

INTERIOR GIRDER MOMENT TABLE	
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I_s	(in ⁴) 23,988
$I_c(n)$	(in ⁴) 68,303
$I_c(3n)$	(in ⁴) 47,609
$I_c(cr)$	(in ⁴) —
S_s	(in ³) 1,311
$S_c(n)$	(in ³) 1,783
$S_c(3n)$	(in ³) 1,642
$S_c(cr)$	(in ³) —
$DC1$	(kip') 1.01
M_{DC1}	('k) 1,730.8
$DC2$	(kip') 0.115
M_{DC2}	('k) 196.8
DW	(kip') 0.35
M_{DW}	('k) 598.9
$M_L + IM$	('k) 2,011
M_u (Strength I)	('k) 6,827.6
$\phi_f M_n$	('k) 8,474.4
$f_s DC1$	(ksi) 15.84
$f_s DC2$	(ksi) 1.44
$f_s DW$	(ksi) 4.38
$f_s (L+IM)$	(ksi) 13.53
f_s (Service II)	(ksi) 39.25
$0.95 R_h F_y r$	(ksi) 47.5
f_s (Total) (Strength I)	(ksi) 51.85
$\phi_f F_n$	(ksi) —
V_f	(k) 58.2

INTERIOR GIRDER REACTION TABLE	
Abut.	
R_{DC1}	(k) 59.2
R_{DC2}	(k) 6.7
R_{DW}	(k) 20.5
$R_L + IM$	(k) 113.7
R_{Total}	(k) 200.0

I_s , S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).

$I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to short-term composite live loads (in.⁴ and in.³).

$I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

$I_c(cr)$, $S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite dead loads (in.⁴ and in.³).

$DC1$: Un-factored non-composite dead load (kips/ft.).

M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).

$DC2$: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW : Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

$M_L + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

1.25 ($M_{DC1} + M_{DC2}$) + 1.5 M_{DW} + 1.75 $M_L + IM$

$\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

$f_s DC1$: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

M_{DC1} / S_n

$f_s DC2$: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

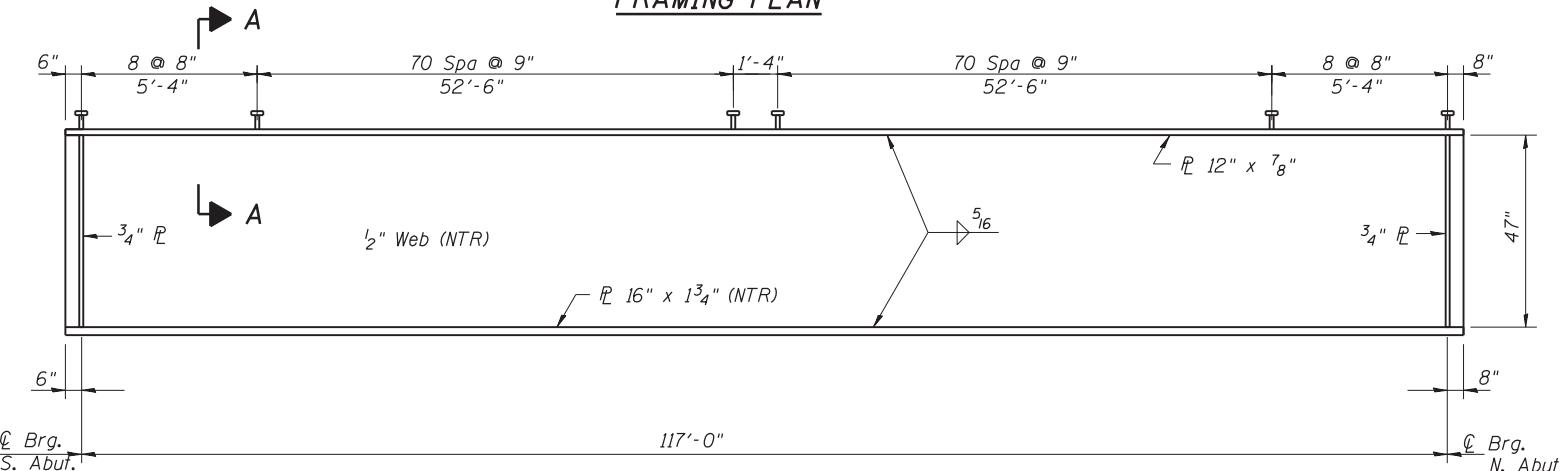
$f_s DW$: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

$M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

$f_s (L+IM)$: Un-factored stress at edge of flange for controlling steel flange due to vertical composite live plus impact loads as calculated below (ksi).

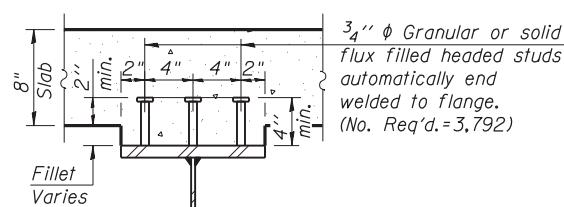
$M_L + IM / S_c(n)$ or $M_L + IM / S_c(cr)$ as applicable.

FRAMING PLAN



GIRDER ELEVATION

"NTR" denotes plates to which notch toughness requirements are applicable.



SECTION A-A

Notes:

- All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
- Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.

f_s (Service II): Sum of stresses as computed below (ksi).

$f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (L+IM)$

0.95 $R_h F_y r$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total) (Strength I): Sum of stresses as computed below on non-compact section (ksi).

1.25 ($f_s DC1 + f_s DC2$) + 1.5 $f_s DW$ + 1.75 $f_s L + IM$

$\phi_f F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7.2 (ksi).

V_f : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN
SB MANNHEIM ROAD BRIDGE - STRUCTURE NO. 016-7942
SHEET NO. 5-18 OF 27 SHEETS

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	HEET NO.
330	0105 WRS&HB	COOK	605	380
		CONTRACT NO.	60637	ILLINOIS FED. AID PROJECT