

Is, Ss:	Non-composite moment of inertia and section modulus of the steel section used for computing $f_{\mathcal{S}}$ (Total-Strength I, and
(n), S <sub>c</sub> (n):	Service II) due to non-composite dead loads (in. <sup>4</sup> and in. <sup>3</sup> ). Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing $f_s$ (Total-Strength I, and Service II) due to short-term composite live loads (in. <sup>4</sup> and in. <sup>3</sup> ).
n), Sc(3n):	Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing $f_s$ (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in. <sup>4</sup> and in. <sup>3</sup> ).
DC1:	Un-factored non-composite dead load (kips/ft.).
MDC1:	Un-factored moment due to non-composite dead load (kip-ft.).
DC2:	Un-factored long-term composite (superimposed excluding
	future wearing surface) dead load (kips/ft.).
MDC2:	Un-factored moment due to long-term composite (superimposed
	excluding future wearing surface) dead load (kip-ft.).
DW:	Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
Mow:	Un-factored moment due to long-term composite (superimposed
	future wearing surface only) dead load (kip-ft.).
M4 + Imp:	Un-factored live load moment plus dynamic load allowance
ing inp	(impact) (kip-ft.),
rength D:	Factored design moment (kip-ft.).
5	1.25 (M <sub>DC1</sub> + M <sub>DC2</sub> ) + 1.5 M <sub>DW</sub> + 1.75 M <sub>L + Imp</sub>
$\phi_f M_n$ :	Compact composite positive moment capacity computed
	according to Article 6.10.7.1 (kip-ft.).
Øf Mnc :	Compact non-composite negative moment capacity computed
	according to Article A6.1.1 (kip-ft.).
ervice II):	Sum of stresses as computed from the moments below (ksi).
	M <sub>DC1</sub> + M <sub>DC2</sub> + M <sub>DW</sub> + 1.3 M4 + Imp
rength D:	
	non-compact section (ksi).
	1.25 (M <sub>DC1</sub> + M <sub>DC2</sub> ) + 1.5 M <sub>DW</sub> + 1.75 M¼ + Imp
Vr:	Factored shear range computed according to Article 6.10.10.

1	
Pier 1 or 2	0.5 SP 2
11600	11600
-	26279
-	19275
686	686
~	899
-	822
0.962	0.962
606	336
0,150	0.150
81	63
0.358	0.358
193	150
696	975
2366	2428
-	<u>3948</u> 5.9
10.6	5.9
1.4	0.9
3.4	2.2
15.8	16.9
31.2	25,9
41.4	-
-	22.2

INTERIOR GIRDER REACTION TABLE						
HL93 Loading						
		Abut.	Pier 1 or 2			
R DCI	(k)	24.4	85.4			
R <sub>DC2</sub>	(k)	3.9	12.9			
Row	(k)	9,3	30.7			
R4 + Imp	(k)	76.0	125.6			
RTotal	(k)	113,6	254.6			

STRUCTURAL STEEL &

_	20.9					
4	-	BEARING DETAILS				
	22.2	STRUCTU	JRE NUMBEI	7 <i>061</i> -	<u>0094</u>	
F.A.P. RTE.		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
322		25BR-1	MARION	43	39	

NO.16	F.A.P. RTE.	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
	322	25BR-1			MARION	43	39
ETS	SN 061-0094			CONTRACT	NO. 76	A83	
	FED. R	DAD DIST. NO.	8 ILLINOIS	FED. A	ID PROJECT 322		