# STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

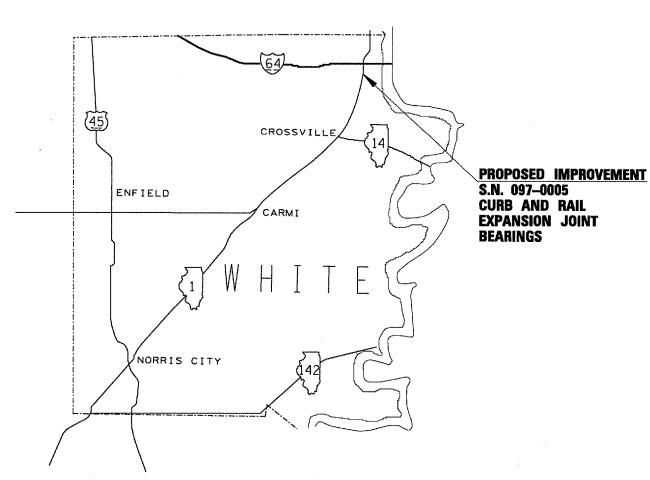
# PROPOSED HIGHWAY PLANS

F.A.P. 332 (ILL. RTE. 1)

SECTION D9 CM BRIDGE REPAIR FY 08–1

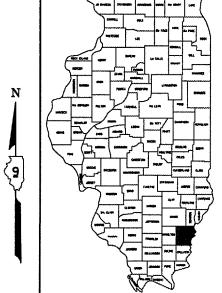
C-99-040-07

WHITE COUNTY



MAP NOT TO SCALE

\* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

Many C. Lamie FGENGIN

Eric E. Harn 10

ENGINEER OF DESIGN AND ENVIRONMEN

DIRECTOR, DIVISION OF HIGHWAYS

PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

## INDEX OF SHEETS

ILLINOIS TOWNSHIP: GRAY 2005 ADT = 3500

**OPERATING RATING HS 38.9** 

**BAR SPLICERS** 

13. BRIDGE RAIL

TRUCKS = 17% ADT
POSTED SPEED = 55 MPH
INVENTORY RATING HS 26.7

HEET NO.	DESCRIPTION
1.	COVER, INDEX OF SHEETS, STANDARDS
2.	GENERAL NOTES
3.	SUMMARY OF QUANTITIES
4.	STAGES OF CONSTRUCTION
5.	CURB AND RAIL REPLACEMENT
6.	CONCRETE REMOVAL
7.	CONCRETE SUPERSTRUCTURE PLAN VIEW
8.	CONCRETE SUPERSTRUCTURE SECTION.
	BILL OF REINFORCEMENT
9.	EXPANSION JOINT DETAILS
10.	FLOOR DRAINS
11.	BEARINGS

#### **STANDARDS**

630001-07	STEEL PLATE BEAM GUARDRAIL
631032- <i>03</i>	TRAFFIC BARRIER TERMINAL TYPE 6A
701321- <i>08</i>	LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
701301-02	LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
702001- <i>0</i> 4	TRAFFIC CONTROL DEVICES
704001- <i>03</i>	TEMPORARY CONCRETE BARRIER
70/201-02	•

#### PLAN DRAWINGS ARE NOT TO SCALE.

J.U.L.I.	E.				
JOINT	UTILITY	LOCATION	INFORMATION	FOR	<b>EXCAVATION</b>
1800-	-892-012	3			

**CONTRACT NO. 78015** 

#### GENERAL NOTES

WHILE SIGNAL HEADS ARE MOUNTED IN PLACE, BUT NOT YET IN OPERATION, THEY SHALL BE SECURELY COVERED IN WHITE PLASTIC.

THE ADVANCE DETECTOR LOOPS ARE TYPICALLY LOCATED 300 FEET IN ADVANCE OF THE STOP BAR. THE BUREAU OF OPERATIONS SHOULD APPROVE THE LOOP LOCATIONS PRIOR TO INSTALLATION.

THE QUANTITY OF WORK ZONE PAVEMENT MARKING REMOVAL INCLUDES REMOVAL OF EXISTING CENTERLINE PAVEMENT MARKING FROM THE STOP BARS TO ATTENUATORS, REMOVAL OF EDGE LINE IN STAGE I, AND REMOVAL OF TEMPORARY EDGE LINES.

TRAFFIC CONTROL SIGNS SHALL BE PLACED SO THAT THEY DO NOT INTERFERE WITH EXISTING SIGNS OR FLASHING BEACONS. THE DIMENSIONS BETWEEN SIGNS MAY BE MODIFIED SLIGHTLY SO AS TO AVOID CONFLICTS WITH EXISTING SIDEROADS, COMMERCIAL ENTRANCES, AND PRIVATE ENTRANCES. THE BUREAU OF OPERATIONS SHOULD APPROVE FINAL PLACEMENT OF TRAFFIC CONTROL SIGNING.

PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING PLANS ARE SUBJECT TO ROUTINE VARIATIONS. THE CONTRACTOR SHALL FIELD VERIFY EXISTING DIMENSIONS AND DETAILS AFFECTING NEW CONSTRUCTION AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OR ORDERING OF MATERIALS. SUCH VARIATIONS SHALL NOT BE CAUSE FOR ADDITIONAL COMPENSATION FOR A CHANGE IN SCOPE OF THE WORK, HOWEVER, THE CONTRACTOR WILL BE PAID FOR THE QUANTITY ACTUALLY FURNISHED AT THE UNIT PRICE BID FOR THE WORK.

THE COST OF ANY SAW CUTS MADE TO COMPLETE THE WORK AS DESCRIBED IN PLAN DETAILS SHALL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS INVOLVED.

PRIOR TO POURING THE NEW PORTIONS OF THE CONCRETE DECK, ALL HEAVY OR LOOSE RUST, LOOSE MILL SCALE, OR OTHER LOOSE OR POTENTIALLY DETRIMENTAL FOREIGN MATERIAL SHALL BE REMOVED FROM THE SURFACES IN CONTACT WITH CONCRETE. THE REMOVAL SHALL BE ACCOMPLISHED BY METHODS WHICH WILL NOT DAMAGE THE STEEL. THE COST IS INCLUDED WITH "CONCRETE REMOVAL".

EXISTING STEEL RAILING, CONNECTION ANGLES, AND PLATES SHALL BE REMOVED. FILL HOLES IN EXISTING BEAM WITH  $\frac{7}{4}$ "  $\phi$  H.S. BOLTS. COST INCLUDED WITH "BRIDGE RAIL REMOVAL".

JOINT OPENING SHALL BE ADJUSTED ACCORDING TO ARTICLE 520.04 OF THE STANDARD SPECIFICATIONS WHEN THE DECK IS POURED AT AN AMBIENT TEMPERATURE OTHER THAN 50°.

THE CONTRACTOR SHALL REMOVE ALL DEBRIS FROM THE PIER CAP. PAYMENT FOR THIS WORK WILL BE DETERMINED ACCORDING TO ARTICLE 109.04 OF THE STANDARD SPECIFICATIONS.

ROUTE NO.	SECTION	GOUNTY		TOTAL CHESTS	GRAGET NO.
FAP 332	•	WHITE		13	2
PED, ROAD CHET, HO. 7		NTMORE	FED. AID PRO	Lagran-	

\* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015

Prepared By:	1 WVW / T (MMMV)
Examined By:	DISTRICT OPERATIONS ENGINEER
Examined By:	Carrie Nelson
Examined By:	DISTRICT PROGRAM DEVELOPMENT ENGINEER
Examined By:	DISTRICT STUDIES & PLANS ENGINEER
Examined By:	Brune W Rubles
Examined By:	DISTRICT MATERIALS ENGINEER
Examined By:	DISTRICT PROJECT IMPLEMENTATION ENGINEER
Approved By:	ASSISTANT REGIONAL ENGINEER  TOTAL  DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER
	August 17 20 07

ROUTE NO.	SECTION	600	жт	TOTAL	MCZ.
FAP 332		WHITE		13	3
PED. NOAD DIST.	NO. 7	#TIMORE	PRO AID PRO	West-	

\* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015

#### SUMMARY OF QUANTITIES

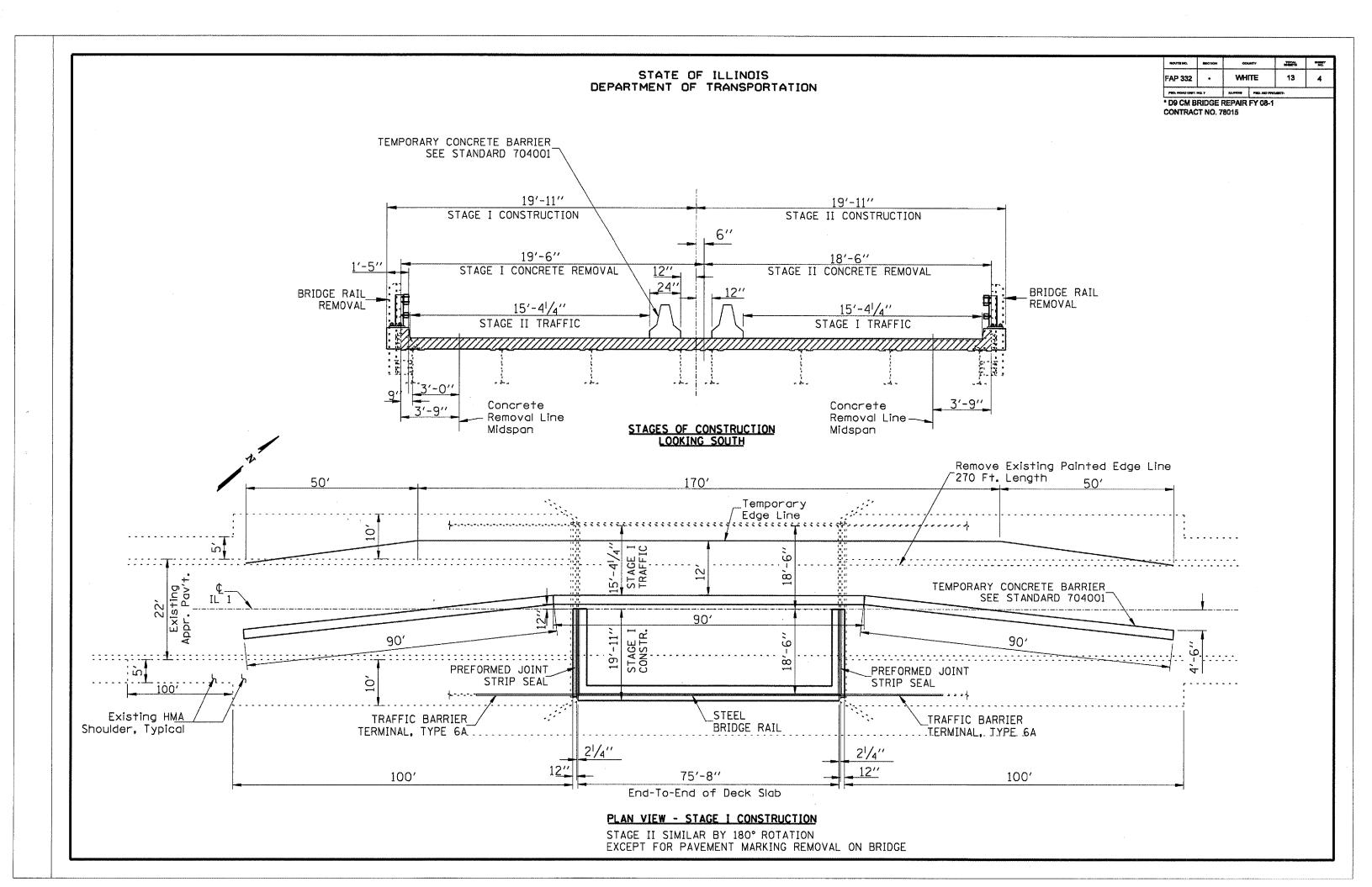
***************************************	100% STATE		
CODE NO.	ITEM DESCRIPTION	UNIT	TOTAL QUANTITY
42001300	PROTECTIVE COAT	SQ YD	114.2
50102400	CONCRETE REMOVAL	CU. YD	22,1
50104000	BRIDGE RAIL REMOVAL	FOOT	156.0
50300100	FLOOR DRAINS	EACH	8
50300255	CONCRETE SUPERSTRUCTURE	CU YD	33.2
50500405	FURNISHING AND ERECTING STRUCTURAL STEEL	POUND	1530
50500715	JACK AND REMOVE EXISTING BEARINGS	EACH	16
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	5630
X0325701	STEEL RAILING, TYPE 2399	FOOT	156.0
52000110	PREFORMED JOINT STRIP SEAL	FOOT	76
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	16
52100520	ANCHOR BOLTS, I"	EACH	32
63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	4
67100100	MOBILIZATION	L SUM	1
70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321	EACH	1
70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1
70301000	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	357

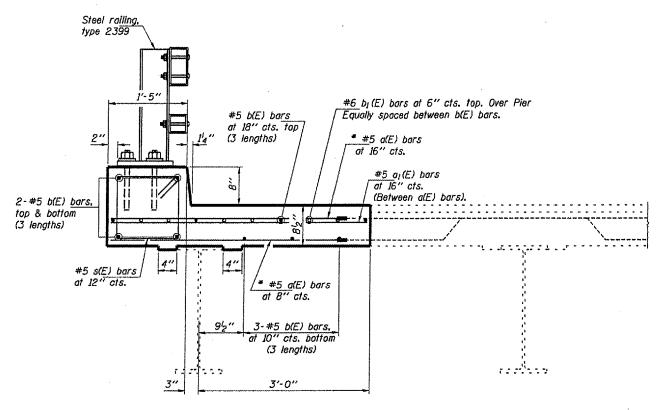
	100% STATE		
CODE NO.	ITEM DESCRIPTION	UNIT	TOTAL QUANTITY
70400100	TEMPORARY CONCRETE BARRIER	FOOT	270
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	270
78001110	PAINT PAVEMENT MARKING LINE - 4"	FOOT	688
X6333500	TRAFFIC BARRIER TERMINAL REMOVAL	EACH	4
X0321781	MECHANICAL SPLICE	EACH	285
70106800	CHANGEABLE MESSAGE SIGN	CAL MO	2
50800515	BAR SPLICERS	EACH	22 ·
Z0030250	IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2
Z0030350	IMPACT ATTENUATOR, RELOCATE (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2
	·		

\* SPECIALTY ITEMS

### PAINT PAVEMENT MARKING - LINE 4"

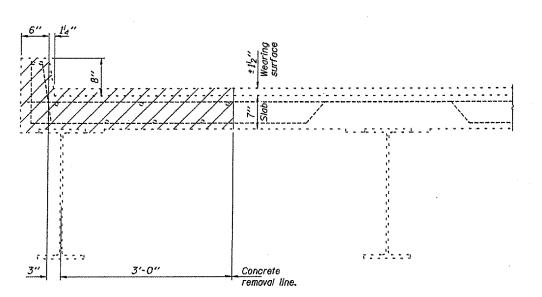
COLOR	QUANTITY (FOOT)
	·····
WHITE	540
	0.0
VELLOW	1.40
YELLOW	148
	· · · · · · · · · · · · · · · · · · ·
I TOTAL	688
1	000





#### PROPOSED REPLACEMENT

\*\* Attach to existing reinforcement with mechanical splicers. Existing reinforcement to extend 6" (min.) into the removal area to allow attachment of the mechanical splicers.



#### PROPOSED REMOVAL

Hatched area indicates concrete removal.

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FAP 332	•	WHITE		13	5
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\* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015

#### GENERAL NOTES

All structural steel shall conform to AASHTO Classification M-270 Gr. 36, unless otherwise noted.

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60 (IL Modified). See Special Provisions.

Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to routine variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished based upon the unit price bid for the work.

Cost of removal and re-installation of all members necessary to complete the work as detailed on the plans and as specified in the Special Provisions shall be included with Furnishing and Erecting Structural Steel.

Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

All structural steel shall be shop painted with the inorganic zinc rich primer per

AASHTO M300, Type I. Cost included with Furnishing and Erecting Structural Steel. Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.

If the analysis submitted by the Contractor for the jacking/temporary support system to be used shows temporary stiffeners are required to prevent web crippling or buckling, the stiffeners shall be steel and bolted to the web. If stiffeners are not required, hardwood timbers shall be installed tightly between the top and bottom flange to prevent flange rotation.

MINIMUM BAR LAPS

#5 Bar = 2'-2" #6 Bar = 2'-7"

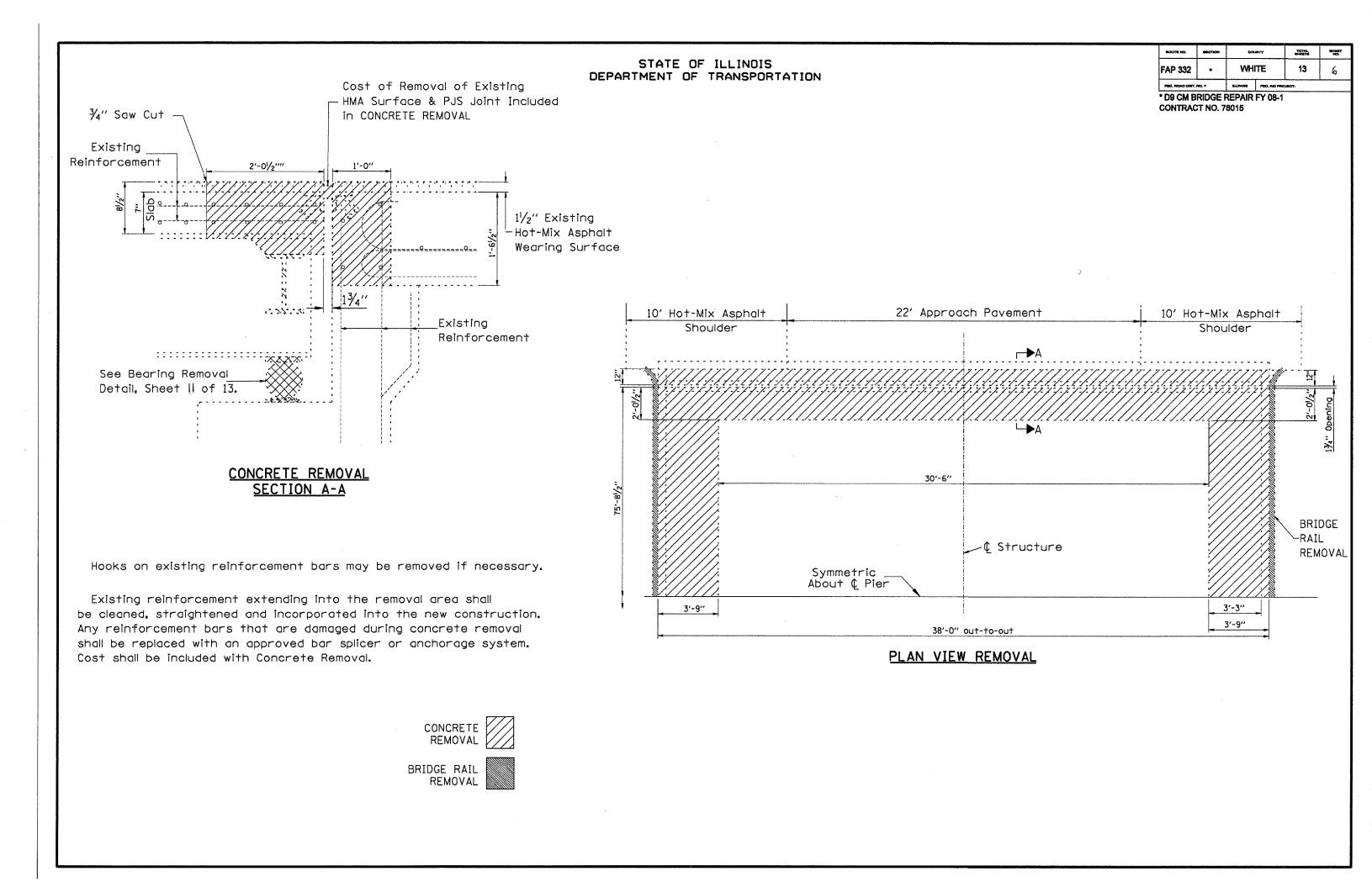
BRIDGE REPAIRS
FAP 332 OVER FRENCH CREEK
WHITE COUNTY
SN 097-0005

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TOTAL SHEETS STATE OF ILLINOIS FAP 332 WHITE DEPARTMENT OF TRANSPORTATION Some existing reinforcement not shown for clarity. KLINOW PROJECT \* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015 10' Hot-Mix Asphalt Shoulder 22' Approach Pavement 10' Hot-Mix Asphalt Shoulder 39'-10" Out-To-Out 1'-5" 37'-0" 1'-5" 38 - #5 x(E) @ 12" cts. 3 - Bar Splicers for #5 h(E) Bars -#5 h(E) bars **├** \_Existing Reinforcement /At Stage Construction Line (6 Total) 12" d(e) Bar-2′-0″ Stage Existing - #5 a(E) bars at 16" cts. Top - #5 a(E) at 8" cts. Each Stage Reinforcement at 16" cts., Top Each Stage Each 8-Bar Splicers For #5 a(E) Bars -At Stage Construction Line  $2 \times 3 - *5 b(E)$  Bars, (16 total) -Top & Bottom 56 - #5 a1 (E) bars (Between a(E) bars), Each Stage  $3 \times 3 - \#5$  b(E) Bars, Bot. a2(E) Bars Each Stage Existing Reinforcement 6 - #6 b1(E) bars at 6" cts., Top Over Pier, Ea Stage 3'-3" Symmetric About ¢ Pier Equally spaced between b(E) bars. 4'-8" 4 x 3 - #5 b(E) bars at 18" cts. Top Each Stage 1'-5" 3'-3" ¢ Structure 4'-8" See Section A-A, Sheet 8 of 13. Minimum Bar Laps #5 Bar = 2'-2" CONCRETE SUPERSTRUCTURE PLAN VIEW \*Attach to existing reinforcement with mechanical splicers. Existing reinforcement to extend 6" (min.) into the removal area to allow attachment of the mechanical splicers.

Existing

3 - #5 h(E) Bars

Each Stage

Reinforcement

Surface

Existing Hot-Mix

├ Asphalt Wearing

See Joint Detail, Sheet 9 of 13

Bonded Construction

Joint, Typical

Existing

Reinforcement

¢ New Brg.

Sheet II of 13

See Bearing Details-

2'-0"

#5 ×(E)

#5 a(E)

V.::\\$2::\

SECTION A-A

Bars

Bars

1¾″ © 50° F

1'-0"

ROLITE NO.	нестион	GOUNTY		TOTAL STEENS	WHERET NO.
FAP 332	•	WHITE		13	-8
PEG. PLOAD DIET.	MED, RIGAD DIRT. NO. 7		PIED. AND PINC		

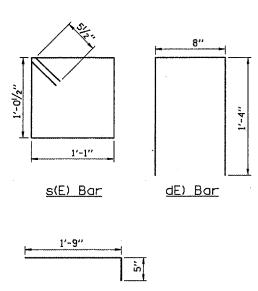
\* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015

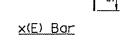
#### BILL OF REINFORCEMENT

Bar	No.	Size	Length	Shape
a(E)	285	#5	3′-11′′	
$a_{I}(E)$	112	#5	4'-5''	
0 <sub>2</sub> (E)	32	#5	19'-8''	
b(E)	66	#5	26'-7"	
b <sub>1</sub> (E)	12	#6	12'-3''	
d(E)	4	#4	3'-4''	
h(E)	12	#5	18'-9''	
s(E)	152	#5	5′-2′′	
x(E)	76	#5	2'-2"	
Mechan	nical Spl	icers	Each	285
Bar Sp			Each	22
Reinfor Epoxy	cement Coated	Bars,	Pound	5630

Reinforcement bars designated (E) shall be epoxy coated.

Bars indicated thus 2 x 3 - #5 etc. indicates 2 lines of bars with 3 lengths per line.





#### \* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std.

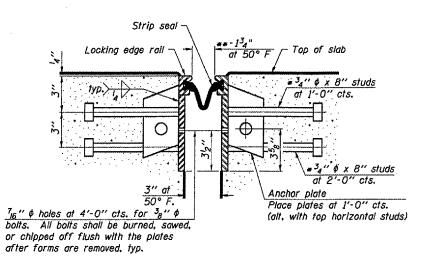
#### \*\* When joint is fixed, dimension is set at 12".

or chipped off flush with the plates

after forms are removed, typ.

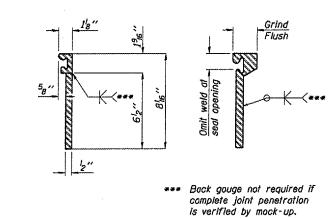
# Specs., automatically end welded.





## Strip seal - Top of slab Locking edge rail \*\*34" \$ x 8" studs at 1'-0" cts. at 2'-0" cts. \* 34" \$ x 8" studs at 1'-0" cts. (alt. with top horizontal studs). <sup>7</sup><sub>16</sub>" φ holes at 4'-0" cts, for <sup>3</sup><sub>8</sub>" φ bolts. All bolts shall be burned, sawed,

SECTION THRU ROLLED RAIL JOINT



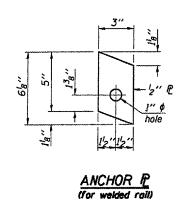
ROLLED (EXTRUDED) RAIL WELDED RAIL

#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.

## LOCKING EDGE RAILS

#### SECTION THRU WELDED RAIL JOINT



TOTAL SHEETS MARKET NO. MOLITHING. WHITE 13 FAP 332 ELIMONS PED, AID PROJECT

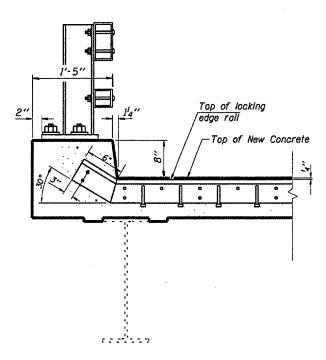
\* D9 CM BRIDGE REPAIR FY 08-1 **CONTRACT NO. 78015** 

The strip seal shall be made continuous and shall have a minimum thickness of 4". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spilced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed. The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

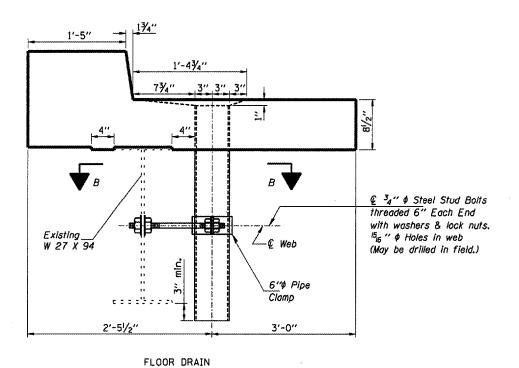
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

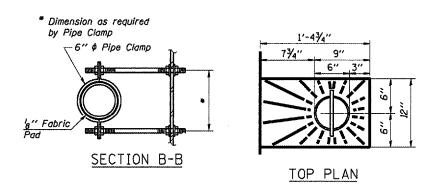


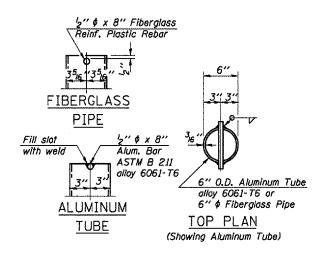
#### TYPICAL END TREATMENTS

#### BILL OF MATERIAL

1tem	Unit	I otal
Preformed Joint Strip Seal	Foot	76
Preformed Joint Strip Sedi	F 00T	76

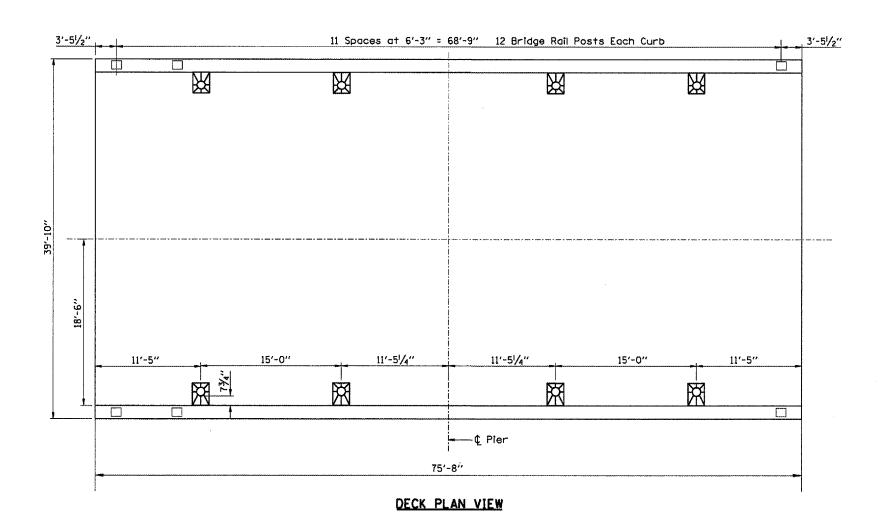


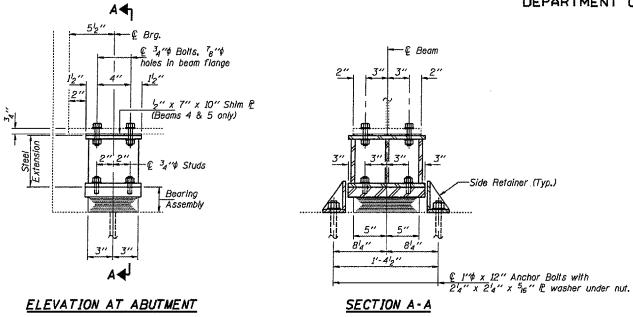




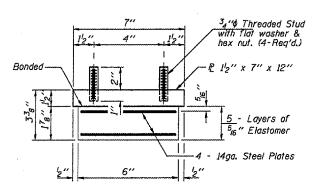
ROUTE NO.	SECTION	WHITE		MICTION COUNTY		TOTAL despite	BHEST NO.
FAP 332				13	10		
PROS. RICHAD SHIFT, NO. 7		BLINOR	PED AID PROJECT.				

\* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015



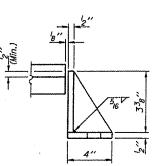


#### TYPE I ELASTOMERIC EXP. BRG.



#### BEARING ASSEMBLY

Shim plates shall not be placed under Bearing Assembly.



## SIDE RETAINER

–*€ 1'4''*¢ Hole

#### BEAM REACTIONS

***************************************		
R₽	(K)	11.7
R4	(K)	26.5
Imp.	(K)	8.0
R (Total)	(K)	46.2

Notes:
Diaphragm removal and reinstallation may be required to facilitate drilling holes. Cost included with Furnishing and Erecting Structural Steel.
New steel extensions, shim plates and connection bolts are included with Furnishing and Erecting Structural Steel, Prior to ordering any material, the Contractor shall verify in the field all bearing height and shim thickness dimensions.
Min. jack capacity \* 25 Tons.
Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in Ileu of ASTM F1554

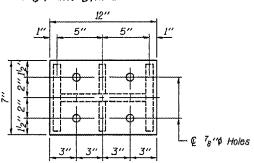
anchor bolts may be used in Ileu of ASTM F1554 Grade 36 (Fy=36ksI). The corresponding specified grade of AASHTO M314 anchor bolts may be used in Ileu of ASTM F1554.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications. Side retainers shall be included in the cost of Elastomeric Bearing Assembly, Type I.

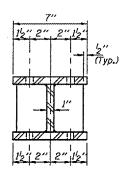
#### Jakin "Mar" FAP JU2 13 WHITE

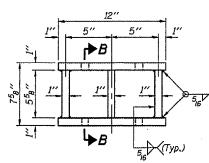
Contract # 78015

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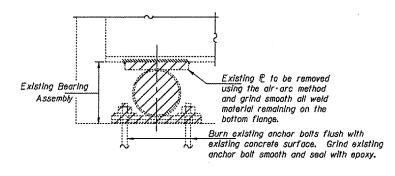
#### PLAN TOP AND BOTTOM PLATE





SECTION B-B

STEEL EXTENSION DETAIL



#### EXISTING BEARING REMOVAL DETAIL

Cost Included with Jack and Remove Existing Bearings.

#### BILL OF MATERIAL

<i>Item</i>	Unit	Total	
Elastomeric Bearing Assembly Type I	Each	16	
Jack and Remove Existing Bearings	Each	16	
Furnishing and Erecting Structural Steel	Pound	1530	
Anchor Balts	Each	32	

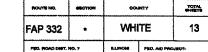
BRIDGE REPAIRS FAP 332 OVER FRENCH CREEK WHITE COUNTY SN 097-0005

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DESIGNED

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Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.



SHEET NO.

12

\* D9 CM BRIDGE REPAIR FY 08-1 CONTRACT NO. 78015

#### NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity (Tension in kips) = 1.25 x fy x A<sub>t</sub> (1)

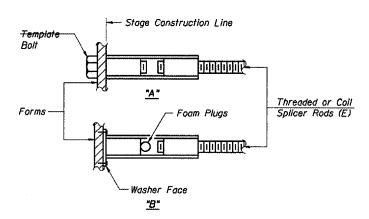
Minimum \*Pull-out Strength = 0.66 x fy x A, (Tension in kips)

Where fy = Yield strength of lapped reinforcement bars in ksi.

A<sub>t</sub> = Tensile stress area of lapped reinforcement bars.

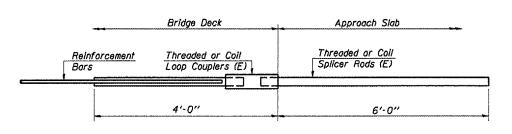
\* = 28 day concrete

BAR SPLICER ASSEMBLIES Strength Requirements Bar Size to Splicer Rod or Min. Capacity Min. Pull-Out Strength be Spliced | Dowel Bar Length | kips - tension kips - tension 1'-8" 14.7 7.9 #5 2'-0" 23.0 12.3 2'-7" 33.1 17.4 #6 23.8 #7 3'-5" 45.1 #8 4'-6" 58.9 31.3 75.0 #9 5'-9" 39.6 #10 7'-3" 95.0 50.3 61.8 #11 9'-0" 117.4



#### INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.



1111111

ROLLED THREAD DOWEL BAR

ningingingingini

"" ONE PIECE

WELDED SECTIONS

BAR SPLICER ASSEMBLY ALTERNATIVES

\*\* Heavy Hex Nuts conforming to ASTM

A 563, Grade C, D or DH may be used.

-Wire Connector

*पंजापा* 

The diameter of this part

of the bar spliced.

is the same as the diameter

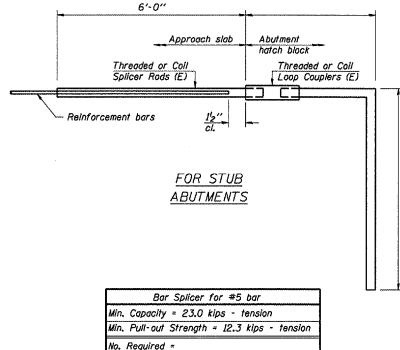
The diameter of this part is

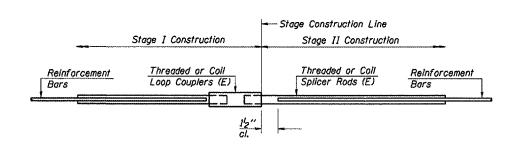
equal or larger than the

diameter of bar spliced.

#### FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

	Bar	Splicer	for	#5	bar		
	Capacity						
Min.	Pull-out	Strength	=	12.3	kips	•	tension
No.	Required	=					

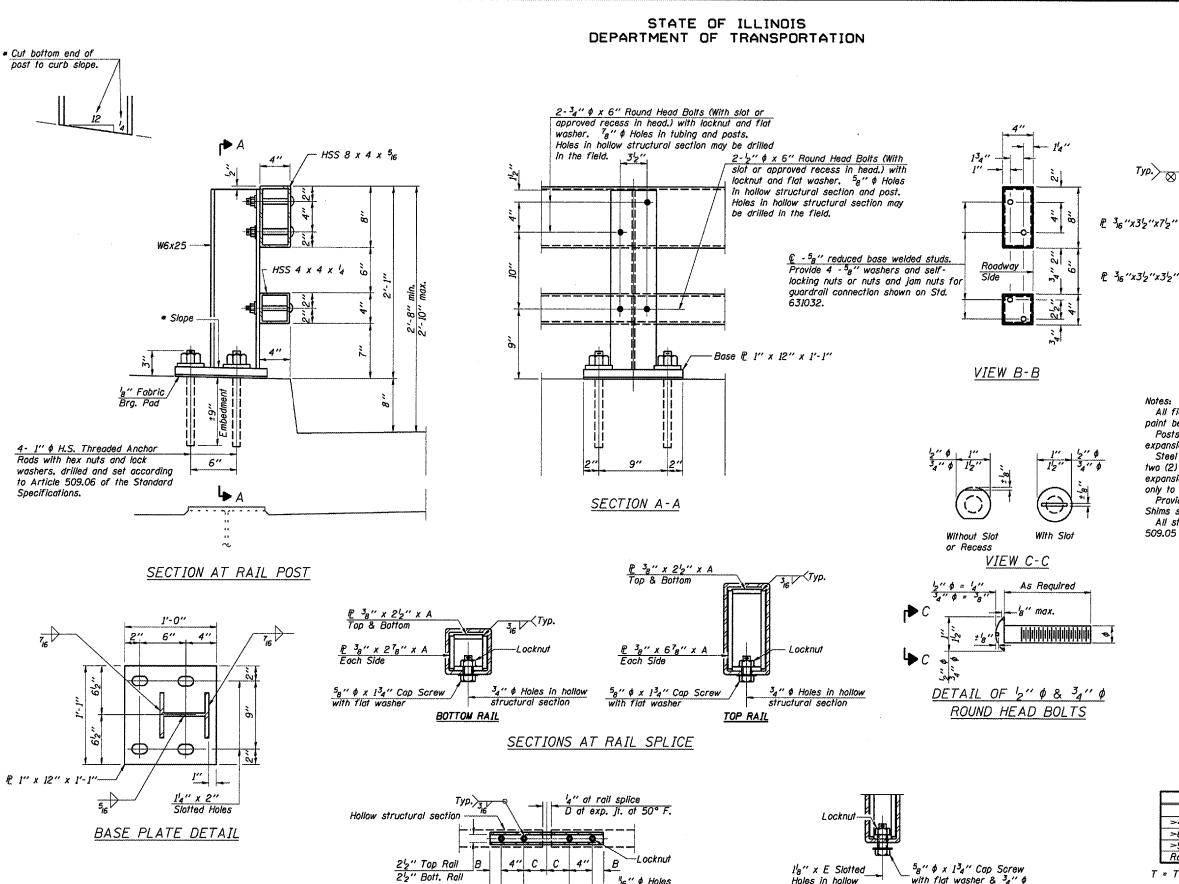




#### STANDARD

Bar Size	No. Assemblies Required	Location		
#5	22	Stage Constr. Line		
		<u> </u>		

BAR SPLICER ASSEMBLY DETAILS



PLAN-BOTT, SPLICE P

TYPICAL

Holes in hollow

structural section

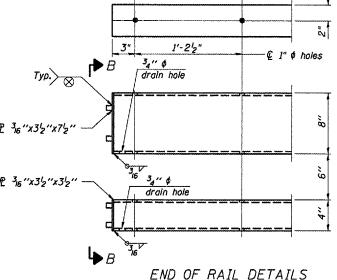
RAIL SPLICE CONNECTION

AT EXPANSION JT.

XS pipe spacer, '2" long.

TOTAL WHITE 13 FAP 332 13 KLINORS PROJECT

\* D9 CM BRIDGE REPAIR FY 08-1 **CONTRACT NO. 78015** 



All field drilled holes shall be coated with an approved zinc rich paint before erection.

Posts shall not be located closer than 1'-3" to an existing bridge expansion joint or end of bridge.

Steel Bridge Rail expansion joint shall be provided between any two (2) posts which span a bridge expansion joint. Bolts located at expansion joint shall be provided with locknuts and shall be tightened only to a point that will allow railing movement.

Provide one g" and two 6" steel shims for 25% of the posts. Shims shall be similar to base plates in size and holes.

All steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.

BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type 2399	Foot	156'-0'

#### SPLICE DIMENSIONS

T	D	Α	В	Ç	E
≤4"	25"	1'-8"	2"	4"	25"
>4"≤6 <sup>1</sup> 2"	334"	2'-0"	2'2"	52"	312"
>6½″≤9″	5"	2'-4"	32"	62"	9"
>9"≤13"	7"	2'-10"	42"	8'2"	11''
Rall Splice	4"	1'-8''	2"	4"	

T = Total movement at expansion joint as shown on the design plans.

STEEL RAILING, TYPE 2399

(6'-3" Maximum Post Spacing)

See post spacing Sheet 10 of 13.