



Illinois Department of Transportation

Abbreviated Structure Geotechnical Report

Original Report Date: 1/13/2017 Proposed SN: 027-0104 Route: FAS 1522 (US 45)
Revised Date: 4/10/2017 Existing SN: 027-0012 Section: 31-X-BR
Geotechnical Engineer: Michael Short, IDOT District 3 County: Ford
Structural Engineer: Lori Sommer, Milhouse Engineering & Const. Contract: 66C84

Indicate the proposed structure type, substructure types, and foundation locations (attach plan and elevation drawing): The proposed structure is a three span bridge with a reinforced concrete deck. Integral abutments are preferred. The preliminary TS&L is attached. The proposed structure will be shifted approximately 8 feet north compared to the existing structure to improve the alignment with the channel.

Discuss the existing boring data, existing plans foundation information, new subsurface exploration and need for any additional exploration to be provided with SGR Technical Memo (attach all data and subsurface profile plot): Soil borings from June 2014 are attached. The existing structure is founded on 18" square precast concrete piles at the abutments and spread footings at the piers.

Provide the location and maximum height of any new soil fill or magnitude of footing bearing pressure. Estimate the amount and time of the expected settlement. Indicate if further testing, analysis, and/or ground improvement/treatment is necessary: The proposed profile of the roadway is not expected to change and a site visit found no signs of settlement at the existing structure. Therefore, the soil is not anticipated to experience any additional loading that would result in settlement. No further analysis for settlement is warranted.

Identify any new cuts or fill slope angles and heights. Estimate the factor of safety against slope failure. Indicate if further testing, analysis or ground improvement/treatment is necessary: Because the proposed side slopes are less than 15 feet high and are a maximum of 1:3 (V:H) and because the existing slopes exhibit no slope stability issues, a side slope analysis was not performed. However, an end slope stability analysis was performed. The IDOT Geotechnical Manual requires a Factor of Safety of 1.5 because the analysis is based on Rimac samples. Based on the analysis of a 1:2 (V:H) end slope, the factor of safety against slope failure is 4.6. A printout of the slope configuration used in the analysis is attached. No further analysis for slope stability is warranted.

Indicate at each substructure, the 100-year and 200-year total scour depths in the Hydraulics report, the non-granular scour depth reduction, the proposed ground surface, and the recommended foundation design scour elevations: The scour results from the hydraulic report indicate 6.0 feet of scour for the 100 year event and 10.7 feet of scour for the 200 year event. The soil borings indicate cohesive soils with unconfined compressive strength greater than 4.0 tsf. Therefore, a 50% scour reduction will be used. Based on this reduction, the scour elevation table will be as follows:

Design Scour Elevations (ft.)					Item 113
Event/Limit State	South Abutment	Pier 1	Pier 2	North Abutment	
Q100	731.50	709.40	709.40	730.50	8
Q200	731.50	707.05	707.05	730.50	
Design	731.50	709.40	709.40	730.50	
Check	731.50	707.05	707.05	730.50	

Determining the seismic soil site class, the seismic performance zone, the 0.2 and 1.0 second design spectral accelerations and indicate if that the soils are liquefiable: The seismic soil site class is D. The seismic performance zone is 1. The 0.2 second design spectral acceleration is 0.204 g. The 1.0 second design spectral acceleration is 0.123 g. Liquefaction is not a concern at this location.

Confirm feasibility of the proposed foundation or wall type and provide design parameters. Attach a pile design table indicating feasible pile types, various nominal required bearings, factored resistances available and corresponding estimated lengths at locations where piles will be used. Provide factored bearing resistance and unit sliding resistance at various elevations and confirm no ground improvement/treatment is necessary where spread footings are proposed. Estimated top of rock elevations as well as preliminary factored unit side and tip resistance values shall be indicated when drilled shafts are proposed:

Preliminary substructure loadings are attached.

The BB&S "Integral Abutment Feasibility Analysis" spreadsheet was used to determine the pile sizes available for use at the abutments, based on the current BB&S policy. The current policy would allow for the use of following piling sizes and shapes: HP10X42, HP12X53, HP10X57, HP12X74, HP14X73, HP12X84, HP14X89, HP14X102, and HP14X117. A draft version of the proposed revisions to the BB&S integral abutment policy was also used. The proposed draft policy expands this list by including MS12X0.179, MS12X0.25, MS14X0.25, MS14X0.312, MS 16X0.312 and MS 16X0.375. A copy of the results is attached. It is recommended to use the draft policy as it is expected that the policy will be adopted in the near future.

One test pile is recommended at each of the substructure units. The District prefers that the same pile type and size be used throughout the structure. Pile design tables were created using the BB&S "Pile Capacity and Length Estimates" spreadsheet. These tables are attached. At Pier 1, the tables were created assuming a 12 inch diameter pre core to elevation 696.0 prior to driving metal shell piles. This is necessary to prevent damage to metal shell piles during driving through the dense granular soils. By using a 12 inch diameter pre core with a 14 or 16 inch diameter metal shell pile, the risk of damage to the pile is minimized while still developing skin friction. This is necessary to ensure a minimum length of pile is installed prior to reaching the bearing capacity of the pile or damaging the pile.

Metal shell piles are preferred by the District because their estimated lengths are shorter than H piles. Additionally, the estimated lengths of metal shell piles are expected to be more predictable than the estimated length of H piles because the H piles will not encounter bedrock.

If metal shell piles are used, pile shoes are recommended to penetrate the dense granular soils. If H piles are used, pile shoes are not needed.

No underground Coal mines were identified near the project site according to the Illinois State Geological Survey website.

Calculate the estimated water surface elevation and determine the need for cofferdams (type 1 or 2), and seal coat: The estimated water surface elevation (EWSE) is 715.9. A Type 1 cofferdam is necessary at each pier. A seal coat is not necessary.

Assess the need for sheeting or soil retention or temporary construction slope and provide recommendation for other construction concerns: Sheet, soil retention, or temporary construction slopes will not be necessary for this structure because the structure is expected to be constructed using a detour. If stage construction becomes necessary, temporary sheet piling is feasible because soils with strengths exceeding 4.5 tsf are below the required embedment depth.

Benchmark:

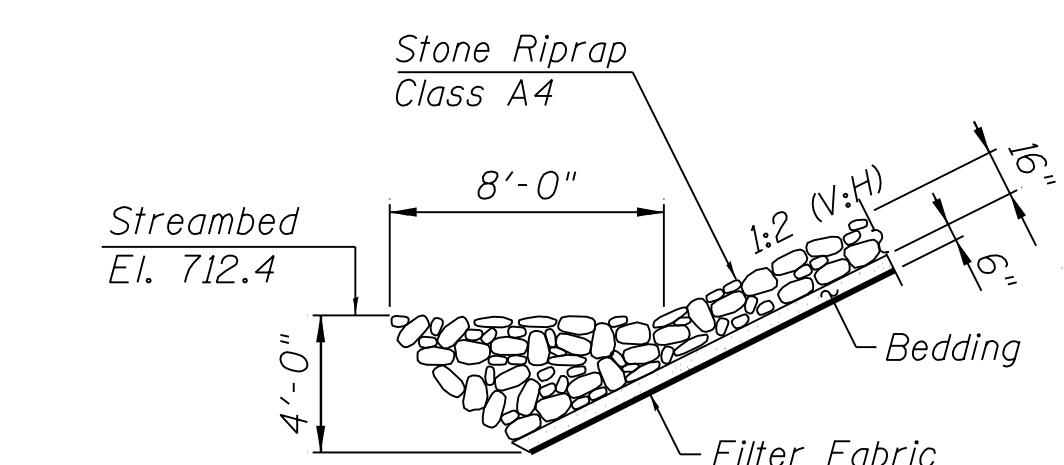
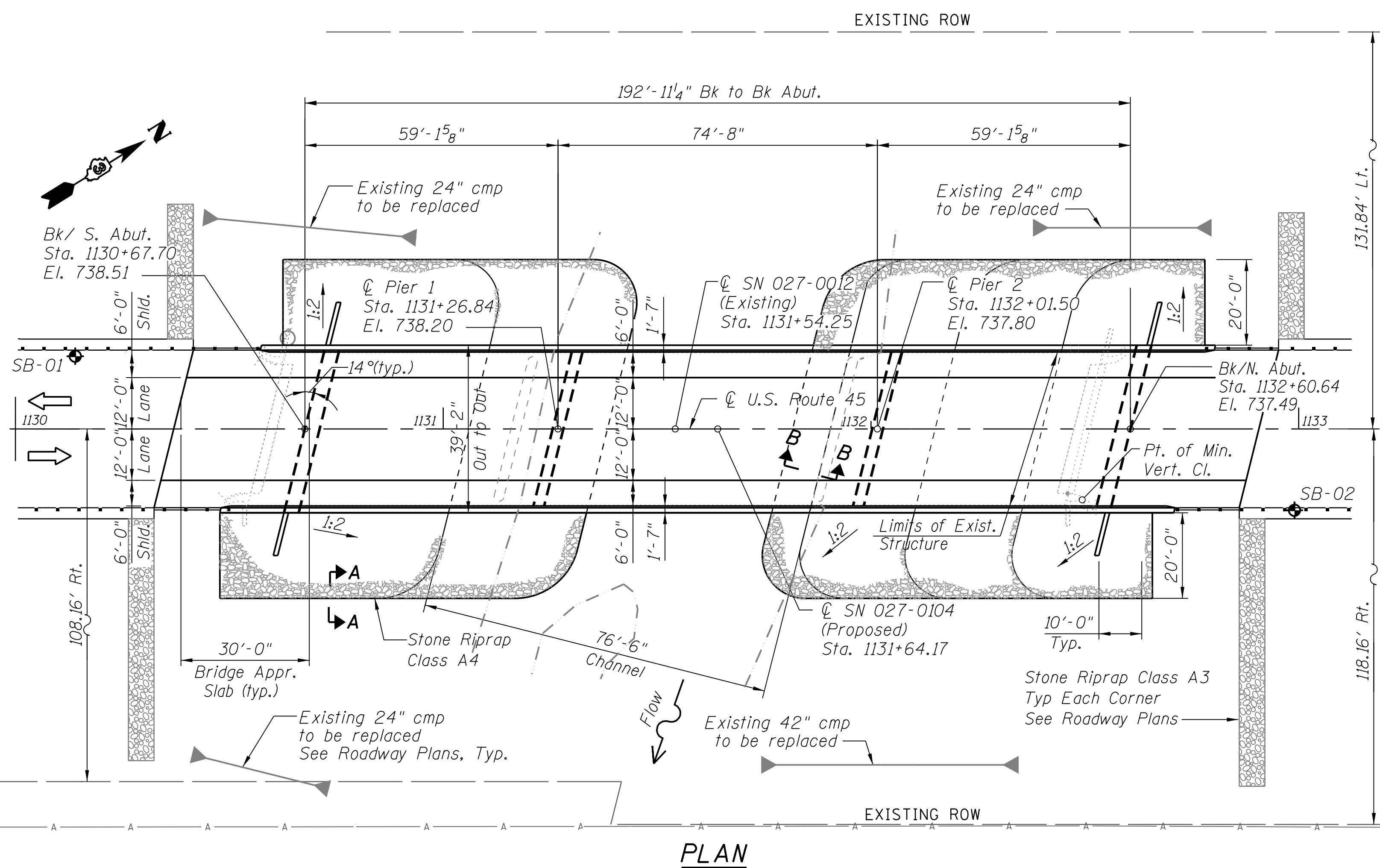
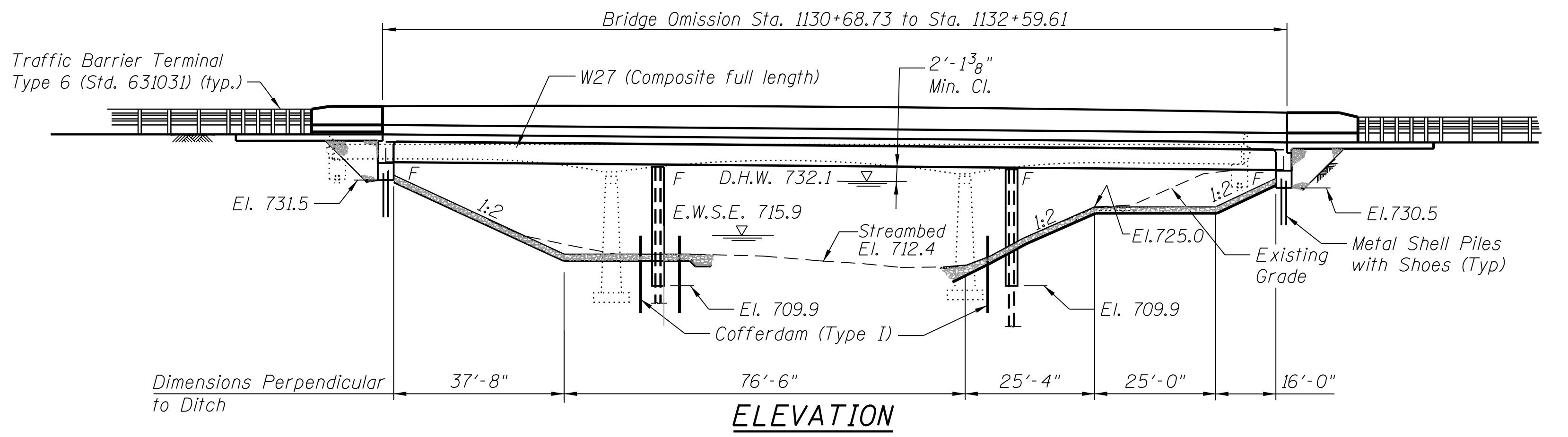
Bm # 41A U.S.G.S. Benchmark disc on top of SW wingwall SN 027-0012.
Sta. 1130+63.65. 21.22 Lt. Elev. 738.97

Existing Structure:

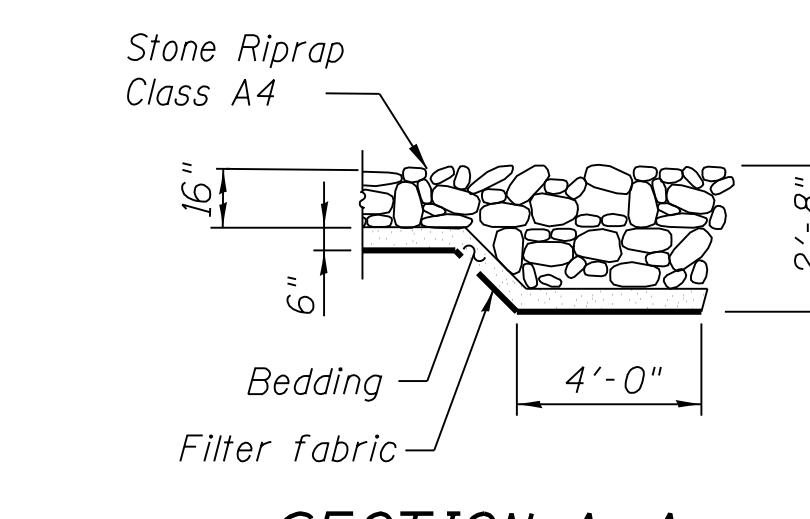
SN 027-0012 was originally constructed in 1953 as SBI Rt 25, Section 31-X-B. The existing structure was patched and the deck expansion devices replaced in 1994 as Section 31 I & 32W & RS-1. The bridge was retrofitted with a curb mounted steel guardrail. The existing superstructure is a three span bridge (57'-5", 74'-8", 57'-5") constructed of reinforced T-beams and a 7" deck plus a 1 $\frac{1}{2}$ " bituminous overlay. The bridge is 193'-10" back to back of abutments and is 35'-8" wide out to out of deck. It is skewed 14° left forward. The abutments are stub abutments supported on piles. The piers are solid shafts supported on spread footings.

Traffic shall be detoured during construction.

No Salvage



SECTION B-B



SECTION A-A

LOADING HL - 93

Allow 50#/sq. ft. for future wearing surface

HIGHWAY CLASSIFICATION

Route: FAS 1522 (US Route 45)
Functional Class: Major Collector
ADT: 3450 (2015), 4284 (2039)
DHV = 1000
ADTT = 110 (2015), 137 (2039)
Design Speed: 55 mph
Posted Speed: 55 mph
Directional Distribution: 50/50
Two Way Traffic

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration @ 1.0 sec (SDI) = .123g
Design Spectral Acceleration @ 0.2 sec (SDS) = .204g
Soil Site Class = D

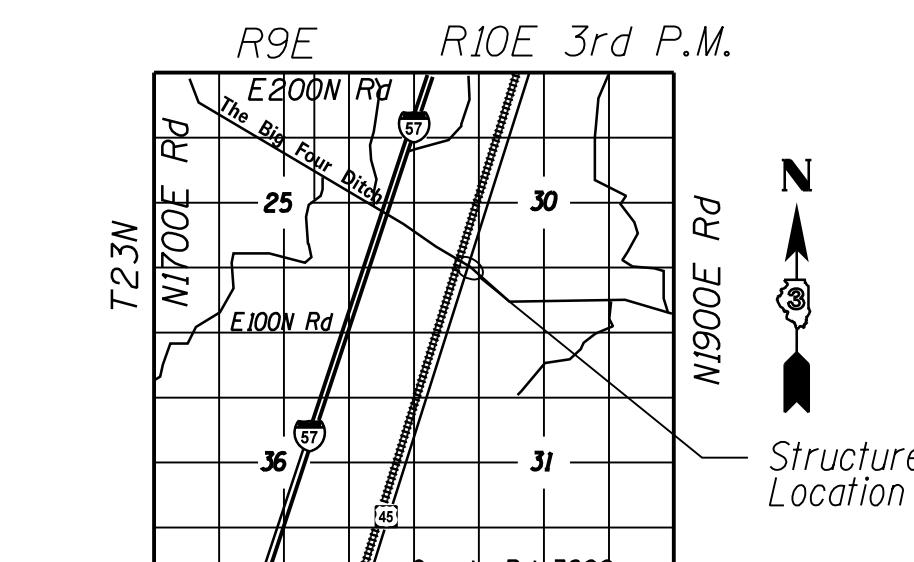
DESIGN SPECIFICATIONS

AASHTO LRFD Bridge Design
Specifications with 2015 & 2016 Interims

DESIGN STRESSES

FIELD UNITS

f'_c = 3,500 psi
 f'_c = 4,000 psi (Superstructure Concrete)
 f_y = 60,000 psi (Reinforcement)
 F_y = 50,000 psi (M270 Grade 50W)



LOCATION SKETCH

GENERAL PLAN

U.S. ROUTE 45 OVER BIG FOUR DITCH

FAS 1522 (US RTE 45)

SECTION 31-X-BR

FORD COUNTY

STA. 1131+64.17

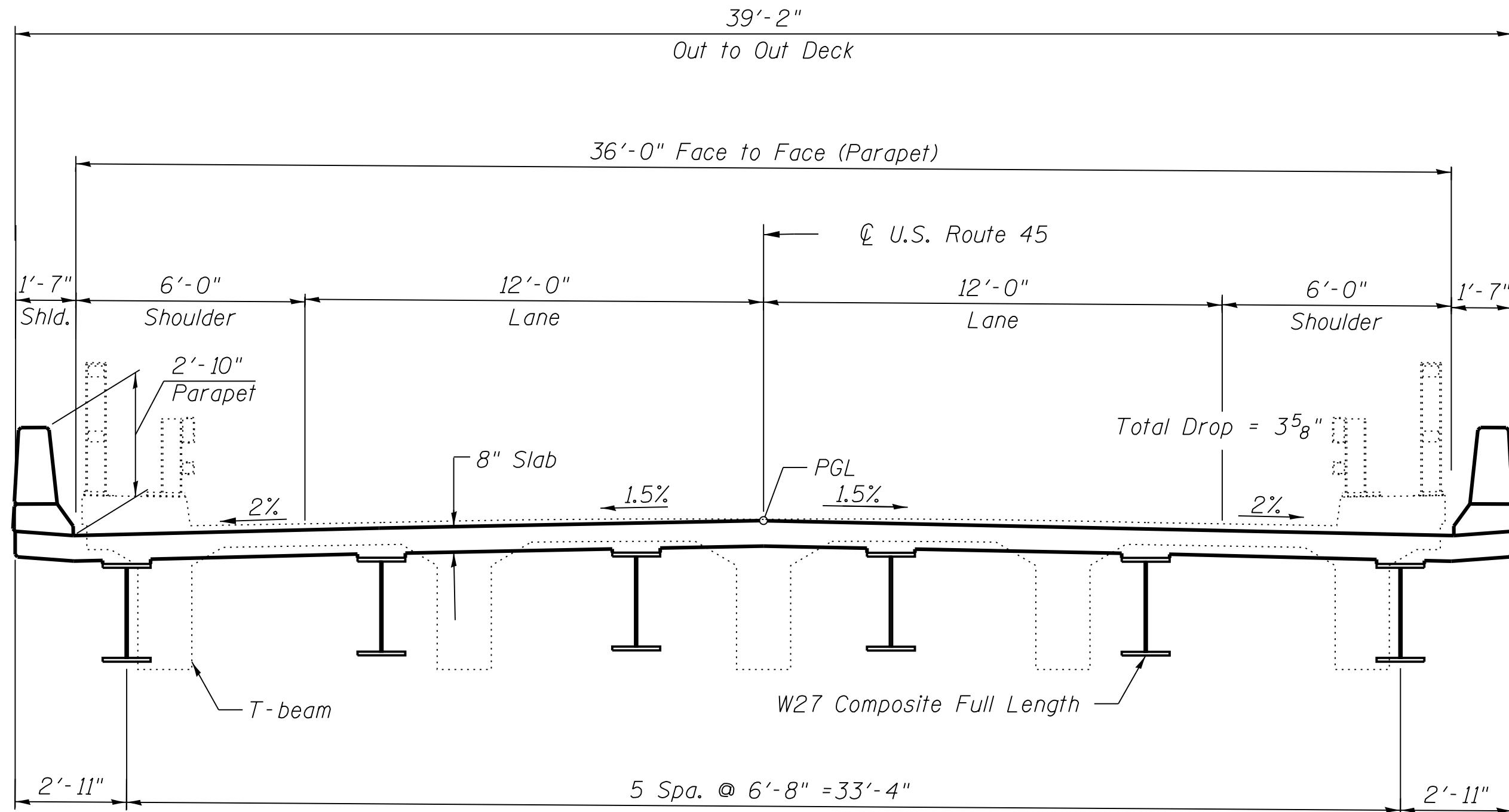
STRUCTURE NO. 027-0104

GENERAL PLAN

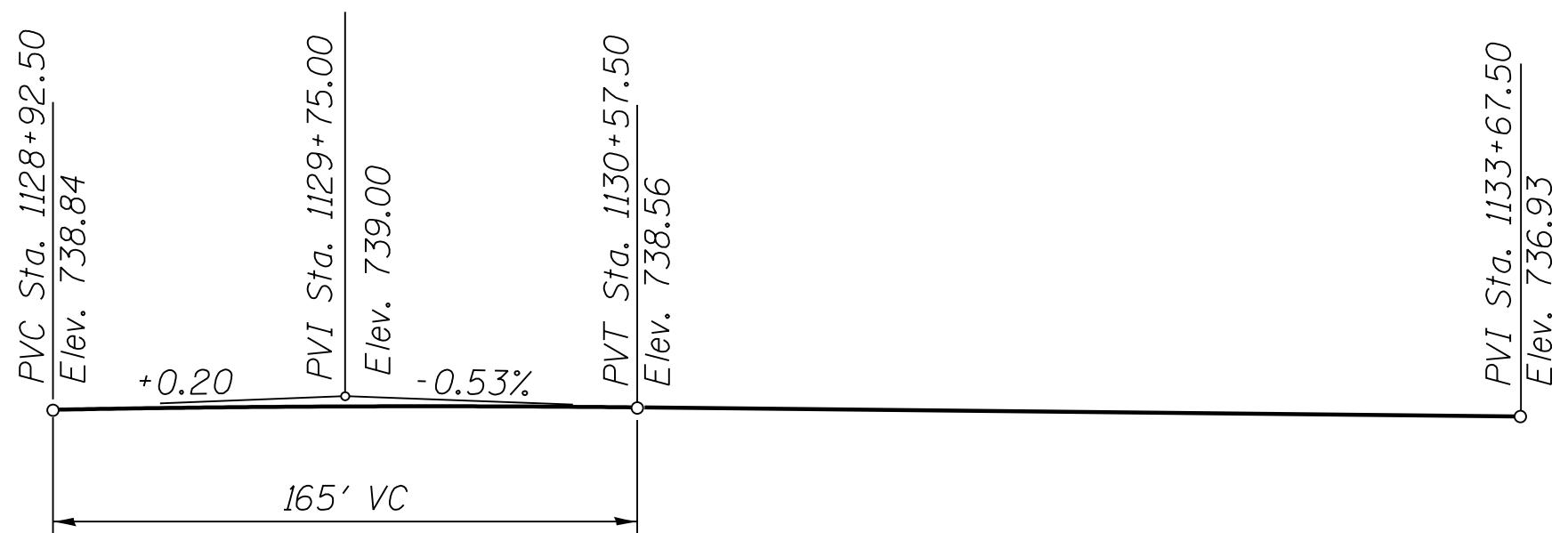
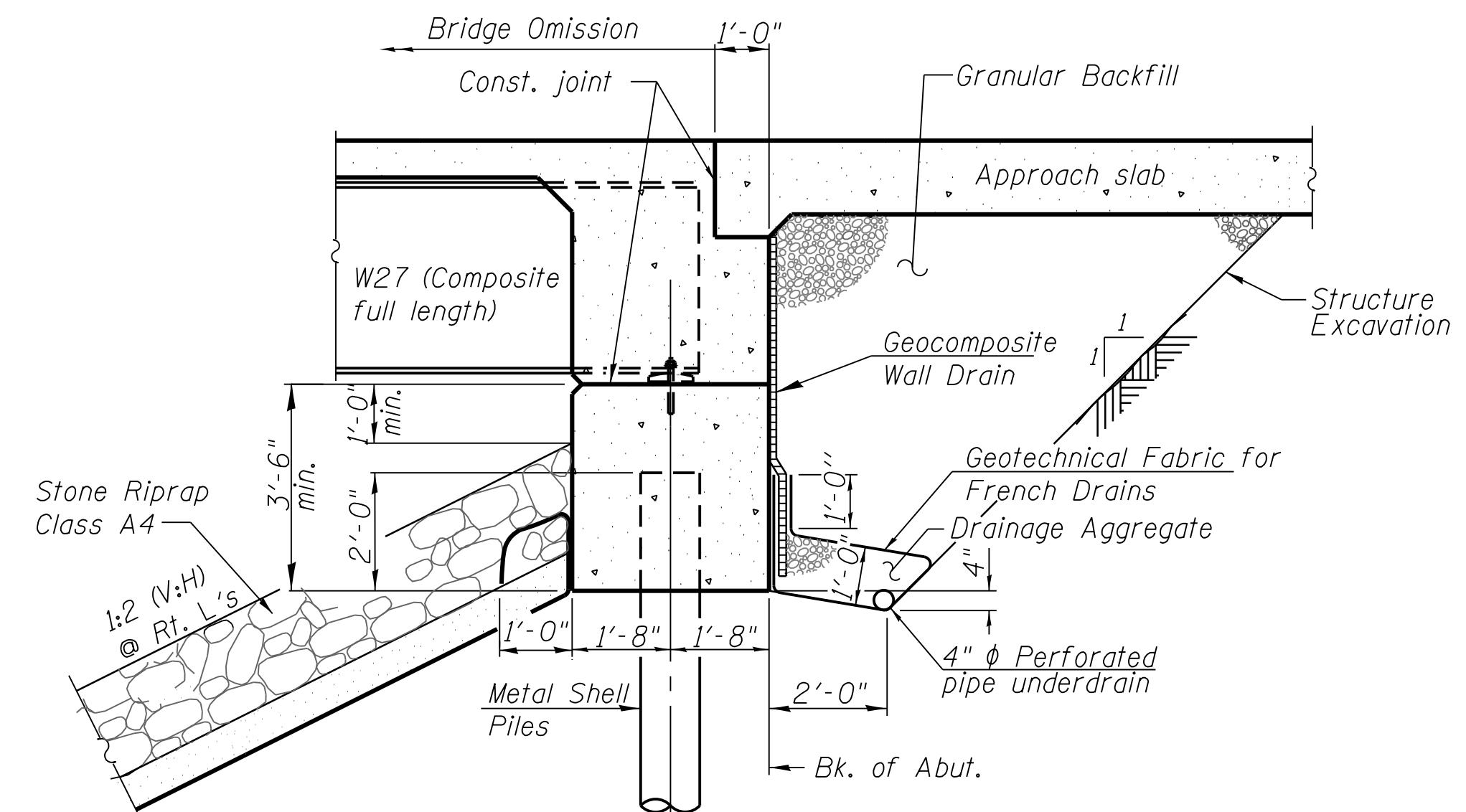
STRUCTURE NO. 027-0104

SHEET NO. 1 OF 2 SHEETS

F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1522	31-X-BR	FORD	1	ILLINOIS FED. AID PROJECT



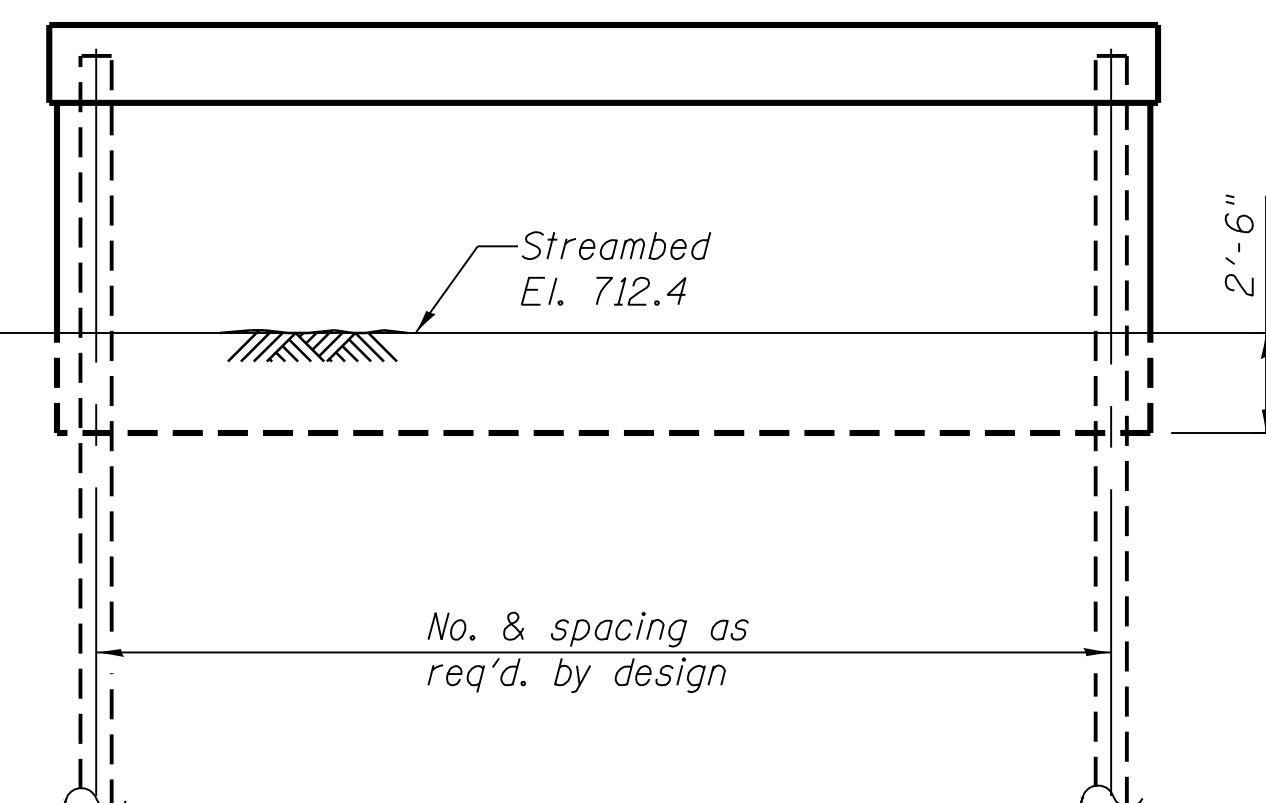
CROSS SECTION
(Looking North)



DESIGN SCOUR ELEVATION TABLE

Event / Limit State	Design Scour Elevations (ft.)				
	S. Abut.	Pier 1	Pier 2	N. Abut.	Item 113
Q100	731.5	709.4	709.4	730.5	
Q200	731.5	707.1	707.1	730.5	
Design	731.5	709.4	709.4	730.5	
Check	731.5	707.1	707.1	730.5	

PROFILE GRADE
(along U.S. Route 45)



PIER SKETCH

SECTIONS
U.S. ROUTE 45 OVER BIG FOUR DITCH

FAS 1522 (US RTE 45)

SECTION 31-X-BR

FORD COUNTY

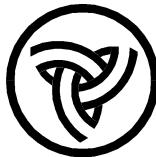
STA. 1131+64.17

STRUCTURE NO. 027-0104

Drainage Area = -	159 sq. mi	Exist. Low Grade Elev. = 736.41 @ Sta. 1134+00 Prop. Low Grade Elev. = 736.41 @ Sta. 1134+00				
Flood	Freq. Yr.	0 C.F.S.	Opening Sq. Ft.	Nat. H.W.E.	Head - Ft.	Headwater El.
	10	5460	1826	1935	730.5	0.1 0.1 730.6 730.6
Hydraulic Design	50	8340	2074	2204	732.1	0.1 0.1 732.2 732.2
Base	100	9560	2188	2326	732.8	0.2 0.2 733.0 732.9
Scour Design	200	10846	2257	2398	733.2	0.2 0.2 733.4 733.4
Max. Calc.	500	12500	2371	2515	733.9	0.3 0.3 734.2 734.1
Overtopping	N/A	N/A	N/A	N/A	N/A	N/A N/A

10-Year Velocity through Existing Bridge = 3.0 fps

10-Year Velocity through Proposed Bridge = 2.8 fps



**Illinois Department
of Transportation**

Division of Highways
IDOT

SOIL BORING LOG

Page 1 of 2

Date 6/20/14

ROUTE US 45 (SBI-25) DESCRIPTION US 45 over Big Four Ditch, 3.04 miles South of IL 9

LOGGED BY Larry Myers

SECTION 31-X-B LOCATION SW 1/4, SEC. 30, TWP. 23N, RNG. 10E, 3rd PM,
Latitude 40.418898, Longitude -88.113004

COUNTY Ford DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO. 027-0012 (Exist.)
Station 1131+54.25

BORING NO. 02 (N.E. Quad.)
Station 1132+99
Offset 19.0 ft Rt.
Ground Surface Elev. 736.91 ft

D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev. <u>715.09</u> ft	D E P T H	B L O W S	U C S Qu	M O I S T
				Stream Bed Elev. <u>713.09</u> ft				
				Groundwater Elev.: First Encounter <u>714.9</u> ft ▼				
				Upon Completion <u>696.9</u> ft ▽				
				After <u>Hrs.</u> ft				

Augered Shoulder Gravel, Brown Silty Clay Loam Till Fill

Hard Brown Silty Clay Loam Till
(continued)

Stiff to Very Stiff Brown & Gray Silty Clay Loam Till Fill & some Silty Clay Fill

Hard Gray Silty Clay Loam / Silty Clay Till

Stiff Black & Dark Brown Silty Clay Loam, Silty Loam, Silty Clay Fill

* Max Rimac @ 10%

Very Stiff Bluish Gray Silty Clay

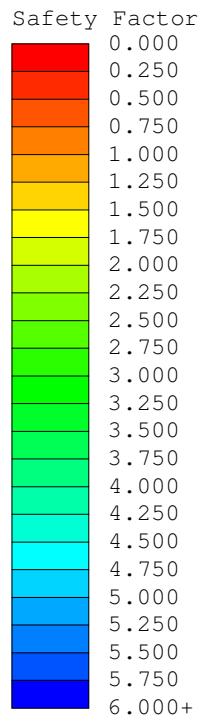
Hard Brownish Gray Silty Loam / Silty Clay Loam Till with Silt Layers

Very Stiff Brown & Gray Silty Clay Loess

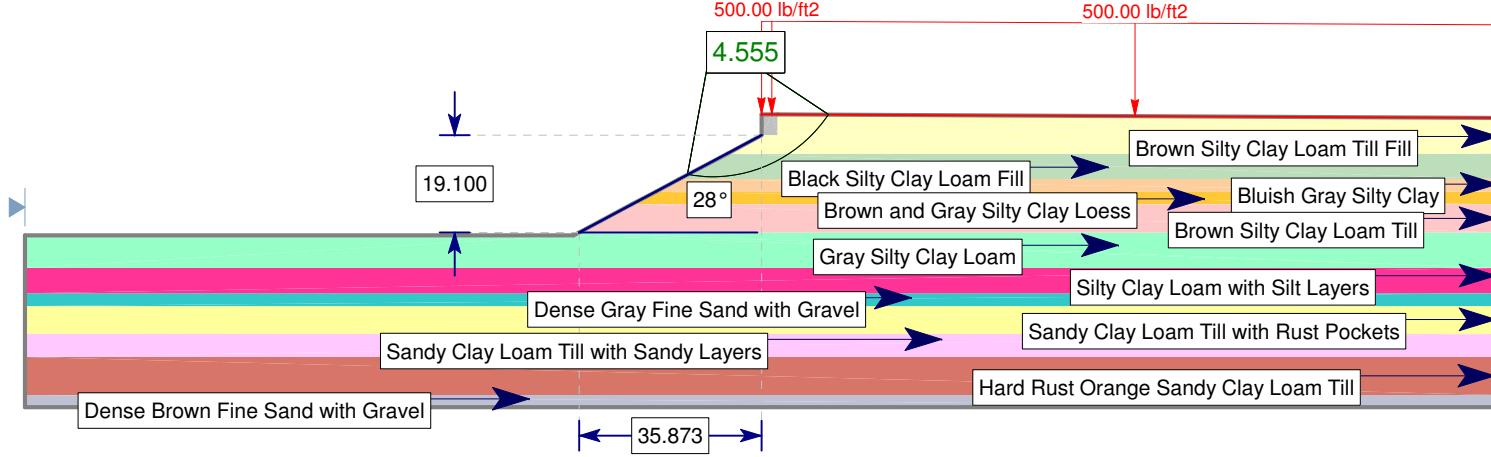
Very Dense Gray Fine Sand to Fine Gravel Bedded with no free water

Hard Brown Silty Clay Loam Till

Hard Gray Sandy Clay Loam Till with Sand Layers

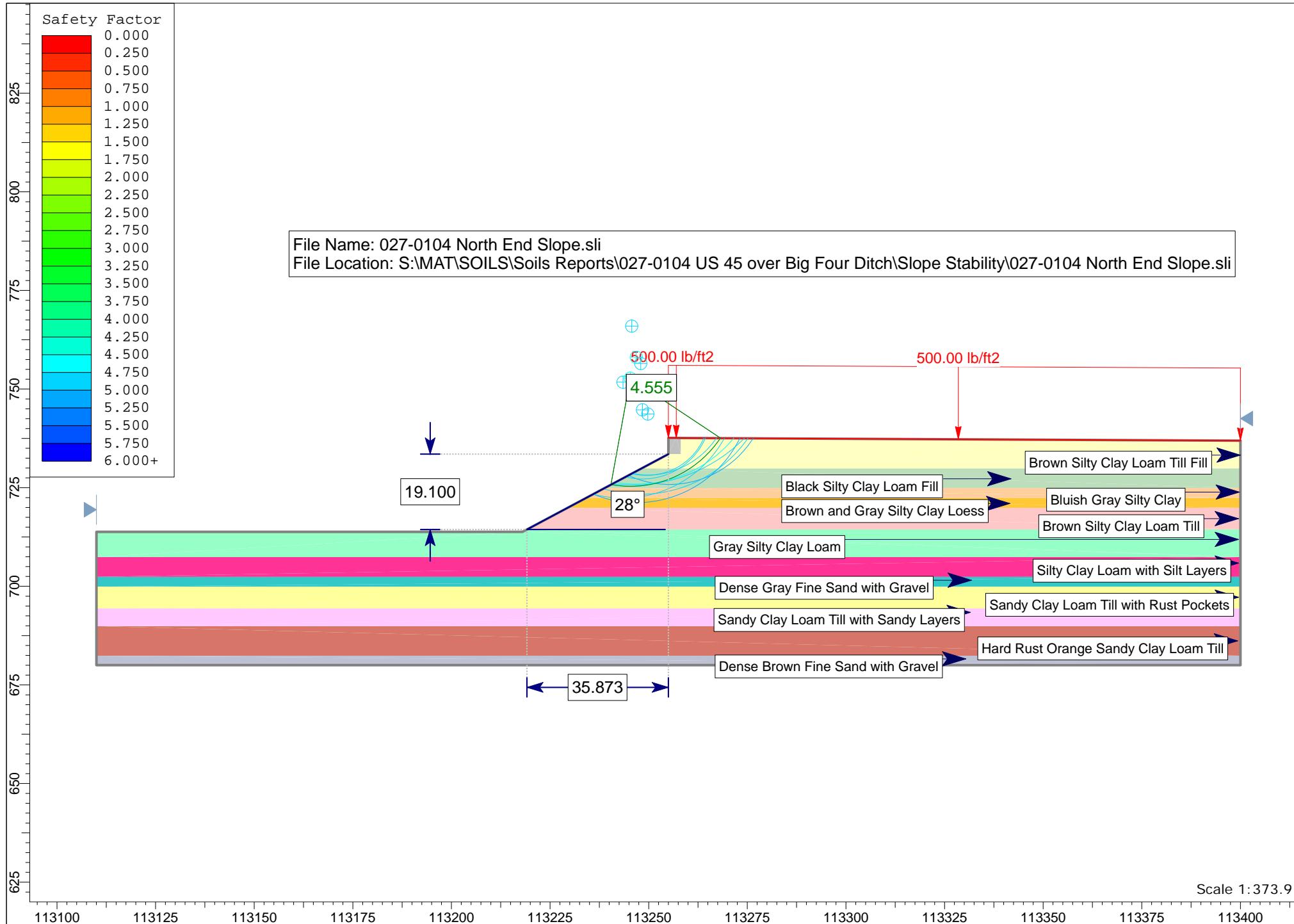


SN 027-0104 North End Slope Stability Analysis



File Name: 027-0104 North End Slope.sli

File Location: S:\MAT\SOILS\Soils Reports\027-0104 US 45 over Big Four Ditch\Slope Stability\027-0104 North End Slope.sli





SEISMIC SITE CLASS DETERMINATION

Substructure 1 South Abutment										
Base of Substruct. Elev. (or ground surf for bents)			731.6 ft.							
Pile or Shaft Dia.			12 inches							
Boring Number			1							
Top of Boring Elev.			738.6 ft.							
Approximate Fixity Elev. 725.6 ft.										
Individual Site Class Definition:										
N (bar): 13 (Blows/ft.) Soil Site Class E										
N _{ch} (bar): NA (Blows/ft.) NA										
s _u (bar): 3.84 (ksf) Soil Site Class C <---Controls										
Seismic Bot. Of Layer										
Soil Column		Sample		Description						
Depth	Elevation	Thickness	N	Qu	Boundary					
(ft)	(ft.)	(ft.)	(tsf)							
736.1	2.50	2	1.00	B						
733.6	2.50	6	2.50							
731.1	2.50	5	2.50							
728.6	2.50	9	3.50							
726.1	2.50	9	2.70	B						
1.5	724.1	2.00	2	2.00	B					
4.0	721.6	2.50	6	2.50	B					
6.5	719.1	2.50	2	1.50						
9.0	716.6	2.50	2	1.00	B					
11.5	714.1	2.50	19		B					
14.0	711.6	2.50	22	6.60						
16.5	709.1	2.50	19	6.80						
19.0	706.6	2.50	18	6.20						
21.5	704.1	2.50	19	6.00	B					
24.5	701.1	3.00	16	5.00	B					
27.0	698.6	2.50	88							
29.0	696.6	2.00	73		B					
31.5	694.1	2.50	18	3.70						
34.0	691.6	2.50	22	4.10						
36.0	689.6	2.00	27	6.30	B					
39.0	686.6	3.00	27	4.50						
42.0	683.6	3.00	27	4.50	B					
44.5	681.1	2.50	32	5.50						
47.0	678.6	2.50	32	5.50	B					
48.5	677.1	1.50	13	3.50	B					
51.5	674.1	3.00	24	4.50						
54.5	671.1	3.00	24	4.50						
57.5	668.1	3.00	24	4.50						
60.5	665.1	3.00	24	4.50						
63.5	662.1	3.00	24	4.50						
66.5	659.1	3.00	24	4.50						
69.5	656.1	3.00	24	4.50						
72.5	653.1	3.00	24	4.50						
75.5	650.1	3.00	24	4.50						
79.0	646.6	3.50	24	4.50						
82.5	643.1	3.50	24	4.50						
86.0	639.6	3.50	24	4.50						
89.5	636.1	3.50	24	4.50						
93.0	632.6	3.50	24	4.50						
96.5	629.1	3.50	24	4.50						
100.0	625.6	3.50	24	4.50	B					

Substructure 2 Pier 1										
Base of Substruct. Elev. (or ground surf for bents)			709.9 ft.							
Pile or Shaft Dia.			12 inches							
Boring Number			1							
Top of Boring Elev.			738.6 ft.							
Approximate Fixity Elev. 703.9 ft.										
Individual Site Class Definition:										
N (bar): 24 (Blows/ft.) Soil Site Class D										
N _{ch} (bar): NA (Blows/ft.) NA										
s _u (bar): 4.5 (ksf) Soil Site Class C <---Controls										
Seismic Bot. Of Layer										
Soil Column		Sample		Description						
Depth	Elevation	Thickness	N	Qu	Boundary					
		(ft.)	(ft.)	(tsf)						
	736.1	2.50	2	1.00	B					
	733.6	2.50	6	2.50						
	731.1	2.50	5	2.50						
	728.6	2.50	9	3.50						
	726.1	2.50	9	2.70	B					
	724.1	2.00	2	2.00	B					
	721.6	2.50	6	2.50	B					
	719.1	2.50	2	1.50						
	716.6	2.50	2	1.00	B					
	714.1	2.50	19		B					
	711.6	2.50	22	6.60						
	709.1	2.50	19	6.80						
	706.6	2.50	18	6.20						
	704.1	2.50	19	6.00	B					
	701.1	3.00	16	5.00	B					
	698.6	2.50	88							
	696.6	2.00	73		B					
	694.1	2.50	18	3.70						
	691.6	2.50	22	4.10						
	689.6	2.00	27	6.30	B					
	686.6	3.00	27	4.50						
	683.6	3.00	27	4.50	B					
	681.1	2.50	32	5.50						
	678.6	2.50	32	5.50	B					
	677.1	1.50	13	3.50	B					
	674.1	3.00	24	4.50						
	671.1	3.00	24	4.50						
	668.1	3.00	24	4.50						
	665.1	3.00	24	4.50						
	662.1	3.00	24	4.50						
	659.1	3.00	24	4.50						
	656.1	3.00	24	4.50						
	653.1	3.00	24	4.50						
	650.1	3.00	24	4.50						
	646.6	3.50	24	4.50						
	643.1	3.50	24	4.50						
	639.6	3.50	24	4.50						
	636.1	3.50	24	4.50						
	632.6	3.50	24	4.50						
	629.1	3.50	24	4.50						
	625.6	3.50	24	4.50	B					

Substructure 3 Pier 2										
Base of Substruct. Elev. (or ground surf for bents)			709.9 ft.							
Pile or Shaft Dia.			12 inches							
Boring Number			2							
Top of Boring Elev.			736.9 ft.							
Approximate Fixity Elev. 703.9 ft.										
Individual Site Class Definition:										
N (bar): 35 (Blows/ft.) Soil Site Class D										
N _{ch} (bar): 42 (Blows/ft.) Soil Site Class D <---Controls										
s _u (bar): 4.8 (ksf) Soil Site Class C										
Seismic Bot. Of Layer										
Soil Column		Sample		Description						
Depth	Elevation	Thickness	N	Qu	Boundary					
		(ft.)	(ft.)	(tsf)						
	736.1	2.50	2	1.00	B					
	733.6	2.50	6	2.50						
	731.1	2.50	5	2.50						
	728.6	2.50	9	3.50						
	726.1	2.50	9	2.70	B					
	724.1	2.00	2	2.00	B					
	721.6	2.50	6	2.50	B					
	719.1	2.50	2	1.50						
	716.6	2.50	2	1.00	B					
	714.1	2.50	19		B					
	711.6	2.50	22	6.60						
	709.1	2.50	19	6.80						
	706.6	2.50	18	6.20						
	704.1	2.50	19	6.00	B					
	701.1	3.00	16	5.00	B					
	698.6	2.50	88							
	696.6	2.00	73		B					
	694.1	2.50	18	3.70						
	691.6	2.50	22	4.10						
	689.6	2.00	27	6.30	B					
	686.6	3.00	27	4.50						
	683.6	3.00	27	4.50	B					
	681.1	2.50	32	5.50						
	678.6	2.50	32	5.50	B					
	677.1	1.50	13	3.50	B					
	674.1	3.00	24	4.50						
	671.1	3.00	24	4.50						
	668.1	3.00	24	4.50						
	665.1	3.00	24	4.50						
	662.1	3.00	24	4.50						
	659.1	3.00	24	4.50						
	656.1	3.00	24	4.50						
	653.1	3.00	24	4.50						
	650.1	3.00	24	4.50						
	646.6	3.50	24	4.50						
	643.1	3.50	24	4.50						
	639.6	3.50	24	4.50						
	636.1	3.50	24	4.50						
	632.6	3.50	24	4.50						
	629.1	3.50	24	4.50						
	625.6	3.50	24	4.50	B					

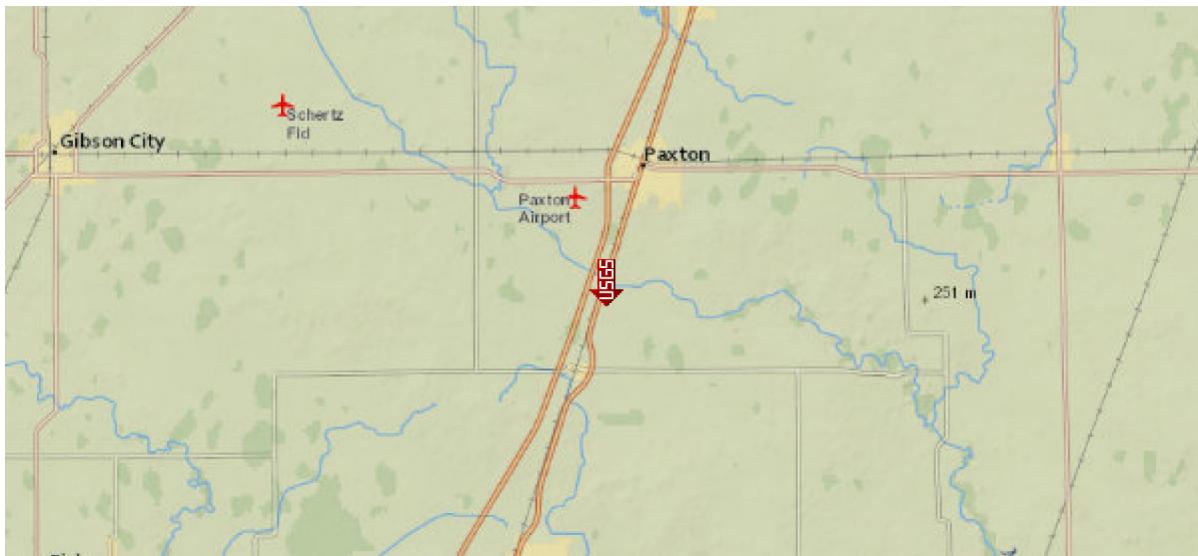
Substructure 4 North Abutment						
Base of Substruct. Elev. (or ground surf for bents)			730.5 ft.			
Pile or Shaft Dia.			12 inches			
Boring Number			2			
Top of Boring Elev.			736.9 ft.			



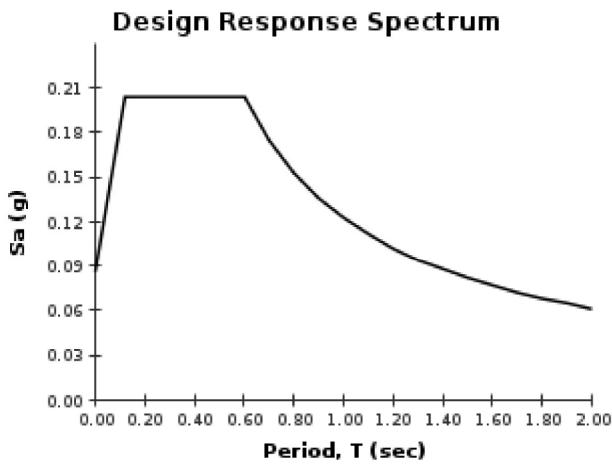
PROJECT TITLE=====

USGS Design Maps Summary Report**User-Specified Input****Report Title** SN 027-0104

Wed January 4, 2017 19:07:00 UTC

Building Code Reference Document 2009 AASHTO Guide Specifications for LRFD Seismic Bridge Design
(which utilizes USGS hazard data available in 2002)**Site Coordinates** 40.41817°N, 88.11345°W**Site Soil Classification** Site Class D – "Stiff Soil"**USGS-Provided Output**

PGA = 0.054 g	A_s = 0.086 g
S_s = 0.128 g	S_{Ds} = 0.204 g
S₁ = 0.051 g	S_{D1} = 0.123 g



Although this information is a product of the U.S. Geological Survey, we provide no warranty, expressed or implied, as to the accuracy of the data contained therein. This tool is not a substitute for technical subject-matter knowledge.

[External] US 45 over Big Four Ditch

[Hide](#)

From: Lori Sommer
Sent: 12/19/2016 2:24 PM
To: Short, Michael A; Ferguson, Steven P
Cc: Deborah Zroka

 [Message](#)  [PTSL US 45 over Big Four Ditch.pdf \(1.9 MB\)](#)

Mike,

I have attached the preliminary TSL for the bridge carrying US 45 over the Big Four Ditch.

These are the preliminary substructure loads:

- Abutments
 - Service Dead Load=305k
 - Service Live Load plus Impact =255k
 - Total Service Load= 560k
 - Factored Dead Load=390k
 - Factored Live Load plus Impact= 440k
 - Total Factored Load = 830k
- Piers
 - Service Dead Load=975k
 - Service Live Load =305k
 - Total Service Load= 1280k
 - Factored Dead Load=1250k
 - Factored Live Load = 535k
 - Total Factored Load = 1785k

Please let me know if you need any other information.

Lori Sommer SE, PE| Senior Structural Engineer

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STRUCTURE NUMBER===== SN 027-0104
 STRUCTURE TYPE ====== MULTI-SPAN
 STRUCTURE SKEW===== 14
 TOTAL STRUCTURE LENGTH===== 192.94
 LONGEST END SPAN LENGTH ===== 59.14

DEGREES
FT
FT

ABUTMENT #1 DATA

ABUTMENT NAME ====== South Abutment
 ABUTMENT REFERENCE BORING===== Boring 01
 BOTTOM OF ABUTMENT ELEVATION===== 731.6
 ESTIMATED NUMBER OF PILES AT ABUT.===== 8

FT

ABUTMENT #2 DATA

ABUTMENT NAME ====== North Abutment
 ABUTMENT REFERENCE BORING===== Boring 02
 BOTTOM OF ABUTMENT ELEVATION===== 730.5
 ESTIMATED NUMBER OF PILES AT ABUT.===== 8

FT

SOIL DATA FOR 10 FT BENEATH BOTTOM OF ABUTMENT #1				
BOT. OF LAYER ELEV. (FT)	LAYER THICKNESS (FT)	UNCONFINED COMPRESSIVE STRENGTH (TSF)	N S.P.T. VALUE (BLOWS/12 IN.)	Qu EQUIV. FOR N VALUE (TSF)
729.10	2.50	3.5		
726.60	2.50	2.7		
724.10	2.50	2.0		
721.60	2.50	2.5		

10.00 FT = TOTAL DEPTH ENTERED

SOIL DATA FOR 10 FT BENEATH BOTTOM OF ABUTMENT #2				
BOT. OF LAYER ELEV. (FT)	LAYER THICKNESS (FT)	UNCONFINED COMPRESSIVE STRENGTH (TSF)	N S.P.T. VALUE (BLOWS/12 IN.)	Qu EQUIV. FOR N VALUE (TSF)
729.90	0.60	1.5		
727.40	2.50	1.5		
724.90	2.50	2.0		
722.40	2.50	2.00		
720.50	1.90	2.50		

10.00 FT = TOTAL DEPTH ENTERED

WEIGHTED AVERAGE Qu FOR ABUTMENT #1===== 2.68 TSF

WEIGHTED AVERAGE Qu FOR ABUTMENT #2===== 1.94 TSF

PILE STIFFNESS MODIFIER FOR ABUTMENT #1
 $= 1/(1.45-[0.3*2.68]) = 1.54$

PILE STIFFNESS MODIFIER FOR ABUTMENT #2
 $= 1/(1.45-[0.3*1.94]) = 1.15$

DISTANCE TO CENTROID OF STIFFNESS FROM ABUTMENT #1 = $[1.54*8*0+1.15*8*192.94]/[1.54*8+1.15*8] = 82.43$ FT

DISTANCE TO CENTROID OF STIFFNESS FROM ABUTMENT #2 = $[1.15*8*0+1.54*8*192.94]/[1.15*8+1.54*8] = 110.51$ FT

EFFECTIVE EXPANSION LENGTH (EEL) CALCULATION

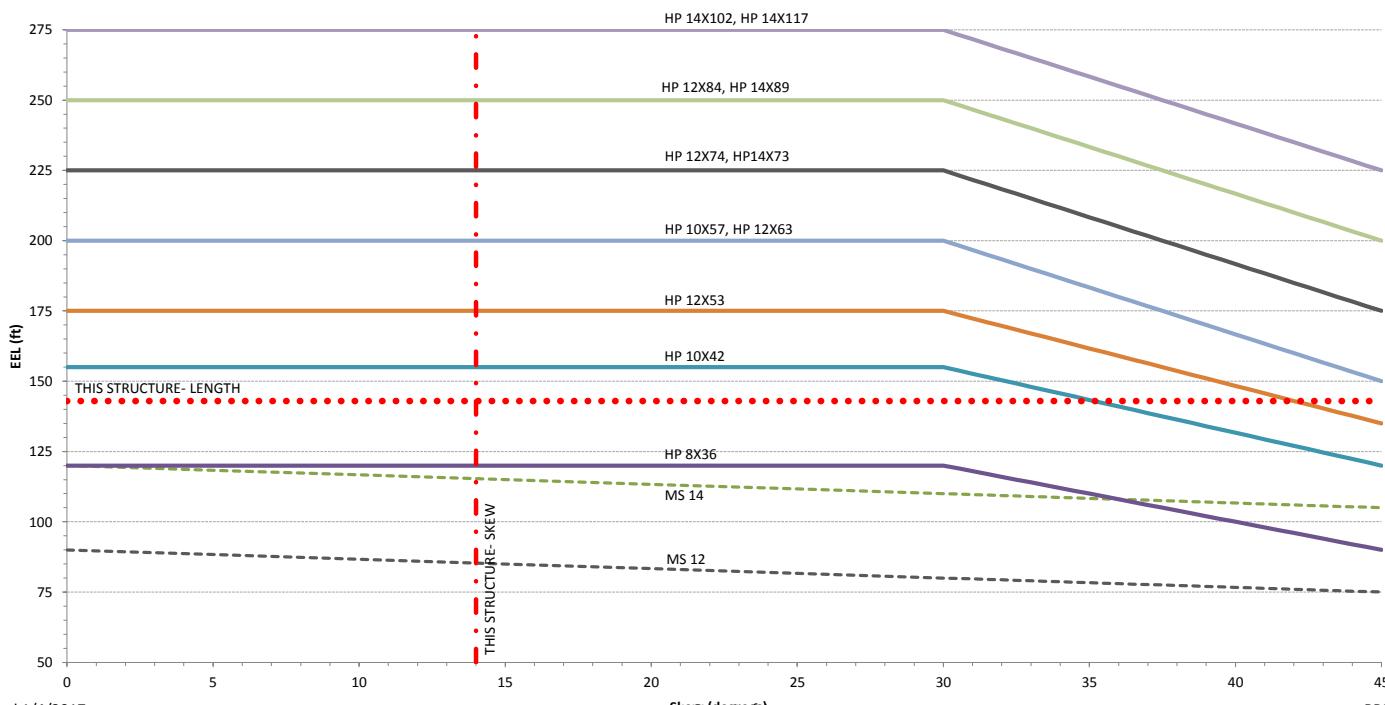
CONTROLLING ABUTMENT===== ABUT. #2 North Abutment
 CONTROLLING EXPANSION LENGTH (DISTANCE TO CENTROID OF STIFFNESS FROM CONTROLLING ABUTMENT)===== 110.51 FT
 WEIGHTED AVE. Qu FOR CONTROLLING ABUTMENT ===== 1.94 TSF
 Qu CORRECTION FACTOR ===== 1.94/1.5 ===== 1.29
 EFFECTIVE EXPANSION LENGTH (EEL) ===== EEL = 110.51*1.29 ===== 142.92 FT

FEASIBLE PILE TYPES PER CHART IN ABD MEMO 12.3 BASED ON SKEW AND EEL OR MODIFIED EEL:

PILE SIZES AT OR ABOVE THE LENGTH LINE AT THE INTERSECTION WITH THE SKEW LINE ARE ALLOWED FOR USE WITH THIS INTEGRAL ABUTMENT STRUCTURE

AVAILABLE PILE SIZES:

HP 10X42, HP 12X53, HP 10X57, HP 12X63, HP 12X74, HP 14X73, HP 12X84, HP 14X89, HP 14X102, HP 14X117

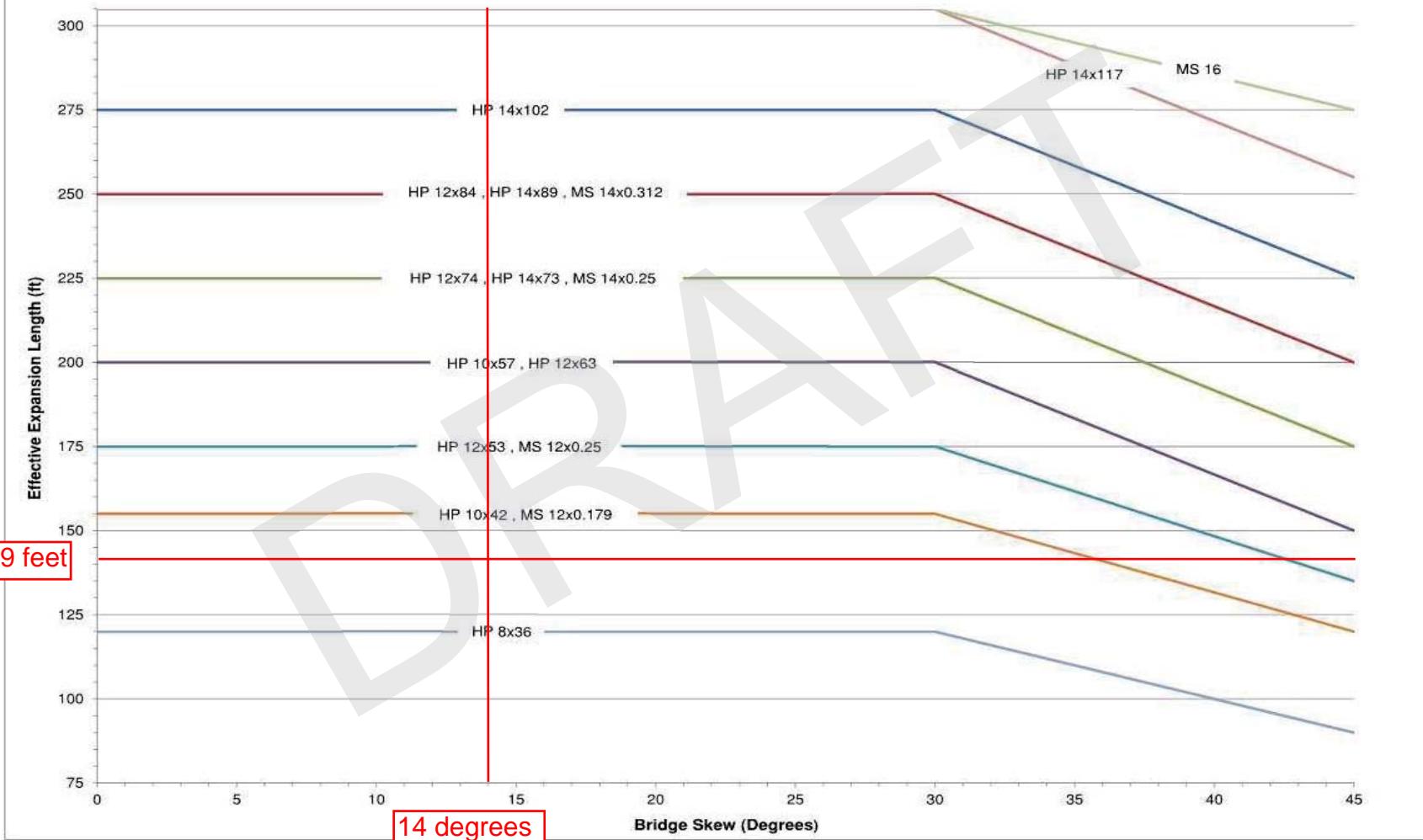


SN 027-0104 Proposed Integral Abutment Policy Pile Selection Chart

Structure Skew = 14 degrees

Effective Expansion Length = 142.9 feet

Integral Abutment Pile Selection Chart



Allowable pile sizes:

MS 12X0.179, MS 12X0.25, MS 14X0.25, MS 14X0.312, MS 16
HP 10X42, HP 10X57, HP 12X53, HP 12X63, HP 12X74, HP 12X84
HP 14X73, HP 14X89, HP 14X102, HP 14X117

SUBSTRUCTURE===== South Abutment
 REFERENCE BORING ===== 1
 LRFD or ASD or SEISMIC ===== LRFD
 PILE CUTOFF ELEV. ===== 733.60 ft
 GROUND SURFACE ELEV. AGAINST PILE DURING DRIVING = 731.60 ft
 GEOTECHNICAL LOSS TYPE (None, Scour, Liquef., DD) ===== None
 BOTTOM ELEV. OF SCOUR, LIQUEF., or DD ===== ft
 TOP ELEV. OF LIQUEF. (so layers above apply DD) ===== ft

Maximum Nominal Req'd Bearing of Pile	Maximum Nominal Req'd Bearing of Boring	Maximum Factored Resistance Available in Boring	Maximum Pile Driveable Length in Boring
588 KIPS	377 KIPS	207 KIPS	31 FT.

TOTAL FACTORED SUBSTRUCTURE LOAD ===== 830 kips
 TOTAL LENGTH OF SUBSTRUCTURE (along skew)===== 37.10 ft
 NUMBER OF ROWS OF PILES PER SUBSTRUCTURE ===== 1
 Approx. Factored Loading Applied per pile at 8 ft. Cts ===== 178.98 KIPS
 Approx. Factored Loading Applied per pile at 3 ft. Cts ===== 67.12 KIPS

PILE TYPE AND SIZE ===== Metal Shell 16"Φ w/.312" walls
 Pile Perimeter===== 4.189 FT.
 Pile End Bearing Area===== 1.396 SQFT.

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. STRENGTH (TSF.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
729.10	2.50	3.50	9	Sandy Gravel	28.1	69.4		69	0	0	38	5
726.60	2.50	2.70	9		23.3	41.4	82.0	82	0	0	45	7
724.10	2.50	2.00	4		19.1	30.6	108.7	109	0	0	60	10
721.60	2.50	2.50	6		22.1	38.3	115.5	115	0	0	64	12
719.10	2.50	1.50	2		15.8	23.0	123.6	124	0	0	68	15
716.60	2.50	1.00	2		11.6	15.3	286.9	287	0	0	158	17
714.10	2.50	1.00	19		23.5	167.0	244.5	245	0	0	134	20
711.60	2.50	6.60	22		34.1	101.1	281.6	282	0	0	155	22
709.10	2.50	6.80	19		34.1	104.1	306.5	307	0	0	169	25
706.60	2.50	6.20	18		34.1	95.0	337.5	338	0	0	186	27
704.10	2.50	6.00	19		34.1	91.9	356.3	356	0	0	196	30
702.60	1.50	5.00	16	Fine Sand Medium Sand	20.4	76.6	376.8	377	0	0	207	31
701.10	1.50	5.00	16		20.4	76.6	1094.1	1094	0	0	602	33
698.60	2.50	88			156.9	773.5	1119.2	1119	0	0	616	35
696.60	2.00	73			111.3	641.6	645.5	646	0	0	365	37
694.10	2.50	3.70	18		29.3	56.7	680.9	681	0	0	375	40
691.60	2.50	4.10	22		31.7	62.8	746.3	746	0	0	410	42
689.60	2.00	6.30	27		27.3	96.5	746.0	746	0	0	410	44
687.10	2.50	4.50	27		34.1	68.9	780.1	780	0	0	429	47
684.60	2.50	4.50	27		34.1	68.9	814.2	814	0	0	448	49
683.60	1.00	4.50	27		13.6	68.9	956.6	957	0	0	526	50
681.10	2.50		30		17.2	197.8	973.8	974	0	0	536	53
678.60	2.50		30		17.2	197.8	846.9	847	0	0	466	55
677.10	1.50	3.50	13			53.6						

Pile Design Table for South Abutment utilizing Boring #1

Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)	Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)	Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)			
Metal Shell 16"Φ w/.312" walls											
115	64	12	106	58	20	97	53	15			
124	68	15	131	72	22	135	74	17			
245	134	20	156	86	25	149	82	20			
282	155	22	177	97	27	186	102	22			
307	169	25	185	102	30	221	121	25			
338	186	27	195	107	31	257	141	27			
356	196	30	236	130	37	286	157	30			
377	207	31	255	140	40	300	165	31			
Metal Shell 16"Φ w/.375" walls											
115	64	12	289	159	44	349	192	37			
124	68	15	306	168	47	378	208	40			
245	134	20	324	178	49	428	235	44			
282	155	22	326	179	50	452	249	47			
307	169	25	326	179	55	475	261	55			
338	186	27	111	61	17	Steel HP 14 X 89					
356	196	30	122	67	20	98	54	15			
377	207	31	153	84	22	137	75	17			
Metal Shell 14"Φ w/.25" walls											
106	59	15	182	100	25	153	84	20			
203	112	20	212	117	27	190	105	22			
235	129	22	228	126	30	225	124	25			
258	142	25	241	132	31	261	144	27			
285	142	25	285	157	37	290	159	30			
303	167	30	309	170	40	304	167	31			
321	177	31	350	192	44	354	195	37			
Metal Shell 14"Φ w/.312" walls											
106	59	15	370	204	47	383	211	40			
203	112	20	391	215	49	433	238	44			
235	129	22	392	216	55	458	252	47			
258	142	25	Steel HP 12 X 63	Steel HP 14 X 102			481	264	55		
285	157	27	114	63	17	99	55	15			
303	167	30	126	69	20	139	76	17			
321	177	31	157	86	22	156	86	20			
Steel HP 8 X 36											
120	66	25	186	103	25	193	106	22			
132	73	27	217	119	27	228	126	25			
140	77	30	231	127	30	264	145	27			
149	82	31	243	134	31	294	162	30			
183	101	37	288	159	37	309	170	31			
197	109	40	312	171	40	358	197	37			
224	123	44	353	194	44	388	213	40			
238	131	47	374	206	47	439	241	44			
252	139	49	395	217	49	464	255	47			
255	140	50	396	218	55	486	268	55			
256	141	55	Steel HP 12 X 74	Steel HP 14 X 117			Steel HP 14 X 102	99	55	15	
Steel HP 10 X 42									139	76	17
102	56	20	160	88	22	156	86	20	160	88	20
128	70	22	189	104	25	198	109	22	198	109	22
152	84	25	219	121	27	233	128	25	233	128	25
173	95	27	235	129	30	269	148	27	269	148	27
180	99	30	247	136	31	298	164	30	298	164	30
191	105	31	292	161	37	313	172	31	313	172	31
230	127	37	316	174	40	363	200	37	363	200	37
249	137	40	358	197	44	393	216	40	393	216	40
282	155	44	380	209	47	445	244	44	445	244	44
299	165	47	401	220	49	470	258	47	470	258	47
317	174	49	401	221	55	492	271	55	492	271	55
319	175	50	Steel HP 12 X 84	Precast 14"x 14"			Steel HP 14 X 117	101	55	15	
319	176	55	117	64	17	141	78	17	141	78	17
Timber Pile									160	88	20
			122	72	20	198	109	22	198	109	22
			131	89	22	233	128	25	233	128	25
			162	106	25	269	148	27	269	148	27
			192	122	27	298	164	30	298	164	30
			238	131	30	313	172	31	313	172	31
			251	138	31	363	200	37	363	200	37
			297	163	37	393	216	40	393	216	40
			321	177	40	445	244	44	445	244	44
			364	200	44	470	258	47	470	258	47
			385	212	47	492	271	55	492	271	55
			406	223	49	Timber Pile			116	64	10
			407	224	55	125	69	12	125	69	12
						136	75	15	136	75	15

SUBSTRUCTURE===== Pier 1
 REFERENCE BORING ===== 1
 LRFD or ASD or SEISMIC ===== LRFD
 PILE CUTOFF ELEV. ===== 733.60 ft
 GROUND SURFACE ELEV. AGAINST PILE DURING DRIVING = 709.90 ft
 GEOTECHNICAL LOSS TYPE (None, Scour, Liquef., DD) = Scour
 BOTTOM ELEV. OF SCOUR, LIQUEF., or DD ===== 709.40 ft
 TOP ELEV. OF LIQUEF. (so layers above apply DD) ===== ft

TOTAL FACTORED SUBSTRUCTURE LOAD ===== 1785 kips
 TOTAL LENGTH OF SUBSTRUCTURE (along skew)===== 37.10 ft
 NUMBER OF ROWS OF PILES PER SUBSTRUCTURE ===== 1
 Approx. Factored Loading Applied per pile at 8 ft. Cts ===== 384.91 KIPS
 Approx. Factored Loading Applied per pile at 3 ft. Cts ===== 144.34 KIPS

PILE TYPE AND SIZE ===== Metal Shell 16"Φ w/.312" walls
 Pile Perimeter===== 4.189 FT.
 Pile End Bearing Area===== 1.396 SQFT.

MAX. REQUIRED BEARING & RESISTANCE for Selected Pile, Soil Profile, & Losses			
	Maximum Nominal Req'd Bearing of Pile	Maximum Nominal Req'd Bearing of Boring	Maximum Factored Resistance Available in Boring
	588 KIPS	579 KIPS	319 KIPS

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. STRENGTH (TSF.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
707.00	2.90	6.20	18		39.5		39.5	40	0	0	22	27
704.10	2.90	6.00	19		39.5		79.1	79	0	0	43	30
703.35	0.75	5.00	16		10.2		89.3	89	0	0	49	30
702.60	0.75	5.00	16		10.2		99.5	100	0	0	55	31
701.85	0.75	5.00	16		10.2		109.7	110	0	0	60	32
701.60	0.25	5.00	16		3.4		113.1	113	0	0	62	32
701.35	0.25	5.00	16		3.4		116.5	117	0	0	64	32
701.10	0.25	5.00	16		3.4		120.0	120	0	0	66	33
700.85	0.25	88		Fine Sand	15.7		135.6	136	0	0	75	33
700.60	0.25	88		Fine Sand	15.7		151.3	151	0	0	83	33
700.10	0.50	88		Fine Sand	31.4		182.7	183	0	0	100	34
698.60	1.50	88		Fine Sand	94.2		276.9	277	0	0	152	35
697.60	1.00	73		Medium Sand	55.7		332.5	333	0	0	183	36
697.10	0.50	73		Medium Sand	27.8		360.4	360	0	0	198	37
696.60	0.50	73		Medium Sand	27.8		444.9	445	0	0	245	37
694.10	2.50	3.70	18		29.3	56.7	480.3	480	0	0	264	40
691.60	2.50	4.10	22		31.7	62.8	545.6	546	0	0	300	42
689.60	2.00	6.30	27		27.3	96.5	545.3	545	0	0	300	44
687.10	2.50	4.50	27		34.1	68.9	579.4	579	0	0	319	47
684.60	2.50	4.50	27		34.1	68.9	613.5	613	0	0	337	49
683.60	1.00	4.50	27		13.6	68.9	756.0	756	0	0	416	50
681.10	2.50	30		Hard Till	17.2	197.8	773.2	773	0	0	425	53
678.60	2.50	30		Hard Till	17.2	197.8	646.2	646	0	0	355	55
677.10	1.50	3.50	13		16.8	53.6	663.1	663	0	0	365	57
674.60	2.50	3.50	13		28.1	53.6	691.1	691	0	0	390	59
672.10	2.50	3.50	13		28.1	53.6	719.2	719	0	0	396	62
669.60	2.50	3.50	13			53.6						

Pile Design Table for Pier 1 utilizing Boring #1

Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)	Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)	Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)
Metal Shell 16"Φ w/.312" walls								
183	100	34	255	140	59	246	135	40
277	152	35	269	148	62	294	162	42
333	183	36	259	142	47	296	163	44
360	198	37	279	154	49	320	176	47
445	245	37	280	154	55	343	188	55
480	264	40	291	160	57	355	195	57
545	300	44	308	169	59	375	206	59
579	319	47	325	178	62	395	217	62
Metal Shell 16"Φ w/.375" walls								
183	100	34	262	144	47	249	137	40
277	152	35	282	155	49	299	165	42
333	183	36	283	156	55	300	165	44
360	198	37	293	161	57	325	178	47
445	245	37	311	171	59	347	191	55
480	264	40	328	180	62	359	198	57
545	300	44	245	135	44	380	209	59
579	319	47	266	146	47	400	220	62
613	337	49	287	158	49	253	139	40
Metal Shell 14"Φ w/.25" walls								
242	133	35	287	158	55	303	166	42
291	160	36	298	164	57	304	167	44
315	173	37	315	173	59	329	181	47
383	211	37	332	183	62	351	193	55
Metal Shell 14"Φ w/.312" walls								
242	133	35	248	137	44	364	200	57
291	160	36	270	148	47	384	211	59
315	173	37	291	160	49	405	223	62
383	211	37	291	160	55	256	141	40
413	227	40	302	166	57	308	169	42
467	257	42	320	176	59	308	169	44
470	258	44	337	185	62	333	183	47
499	275	47				356	196	55
Steel HP 8 X 36								
210	115	62				368	203	57
Steel HP 10 X 42								
249	137	59				389	214	59
263	145	62				410	225	62
Steel HP 10 X 57								
			259	142	47	Steel HP 14 X 73		
			279	154	49	246	135	40
			280	154	55	294	162	42
			291	160	57	296	163	44
			308	169	59	320	176	47
						343	188	55
						355	195	57
						375	206	59
						395	217	62
Steel HP 12 X 53								
			282	155	49	Steel HP 14 X 89		
			283	156	55	249	137	40
			293	161	57	299	165	42
			311	171	59	300	165	44
			328	180	62	325	178	47
						347	191	55
						359	198	57
						380	209	59
						400	220	62
Steel HP 12 X 63								
			245	135	44	Steel HP 14 X 102		
			266	146	47	253	139	40
			287	158	49	303	166	42
			287	158	55	304	167	44
			298	164	57	329	181	47
			315	173	59	351	193	55
						364	200	57
Steel HP 12 X 74								
			332	183	62	384	211	59
						405	223	62
Steel HP 12 X 84								
			248	137	44	Steel HP 14 X 117		
			270	148	47	256	141	40
			291	160	49	308	169	42
			291	160	55	308	169	44
			302	166	57	333	183	47
			320	176	59	356	196	55
			337	185	62	368	203	57
Steel HP 8 X 36								
						389	214	59
						410	225	62
Steel HP 10 X 42								
						Precast 14"x 14"		
						204	112	34
Timber Pile								
						151	83	33

SUBSTRUCTURE===== Pier 2
 REFERENCE BORING ===== 2
 LRFD or ASD or SEISMIC ===== LRFD
 PILE CUTOFF ELEV. ===== 732.50 ft
 GROUND SURFACE ELEV. AGAINST PILE DURING DRIVING = 709.90 ft
 GEOTECHNICAL LOSS TYPE (None, Scour, Liquef., DD) = Scour
 BOTTOM ELEV. OF SCOUR, LIQUEF., or DD ===== 709.40 ft
 TOP ELEV. OF LIQUEF. (so layers above apply DD) ===== ft

Pier 2
 2
 LRFD
 732.50 ft
 709.90 ft
 Scour
 709.40 ft

Maximum Nominal Req'd Bearing of Pile	Maximum Nominal Req'd Bearing of Boring	Maximum Factored Resistance Available in Boring	Maximum Pile Driveable Length in Boring
704 KIPS	561 KIPS	309 KIPS	50 FT.

TOTAL FACTORED SUBSTRUCTURE LOAD ===== 1785 kips
 TOTAL LENGTH OF SUBSTRUCTURE (along skew)===== 37.10 ft
 NUMBER OF ROWS OF PILES PER SUBSTRUCTURE ===== 1
 Approx. Factored Loading Applied per pile at 8 ft. Cts ===== 384.91 KIPS
 Approx. Factored Loading Applied per pile at 3 ft. Cts ===== 144.34 KIPS

PILE TYPE AND SIZE ===== Metal Shell 16"Φ w/.375" walls
 Pile Perimeter===== 4.189 FT.
 Pile End Bearing Area===== 1.396 SQFT.

BOT. OF LAYER ELEV. (FT.)	LAYER THICK.	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)	
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)						
707.40	2.50	5.30	17		34.1		104.5		105	0	0	57	25
704.90	2.50	4.60	14		34.1	70.5	130.9		131	0	0	72	28
702.40	2.50	4.10	21		31.7	62.8	469.0		469	0	0	258	30
699.90	2.50		56	Sandy Gravel	153.3	369.2	317.4		317	0	0	175	33
697.40	2.50	4.20	13		32.3	64.3	378.8		379	0	0	208	35
694.90	2.50	6.10	9		34.1	93.4	412.9		413	0	0	227	38
692.40	2.50	6.10	13		34.1	93.4	447.0		447	0	0	246	40
689.90	2.50	6.10	18		34.1	93.4	470.3		470	0	0	259	43
687.40	2.50	5.40	15		34.1	82.7	502.9		503	0	0	277	45
684.90	2.50	5.30	14		34.1	81.2	533.9		534	0	0	294	48
683.90	1.00	5.10	16		13.6	78.1	547.5		548	0	0	301	49
683.40	0.50	5.10	16		6.8	78.1	554.3		554	0	0	305	49
682.90	0.50	5.10	16		6.8	78.1	561.1		561	0	0	309	50
682.40	0.50	5.10	16		6.8	78.1	815.1	815	0	0	448	50	
679.90	2.50		37	Sandy Gravel	63.2	325.2	895.8	896	0	0	493	53	
677.90	2.00		39		56.3	342.8	1040.1	1040	0	0	572	55	
675.40	2.50		49			430.7							

Pile Design Table for Pier 2 utilizing Boring #2

SUBSTRUCTURE===== North Abutment
 REFERENCE BORING ===== 2
 LRFD or ASD or SEISMIC ===== LRFD
 PILE CUTOFF ELEV. ===== 732.50 ft
 GROUND SURFACE ELEV. AGAINST PILE DURING DRIVING = 730.50 ft
 GEOTECHNICAL LOSS TYPE (None, Scour, Liquef., DD) ===== None
 BOTTOM ELEV. OF SCOUR, LIQUEF., or DD ===== ft
 TOP ELEV. OF LIQUEF. (so layers above apply DD) ===== ft

Maximum Nominal Req'd Bearing of Pile	Maximum Nominal Req'd Bearing of Boring	Maximum Factored Resistance Available in Boring	Maximum Pile Driveable Length in Boring
588 KIPS	331 KIPS	182 KIPS	28 FT.

TOTAL FACTORED SUBSTRUCTURE LOAD ===== 830 kips
 TOTAL LENGTH OF SUBSTRUCTURE (along skew)===== 37.10 ft
 NUMBER OF ROWS OF PILES PER SUBSTRUCTURE ===== 1
 Approx. Factored Loading Applied per pile at 8 ft. Cts ===== 178.98 KIPS
 Approx. Factored Loading Applied per pile at 3 ft. Cts ===== 67.12 KIPS

PILE TYPE AND SIZE ===== Metal Shell 16"Φ w/.312" walls
 Pile Perimeter===== 4.189 FT.
 Pile End Bearing Area===== 1.396 SQFT.

BOT. OF LAYER ELEV. (FT.)	LAYER THICK. STRENGTH (TSF.)	UNCONF. COMPR. STRENGTH (TSF.)	S.P.T. N VALUE (BLOWS)	GRANULAR OR ROCK LAYER DESCRIPTION	NOMINAL			NOMINAL REQ'D BEARING (KIPS)	FACTORED GEOTECH. LOSS FROM SCOUR or DD (KIPS)	FACTORED GEOTECH. LOSS LOAD FROM DD (KIPS)	FACTORED RESISTANCE AVAILABLE (KIPS)	ESTIMATED PILE LENGTH (FT.)
					SIDE RESIST. (KIPS)	END BRG. RESIST. (KIPS)	TOTAL RESIST. (KIPS)					
729.90	0.60	1.50	4	Hard Till	3.8	26.8		27	0	0	15	3
727.40	2.50	1.50	4		15.8	50.2		50	0	0	28	5
724.90	2.50	2.00	5		19.1	30.6	69.3	69	0	0	38	8
722.40	2.50	2.00	7		19.1	30.6	96.0	96	0	0	53	10
719.90	2.50	2.50	8		22.1	38.3	170.2	170	0	0	94	13
717.40	2.50	5.90	20		34.1	90.4	242.5	243	0	0	133	15
714.40	3.00	8.40	27		40.9	128.6	365.7	366	0	0	201	18
711.90	2.50	32			18.4	211.0	295.7	296	0	0	163	21
709.40	2.50	8.00	24		34.1	122.5	288.5	288	0	0	159	23
707.40	2.00	5.30	20		27.3	81.2	305.0	305	0	0	168	25
704.90	2.50	4.60	19	Sandy Gravel	34.1	70.5	331.4	331	0	0	182	28
702.40	2.50	4.10	19		31.7	62.8	792.5	793	0	0	436	39
699.90	2.50	56			153.3	492.2	517.9	518	0	0	285	33
697.40	2.50	4.20	20		32.3	64.3	579.3	579	0	0	319	35
694.90	2.50	6.10	24		34.1	93.4	613.4	613	0	0	337	38
692.40	2.50	6.10	25		34.1	93.4	647.4	647	0	0	356	40
689.90	2.50	6.10	25		34.1	93.4	670.8	671	0	0	369	43
687.40	2.50	5.40	21		34.1	82.7	703.3	703	0	0	387	45
684.90	2.50	5.30	15		34.1	81.2	734.4	734	0	0	404	48
682.40	2.50	5.10	19		34.1	78.1	1015.5	1016	0	0	559	50
679.90	2.50	37		Sandy Gravel	63.2	325.2	1096.3	1096	0	0	603	53
677.40	2.50	39			70.4	342.8	1254.6	1255	0	0	690	55
675.40	2.00	49			430.7							

Pile Design Table for North Abutment utilizing Boring #2

Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)	Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)	Nominal Required Bearing (Kips)	Factored Resistance Available (Kips)	Estimated Pile Length (Ft.)
Metal Shell 16"Φ w/.312" walls								
96	53	10	122	67	18	97	53	13
170	94	13	133	73	21	139	76	15
243	133	15	152	84	23	174	95	18
288	159	23	159	88	25	187	103	21
305	168	25	172	95	28	218	120	23
331	182	28	214	117	33	246	135	25
Metal Shell 16"Φ w/.375" walls								
96	53	10	249	137	35	263	144	28
170	94	13	266	147	38	321	177	33
243	133	15	284	156	40	381	210	35
288	159	23	295	162	43	406	223	38
305	168	25	311	171	45	430	237	40
331	182	28	327	180	48	441	243	43
Metal Shell 14"Φ w/.25" walls								
80	44	10	359	197	50	463	255	45
139	76	13	372	205	53	484	266	48
198	109	15	401	221	55	537	295	50
244	134	23	Steel HP 12 X 53					
259	143	25	113	62	15	558	307	53
283	156	28	143	79	18	Steel HP 14 X 89		
Metal Shell 14"Φ w/.312" walls								
80	44	10	154	85	21	101	55	13
139	76	13	180	99	23	143	79	15
198	109	15	198	109	25	177	97	18
244	134	23	211	116	28	Steel HP 14 X 102		
259	143	25	261	143	33	192	106	21
283	156	28	306	168	35	222	122	23
Steel HP 8 X 36								
120	66	25	327	180	38	250	137	25
131	72	28	348	191	40	266	146	28
164	90	33	358	197	43	326	179	33
190	104	35	378	208	45	387	213	35
204	112	38	396	218	48	411	226	38
218	120	40	Steel HP 12 X 63					
227	125	43	117	64	15	436	240	40
241	132	45	147	81	18	447	246	43
253	139	48	158	87	21	469	258	45
277	152	50	184	101	23	Steel HP 12 X 74		
Steel HP 10 X 42			200	110	25	490	270	48
119	65	18	214	117	28	544	299	50
129	71	21	263	145	33	565	311	53
148	82	23	310	170	35	616	339	55
155	86	25	330	182	38	Steel HP 14 X 117		
168	92	28	351	193	40	103	57	13
209	115	33	362	199	43	147	81	15
243	134	35	381	210	45	179	98	18
260	143	38	399	220	48	196	108	21
277	153	40	441	243	50	225	124	23
288	158	43	458	252	53	252	139	25
304	167	45	496	273	55	270	148	28
319	176	48	Steel HP 12 X 84					
203	112	25	330	182	38	330	181	33
217	119	28	351	193	40	392	216	35
267	147	33	362	199	43	417	229	38
315	173	35	381	210	45	442	243	40
336	185	38	399	220	48	453	249	43
357	196	40	441	243	50	476	262	45
368	202	43	458	252	53	496	273	48
387	213	45	496	273	55	551	303	50
405	223	48	504	277	55	573	315	53
448	246	50	Steel HP 14 X 102					
465	256	53	203	112	25	625	343	55
504	277	55	217	119	28	Steel HP 14 X 117		
Steel HP 12 X 84								
86	48	13	267	147	33	106	58	13
123	68	15	315	173	35	152	83	15
150	83	18	336	185	38	182	100	18
165	91	21	357	196	40	201	111	21
189	104	23	368	202	43	229	126	23
206	114	25	387	213	45	256	141	25
220	121	28	405	223	48	274	151	28
271	149	33	448	246	40	334	184	33
319	176	35	459	252	43	397	219	35
341	187	38	482	265	45	423	232	38
362	199	40	503	277	48	448	246	40
373	205	43	558	307	50	482	265	45
393	216	45	580	319	53	503	277	48
411	226	48	633	348	55	Precast 14"x 14"		
454	250	50	Timber Pile					
472	260	53	102	56	10	102	56	10
512	281	55	177	97	13	177	97	13
			252	139	15	252	139	15
			97	53	13			
			138	76	15			