

Abbreviated Structure Geotechnical Report

Original Report Date:	12-23-2016	Proposed SN:	027-2554	Route:	FAP 326 (IL 47)
Revised Date: N/A		Existing SN:	027-2524	Section:	126C-1
Geotechnical Enginee	r: Michael Shor	County:	Ford		
Structural Engineer:	David Alexande	, IDOT District 3		Contract:	66D34

Indicate the proposed structure type, substructure types, and foundation locations (attach plan and elevation drawing): The proposed structure is a double barrel 8 ft span by 5 ft rise precast concrete box culvert with no skew. Grated end sections are proposed. A preliminary plan and profile sheet is attached.

Discuss the existing boring data, existing plans foundation information, new subsurface exploration and need for any additional exploration to be provided with SGR Technical Memo (attach all data and subsurface profile plot): The existing structure is a double barrel 8 ft span by 5 ft rise box culvert with no skew. Two soil borings were taken in September of 2015.

Provide the location and maximum height of any new soil fill or magnitude of footing bearing pressure. Estimate the amount and time of the expected settlement. Indicate if further testing, analysis, and/or ground improvement/treatment is necessary: The profile of the roadway is not expected to change, therefore there will not be any significant additional load applied to the soils. A site visit indicated no signs of settlement problems with the existing structure. No further settlement analysis is required.

Identify any new cuts or fill slope angles and heights. Estimate the factor of safety against slope failure. Indicate if further testing, analysis or ground improvement/treatment is necessary: The proposed side slopes are not expected to change significantly. The proposed side slopes are 1:4 or flatter and are a maximum of 7 feet high at the culvert ends between Sta. 198+00 to Sta. 199+00 LT/RT. A site visit indicated no slope stability problems with the existing structure. No further slope stability analysis is required.

Indicate at each substructure, the 100-year and 200-year total scour depths in the Hydraulics report, the nongranular scour depth reduction, the proposed ground surface, and the recommended foundation design scour elevations: Not applicable to closed bottom box culverts per ABD Memo 14.2.

Determining the seismic soil site class, the seismic performance zone, the 0.2 and 1.0 second design spectral accelerations and indicate if that the soils are liquefiable: Not applicable to culverts.

Confirm feasibility of the proposed foundation or wall type and provide design parameters. Attach a pile design table indicating feasible pile types, various nominal required bearings, factored resistances available and corresponding estimated lengths at locations where piles will be used. Provide factored bearing resistance and unit sliding resistance at various elevations and confirm no ground improvement/treatment is necessary where spread footings are proposed. Estimated top of rock elevations as well as preliminary factored unit side and tip resistance values shall be indicated when drilled shafts are proposed: A precast concrete box culvert is acceptable. The soils under the proposed box culvert will not require removal and replacement to support the proposed structure. The only aggregate needed under the precast concrete box culvert is the 6 inches required by Article 540.05 of the Standard Specifications.

Calculate the estimated water surface elevation and determine the need for cofferdams (type 1 or 2), and seal coat: The structure can be constructed using conventional methods of water diversion determined by the contractor.

Assess the need for sheeting or soil retention or temporary construction slope and provide recommendation for other construction concerns: If stage construction is required, temporary sheet piling is not feasible because the soils with a strength exceeding 4.5 tsf are above the depth of sheet pile embedment. A temporary soil retention system will be required.



SOIL BORING LOG

Illinois Department of Transportation

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Date 9/14/15

ROUTE FAP 326 (IL 47)	DE	SCR	IPTION	I	IL 47 o	ver a Ditch, 1.56 miles North of IL 9	OGG	ED BY	Larry	Myers	
SECTION 126 C-1		_ เ			SW 1/	4, SEC. 35, TWP. 24N, RNG. 7E, 3 rd PM , de 40.486838, Longitude -88.380662					
COUNTY Ford D	RILLING					low Stem Auger HAMMER TYPE	(CME Automatic			
STRUCT. NO. 027-2524 (Exist. Station 198+61.33 BORING NO. 01 (N.E. Quad.) Station 198+93		D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev.ftStream Bed Elev.756.62Groundwater Elev.:First Encounter741.2ft<▼	D E P T H	B L O W S	U C S Qu	M O I S T	
Offset 16.0 ft Rt. Ground Surface Elev. 763.68		(ft)	(/6")	(tsf)	(%)	Upon Completion 743.7 ft $\overline{\nabla}$		(/6'')	(tsf)	(%)	
Bituminous Shoulder, Black Silty Clay Loam Fill						Hard Gray Silty Clay Loam Till (continued)		5 8 10	4.8 S	17	
Hard to Stiff Black Silty Clay Loam	761.18 1		5	4.5	22		⊻	4	4.8	14	
		 	7	P			-25	7	S		
			2 3 2	2.0 P	29		_	5 4 5	4.3 S	14	
Stiff Gray & Brown Silty Clay	756.68		2 2 2 2	1.8 P	25			4 5 7	4.4 S	13	
Stiff Gray & Brown Silty Clay Till with Gravel Pieces & Minor Silt / Sand / Gravel Seams	754.18	-10	1	1.0	21		-30	3	4.0	14	
			1	Р				6	S		
Hard Grav Silty Clay Till	749.18		5 7	1.0 P	17						
Hard Gray Silty Clay Till		-15 	5 8 11	6.1 S	17	727.18	-35	4 6 9	4.8 S	16	
			7			End of Boring			_		
Hard Gray Silty Clay Loam Till	744.18	 7 -20	10 15	7.4 S	13						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

SOIL BORING LOG

Illinois Department of Transportation

Division of Highways

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Date 9/14/15

	ROUTE	FAP 326 (IL 47)				N	IL 47 o	47 over a Ditch, 1.56 miles North of IL 9 LOGGED BY Larry Mye							
	SECTION	126 C-1		_ I			SW 1/	4, SEC. 35, TWP. 24N,	RNG. 7E, 3 rd	PM,					
								de 40.486654, Longitu							
	COUNTY Ford DRILLING METHO										CME Automatic				
	STRUCT NO	007 0504 (Eviot		D	в	U	м	Surface Weter Flow			D	в	U	м	
	STRUCT. NU.	027-2524 (Exist)	E	L	c	0	Surface Water Elev.	750.00	_ IL	E	L	c	0	
	Station	198+61.33		P	ō	s	Ĩ	Stream Bed Elev.	756.62	_ π	P	ō	S	Ĭ	
		02 (S.W. Quad.)	Т	Ŵ		S	Groundwater Elev.:			T	Ŵ		S	
	Station	198+27		н	S	Qu	Т	First Encounter	741.7	ft 🔻	н	S	Qu	Т	
	Offset	198+27 16.0 ft Lt.						Upon Completion	741.7	ft⊽					
	Ground Surf	face Elev. 763.66	3 ft	(ft)	(/6")	(tsf)	(%)	After Hrs.		ft	(ft)	(/6")	(tsf)	(%)	
		minous Shoulder,						Hard Gray Silty Clay	Fill			5			
	Black Silty Cla	ay Loam Fill						(continued)				7	6.8	17	
												12	S		
										741.66	▼				
			761.16					Hard Gray Silty Clay L	_oam Till			_			
	Stiff Black Silf	ty Clay Loam Fill			3	<u> </u>						5			
					3	1.5	21					8	4.7	13	
					3	Р						9	S		
				_	_										
				5	2						-25	5			
					4	2.0	30					5	4.6	12	
			757.16		5	P 2.0						7	0 S	12	
	Stiff Brown &	tiff Brown & Gray Silty Clay	757.10		-	-						-			
	Loam / Silty C	Clay with Silt, Sand,			_										
	Gravel Pocke	ts / Seams			2							6			
					3	1.8	23					5	4.0	15	
					3	P						5	S		
				-10							-30				
					3							4			
					4	2.0	19					5	4.0	16	
					6	P						6	S		
					_										
			751.16												
	Hard Brown S	Silty Clay Loam Till			3	47	10								
				_	5	4.7 S	19								
0/15			740.46			3									
10/2	Hard Gray Sil		749.16	-15	_						-35				
5DT	Halu Glay Sil	ty Clay Till		-15	6						-35	6			
OT.0					8	7.1	16					6	4.4	15	
SOIL BORING 027-2524.GPJ IL_DOT.GDT 10/20/15					12	S				727.16		7	S		
L L L						-		End of Boring					-		
24.G					1										
21-25					6							1			
G 02					7	6.8	22								
RING					10	S									
L BO				_							_				
SOIL				-20							-40				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)