

ELEVATION AT ABUTMENTSECTION A-A

TOP OF BEAM ELEVATIONS

(For Fabrication Only)

Beam No.	Q Brdg. N. Abut.	Q Brdg. S. Abut.
1	504.516	504.538
2	504.632	504.646
3	504.726	504.732
4	504.814	504.811
5	504.817	504.807
6	504.738	504.720
7	504.651	504.625
8	504.543	504.509

Notes:

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 ($F_y=36\text{ksi}$). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

All bearing plates shall conform to the requirements of AASHTO M 270, Grade 50.

FIXED BEARING

Notes:

Anchor bolts at fixed bearings may be built into the masonry.

INTERIOR GIRDER MOMENT TABLE 0.5 Span	
I_s	(in ⁴) 8,230
$I_c(n)$	(in ⁴) 18,447
$I_c(3n)$	(in ⁴) 13,427
S_s	(in ³) 541
$S_c(n)$	(in ³) 728
$S_c(3n)$	(in ³) 658
M_{DC1}	(kip) 0.76
M_{DC1}	(kip) 568
M_{DC2}	(kip) 0.11
M_{DC2}	(kip) 82
M_{DW}	(kip) 0.25
M_{DW}	(kip) 187
M_{L+IM}	(kip) 974
M_u (Strength I)	(kip) 2,798
$\phi_f M_n$	(kip) 3,360
$f_s DC1$	(ksi) 12.6
$f_s DC2$	(ksi) 1.5
$f_s DW$	(ksi) 3.4
$f_s 1.3(L+IM)$	(ksi) 20.9
f_s (Service II)	(ksi) 38.4
V_f	(kip) 22.6

I_s , S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).

$I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in.⁴ and in.³).

$I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

$DC1$: Un-factored non-composite dead load (kips/ft.).

M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).

$DC2$: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW : Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{L+IM} : Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

1.25 ($M_{DC1} + M_{DC2}$) + 1.5 M_{DW} + 1.75 M_{L+IM}

$\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

f_s (Service II): Sum of stresses as computed from the moments below (ksi).

$M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_{L+IM}$

V_f : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

BILL OF MATERIAL

Item	Unit	Total
Anchor Bolts, 1"	Each	32

INTERIOR GIRDER REACTION TABLE HL93 Loading	
	Abutment
R_{DC1}	(kip) 29.4
R_{DC2}	(kip) 4.4
R_{DW}	(kip) 9.7
R_{L+IM}	(kip) 67.8
R_{Total}	(kip) 111.3

DESIGNED	B.G.H.
CHECKED	L.D.G.
DRAWN	K.H.L.
CHECKED	B.G.H.

SHEET NO. 16 21 SHEETS	F.A.P. RTE. 328	SECTION (4BR-1B)	COUNTY CLAY	TOTAL SHEETS	SHEET NO.
				42	34
				S.N. 013-0039	CONTRACT NO. 74310
				FED. ROAD DIST. NO. ILLINOIS	FED. AID PROJECT