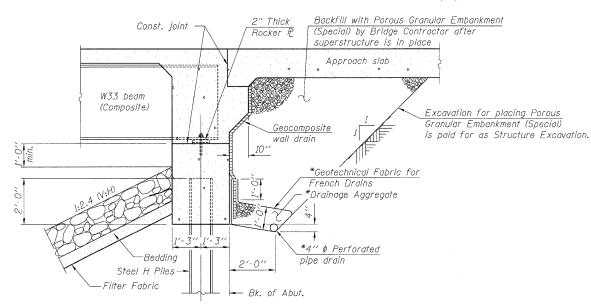
- 2. General Data
- 3. Stage Construction Details
- 4. Top of Slab Elevations 1
- 5. Top of Slab Elevations 2
- 6. North Approach Top of Slab Elevations
- 7. South Approach Top of Slab Elevations
- 8. Superstructure Plan
- 9. Superstructure Details
- 10. Integral Abutment Diaphragm Details
- 11. Bridge Approach Slab Details
- 12. Bridge Approach Slab Details
- 13. Framing Plan
- 14. Structural Steel Details
- 15. North Abutment
- 16. South Abutment
- 17. Bar Splicer Assembly and Mechanical Splicer Details
- 18. HP Pile Details
- 19. Temporary Concrete Barrier For Stage Construction
- 20. Boring Logs 1
- 21. Boring Logs 2

GENERAL NOTES

- 1. Fasteners shall be AASHTO M164 Type 1, mechanically galvanized botts in painted areas and M164 Type 3 in unpainted areas. Botts 3_4 in. dia., holes $^{13}_6$ in. dia., unless otherwise noted.
- 2. Calculated weight of structural steel = 59,430 pounds.
- 3. All structural steel shall be AASHTO M 270 Grade 50W. All structural steel shall be cleaned as specified in the Special Provision for "Surface Preparation and Painting Requirements for Weathering
- 4. No field welding is permitted except as specified in the contract documents.
- 5. Reinforcement bars shall conform to the requirements of ASTM A706 Gr 60. See Special Provisions.
- 6. Reinforcement bars designated (E) shall be epoxy coated.
- 7. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- 8. Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete cap plus 3 in. Painted areas shall be primed in the shop with a Department approved zinc rich primer. Field painting will not be required.
- 9. Slipforming of the parapets is not allowed.
- 10. The Contractor is advised that the existing PPC Deck Beams are in a deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the beams when developing construction procedures for removal and replacement of the
- 11. If the Contractor's procedures for existing deck beam removal or construction of the new superstructure involves placement of heavy equipment on the existing deck beams, a detailed procedure shall be submitted to the Engineer for approval. The procedure shall include calculations, sealed by an Illinois Licensed Structural Engineer, verifying the structural adequacy of the deck beams for the proposed loads. Cost included with Removal of Existing Structures.

TOTAL BILL OF MATERIAL

	1017	SUDER	CUE	Toru
ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment, Special	Cu. Yd.		119	119
Stone Riprap Class A4	Sq. Yd.		910	910
Filter Fabric	Sq. Yd.		910	910
Removal of Existing Structures	Each		1	1
Structure Excavation	Cu. Yd.		141	141
Floor Drains	Each	8		8
Concrete Structures	Cu. Yd.		53.3	53.3
Concrete Superstructure	Cu. Yd.	214.1		214.1
Bridge Deck Grooving	Sq. Yd.	436		436
Concrete Encasement	Cu. Yd.		4.0	4.0
Protective Coat	Sq. Yd.	541		541
Furnishing and Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	990		990
Reinforcement Bars, Epoxy Coated	Pound	45,050	12,630	57,680
Bar Splicers	Each	453	126	579
Furnishing Steel Piles HP 12x53	Foot		236	236
Driving Piles	Foot		236	236
Test Pile Steel HP 12x53	Each		1	1
Name Plates	Each	1		1
Anchor Bolts, 1"	Each	24		24
Geocomposite Wall Drain	Sq. Yd.		46	46
Pipe Underdrains for Structures 4"	Foot		118	118
Temporary Soil Retention System	Sg. Ft.		197	197
Asbestos Bearing Pad Removal	Each	16		16



SECTION THRU INTEGRAL ABUTMENT

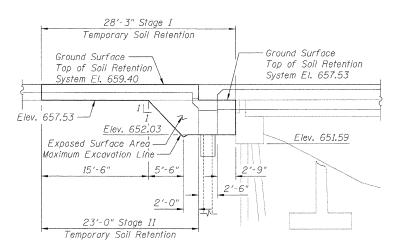
All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101.)

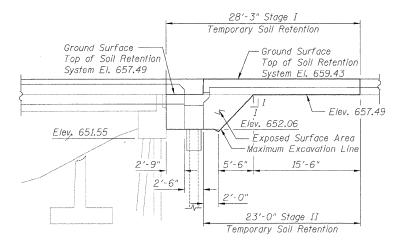
*Included in the cost of Pipe Underdrains for Structures.

ROKA Chicago, IL 60613 engineering DESIGNED LAS CHECKED JLA DRAWN SAW CHECKED LAS



See Std. 515001





TEMPORARY SOIL RETENTION SYSTEM

A cantilevered sheet piling system does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.

WATERWAY INFORMATION

Drainage Ared	nage Area = 11.25 Sq. Mi.			Exist. Low Grade Elev. = 656.63 © Sta. 1719+00.00 Prop. Low Grade Elev. = 656.63 © Sta. 1719+00.00					
Flood	Freq.	Q	Opening Sq. Ft.		Nat.	Head - Ft.		Headwater El.	
r 100a	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
	10	583	243	243	652.4	0.00	0.00	652.4	652.4
Design	50	893	330	330	654.5	0.00	0.00	654.5	654.5
Base	100	1,025	349	364	655.2	0.1	0.00	655.3	655.2
Overtopping	N/A								
Max. calc.	500	1,339	349	394	656.6	0.2	0.1	656.8	656.7

DESIGN SCOUR ELEVATION TABLE

Location	N. Abut	S. Abut	
Design Scour Elevation	652.0	652.1	

GENERAL DATA STRUCTURE NO. 027-0099

CHEET NO. O		F.A.P. RTF.	SEC ⁻	TION	COUNTY	TOTAL	SHEET NO.
	SHEET NO. 2	796	(106)	3R-3	FORD	48	13
	OF 21 SHEETS		SN 027-0	CONTRACT NO. 66916			
		FED. RO	DAD DIST. NO	ILLINOIS FED. A	ID PROJECT		