

Benchmark: Rail Road Spike in power pole Sta. 1115+74.96, 35.60' L1.
Elev. = 792.14
Existing Structures: SN 050-2544 Built in 1924 as a single 9' x 6' R.C. box culvert, 41'-0" face to face of curb with culvert length of 42'-10 3/4", 17° L1 fwd skew. Traffic will be maintained utilizing detour.
No salvage.

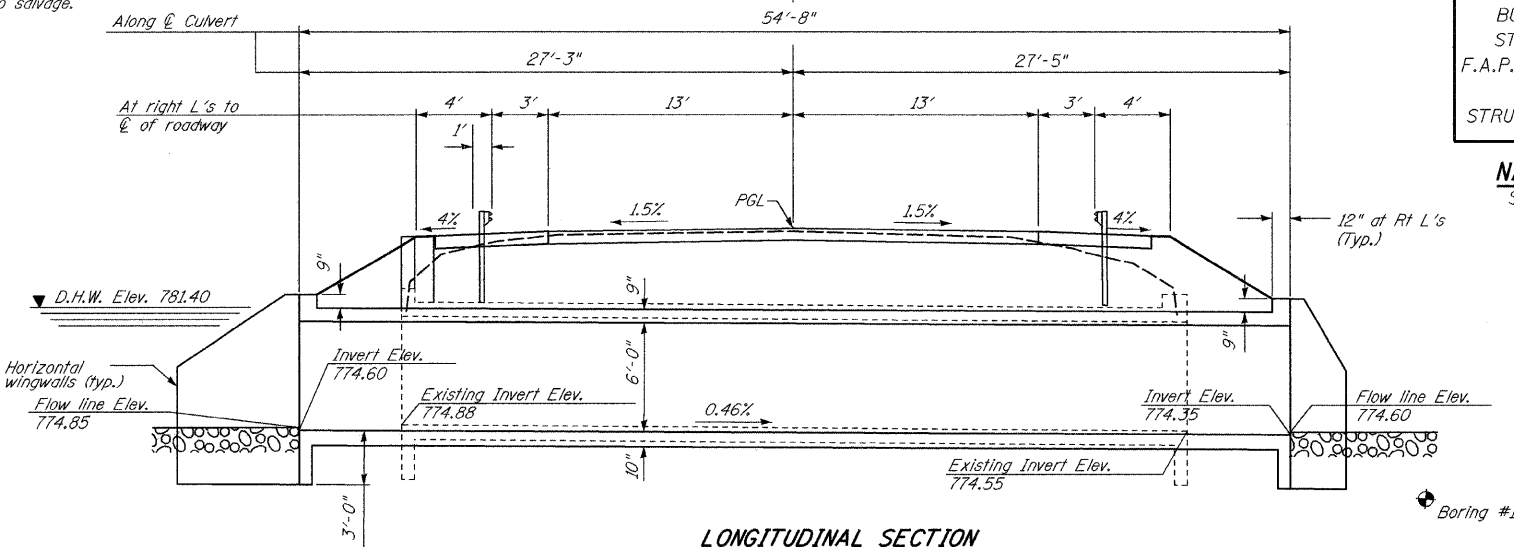
DESIGN SCOUR ELEVATION TABLE

Design Scour Elevation (ft.)	D.S. Invert	U.S. Invert
	771.35	771.60

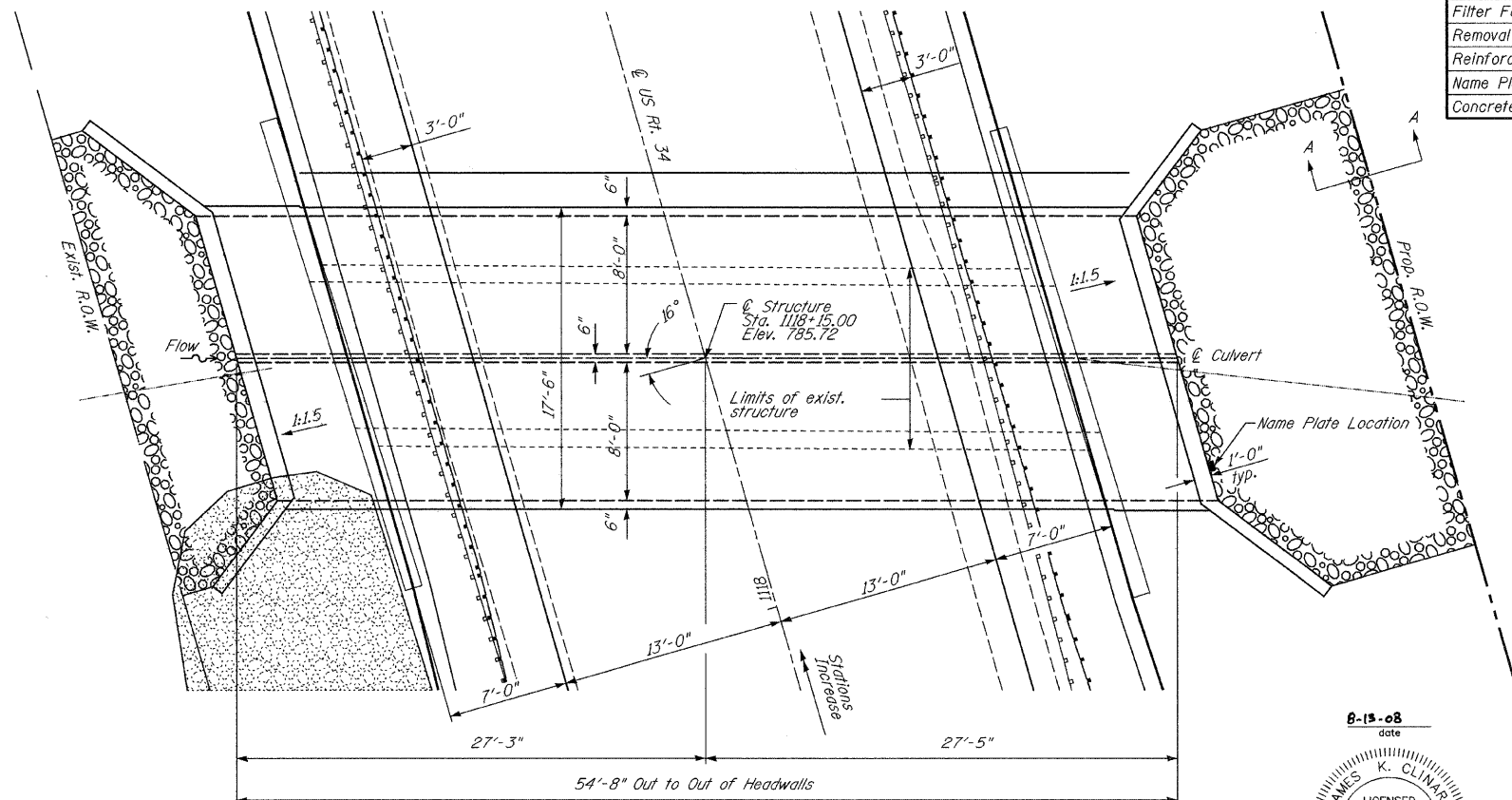
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

STATION 1118+15.00
BUILT 20__ BY
STATE OF ILLINOIS
F.A.P. 587 SEC.(22)I & I-1
LOADING HS20
STRUCTURE NO. 050-2051

NAME PLATE See Std. 515001



LONGITUDINAL SECTION



PLAN

DESIGNED	JKC
CHECKED	JLS
DRAWN	NV
CHECKED	JKC

Boring #2

APPROVED
FOR STRUCTURAL ADEQUACY ONLY
Ralph E. Anderson
ENGINEER OF BRIDGES AND STRUCTURES



TOTAL BILL OF MATERIALS		
Description	Unit	Total
Stone Riprap Class A4	Sq. Yd.	57
Filter Fabric	Sq. Yd.	57
Removal of Existing Structures No. 1	Each	1
Reinforcement Bars	Pound	12700
Name Plates	Each	1
Concrete Box Culverts	Cu. Yd.	84.0

INDEX OF CULVERT PLANS

- General Plan & Elevation
- Culvert Details
- Soil Borings

GENERAL NOTES

- Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions.
- Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- Precast alternate is not allowed.
- The contractor is advised that the existing box culvert is in a deteriorated condition with reduced load carrying capacity. It is the contractor's responsibility to account for the condition of the culvert when developing construction procedures for the removal and replacement of the culvert.

LOADING HS20-44

Allow 50#/sq. ft. for future wearing surface

DESIGN SPECIFICATIONS

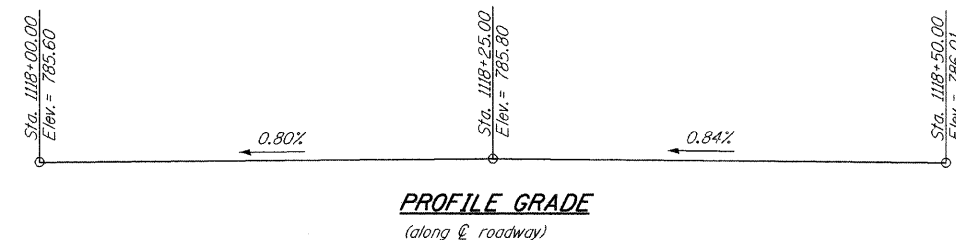
2002 AASHTO

DESIGN STRESSES

FIELD UNITS

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (reinforcement)

CHAMLIN & ASSOCIATES
PERU ILLINOIS MORRIS



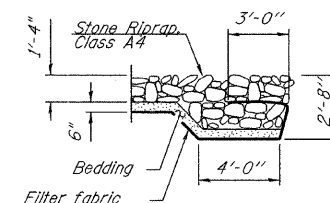
PROFILE GRADE (along roadway)

WATERWAY INFORMATION

Drainage Area= 0.77 Sq. Mi. Exist. Low Grade El. 785.41 @ Sta. 1118+00 Prop. Low Grade El. 785.60 @ Sta. 1118+00									
Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.		NAT. H.W.E.	Head - Ft.		Headwater El.	
			Exist.**	Prop.**		Exist.	Prop.	Exist.	Prop.
	10	411	48	89	780.2	1.1	0.0	781.3	780.2
Design	50	713	54	96	781.4	4.0	0.0	785.4	781.4
Base	100	855	54	96	781.7	4.4	0.4	786.2	782.1
Overtopping	-	-	-	-	-	-	-	-	-
Max. Calc.	500	1213	54	96	782.2	4.7	2.2	786.9	784.4

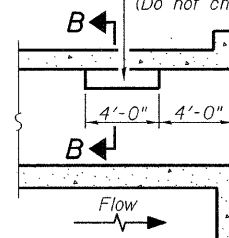
10-year velocity through exist. struct. = 10.0 fps
** Gross opening = 54 sq. ft.(exist.) 96 sq. ft.(prop.)

10-year velocity through prop. struct. = 5.4 fps

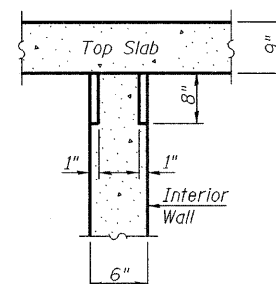


SECTION A-A

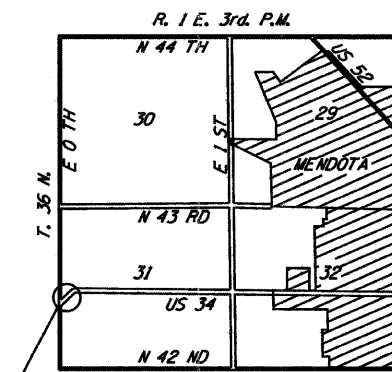
Notch formed by rough finished board attached to and removed with form work, each interior wall. (Do not chamfer).



LONGITUDINAL SECTION



SECTION B-B



PROPOSED STRUCTURE LOCATION SKETCH

GENERAL PLAN & ELEVATION
US 34 OVER
TRIBUTARY TO SPRING CREEK
FAP 587 - SECTION (22) I & I-1
LASALLE COUNTY
STA. 1118+15.00
SN 050-2051

SHEET NO. 1	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	587	(22) I & I-1	LASALLE	31	15
3 SHEETS		SN 050-2051	CONTRACT NO. 66835		
		FED. ROAD DIST. NO. _	ILLINOIS	FED. AID PROJECT	