	FOOT	FOOT	SQ YD
BRIDGE	0	0	0	0	0	0	0	0	0	0	0
WEST OF BRIDGE	100	0.08	0.08	0.08	23	23	0.5	395	493	229	73
EAST OF BRIDGE	158	0.12	0.12	0.12	0	0	0	599	430	640	0
TOTAL	258	0.25	0.25	0.25	23	23	0.5	994	923	869	73

GUARDRAIL

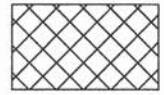
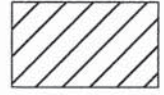

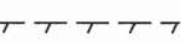

LOCATION	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	TRAFFIC BARRIER TERMINAL, TYPE 6	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	TERMINAL MARKER - DIRECT APPLIED	GUARDRAIL MARKERS, TYPE A	GUARDRAIL MARKERS, TYPE B
	FOOT	EACH	EACH	EACH	EACH	EACH
BRIDGE	0	0	0	0	0	4
WEST OF BRIDGE	25	2	2	2	3	0
EAST OF BRIDGE	0	2	2	2	2	0
TOTAL	25	4	4	4	5	4

REMOVAL

LOCATION	TREE REMOVAL (6 to 15 UNITS DIAMETER)	PAVEMENT REMOVAL (SPECIAL)	GUARDRAIL REMOVAL
	UNIT	SQ YD	FOOT
BRIDGE	41	0	0
WEST OF BRIDGE	32	421	152
EAST OF BRIDGE	27	355	151
TOTAL	100	777	303



REMOVAL LEGEND

- PAVEMENT REMOVAL 
- HMA SURFACE REMOVAL BUTT JOINT 
- EXISTING WETLANDS 
- GUARDRAIL REMOVAL 
- TREE REMOVAL 

FILE NAME =	DESIGNED - EJA	REVISED -
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PLOT DATE = #DATE#	DATE - DECEMBER 9, 2013	REVISED -

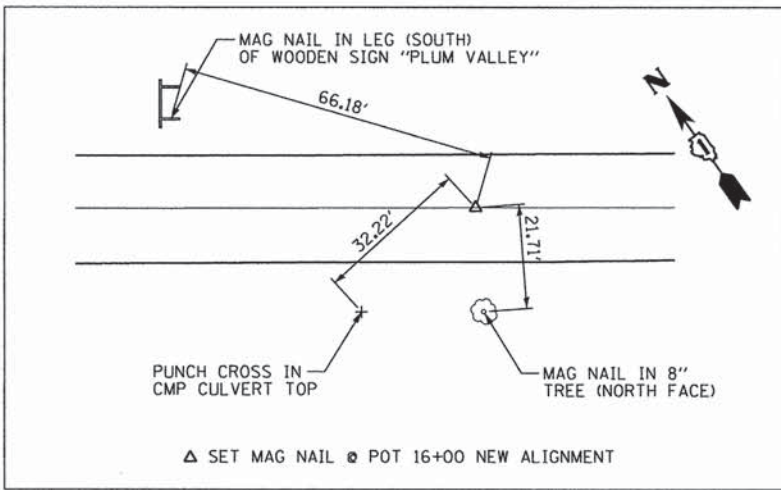
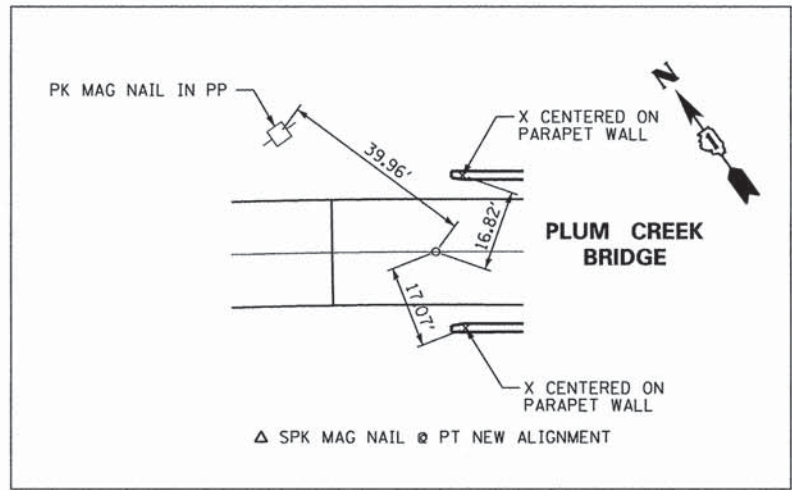


**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

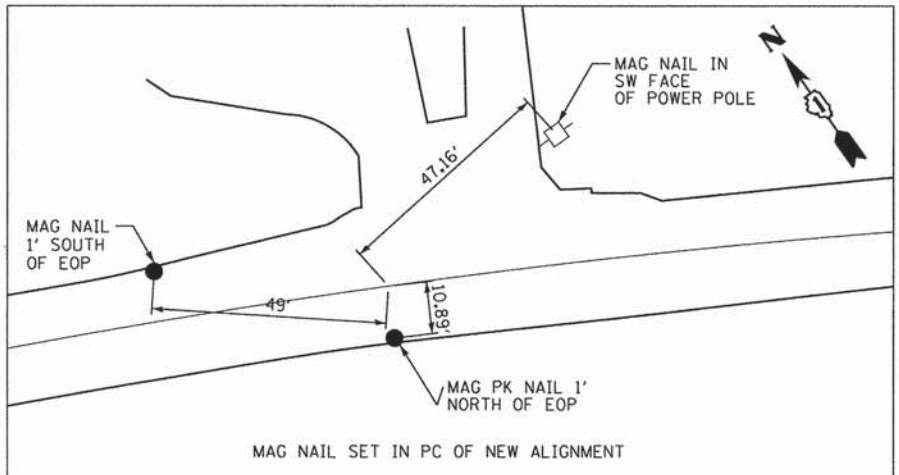
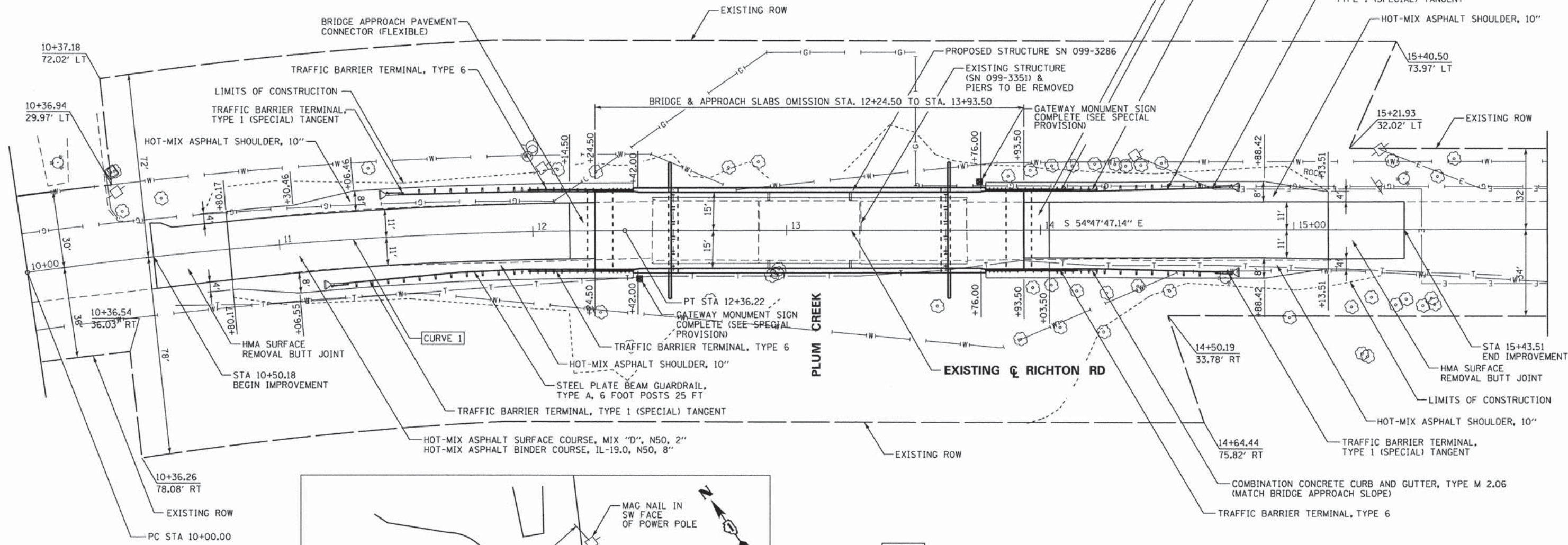
REMOVAL PLAN RICHTON ROAD

SCALE: 1"=20' SHEET NO. 1 OF 1 SHEETS STA. TO STA.

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	10
ILLINOIS FED. AID PROJECT			CONTRACT NO. 61A02	



BENCHMARK: CHISELED SQUARE ON NW WINGWALL AGAINST PARAPET WALL. STA. 12+47.11, 13.06' LT. ELEV. 674.93 (NAVD88)



CURVE 1
 P.I. STA = 11+18.30 (N 1,745,772.3068 E 1,198,760.9288)
 Δ = 7°49'51.79" (RT)
 R = 1728.3350'
 T = 118.2966'
 L = 236.2248'
 E = 4.0437'
 e = N.C.
 P.C. STA = 10+00.00 (N 1,745,826.6965 E 1,198,655.8771)
 P.T. STA = 12+36.22 (N 1,745,704.1108 E 1,198,857.5900)

FILE NAME =	DESIGNED - EJA	REVISED -
#FILE#	DRAWN - JLW	REVISED -
USER NAME = #USER#	CHECKED - MPM	REVISED -
PLOT DATE = #DATE#	DATE - DECEMBER 9, 2013	REVISED -



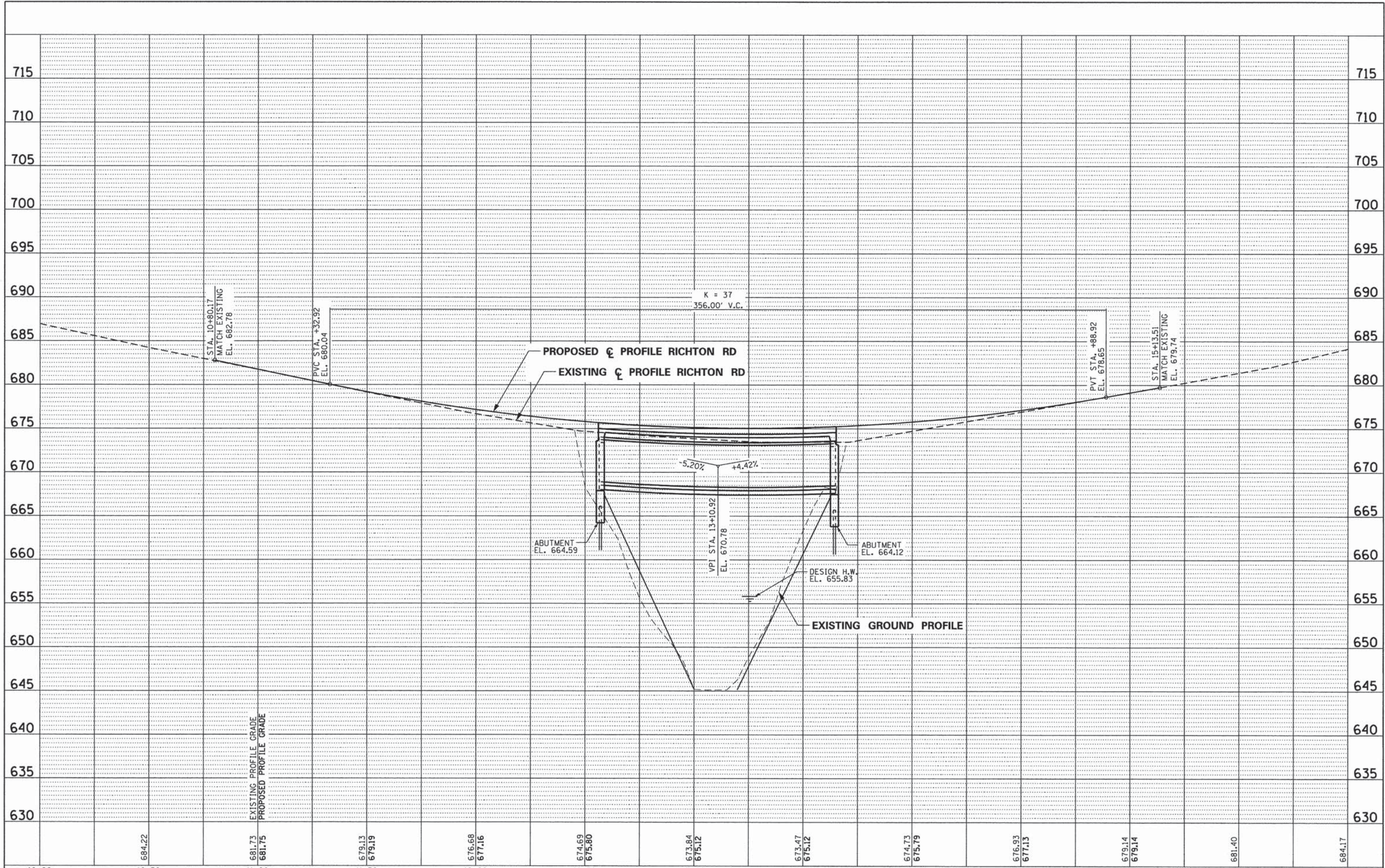
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SCALE: 1"=20'		SHEET NO. 1 OF 1 SHEETS		STA.	TO STA.
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T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	11
CONTRACT NO. 61A02				
ILLINOIS FED. AID PROJECT				

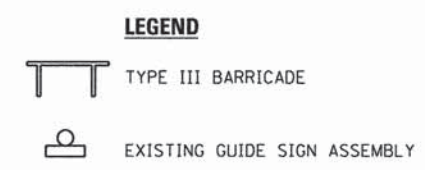
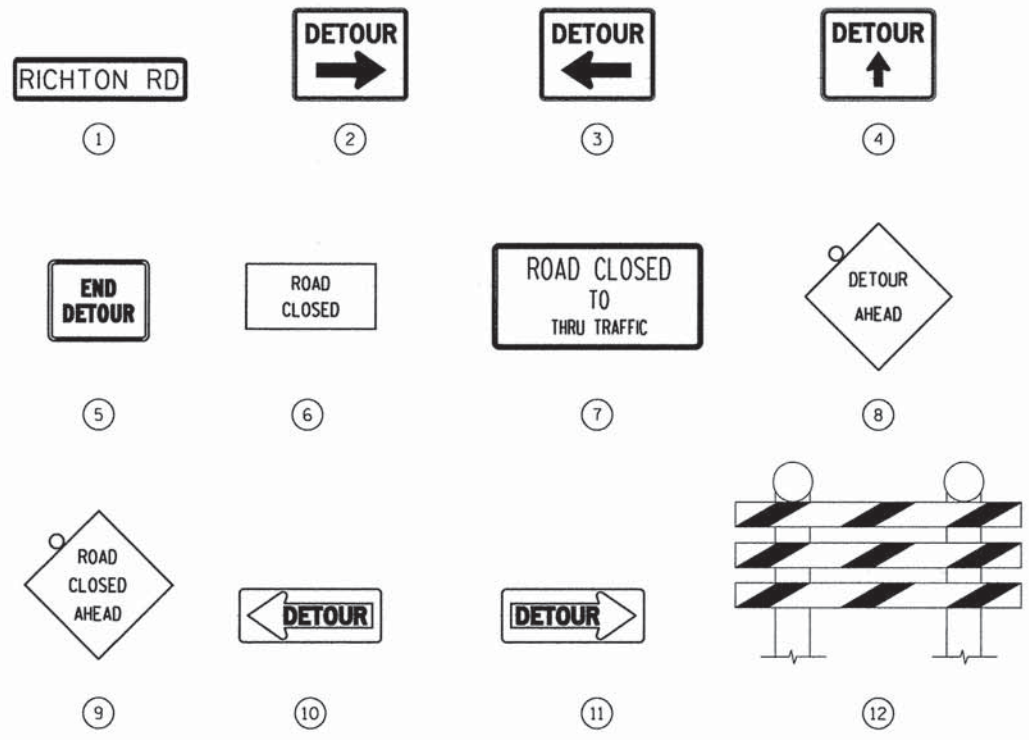
PLAN	SURVEYED	BY	DATE
	PLOTTED		
	CHECKED		
	ALIGNED		
	FILE NAME		
	NO.		

PROFILE	SURVEYED	BY	DATE
	PLOTTED		
	CHECKED		
	GRADES		
	STRUCTURE		
	NOTATION		
	NO.		

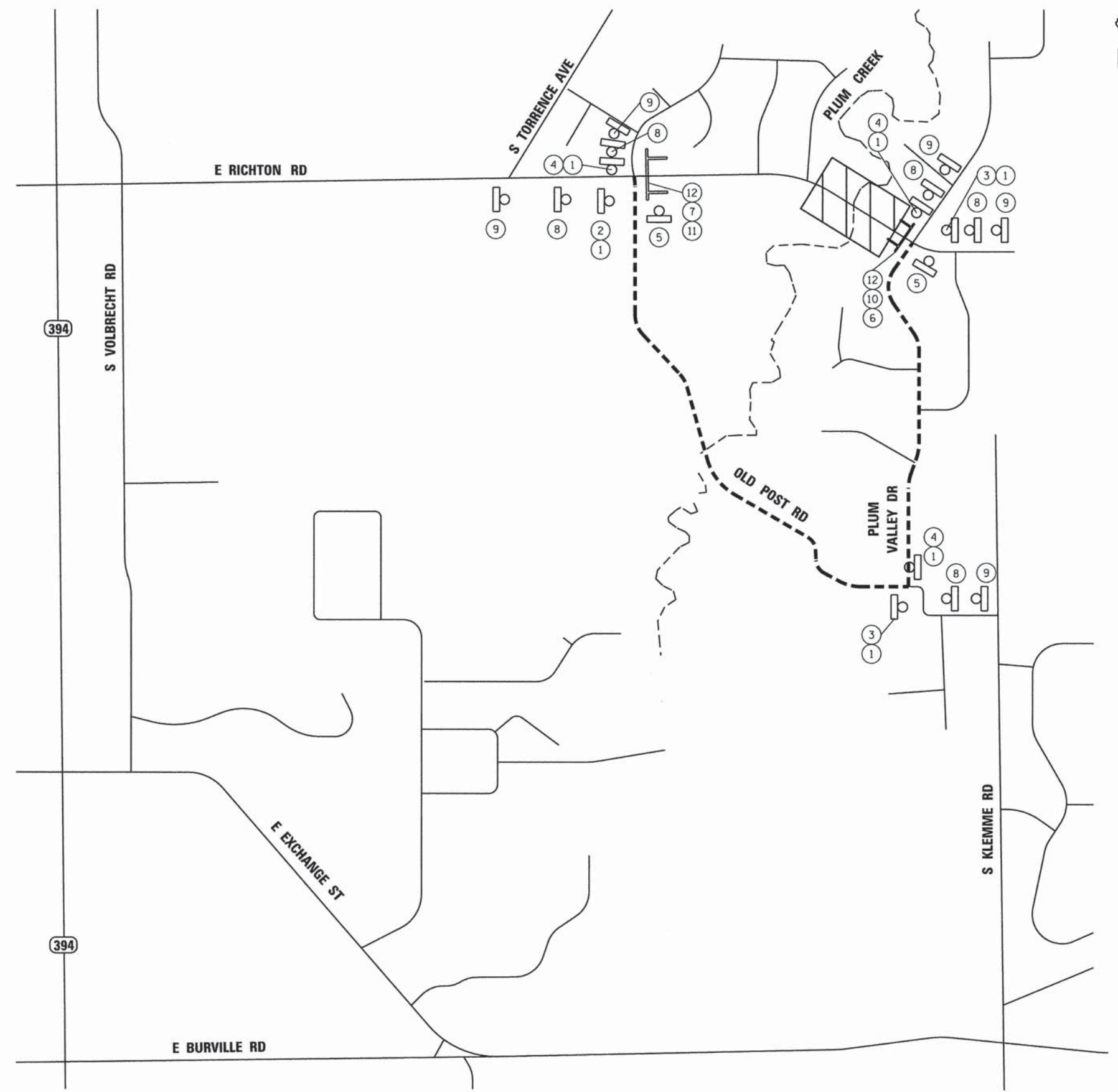


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PLOT DATE = #DATE#	DATE - DECEMBER 9, 2013	REVISED -										ILLINOIS FED. AID PROJECT				

SCALE: H 1"=20', V 1"=5' SHEET NO. 1 OF 1 SHEETS STA. 10+80.17 TO STA. 15+13.51

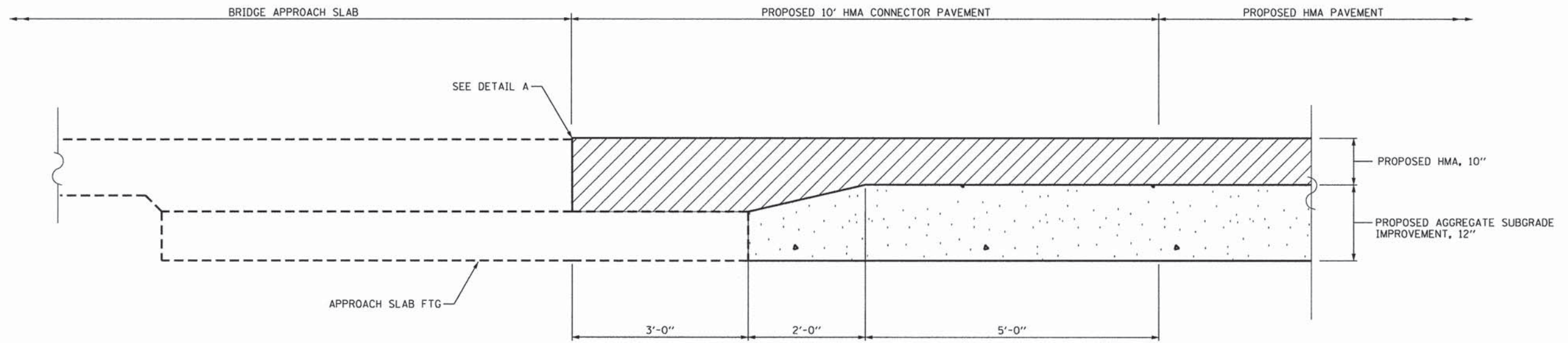


NOTE
 THE CONTRACTOR SHALL INSPECT ALL EXISTING DETOUR SIGNING AND CONFIRM WITH THE TOWNSHIP THE TYPE, CONDITION, AND LOCATION OF ALL EXISTING SIGNS. THE CONTRACTOR WILL ASSUME FULL RESPONSIBILITY FOR INSPECTION, MAINTENANCE AND REPLACEMENT OF ALL DETOUR SIGNS THROUGHOUT THE DURATION OF THE PROJECT. ANY SIGNING THAT IS DAMAGED OR REMOVED DUE TO NO FAULT OF THE CONTRACTOR SHALL BE REPLACED IN KIND. THE COST OF SIGN REPLACEMENT WILL BE INCLUDED IN TRAFFIC CONTROL AND PROTECTION (SPECIAL). AT THE END OF THE PROJECT, THE CONTRACTOR SHALL REMOVE ALL SIGNS AND RETURN THEM TO THE TOWNSHIP.

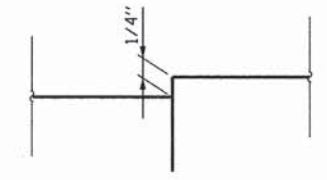


(NOT TO SCALE)

FILE NAME *	DESIGNED - EJA	REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION			RICHTON ROAD DETOUR PLAN			T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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PLOT DATE * #DATE#	DATE - DECEMBER 9, 2013	REVISED -		ILLINOIS FED. AID PROJECT										




BRIDGE APPROACH PAVEMENT CONNECTOR (HMA)

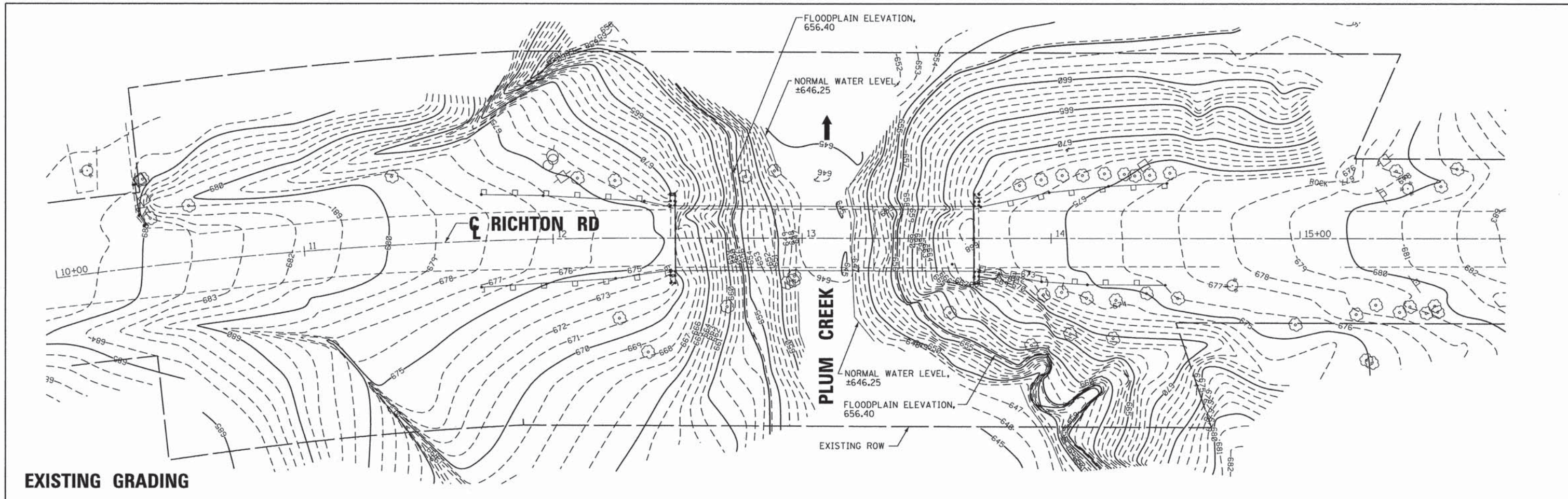


DETAIL A

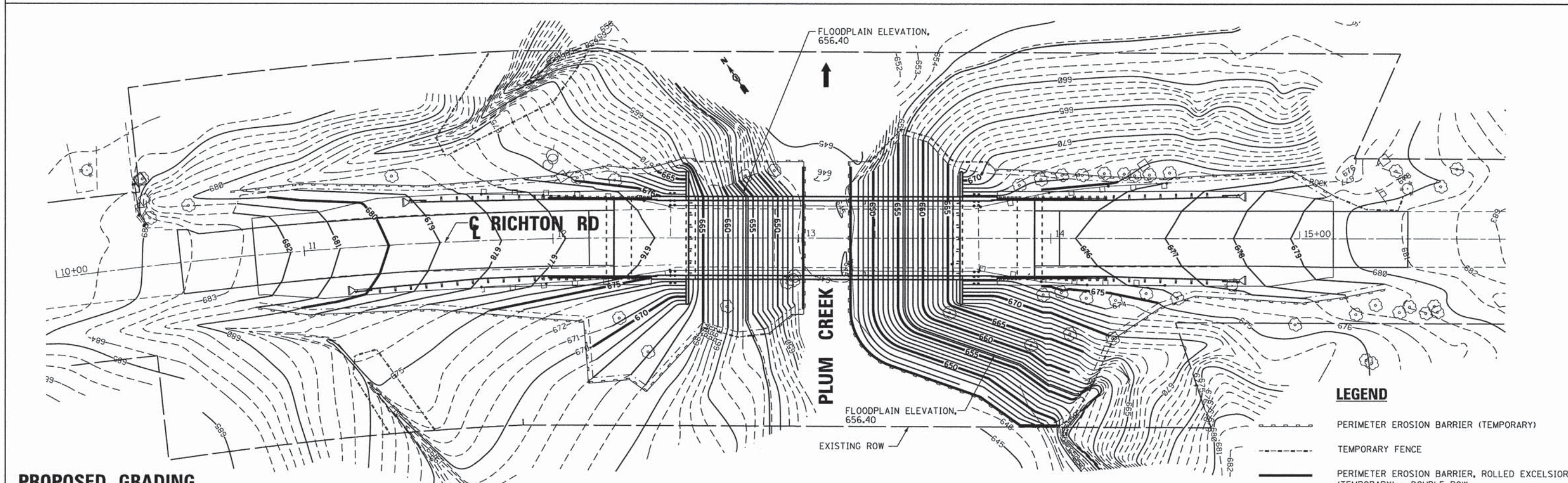
NOTE:

- 1. SEE HWY. STD. 420401 FOR DETAILS NOT SHOWN.

FILE NAME =	DESIGNED - EJA	REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	ROADWAY DETAILS				T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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PLOT DATE = #DATE#	DATE - DECEMBER 9, 2013	REVISED -											



EXISTING GRADING



PROPOSED GRADING

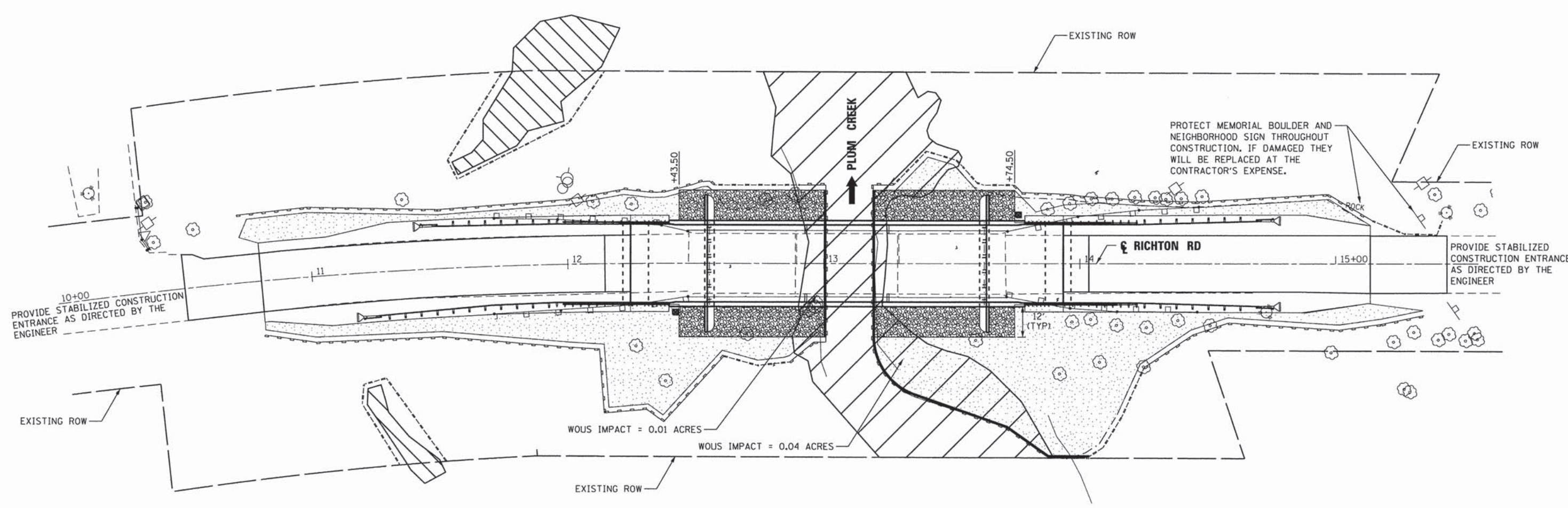
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PLOT DATE = #DATE#	DATE - DECEMBER 9, 2013	REVISED -










STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PROPOSED GRADING PLAN	
SCALE: 1"=20'	SHEET NO. 1 OF 1 SHEETS
STA.	TO STA.

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	15
CONTRACT NO. 61A02				
[ILLINOIS] FED. AID PROJECT				



LEGEND

-  PERIMETER EROSION BARRIER (TEMPORARY)
-  TEMPORARY FENCE
-  PERIMETER EROSION BARRIER, ROLLED EXCELSIOR (TEMPORARY) - DOUBLE ROW
-  STONE RIPRAP, CLASS A4 (SPECIAL)
-  SEEDING, CLASS 4A, 5, AND 7 WITH EROSION CONTROL BLANKET (FOR LIMITS OF SEEDING SEE GENERAL NOTE 1)
-  EXISTING WETLANDS (NOT IMPACTED)
-  WATERS OF THE U.S. (NOT IMPACTED)

FILE NAME =	DESIGNED - EJA	REVISED -
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PLOT DATE = #DATE#	DATE - DECEMBER 9, 2013	REVISED -

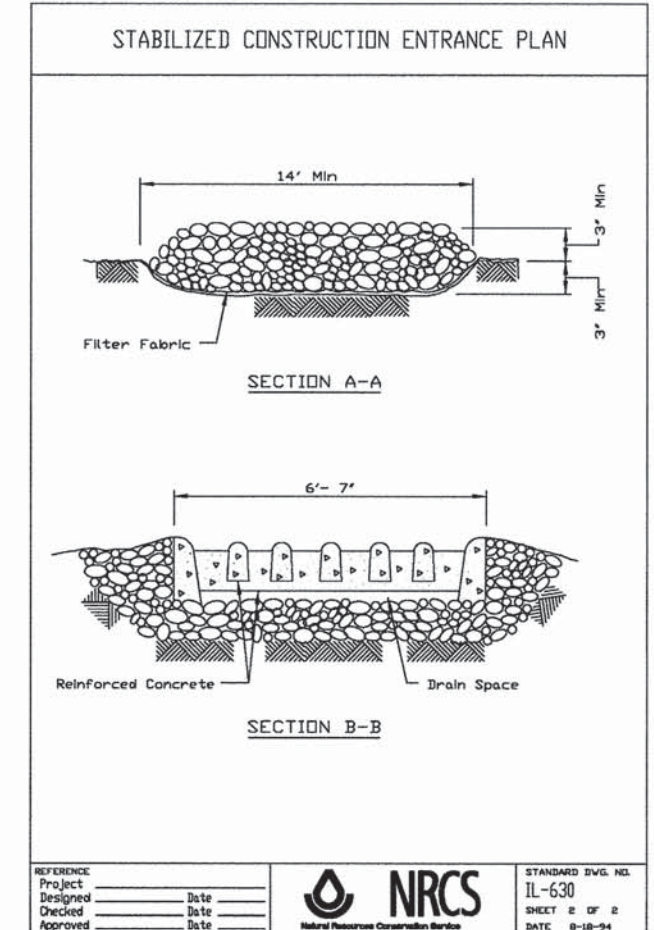
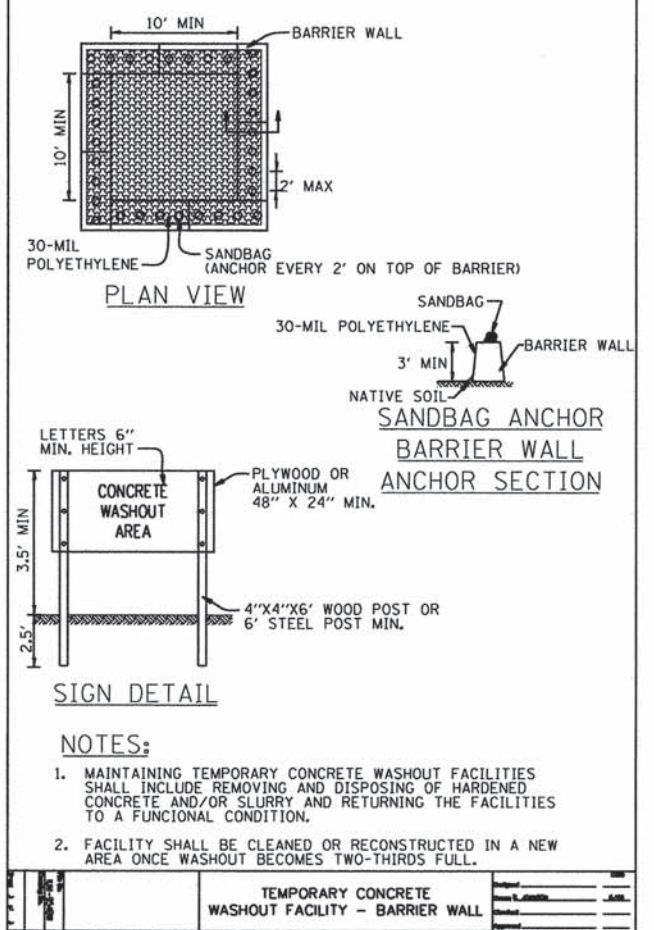
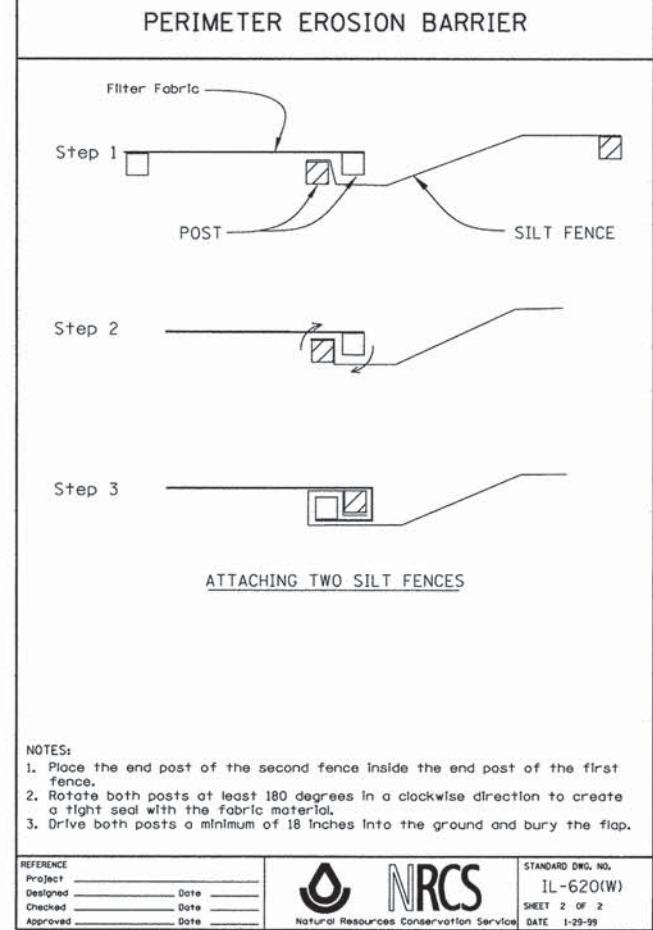
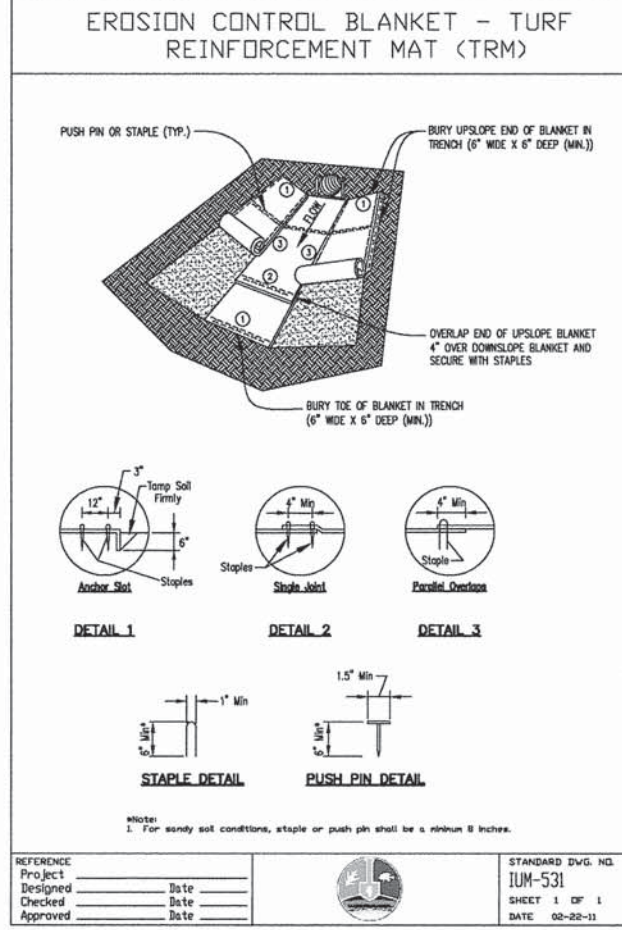
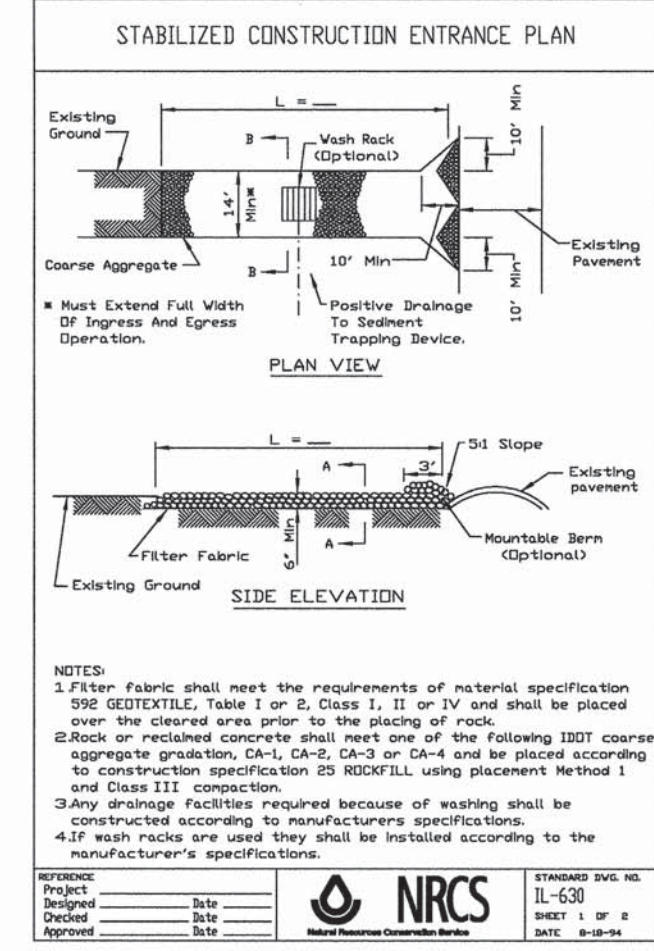
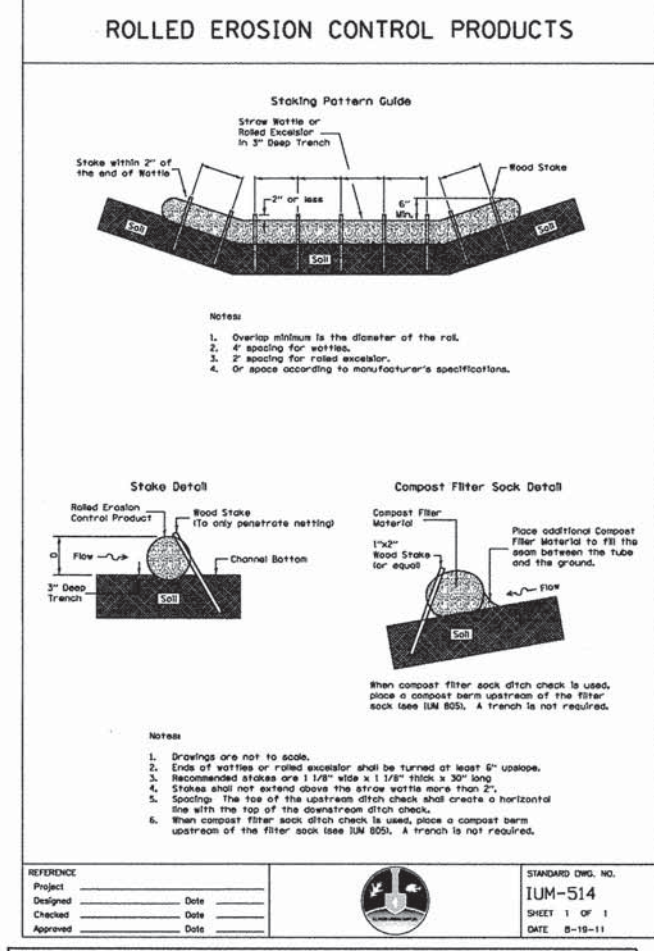
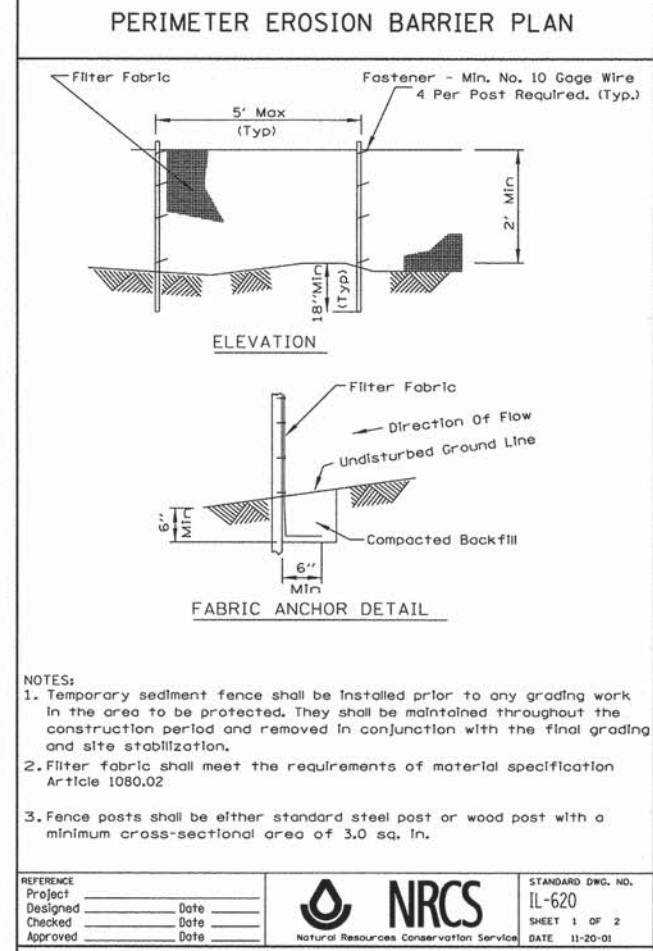
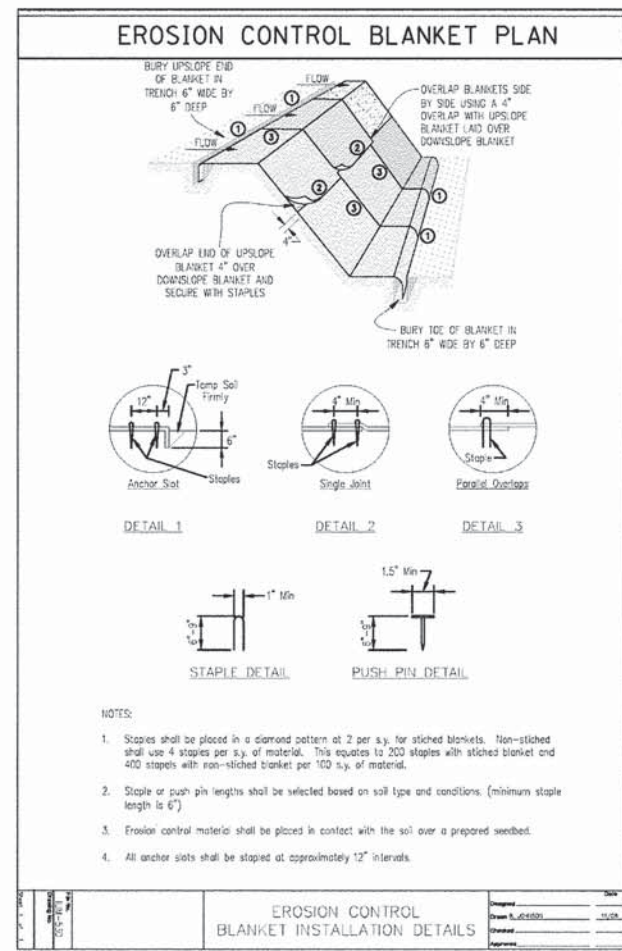


**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

EROSION AND SEDIMENT CONTROL PLAN AND LANDSCAPING PLAN

SCALE: 1"=20' SHEET NO. 1 OF 1 SHEETS STA. TO STA.

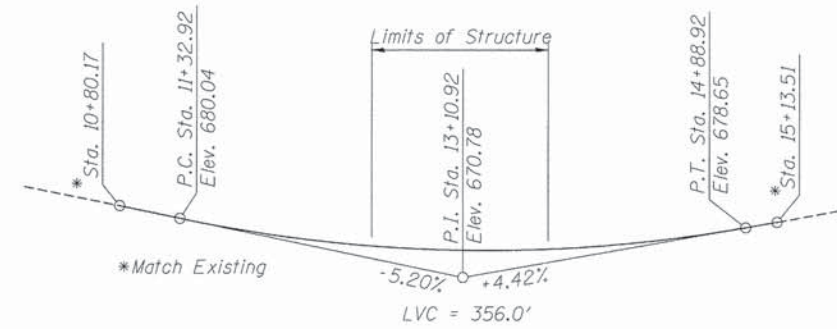
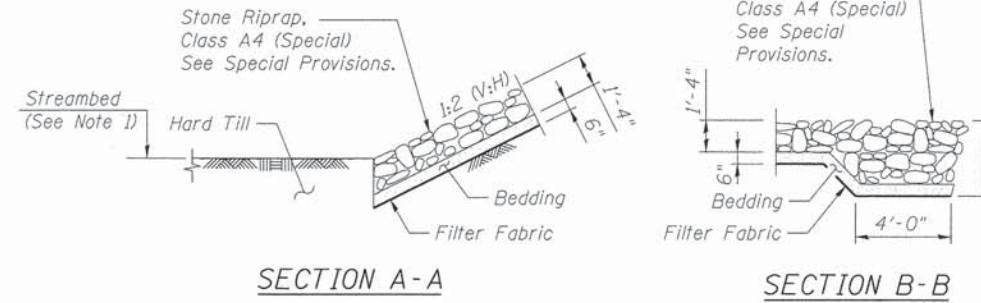
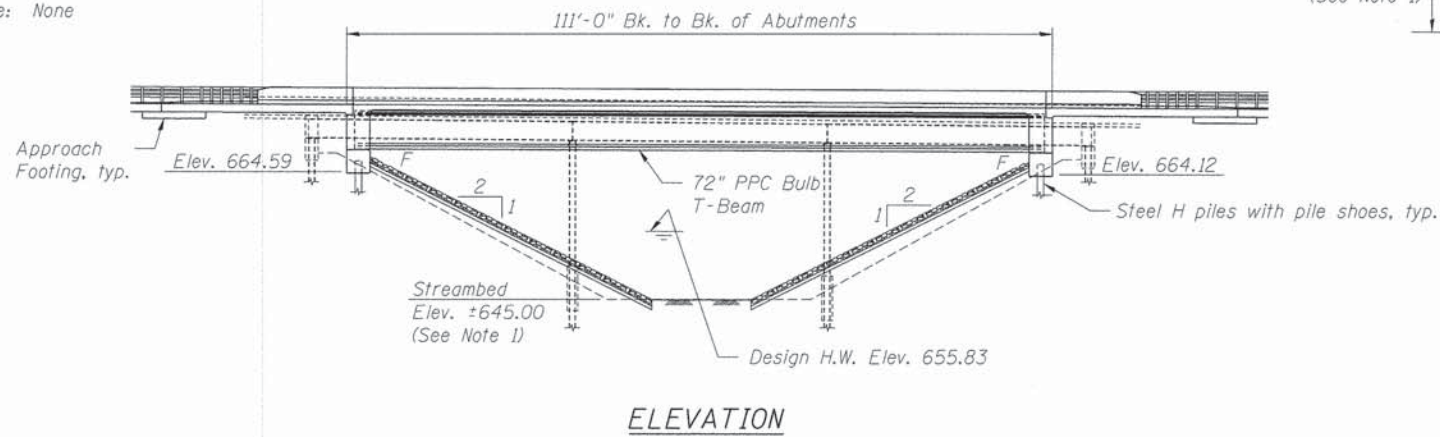
T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	16
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	



Benchmark: Chiseled square on NW wingwall against parapet wall, Sta. 12+47.11, 13.06' / Lt. Elev. 674.93 (NAVD88).

Existing Structure: SN 099-3351 was originally built in 1978 as Richton Road over Plum Creek and consists of a three span non-continuous W30x90 weathering steel beam superstructure on pile bent abutments and piers. The bridge is 124'-1 1/2" back to back of abutments and 26'-0" out to out. Bridge was closed to traffic in 2012. Structure to be removed and replaced while traffic utilizes the existing detour.

Salvage: None



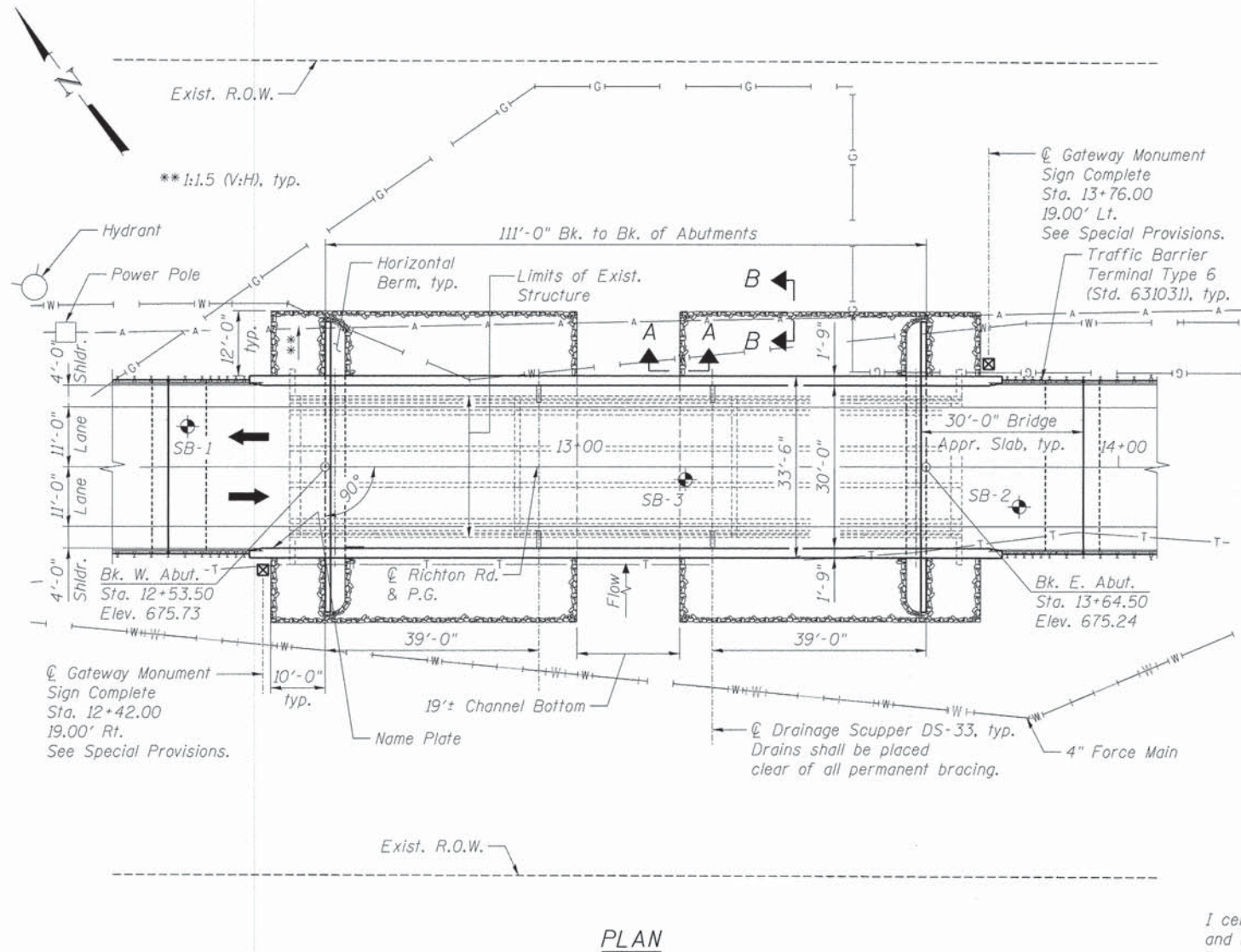
PROFILE GRADE
(along \bar{C} Richton Road)

WATERWAY INFORMATION

Flood	Freq. Yr.	C.F.S.	Opening Sq. Ft.		Natural H.W.E.	Head - Ft.		Headwater El.	
			Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
Design	30	1500	401	379	655.83	0.69	0.05	656.52	655.88
Base	100	1800	436	413	656.40	0.70	0.05	657.10	656.45
Overtopping Existing	-	-	-	-	-	-	-	-	-
Overtopping Proposed	-	-	-	-	-	-	-	-	-
Max. Calc.	500	2370	493	469	657.29	0.72	0.04	658.01	657.33

DESIGN SCOUR ELEVATION TABLE

Design Scour Elevations (ft.)	
W. Abut.	E. Abut.
664.59	664.12



PLAN

DESIGN SPECIFICATIONS

2012 AASHTO LRFD Bridge Design Specifications, 6th Edition, with 2013 Interims

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface

DESIGN STRESSES

FIELD UNITS

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinforcement)

PRECAST PRESTRESSED UNITS

$f'_c = 6,000$ psi
 $f'_ci = 5,000$ psi
 $f_{pu} = 270,000$ psi ($1/2$ " ϕ low lax. strands)
 $f_{pbt} = 201,960$ psi ($1/2$ " ϕ low lax. strands)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration at 1.0 sec. (S_{D1}) = 0.068 g
Design Spectral Acceleration at 0.2 sec. (S_{D5}) = 0.120 g
Soil Site Class = C

PLUM CREEK
BUILT 20__ BY
CRETE TOWNSHIP
SEC. 11-02118-01-BR
T.R. 428 STA. 13+09.00
STR. NO. 099-3286 LOADING HL-93

NAME PLATE

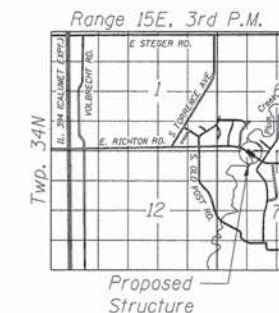
(See Std. 515001)

NOTES:

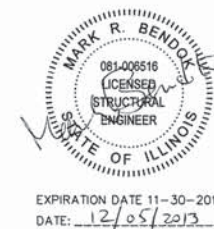
1. Replace disturbed stream bed with natural earth material.
2. For utilities to be relocated, see Roadway Plans and Special Provisions.

LEGEND

- SB-1 - Indicates Soil Boring Location and Number
- G— Exist. Underground Gas Line
- W— Exist. Underground Water Line
- A— Exist. Aerial Line
- T— Exist. Underground Telephone



LOCATION SKETCH



I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AASHTO LRFD Bridge Design Specification.

benesch
engineers - scientists - planners
Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-585-0450 Job No. 10204

FILE NAME =	USER NAME = rgr:mm	DESIGNED - AWH	REVISED -
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	PLOT SCALE =	DRAWN - RMG	REVISED -
	PLOT DATE = 12/5/2013	CHECKED - MRB/MFH	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SHEET NO. S1 OF S26 SHEETS

F.A.J. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	18
			CONTRACT NO. 61A02	
ILLINOIS FED. AID PROJECT				

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GENERAL NOTES

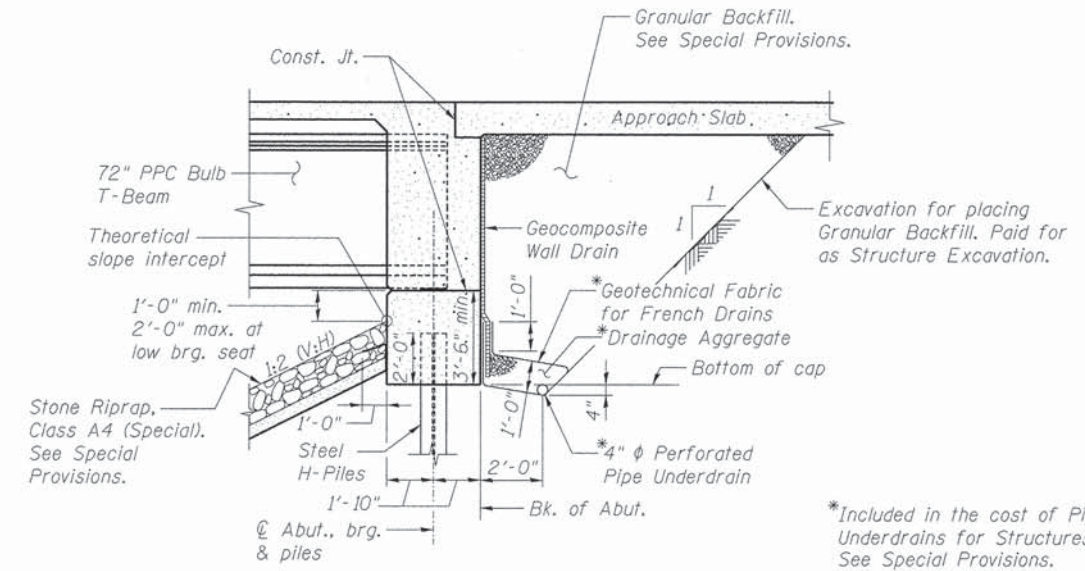
1. Reinforcement bars designated (E) shall be epoxy coated.
2. Layout of slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
3. The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.
4. Slip forming of the parapets is not allowed.
5. The existing bridge is closed to traffic and has a Sufficiency Rating = 29.0. Prior to starting the work, the Contractor shall submit to the Engineer for review and acceptance a Demolition Plan in accordance with Article 501 of the Standard Specifications.
6. The removal of the existing broken concrete under the bridge shall be included in the cost of "Removal of Existing Structures" and shall be done to the limits and depth of the proposed grouted riprap. No additional compensation will be made for removal outside these limits than is required to complete the work.
7. The contractor shall not allow concrete or other demolition or construction materials or debris to fall or roll into the creek during demolition and construction operations. Any cost associated with preventive measures shall not be paid for separately but be considered as included in the unit price(s) for the associated work.
8. The Contractor shall maintain at all times a 10 foot minimum horizontal clearance from the overhead ComEd 12 kV lines. If this minimum clearance cannot be maintained, the Contractor shall request from ComEd a power outage. ComEd requires up to 16 weeks of lead time for any outage. Any necessary outage will be at the expense of the Contractor.

INDEX OF SHEETS

- S1 General Plan and Elevation
- S2 General Data
- S3 Footing Layout
- S4 Top of Deck Elevation Layout
- S5 Top of Deck Elevations
- S6 Top of Approach Slab Elevations
- S7 Superstructure
- S8 Superstructure Details
- S9 Aluminum Rail Details
- S10 Diaphragm Details
- S11 Bridge Approach Slab Details 1 of 2
- S12 Bridge Approach Slab Details 2 of 2
- S13 Drainage Scupper DS-33
- S14 Framing Plan
- S15 72" PPC Bulb T-Beam
- S16 72" PPC Bulb T-Beam Details
- S17 West and East Abutment Details
- S18 HP Pile Details
- S19 Soil Boring Logs 1 of 3
- S20 Soil Boring Logs 2 of 3
- S21 Soil Boring Logs 3 of 3
- S22 Existing Plans 1 of 5
- S23 Existing Plans 2 of 5
- S24 Existing Plans 3 of 5
- S25 Existing Plans 4 of 5
- S26 Existing Plans 5 of 5

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Filter Fabric	Sq Yd		707	707
Removal of Existing Structures	Each			1
Structure Excavation	Cu Yd		49	49
Concrete Structures	Cu Yd		65.3	65.3
Concrete Superstructure	Cu Yd	290.8		290.8
Bridge Deck Grooving	Sq Yd	526		526
Protective Coat	Sq Yd	674		674
Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams 72"	Foot	542.0		542.0
Reinforcement Bars, Epoxy Coated	Pound	54,250	11,820	66,070
Furnishing Steel Piles HP14x73	Foot		650	650
Driving Piles	Foot		650	650
Test Pile Steel HP14x73	Each		2	2
Pile Shoes	Each		12	12
Name Plates	Each	1		1
Geocomposite Wall Drain	Sq Yd		109	109
Drainage Scuppers, DS-33	Each	4		4
Pipe Underdrains For Structures, 4"	Foot		142	142
Stone Riprap, Class A4 (Special)	Sq Yd		707	707
Granular Backfill for Structures	Cu Yd		293	293
Gateway Monument Sign Complete	Each	2		2
Railing	Foot	258		258



SECTION THRU INTEGRAL ABUTMENT

Note:

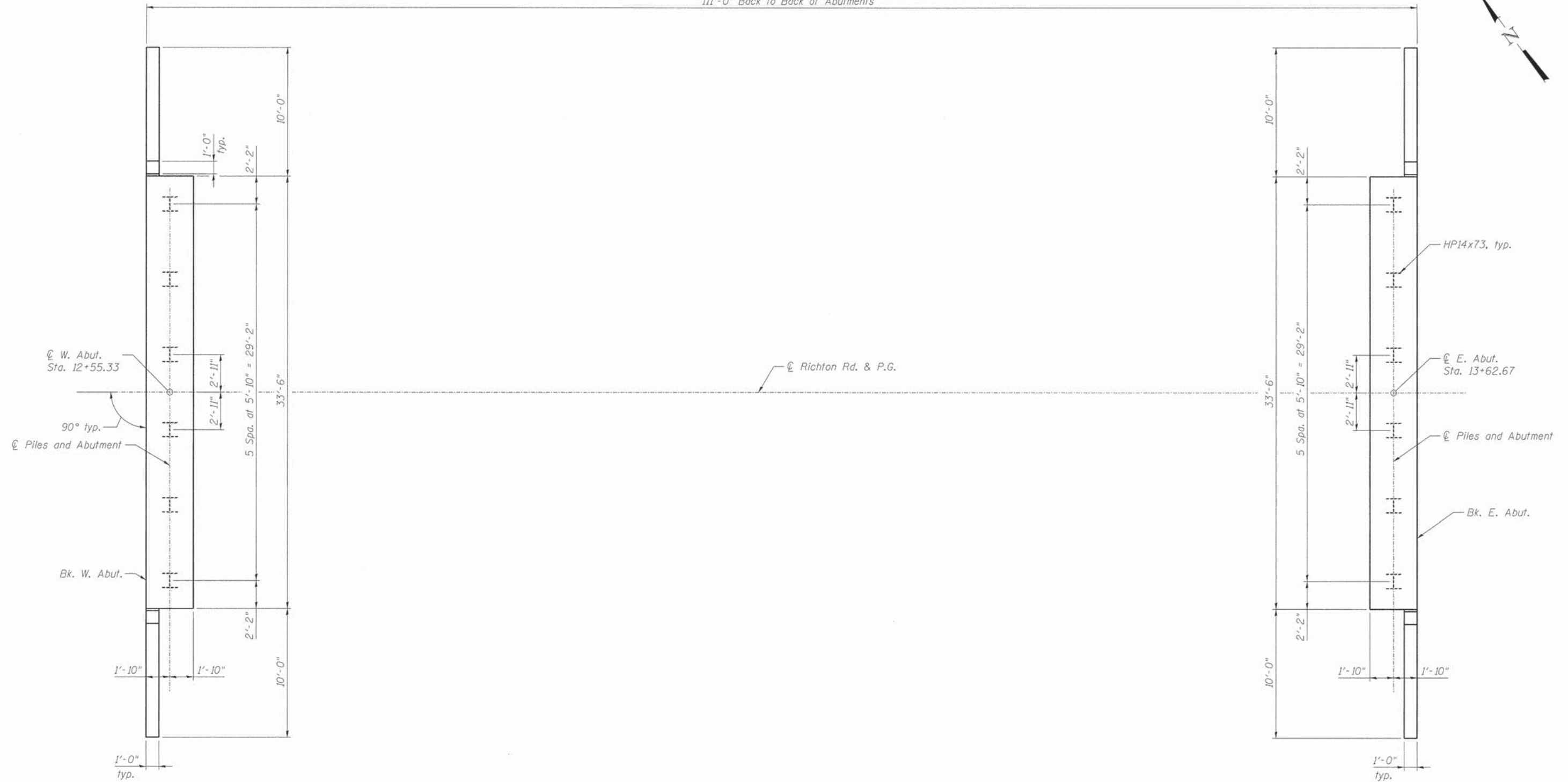
All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 60110).

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		DRAWN - RMG	REVISED -
		CHECKED - MRB/MFH	REVISED -

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	19
CONTRACT NO. 61A02				
ILLINOIS FED. AID PROJECT				

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111'-0" Back to Back of Abutments



PLAN

LEGEND:

⋮ Indicates vertical pile.

NOTE:

There will be one test pile at each abutment. Location to be determined in the field by the Engineer.

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FILE NAME =
099_3286_61A02_03_f.tlyt.dgn

USER NAME = rgrimm
PLOT SCALE =
PLOT DATE = 12/5/2013

DESIGNED - AWH
CHECKED - MRB/MFH
DRAWN - RMG
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REVISED -
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REVISED -

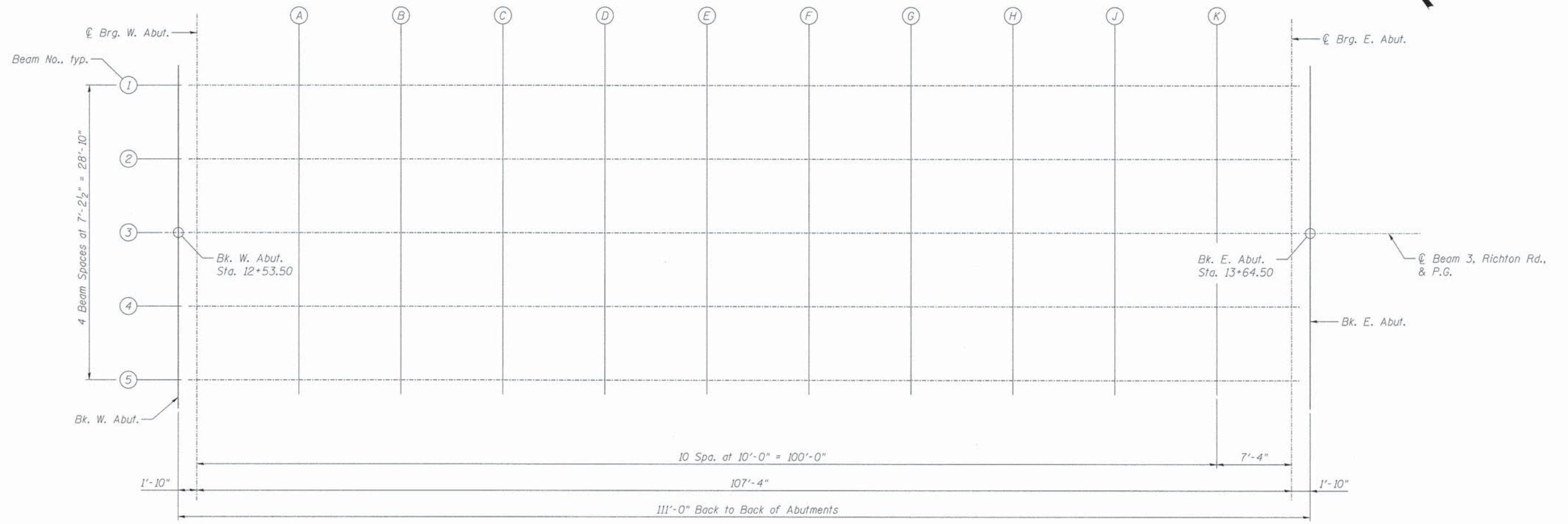
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FOOTING LAYOUT
STRUCTURE NO. 099-3286

SHEET NO. 53 OF 526 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	20
				CONTRACT NO. 61A02
ILLINOIS FED. AID PROJECT				

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PLAN

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FILE NAME =	USER NAME = rgrimm	DESIGNED - AWH	REVISED -
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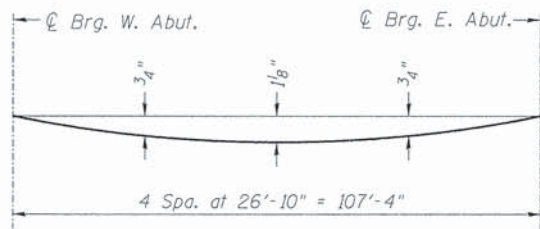
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

TOP OF DECK ELEVATION LAYOUT
 STRUCTURE NO. 099-3286

SHEET NO. S4 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	21
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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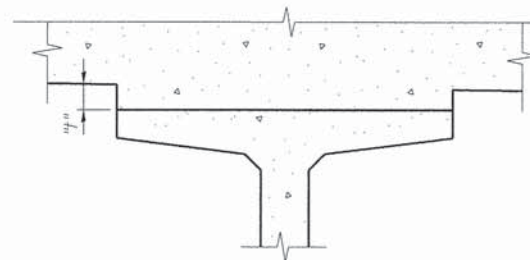


DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete, excluding beams)

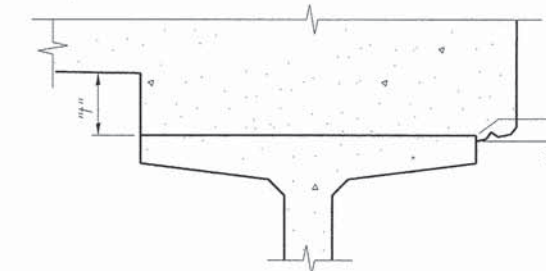
Note:

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.



INTERIOR BEAM

To determine "t": After all PPC beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted For Dead Load Deflection" shown below, minus slab thickness, equals the fillet height "t" above top flange of beams.



EXTERIOR BEAM

FILLET HEIGHTS

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	12+53.50	-14.42	675.49	675.49
☉ Brg. W. Abut.	12+55.33	-14.42	675.45	675.45
A	12+65.33	-14.42	675.28	675.30
B	12+75.33	-14.42	675.13	675.18
C	12+85.33	-14.42	675.01	675.08
D	12+95.33	-14.42	674.91	674.99
E	13+05.33	-14.42	674.85	674.93
F	13+15.33	-14.42	674.80	674.89
G	13+25.33	-14.42	674.79	674.87
H	13+35.33	-14.42	674.80	674.87
J	13+45.33	-14.42	674.85	674.89
K	13+55.33	-14.42	674.91	674.93
☉ Brg. E. Abut.	13+62.67	-14.42	674.98	674.98
Bk. E. Abut.	13+64.50	-14.42	675.00	675.00

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	12+53.50	-7.21	675.62	675.62
☉ Brg. W. Abut.	12+55.33	-7.21	675.58	675.58
A	12+65.33	-7.21	675.41	675.44
B	12+75.33	-7.21	675.26	675.31
C	12+85.33	-7.21	675.14	675.21
D	12+95.33	-7.21	675.04	675.13
E	13+05.33	-7.21	674.98	675.07
F	13+15.33	-7.21	674.94	675.03
G	13+25.33	-7.21	674.92	675.01
H	13+35.33	-7.21	674.94	675.00
J	13+45.33	-7.21	674.98	675.02
K	13+55.33	-7.21	675.04	675.06
☉ Brg. E. Abut.	13+62.67	-7.21	675.11	675.11
Bk. E. Abut.	13+64.50	-7.21	675.13	675.13

BEAM 3 & ☉ RICHTON ROAD & P.G.

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	12+53.50	0.00	675.73	675.73
☉ Brg. W. Abut.	12+55.33	0.00	675.70	675.70
A	12+65.33	0.00	675.52	675.55
B	12+75.33	0.00	675.37	675.43
C	12+85.33	0.00	675.25	675.32
D	12+95.33	0.00	675.16	675.24
E	13+05.33	0.00	675.09	675.18
F	13+15.33	0.00	675.05	675.14
G	13+25.33	0.00	675.03	675.12
H	13+35.33	0.00	675.05	675.12
J	13+45.33	0.00	675.09	675.14
K	13+55.33	0.00	675.16	675.18
☉ Brg. E. Abut.	13+62.67	0.00	675.22	675.22
Bk. E. Abut.	13+64.50	0.00	675.24	675.24

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	12+53.50	7.21	675.62	675.62
☉ Brg. W. Abut.	12+55.33	7.21	675.58	675.58
A	12+65.33	7.21	675.41	675.44
B	12+75.33	7.21	675.26	675.31
C	12+85.33	7.21	675.14	675.21
D	12+95.33	7.21	675.04	675.13
E	13+05.33	7.21	674.98	675.07
F	13+15.33	7.21	674.94	675.03
G	13+25.33	7.21	674.92	675.01
H	13+35.33	7.21	674.94	675.00
J	13+45.33	7.21	674.98	675.02
K	13+55.33	7.21	675.04	675.06
☉ Brg. E. Abut.	13+62.67	7.21	675.11	675.11
Bk. E. Abut.	13+64.50	7.21	675.13	675.13

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	12+53.50	14.42	675.49	675.49
☉ Brg. W. Abut.	12+55.33	14.42	675.45	675.45
A	12+65.33	14.42	675.28	675.30
B	12+75.33	14.42	675.13	675.18
C	12+85.33	14.42	675.01	675.08
D	12+95.33	14.42	674.91	674.99
E	13+05.33	14.42	674.85	674.93
F	13+15.33	14.42	674.80	674.89
G	13+25.33	14.42	674.79	674.87
H	13+35.33	14.42	674.80	674.87
J	13+45.33	14.42	674.85	674.89
K	13+55.33	14.42	674.91	674.93
☉ Brg. E. Abut.	13+62.67	14.42	674.98	674.98
Bk. E. Abut.	13+64.50	14.42	675.00	675.00

NORTH EDGE OF SHOULDER

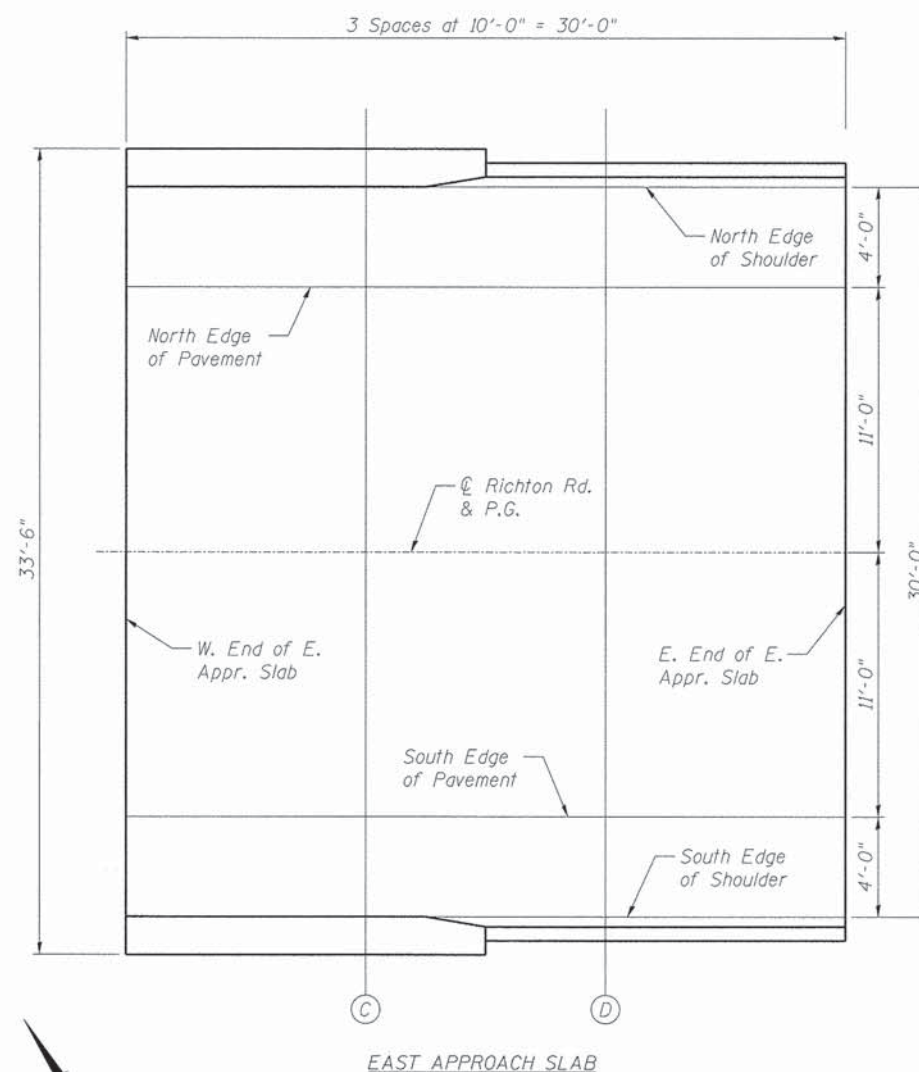
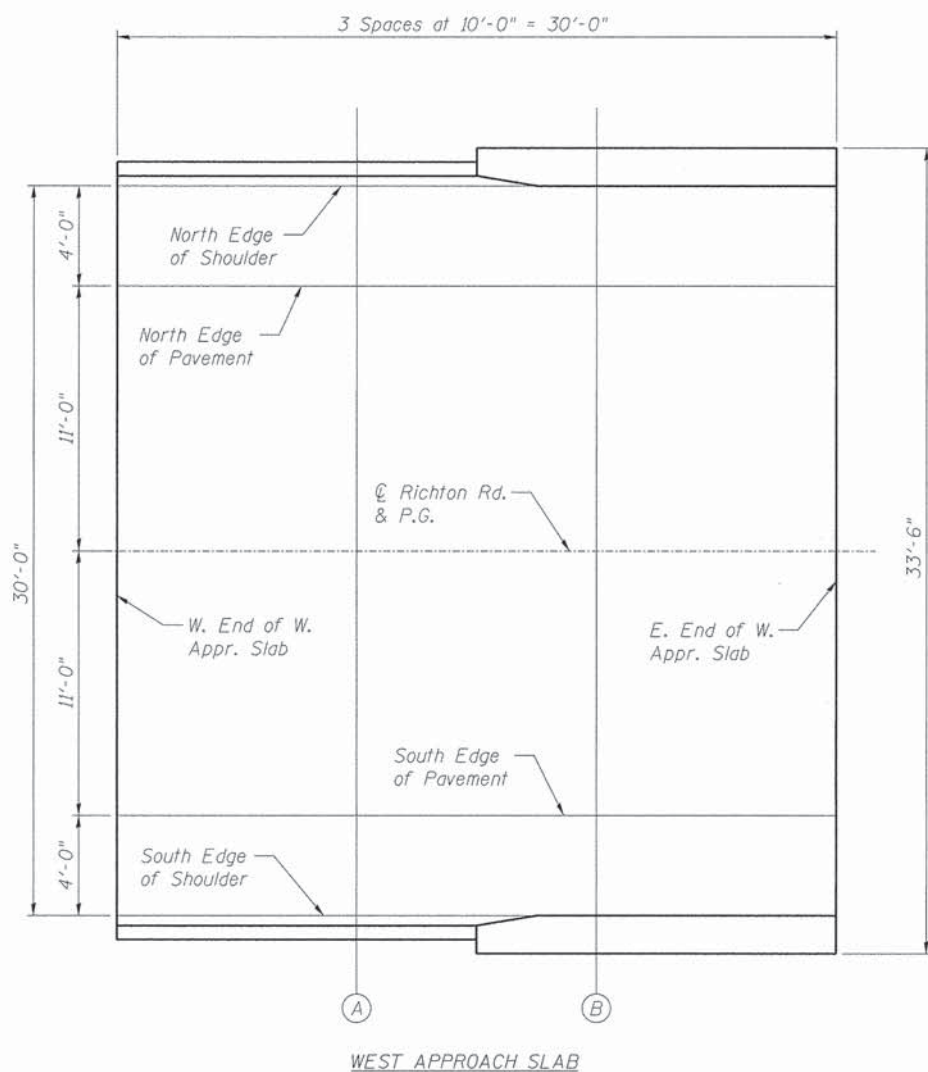
Location	Station	Offset	Theoretical Grade Elevations
W. End of W. Appr. Slab	12+24.50	-15.00	676.15
A	12+34.50	-15.00	675.90
B	12+44.50	-15.00	675.66
E. End of W. Appr. Slab	12+54.50	-15.00	675.46
W. End of E. Appr. Slab	13+63.50	-15.00	674.98
C	13+73.50	-15.00	675.09
D	13+83.50	-15.00	675.24
E. End of E. Appr. Slab	13+93.50	-15.00	675.41

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. Appr. Slab	12+24.50	-11.00	676.24
A	12+34.50	-11.00	675.98
B	12+44.50	-11.00	675.75
E. End of W. Appr. Slab	12+54.50	-11.00	675.54
W. End of E. Appr. Slab	13+63.50	-11.00	675.06
C	13+73.50	-11.00	675.18
D	13+83.50	-11.00	675.32
E. End of E. Appr. Slab	13+93.50	-11.00	675.49

CL RICHTON ROAD & P.G.

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. Appr. Slab	12+24.50	0.00	676.41
A	12+34.50	0.00	676.15
B	12+44.50	0.00	675.92
E. End of W. Appr. Slab	12+54.50	0.00	675.71
W. End of E. Appr. Slab	13+63.50	0.00	675.23
C	13+73.50	0.00	675.35
D	13+83.50	0.00	675.49
E. End of E. Appr. Slab	13+93.50	0.00	675.66



SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. Appr. Slab	12+24.50	11.00	676.24
A	12+34.50	11.00	675.98
B	12+44.50	11.00	675.75
E. End of W. Appr. Slab	12+54.50	11.00	675.54
W. End of E. Appr. Slab	13+63.50	11.00	675.06
C	13+73.50	11.00	675.18
D	13+83.50	11.00	675.32
E. End of E. Appr. Slab	13+93.50	11.00	675.49

SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. Appr. Slab	12+24.50	15.00	676.15
A	12+34.50	15.00	675.90
B	12+44.50	15.00	675.66
E. End of W. Appr. Slab	12+54.50	15.00	675.46
W. End of E. Appr. Slab	13+63.50	15.00	674.98
C	13+73.50	15.00	675.09
D	13+83.50	15.00	675.24
E. End of E. Appr. Slab	13+93.50	15.00	675.41

PLAN



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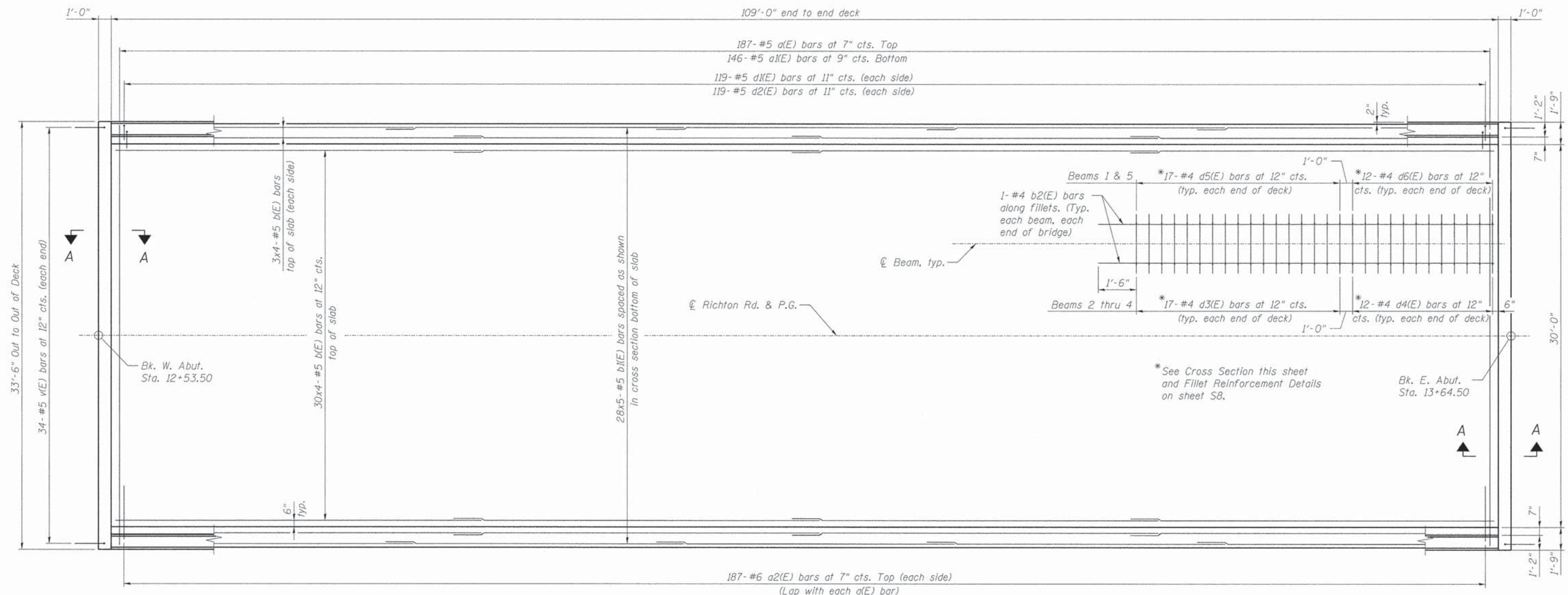
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF APPROACH SLAB ELEVATIONS
STRUCTURE NO. 099-3286

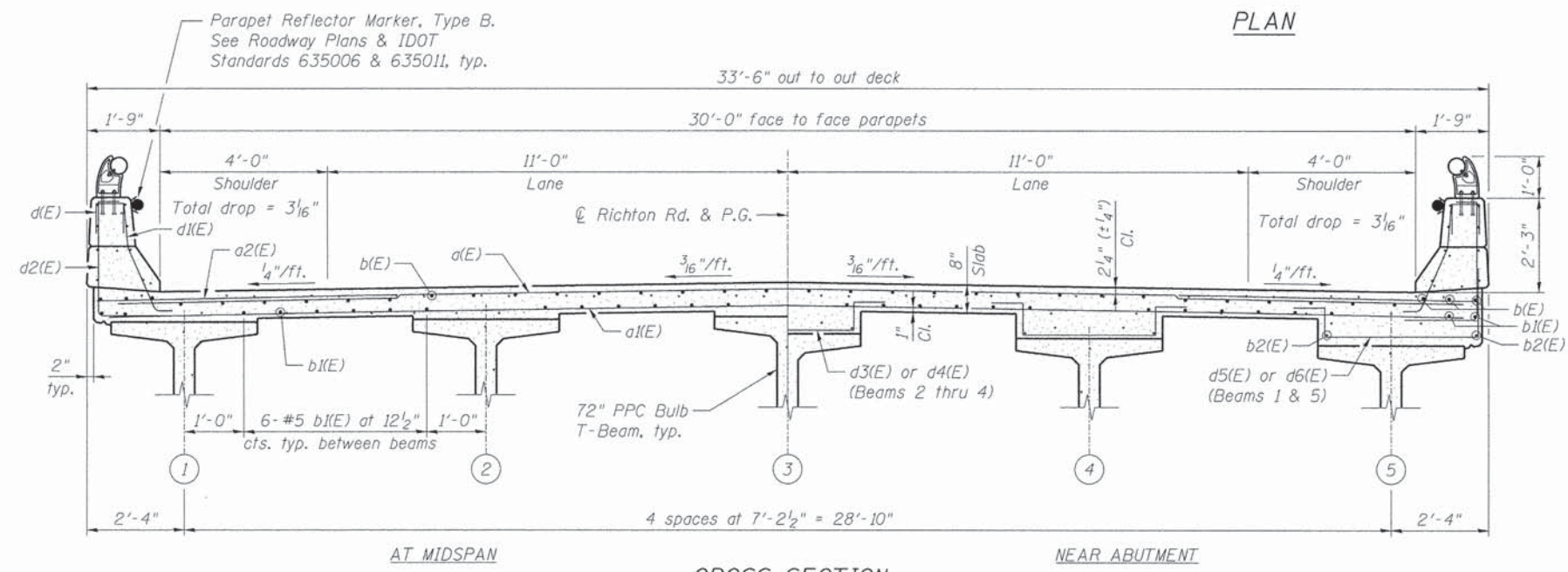
SHEET NO. 56 OF 526 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

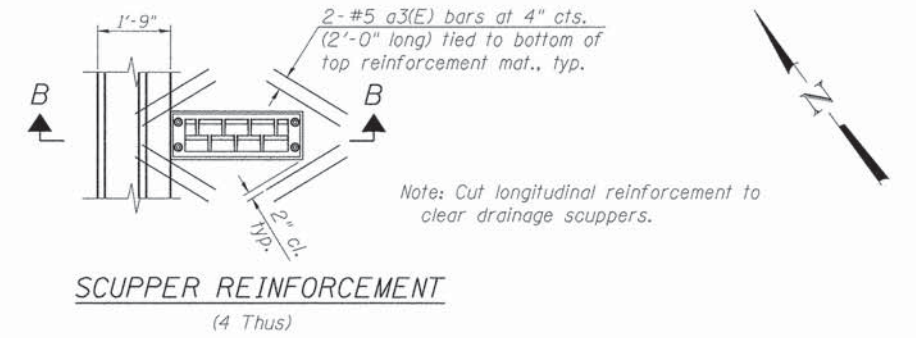


187-#6 a2(E) bars at 7" cts. Top (each side)
(Lap with each a(E) bar)

PLAN



CROSS SECTION
(Looking Upstation)



SCUPPER REINFORCEMENT
(4 Thus)

- NOTES:**
1. For Superstructure Details, Bill of Material, Section B-B, and parapet reinforcement, see sheet S8
 2. For Section A-A and diaphragm details, see sheet S10.
 3. Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.

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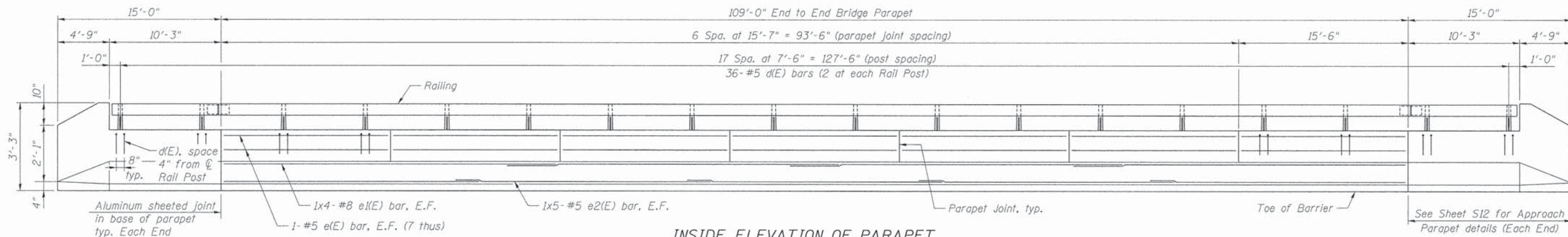
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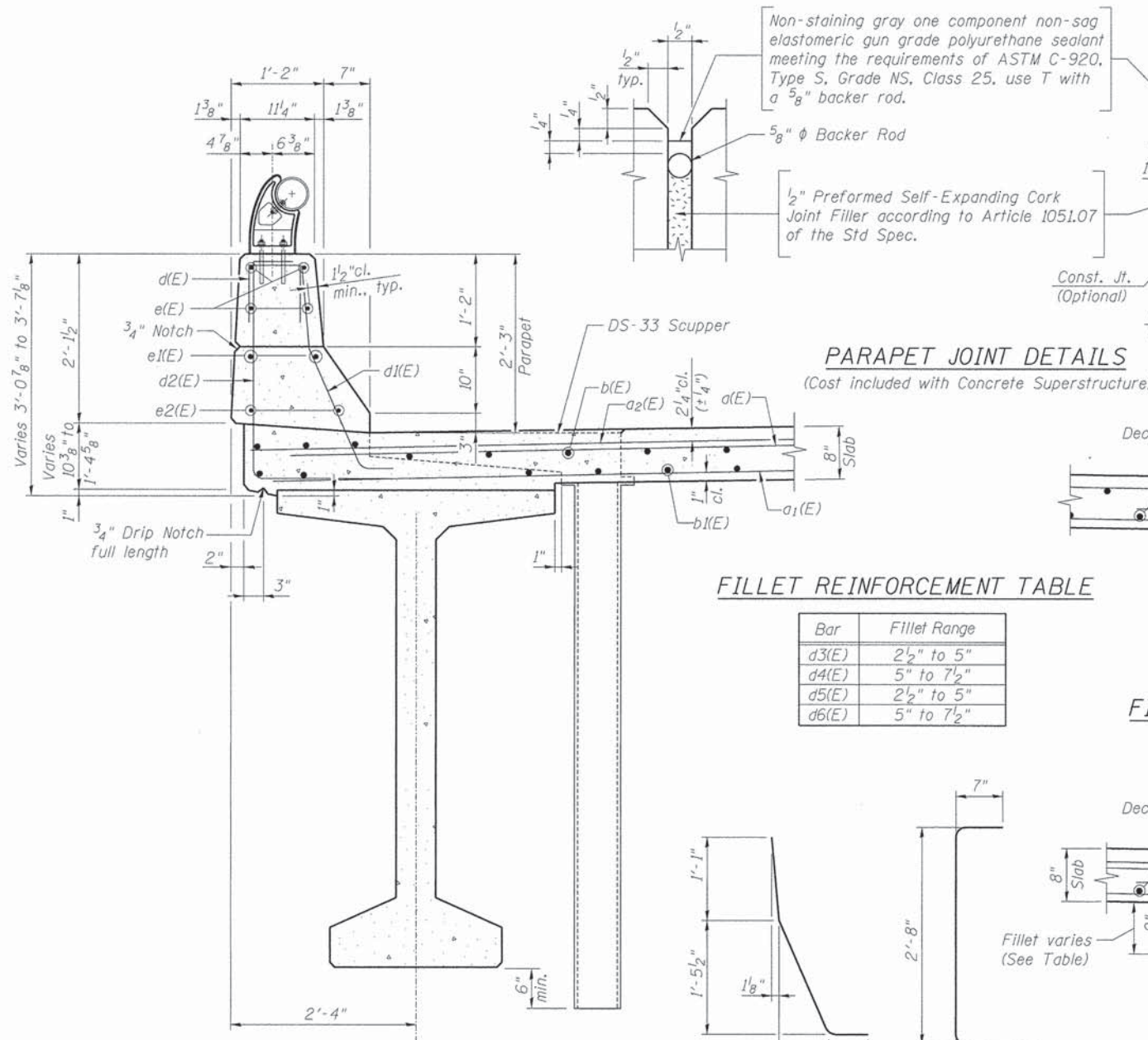
SUPERSTRUCTURE
STRUCTURE NO. 099-3286
SHEET NO. S7 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	24
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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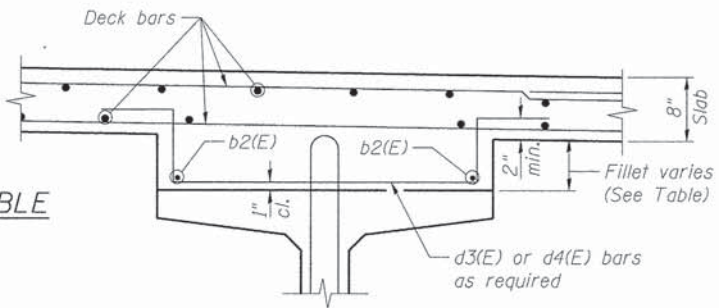
INSIDE ELEVATION OF PARAPET



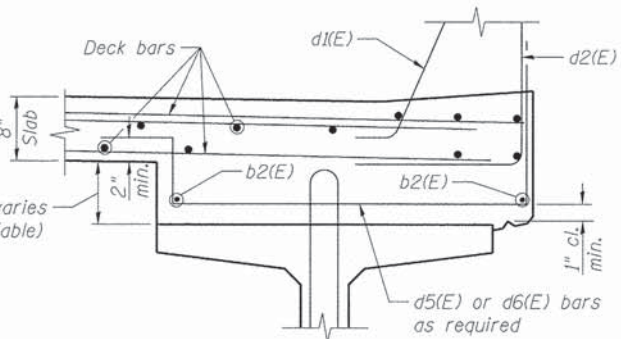
PARAPET JOINT DETAILS
(Cost included with Concrete Superstructure)

FILLET REINFORCEMENT TABLE

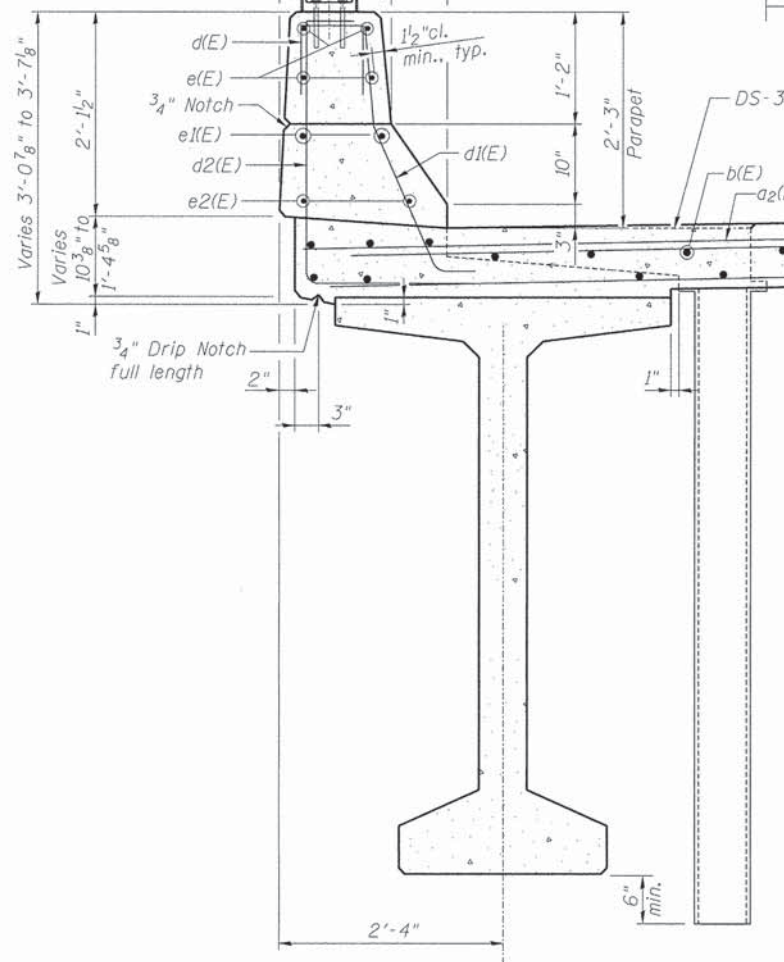
Bar	Fillet Range
d3(E)	2 1/2" to 5"
d4(E)	5" to 7 1/2"
d5(E)	2 1/2" to 5"
d6(E)	5" to 7 1/2"



FILLET REINFORCEMENT DETAIL
(Beams 2 thru 4)



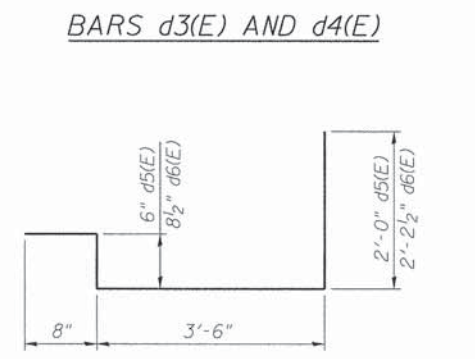
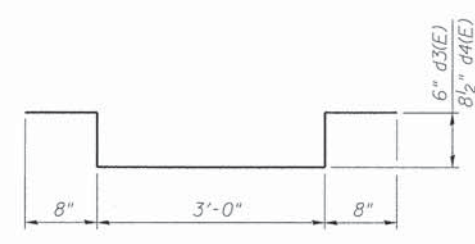
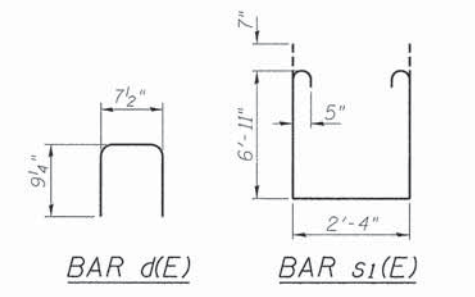
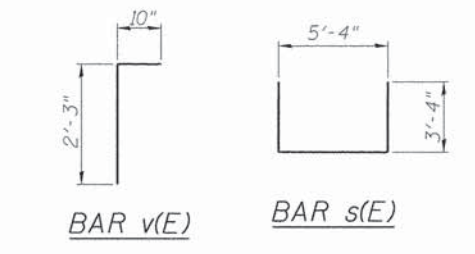
FILLET REINFORCEMENT DETAIL
(Beams 1 & 5)



SECTION B-B

BAR d1(E)

BAR d2(E)



SUPERSTRUCTURE
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	187	#5	32'-10"	—
a1(E)	146	#5	32'-10"	—
a2(E)	374	#6	6'-6"	—
a3(E)	32	#5	2'-0"	—
b(E)	144	#5	29'-8"	—
b1(E)	140	#5	24'-4"	—
b2(E)	20	#4	29'-10"	—
d(E)	72	#5	2'-2"	U
d1(E)	238	#5	3'-3"	U
d2(E)	238	#5	5'-1"	U
d3(E)	102	#4	5'-4"	U
d4(E)	72	#4	5'-9"	U
d5(E)	68	#4	6'-8"	U
d6(E)	48	#4	7'-1"	U
e(E)	56	#5	15'-3"	—
e1(E)	16	#8	31'-1"	—
e2(E)	20	#5	23'-9"	—
m(E)	14	#6	33'-2"	—
m1(E)	40	#6	6'-4"	—
m2(E)	20	#6	1'-9"	—
m3(E)	8	#6	4'-8"	—
m4(E)	4	#6	0'-11"	—
m5(E)	30	#5	4'-0"	—
s(E)	56	#5	12'-0"	U
s1(E)	44	#5	17'-4"	U
v(E)	68	#5	3'-1"	L
Reinforcement Bars, Epoxy Coated		Lbs.	32,680	
Concrete Superstructure		Cu. Yds.	187.6	

Bars indicated thus 1 x 2-#8 etc. indicates 1 line of bars with 2 lengths per line.

MINIMUM BAR LAP

(Deck)	(Parapet)
#4 bar = 2'-7"	#4 bar = 2'-0"
#5 bar = 3'-3"	#5 bar = 2'-6"
#6 bar = 3'-10"	#8 bar = 5'-2"

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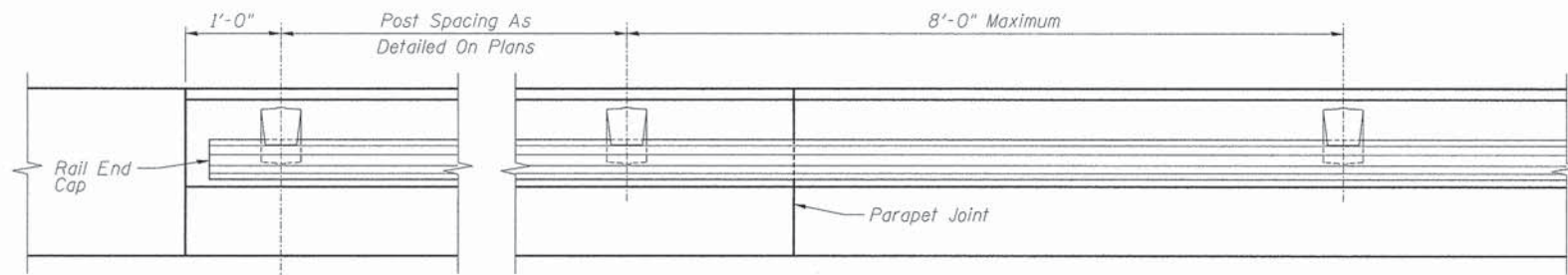
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS
STRUCTURE NO. 099-3286

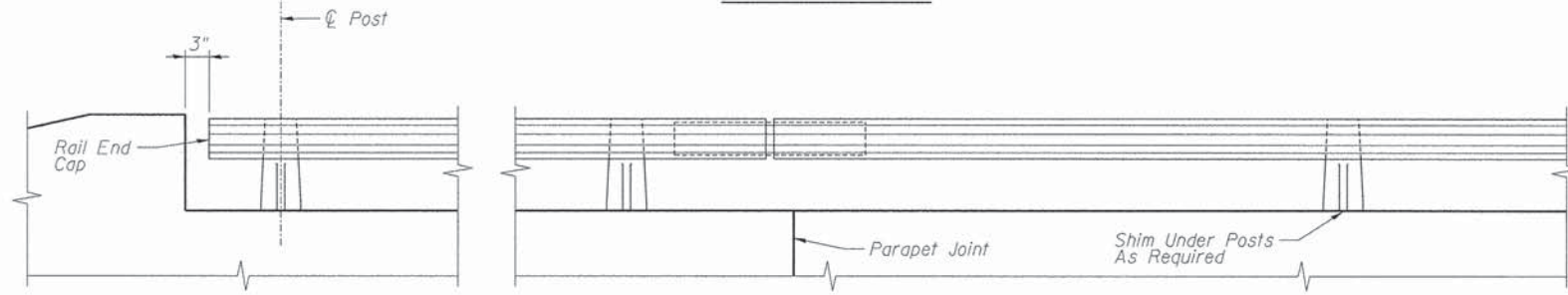
SHEET NO. 58 OF 526 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	25
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

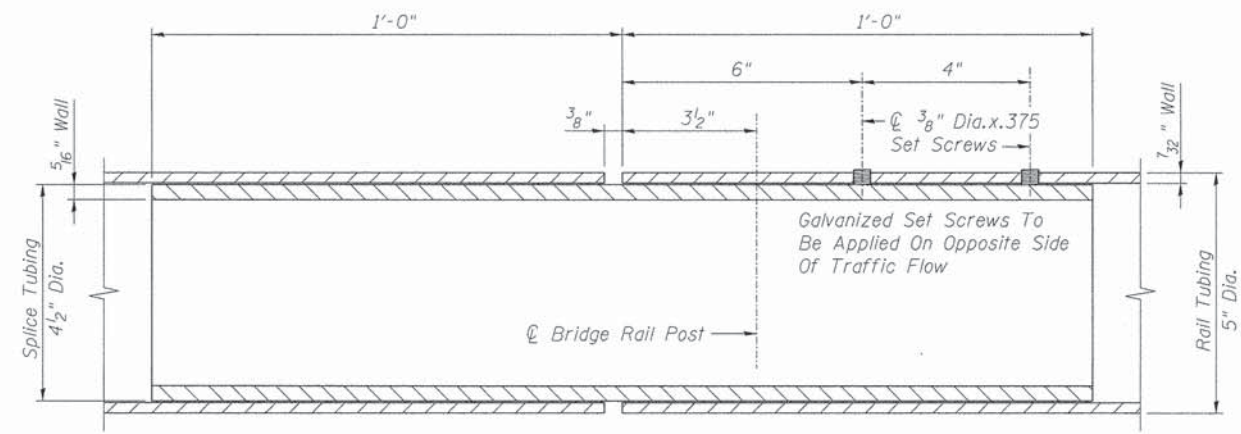
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PARTIAL PLAN



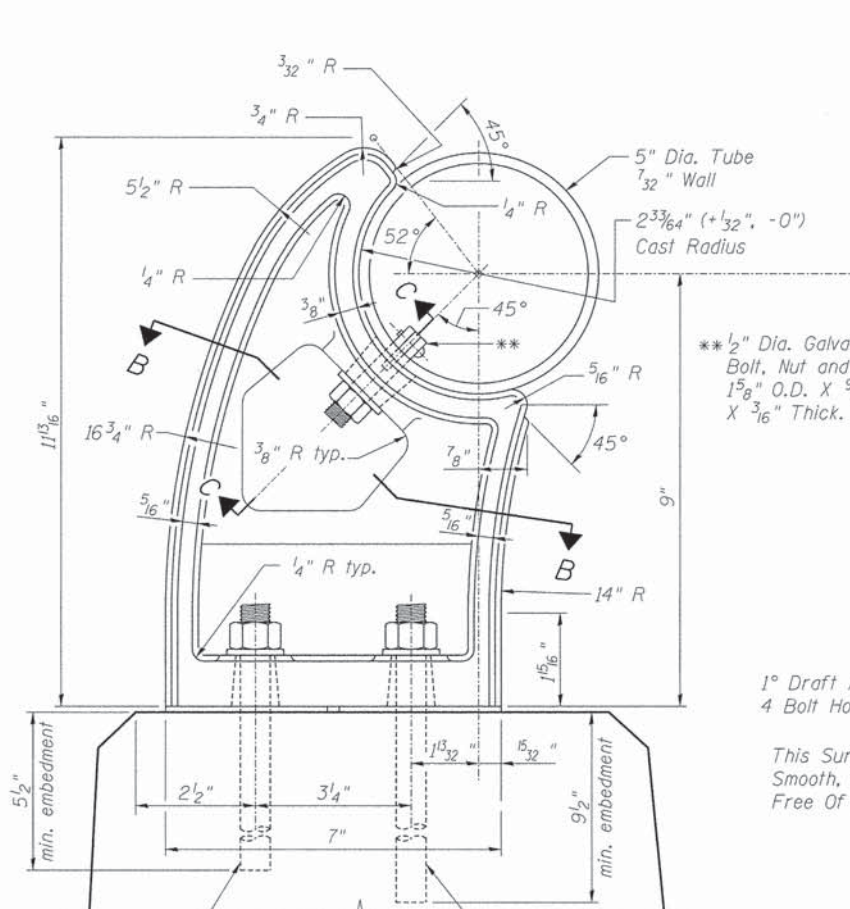
PARTIAL ELEVATION



INSIDE SPLICE DETAIL

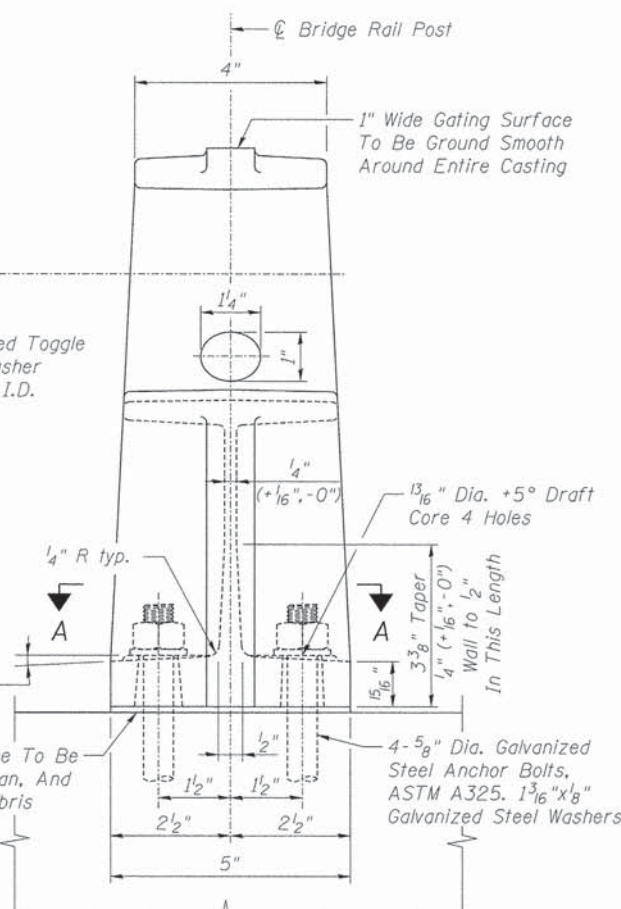
NOTES:

1. Railing to conform to vertical and horizontal alignment.
2. Joints to be placed 25'-0" center to center, max.
3. Slip joint to be placed in panels to match end of deck.
4. Design weight: 6 1/4 lbs. per foot.
5. Unless otherwise specified all draft to be 3°.
6. All unmarked Radii to be 1/8" R.
7. After fabrication all exposed surfaces of aluminum shall be given an anodic oxide coating, dyed black, conforming to the requirements of ASTM designation: B 580, Type B, Architectural Class I. See Special Provisions.
8. Three aluminum shims per post, one at 1/8" and two at 1/16" shall be provided for 25 percent of the posts.
9. At the Contractor option, either cast in place anchor devices or drilled and set anchor rods may be used to attach the posts to the concrete. Drilling and setting of anchor rods shall be according to Article 509.06. Cost of anchor devices and rods shall be included in the cost of Railing.



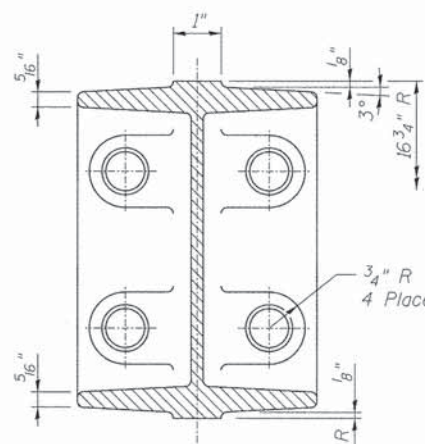
2'-0"-8" x 5/8" Dia. Galvanized Steel Anchor Rods, ASTM F1554 Grade 105. 1 3/16" x 1/8" Galvanized Steel Washers.

2'-1'-0" x 5/8" Dia. Galvanized Steel Anchor Rods, ASTM F1554 Grade 105. 1 3/16" x 1/8" Galvanized Steel Washers.

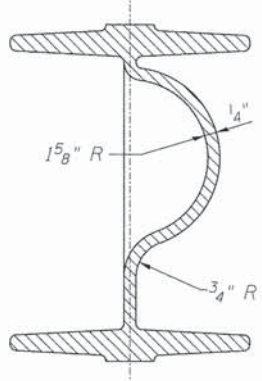


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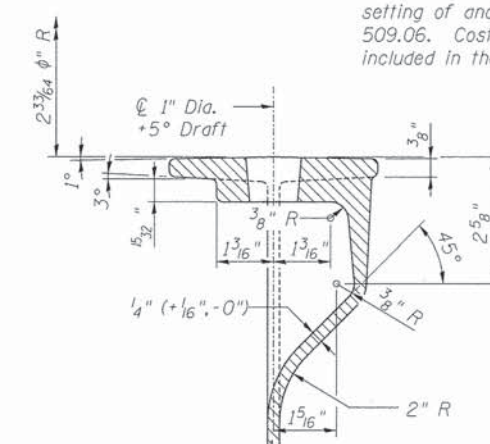
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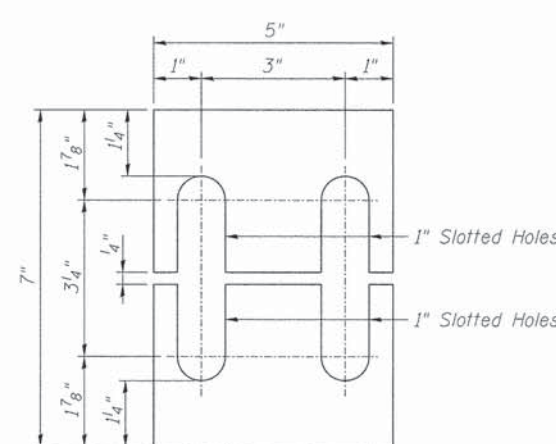
SECTION A-A



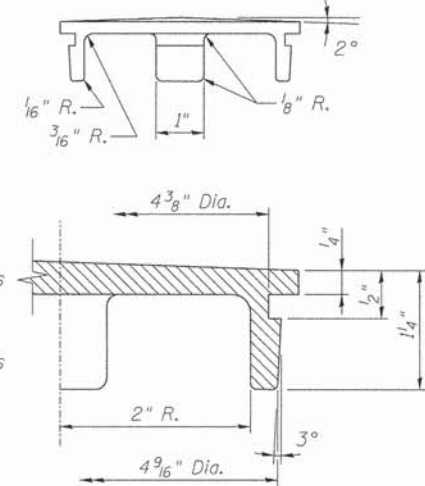
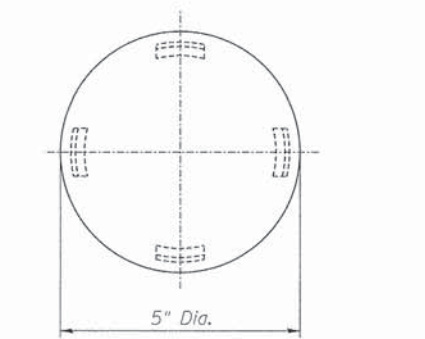
SECTION B-B



SECTION C-C



SHIM DETAIL



RAIL END CAP DETAILS

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RAILING DETAILS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ALUMINUM RAIL DETAILS
STRUCTURE NO. 099-3286
SHEET NO. S9 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	26
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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USER NAME = rgrimm

PLOT SCALE =

PLOT DATE = 12/5/2013

DESIGNED - AWH

CHECKED - MFH

DRAWN - RMG

CHECKED - MFH/SLD

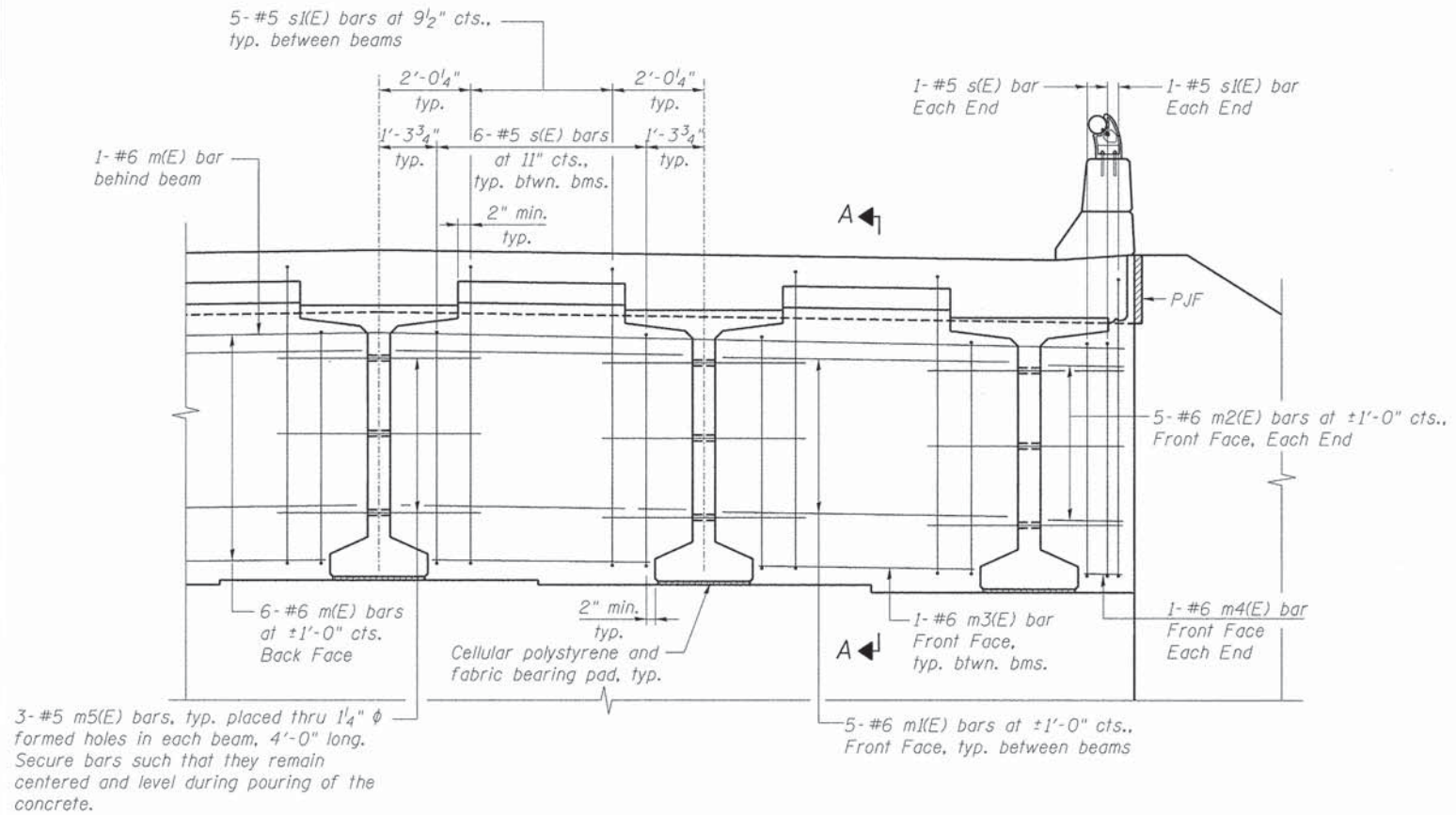
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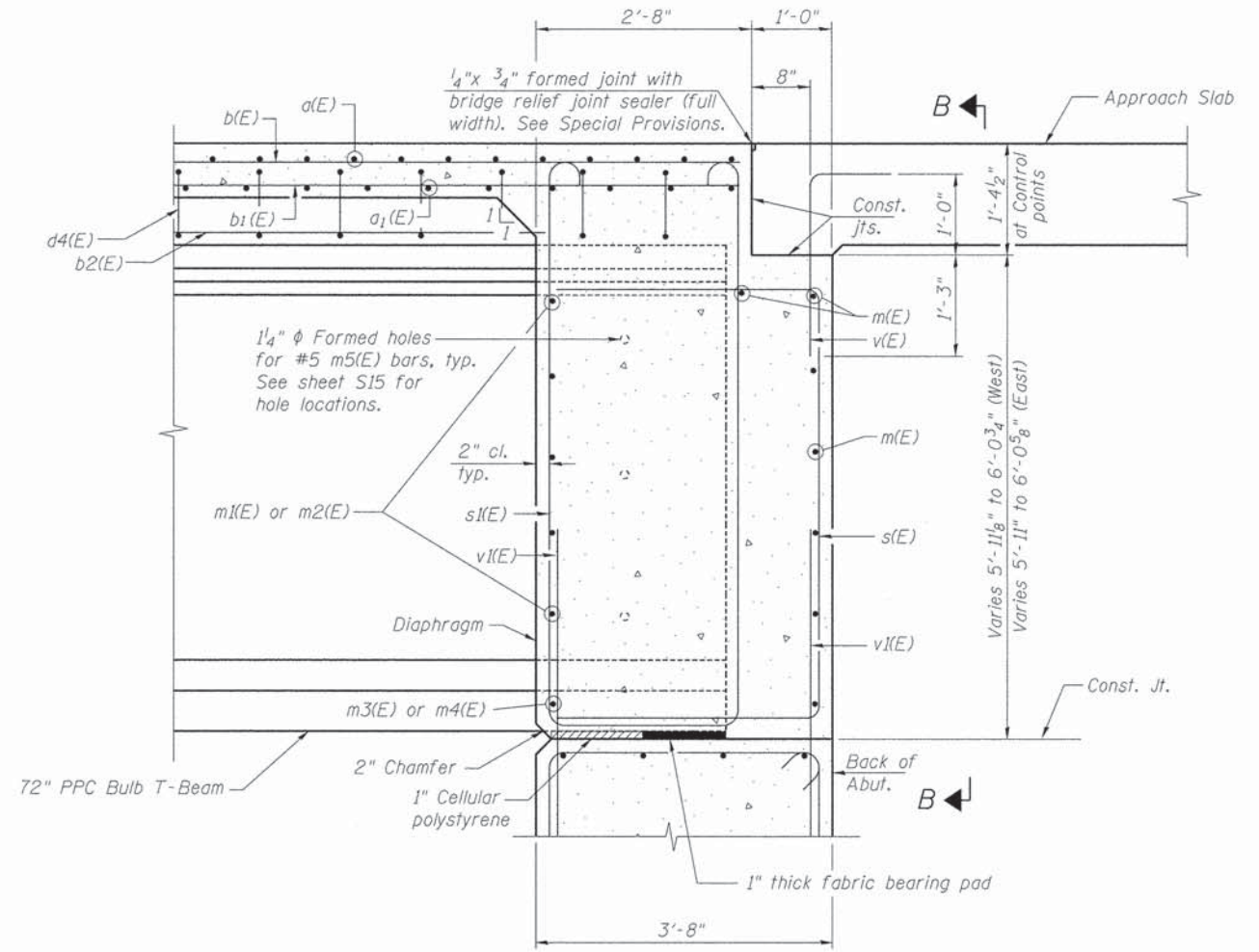
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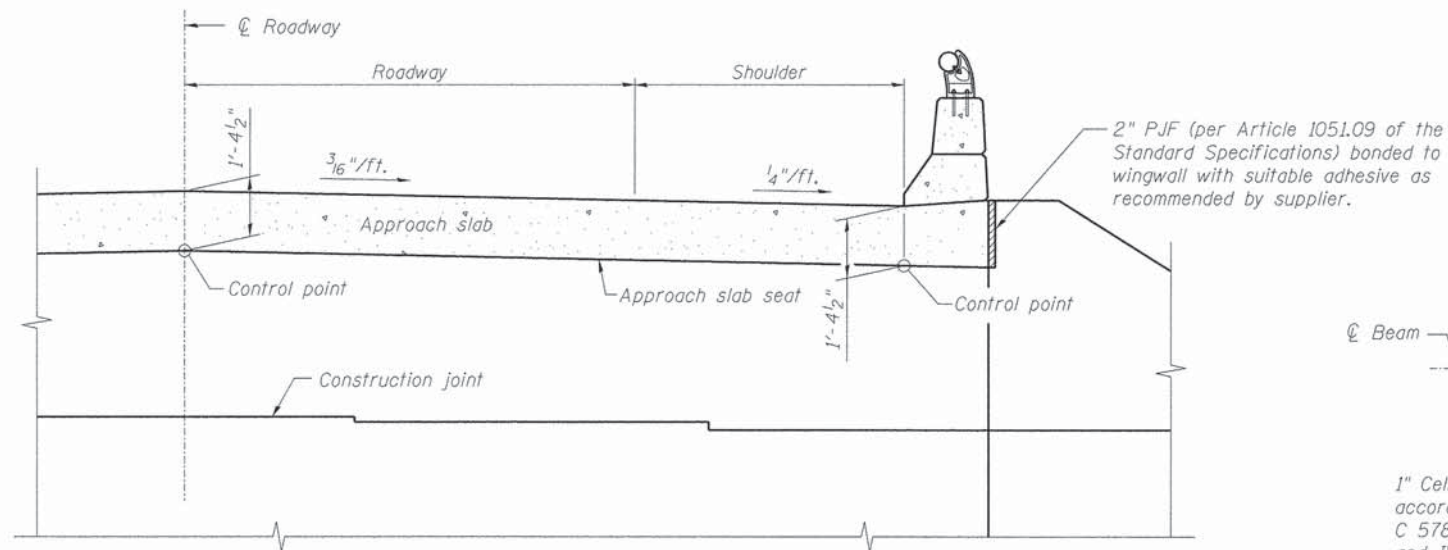
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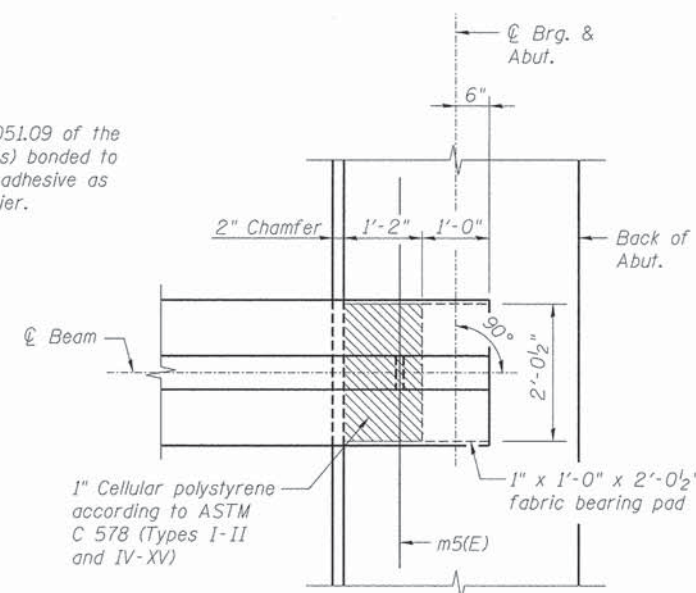
DIAPHRAGM ELEVATION AT ABUTMENT



SECTION A-A



SECTION B-B



PARTIAL PLAN AT ABUTMENT
(Showing bottom flange of beam)

NOTES:

1. Reinforcement bars in diaphragm are billed with superstructure on sheet S8.
2. Concrete in diaphragm is included with Concrete Superstructure on sheet S8.
3. For details of bars s(E), s1(E) and v(E) see sheet S8.
4. The approach slab seat shall have a constant slope determined from the control points shown.
5. Cost of cellular polystyrene is included with Concrete Superstructure.
6. Cost of fabric bearing pad is included with PPC Bulb T-Beams.

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DPBTI-0

8-31-12

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STATE OF ILLINOIS
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DIAPHRAGM DETAILS
STRUCTURE NO. 099-3286

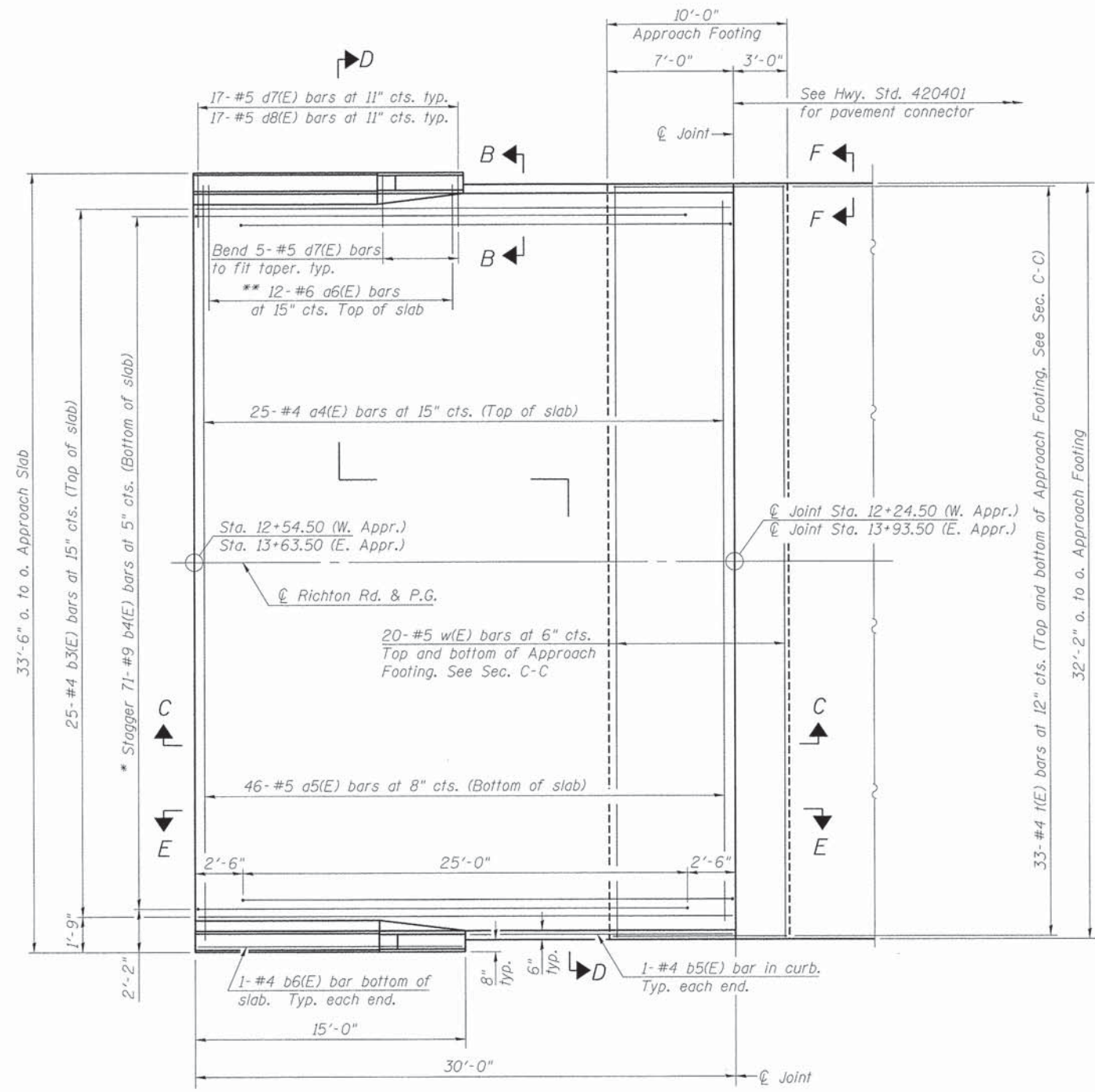
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ILLINOIS FED. AID PROJECT			CONTRACT NO. 61A02	

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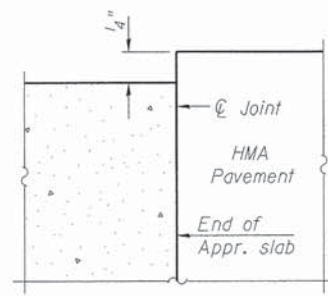
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12/5/2013

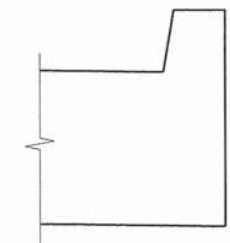


PLAN
(East approach slab shown, west slab opposite hand)

* Tilt #9 b4(E) bars as required to maintain clearance.
** Space between a4(E) bars, typ. ea. parapet.

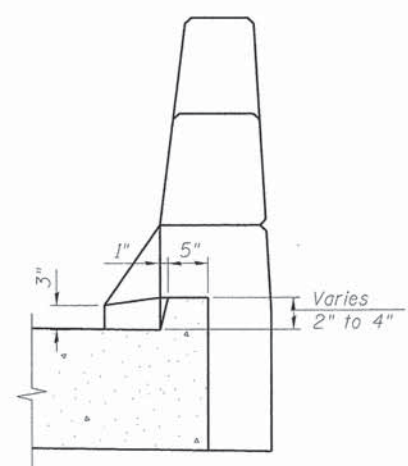


FLEXIBLE PAVEMENT



VIEW F-F

DETAIL A



VIEW B-B

NOTE:
See sheet S12 for Sections C-C & D-D and View E-E.

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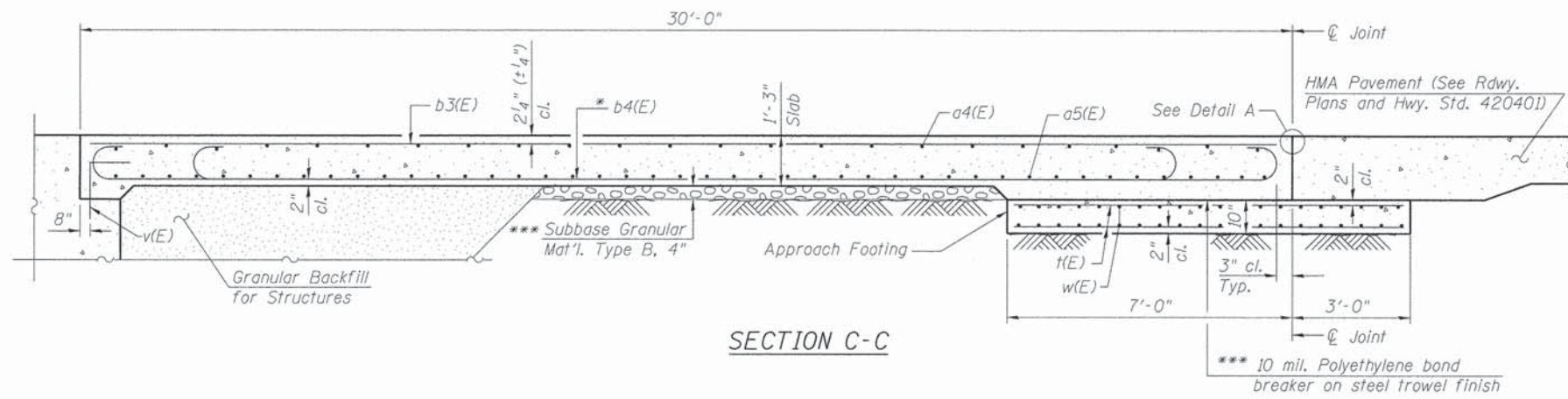
STATE OF ILLINOIS
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BRIDGE APPROACH SLAB DETAILS 1 OF 2
STRUCTURE NO. 099-3286

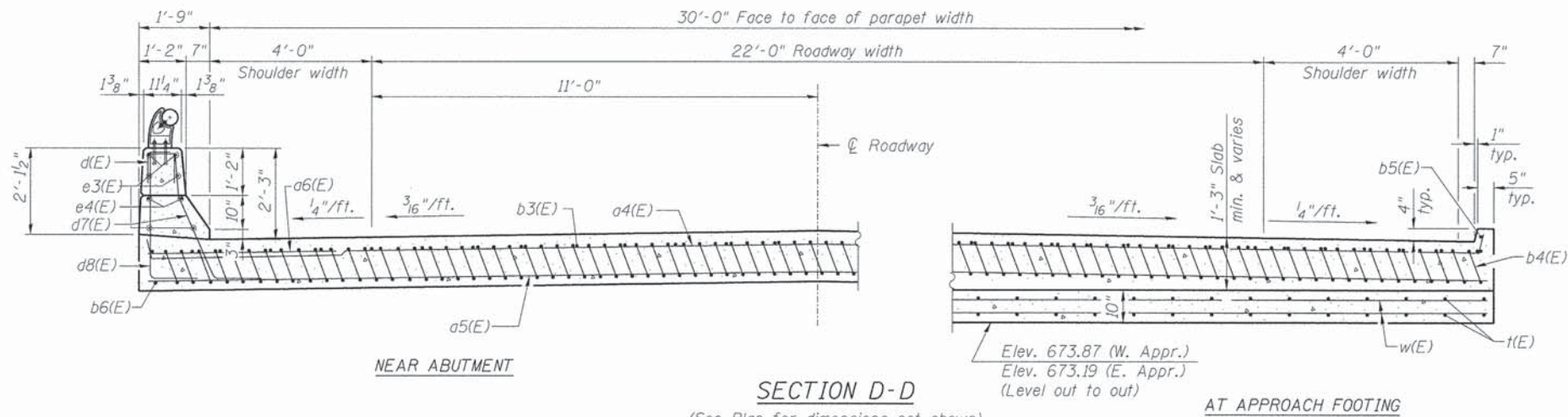
SHEET NO. S11 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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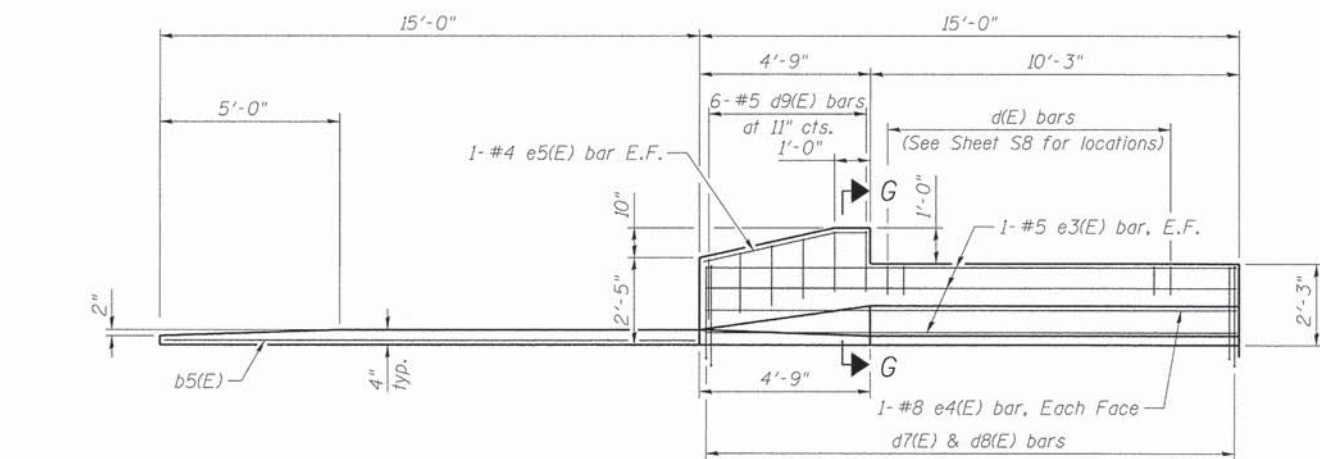


SECTION C-C

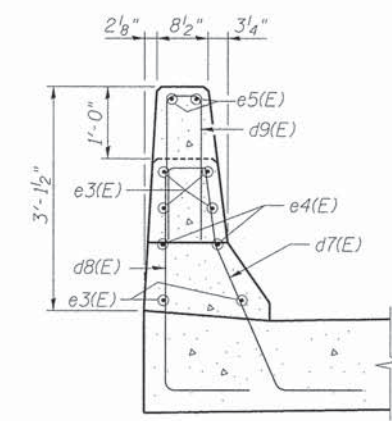


SECTION D-D

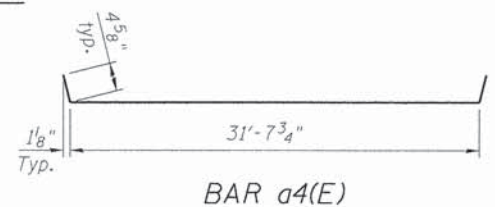
(See Plan for dimensions not shown)



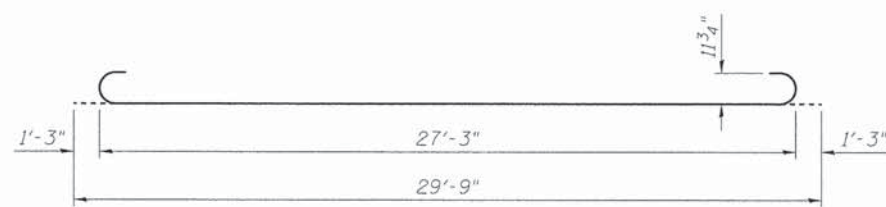
VIEW E-E



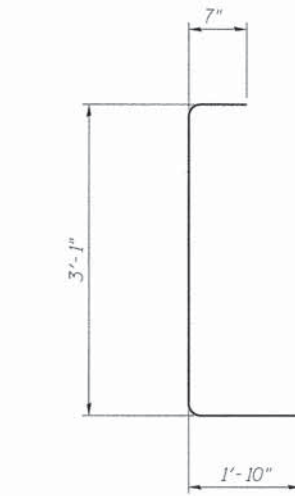
SECTION G-G



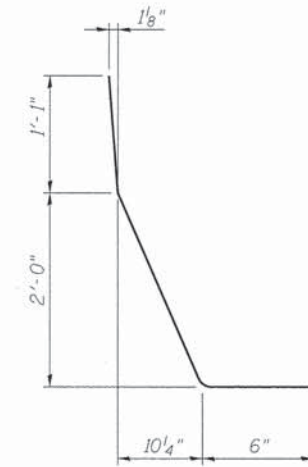
BAR a4(E)



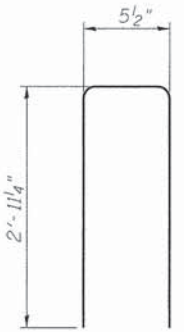
BAR b4(E)



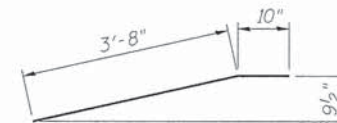
BAR d8(E)



BAR d7(E)



BAR d9(E)



BAR e5(E)

* Tilt #9 b4(E) bars as required to maintain clearance.

*** Cost included with Concrete Superstructure.

TWO APPROACHES
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a4(E)	50	#4	32'-5"	U
a5(E)	92	#5	31'-10"	—
a6(E)	48	#6	6'-6"	—
b3(E)	50	#4	29'-8"	—
b4(E)	142	#9	29'-9"	U
b5(E)	4	#4	14'-8"	—
b6(E)	4	#4	14'-8"	—
d7(E)	68	#5	3'-10"	—
d8(E)	68	#5	5'-6"	U
d9(E)	24	#5	6'-4"	U
e3(E)	24	#5	14'-8"	—
e4(E)	8	#8	14'-8"	—
e5(E)	8	#4	4'-6"	—
t(E)	132	#4	9'-8"	—
w(E)	80	#5	31'-10"	—
Concrete Superstructure		Cu. Yd.	103.2	
Concrete Structures		Cu. Yd.	19.9	
Reinforcement Bars, Epoxy Coated		Pound	25,080	

Note: d(E) bars in parapet included in Bill of Materials on sheet S8.

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	PLOT DATE = 12/5/2013	DRAWN - RMG	REVISED -
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DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB DETAILS 2 OF 2
STRUCTURE NO. 099-3286

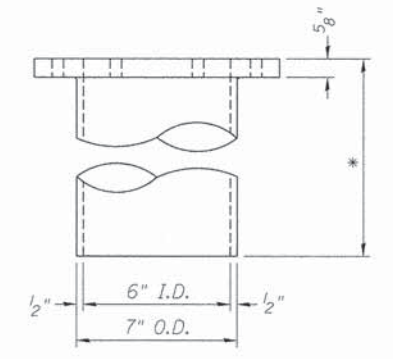
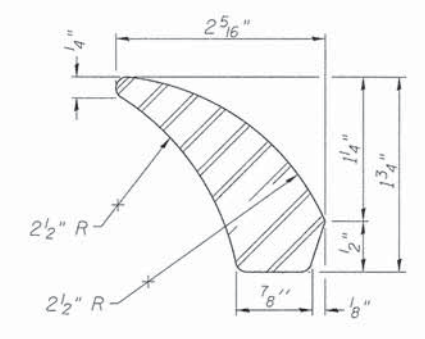
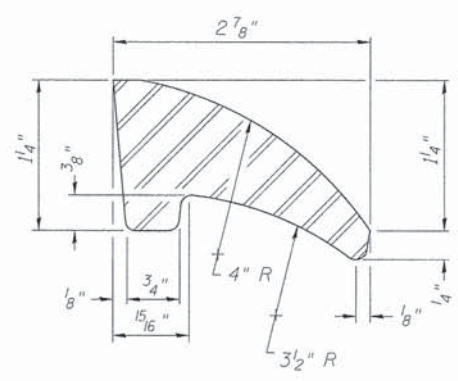
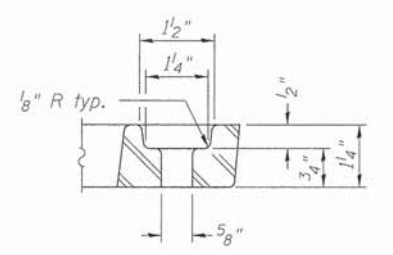
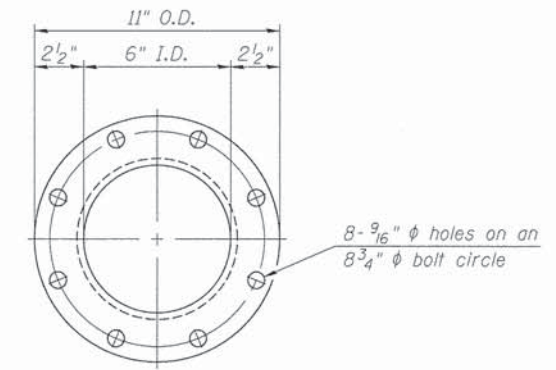
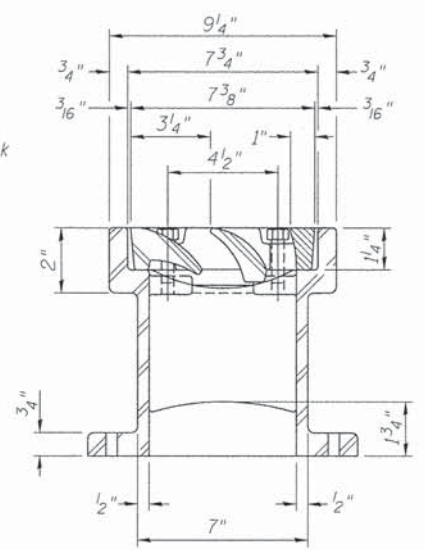
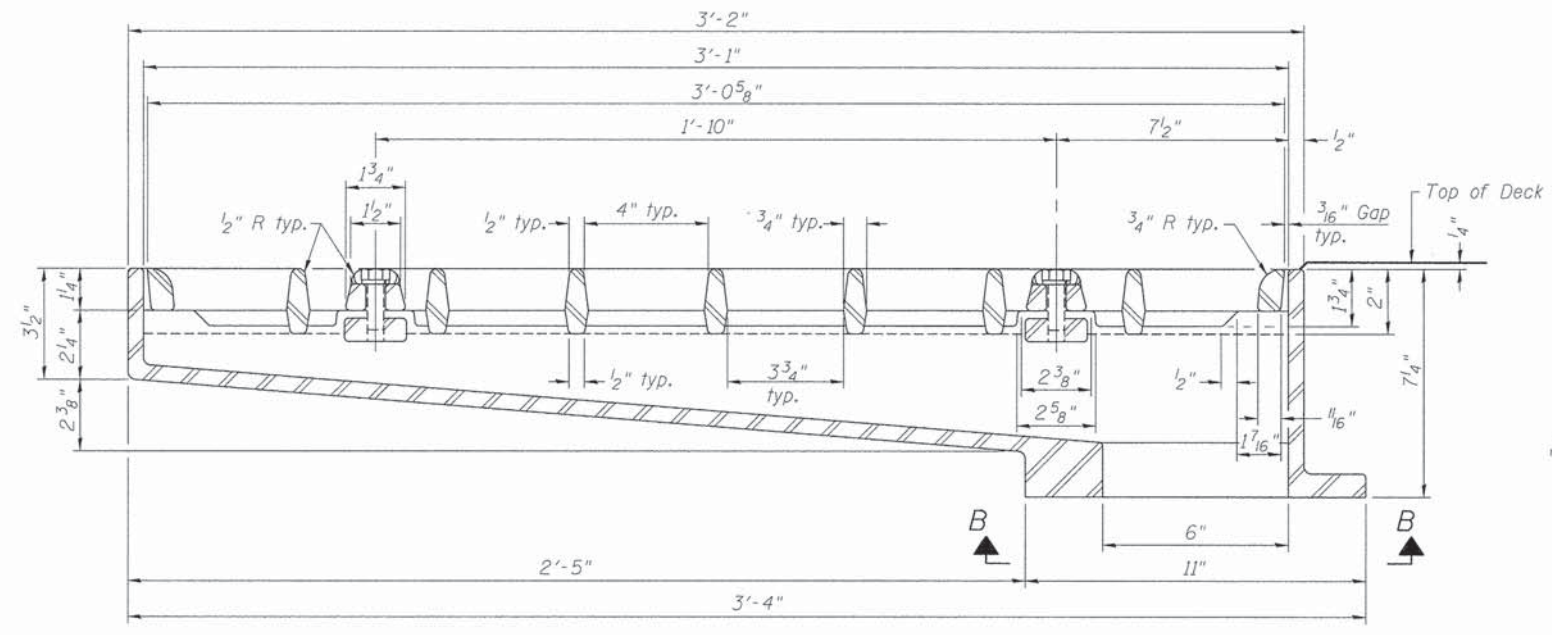
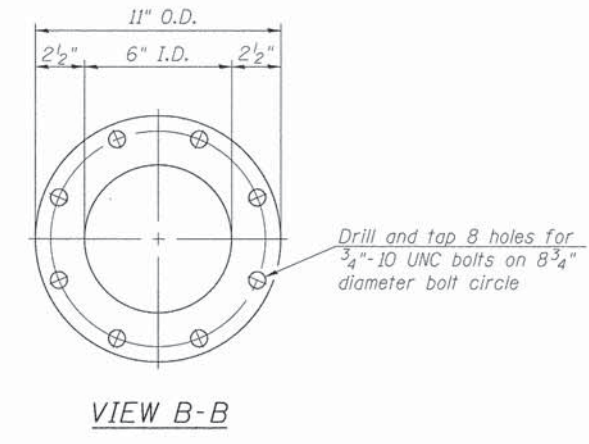
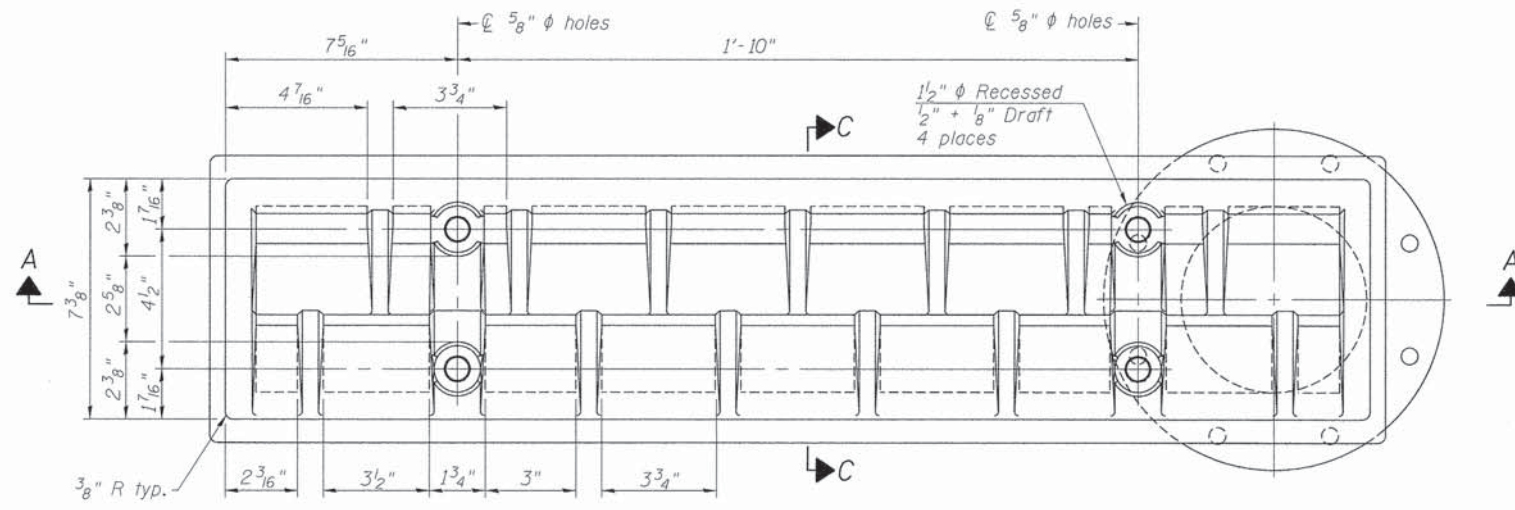
SHEET NO. S12 OF S26 SHEETS

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CONTRACT NO. 61A02				
ILLINOIS FED. AID PROJECT				

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* 72" beam plus fillet plus 6" (min). See sheet S8.

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper, DS-33	Each	4

Notes:

All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.

Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.

As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.

The exterior surfaces of the floor drains shall be coated or pigmented by the manufacturer with a color that matches the concrete.

Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M11.

The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.

Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-33.

Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.

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DS-33 7-1-10

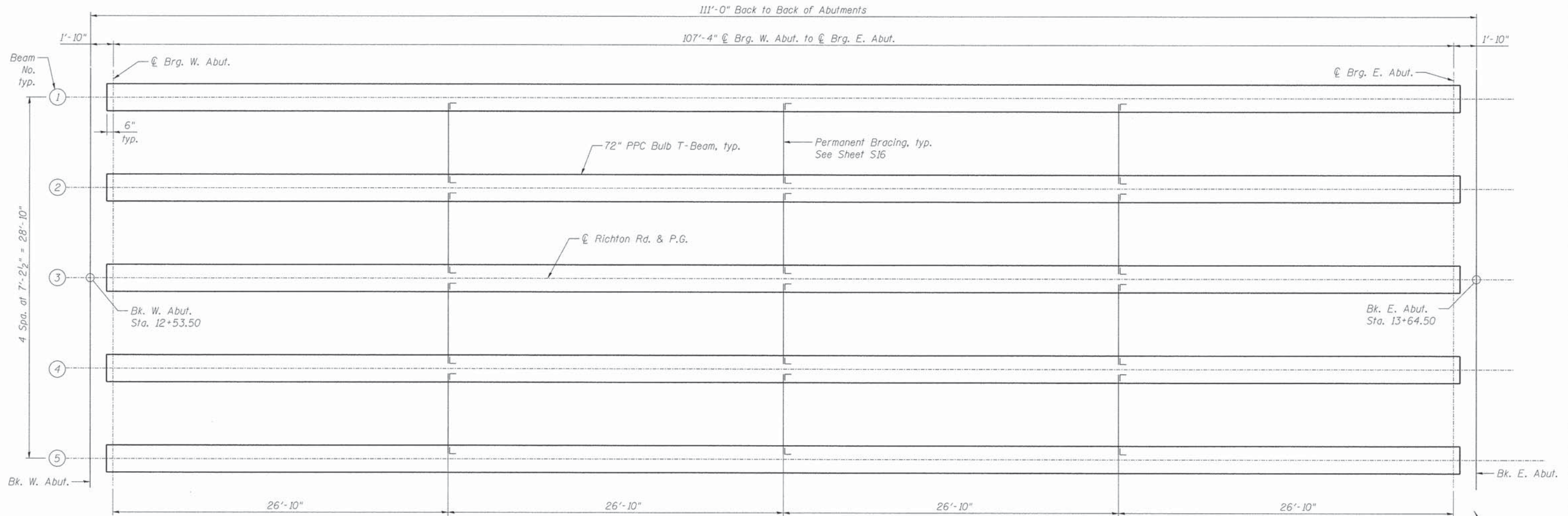
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STATE OF ILLINOIS
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DRAINAGE SCUPPER, DS-33
STRUCTURE NO. 099-3286
SHEET NO. S13 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	30
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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FRAMING PLAN

INTERIOR BEAM MOMENT TABLE	
	0.5 Span
I	(in ⁴) 545,894
I'	(in ⁴) 1,034,456
S_b	(in ³) 14,915
S_b'	(in ³) 19,640
S_t	(in ³) 15,421
S_t'	(in ³) 53,516
$DC1$	(k/ft) 1.699
M_{DC1}	(k) 2,448
$DC2$	(k/ft) 0.180
M_{DC2}	(k) 259
DW	(k/ft) 0.300
M_{DW}	(k) 432
$M_L + IM$	(k) 2,005

INTERIOR BEAM REACTION TABLE	
	Abutment
R_{DC1}	(k) 91.2
R_{DC2}	(k) 9.7
R_{DW}	(k) 16.1
$R_L + IM$	(k) 92.3
R_{Total}	(k) 209.3

I : Non-composite moment of inertia of beam section (in.⁴).
 I' : Composite moment of inertia of beam section (in.⁴).
 S_b : Non-composite section modulus for the bottom fiber of the prestressed beam (in.³).
 S_b' : Composite section modulus for the bottom fiber of the prestressed beam (in.³).
 S_t : Non-composite section modulus for the top fiber of the prestressed beam (in.³).
 S_t' : Composite section modulus for the top fiber of the prestressed beam (in.³).
 $DC1$: Un-factored non-composite dead load (kips/ft.).
 M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).
 $DC2$: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 DW : Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 $M_L + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

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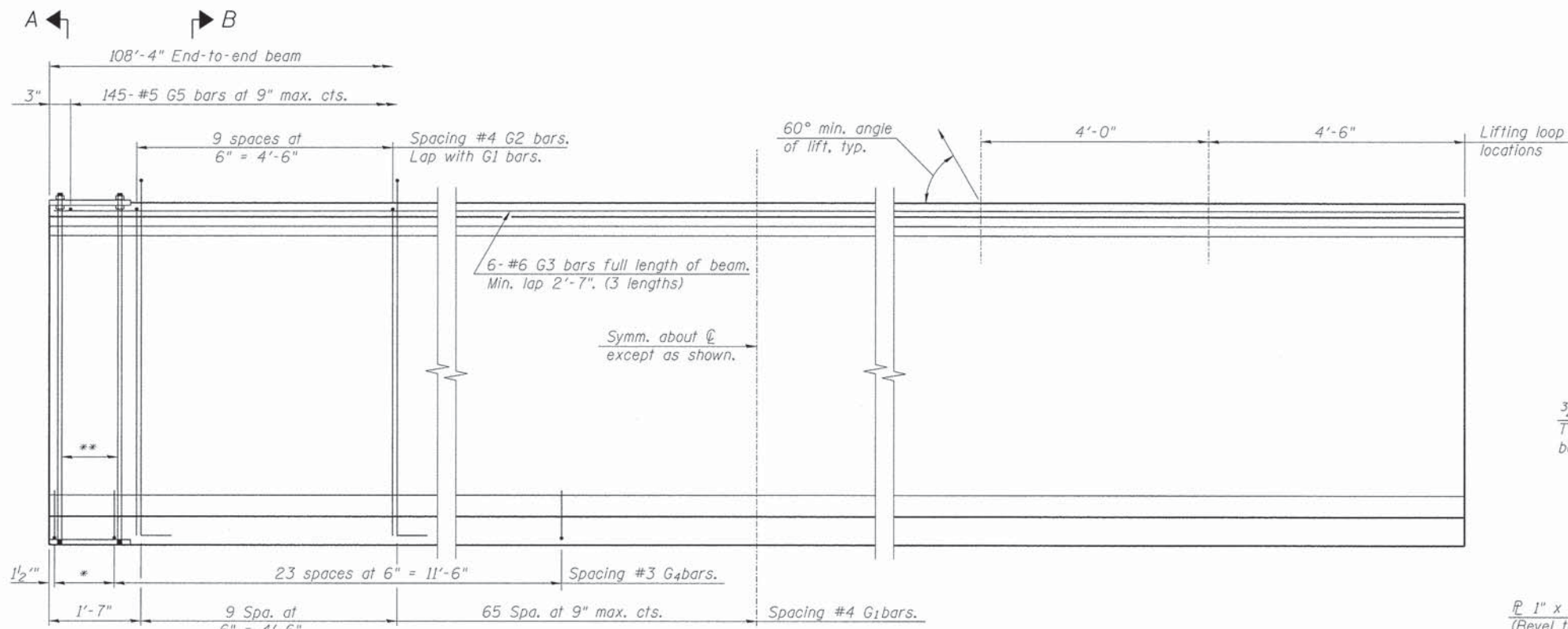
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FRAMING PLAN
STRUCTURE NO. 099-3286

SHEET NO. S14 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

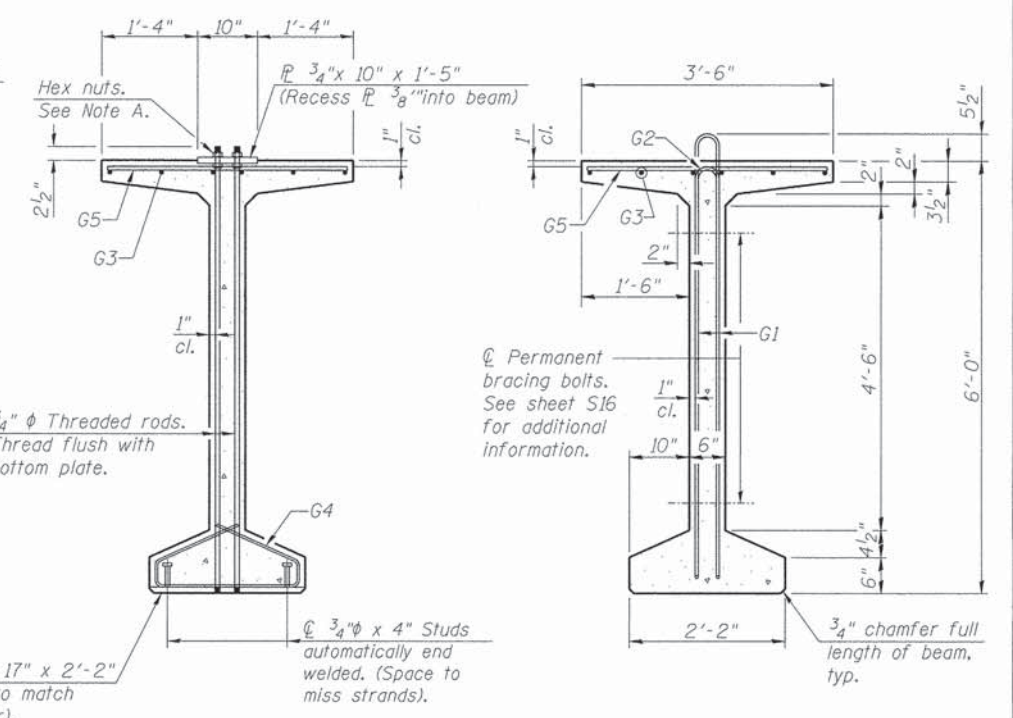


A ← → B

* 4 spaces at 3 1/4" = 1'-1".
 ** 5-3/4" φ threaded dowel rods at 3 1/4" cts., each face.

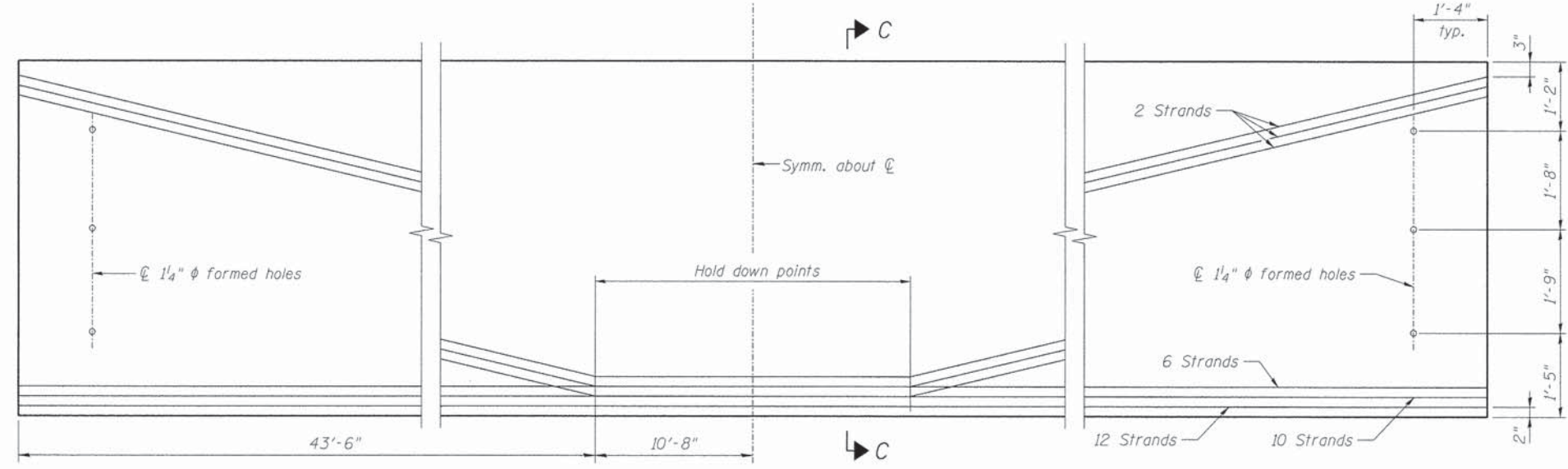
ELEVATION OF BEAM
 (Showing reinforcement & dimensions)

Note A:
 Hex nuts (top and bottom) with lock washers (top). Only tighten sufficiently to compress lock washers.

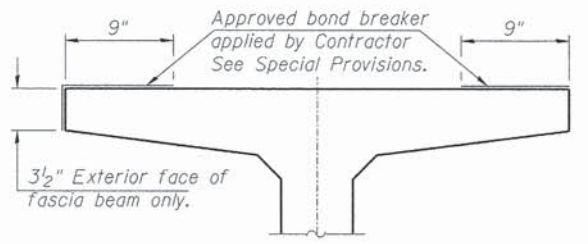


SECTION A-A

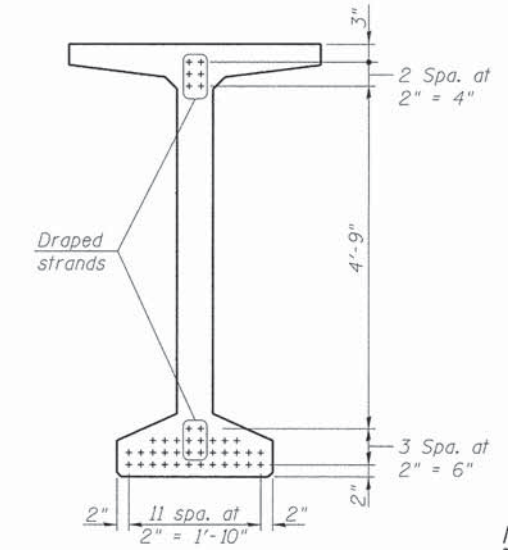
SECTION B-B



ELEVATION OF BEAM
 (Showing prestressing steel)



SECTION THRU TOP FLANGE
 (Showing limits of bond breaker)



SECTION C-C

*****BAR LIST
 ONE BEAM ONLY**

Bar	No.	Size	Length	Shape
G1	149	#4	13'-7"	⊏
G2	20	#4	11'-8"	⊏
G3	18	#6	37'-9"	⊏
G4	56	#3	4'-11"	⊏
G5	145	#5	3'-4"	⊏

***For information only

- NOTES:**
- See sheet S16 for additional details and Bill of Material.
 - Required release strength, f'_{ci} , shall be 5,000 psi.
 - Apply approved bond breaker as shown in Section thru Top Flange full length of beam. See Special Provisions.

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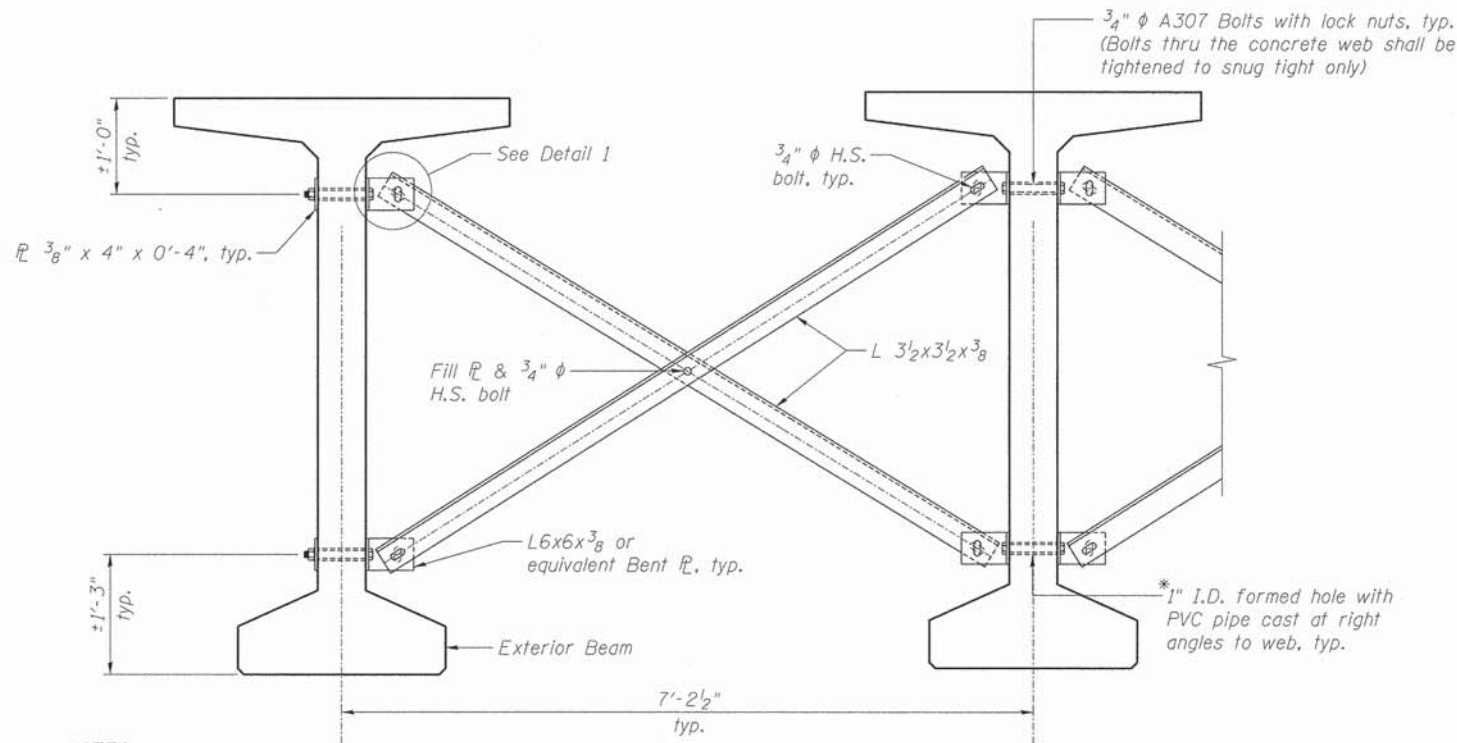
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	PLOT DATE = 12/5/2013	CHECKED - MRB/MFH	REvised -

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**72" PPC BULB T-BEAM
 STRUCTURE NO. 099-3286**
 SHEET NO. S15 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	32
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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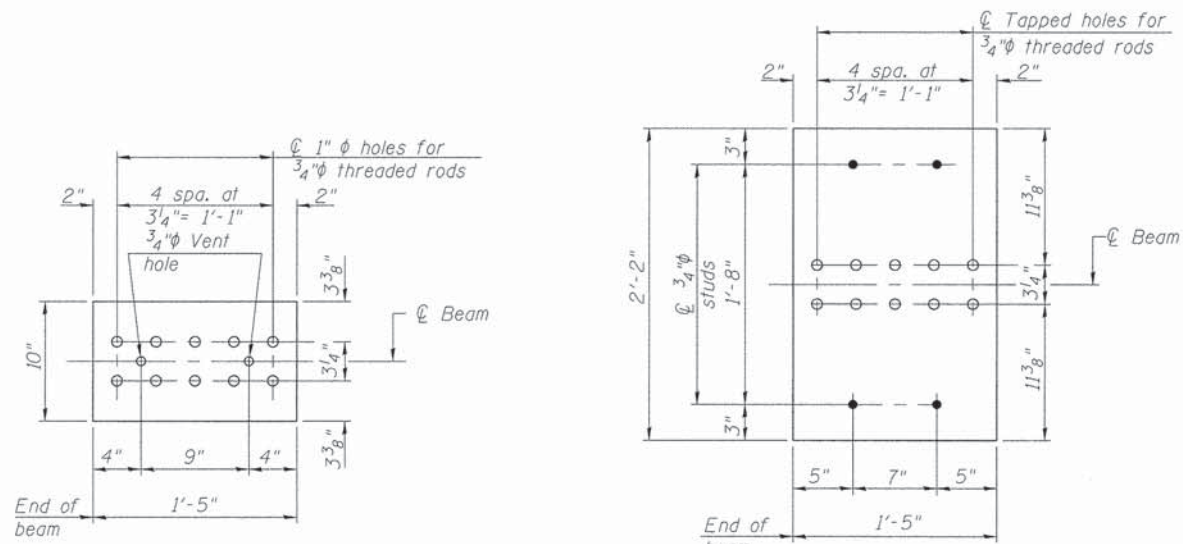


NOTES:

1. All material for bracing shall be hot dip galvanized according to AASHTO M111 unless otherwise noted.
2. Two hardened washers are required for each set of oversized holes.
3. All holes shall be 15/16" unless otherwise noted.
4. 5/16"x3"x0'-3" plate washers are required over all slotted holes.
5. All bolts shall be galvanized according to AASHTO M232.
6. Bracing shall be installed as beams are erected and tightened as soon as possible during erection.
7. Permanent bracing shall not be paid for separately, but shall be included in the cost of Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams, 72".

* Fabricator shall locate hole to miss strands within permissible tolerances.

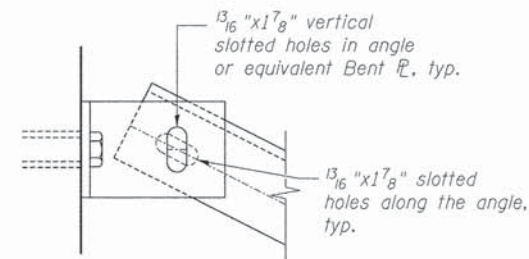
PERMANENT BRACING DETAILS FOR 72" BULB T-BEAMS



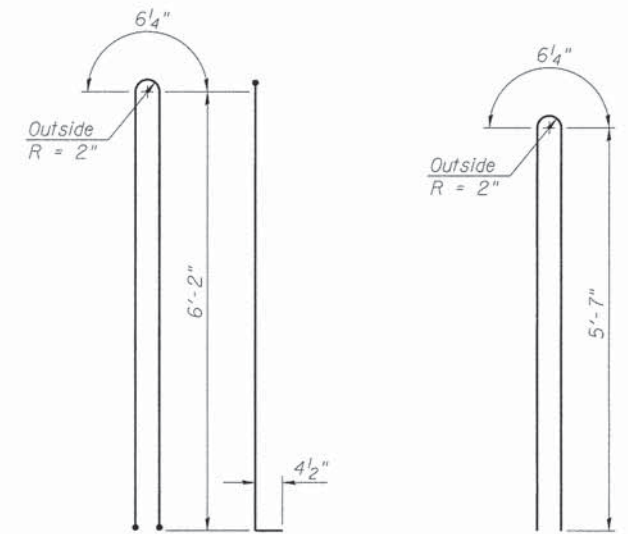
TOP PLATE

BOTTOM PLATE

See bearing details for pintle hole locations when required.

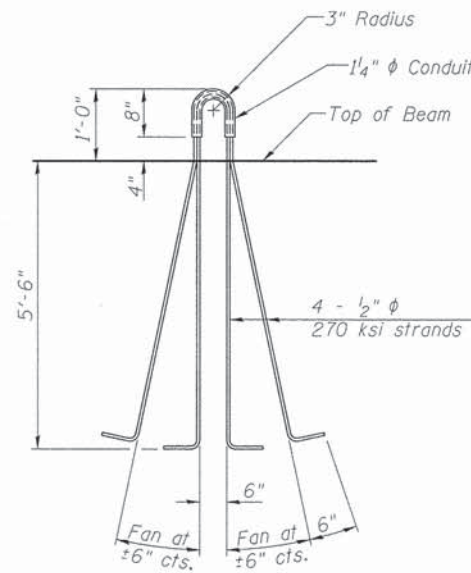


DETAIL 1

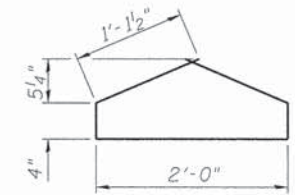


BAR G1

BAR G2



LIFTING LOOP DETAIL



BAR G4

BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams, 72"	Ft.	542

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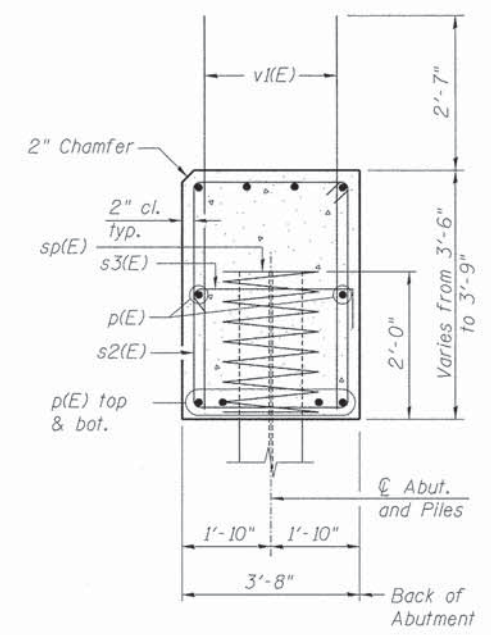
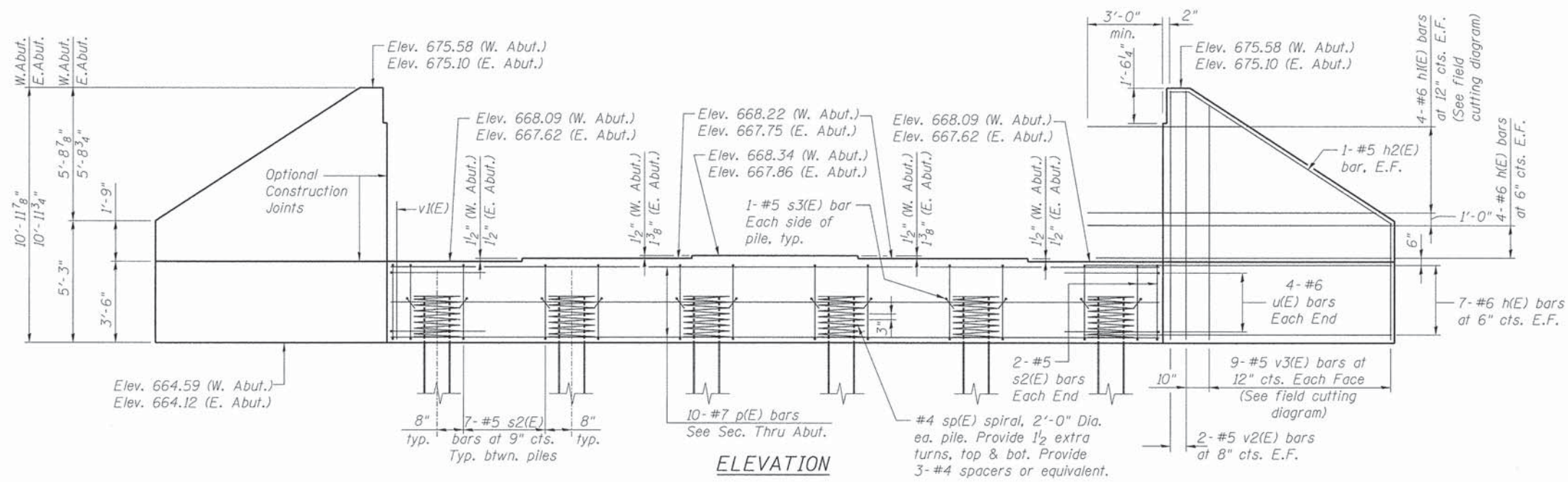
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	PLOT DATE = 12/5/2013	DRAWN - RMG	REVISED -
		CHECKED - MFH	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**72" PPC BULB T-BEAM DETAILS
STRUCTURE NO. 099-3286**

SHEET NO. S16 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	33
CONTRACT NO. 61A02				
ILLINOIS FED. AID PROJECT				



Elev. 664.59 (W. Abut.)
Elev. 664.12 (E. Abut.)

8\"/>

10-#7 p(E) bars
See Sec. Thru Abut.

2-#5 s2(E) bars
Each End

9-#5 v3(E) bars at
12\"/>

7-#6 h(E) bars
at 6\"/>

ELEVATION

SEC. THRU ABUT.

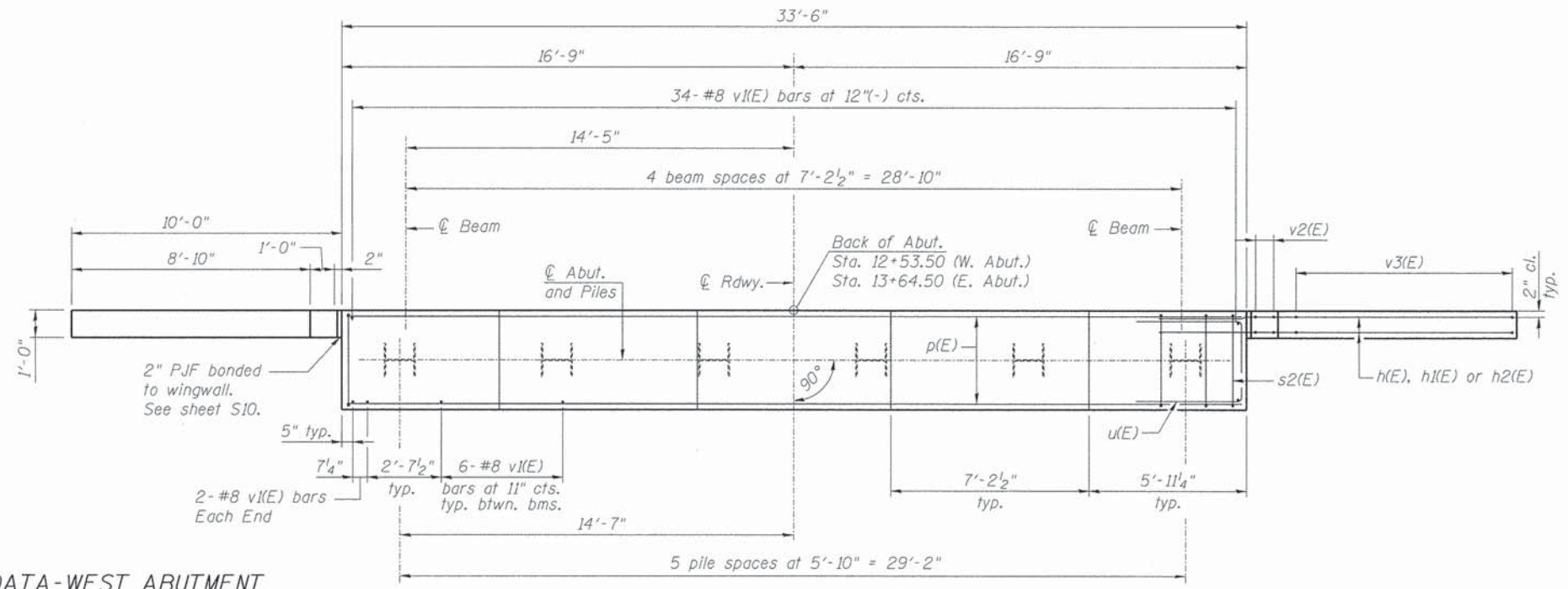
**BOTH ABUTMENTS
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
h(E)	88	#6	12'-10"	—
h1(E)	16	#6	17'-11"	—
h2(E)	8	#5	11'-2"	—
p(E)	20	#7	33'-2"	—
s2(E)	78	#5	13'-11"	□
s3(E)	24	#5	4'-4"	┌
sp(E)	12	#4	2'-0"	
u(E)	16	#6	10'-11"	□
v1(E)	124	#8	5'-11"	—
v2(E)	16	#5	10'-7"	—
v3(E)	36	#5	15'-1"	—
Structure Excavation			Cu. Yd.	49
Concrete Structures			Cu. Yd.	45.4
Reinforcement Bars, Epoxy Coated			Pound	8,310
Furnishing Steel Piles, HP 14x73			Foot	650
Driving Piles			Foot	650
Test Pile Steel HP 14x73			Each	2
Pile Shoes			Each	12

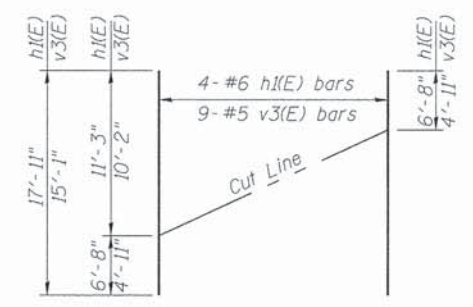
* Length is height of spiral.

NOTES:

- For details of piles, see sheet S18.
- Pour steps monolithically with cap.
- E.F. indicates Each Face.



PLAN



FIELD CUTTING DIAGRAM

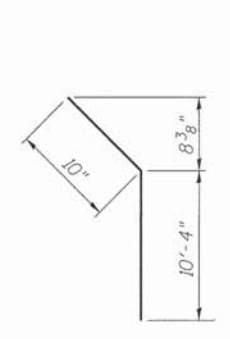
Order h1(E) and v3(E) full length. Cut as shown and use remainder of bars in opposite face.

PILE DATA-WEST ABUTMENT

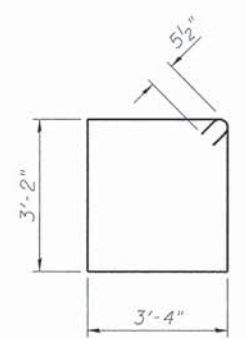
Type: HP14x73
Nominal Required Bearing: 505K
Factored Resistance Available: 278K
Est. Length: 63 ft.
No. Production Piles: 5
No. Test Piles: 1

PILE DATA-EAST ABUTMENT

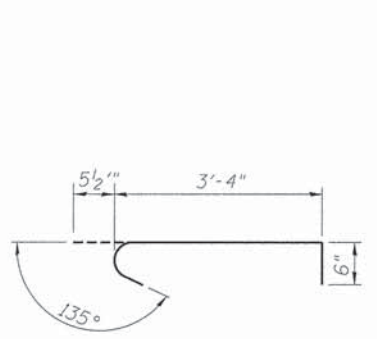
Type: HP14x73
Nominal Required Bearing: 505K
Factored Resistance Available: 278K
Est. Length: 67 ft.
No. Production Piles: 5
No. Test Piles: 1



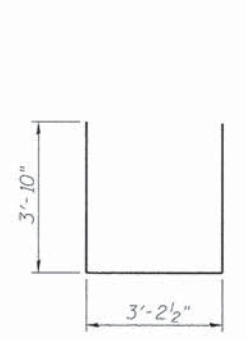
BAR h2(E)



BAR s2(E)



BAR s3(E)



BAR u(E)

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FILE NAME =	USER NAME = rgr1mm	DESIGNED - AWH	REVISED -
		CHECKED - MRB/MFH	REVISED -
		DRAWN - RMG	REVISED -
		CHECKED - MRB/MFH	REVISED -

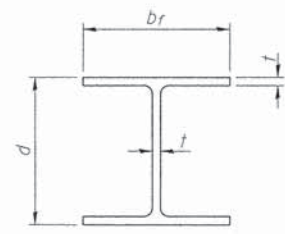
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**WEST AND EAST ABUTMENT DETAILS
STRUCTURE NO. 099-3286**

SHEET NO. S17 OF S26 SHEETS

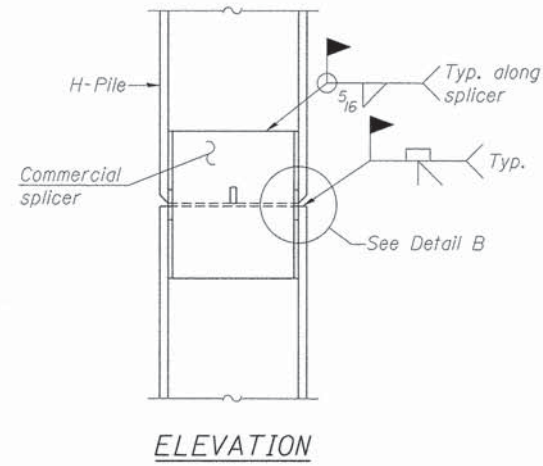
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	34
CONTRACT NO. 61A02				
[ILLINOIS] FED. AID PROJECT				

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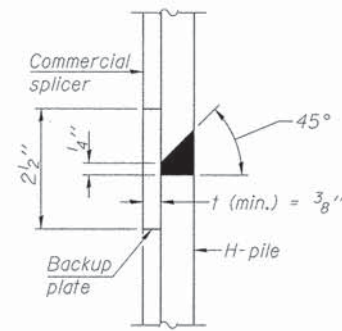


STEEL PILE TABLE

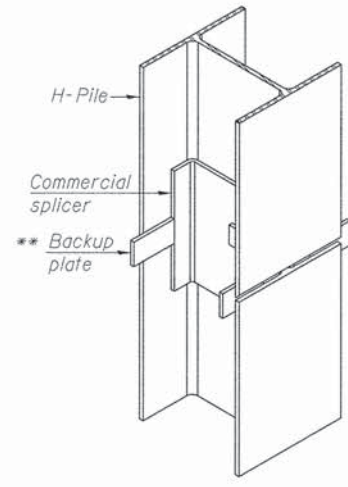
Designation	Depth d	Flange width br	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	13/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

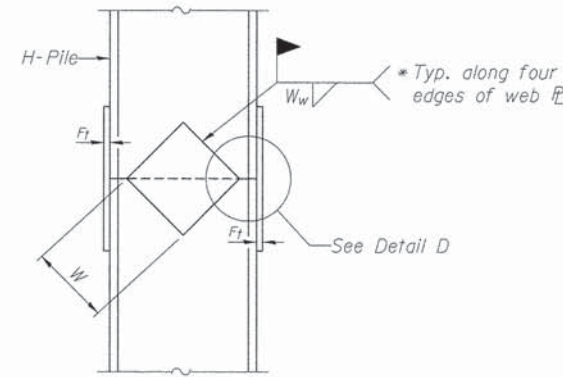


DETAIL "B"

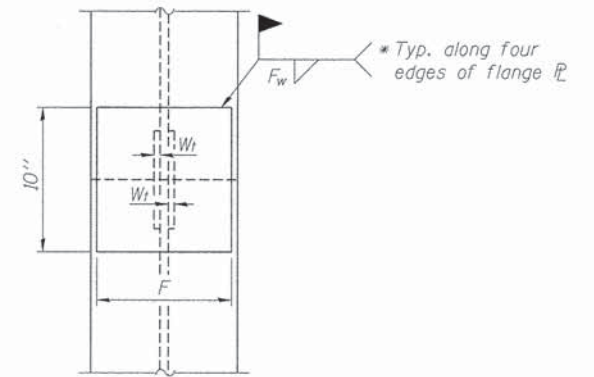


ISOMETRIC VIEW

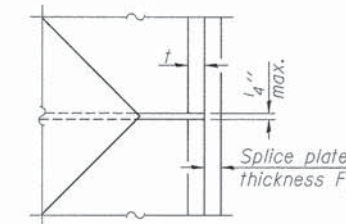
WELDED COMMERCIAL SPLICE



ELEVATION



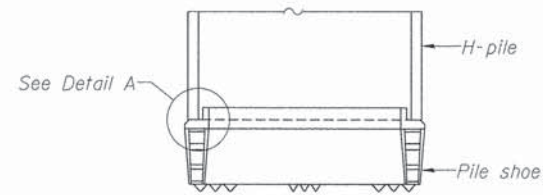
END VIEW



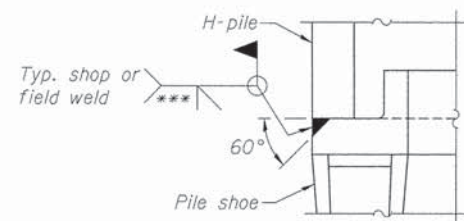
DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	F _t	F _w	W	W _t	W _w
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5 9/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5 9/8"	1/2"
x89	12 1/2"	3/4"	1/16"	7 3/4"	5 9/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5 9/8"	1/2"
HP 12x84	10"	7/8"	1/16"	6 1/2"	5 9/8"	1/2"
x74	10"	7/8"	1/16"	6 1/2"	5 9/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

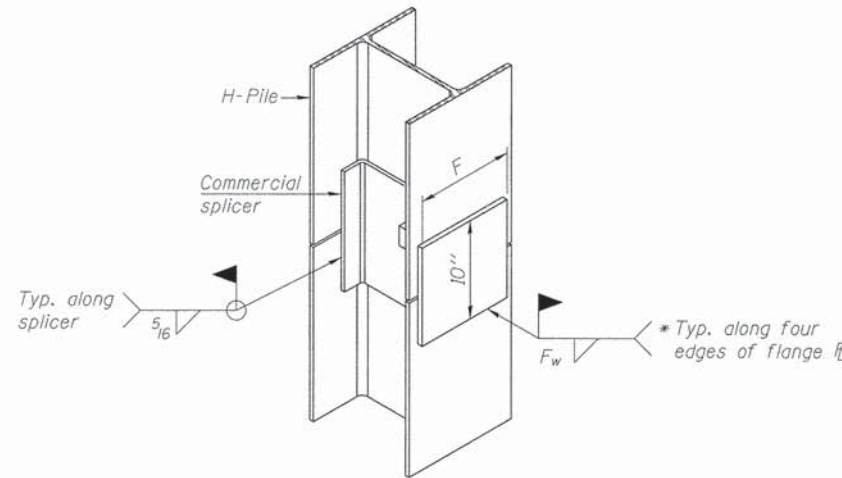


ELEVATION



DETAIL A

H-PILE SHOE ATTACHMENT



ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).

Note:
The steel H-piles shall be according to AASHTO M270 Grade 50.

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FILE NAME =	USER NAME = rgrimm	DESIGNED - AWH	REVISED -
099_3286_61A02.18_pile.dgn	PLOT SCALE =	CHECKED - MRB/MFH	REVISED -
	PLOT DATE = 12/5/2013	DRAWN - RMG	REVISED -
		CHECKED - MRB/MFH	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HP PILE DETAILS
STRUCTURE NO. 099-3286

SHEET NO. S18 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	35
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	



Illinois Department of Transportation
Division of Highways
Alfred Benesch

SOIL BORING LOG

Page 1 of 3

Date 11/5/12

ROUTE Richton Road DESCRIPTION West Abutment LOGGED BY JL

SECTION LOCATION Near Crete, IL, SEC. 7, TWP. 34N, RNG. 15E

COUNTY Will DRILLING METHOD Mud Rotary HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTHS	SOIL DESCRIPTION	DEPTH (ft)	BLOWS (/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)	SURFACE WATER ELEV.	DEPTH (ft)	BLOWS (/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)
099-3286		Asphalt	674.10				646.41				
		SANDY LOAM (SM) 5-15% fine gravel; 60-70% fine to coarse sand; nonplastic; light gray; moist; medium dense (Crushed Limestone)	671.60	13			644.41				
		CLAY (CH) 10-20% fine to medium sand; high plasticity; dark grayish brown with dark greenish gray; wet; stiff (Till)	669.10	2							
		SILTY CLAY LOAM (CL) 10-20% fine to coarse sand; medium plasticity; dark grayish brown with yellowish red; wet; stiff (Till)	668.10	4.5	19.3						
		CLAY (CH) 10-20% fine to coarse sand; high plastic; dark grayish brown with dark greenish gray; wet; stiff (Till)	664.10	3	2.8						
		SILTY CLAY LOAM (CL) 10-20% fine to coarse sand; medium plastic; dark grayish brown with dark gray and yellowish red; wet; very stiff (Till)	663.00	2.8	17.6						
		CLAY (CH) 10-20% fine to coarse sand; high plastic; grayish brown with gray; wet; medium stiff (Till)	659.10	2	0.8						
		SILTY CLAY (CL) 5-15% fine to medium sand; medium plasticity; greenish gray with grayish brown slightly mottled with yellowish red; wet; medium stiff (Till)	658.30	1.0	23.5						
		SILTY CLAY (CL) same as above except stiff (Till)	655.10	3	3.6						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



Illinois Department of Transportation
Division of Highways
Alfred Benesch

SOIL BORING LOG

Page 2 of 3

Date 11/5/12

ROUTE Richton Road DESCRIPTION West Abutment LOGGED BY JL

SECTION LOCATION Near Crete, IL, SEC. 7, TWP. 34N, RNG. 15E

COUNTY Will DRILLING METHOD Mud Rotary HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTHS	SOIL DESCRIPTION	DEPTH (ft)	BLOWS (/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)	SURFACE WATER ELEV.	DEPTH (ft)	BLOWS (/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)
099-3286		SILTY CLAY LOAM (CL) 10-20% fine to medium sand; medium plasticity; dark grayish brown; saturated; hard (Till)	615.10	12			646.41				
		CLAY (CL) 35-45% fine to medium sand; medium plasticity; grayish brown; saturated; hard (Till)	610.10	13	2.6		644.41				
		SILTY CLAY LOAM (CL) 15-25% fine to medium sand; medium plasticity; grayish brown; saturated; hard (Till)	603.10	14	2.6						
		CLAY (CL) 30-40% fine to medium sand; medium plasticity; grayish brown mottled with yellowish red; saturated; hard (Till)	625.90	10	4.9						
		SAND (SP) 95-100% fine sand; nonplastic; grayish brown; saturated; very dense (Alluvium)	600.10	14	2.6						
		CLAY (CL) 40-50% fine to medium sand; medium plasticity; dark grayish brown; saturated; hard (Till)	620.10	30							
		SAND (SP/SM) 0-10% fine to coarse gravel 85-95% fine to coarse sand; nonplastic; grayish brown; saturated; very dense (Alluvium)	595.10	24							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



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SOIL BORING LOG

Page 3 of 3

Date 11/5/12

ROUTE Richton Road DESCRIPTION West Abutment LOGGED BY JL

SECTION LOCATION Near Crete, IL, SEC. 7, TWP. 34N, RNG. 15E

COUNTY Will DRILLING METHOD Mud Rotary HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTHS	SOIL DESCRIPTION	DEPTH (ft)	BLOWS (/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)	SURFACE WATER ELEV.	DEPTH (ft)	BLOWS (/6")	UNCONSOLIDATED QUANTITY (tsf)	MOISTURE (%)
099-3286		SAND (SP/SM) same as above (Alluvium)	594.60				646.41				
		LIMESTONE fractured, rough drilling	593.60				644.41				
		LIMESTONE	592.60								
		End of Boring	500.25								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

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099_3286_61A02_19_log1.dgn

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CHECKED - MRB
DRAWN - RMG
CHECKED - MRB
PLOT SCALE =
PLOT DATE = 12/5/2013

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REVISED -
REVISED -
REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS 1 OF 3
STRUCTURE NO. 099-3286

SHEET NO. S19 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	36
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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SOIL BORING LOG

Date 11/6/12

ROUTE Richton Road DESCRIPTION East Abutment LOGGED BY JL
SECTION _____ LOCATION Near Crete, IL, SEC. 7, TWP. 34N, RNG. 15E
COUNTY Will DRILLING METHOD Mud Rotary HAMMER TYPE CME Automatic

STRUCT. NO.	BORING NO.	Station	Offset	Ground Surface Elev.	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOIST (%)	DESCRIPTION	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOIST (%)
099-3286	SB-2	13+82	7.00ft Rt	674.10					Asphalt				
					673.10								
					672.60	5			SANDY LOAM (SM) 5-15% fine gravel 70-80% fine to coarse sand; nonplastic; light grayish brown; moist; medium dense (Fill)		4	1.8	
						2			CLAY (CL) 30-40% fine sand; medium plasticity; dark gray with reddish brown; wet; very stiff (Till)		8	B	
					670.60	3			SILTY CLAY LOAM (CL) 10-20% fine to coarse sand; medium plasticity; yellowish brown with grayish brown; wet; stiff (Fill)		4	3.7	
						4	5.5		CLAY (CL) same as above except stiff (Till)		4		
						5			SILTY CLAY LOAM (CL) 10-20% fine to coarse sand; medium plasticity; yellowish brown with grayish brown; wet; stiff (Till)		6	B	
						5			CLAY (CL) 30-40% fine sand; medium plasticity; grayish brown heavily mottled with yellowish red and; saturated; stiff (Till)		1.3	22.8	
						4	5.4		SILTY CLAY (CL) 10-20% fine sand; medium plasticity; grayish brown heavily mottled with yellowish red and; saturated; medium stiff (Till)		0.8	P	
						6			CLAY (CL) 30-40% fine to coarse sand; medium plasticity; yellowish brown with grayish brown; wet; stiff (Till)		2		
					665.60	3			SILTY CLAY (CL) 10-20% fine sand; medium plasticity; grayish brown heavily mottled with yellowish red and; saturated; medium stiff (Till)		3	5.9	
						4			CLAY (CL) 40-50% fine to coarse sand; medium plasticity; grayish brown with dark gray; saturated; medium stiff (Till)		2		
						5			SILTY CLAY LOAM (CL) 10-20% fine sand; medium plasticity; dark gray; saturated; stiff (Till)		15	B	
					663.10				CLAY (CL) 40-50% fine to coarse sand; medium plasticity; grayish brown with dark gray; saturated; medium stiff (Till)		6		
					662.75				SILTY CLAY LOAM (CL) 10-20% fine sand; medium plasticity; dark gray; saturated; stiff (Till)		11	5.9	
					662.00		2.5	18.5	SILTY CLAY LOAM (CL) 10-20% fine sand; medium plasticity; dark gray; saturated; stiff (Till)		15	B	
						3			SILTY CLAY LOAM (CL) 15-25% fine to coarse sand; medium plasticity; black with yellowish brown slightly mottled with; wet; very stiff (Till)		9		
						4	2.8		CLAY (CL) 30-40% fine to medium sand; medium plasticity; grayish brown with yellowish brown; wet; very stiff (Till)		12		
						7			SANDY LOAM (SM) 55-65% fine sand; low plasticity; dark grayish brown; saturated; (Till)		18		
					658.10	2			CLAY (CL) 20-30% fine to medium sand; medium plasticity; dark grayish brown; saturated;		6		
						4	2.4		SAND (SP) 95-100% fine sand; nonplastic; grayish brown; saturated; very dense (Alluvium)		10	4.6	
					656.10	4			rough drilling driller noted gravel (Alluvium)		15	B	
					655.60			16.3	SILT (ML) 5-15% fine sand; low plasticity; grayish brown; saturated; very dense (Alluvium)		6		
						10			rough drilling driller noted gravel (Alluvium)		10	4.6	
					654.10	20			CLAY (CL) 20-30% fine to medium sand; medium plasticity; dark grayish brown; saturated;		15	B	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

Date 11/6/12

ROUTE Richton Road DESCRIPTION East Abutment LOGGED BY JL
SECTION _____ LOCATION Near Crete, IL, SEC. 7, TWP. 34N, RNG. 15E
COUNTY Will DRILLING METHOD Mud Rotary HAMMER TYPE CME Automatic

STRUCT. NO.	BORING NO.	Station	Offset	Ground Surface Elev.	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOIST (%)	DESCRIPTION	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOIST (%)
099-3286	SB-2	13+82	7.00ft Rt	674.10					SILTY CLAY LOAM (CL) 15-25% fine to medium sand; medium plasticity; dark grayish brown; saturated; hard (Till)				
									hard (Till)				
									SILTY CLAY LOAM (CL) 15-25% fine to medium sand; medium plasticity; dark grayish brown; saturated; hard (Till)				
									rough drilling driller noted gravel (Till)				
									CLAY (CH) 5-15% fine sand; high plasticity; dark gray; saturated; hard (Till)				
									SAND (SP) 95-100% fine sand; nonplastic; grayish brown; saturated; very dense (Alluvium)				
									rough drilling driller noted gravel (Alluvium)				
									SILT (ML) 5-15% fine sand; low plasticity; grayish brown; saturated; very dense (Alluvium)				
									rough drilling driller noted gravel (Alluvium)				
									SILT (ML) 5-15% fine sand; low plasticity; grayish brown; saturated; very dense (Alluvium)				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

Date 11/6/12

ROUTE Richton Road DESCRIPTION East Abutment LOGGED BY JL
SECTION _____ LOCATION Near Crete, IL, SEC. 7, TWP. 34N, RNG. 15E
COUNTY Will DRILLING METHOD Mud Rotary HAMMER TYPE CME Automatic

STRUCT. NO.	BORING NO.	Station	Offset	Ground Surface Elev.	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOIST (%)	DESCRIPTION	DEPTH (ft)	BLOWS (/6")	UCS (tsf)	MOIST (%)
099-3286	SB-2	13+82	7.00ft Rt	674.10					LIMESTONE (continued)				
									End of Boring				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



Illinois Department of Transportation
Division of Highways
Alfred Benesch

SOIL BORING LOG

Page 1 of 1

Date 11/5/12

ROUTE Richton Road DESCRIPTION Channel Flow Line LOGGED BY JL
SECTION _____ LOCATION Near Crete, IL, SEC. 7, TWP. 34N, RNG. 15E
COUNTY Will DRILLING METHOD Soil Probe HAMMER TYPE _____

STRUCT. NO. 099-3286 DEPTHS UCS MOIST
Station _____ H S Qu T
BORING NO. SB-3 Surface Water Elev. 646.41 ft
Station 13+20 Stream Bed Elev. 644.41 ft
Offset 2.00ft Rt Groundwater Elev.:
First Encounter _____ ft
Upon Completion _____ ft
Ground Surface Elev. 649.90 ft (ft) (/6") (tsf) (%) After _____ Hrs. _____ ft

(CL) % sand; medium plasticity; yellowish brown slightly mottled with gray; wet; stiff (Peoria)	648.90				
(CL) 0-10% fine gravel 10-20% fine to coarse sand; medium plasticity; grayish brown with yellowish brown; wet; stiff (Till)	647.90				
(CL) 10-20% fine sand; medium plasticity; greenish gray slightly mottled with black; wet; stiff (Till)	646.90				
(CL) 10-20% fine sand; medium plasticity; greenish gray slightly mottled with black; wet; stiff (Till)	645.90				
(CL) same as above except medium stiff (Till)	644.40				
(CL) % sand; medium plasticity; greenish gray slightly mottled with black; wet; very stiff (Till)	644.40				
(CL) 5-15% fine sand; medium plasticity; grayish brown; wet; very stiff (Till)	641.90				
End of Boring					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10204

FILE NAME *	USER NAME = rgrimm	DESIGNED - AWH	REVISED -
099_3286_61A02_21_log3.dgn		CHECKED - MRB	REVISED -
	PLOT SCALE *	DRAWN - RMG	REVISED -
	PLOT DATE = 12/5/2013	CHECKED - MRB	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS 3 OF 3
STRUCTURE NO. 099-3286

SHEET NO. S21 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	38
CONTRACT NO. 61A02				

ILLINOIS FED. AID PROJECT

PLUM VALLEY BRIDGE

GENERAL NOTES AND SPECIFICATIONS

1. Drawings and Specifications

These drawings and the specifications and notes hereon are an instrument of service only. They shall be used for no other purpose than the construction of the structure named hereon without the written consent of the Structural Engineer, George P. Wright, Jr.

The Structural Engineer shall make any and all revisions, additions and deletions to these working drawings and specifications. Revisions, additions and deletions to the Structural Engineer's working drawings and specifications by others shall void the Structural Engineer's responsibility for any part of the working drawings and specifications affected directly or indirectly by the changes.

2. Dimensions and Quantities

All dimensions shown on these working drawings, with the exception of footing depth, are believed to be correct; however, no guarantee of dimensional accuracy is made or implied. The Contractor shall be solely responsible for confirming all measurements affecting his work. The dimensions and quantities shown hereon are for guidance only. The work to be completed shall be paid for on a lump sum basis, and the contract price shall include all items of material, labor and equipment usage required to complete the work called for in the contract in a finished and workman-like manner acceptable to the Structural Engineer and the Owner. Any differences between "As Built" dimensions and quantities and those shown on these working drawings, except for piling depth, shall be assumed to have been verified by the Contractor before making his lump sum bid proposal for the complete work. Piling shall be furnished and driven at the unit prices per foot stated in the contract. If it is required by site conditions that the footings be redesigned to accommodate lower allowable soil pressure than those used in the original design, quantities of excess concrete, reinforcing steel, excavation and sheeting shall be paid for at the unit prices stated in the contract for extra work.

3. Referenced Specifications

The design of the work and the construction thereof are intended to be in conformance with the following referenced specifications:

- Standard Specifications for Road and Bridge Construction, adopted 7-1-76 Illinois Department of Transportation. (abbreviated IDOT hereinafter)
- Standard Specifications for Highway Bridges, adopted by the American Association of State Highway and Transportation Officials Eleventh Edition 1972. (abbreviated AASHTO hereinafter)
- Building Code Requirements for Reinforced Concrete ACI 318-71, American Concrete Institute. (abbreviated ACI hereinafter)
- American Society for Testing Materials, individual sections as applicable. (abbreviated ASTM hereinafter)

The above specifications are hereby made a part of the specifications for the work shown on these drawings. Specific references are given in the text of Notes and Specifications for Excavation, Backfill, Concrete, Reinforcing Steel, etc., on following drawings, and as follows on this drawing.

The designation Engineer in IDOT shall mean the Structural Engineer for this project.

4. Contractor's Responsibilities

The responsibilities of the Contractor shall include but not be limited to the items which follow. The Owner, Owner's employees, the Civil Engineer or the Structural Engineer shall not be responsible in these matters whatsoever.

- The Contractor shall have sole responsibility to comply with and enforce all applicable local, county, state and federal laws and requirements for the safety of his employees, suppliers, subcontractors, the public. Such responsibility shall include enforcement of all applicable provisions of the Occupational Safety and Health Act (OSHA), and regulations adopted pursuant thereto.
- The Contractor shall be responsible for all construction means, methods, techniques, sequences and procedures; and for safety precautions and programs, including the safety and adequacy of shoring, forms, sheeting, safety railing, and scaffolding to be used on this project which shall be designed to adequately support any and all dead loads and construction loads which may come upon them.
- The Contractor shall indemnify and hold harmless the Owner, the Civil Engineer, and the Structural Engineer and their directors, officers, agents, servants and employees from any and all losses, expenses, damages (including loss of use), demands and claims, arising out of or occasioned by matters covered by this Contract and the work hereunder, and to defend any suit or action brought against them, or any of them, based upon any such injury or alleged injury (including death) or damage (including loss of use), and to pay all damages, judgments, costs and expenses, including attorney's fees, in connection with said damages, injuries, claims, demands or suits, or resulting therefrom.
- In furtherance of the indemnity set forth in subparagraph c, above, but not in limitation thereof, the Contractor shall take out and maintain, at its own cost and expense, and before commencing any of its operations hereunder, the following kinds and minimum limits of insurance coverage:
 - Workmen's Compensation, Occupation Disease Statutory or unlimited amounts
 - Employer's Liability \$100,000
 - Comprehensive General (Including blanket Assumed Contractual Liability) Liability Insurance
 - Bodily Injury \$500,000 per person
\$1,000,000 each occurrence
\$50,000 each occurrence
 - Property Damage \$100,000 aggregate premises and contractual
 - Comprehensive Automobile Liability (Including owned, non-owned, hired vehicles)
 - Bodily Injury \$100,000 each person
\$300,000 each occurrence
 - Property Damage \$50,000 each occurrence

All such policies shall be written and issued by companies satisfactory to the Engineer. Compliance with the required insurance coverage shall be evidenced by certificates of such insurance to be forwarded to the Engineer at its business address. Such certificates shall contain a stipulation to the effect that the insurance, covered by the certificate will not be cancelled by lapse of time or otherwise except upon not less than ten (10) days prior written notice to that effect, by United States Registered or Certified Mail to the Engineer at its business address.

NOTES AND SPECIFICATIONS FOR CLEARING, EXCAVATION, BACKFILLING AND EMBANKMENT

The work specified herein shall be that done between a north-south line 40 feet west of the face of the west abutment and a north-south line 40 feet east of the face of the east abutment. Embankment east and west of the area above defined shall be furnished by the Owner.

1. Clearing

Any clearing required as designated by the Structural Engineer shall conform to the applicable provisions of IDOT as follows: Sections 201.01 thru 201.04 and 201.06 thru 201.08.

2. Excavation

Excavation shall be that required to provide for the new footings with the slopes above the level of the top of footing being maintained in a stable condition until all project work prior to permitted backfilling has been completed.

The work shall conform to the applicable provisions of IDOT as follows: Sections 502.01, 502.02, 502.03, 502.05, 502.06 thru 502.10. The following provisions shall supersede IDOT where there is a conflict:

Excavation for footings shall be accomplished so as to allow footing concrete to be placed against: (a) undisturbed earth, or (b) sheeting drive into undisturbed earth along the footing outline with subsequent excavation within the sheeting to plan depth. If the latter method is used, the sheeting shall be left in place.

The use of sheeting shall be at the option of the Contractor, and no compensation for sheeting material or labor to place same shall be paid him, whether he uses sheeting for the footings as shown on the plans or for any revised footing design needed because of soil bearing capacity.

3. Backfilling and Approach Embankment

Backfilling and approach embankment within the area defined above shall be completed by the Contractor in conformance with IDOT Sections 502.11 and 502.14.

NOTES AND SPECIFICATIONS FOR CAST-IN PLACE CONCRETE STRUCTURES

The work specified herein shall conform to the applicable provisions of IDOT as follows: Sections 503.01 thru 503.06, 503.07(b), 503.09 (as modified by notes on drawings), 503.12, and 503.15. The cost of any and all apparatuses called for on the drawings shall be included in the lump sum price stated in the contract.

All references in IDOT to the "Engineer" shall mean the Structural Engineer, George P. Wright, Jr., except that any proportioning of concrete mixtures shall be determined by the Contractor or his Concrete Supplier so as to furnish concrete in the completed structure in accordance with all applicable provisions of IDOT Section 504.

The Structural Engineer shall engage a Testing Laboratory to make all tests as required by Section 504 of IDOT. The Contractor shall reimburse the Structural Engineer for the cost of all testing expense including transportation as billed to the Structural Engineer by the Testing Laboratory plus a service charge of 20% to compensate the Structural Engineer for his handling and review of the testing reports. A schedule of Testing Laboratory fees shall be available to bidders at the Structural Engineer's Office prior to bidding.

The Structural Engineer shall not furnish to prospective bidders any approximate proportions for concrete as is called for in IDOT Section 504.08 (the last paragraph beginning on page 288). The responsibility for such approximate proportions shall rest solely with the prospective bidder or his concrete supplier.

1. Concrete

All concrete shall be Class X as called for in IDOT, except that Handrail Concrete shall have a maximum size of coarse aggregate passing a 1" sieve. All concrete shall conform to the requirements of applicable provisions of IDOT as follows: Sections 504.01 thru 504.04, 504.07 thru 504.15 except that all proportioning of mixes shall be the responsibility of the Contractor or his Concrete Supplier. The Contractor shall furnish his proposed portions for concrete mixes to the Structural Engineer at least 7 calendar days before ordering any concrete delivered to the job site. The furnishing of such proposed proportions to the Structural Engineer shall be for information purposes to assure that proper controls of the mixes are being proposed. The Structural Engineer's review shall in no way relieve the Contractor's responsibility to provide concrete in the completed structure meeting the requirements of IDOT Section 504.

A minimum of three test specimens taken from each days pour shall be required. The specimens shall be taken by the Contractor at the direction of the Testing Laboratory and shall be transported to the Testing Laboratory by Testing Laboratory personnel.

Superstructure concrete shall not be placed until the last concrete placed in the substructure has cured and attained a modulus of rupture of 650 psi. A minimum

of 24 hours notice shall be given to the Structural Engineer, so that he may inspect the footing excavation with bars in place and inspect the wall and superstructure forms with bars in place at least 2 hours before any scheduled placement of concrete.

2. Falsework and Forms

All falsework and forms shall conform to the applicable provisions of IDOT as follows: Sections 504.05 and 504.06. All provisions and understandings of Sections 504.05 and 504.06 shall apply and be conformed to by the Contractor. The Contractor shall submit detailed plans for falsework and form work for examination by the Structural Engineer. If such plans are not satisfactory to the Structural Engineer, the Contractor shall make such changes in them as may be required, but it is understood that the Structural Engineer's concurrence in the use of the plans as submitted or corrected shall in no way relieve the Contractor of full responsibility for obtaining satisfactory results and for safety and adequacy of load carrying capacity.

3. Curing of Concrete

All concrete placed on this project shall be cured and protected in conformance with the applicable provisions of IDOT as follows: Section 625.01, 625.02, 625.03, 625.04(c) and (d), 625.06, 625.07, and 625.08.

4. Reinforcing Steel for Concrete

The furnishing and placement of all reinforcing steel for this project shall conform to the applicable provisions of IDOT as follows: Section 512.01 thru 512.06 and 710.13 (a) and (b). The bars shall conform to ASTM A615 Grade 40 deformed bars.

5. Preformed Expansion Joint Filler

The preformed joint filler shall conform to the provisions of IDOT Sections 715.01 and 715.04.

6. Scuppers and Weep Holes

The pipe used for the deck scuppers and the abutment and wing wall weepers shall be of a good quality commercially available galvanized tubing.

NOTES AND SPECIFICATIONS FOR STRUCTURAL STEEL, STEEL PILING, AND STEEL RAILING

The work specified herein shall conform to the applicable provisions of IDOT as follows: Sections 507.01; 507.02 as herein modified; 507.03 thru 507.05; 507.07; 507.08; 507.11 thru 507.13; 508.01 thru 508.03; 513.01; 513.02 (h); 513.05 (a) thru (c); 513.07 (a) and (d); 513.08 thru 513.11; 513.13 thru 513.15; 513.17; 513.18; 513.19 (b), (c), (f), and (i).

Sections 507.02 (a) and 513.02 (h) shall be modified as follows: Structural Steel and Steel Piles shall conform to ASTM-A588 - "weathering type" - high - strength low - alloy structural steel. (AASHTO #222)

Steel Railings including posts, fastening material, and fasteners shall be galvanized as per AASHTO-M11 and ASTM-A385.

Structural Steel and Steel Piles shall not be painted.

Steel Railings, if that alternate is included in the contract, shall conform to Base Sheet R-24 in the IDOT Bridge Manual except as modified on these project drawings.

Payment for all Structural Steel and Steel Railings including all fastening material and fasteners shall be included in the lump sum contract price for the project.

Payment for all Steel Piles and Pile Driving shall be as follows:

1. Test Piles

Lump sum for furnishing and driving one Test Pile at the East Abutment in the permanent location of an abutment pile. The test pile shall be 70 feet in length and shall be driven 10 feet deeper after the bearing capacity called for on the substructure drawing is attained, or to refusal, whichever occurs first.

2. Furnishing Steel Piles

Price per foot for furnishing the remaining 15 piles from a list prepared by the Structural Engineer based on the results of the test pile.

3. Driving Steel Piles

Price per foot for driving the remaining 15 piles to specified bearing capacity or to refusal except that minimum penetration shall be 20 feet in natural ground unless refusal shall occur first.

NOTES AND SPECIFICATIONS FOR PRECAST TRANSVERSE DECK SLABS

All Precast Deck Slabs shall conform to the applicable provisions of IDOT as follows: Sections 505.01 thru 505.05. They shall also conform to the design shown on these project drawings.

Payment for furnishing and erecting Precast Deck Slabs including all fastening material, fasteners, curb stirrups and bolting inserts shall be included in the lump sum contract price for the bridge.

NOTES AND SPECIFICATIONS FOR BITUMINOUS PAVEMENT

It is intended that the Bituminous Wearing Surface shown on these project drawings will be furnished and applied to the bridge by the contractor constructing and paving the road abutting the bridge site.

**RIGHTON ROAD
OVER
PLUM CREEK**

George P. Wright, Jr.

GEORGE P. WRIGHT
Consulting Structural Engineer
306 Douglas Park Forest, Ill.
312-748-2880

FOR INFORMATION ONLY

099-3351

JOSEPH A. SCHUDT & ASSOCIATES
civil engineers surveyors land planners

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Matteson, Illinois 60448
phones: local 312-748-1683
chicago 312-785-6162

revisions: APRIL 22nd 1977

BUILT 1978

date: APRIL 5, 1977
scale:
design:
drawn: LAM AD
checked: SPW

BRIDGE OVER PLUM CREEK
RIGHTON ROAD - PLUM VALLEY - COATS TWP. ILL.

COVER SHEET - NOTES & SPECIFICATIONS

sheet 51 of 55
project number: 75-35



Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10204

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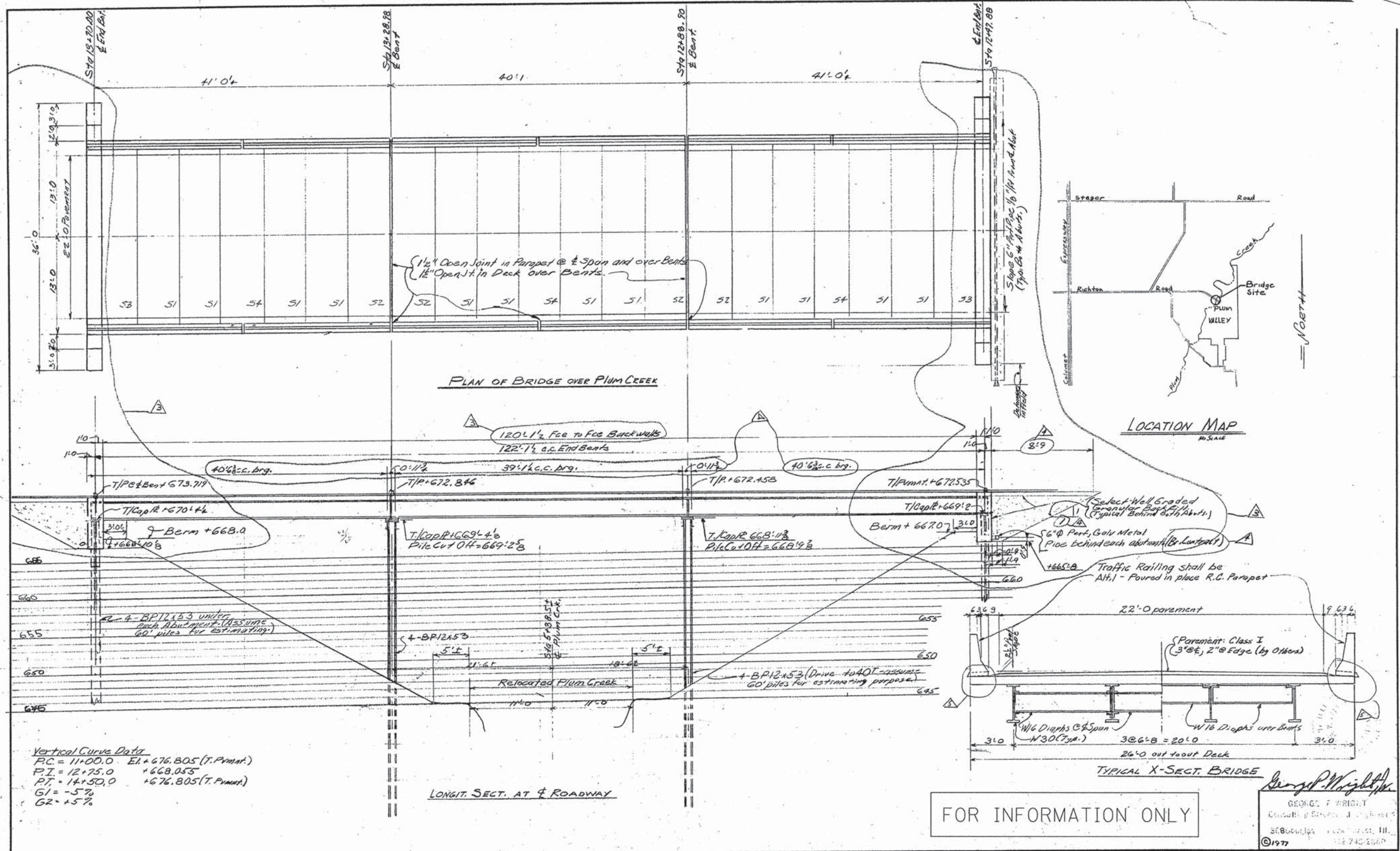
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CHECKED - MRB	REVISÉD -
DRAWN - RMG	REVISÉD -
CHECKED - MRB	REVISÉD -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS 1 OF 5
STRUCTURE NO. 099-3286

SHEET NO. S22 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	39
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	



Vertical Curve Data
 P.C. = 11+00.0 E.I. + 676.805 (T.P. Int.)
 P.I. = 12+75.0 + 668.055
 P.T. = 14+50.0 + 676.805 (T.P. Int.)
 G1 = -5%
 G2 = +5%

FOR INFORMATION ONLY

George F. Wright
 GEORGE F. WRIGHT
 CONSULTING ENGINEER
 3880 S. W. 11th St., Suite 100
 Miami, FL 33155
 (305) 551-1111
 ©1977

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revisions:
 1. Add L.A. Map, Corr. set up
 2. Corr. Abut. to Contract Ref. for G.L.
 3. Lower Br. 5% Increase
 4. V.C. from 100' to 350'; Scale for
 5/27/77 A.L. to Abut.; Add Abut. Drains

4-7-77 - Correct Drive Admit Drain
 Pipe @ Abut. to Contract Ref. for G.L.
 Revise scupper - As Built 8-18-78
 J.P.W.

date: 4/5/77
 scale:
 design: J.P.W.
 drawn: J.P.W.
 checked: J.P.W.

Bridge over Plum Creek
 Richton Road - Plum Valley - Crete Twp, Illinois
 General Plan & Elevation

sheet 52 of 55
 project number: 75-35

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 Chicago, Illinois 60601
 312-565-0450 Job No. 10204

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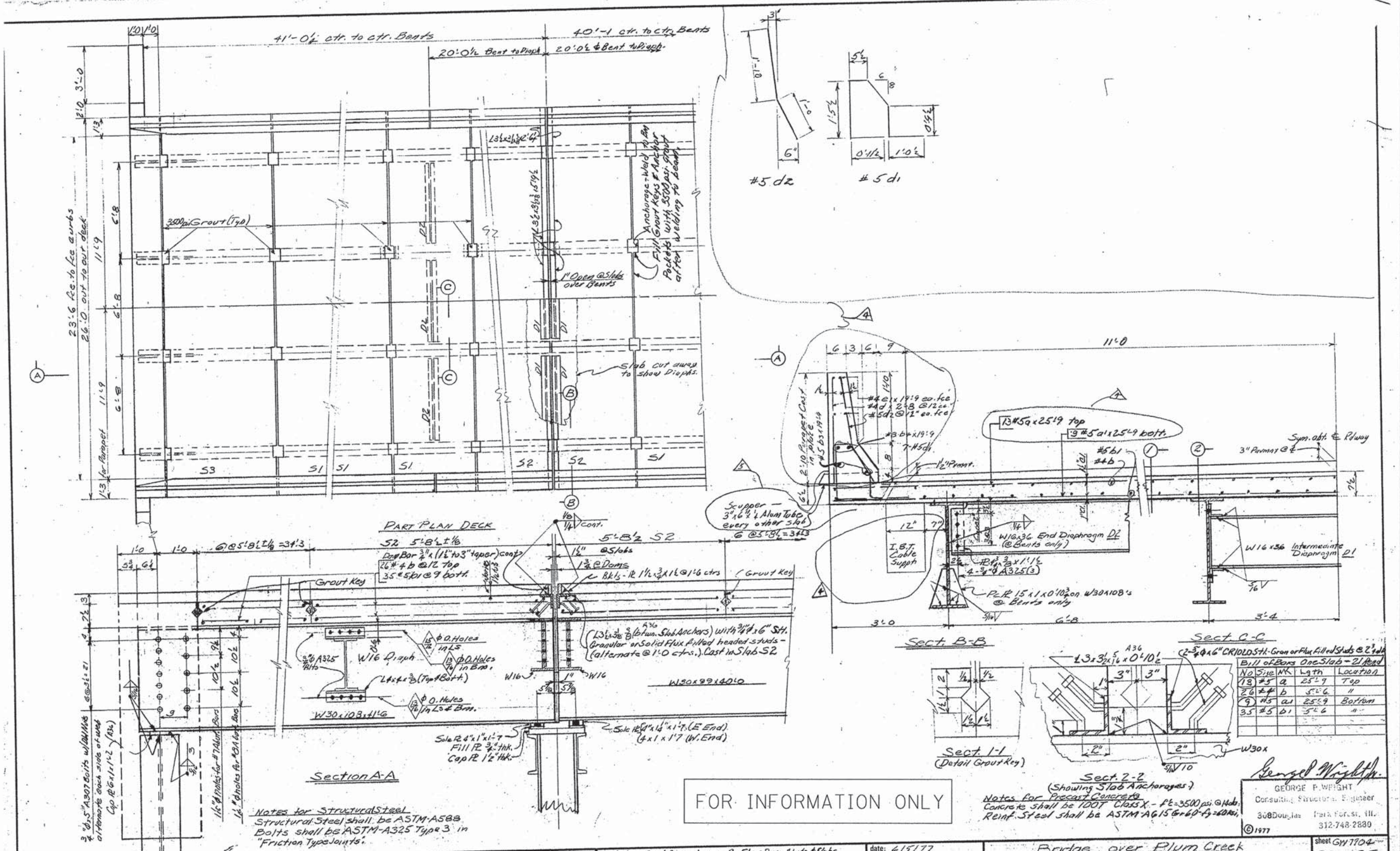
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STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

EXISTING PLANS 2 OF 5
 STRUCTURE NO. 099-3286
 SHEET NO. S23 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	40
ILLINOIS FED. AID PROJECT			CONTRACT NO. 61A02	

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FOR INFORMATION ONLY

NOTES FOR STRUCTURAL STEEL
 Structural Steel shall be ASTM-A588
 Bolts shall be ASTM-A325 Type 3 in
 "Friction Type Joints".

NOTES FOR PRECAST CONCRETE
 Concrete shall be 100T Class X - f_c = 3500 psi @ 28 days
 Reinf. Steel shall be ASTM-A615 Gr 60 - f_y = 60,000 psi

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 GEORGE P. WRIGHT
 Consulting Structural Engineer
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revisions:
 8/9/77 - Genl. Revis. - Lower Br. S's - Revis. Apts. Nicks.
 9-7-77 - Release Permit, Add Slab Reinf. acc. I.B.T.
 Cable Supports, Detail Supports - Release for Construction.
 8-18-78 - Revis. Scupper - R. Bui. Lt. & P.W.

date: 4/15/77
 scale:
 design: G.P.W.
 drawn: J.P.W., G.P.W.
 checked: G.P.W.

Bridge over Plum Creek
 Richton Road - Plum Valley - Crete Twp., Illinois
 Superstructure Design

sheet GY1104
 S3 of 55
 project number:
 15-35

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 Chicago, Illinois 60601
 312-565-0450 Job No. 10204

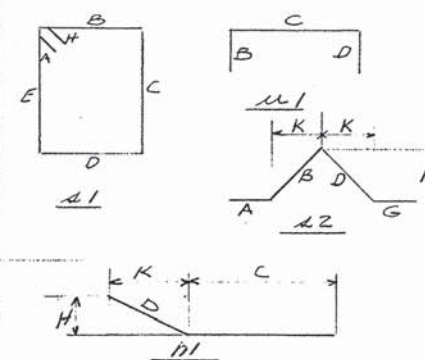
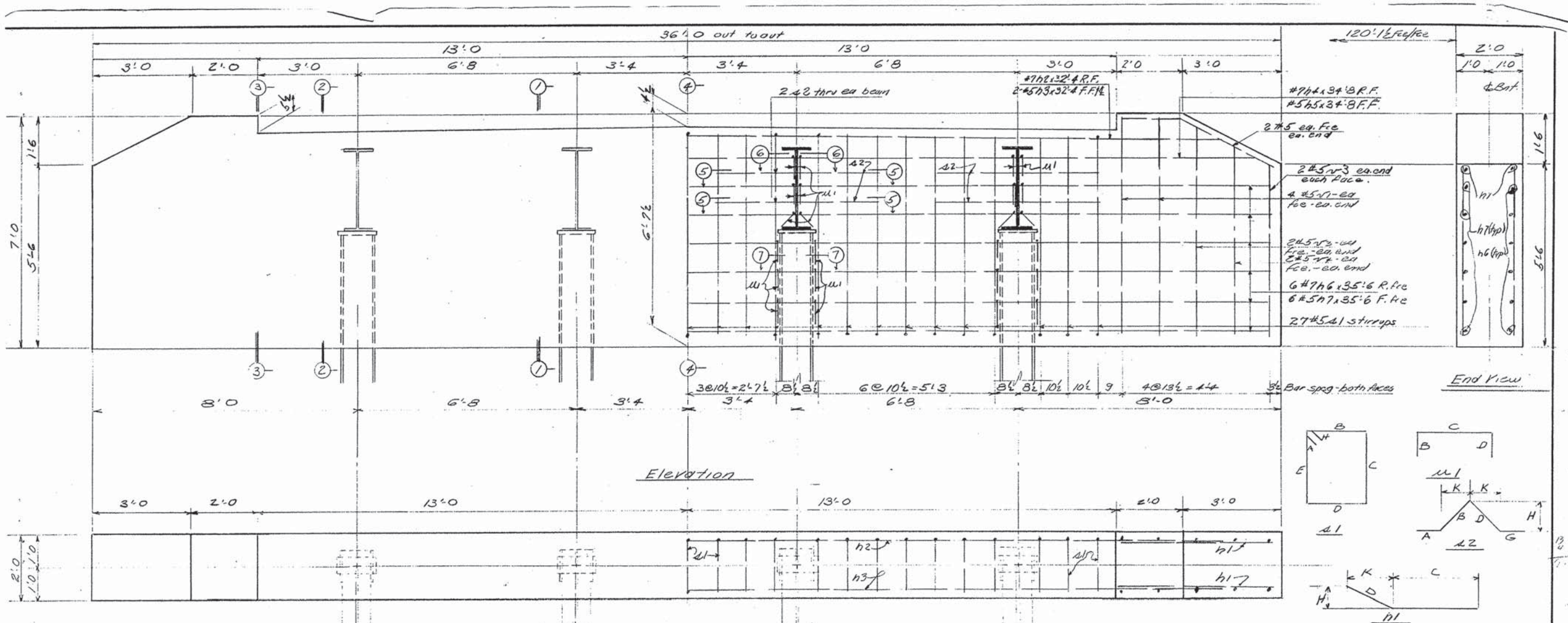
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STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

EXISTING PLANS 3 OF 5
 STRUCTURE NO. 099-3286
 SHEET NO. S24 OF S26 SHEETS

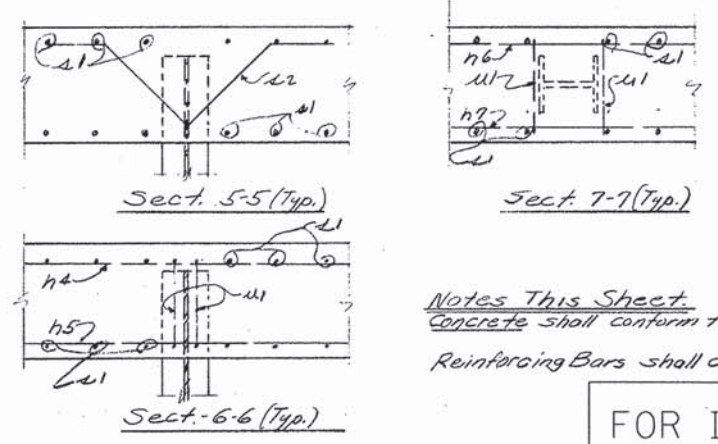
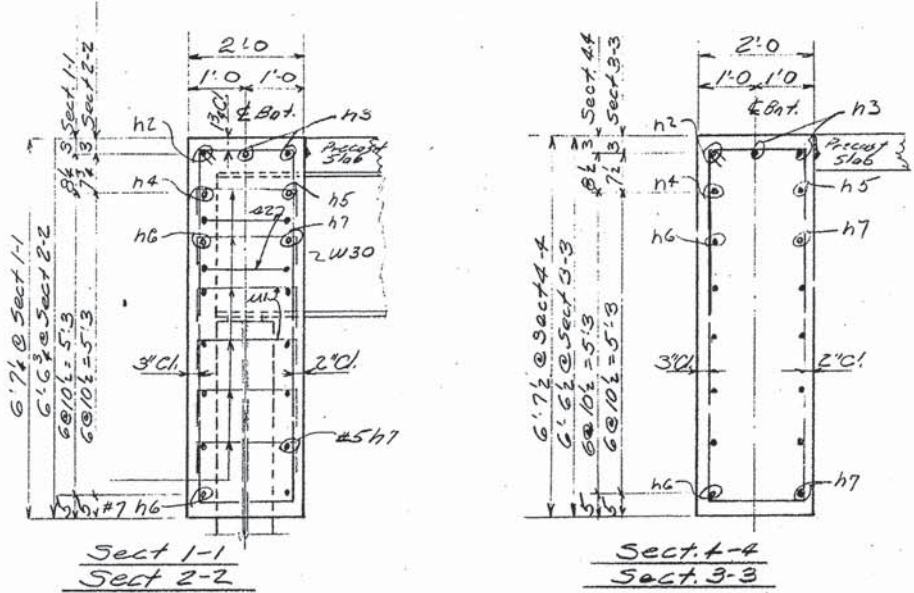
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	41
ILLINOIS FED. AID PROJECT			CONTRACT NO. 61A02	

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Bill of Bars for One Abutment - Two Required

NK. No.	Size	A/B	B	C	D	E	H	K	Lgth	Location
#1	8	5							6'6"	Wing vert
#13	4	5							6'3"	" "
#14	4	5							5'2"	" "
#15	4	5							5'13"	" "
#11	4	5								Topking
#12	1	7							32'4"	Horiz R.F.
#13	2	5								" F.F.F.F.
#14	1	7							3'18"	" R.F.
#15	1	5							"	" R.F.
#16	6	7							35'6"	" R.F.
#17	6	5							"	" R.F.
#1	27	5	5 1/2	11 7/8	6 1/2	11 7/8	6 1/2		18'6"	Body vert.
#2	8	7	12	26	0	26			18'2 1/8"	9'4" Thru Bms.
#1	48	6							2'7"	F.F.F.F.



Notes This Sheet:
 Concrete shall conform to IDOT-Class X₁ f_c = 3500 psi at 14ds.
 Reinforcing Bars shall conform to ASTM-A615 Gr60 (f_y = 60 ksi.)

FOR INFORMATION ONLY

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 Consulting Structural Engineer
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 Matteson, Illinois 60443
 phones: local 312-748-1683
 chicago 312-785-6162

revisions: 8-9-77: General Revision
 9-7-77 - Release for Constr
 8-18-78 "As Built" JPM

date: 8/9/77
 scale: N.T.S.
 design: JPM
 drawn: G.P.W., J.P.W.
 checked: JPM

Bridge over Plum Creek
 Richton Road - Plum Valley - Crete Twp., Ill.
 Abutment Design

sheet 4 of 55
 project number: 75-35

benesch
 engineers - scientists - planners
 Alfred Benesch & Company
 205 North Michigan Avenue, Suite 2400
 Chicago, Illinois 60601
 312-565-0450 Job No. 10204

FILE NAME	USER NAME	DESIGNED	CHECKED	PLOT SCALE	PLOT DATE
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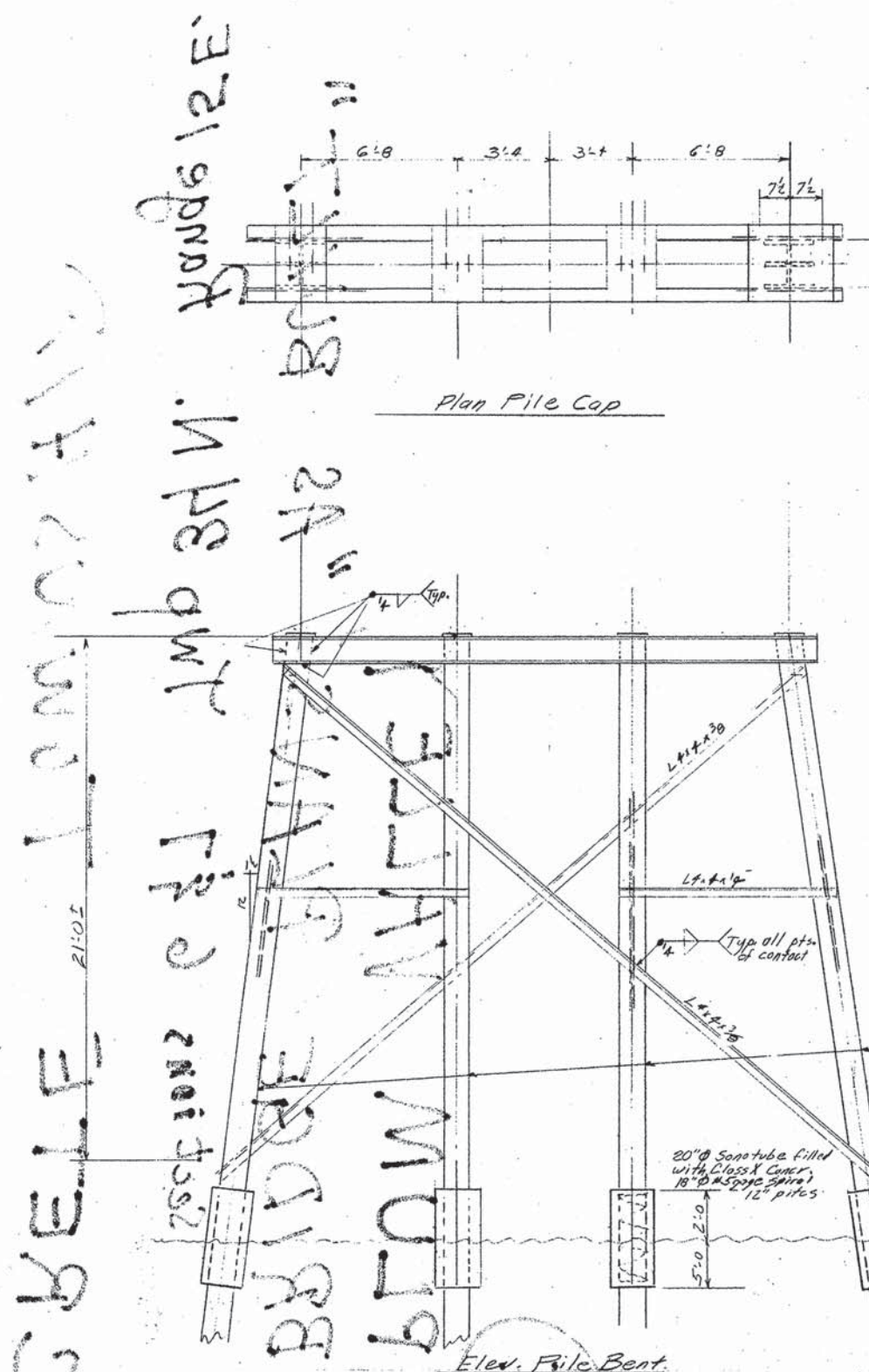
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

EXISTING PLANS 4 OF 5
 STRUCTURE NO. 099-3286
 SHEET NO. S25 OF S26 SHEETS

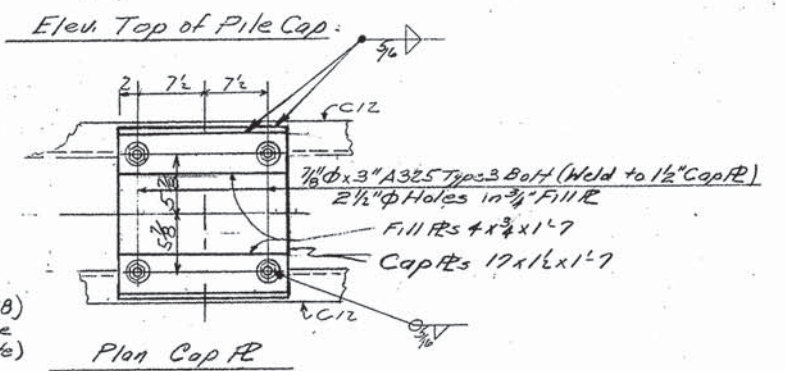
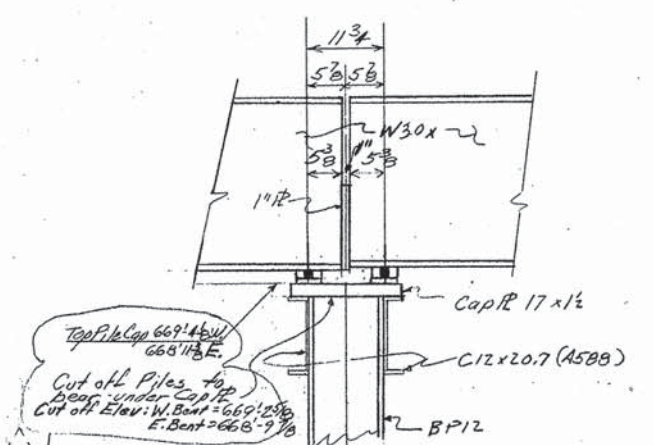
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	11-02118-01-BR	WILL	50	42

CONTRACT NO. 61A02
 (ILLINOIS) FED. AID PROJECT

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 12/5/2013



Width to be determined in field. Shims shall be sized after piling driven and a survey made.



BP12x53 (ASTM A500)
Drive to 40T (Assume 60' piles for estimate)

20" Ø Sonotube filled with Class X concrete, 18" Ø rebar spire, 12" piles

FOR INFORMATION ONLY

Note: All Material Billed this Sht. A500

See S4 of S5 for notes on Piling & Pile Driving
George P. Wright

GEORGE P. WRIGHT
Consulting Structural Engineer
308 Douglas - Park Forest, Ill.
© 1977 312-748-2800

JOSEPH A. SCHUDT & ASSOCIATES
civil engineers surveyors land planners
3020 West 216th Street
Matteson, Illinois 60448
phones: local 312-748-1683
chicago 312-785-6162

revisions:
2/9/77 - Revs. Cap IR & Fills & Brgs to suit available matl.
7-7-77 - Release for Const'n
8-18-78 "As-Built" *J.P.W.*

date: APRIL 5, 1977
scale:
design: *APW*
drawn: *APW*
checked: *J.P.W.*

Bridge over Plum Creek
Richton Road - Plum Valley - Crete Twp, Illinois
Steel Pile Bents
sheet 55 of 55
project number: 75-35

benesch
engineers · scientists · planners
Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10204

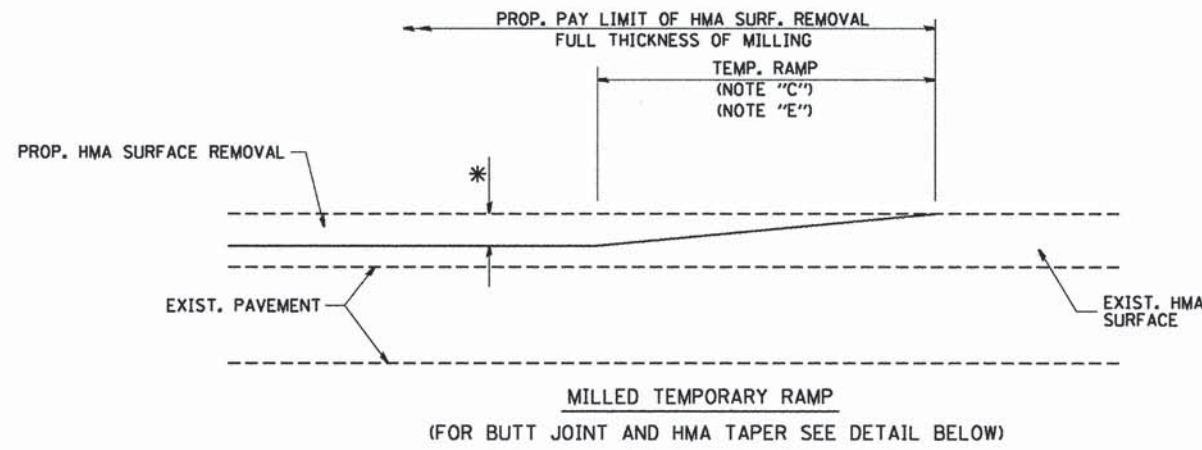
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

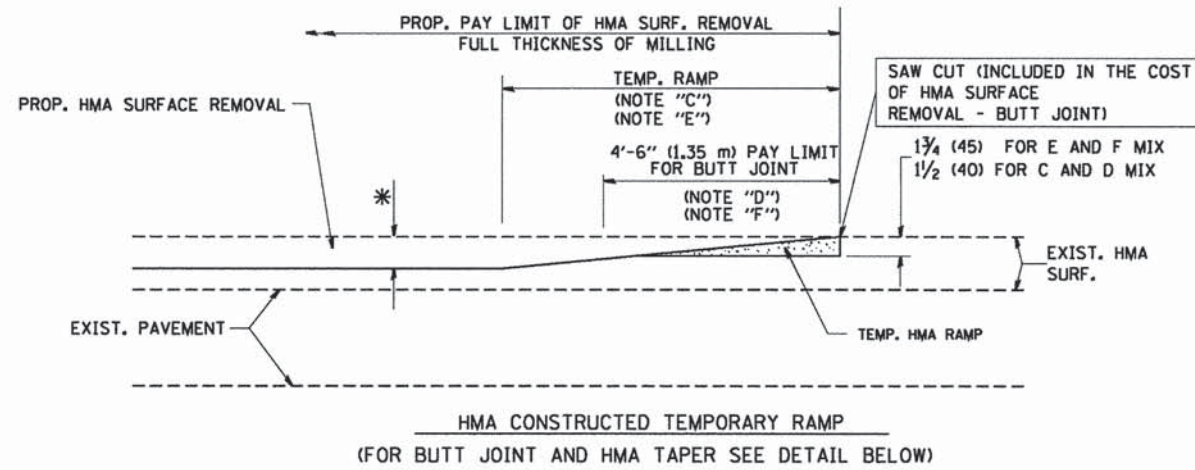
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SHEET NO. S26 OF S26 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	11-02118-01-BR	WILL	50	43
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	

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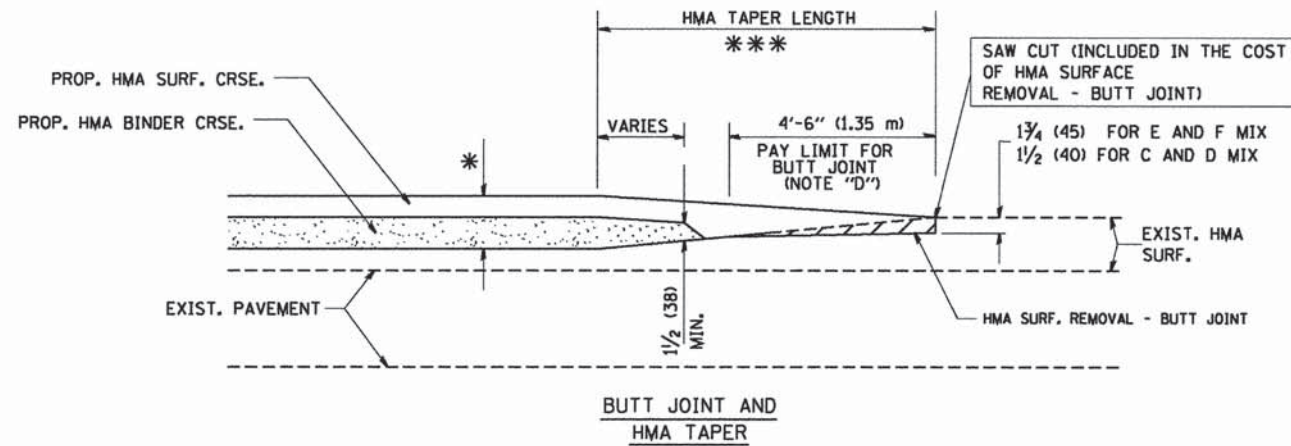


OPTION 1

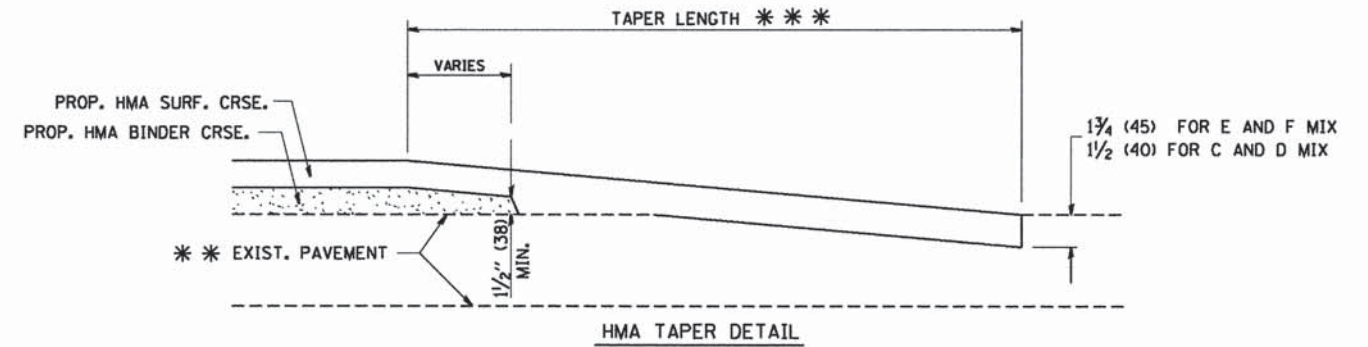
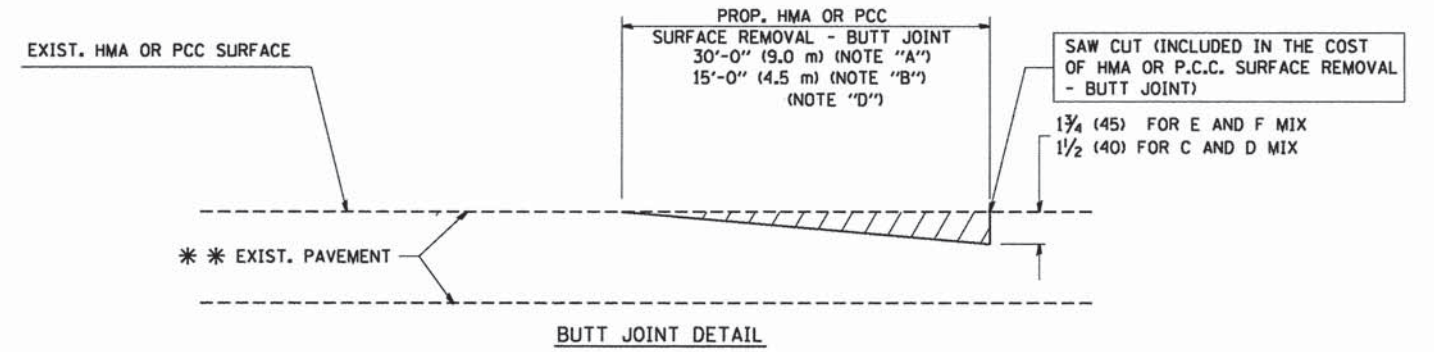


OPTION 2

TYPICAL TEMPORARY RAMP



**TYPICAL BUTT JOINT AND HMA TAPER
FOR MILLING AND RESURFACING**



**TYPICAL BUTT JOINT AND HMA TAPER
FOR RESURFACING ONLY**

*** PC CONCRETE, HMA OR HMA RESURFACED PAVEMENT.

NOTES

- A: MAINLINE ROADWAYS AND MAJOR SIDE ROADS.
- B: MINOR SIDE ROADS.
- C: THE TEMP. RAMP SHALL BE CONSTRUCTED IMMEDIATELY UPON REMOVAL OF THE EXISTING HMA SURFACE.
- D: THE BUTT JOINT SHALL BE CONSTRUCTED IMMEDIATELY PRIOR TO PLACING THE PROPOSED HMA COURSES.
- E: TAPER THE TEMP. RAMP AT A RATE OF 3'-0" (900 mm) PER 1 INCH (25 mm) OF MILLING THICKNESS.
- F: INSTALLATION AND REMOVAL OF THE 4'-6" (1.35 m) TEMP. RAMP IS INCLUDED IN COST OF HMA SURFACE REMOVAL - BUTT JOINT
- G: SEE ARTICLE 406.08 AND 406.14 OF THE STANDARD SPECIFICATIONS FOR "HMA AND/OR PCC SURFACE REMOVAL, BUTT JOINT".

* SEE TYPICAL SECTIONS FOR MILLING THICKNESS.

*** 20'-0" (6.1 m) PER 1 (25) RESURFACING (NOTE "A")
10'-0" (3.0 m) PER 1 (25) RESURFACING (NOTE "B")

BASIS OF PAYMENT:

THE BUTT JOINT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER SQUARE YARD (SQUARE METER) FOR "HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT" OR FOR "PORTLAND CEMENT CONCRETE SURFACE REMOVAL - BUTT JOINT".

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

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DRAWN -
CHECKED -
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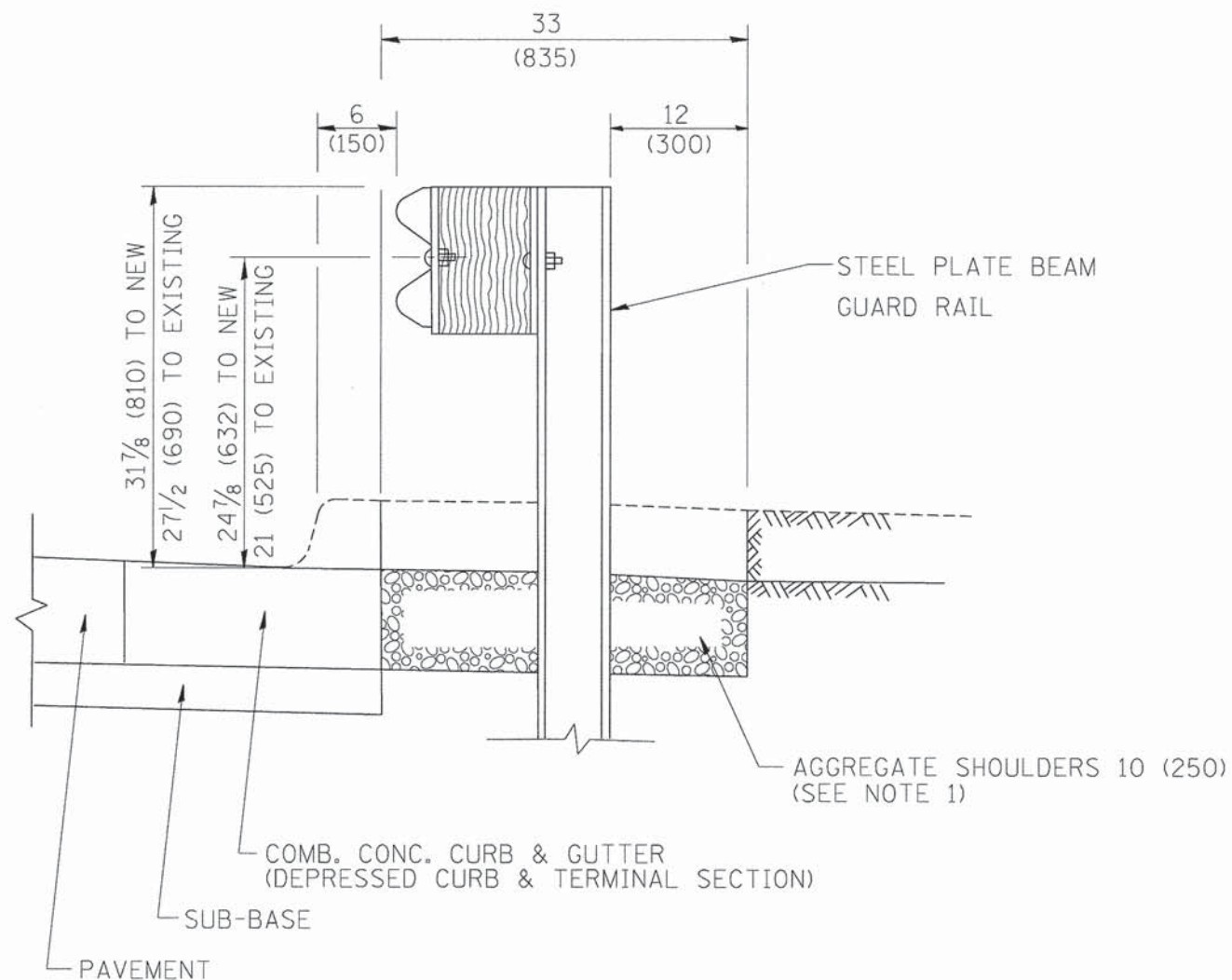
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REVISED - M. GOMEZ 04-06-01
REVISED - R. BORO 01-01-07

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**BUTT JOINT AND
HMA TAPER DETAILS**

SCALE: NONE SHEET NO. 1 OF 1 SHEETS STA. TO STA.

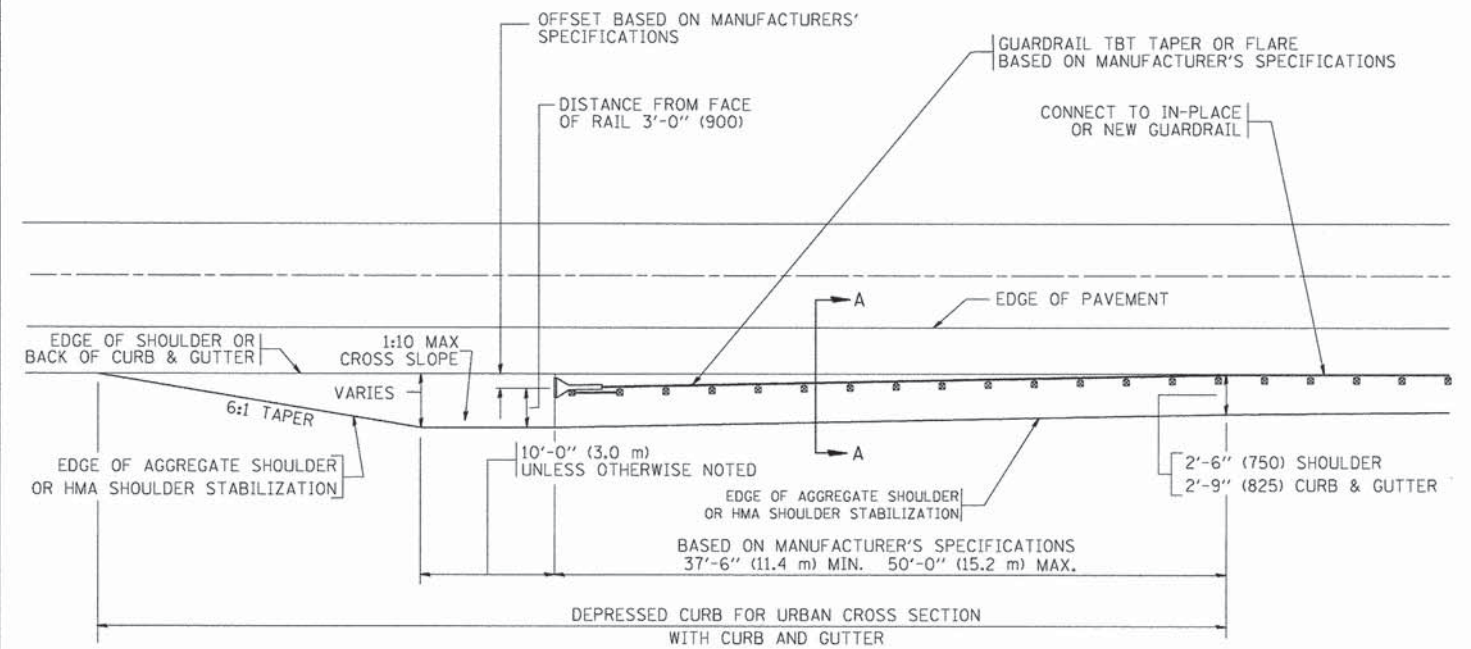
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BD400-05	BD32		50	44
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT			CONTRACT NO. 61A02	



SECTION A-A

- NOTES:
1. THE AGGREGATE SHOULDER, 10" OR HMA SHOULDER, 6" (IF REQUIRED) SHALL EXTEND UNDER THE TRAFFIC BARRIER TERMINAL.
 2. "EXISTING" GUARDRAIL REFERS TO CONNECTING TERMINAL SECTION TO GUARD RAILING PRIOR TO THE MIDWEST GUARDRAIL SYSTEM.
 3. THE CONTRACTOR SHALL VERIFY THE TYPE/HEIGHT OF GUARDRAIL IN-PLACE BEFORE ORDERING THE NEW TERMINAL SECTION. COST INCLUDED WITH THE COST OF THE TERMINAL. THE TERMINAL SECTION HEIGHT TO BE PLACED MUST MATCH THE HEIGHT OF THE IN-PLACE GUARDRAIL.

**DETAILS FOR STEEL PLATE BEAM
GUARD RAIL ADJACENT TO CURB AND GUTTER
[FOR ROADWAY SPEED 35 MPH (60 kmh) TO 45 MPH (70 kmh)]**



**DEPRESSED CURB AND GUTTER AND
SHOULDER TREATMENT AT TBT TY. 1 SPL.**

BASIS OF PAYMENT: HMA SHOULDERS 6 (150) (IF REQUIRED) WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER SQUARE YARD (SQUARE METER) FOR "HOT-MIX ASPHALT SHOULDERS 6" (150 mm)".

STEEL PLATE BEAM GUARD RAIL AND TRAFFIC BARRIER TERMINAL, OF THE TYPE SPECIFIED WILL BE PAID FOR SEPARATELY.

TBT = TRAFFIC BARRIER TERMINAL
ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

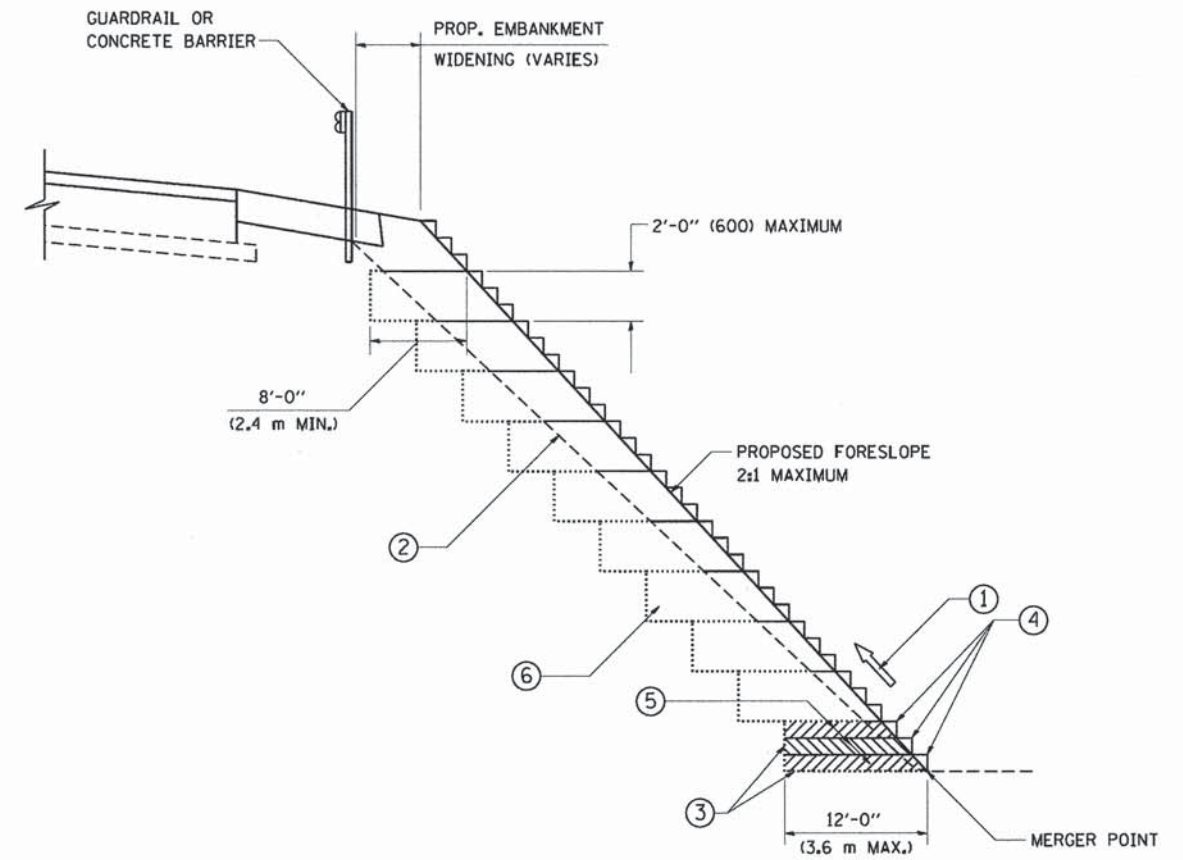
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**DETAILS FOR DEPRESSED CURB & GUTTER AND
SHOULDER TREATMENT AT TBT TY 1 SPL.**

SCALE: NONE SHEET NO. 1 OF 1 SHEETS STA. TO STA.

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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BD600-10 (BD 34)		CONTRACT NO. 61A02		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



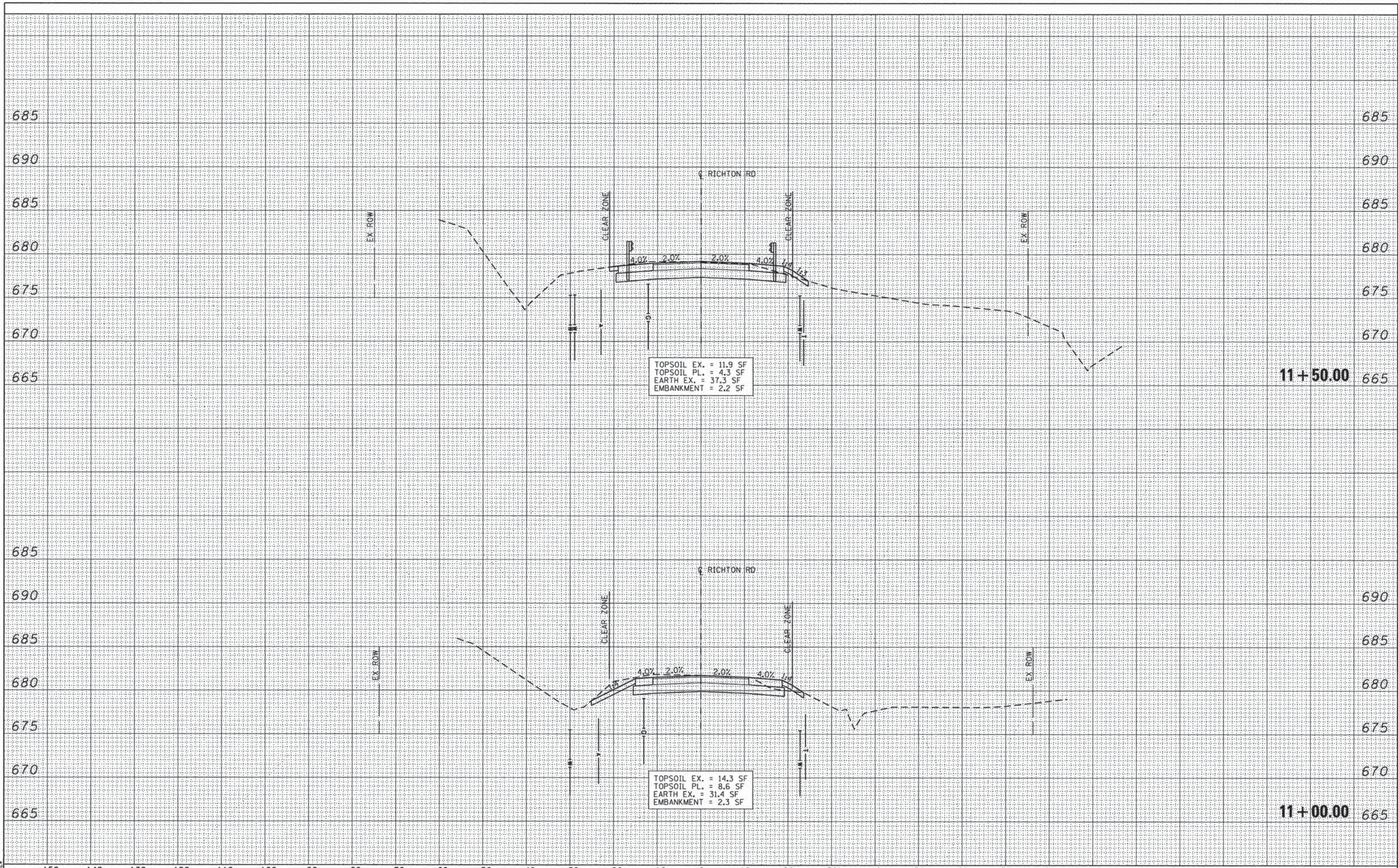
**TYPICAL BENCHING DETAIL
FOR EMBANKMENT**

NOTES:

- ① CONSTRUCT SUCCEEDING BENCH CUTS AND EMBANKMENT PLACEMENT AND COMPACTION FROM BOTTOM TO TOP IN STAIRSTEP FASHION.
- ② EXISTING FORESLOPE PREPARED IN ACCORDANCE WITH ARTICLE 205.03 OF THE STANDARD SPECIFICATIONS.
- ③ BENCH CUT EXISTING SLOPE TYPICAL FOR EACH STEP.
- ④ TRIM TO FINAL SLOPE.
- ⑤ EQUAL 8-INCH (200) LIFTS OF EMBANKMENT COMPACTED IN ACCORDANCE WITH ARTICLE 205.05 OF THE STANDARD SPECIFICATIONS.
- ⑥ EXCAVATION OF BENCH CUTS WITHIN EXISTING EMBANKMENT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC METER OR CUBIC YARD FOR "EARTH EXCAVATION". THIS PRICE WILL INCLUDE ALL LABOR AND MATERIAL, NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- ⑦ SLOPES SHALL BE BENCHED ACCORDING TO THIS DETAIL WHEN THE SLOPE IS STEEPER THAN 4:1 AND THE HEIGHT IS GREATER THAN 5' (1.5 m).

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS)
UNLESS OTHERWISE SHOWN.

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 CHECKED - MPM
 DATE - DECEMBER 9, 2013

REVISED -
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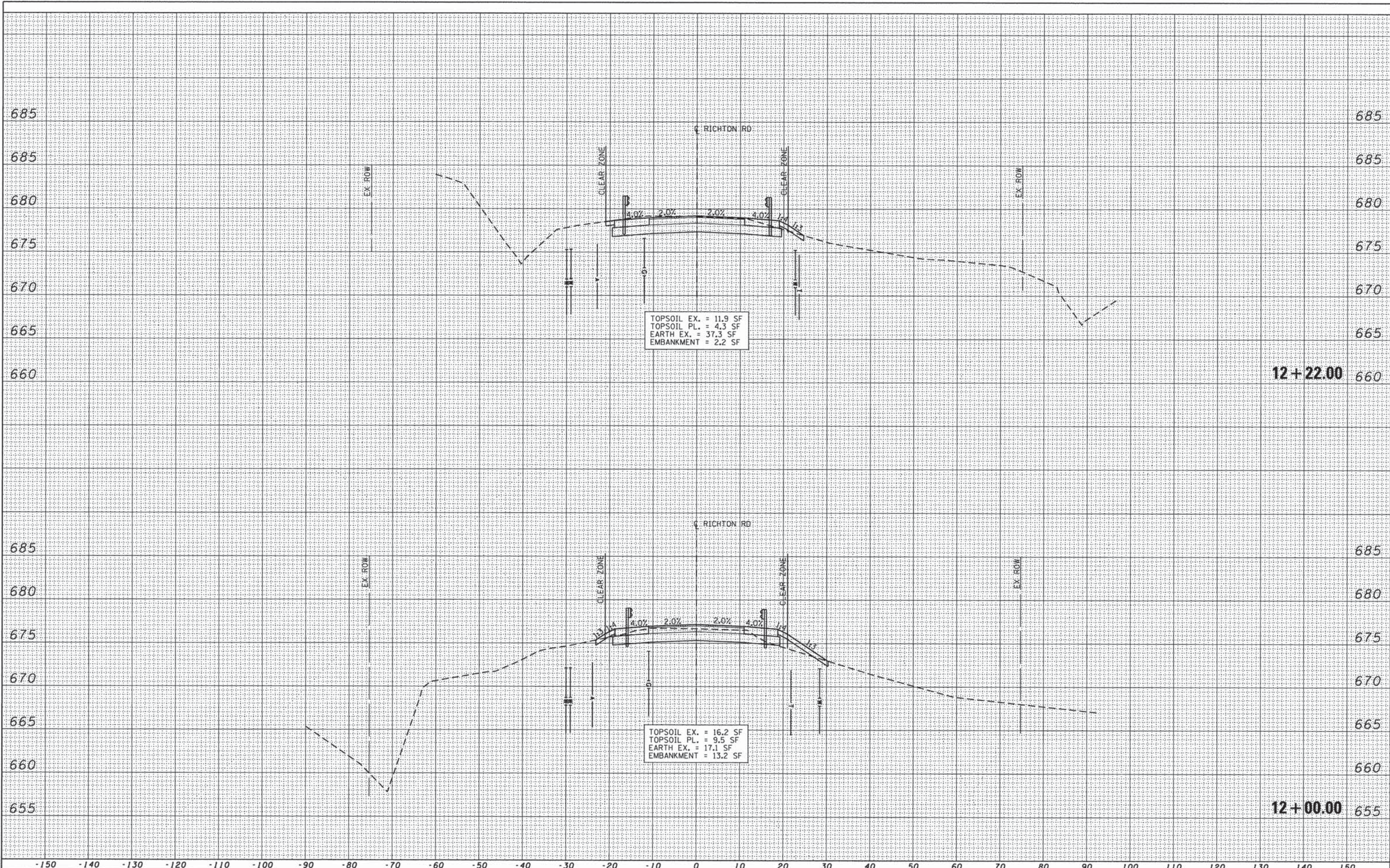
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

PROPOSED CROSS SECTIONS RICHTON ROAD

SCALE: H 1"=10', V 1"=5' SHEET NO. 1 OF 4 SHEETS STA. TO STA.

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	47
CONTRACT NO. 61A02				

ILLINOIS FED. AID PROJECT



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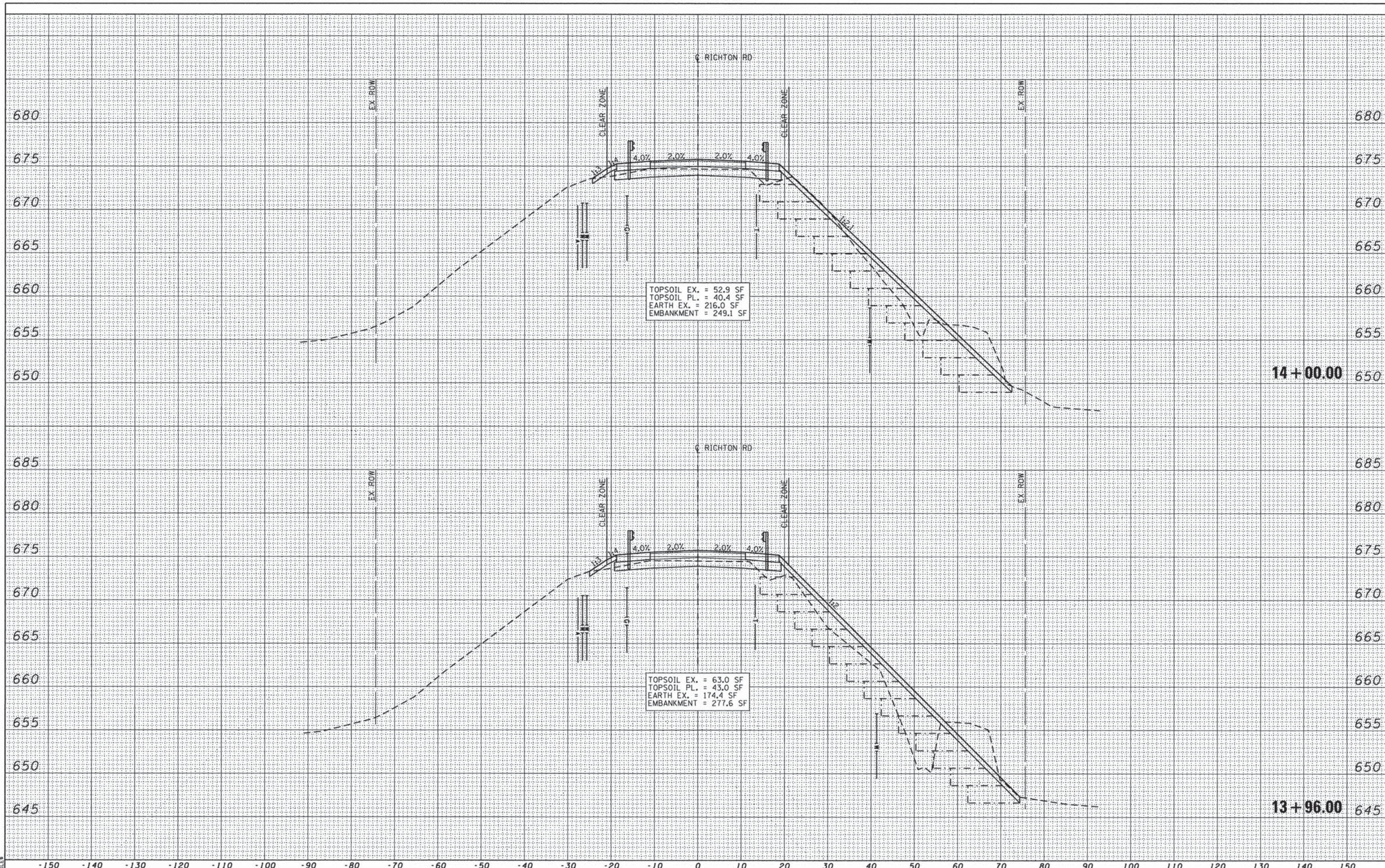
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PROPOSED CROSS SECTIONS RICHTON ROAD

SCALE: H 1"=10', V 1"=5' SHEET NO. 2 OF 4 SHEETS STA. TO STA.

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	48

CONTRACT NO. 61A02
ILLINOIS FED. AID PROJECT



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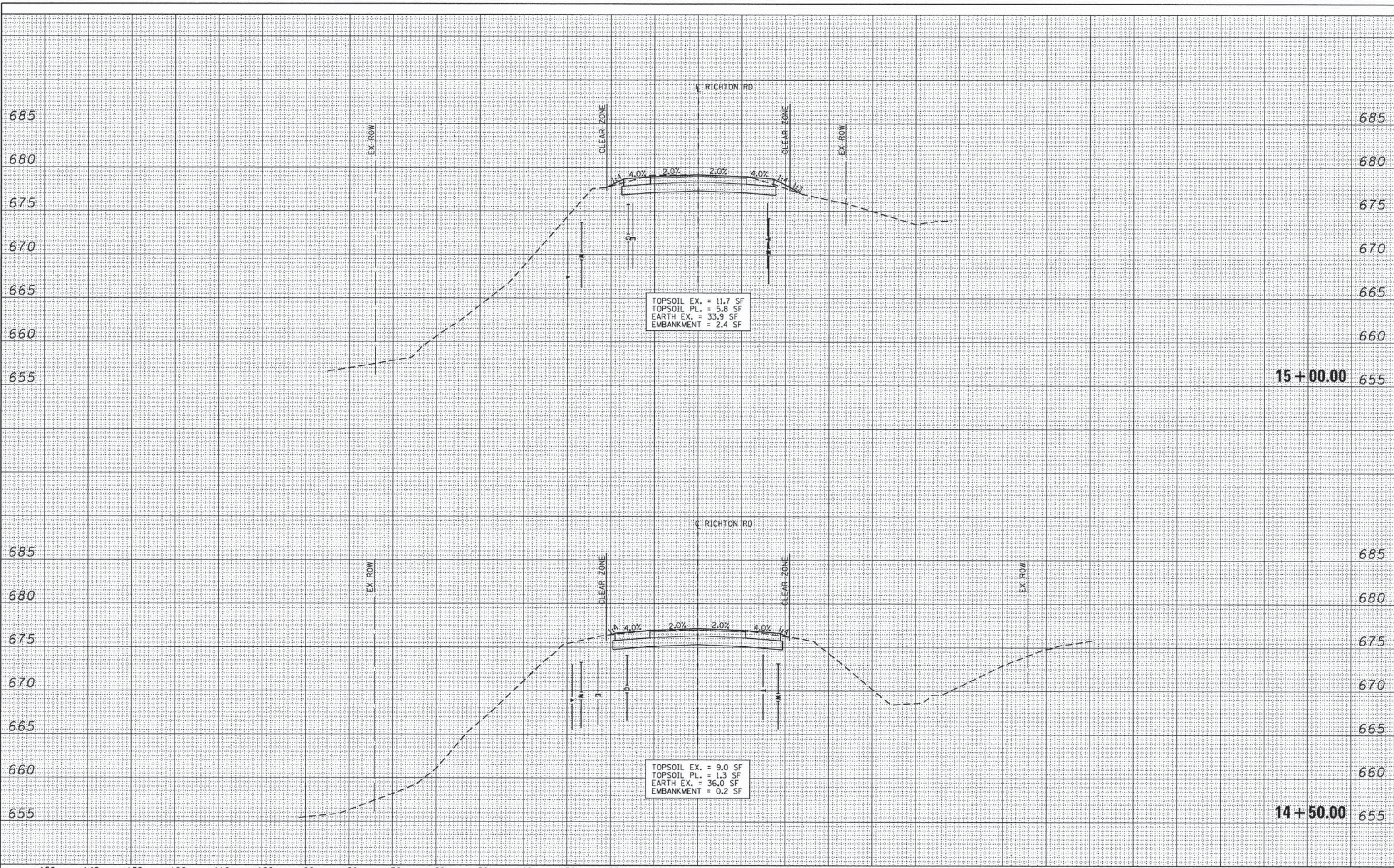
REVISED -
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STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

PROPOSED CROSS SECTIONS RICHTON ROAD
 SCALE: H 1"=10', V 1"=5'
 SHEET NO. 3 OF 4 SHEETS STA. TO STA.

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	49
CONTRACT NO. 61A02			ILLINOIS FED. AID PROJECT	



TOPSOIL EX. = 11.7 SF
 TOPSOIL PL. = 5.8 SF
 EARTH EX. = 33.9 SF
 EMBANKMENT = 2.4 SF

TOPSOIL EX. = 9.0 SF
 TOPSOIL PL. = 1.3 SF
 EARTH EX. = 36.0 SF
 EMBANKMENT = 0.2 SF

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STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

PROPOSED CROSS SECTIONS RICHTON ROAD

SCALE: H 1"=10', V 1"=5' SHEET NO. 4 OF 4 SHEETS STA. TO STA.

T.R.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
428	11-02118-01-BR	WILL	50	50
			CONTRACT NO. 61A02	
ILLINOIS FED. AID PROJECT				