-ÇW. Abut. |**→**¢ Pier 2 - ⊊ Pier 1 - ⊈ €. Brg. Pier 1 Ç W. Brg. Pier - C E. Brg. Pier 2 © W. Brg. Pier 2-23′-9<sup>5</sup>8″ Bk. W. Abut.-23'-958' 21'-5'8' & End of Deck (1)Diaph 2 gge -3" 3 ò (4)0 S (5) 97 S 6 611 6'1 44 6¼" Typ. Ea. Abut. 9" Typ. Ea, Abut. 22'-0" 22'-0" 24'-4'2 24'-4'2" 25'-0" 24'-4'2 24'-4'2" 48'-9" 48'-9" 69'-0" 70′-0" (Span 2) 50′-0″ (Span 1) 50′-0" (Span 3)

FRAMING PLAN

- I: Non-composite moment of inertia of beam section (in.<sup>4</sup>).
- I': Composite moment of inertia of beam section (in.4).  $S_{\textit{b:}}\xspace$  Non-composite section modulus for the bottom fiber of
- the prestressed beam (in.<sup>3</sup>).

Ν

- $S_{b}$ : Composite section modulus for the bottom fiber of the prestressed beam (in.<sup>3</sup>).
- St: Non-composite section modulus for the top fiber of the prestressed beam (in.<sup>3</sup>).
- St': Composite section modulus for the top fiber of the prestressed beam (in.<sup>3</sup>).
- DC1: Un-factored non-composite dead load (kips/ft.).
- M<sub>DC1</sub>: Un-factored moment due to non-composite dead load
- (kip-ft.). DC2: Un-factored long-term composite (superimposed excluding
- future wearing surface) dead load (kips/ft.). MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
- DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
- Mow: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft).
- M4 + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

INTERIOR BEAM MOMENT TABLE						
		0.4 Sp. 1 0.6 Sp. 3	Pier 1 or 2	0.5 Sp. 2		
Ι	(in4)	90,956	-	90,956		
I'	(in4)	282,060	-	282,060		
Sb	(in <sup>3</sup> )	5153	-	5153		
Sb'	(in <sup>3</sup> )	8822	-	8822		
St	(in <sup>3</sup> )	3736	-	3736		
St'	(in <sup>3</sup> )	28,130	-	28,130		
DC1	(k/′)	1.16	-	1.16		
MDC1	('k)	330.8	-	670.5		
DC2	(k/′)	0.15	0.15	0.15 35.9 0.325		
M DC2	('k)	20.4	56.0			
DW	(k/′)	0.325	0.325			
Mow	(′k)	44.2	121.3	77.8		
M4_ + IM	(′k)	538.3	551.4	617.3		

INTERIOR BEAM REACTION TABLE						
HL93 Loading	Abut.	Pier 1 Span 1 Pier 2 Span 3	Pier 1 Span 2 Pier 2 Span 2			
R DC1 (k)	28.3	28.3	40.6			
R DC2 (k)	2.5	5.3	5.3			
Row (k)	5.4	11.4	11.4			
R Ł + IM (k)	64.6	50.2	50.2			
R Total (k)	100.8	95,2	107.5			

\* The total RDc2, RDw and RL + IM are assumed to be distributed evenly to each bearing line at a pier regardless of the span ratios. The bearing design at a pier is based on the maximum reactions of either span.

		<sup>I3</sup> l6 ′′ x 1 <sup>7</sup> 8′′ ver / slotted holes in / equivalent Bent
	1yp.	l <sup>3</sup> <sub>16</sub> ′′ x 1 <sup>7</sup> 8 ′′ hc holes in channe
Æ 4	" x 4" x <sup>3</sup> g", typ.	
	typ.	L <u>6 x 6 x <sup>3</sup>g'' or en</u> Bent <u>P</u> +/- <u>1'-7'' lo</u> Exterior Beam

## Notes:

All material for bracing shall be hot dip galvanized according to AASHTO M111 unless otherwise noted. Two hardened washers are required for each set of oversized holes.

All holes shall be  ${}^{15}_{16}$  '' $\phi$  unless otherwise noted.

5<sub>16</sub> " x 3" x 3" plate washers are required over all slotted holes.

All bolts shall be galvanized according to AASHTO M232. Bracing shall be installed as beams are erected and tightened as soon as possible during erection.

## PERMANENT BRACING DETAILS

USER NAME:	DESIGNED - RTM	REVISED -				
	DRAWN - MSD	REVISED -	STATE OF ILLINOIS		FRAMING PL	.AN
PLOT SCALE:	CHECKED - KEB	REVISED -	DEPARTMENT OF TRANSPORTATION			
PLOT DATE:	DATE -	REVISED -		SCALE;	SHEET NO. 16 OF 27 SHEETS	STA.

SODEMANN AND ASSOCIATES, INC.



109B-3

ILLINOIS

717

FED. ROAD DIST. NO.

TO STA.

LOGAN

FED. AID PROJECT

51

73

CONTRACT NO. 72A88